

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 1559185 - O'Quinn Residence

MiTek USA, Inc.

Site Information:

6904 Parke East Blvd. Tampa, FL 33610-4115

Customer Info: Doug Edgley Construction Project Name: 1559185 Model: Custom

Subdivision: High Pointe Farms

Lot/Block: 5 Address:

City: Columbia County

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: unknown at time of seals

License #: unknown at time of seals

Address: unknown at time of seals City: unknown at time of seals

State: unknown at time of seals General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014

Design Program: MiTek 20/20 8.2

Wind Code: ASCE 7-10

Wind Speed: 120 mph

Roof Load: 40.0 psf

Floor Load: N/A psf

This package includes 41 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No. Seal# Truss Name Date No. Seal# Truss Name Date 1 T17227284 CJ1 6/3/19 23 T17227306 T13 6/3/19 2 T17227285 CJ3 6/3/19 24 T17227307 T14 6/3/19 3 T17227286 CJ5 6/3/19 25 T17227308 T15 6/3/19 4 T17227287 EJ7 6/3/19 26 T17227309 T15A 6/3/19 5 T17227288 EJ7A 6/3/19 27 T17227310 T16A 6/3/19 6 T17227289 HJ9 6/3/19 28 T17227311 T17A 6/3/19 7 T17227291 PB1G 6/3/19 29 T17227312 T17G 6/3/19 8 T17227291 PB1G 6/3/19 30 T17227313 T18 6/3/19 9 T17227293 T02 6/3/19 31 T17227314 T18G 6/3/19	No. Seal# Truss Name Date No Seal# Truss Na	me Date
22 11/22/305 112 6/3/19	1 T17227284 CJ1 6/3/19 23 T17227306 T13 2 T17227285 CJ3 6/3/19 24 T17227307 T14 4 T17227287 EJ7 6/3/19 26 T17227308 T15 5 T17227288 EJ7A 6/3/19 26 T17227309 T15A 6 T17227289 HJ9 6/3/19 28 T17227311 T17A 7 T17227290 PB1 6/3/19 29 T17227312 T17G 8 T17227291 PB1G 6/3/19 30 T17227312 T17G 9 T17227292 T01 6/3/19 30 T17227313 T18 10 T17227293 T02 6/3/19 31 T17227314 T18G 11 T17227294 T03 6/3/19 32 T17227315 V1A 12 T17227295 T04 6/3/19 33 T17227316 V2 13 T17227296 T05 6/3/19 34 T17227317 V3 14 T17227297 T06 6/3/19 35 T17227318 V4 17 T17227298 T07 6/3/19 36 T17227319 V5 16 T17227298 T07 6/3/19 37 T17227320 V6 17 T17227300 T09 6/3/19 38 T17227321 V7 18 T17227301 T10 6/3/19 38 T17227322 V8 18 T17227301 T10 6/3/19 39 T17227323 V9 19 T17227302 T10G 6/3/19 40 T17227323 V9 20 T17227303 T11 6/3/19 21 T17227304 T11G 6/3/19	6/3/19 6/3/19 6/3/19 6/3/19 6/3/19 6/3/19 6/3/19 6/3/19 6/3/19 6/3/19 6/3/19 6/3/19 6/3/19



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Albani, Thomas My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

June 3,2019



Job Truss Truss Type O'Quinn Residence Qty 1559185 CJ1 T17227284 Jack-Open Buiders FirstSource, Job Reference (optional) Lake City, FL 8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 41 2019 Page 1 ID:BVHjtU?4L8iDBBHQFc123Azlu1B-EJ?o2IJp2eQLaSsN7i_4uirX6N?iz5pSiyUeCxzA_i 1-0-0 Scale = 195 0-10-3 6.00 12 0-4-3

			1-0-0	
LOADING (pst) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.28 BC 0.01 WB 0.00 Matrix-P	DEFL. in (loc) l/defl L/d Vert(LL) -0.00 2 >999 240 Vert(CT) -0.00 2 >999 180 Horz(CT) -0.00 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 7 lb FT = 20%
				Weight: 7 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=-105/Mechanical, 2=294/0-5-8, 4=9/Mechanical

Max Horz 2=37(LC 12)

Max Uplift 3=-105(LC 1), 2=-76(LC 12)

Max Grav 3=42(LC 8), 2=294(LC 1), 4=19(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb)



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



O'Quinn Residence Qty Ply Joh Truss Truss Type T17227285 1559185 CJ3 Jack-Open Job Reference (optional) 8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun. 3 13 30 42 2019 Page 1 ID BVHjtU?4L8iDBBHQFc123Azlu1B-iWZAGeKRpyYCBcRagPVJRvNh4nKniY3bzcDBkOzA_h Buiders FirstSource, Lake City, FL -2-0-0 2-0-0 Scale = 1:14.6 0-4-11 6.00 12 0.4-3 3-0-0 GRIP DEFL. I/defi 1./d PLATES LOADING (psf) SPACING-2-0-0 244/190 TCLL 20.0 Plate Grip DOL 1.25 TC 0.33 Vert(LL) -0.00 2-4 >999 240 MT20 TCDL 10,0 Lumber DOL 1.25 BC 0.08 Vert(CT) -0.01 2-4 >999 180 -0.00 **BCLL** 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) 3 n/a n/a Code FBC2017/TPI2014 Weight: 13 lb FT = 20%Matrix-P BCDL 100

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=26/Mechanical, 2=297/0-5-8, 4=27/Mechanical

Max Horz 2=65(LC 12)

Max Uplift 3=-23(LC 12), 2=-57(LC 12)

Max Grav 3=28(LC 19), 2=297(LC 1), 4=54(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019



Job Truss Truss Type Qty O'Quinn Residence T17227286 1559185 CJ5 Jack-Open Job Reference (optional) Buiders FirstSource, Lake City, FL 8,240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13,30 43 2019 Page 1 ID:BVHjtU?4L8iDBBHQFc123Azlu1B-Ai7YT_L3aGg3pm0mE70Y_7wsqBd1R?IICGzIGqzA__g 5-0-0 5-0-0 Scale = 1:19.5 6 00 12 0-4-3

DEFL.

Vert(LL)

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

(loc)

2-4

2-4

-0.03

-0.06

-0.00

l/defl

>999

>958

L/d

240

180

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 5-0-0 oc purlins.

PLATES

Weight: 19 lb

MT20

GRIP

244/190

FT = 20%

LUMBER-

TCLL

TCDL

BCLL

BCDL

LOADING (psf)

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

20,0

10,0

10.0

0.0

REACTIONS. (lb/size) 3=110/Mechanical, 2=354/0-5-8, 4=47/Mechanical

Max Horz 2=94(LC 12)

Max Uplift 3=-58(LC 12), 2=-57(LC 12)

SPACING-

Plate Grip DOL

Rep Stress Incr

Code FBC2017/TPI2014

Lumber DOL

Max Grav 3=110(LC 1), 2=354(LC 1), 4=94(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

CSI.

TC

вс

WB

Matrix-P

0.33

0.27

0.00

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

2-0-0

1.25

1.25

YES

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi,
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2

No 39380

No 39380

No 39380

Thomas A. Albani PE No.39380

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019

WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Compone Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



6904 Parke East Blv

Job Truss Truss Type Qty O'Quinn Residence T17227287 1559185 EJ7 Jack-Open 10 Job Reference (optional) Buiders FirstSource Lake City, FL 8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 44 2019 Page 1 ID BVHjtU?4L8iDBBHQFc123Azlu1B-eugwhKMhLZowRwbyoqXnWKTy0buLASYuRwilpGzA 7-0-0 7-0-0 Scale 1/2"=1 6 00 12 0-4-3 2x4 = 7-0-0 LOADING (psf) SPACING-2-0-0 CSI. DEFL. **PLATES** in (loc) l/defl L/d GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.68 Vert(LL) -0.12 >663 240 MT20 244/190 TCDL 10.0 BC WB Lumber DOL 1.25 0.59 Vert(CT) -0.24 >331 180 BCLL 0.0 Rep Stress Incr YES 0.00 Horz(CT) -0.00 3 n/a BCDL Code FBC2017/TPI2014 Matrix-P Weight: 26 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

(lb/size) 3=179/Mechanical, 2=424/0-5-8, 4=67/Mechanical

Max Horz 2=123(LC 12)

Max Uplift 3=-88(LC 12), 2=-61(LC 12)

Max Grav 3=179(LC 1), 2=424(LC 1), 4=134(LC 3)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2,

No 39380

No 39380

No 39380

Thomas A. Albani PE No.39380

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019



Job Truss Truss Type Qty O'Quinn Residence T17227288 1559185 EJ7A Jack-Open Job Reference (optional) 8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13:30:44 2019 Page 1 Buiders FirstSource Lake City, FL $ID. BVHjtU?4L8iDBBHQFc123AzIu1B-eugwhKMhLZowRwbyoqXnWKTxTbyLASYuRwilpGz \cite{Alicenter} \cite{Alicenter}$ Scale 1/2"=1 6.00 12 3-10-3 0-4-3 2x4 = 7-0-0 6-4-8

LOADING (psf) SPACING-2-0-0 CSI DEFL. L/d **PLATES** GRIP TCLL 20.0 1.25 Plate Grip DOL TC BC 0.72 Vert(LL) 0.08 2-5 >961 240 MT20 244/190 TCDL 10.0 Lumber DOL 1.25 0.33 Vert(CT) -0.08 2-5 >938 180 BCLL 0.0 Rep Stress Incr WB YES 0.00 Horz(CT) -0.00 3 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-P Weight: 26 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

BRACING-TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. All bearings Mechanical except (jt=length) 2=0-3-8, 5=0-5-8.

(lb) - Max Horz 2=123(LC 12)

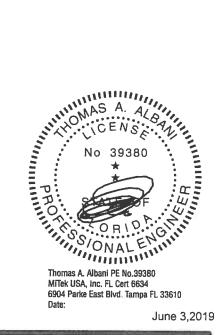
Max Uplift All uplift 100 lb or less at joint(s) 3, 2 except 4=-220(LC 3), 5=-129(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 3, 4 except 2=400(LC 1), 5=397(LC 3)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2 except (jt=lb) 4=220, 5=129.



June 3,2019

⚠ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters and report on individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent occlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Comp. Safety Information available from Truss Plate Institute, 216 N. Lee Street, Suite 312, Alexandria, VA 22314



Job Truss Truss Type Qty O'Quinn Residence T17227289 1559185 НЈ9 Diagonal Hip Girder Job Reference (optional) Lake City, FL Buiders FirstSource 8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 46 2019 Page 1 ID BVHjtU?4L8iDBBHQFc123Azlu1B-aHoh6?NxtB2egDlLvFaFblYFyOXueExBuEBPt9zA d 9-9-5 5-8-11 4-0-9 4-0-9 Scale 1/2"=1" 0-4-4 4.24 12 3x4 = 0-4-3 М 10 11 3x4 == 2x4 || 4-0-9 9-9-5 Plate Offsets (X,Y)-[2:0-4-1,0-2-2] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) l/defi L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 -0.07 6-7 TC 0.85 Vert(LL) >999 240 244/190 MT20 TCDL 10.0 Lumber DOL 1.25 BC 0.71 Vert(CT) -0.15 6-7 >742 180 BCLL 0.0 Rep Stress Incr NO WB 0.46 Horz(CT) 0.01 5 BCDL 10.0 Code FBC2017/TPI2014 Matrix-S Weight: 44 lb FT = 20% BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2

BOT CHORD 2x4 SP No 3

4=173/Mechanical, 2=509/0-7-6, 5=219/Mechanical REACTIONS. (lb/size)

Max Horz 2=141(LC 4)

Max Uplift 4=-85(LC 4), 2=-186(LC 4), 5=-30(LC 8) Max Grav 4=173(LC 1), 2=548(LC 35), 5=260(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-834/111

BOT CHORD 2-7=-181/699, 6-7=-181/699 3-7=0/277, 3-6=-724/188

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4,2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5 except (jt=lb)
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 80 lb down and 190 lb up at 1-6-1, 80 lb down and 190 lb up at 1-6-1, 71 lb down and 19 lb up at 4-4-0, 71 lb down and 19 lb up at 4-4-0, and 88 lb down and 64 lb up at 7-1-15, and 88 lb down and 64 lb up at 7-1-15 on top chord, and 1 lb down at 1-6-1, 1 lb down at 1-6-1, 19 lb down at 4-4-0, 19 lb down at 4-4-0, and 39 lb down at 7-1-15, and 39 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

LOAD CASE(S) Standard

Dead + Roof Live (balanced): Lumber Increase=1,25, Plate Increase=1,25

Uniform Loads (plf)

Vert: 1-4=-60, 2-5=-20 Concentrated Loads (lb)

Vert: 8=127(F=63, B=63) 9=-49(F=-25, B=-25) 11=-38(F=-19, B=-19)



Structural wood sheathing directly applied or 5-2-11 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019

⚠ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MTelk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design in the prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



Job Truss Truss Type Qty O'Quinn Residence T17227290 1559185 PB1 26 Piggyback 1 Job Reference (optional) Buiders FirstSource Lake City, FL 8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun. 3 13 30 46 2019 Page ID:BVHjtU?4L8iDBBHQFc123Azlu1B-aHoh6?NxtB2egDlLvFaFblYQ3OhceLYBuEBPt9zA__c 8-0-0 4-0-0 Scale = 1 15.4 4×4 = 6.00 12 D-4-B 2x4 = 2x4 | 2x4 = LOADING (psf) SPACING-2-0-0 CSI DEFL. (loc) l/defl L/d **PLATES** GRIP 20.0 TCLL Plate Grip DOL 1.25 TC BC 0.13 Vert(LL) 0.00 5 n/r 120 MT20 244/190 TCDL 10.0 Lumber DOL 1.25 0.09 Vert(CT) 0.01 120 5 n/r BCLL 0.0 Rep Stress Incr YES WB 0.03 Horz(CT) 0.00

BRACING-

TOP CHORD

BOT CHORD

n/a

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 6-0-0 oc purlins

Weight: 24 lb

FT = 20%

LUMBER-

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

10.0

OTHERS 2x4 SP No.3

REACTIONS. (lb/size) 2=166/5-11-10, 4=166/5-11-10, 6=227/5-11-10

Code FBC2017/TPI2014

Max Horz 2=-21(LC 10)

Max Uplift 2=-38(LC 12), 4=-41(LC 13), 6=-11(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl.. GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20 Opsf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6
- 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building



MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019



Joh Truss Truss Type Qty O'Quinn Residence T17227291 1559185 PB1G iggyback 2 Job Reference (optional) Buiders FirstSource Lake City, FL 8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 47 2019 Page 1 ID BVHjtU?4L8iDBBHQFc123Azlu1B-2TM3JLOaeUAVINJXTy5U8y5dJo2pNo6K7txyQbzA 8-0-0 4-0-0 Scale = 1 12 9 4x4 = 6.00 12 0-4-8 2x4 = 8-0-0 LOADING SPACING-(psf) 2-0-0 CSI DEFL in (loc) l/defl L/d **PLATES** GRIP TCLL 20,0 Plate Grip DOL 1.25 тс 0.04 0.00 Vert(LL) n/r 120 MT20 244/190 TCDL 10,0 Lumber DOL 1.25 ВC 0.03 Vert(CT) 0.00 5 n/r 120 BCLL 0.0 Rep Stress Incr YES WB 0.01 Horz(CT) 0.00 4 BCDL Code FBC2017/TPI2014 10.0 Matrix-P Weight 39 lb FT = 20% LUMBER-BRACING-

TOP CHORD

BOT CHORD

REACTIONS. (lb/size) 2=140/4-7-14, 4=140/4-7-14, 6=173/4-7-14

Max Horz 2=-27(LC 17)

2x4 SP No.2

2x4 SP No.2

2x4 SP No.3

Max Uplift 2=-57(LC 12), 4=-62(LC 13), 6=-24(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

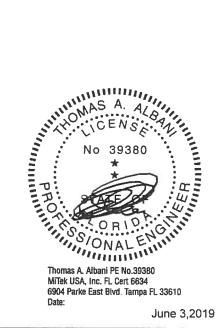
NOTES-

TOP CHORD

BOT CHORD

OTHERS

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6,
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

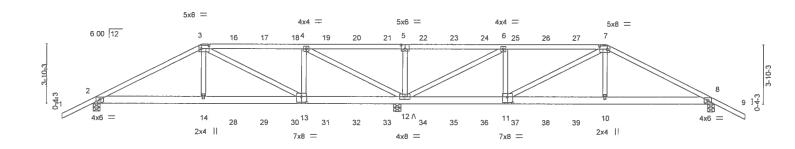
June 3,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type		Qty	Ply	O'Quinn Residence		
		.5: 50.						T17227292
1559185	T01	Hip Girder		1	2			
					_	Job Reference (optional)		
Buiders FirstSource,	Lake City, FL			8	240 s May	y 13 2019 MiTek Industries, In	ic. Mon Jun 3 13 30 5	52 2019 Page 1
				ID:BVHjtU?4L8iDE	BHQFc12	3Azlu1B-PR9yM3SiS1onO80	VGWhfr0oJmph62vC	4G9ej5ozA_X
₁ -2-0-0	7-0-0	13-8-2	20-2-8	26-8-14	,	33-5-0	40-5-0	42-5-0
2-0-0	7-0-0	6-8-2	6-6-6	6-6-6	,	6-8-2	7-0-0	2-0-0

Scale = 1.72 4



	<u> </u>	7-0-0 7-0-0	13-8-2 6-8-2		19-6-0 5-9-14		8-14 6-6		-5-0 -8-2	40-5-0 7-0-0	——
Plate Offse	ets (X,Y)-	[3:0-6-0,0-2-8], [5:0-3-0,0							-0-2	17-0-0	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.70	Vert(LL)	0.26 11-12	>932	240	MT20	244/190
FCDL	10.0	Lumber DOL	1.25	BC	0.28	Vert(CT)	0.26 12-13	>940	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.66	Horz(CT)	-0.04 8	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matrix	r-S	' '				Weight: 457 lb	FT = 20%

BRACING-

TOP CHORD BOT CHORD

LUMBER-

2x4 SP No.2 2x6 SP No.2

TOP CHORD BOT CHORD 2x4 SP No 3 WEBS

REACTIONS. (lb/size) 2=1088/0-5-8, 12=3251/0-5-8, 8=1311/0-5-8

Max Horz 2=52(LC 7)

Max Uplift 2=-895(LC 8), 12=REL, 8=-837(LC 9)

Max Grav 2=1092(LC 19), 12=3274(LC 43), 8=1314(LC 20)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown TOP CHORD 2-3=-1708/1884, 3-4=-960/2791, 4-5=0/3013, 5-6=0/3013, 6-7=-1398/2585,

7-8=-2126/1756

2-14=-1646/1504, 13-14=-1645/1525, 12-13=-2734/1021, 11-12=-2506/1398, BOT CHORD

10-11=-1499/1827, 8-10=-1501/1808

3-14=0/642, 3-13=1271/0, 4-13=-44/523, 4-12=-2368/0, 5-12=-868/393, 6-12=-2873/0, WEBS

6-11=0/776, 7-11=-1168/0, 7-10=0/695

NOTES-

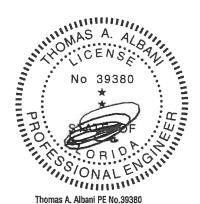
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
 4) Wind: ASCE 7-10, Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18, MWFRS (envelope), Lumber DOL=1.60 plate grip DOL=1.60
- 5) Provide adequate drainage to prevent water ponding.6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=895, 8=837
- 10) "A" indicates Released bearing: allow for upward movement at joint(s) 12



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 12-13.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019

Continued on page 2

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Ptate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	
1559185	T01	Hip Girder	1			T17227292
Builder EiretCourse	Lako City El				Job Reference (optional)	

ay 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 52 2019 Page ID BVHjtU?4L8iDBBHQFc123Azlu1B-PR9yM3SiS1onO8CVGWhfr0oJmph62vC4G9ej5ozA

NOTES-

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 254 lb down and 204 lb up at 7-0-0, 119 lb down and 102 lb up at 9-0-12, 123 lb down and 103 lb up at 11-0-12, 123 lb down and 103 lb up at 11-0-12, 123 lb down and 103 lb up at 15-0-12, 123 lb lb down and 103 lb up at 19-0-12, 119 lb down and 102 lb up at 20-2-8, 119 lb down and 102 lb up at 21-4-4, 119 lb down and 102 lb up at 23-4-4, 119 lb down an Ib up at 25-4-4, 119 lb down and 102 lb up at 27-4-4, 119 lb down and 102 lb up at 29-4-4, and 119 lb down and 102 lb up at 31-4-4, and 254 lb down and 204 lb up at 33-5-0 on top chord, and 298 lb down and 58 lb up at 7-0-0, 94 lb down at 9-0-12, 76 lb down and 130 lb up at 11-0-12, 76 lb down and 130 lb up at 15-0-12, 76 lb down and 130 lb up at 15-0-12, 76 lb down and 130 lb up at 15-0-12, 76 lb down at 21-4-4, 94 lb down a 23-4-4, 94 lb down at 25-4-4, 94 lb down at 27-4-4, 94 lb down at 29-4-4, and 94 lb down at 31-4-4, and 298 lb down and 58 lb up at 33-4-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3-60, 3-7-60, 7-9-60, 2-8-20

Concentrated Loads (lb)

Vert: 3=-207(F) 7=-207(F) 14=-238(F) 12=-47(F) 5=-119(F) 10=-238(F) 16=-119(F) 17=-123(F) 18=-123(F) 19=-123(F) 20=-123(F) 21=-123(F) 22=-119(F) 23=-119(F) 24=-119(F) 25=-119(F) 26=-119(F) 27=-119(F) 28=-47(F) 29=130(F) 30=130(F) 31=130(F) 32=130(F) 33=130(F) 34=-47(F) 35=-47(F) 36=-47(F) 37=-47(F) 38=-47(F) 39=-47(F)

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227293
1559185	T02	Hip	1	1	Job Reference (optional)	
Buiders FirstSource L. 2-0-0 4-9-4 2-0-0 4-9-4	9-0-0 4-2-12	14-8-2 20-2-8 5-8-2 5-6-6	8 ID BVHjtU?4L8 25-8-14 5-6-6	240 s May DBBHQFo	13 2019 MiTek Industries, Inc. Mon Jun. 3 13 30 54 20: 123Azlu1B-LpHinkTz_e2VeSMtNwj7wRtiTdDcWulMkT7 31-5-0 35-7-12 40-5-0 5-8-2 4-2-12 4-9-4	19 Page 1 q9hzAV 42-5-0 2-0-0

Scale = 1.72.4

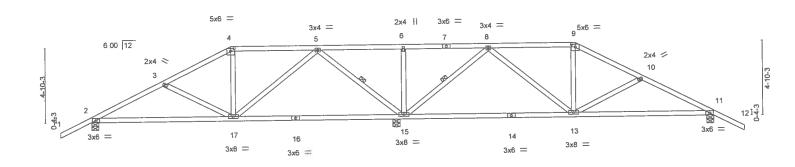


Plate Offsets (X,Y)— [4:	9-0-0 9-0-0 0-3-0,0-2-0], [9:0-3-0,0-	2-01	19-6-0 10-6-0	20-2-8 0-8-8	31-5-0 11- <u>2-8</u>			40-5-0 9-0-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TF	2-0-0 1.25 1.25 YES	CSI. TC 0.49 BC 0.91 WB 0.34 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.20 15-17 -0.40 15-17 0.03 11	l/defl >999 >594 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 205 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

BRACING-

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 5-10-1 oc purlins. Rigid ceiling directly applied or 2-7-8 oc bracing.

5-15, 8-15 1 Row at midpt

(lb/size) 2=752/0-5-8, 15=1960/0-5-8, 11=752/0-5-8 REACTIONS.

Max Horz 2=-64(LC 10) Max Uplift 2=-127(LC 12), 15=-272(LC 9), 11=-135(LC 13) Max Grav 2=772(LC 23), 15=1960(LC 1), 11=772(LC 24)

FORCES.

(lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown. RD 2-3=-977/400, 3-4=-699/285, 4-5=-570/294, 5-6=-161/743, 6-8=-161/743, 8-9=-570/285, TOP CHORD

9-10=-699/275, 10-11=-977/390

2-17=-218/806, 11-13=-241/806 **BOT CHORD**

3-17=-268/226, 5-17=-123/543, 5-15=-1107/442, 6-15=-323/183, 8-15=-1107/449, WEBS

8-13=-130/543, 10-13=-268/226

NOTES-

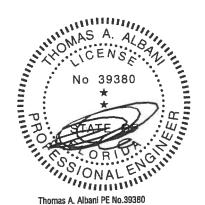
- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber and C-C Exterior(2) zone; C-C for members and forces and continuous con DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=127, 15=272, 11=135.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

June 3,201

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev., 10/03/2015 BEFORE USE, Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the flabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty Ply	O'Quinn Residence
1559185	Т03	Hip	1 1	T17227294
Buiders FirstSource. L	Lake City, FL			Job Reference (optional)
builders i listoudice.	Lake City, PL	ID:RV	8 240 s May	r 13 2019 MiTek Industries, Inc. Mon Jun. 3 13 30:55 2019 Page 1 123Azlu1B-p0r4_4UblyAMFcx4xeEMTeQv21d0FBiWz7tNi7zA U
_2-0-0 5-9-4	4 11-0-0	15-8-2 , 20-2-8 , 24-8	-14 , 29-5	
2-0-0 5-9-4	5-2-12	4-8-2 4-6-6 4-6		40 0.0

Scale = 1.72.4

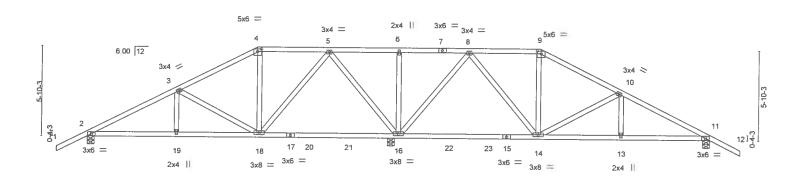


Plate Offsets (X,Y)-	5-9-4 11-0-0 5-9-4 5-2-12 [4:0-3-0,0-2-0], [9:0-3-0,0-2	-0]	19-6-0 8-6-0	20-2-8 0-8-8	29-5-0 9-2-8		34-7-12 5-2-12	40-5-0 5-9-4	= = = = = = = = = = = = = = = = = = = =
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TPI2	2-0-0 1.25 1.25 YES 2014	CSI. TC 0.37 BC 0.65 WB 1.00 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.11 16-18 -0.23 16-18 0.02 11	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 220 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS

2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-9-6 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (lb/size) 2=719/0-5-8, 16=2026/0-5-8, 11=719/0-5-8

Max Horz 2=-75(LC 10)

Max Uplift 2=-131(LC 12), 16=-238(LC 9), 11=-142(LC 13) Max Grav 2=752(LC 23), 16=2026(LC 1), 11=752(LC 24)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-941/356, 3-4=-479/236, 4-5=-356/254, 5-6=-130/748, 6-8=-130/748, 8-9=-356/247,

9-10=-479/227, 10-11=-941/344

BOT CHORD

2-19=-170/766, 18-19=-170/766, 13-14=-191/766, 11-13=-191/766 3-18=-467/271, 5-18=-187/636, 5-16=-1008/394, 6-16=-268/152, 8-16=-1008/400, WEBS

8-14=-192/636, 10-14=-467/271

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=131, 16=238, 11=142.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

June 3,2019

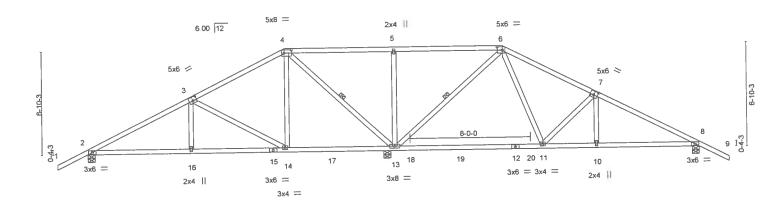
🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev., 10:03/2015 BEFORE USE, Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITEM Quality Criteria, DSB-89 and BCSI Building Component Safety Information. Available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



			Oty Ply	O'Quinn Residence	
Job	Truss	Truss Type	Qty Ply	O Quint resolution	T17227295
1559185	T04	Hip	1 1	Job Reference (optional)	
Buiders FirstSource,	Lake City, FL		8.240 s Ma ID:BVHitU?4L8iDBBHQFc12	av 13 2019 MiTek Industries.	inc Mon Jun 3 13 30 56 2019 Page 1 tmVGVLib?syxYQw8_k7fBncxEazAT
-2-0-0	6-9-9	13-0-0 20-2-8 6-2-7 7-2-8	27-5-0 7-2-8	33-7-7 6-2-7	40-5-0 6-9-9 42-5-0 2-0-0

Scale = 1:73.5



<u> </u>	6-9-9 6-9-9	13-0-0 6-2-7	19-6-0 6-6-0	20-2-8 0-8-8	30-0-8 9-10-0		33-7-7 3-6-15	40-5-0	
Plate Offsets (X,Y)- LOADING (psf) TCLL 20.0	SPACING- Plate Grip DOL	2-0-0 1.25 1.25	CSI. TC 0.90 BC 0.85	DEFL. Vert(LL) Vert(CT)	in (loc) -0.38 11-13 -0.55 11-13	l/defl >628 >435	L/d 240 180	PLATES MT20	GRIP 244/190
TCDL 10,0 BCLL 0,0 * BCDL 10,0	Lumber DOL Rep Stress Incr Code FBC2017/1	NO	WB 0.60 Matrix-S	Horz(CT)	0.03 8	n/a	n/a	Weight: 215 lb	FT = 20%

LUMBER-

2x4 SP No.2 TOP CHORD BOT CHORD 2x4 SP No.1 **WEBS** 2x4 SP No.3 BRACING-TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 5-6-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing. 4-13, 6-13 1 Row at midpt

(lb/size) 2=688/0-5-8, 13=2328/0-5-8, 8=768/0-5-8 REACTIONS.

Max Horz 2=-87(LC 10)

Max Uplift 2=131(LC 12), 13=201(LC 9), 8=-143(LC 13) Max Grav 2=754(LC 23), 13=2328(LC 1), 8=768(LC 1)

(lb) - Max. Comp./Max, Ten, - All forces 250 (lb) or less except when shown FORCES.

2-3=-908/316, 3-4=-323/173, 4-5=-85/662, 5-6=-85/662, 6-7=-615/246, 7-8=-911/309 TOP CHORD

BOT CHORD

2-16=122/727, 14-16=-123/724, 10-11=-149/732, 8-10=-148/731 3-16=0/287, 3-14=-608/327, 4-14=-121/452, 4-13=-1000/378, 5-13=-504/276, WEBS

6-13=-1163/393, 7-11=-462/303, 6-11=-176/723

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat, II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 state and DOL=1.60 DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=131, 13=201, 8=143.
- 8) Load case(s) 1 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

LOAD CASE(S) Standard Except:

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert; 1-4=-60, 4-6=-60, 6-9=-60, 2-18=-20, 18-20=-60(F=-40), 8-20=-20

No 39380

No 39380

No 39380

Thomas A. Albani PE No.39380

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

June 3,201

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev., 10/03/2015 BEFORE USE, Design valid for use only with MiTek® connectors. This design is based only upon parameters and properly incorporate this design into the operations a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TPH Quality Criteria, DSB-89 and BCSI Building Composition available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type		Qty	Ply	O'Quinn Residence
1559185	T05	Hip		1	1	T17227296
Puidoss FirstCourses	Later City El					Job Reference (optional)
Buiders FirstSource,	Lake City, FL				8,240 s Ma	lay 13 2019 MiTek Industries, Inc. Mon Jun. 3 13 30 58 2019. Page 1
-2-0-0	6-9-9	45.00		ID BVHjtU	?4L8iDBBHC	IQFc123Azlu1B-EbXDd6XT2tZx63ffcmo35H2llEczSgCyf552lSzA_R
2-0-0	6-9-9	15-0-0 8-2-7	20-2-8 5-2-8	25-5-0 5-2-8		32-7-12 40-5-0 42-5-0 7-2-12 7-9-4 2-0-0

Scale = 1.73.5

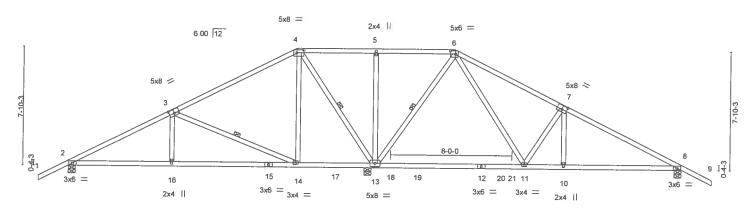


Plate Offsets (X,Y)-	6-9-9 6-9-9 [3:0-4-0,0-3-0], [4:0-5-8,0-	15-0-0 8-2-7 2-4], [6:0-4-0,0	4-	-6-0 20-2-8 6-0 0 ¹ -8-8	30-0-8 9-10-0		32-7-12 2-7-4	40-5-0 7-9-4	
TCLL 20,0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TF	2-0-0 1.25 1.25 NO Pl2014	CSI. TC 0.86 BC 0.82 WB 0.50 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.35 11-13 -0.53 11-13 0.02 8	l/defl >690 >454 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 223 lb	GRIP 244/190 FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.1 WEBS

2x4 SP No 3

(lb/size) 2=636/0-5-8, 13=2432/0-5-8, 8=716/0-5-8

Max Horz 2=98(LC 10)

Max Uplift 2=-128(LC 12), 13=-204(LC 12), 8=-144(LC 13) Max Grav 2=724(LC 23), 13=2432(LC 1), 8=724(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-879/301, 3-4=-120/297, 4-5=-88/738, 5-6=-88/738, 6-7=-529/260, 7-8=-795/264

BOT CHORD 2-16=132/710, 14-16=133/706, 13-14=171/266, 11-13=181/265, 10-11=98/619, 8-10=-98/621

WEBS 3-16=0/341, 3-14=-781/418, 4-14=-125/461, 4-13=-997/396, 5-13=-347/168,

6-13=1116/407, 6-11=286/913, 7-11=527/354

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

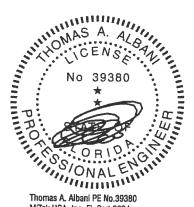
This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=128, 13=204, 8=144.
- 8) Load case(s) 1 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard Except:

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-60, 4-6=-60, 6-9=-60, 2-18=-20, 18-21=-60(F=-40), 8-21=-20



Structural wood sheathing directly applied or 5-3-6 oc purlins.

3-14, 4-13, 6-13

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019

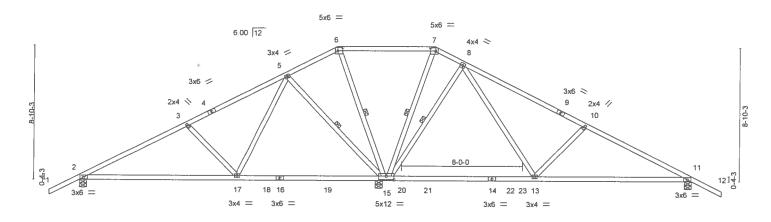
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ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information. available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria. VA 22314.



Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence		
1559185	T06	Hip	1	1			T17227297
	l				Job Reference (optional)		
Buiders FirstSource, L.	ake City, FL			8 240 s May	y 13 2019 MiTek Industries,	Inc. Mon Jun 3 13 30 59 20	019 Page 1
			ID:BVHjtU?4	L8iDBBHQF	c123Azlu1B-in4bq\$X5pAho	kDErAUJIdUaUCeyyB9W6t	IrbruzA Q
-2-0-0		3-7-13 17-0-0		2-10,	33-3-13	40-5-0	42-5-0
2-0-0	7-1-3	3-4-3	6-5-0 '1-	9-10	8-1-3	7-1-3	2-0-0

Scale = 1:73.5



<u> </u>	10-4-8 10-4-8		19-6-0 9-1-8	20-2-8 0-8-8	30-0-8 9-10-0		+	40-5-0 10-4-8	
Plate Offsets (X,Y)	[6:0-4-0,0-2-8], [7:0-4-0,0	0-2-8], [15:0-6-0	,0-2-12]						
COADING (psf) FCLL 20.0 FCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/1	2-0-0 1.25 1.25 NO PI2014	CSI. TC 0.81 BC 0.77 WB 0.37 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0,49 13-15 -0.57 13-15 0.01 15	l/defl >494 >422 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 224 lb	GRIP 244/190 FT = 20%

LUMBER-

WEBS

TOP CHORD 2x4 SP No.2

2x4 SP No.1

BOT CHORD

2x4 SP No.3

BRACING-

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 5-10-15 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing

1 Row at midpt 5-15, 6-15, 7-15, 8-15

REACTIONS.

(lb/size) 2=587/0-5-8, 15=2531/0-5-8, 11=667/0-5-8 Max Horz 2=-110(LC 10) Max Uplift 2=-119(LC 12), 15=-245(LC 12), 11=-136(LC 13) Max Grav 2=703(LC 23), 15=2531(LC 1), 11=698(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-739/246, 3-5=-435/177, 5-6=-92/704, 6-7=-119/811, 7-8=-76/687, 8-10=-427/172,

10-11=-744/245

BOT CHORD

2-17=-125/582, 13-15=-250/303, 11-13=-94/590 3-17=-412/325, 5-17=-191/610, 5-15=-641/359, 6-15=-617/205, 7-15=-501/105, WEBS

8-15=-795/424, 8-13=-232/851, 10-13=-446/360

NOTES-

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4,2psf; BCDL=3,0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=119, 15=245, 11=136.
- 8) Load case(s) 1 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard Except:

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-6=-60, 6-7=-60, 7-12=-60, 2-20=-20, 20-23=-60(F=-40), 11-23=-20



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610

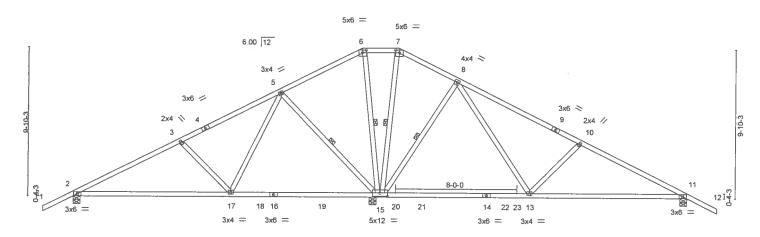
June 3,2019

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	Jop	Truss	Truss Type		Qty	Ply	O'Quinn Residence		
	1559185	Т07	Hip		1	1			T17227298
Į							Job Reference (optional)		
	Buiders FirstSource La	ike City, FL						Inc. Mon Jun 3 13 31 01	
				ID:BVHjtl	J?4L8iDBI	BHQFc123	BAZIu1B-eACMF8ZMLoxV	zXODIvLmivgqNReNf3 OI	3KivnzA O
			7-13 19-0	0-0 21-5-0	25-2-10		33-3-13	40-5-0	42-5-0
	2-0-0	7-1-3 6-6	-10 5-4	-3 2-5-0	3-9-10	i	8-1-3	7-1-3	2-0-0

Scale = 1:73.5



	10-4-8	19-6-0	20-2-8	30-0-8		40-5-0	
	10-4-8	9-1-8	0 <u>-</u> 8-8	9-10-0		10-4-8	
Plate Offsets (X,Y)-	[6:0-3-0,0-2-0], [7:0-3-0,0-2-0], [15:0-	6-0,0-2-12]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr NO Code FBC2017/TPI2014	CSI. TC 0.77 BC 0.78 WB 0.37 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) I/defl -0.49 13-15 >491 -0.57 13-15 >419 0.01 15 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 226 lb	GRIP 244/190 FT = 20%

LUMBER-

2x4 SP No.2 TOP CHORD 2x4 SP No.1

BOT CHORD 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 5-11-3 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt 5-15, 6-15, 7-15, 8-15

REACTIONS. (lb/size) 2=578/0-5-8, 15=2546/0-5-8, 11=660/0-5-8

Max Horz 2=121(LC 11)
Max Uplift 2=-115(LC 12), 15=-269(LC 12), 11=-135(LC 13)
Max Grav 2=697(LC 23), 15=2546(LC 1), 11=695(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=723/235, 3-5=422/170, 5-6=-109/790, 6-7=-60/722, 7-8=-85/756, 8-10=-420/166,

10-11=-737/239

BOT CHORD 2-17=-128/566, 13-15=-275/309, 11-13=-89/584

WEBS 3-17=-401/316, 5-17=-185/606, 5-15=-713/409, 6-15=-562/187, 7-15=-429/82,

8-15=-827/434, 8-13=-234/853, 10-13=-447/361

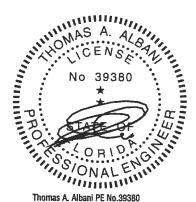
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=115, 15=269, 11=135.
- 8) Load case(s) 1 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard Except:

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (ptf)

Vert: 1-6=-60, 6-7=-60, 7-12=-60, 2-20=-20, 20-23=-60(F=-40), 11-23=-20



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

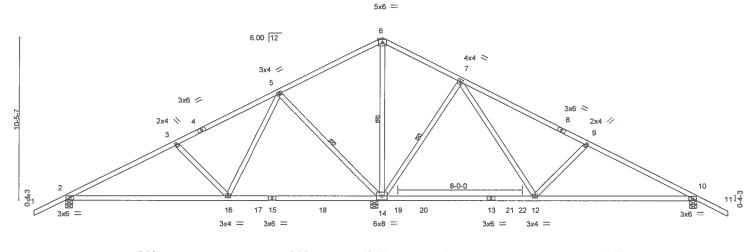
June 3,2019

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Job	Truss	Truss Type			Qty	Ply	O'Quinn Residence		
1559185	T08	Common			1	1			T17227299
L							Job Reference (optional))	
Buiders FirstSource,	Lake City, FL				8	.240 s May	/ 13 2019 MiTek Industries	s, Inc. Mon Jun 3 13 31 02	2019 Page 1
				ID:BVHjtU	1?4L8iDB	BHQFc123	BAzlu1B-6MmkSTa_653M	bhzQrcs?F7C0Er_fOUWYa	aj3FRDzAN
-2-0-0	7-1-3	13-7-13	20-2-8	2	-2-10		33-3-13	40-5-0	, 42-5-0
2-0-0	7-1-3	6-6-11	6-6-11	, ,	5-0-2	F	8-1-4	7-1-3	2-0-0

Scale = 1:71.2



	-	10-4-8 10-4-8	+	19-6-0 9-1-8	20-2-8 0-8-8	30-0-8 9-10-0	+	40-5-0 10-4-8	
LOADIN TCLL TCDL BCLL BCDL	G (psf) 20.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TI	2-0-0 1.25 1.25 NO PI2014	CSI. TC 0.76 BC 0.77 WB 0.48 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.49 12-14 >492 -0.57 12-14 >421 0.01 10 n/a	L/d 240 180 n/a		GRIP 244/190 FT = 20%

LUMBER-

2x4 SP No.2 2x4 SP No.1 TOP CHORD BOT CHORD WEBS 2x4 SP No.3

BRACING-

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 5-9-9 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing. 6-14, 5-14, 7-14 1 Row at midpt

REACTIONS.

(lb/size) 2=609/0-5-8, 14=2486/0-5-8, 10=689/0-5-8 Max Horz 2=128(LC 11) Max Uplift 2=-117(LC 12), 14=-277(LC 12), 10=-136(LC 13) Max Grav 2=718(LC 23), 14=2486(LC 1), 10=713(LC 24)

FORCES. (lb) - Max. Comp /Max. Ten. - Ali forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-765/249, 3-5=-468/186, 5-6=-96/760, 6-7=-69/734, 7-9=-459/181, 9-10=-775/255

BOT CHORD 2-16=-135/602, 12-14=-240/296, 10-12=-103/618

WEBS 6-14-893/237, 9-12-445/361, 5-14-752/439, 5-16-178/603, 3-16-388/307,

7-14=-831/436, 7-12=-233/851

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads,
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=117, 14=277, 10=136.
- 7) Load case(s) 1 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard Except:
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-6=-60, 6-11=-60, 2-19=-20, 19-22=-60(F=-40), 10-22=-20



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTeMo connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TEM Quality Criteria, DSB-89 and BCSI Building Component Safety Information. available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	
1559185	T09	Monopitch Girder	1	2		T17227300
					Job Reference (optional)	
Buiders FirstSource,	Lake City, FL		8	240 s May	13 2019 MiTek Industries, Inc.	Mon Jun 3 13 31 03 2019 Page 1

ID BVHjtU?4L8iDBBHQFc123AzIu1B-aYK6gpacsPBDDrYcPJNEnKlHuFK07rEhoNpp_gzA__M 10-3-4 14-11-10 19-11-8 4-11-14

19-11-8

except end verticals.

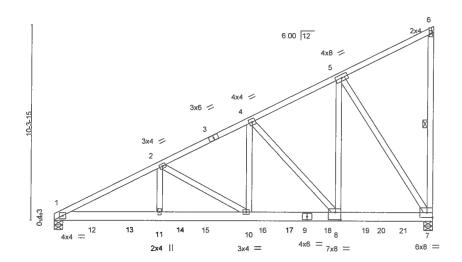
1 Row at midpt

Structural wood sheathing directly applied or 5-3-4 oc purlins,

6-7

Rigid ceiling directly applied or 10-0-0 oc bracing

Scale = 1:58.8



		5-6-14	4-8-6		4-8-6	4-11-14		
Plate Offsets (X,Y)-	[8:0-3-8,0-4-12]							
LOADING (psf) TCLL 20.0	SPACING- Plate Grip DOL	2-0-0 1.25	CSI. TC 0.38	DEFL. Vert(LL)	in (loc) -0.08 10-11	l/defl L/d >999 240	PLATES MT20	GRIP 244/190

14-11-10

BRACING-

TOP CHORD

BOT CHORD

WEBS

10-3-4

180 10-11 BCLL 0.0 Rep Stress Incr NO WB 0.77 Horz(CT) 0.04 7 n/a n/a BCOL Code FBC2017/TPI2014 10.0 Matrix-S

5-6-14

5-6-14

Weight: 290 lb FT = 20%

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 2x6 SP No.2

BOT CHORD 2x4 SP No.3 *Except* WEBS

5-7: 2x4 SP M 31

(lb/size) 7=4318/0-5-8, 1=2652/0-5-8

Max Horz 1=285(LC 26)

Max Uplift 7=-946(LC 8), 1=-599(LC 8)

FORCES. (lb) - Max, Comp./Max, Ten, - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-4953/1075, 2-4=-3811/761, 4-5=-2465/437

BOT CHORD 1-11=-1195/4362, 10-11=-1195/4362, 8-10=-842/3348, 7-8=-489/2155 WEBS

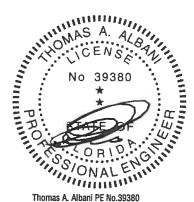
2-11=-242/826, 2-10=-1174/409, 4-10=-441/1633, 4-8=-1760/521, 5-8=-840/4038,

NOTES-

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 - Top chords connected as follows: 2x4 1 row at 0-9-0 oc
 - Bottom chords connected as follows: 2x6 2 rows staggered at 0-9-0 oc.
 - Webs connected as follows: 2x4 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated
- Wind. ASCE 7-10, Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (ji=lb)
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 275 lb down and 116 lb up at 2-0-0, 275 lb down and 116 lb up at 4-0-0, 275 lb down and 116 lb up at 6-0-0, 275 lb down and 116 lb up at 8-0-0, 364 lb down and 104 lb up at 10-0-0, 364 lb down and 104 lb up at 11-0-0, 364 lb down and 104 lb up at 12-5-0, 1069 lb down and 208 lb up at 14-5-0, and 1069 lb down and 208 lb up at 16-5-0, and 1069 lb down and 208 lb up at 18-5-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced); Lumber Increase=1.25, Plate Increase=1.25



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019

Continued on page 2

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev., 10/03/2015 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters and on individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Pracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the abrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Compositions.

Safety Information available from Truss Plate Institute. 218 N Lee Street, Suffe 312, Alexandra, VA 22314.



	Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence
į	1559185	T09	Monopitch Girder	1		T17227300
	Buiders FirstSource, La	ke City, FL		B.	240 c Mou	Job Reference (optional)
			ID:BVHjt	U?4L8iDB	BHQFc123	13 2019 MiTek Industries, Inc. Mon Jun. 3 13 31 03 2019 Page 2 3Azlu1B-aYK6gpacsPBDDrYcPJNEnKIHuFK07rEhoNpp_gzAM
	LOAD CASE(S) Standard					

LOAD CASE(S) Standard Uniform Loads (plf) Vert 1-6=-60, 1-7=-20

Concentrated Loads (lb)

Vert: 10=364(B) 12=275(B) 13=-275(B) 14=-275(B) 15=-275(B) 16=-364(B) 17=-364(B) 18=-1069(B) 19=-1069(B) 21=-1069(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a russ system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandna, VA 22314.



Truss Type Qty Ply O'Quinn Residence Job T17227301 T10 ROOF SPECIAL 1559185 Job Reference (optional) 8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 05 2019 Page 1 Buiders FirstSource Lake City, FL ID BVHjtU?4LBiDBBHQFc123Azlu1B-XxSs5VcsO0RxS8h?XkQitlqYk33LblG_Ghlv2YzA__K 17-7-0 19-7-0 8-9-8 4-3-14 13-1-6 4-3-14 4-5-10 Scale = 1 39 4

4x6 || 6.00 12 4x4 = 5x8 = 5x6 / 5x6 > 6 5x6 <> 5x6 / 14-15 6.00 12 X 12 3x6 || 3x6 4-5-10 8-9-8 Plate Offsets (X,Y)--[2:0-2-11,0-2-8], [6:0-2-11,0-2-8] PLATES LOADING (psf) CSI. DEFL I/defl L/d SPACING-(loc) 2-0-0

LUMBER-

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

20.0

10.0

10.0

0.0

BRACING-

Vert(LL)

Vert(CT)

Horz(CT)

0.30 9-10

-0.34

0.38

TOP CHORD

Structural wood sheathing directly applied or 3-3-5 oc purlins,

MT20

Weight: 100 lb

244/190

FT = 20%

except end verticals.

>691

n/a

10 >603

8

BOT CHORD Rigid ceiling directly applied or 3-10-3 oc bracing.

240

180

n/a

REACTIONS. (lb/size) 12=820/0-3-8, 8=820/0-3-8

Max Horz 12=62(LC 11)

Max Uplift 12=-204(LC 9), 8=-204(LC 8)

Plate Grip DOL

Rep Stress Incr

Code FBC2017/TPI2014

Lumber DOL

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-12=-783/904, 2-3=-1957/2358, 3-4=-2586/2479, 4-5=-2586/2479, 5-6=-1957/2381,

1.25

1.25

YES

TC

вС

WB

Matrix-S

0.62

0.52

0.80

6-8=-783/886

BOT CHORD 10-11=-1967/1904, 9-10=-1994/1904

WEBS 4-10=-2038/2007, 5-10=-31/602, 5-9=-409/179, 6-9=-1972/1696, 3-10=-26/587,

3-11=-409/175, 2-11=-1964/1696

NOTES-

1) Unbalanced roof live loads have been considered for this design.

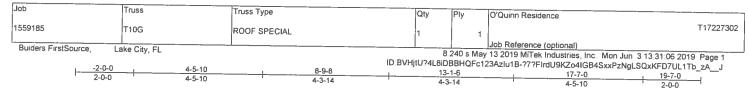
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads,
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Bearing at joint(s) 12, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 12=204, 8=204.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3.2019





Scale = 1.38.3

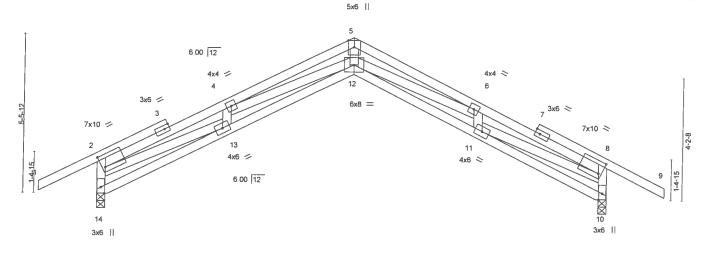


Plate Offsets (X,Y) [4-5-10 4-5-10 2:0-1-4,0-5-0]. [8:0-1-4,0-5-0]	8-9-8 4-3-14		13-1-6 4-3-14			17-7-0 4-5-10	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.82 BC 0.50 WB 0.63 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.44 11-12 -0.51 12 0.56 10	l/defl >468 >403 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 106 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

WEBS

2x4 SP No.3 *Except*

5-12: 2x4 SP No.2, 6-12,8-11,4-12,2-13: 2x4 SP M 31

BRACING-

TOP CHORD

Structural wood sheathing directly applied or 2-11-11 oc purlins, except end verticals. **BOT CHORD** Rigid ceiling directly applied or 3-5-6 oc bracing

REACTIONS. (lb/size) 14=820/0-3-8, 10=820/0-3-8 Max Horz 14=-91(LC 13)

Max Uplift 14=-250(LC 9), 10=-250(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 2-14=-834/974, 2-4=-2544/3103, 4-5=-3410/3289, 5-6=-3410/3290, 6-8=-2544/3129,

8-10=-834/955

BOT CHORD 12-13=-2810/2577, 11-12=-2845/2577

5-12=-2797/2774, 6-12=-132/820, 6-11=-563/381, 8-11=-2599/2183, 4-12=-8/747, **WEBS**

4-13=-563/367, 2-13=-2559/2183

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

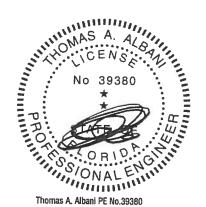
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi

6) Bearing at joint(s) 14, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 14=250, 10=250.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019

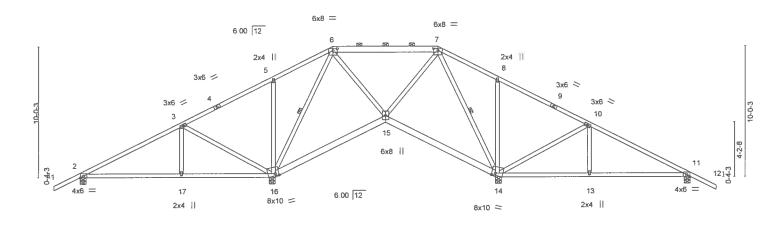
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITEM Quality Criteria, DSB-89 and BCSI Building Component Safety Information. Available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss T	ype		Qty	Ply	O'Quinn Residence		
									T17227303
1559185	T11	PIGGYE	BACK BASE		1	1			
1		ļ					Job Reference (optional)		
Buiders FirstSource,	Lake City, FL					.240 s Ma	y 13 2019 MiTek Industries,	Inc. Mon Jun 3 13 3	1:08 2019 Page 1
					ID BVHjtU?4L8iD	BBHQFc1	23Azlu1B-xW7?jXelhxpWJc	:QaCtzPUOS2UG6Eo	55QyfWaftzAH
,-2-0-0 ,	7-8-12	14-11-0	19-4-0	27-4-0) .	31-9-0	38-11-4	46-8-0	48-8-0
2-0-0	7-8-12	7-2-4	4-5-0	8-0-0	l	4-5-0	7-2-4	7 - 8-12	2-0-0

Scale = 1.85.3



1	7-8-12	14-11-0	23-4-0	31-9-0	, 38-11-4	46-8-0
	7-8-12	7-2-4	8-5-0	8-5-0	7-2-4	7-8-12
ets (X,Y)-	[6:0-4-0,0-1-15], [7:0-4-0	,0-1-15], [14:0-	3-6,0-1-15], [16:0-3-6,0-1-15	1		
(psf)	SPACING-	2-0-0	CSI.	DEFL. in (loc)	l/defi L/d	PLATES GRIP
20.0	Plate Grip DOL	1.25	TC 0.65	Vert(LL) 0.13 11-13	>999 240	MT20 244/190
10.0	Lumber DOL	1.25	BC 0.51	Vert(CT) -0.17 2-17	>999 180	
0.0 *	Rep Stress Incr	YES	WB 0.86	Horz(CT) 0.03 11	n/a n/a	
10.0	Code FBC2017/T	PI2014	Matrix-S			Weight: 284 lb FT = 20%
	(psf) 20.0 10.0 0.0 *	7-8-12 ts (X,Y)— [6:0-4-0,0-1-15], [7:0-4-0 (psf) SPACING- 20.0 Plate Grip DOL 10.0 Lumber DOL 0.0 Rep Stress Incr	7-8-12 7-2-4 ts (X,Y)— [6:0-4-0,0-1-15], [7:0-4-0,0-1-15], [14:0-1 (psf) SPACING- 2-0-0 20.0 Plate Grip DOL 1.25 10.0 Lumber DOL 1.25 0.0 Rep Stress Incr YES	ts (X,Y)— [6:0-4-0,0-1-15], [7:0-4-0,0-1-15], [14:0-3-6,0-1-15], [16:0-3-6,0-1-15] (psf) SPACING- 2-0-0 CSI. 20.0 Plate Grip DOL 1.25 TC 0.65 10.0 Lumber DOL 1.25 BC 0.51 0.0 Rep Stress Incr YES WB 0.86	7-8-12 7-2-4 8-5-0 8-5-0 ts (X,Y)— [6:0-4-0,0-1-15], [7:0-4-0,0-1-15], [14:0-3-6,0-1-15], [16:0-3-6,0-1-15] (psf) SPACING- 2-0-0 CSI. DEFL. in (loc) 20.0 Plate Grip DOL 1.25 TC 0.65 Vert(LL) 0.13 11-13 10.0 Lumber DOL 1.25 BC 0.51 Vert(CT) -0.17 2-17 0.0 Rep Stress Incr YES WB 0.86 Horz(CT) 0.03 11	7-8-12 7-2-4 8-5-0 8-5-0 7-2-4 ts (X,Y)— [6:0-4-0,0-1-15], [7:0-4-0,0-1-15], [14:0-3-6,0-1-15], [16:0-3-6,0-1-15] (psf) SPACING- 2-0-0 CSI. DEFL. in (loc) I/defl L/d 20.0 Plate Grip DOL 1.25 TC 0.65 Vert(LL) 0.13 11-13 >999 240 10.0 Lumber DOL 1.25 BC 0.51 Vert(CT) -0.17 2-17 >999 180 0.0 * Rep Stress Incr YES WB 0.86 Horz(CT) 0.03 11 n/a n/a

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

2x4 SP No.2 *Except* TOP CHORD

6-7: 2x6 SP No.2 **BOT CHORD** 2x4 SP No.2 *Except*

15-16,14-15: 2x6 SP No.2 WEBS

2x4 SP No.3 *Except* 6-16,7-14: 2x4 SP No.2

REACTIONS. All bearings 0-5-8

(lb) -Max Horz 2=123(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) except 2=149(LC 8), 16=-349(LC 9), 14=-320(LC 8), 11=-165(LC 8)

Max Grav All reactions 250 lb or less at joint(s) except 2=536(LC 23), 16=1477(LC

23), 14=1477(LC 24), 11=536(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-397/408, 3-5=127/485, 5-6=-40/456, 7-8=-6/449, 8-10=-134/485, 10-11=-397/398

2-17=199/268, 16-17=199/268, 15-16=205/534, 14-15=203/531, 13-14=212/268,

BOT CHORD

WEBS

3-17=-270/332, 3-16=-696/714, 5-16=-351/286, 6-16=-689/49, 6-15=0/331, 7-15=0/336, 7-14=-689/4, 8-14=-351/286, 10-14=-696/714, 10-13=-270/332

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vuit=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 149 lb uplift at joint 2, 349 lb uplift at joint 16, 320 lb uplift at joint 14 and 165 lb uplift at joint 11.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 6-0-0 oc purlins, except

6-16, 7-14

2-0-0 oc purlins (6-0-0 max.): 6-7

1 Row at midpt

Rigid ceiling directly applied or 6-0-0 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019

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	Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence
	1559185	T11G	GABLE I I	1	2	T17227304
,	Buiders FirstSource, La	ake City, FL		8	240 s May	Job Reference (optional) / 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 13 2019 Page 1
	7-2-0-0		-11-0 19-11-14 26-8 -2-4 5-0-14 6-8-	-2 -2	HQFc123A 31-9-0 5-0-14	ZZIu1B-ITxumEritWUSoPNJX?QZaBRA_KHsVTUZ96WEKK5ZA_C 38-11-4

Scale = 1:88.2

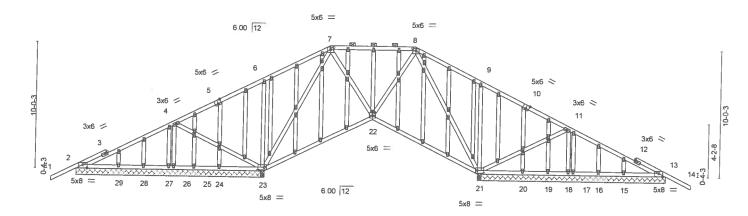


Plate Offsets (X,Y)-	7-8-12 7-8-12 [2:0-4-0,0-3-1], [5:0-3-0,1 [22:0-2-0,0-0-4], [23:0-6-	14-11-0 7-2-4 Edge}, [5:0-0-0,0- -0,0-2-8], [44:0-1-	23-4-0 8-5-0 -1-12], [7:0-3-0,0-2-0], [8: -15,0-1-0], [45:0-1-15,0-0	:0-3-0,0-2-0], [10: -0], [46:0-1-15,0-	31-9-0 8-5-0 :0-3-0,Edge], [1 -1-0], [64:0-1-15	0,0-0-0.0 1,[0-0-0,i	38-11-4 7-2-4 1-1-12], [13:0- 65:0-1-15,0-1	46-8-0 7-8-12 4-0,0-3-1], [21:0-6-0,0- 1-0], [67:0-1-15,0-1-0]	2-8],
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/T	2-0-0 1.25 1.25 YES PI2014	CSI. TC 0.32 BC 0.31 WB 0.33 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.09 21-22 -0.19 21-22 0.02 13	l/defi >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 824 lb	GRIP 244/190 FT = 20%

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

OTHERS 2x4 SP No.3 TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 10-0-0 oc purlins,

2-0-0 oc purlins (6-0-0 max.): 7-8.

Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 22-23,21-22.

All bearings 14-11-0.

Max Horz 2=-191(LC 13) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 13, 27, 29, 17, 15 except 23=-259(LC 12), 21=-197(LC 13), 26=-345(LC 12), 18=-359(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 13, 24, 25, 27, 28, 29, 20, 19,

17, 16, 15 except 23=1045(LC 1), 23=1045(LC 1), 21=1045(LC 1), 21=1045(LC

1), 26=487(LC 23), 18=487(LC 24)

BOT CHORD

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-274/408, 4-6=-65/401, 6-7=0/410, 8-9=0/398, 9-11=0/362, 11-13=-190/408 2-29=-272/283, 28-29=-272/283, 27-28=-272/283, 26-27=-272/283, 25-26=-272/283

24-25-272/283, 23-24-272/283, 22-23-104/343, 21-22-101/338, 20-21-272/268, 19-20=-272/268, 18-19=-272/268, 17-18=-272/268, 16-17=-272/268, 15-16=-272/268,

13-15=-272/268

4-26=-496/396, 6-23=-371/309, 7-23=-665/96, 8-22=0/260, 8-21=-665/70,

9-21=-371/308, 11-18=-496/394

NOTES-

WEBS

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated

- 3) Unbalanced roof live loads have been considered for this design.
 4) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Provide adequate drainage to prevent water ponding.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. Continued on page 2

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ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

June 3,2019



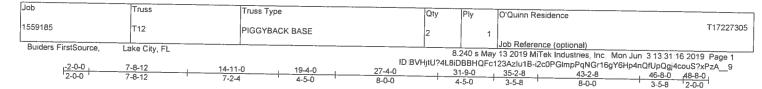
i	Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	
	1559185	T11G	GABLE I I	1	2	In Defense (adiabat)	T17227304
į				<u> </u>		Job Reference (optional)	
						. 12 2010 MiTal Indications Inc. Man Ivo 2 12 21 12 2011	

ID.BVHjtU?4L8iDBBHQFc123Azlu1B-ITxumEitWUSoPNJX?QZaBRA_KHsVTUZ96wEKK5zA__C

NOTES-

- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 13, 27, 29, 17, 15 except (jt=lb) 23=259, 21=197, 26=345, 18=359.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





Scale = 1:87:3

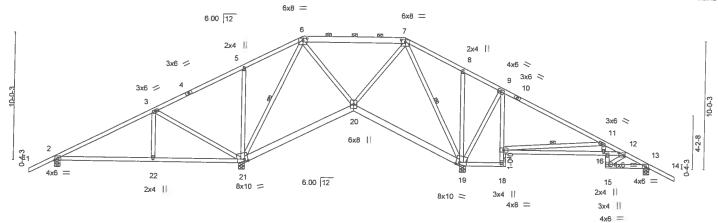


Plate Offsets (X,Y)-		4-11-0 7-2-4 -15], [12:0-4-13	23-4-0 8-5-0 3,0-2-3]. [19:0-3-6,0-1-15	31-9-0 8-5-0], [21:0-3-6,0-1-15]		35-2-8 3-5-8	1	43-2-8 46-8 8-0-0 3-5-	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25 YES 014	CSI. TC 0.70 BC 0.61 WB 0.86 Matrix-S	Vert(LL) -0.	in (loc) 12 16-17 27 16-17 04 13	l/defl >999 >654 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 302 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

6-7: 2x6 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

20-21,19-20: 2x6 SP No.2, 9-18,15-16: 2x4 SP No.3 WEBS

2x4 SP No.3 *Except*

6-21,7-19: 2x4 SP No.2

TOP CHORD

BRACING-

BOT CHORD

WEBS

2-0-0 oc purlins (10-0-0 max.): 6-7. Rigid ceiling directly applied or 6-0-0 oc bracing

1 Row at midpt

6-21, 7-19, 11-17

Structural wood sheathing directly applied or 4-10-10 oc purlins,

REACTIONS. All bearings 0-5-8

(lb) -Max Horz 2=123(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2 except 21-242(LC 12), 19-175(LC

13), 13=140(LC 13)

Max Grav All reactions 250 lb or less at joint(s) except 2=534(LC 23), 21=1464(LC 23), 19=1591(LC 24), 13=471(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-392/215, 3-5=-33/498, 5-6=0/466, 7-8=0/612, 8-9=0/560, 9-11=0/364, 11-12=-1091/557, 12-13=-341/239

BOT CHORD

2-22=-71/273, 21-22=-71/273, 20-21=-261/366, 19-20=-290/328, 9-17=-26/269, 16-17=-481/939, 12-16=-427/1031

3-22=0/332, 3-21=-696/370, 5-21=-351/289, 6-21=-666/85, 7-20=-1/251, 7-19=-807/0, WEBS

9-19=-543/316, 11-16=0/526, 12-15=-281/89, 11-17=-1167/596

Unbalanced roof live loads have been considered for this design.

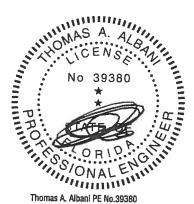
2) Wind: ASCE 7-10, Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 21=242, 19=175, 13=140.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610

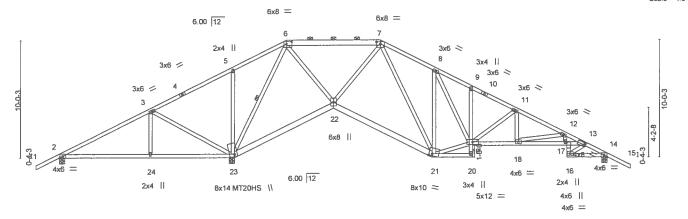
June 3.2019

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**ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Components of the property damage. For general guidance regarding the fabrication available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



dot	Truss	Truss Type		10	Qtv	Ply	O'Quinn Residence	
	1,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	1				•		T17227306
1559185	T13	PIGGYBACK	BASE	12	2	1		
							Job Reference (optional)	
Buiders FirstSource,	Lake City, FL	· · · · · · · · · · · · · · · · · · ·			8.2	40 s May	13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 17	2019 Page 1
	75			ID BVHjtL	J24L8iDB8	BHQFc12	3Azlu1B-AFAPcclOZiyEu_clEGdWMHKYVu2zP8Rl0	YCYTszA8
₋ 2-0-0	7-8-12	14-11-0	19-4-0	27-4-0	31-9	-0 3	35-2-8 38-11-4 43-2-8 46-8-0 48-8-0	
2-0-0	7_8_12	7-2-4	4-5-0	8-0-0	4-5-	v	3-5-8 3-8-12 4-3-4 3-5-8 2-0-0	



		7-8-12	14-11-0	23-4-0	31-9-0	35-2-8	38-11-4	43-2-8 46-8-0	
		7-8-12	7-2-4	8-5-0	8-5-0	3-5-8	3-8-12	4-3-4 3-5-8	
Plate Offse	ets (X,Y)-	[6:0-5-4,0-2-8], [7:0-4-0,0-	-1-15], [13:0-1-	2,0-2-4], [17:0-0-8,0-1-12], [21:0-3-6,0-1-15], [23:0-2	-6,Edge)			
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL. in	(loc) Vdefl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC 0.89	Vert(LL) -0.16 17	7-18 >999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC 0.98	Vert(CT) -0.32 17	7-18 >999	180	MT20HS	187/143
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.98	Horz(CT) 0.14	14 n/a	n/a		
BCDL	10.0	Code FBC2017/TF	PI2014	Matrix-S				Weight: 312 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

2x4 SP No.2 *Except* TOP CHORD

6-7: 2x6 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

22-23,21-22: 2x6 SP No.2, 9-20,16-17: 2x4 SP No.3 2x4 SP No.3 *Except*

WEBS

6-23,7-21: 2x4 SP No.2

REACTIONS. 2=19/0-5-8, 23=2865/0-5-8, 14=1080/0-5-8 (lb/size)

Max Horz 2=-123(LC 10)
Max Uplift 2=-304(LC 24), 23=-287(LC 12), 14=-201(LC 13)
Max Grav 2=239(LC 23), 23=2865(LC 1), 14=1097(LC 24)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-241/1066, 3-5=-390/1598, 5-6=-233/1557, 7-8=-883/607, 8-9=-1328/682, 9-11=-1365/608, 11-12=-1955/772, 12-13=-3585/1299, 13-14=-1560/604 TOP CHORD

2-24=-911/328, 23-24=-911/328, 22-23=-630/544, 21-22=0/596, 18-19=-520/1724, **BOT CHORD**

17-18=-992/2908, 13-17=-1017/3046, 16-17=-260/843, 14-16=-422/1260

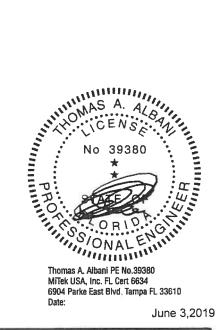
3-24=0/333, 3-23=-719/378, 5-23=-364/293, 6-23=-1928/405, 6-22=-81/928

7-22=-733/512, 7-21=-492/603, 8-21=-864/446, 11-19=-696/315, 11-18=-92/414, 12-18=-1208/481, 8-19=-377/935, 19-21=-72/707, 12-17=-230/877, 13-16=-1375/461

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 2-2-0 oc purlins, except

6-23

2-0-0 oc purlins (6-0-0 max.): 6-7.

1 Row at midpt

Rigid ceiling directly applied or 2-2-0 oc bracing.

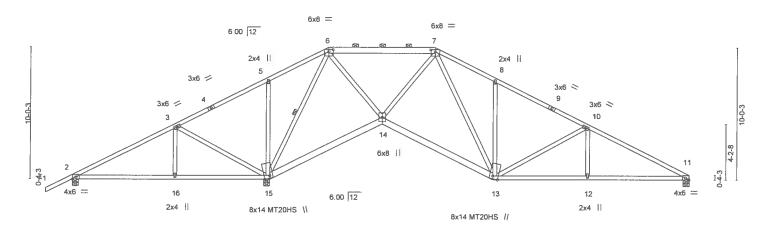
June 3.2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly demange. For general guildance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Jo	b	Truss	Truss Type		Qty	Piy	O'Quinn Residence	
	50405	T	D.O.O.V.D.A.O.V.D.A.O.E				1	T17227307
110	59185	T14	PIGGYBACK BASE		10	1	,	
		<u> </u>				l	Job Reference (optional)	
E	Buiders FirstSource, La	ake City, FL			8	240 s May	13 2019 MiTek Industries, In	nc. Mon Jun 3 13:31:20 2019 Page 1
					ID BVHjtU?4L8iD	BBHQFc12	23Azlu1B-aqsXEdoGsdKplSL	tvOBD_wy1F68jcWlBjWQC4BzA 5
		8-12 , 14-11	-0 19-4-0	27-4	1-0	31-9-0	38-11-4	46-8-0
		8-12 7-2-	4-5-0	8-0	-0	4-5-0	7-2-4	7-8-12



7-8-12	14-11-0	23-4-0	31-9-0	38-11-4	46-8-0	
7-8-12	7-2-4	8-5-0	8-5-0	7-2-4	7-8-12	
)- [6:0-4-0,0-1-15], [[7:0-4-0,0-1-15], [9:0-0-	0,0-0-0], [13:0-2-6,Edge], [1	5:0-2-6,Edge]			***
SPACING	- 2-0-0	CSI.	DEFL. in (loc)	l/defl L/d	PLATES	GRIP
Plate Grip	DOL 1.25	TC 0.92	Vert(LL) -0.10 11-12	>999 240	MT20	244/190
Lumber DO	DL 1.25	BC 0.72	Vert(CT) -0.24 11-12	>999 180	MT20HS	187/143
Rep Stress	s Incr YES	WB 0.91	Horz(CT) 0.07 11	n/a n/a		
Code FBC	2017/TPI2014	Matrix-S	, ,		Weight: 280 lb	FT = 20%
	7-8-12 ()— [6:0-4-0,0-1-15], SPACING Plate Grip Lumber Do Rep Stress	7-8-12 7-2-4 ()— [6:0-4-0,0-1-15], [7:0-4-0,0-1-15], [9:0-0- SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25	7-8-12 7-2-4 8-5-0 ()— [6:0-4-0,0-1-15], [7:0-4-0,0-1-15], [9:0-0-0,0-0-0], [13:0-2-6,Edge], [1 SPACING- 2-0-0 CSI. Plate Grip DOL 1.25 TC 0.92 Lumber DOL 1.25 BC 0.72 Rep Stress Incr YES WB 0.91	7-8-12 7-2-4 8-5-0 8-5-0 ()— [6:0-4-0,0-1-15], [7:0-4-0,0-1-15], [9:0-0-0,0-0-0], [13:0-2-6,Edge], [15:0-2-6,Edge] SPACING- 2-0-0 CSI. DEFL. in (loc) Plate Grip DOL 1.25 TC 0.92 Vert(LL) -0.10 11-12 Lumber DOL 1.25 BC 0.72 Vert(CT) -0.24 11-12 Rep Stress Incr YES WB 0.91 Horz(CT) 0.07 11	7-8-12 7-2-4 8-5-0 8-5-0 7-2-4 ()— [6:0-4-0,0-1-15], [7:0-4-0,0-1-15], [9:0-0-0,0-0-0], [13:0-2-6,Edge], [15:0-2-6,Edge] SPACING- 2-0-0 CSI. DEFL. in (loc) I/defl L/d Plate Grip DOL 1.25 TC 0.92 Vert(LL) -0.10 11-12 >999 240 Lumber DOL 1.25 BC 0.72 Vert(CT) -0.24 11-12 >999 180 Rep Stress Incr YES WB 0.91 Horz(CT) 0.07 11 n/a n/a	7-8-12 7-2-4 8-5-0 8-5-0 7-2-4 7-8-12 ()— [6:0-4-0,0-1-15], [7:0-4-0,0-1-15], [9:0-0-0,0-0-0], [13:0-2-6,Edge] SPACING- 2-0-0 CSI. DEFL. in (loc) I/defl L/d PLATES Plate Grip DOL 1.25 TC 0.92 Vert(LL) -0.10 11-12 >999 240 MT20 Lumber DOL 1.25 BC 0.72 Vert(CT) -0.24 11-12 >999 180 MT20HS * Rep Stress Incr YES WB 0.91 Horz(CT) 0.07 11 n/a n/a

BRACING-

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied, except

6-15

2-0-0 oc purlins (6-0-0 max.); 6-7.
Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

6-7: 2x6 SP No.2

BOT CHORD 2x4 SP No.2 *Except* 14-15,13-14: 2x6 SP No.2

WERS 2x4 SP No.3 *Except*

6-15.7-13: 2x4 SP No.2

REACTIONS. (lb/size) 2=252/0-5-8, 15=2530/0-5-8, 11=1049/0-5-8

Max Horz 2=126(LC 11)

Max Uplift 2=-81(LC 12), 15=-280(LC 12), 11=-180(LC 13) Max Grav 2=369(LC 23), 15=2530(LC 1), 11=1067(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 2-3=89/553, 3-5=242/1088, 5-6=87/1049, 6-7=-559/238, 7-8=-1157/690,

8-10=-1200/552, 10-11=-1858/721 BOT CHORD 2-16=-456/154, 15-16=-456/154, 14-15=-264/348, 13-14=-40/918, 12-13=-549/1589,

11-12=-549/1589

WEBS 3-16=0/332, 3-15=-707/375, 5-15=-360/292, 6-15=-1787/397, 6-14=-150/1048,

7-14-425/390, 7-13-444/494, 8-13-330/273, 10-13-699/382, 10-12-0/335

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft, Cat. II, Exp C; Encl., GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 15=280, 11=180.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



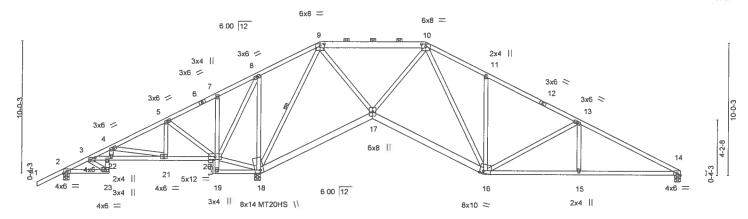
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

June 3,2019

M WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE



Job	Truss		Truss Type		Qty	Ply	O'Quinn Residence	
1559185	T15		PIGGYBACK BASE		1	1		T17227308
							Job Reference (optional)	
Buiders FirstSource,	Lake City, FL						y 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31.	
					ID BVHjtU?4L8it	DBBHQFc12	3Azlu1B-XCzlfJpXOFaX?mVG0pDh3L1NNvqC4Pl	kUAqvJ93zA 3
<u>-2-0-0 , 3-5-8</u>	7-8-12	11-5-8	14-11-0 , 19-4	-0 ,	27-4-0	31-9-0	38-11-4 , 46-8-0	
2-0-0 3-5-8	4-3-4	3-8-12	3-5-8 4-5-	0	8-0-0	4-5-0	7-2-4 7-8-12	



	- 3-5- 3-5-			23-4-0 8-5-0	31-9-0 8-5-0	38-11-4 7-2-4	46-8-0 7-8-12
Plate Offse	ets (X,Y)-	[3:0-4-13,0-2-3], [9:0-4-0	,0-1-15], [10:0-4-	0,0-1-15], [16:0-3-6,0-1-	[15], [18:0-2-6,Edge]		
LOADING TCLL TCDL BCLL	(psf) 20.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI. TC 0.94 BC 0.72 WB 0.91	DEFL. in (loc) Vert(LL) -0.10 14-15 Vert(CT) -0.24 14-15	Vdefi L/d >999 240 >999 180	PLATES GRIP MT20 244/190 MT20HS 187/143
CDL	10.0	Code FBC2017/T		WB 0.91 Matrix-S	Horz(CT) 0.06 14	n/a n/a	Weight: 309 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied, except 2-0-0 oc purlins (6-0-0 max.): 9-10.
Rigid ceiling directly applied or 6-0-0 oc bracing.

9-18

1 Row at midpt

LUMBER-

2x4 SP No.2 *Except* TOP CHORD

9-10: 2x6 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

22-23,7-19: 2x4 SP No.3, 17-18,16-17: 2x6 SP No.2 2x4 SP No.3 *Except*

WEBS

9-18,10-16: 2x4 SP No.2

REACTIONS. (lb/size) 2=271/0-5-8, 18=2514/0-5-8, 14=1046/0-5-8

Max Horz 2=126(LC 11) Max Uplift 2=-59(LC 12), 18=-313(LC 12), 14=-188(LC 13) Max Grav 2=342(LC 23), 18=2514(LC 1), 14=1092(LC 24)

(lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown.
3-4=-240/479, 4-5=-122/558, 5-7=-209/901, 7-8=-128/905, 8-9=-102/1047,
9-10=-667/251, 10-11=-1212/696, 11-13=-1254/558, 13-14=-1911/727
8D 3-22=-393/233, 21-22=-406/210, 20-21=-483/231, 17-18=-287/352, 16-17=-48/990, FORCES.

TOP CHORD

BOT CHORD 15-16=-554/1637, 14-15=-554/1637

WEBS

4-21=-427/186, 5-20=-443/219, 8-18=-457/291, 9-18=-1793/416, 9-17=-149/1072,

10-17=-431/392, 10-16=-447/490, 11-16=-330/274, 13-16=-699/382, 13-15=0/335, 18-20=-881/481, 8-20=-139/312

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10, Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60

 3) Provide adequate drainage to prevent water ponding.

 4) All plates are MT20 plates unless otherwise indicated.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 18=313, 14=188.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord

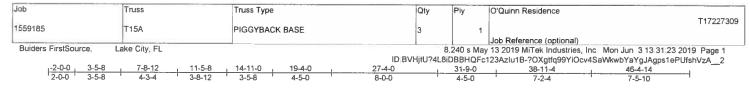


Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

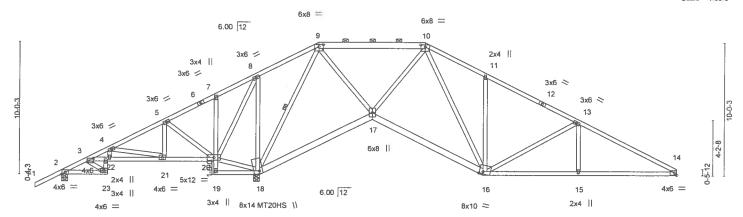
June 3,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev., 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see MSITP11 Quelity Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Scale = 1:83.8



2-0-0 2-0-0	3-5-8, 7-8-12 11-5-8 1-5-8 4-3-4 3-8-12	14-11-0 3-5-8	23-4-0 8-5-0		-9-0 5-0		<u>-11-4</u> -2-4	46-4-14 7-5-10
Plate Offsets (X,Y)-	[3:0-4-13,0-2-3], [9:0-4-0,0-1-1	15], [10:0-4-0,0-1-15	5], [16:0-3-6,0-1-15], [, ,		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	Plate Grip DOL 1 Lumber DOL 1	0-0 CS .25 TC .25 BC ES W	0.91 0.70	Vert(CT) -(in (loc) 0.09 14-15 0.22 14-15 0.06 14	l/defl L/ >999 24 >999 18 n/a n/	0 MT20 0 MT20H	244/190 S 187/143

BRACING-

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied, except

Rigid ceiling directly applied or 6-0-0 oc bracing

9-18

2-0-0 oc purlins (6-0-0 max.): 9-10.

1 Row at midpt

LUMBER-

2x4 SP No.2 *Except* TOP CHORD

9-10: 2x6 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

22-23,7-19: 2x4 SP No.3, 17-18,16-17: 2x6 SP No.2 2x4 SP No.3 *Except*

WEBS

9-18,10-16; 2x4 SP No.2

REACTIONS. (lb/size) 2=274/0-5-8, 18=2506/0-5-8, 14=1043/Mechanical

Max Horz 2=126(LC 11)

Max Uplift 2=-59(LC 12), 18=-312(LC 12), 14=-188(LC 13) Max Grav 2=344(LC 23), 18=2506(LC 1), 14=1089(LC 24)

FORCES. (lb) - Max, Comp./Max, Ten, - All forces 250 (lb) or less except when shown

TOP CHORD

3-4--247/465, 4-5-119/549, 5-7--206/892, 7-8--125/897, 8-9--99/1040, 9-10--668/251, 10-11--1207/695, 11-13--1249/556, 13-14--1894/721 3-22--381/232, 21-22--395/208, 20-21--474/228, 17-18--285/350, 16-17--47/988,

BOT CHORD 15-16=-546/1618, 14-15=-546/1618

WEBS 4-21=-429/188, 5-20=-443/219, 8-18=-458/292, 9-18=-1787/414, 9-17=-147/1069,

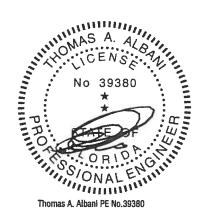
10-17=-426/390, 10-16=-445/485, 11-16=-332/275, 13-16=-682/375, 13-15=0/333, 18-20=-875/479, 8-20=-140/314

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4,2psf; BCDL=3,0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 18=312, 14=188.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Bivd, Tampa FL 33610

June 3,2019

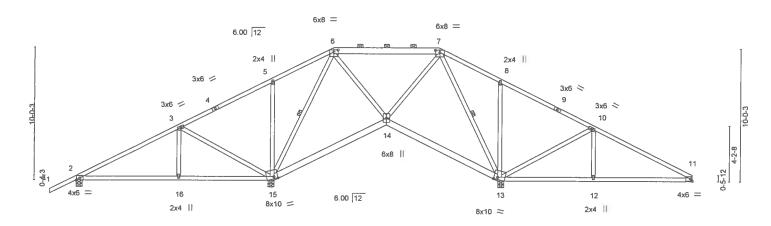
🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev., 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Col. Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Typ	е		Qty	Ply	O'Quinn Residence	
1559185	T16A	PIGGYBA	CK BASE		3	1		T17227310
							Job Reference (optional)	
Buiders FirstSource,	Lake City, FL				8	240 s May	13 2019 MiTek Industries, In	c. Mon Jun 3 13:31:25 2019 Page 1
				ID:B\	/HjtU?4L8	iDBBHQF	c123Azlu1B-xnfQHLsPhAy5s	DDrixnOhzfyR7u2Hnowso8zlOzA 0
2-0-0	7-8-12	14-11-0	19-4-0	27-4-0	4	31-9-0	38-11-4	46-4-14
2-0-0	7-8-12	7-2-4	4-5-0	8-0-0	1	4-5-0	7-2-4	7-5-10

Scale = 1.83.8



		7-8-12	14-11-0	23-4-0		31-9-0		38-11-4	46-4-14	- 1
		7-8-12	7-2-4	8-5-0		8-5-0		7-2-4	7-5-10	
Plate Offse	ets (X,Y)-	[6:0-4-0,0-1-15], [7:0-4-0),0-1-15], [13:0-	3-6,0-1-15], [15:0-3-6,0-1-15]						
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC 0.70	Vert(LL)	-0.08 11-12	>999	240	MT20	244/190
CDL	10.0	Lumber DOL	1.25	BC 0.52	Vert(CT)	-0.18 11-12	>981	180		
3CIT	0.0	Rep Stress Incr	YES	WB 0.89	Horz(CT)	0.03 11	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matrix-S	. ,				Weight: 279 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

2x4 SP No.2 *Except* TOP CHORD

6-7: 2x6 SP No.2 2x4 SP No.2 *Except*

BOT CHORD

14-15,13-14: 2x6 SP No.2 WEBS

2x4 SP No.3 *Except* 6-15,7-13: 2x4 SP No.2

REACTIONS. All bearings 0-5-8 except (jt=length) 11=Mechanical.

(lb) -Max Horz 2=126(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 2, 11 except 15=-232(LC 12),

13=-208(LC 13)

Max Grav All reactions 250 lb or less at joint(s) except 2=536(LC 23), 15=1476(LC 23), 13=1489(LC 24), 11=384(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-397/172, 3-5=-48/485, 5-6=0/450, 7-8=0/458, 8-10=-21/490, 10-11=-387/180 TOP CHORD **BOT CHORD** 2-16=-76/278, 15-16=-76/278, 14-15=-226/327, 13-14=-225/329, 12-13=-68/285,

11-12=-68/285

WEBS 3-16=0/332, 3-15=-696/371, 5-15=-351/289, 6-15=-688/69, 7-13=-695/30,

8-13=-344/279, 10-13=-723/390, 10-12=0/335

NOTES-

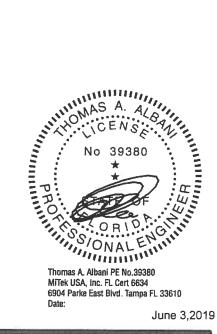
- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

Refer to girder(s) for truss to truss connections.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11 except (jt=lb) 15=232, 13=208.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 6-0-0 oc purlins, except

6-15, 7-13

2-0-0 oc purlins (6-0-0 max.): 6-7.

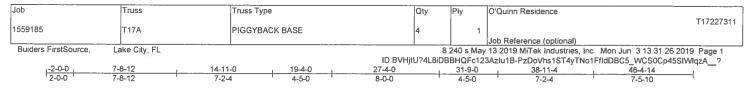
1 Row at midpt

Rigid ceiling directly applied or 6-0-0 oc bracing.

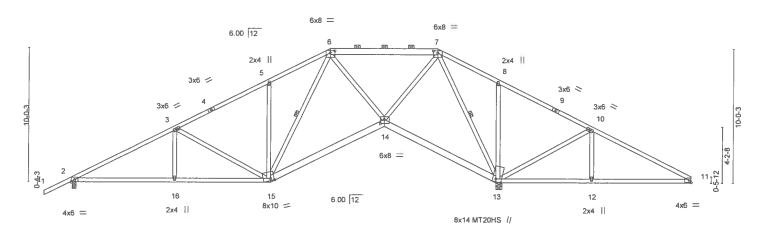
June 3,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev., 10/03/2015 BEFORE USE, Design valid for use only with MTelk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters shown, and is for an individual building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see. ASI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Scale = 1:83.1



0-1 <u>-0</u> 0-1-0	7-8-12 7-7-12	14-11-0 7-2-4	23-4-0 8-5-0	31-9-0 8-5-0	38-11-4 7-2-4	46-4-14 7-5-10
Plate Offsets (X,Y)-	[6:0-4-0,0-1-15], [7:0-4-0,	0-1-15], [13:Ed	ge,0-5-0], [14:0-3-8,0-3-12],			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TI	2-0-0 1.25 1.25 YES PI2014	CSI. TC 0.77 BC 0.70 WB 0.97 Matrix-MS	DEFL. in (loc) VerI(LL) 0.27 14-15 Vert(CT) 0.23 14-15 Horz(CT) 0.08 13	l/defl L/d >999 240 >999 180 n/a n/a	PLATES GRIP MT20 244/190 MT20HS 187/143 Weight: 279 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

REACTIONS.

NOTES-

TOP CHORD 2x4 SP No.2 *Except*

6-7: 2x6 SP No.2

2x4 SP No.2 *Except* BOT CHORD 14-15,13-14: 2x6 SP No.2

2x4 SP No.3 *Except* WEBS

6-15,7-13; 2x4 SP No.2

(lb/size) 13=2441/0-5-8, 11=184/Mechanical, 2=1208/0-3-8

Max Horz 2=128(LC 12)

Max Uplift 13=549(LC 9), 11=96(LC 13), 2=376(LC 9) Max Grav 13=2441(LC 1), 11=295(LC 24), 2=1215(LC 23)

FORCES.

(Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. RD 2-3=-1910/1904, 3-5=-1229/1415, 5-6=-1187/1551, 6-7=-618/935, 7-8=-483/900, TOP CHORD

8-10=-633/926, 10-11=-449/433

2-16-1614/1633, 15-16-1614/1633, 14-15-600/959, 13-14-157/396, 12-13-349/467, BOT CHORD

11-12=-349/467

WEBS 3-16--252/330, 3-15--712/704, 5-15--342/281, 6-15--777/453, 6-14--351/284, 7-14=-1172/1064, 7-13=-1737/1490, 8-13=-345/286, 10-13=-746/393, 10-12=0/338

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11 except (jt=lb) 13=549, 2=376,
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord,



Structural wood sheathing directly applied or 3-4-11 oc purlins,

6-15, 7-13

2-0-0 oc purlins (6-0-0 max.): 6-7.

1 Row at midpt

Rigid ceiling directly applied or 4-2-2 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

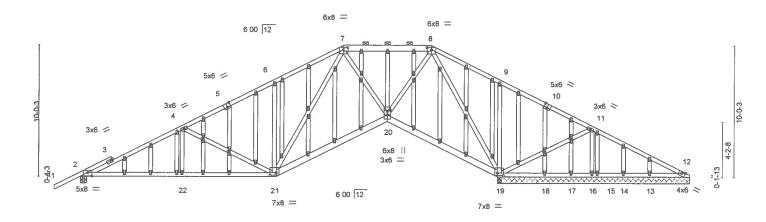
June 3,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence
				1	T17227312
1559185	T17G	GABLE GABLE L GABLE	1	2	3
					Job Reference (optional)
Buiders FirstSource	Lake City, FL		8	240 s May	/ 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 32 2019 Page 1
			ID BVHjtU?4L8i[DBBHQFc1	123Azlu1B-E7a4lkxo1Jr6BIGBcvP1TSSCMxK4Qx5yTQKrUUz9zzv
-2-0-0	7-8-12	14-11-0 , 19-11-14 ,	26-8-2	31-9-0	, 38-11-4 , 46-4-14 ,
2-0-0	7-8-12	7-2-4 5-0-14	6-8-4	5-0-14	7-2-4 7-5-10

Scale = 1:84.8



		7-8-12	14-11-0	23-4-0		31-9-0	38-11-4	45-4-14	
	-	7-8-12	7-2-4	8-5-0	1	8-5-0	7-2-4	7-5-10	1
Plate Offs	ets (X,Y)-	[2:0-4-0,0-3-1], [5:0-3-0,E	dge], [5:0-0-0,0	-1-12], [7:0-4-0,0-1-15], [[8:0-4-0,0-1-15], [10:0-3-0, Edgel,	[10:0-0-0,0-1-12], [1	9:0-5-8,0-2-41, [20:0-3-0,	0-0-151.
	, ,	[21:0-5-8,0-2-4], [29:0-1-							•"
		-							
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC 0.46	Vert(LL)	0.15 20-21	>999 240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC 0.36	Vert(CT)	-0.13 2-22	>999 180		
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.92	Horz(CT)	0.04 19	n/a n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matrix-S	' '			Weight: 839 lb	FT = 20%
					1			3	==

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

2x4 SP No.2 *Except* TOP CHORD

7-8: 2x6 SP No.2

2x4 SP No.2 *Except* **BOT CHORD**

20-21,19-20: 2x6 SP No.2

WEBS 2x4 SP No.3 2x4 SP No.3 OTHERS

All bearings 14-7-14 except (jt=length) 2=0-5-8.

Max Horz 2=207(LC 16) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 17, 15, 13 except 2=-424(LC 9),

19=-963(LC 9), 12=-104(LC 23), 16=-466(LC 23) Max Grav All reactions 250 lb or less at joint(s) 12, 16, 18, 17, 15, 14, 13 except 2=1172(LC 1), 19=2587(LC 1), 19=2587(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-4=-1889/1867, 4-6=-1163/1329, 6-7=-1133/1476, 7-8=-433/696, 8-9=-841/1172, 9-11=-1009/1207, 11-12=-390/474 2-22=-1603/1633, 21-22=-1603/1633, 20-21=-392/794, 19-20=-215/556, 18-19=-346/410, TOP CHORD

BOT CHORD

17-18=-346/410, 16-17=-346/410, 15-16=-346/410, 14-15=-346/410, 13-14=-346/410,

12-13=-346/410

WEBS

4-22=-250/333,4-21=-770/764, 6-21=-356/286, 7-21=-938/584, 7-20=-471/451, 8-20=-1091/1003, 8-19=-1899/1683, 9-19=-389/319, 11-19=-755/818, 11-16=-356/467

NOTES-

- 1) 2-pty truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

- 4) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Provide adequate drainage to prevent water ponding.
- 7) All plates are 2x4 MT20 unless otherwise indicated.

8) Gable studs spaced at 2-0-0 oc.

Continued us beginned for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

No 39380

No 39380

No Albani PE No.39380

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 9-7-12 oc bracing: 2-22

2-0-0 oc purlins (6-0-0 max.): 7-8.

10-0-0 oc bracing: 21-22,20-21.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

June 3,2019

Marking - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stablity and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, eraction and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Pły	O'Quinn Residence	
			İ		T1722	27312
1559185	T17G	GABLE GABLE L GABLE	1	2		
				<u> </u>	Job Reference (optional)	
Buiders FirstSource, La	ake City, FL		8	240 s May	13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 32 2019 Page	2

ID BVHjtU?4L8iDBBHQFc123Azlu1B-E7a4lkxo1Jr6BIGBcvP1TSSCMxK4Qx5yTOKrUUz9zzv

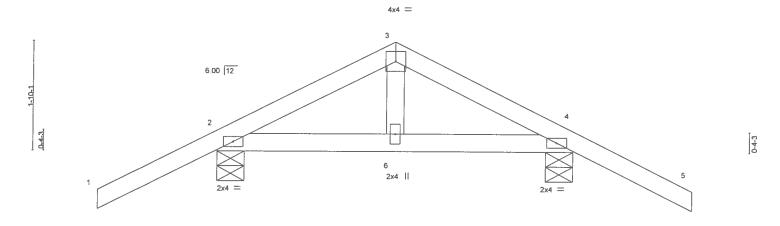
NOTES-

- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17, 15, 13 except (jt=lb) 2=424, 19=963, 12=104, 16=466
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Job Truss Truss Type Qty O'Quinn Residence T17227313 1559185 T18 Common Job Reference (optional) Buiders FirstSource, Lake City, FL 8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun. 3 13 31 33 2019 Page 1 ID BVHjtU?4L8iDBBHQFc123Azlu1B-iK8Sz4yQodzzpSqNAdwG?f?PDLkZ9cv6i24O1wz9zzu -2-0-0 2-0-0 5-11-8 2-11-12 7-11-8 2-0-0 2-11-12

Scale = 1:18.7



	 	2-11-12 2-11-12	5-11-8 2-11-12	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-1 Plate Grip DOL 1,2: Lumber DOL 1,2: Rep Stress Incr YES Code FBC2017/TPI2014	5 TC 0.33 5 BC 0.09 S WB 0.05	DEFL. in (loc) l/defi L/d Vert(LL) -0.00 6 >999 240 Vert(CT) -0.00 4-6 >999 180 Horz(CT) 0.00 4 n/a n/a	PLATES GRIP MT20 244/190 Weight 28 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 5-11-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=354/0-5-8, 4=354/0-5-8

Max Horz 2=-29(LC 10)

Max Uplift 2=-68(LC 12), 4=-68(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

June 3,2019

MARNING - Verify design perameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters whom, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent occlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Truss	Truss Type	Qty	Ply	O'Quinn Residence
				T17227314
T18G	Common Supported Gable	1	1	
				Job Reference (optional)
Lake City, FL	-	8	240 s May	13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 33 2019 Page 1
	ID	BVHjtU?4L8i	DBBHQFc	123Azlu1B-iK8Sz4yQodzzpSqNAdwG?f?Q8Lkd9cN6i24O1wz9zzu
-2-0-0	2-11-12		5-11-	B 7-11-8
2-0-0	2-11-12		2-11-1	2 2-0-0
	T18G Lake City, FL -2-0-0	T18G Common Supported Gable Lake City, FL ID -2-0-0 2-11-12	T18G Common Supported Gable 1 Lake City, FL 8 ID BVHjtU?4L8i -2-0-0 2-11-12	T18G Common Supported Gable 1 1 Lake City, FL 8.240 s May ID:BVHjtU?4L8iDBBHQFc -2-0-0 2-11-12 5-11-1

Scale = 1 19.3

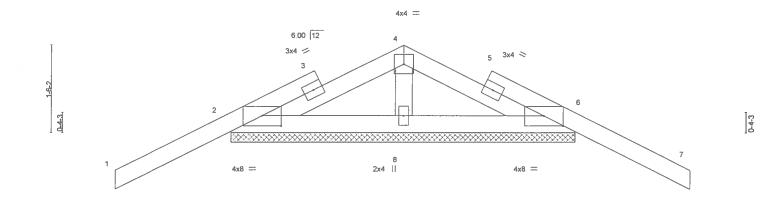


Plate Offsets (X,Y)- [2:0-4-0.0-2-1], [6:0-4-0.0-2-1]					5-11-8						53	
LOADING (ps		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20	,	Plate Grip DOL	1.25	TC TC	0.27	Vert(LL)	-0.02	7	n/r	120	MT20	244/190
TCDL 10	0	Lumber DOL	1.25	BC	0.09	Vert(CT)	-0.03	7	n/r	120		
BCLL 0	.0 *	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	6	n/a	n/a		
BCDL 10	.0	Code FBC2017/TF	PI2014	Matrix	c-P						Weight: 30 lb	FT = 20%

5-11-8

BRACING-TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 **OTHERS**

2x4 SP No.3

(lb/size) 2=265/5-11-8, 6=265/5-11-8, 8=187/5-11-8 Max Horz 2=39(LC 12)

Max Uplift 2=-115(LC 12), 6=-121(LC 13), 8=-24(LC 12) Max Grav 2=267(LC 23), 6=267(LC 24), 8=187(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat, II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 2=115, 6=121.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



Structural wood sheathing directly applied or 5-11-8 oc purlins, Rigid ceiling directly applied or 6-0-0 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MTTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and propenty damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Comp. Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



Job Truss Truss Type Qty O'Quinn Residence T17227315 1559185 V1A Valley Job Reference (optional)
8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 36 2019 Page 1 Lake City, FL Buiders FirstSource, ID.BVHjtU?4L8iDBBHQFc123Azlu1B-6vpab6_J5YLYgvZyrlTzdlcv1Yj_MwvYO?l2dFz9zzr Scale = 1 50.9 6 6.00 12 5 3x6 / X 3x4 / 13 12 10 3x6 = 20-2-8 LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) I/defi L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.35 Vert(LL) 999 n/a n/a MT20 244/190 TCDL 10.0 Lumber DOL 1.25 вс 0.24 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.23 Horz(CT) -0.00 8 n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-S Weight: 109 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, BOT CHORD 2x4 SP No.2 except end verticals. 2x4 SP No.3 WEBS **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing. 2x4 SP No.3 OTHERS WEBS 1 Row at midpt 7-8, 6-9 REACTIONS. All bearings 20-2-0. (lb) -Max Horz 1=281(LC 12) Max Uplift All uplift 100 lb or less at joint(s) 8, 12, 10, 9 except 13=-124(LC 12) Max Grav

All reactions 250 lb or less at joint(s) 8, 1 except 12=303(LC 19), 13=458(LC 1), 10=445(LC 19), 9=307(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-444/180, 2-4=-305/117

2-13=-332/266, 5-10=-257/208

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 2) All plates are 2x4 MT20 unless otherwise indicated.
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 12, 10, 9 except (jt=lb) 13=124.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019

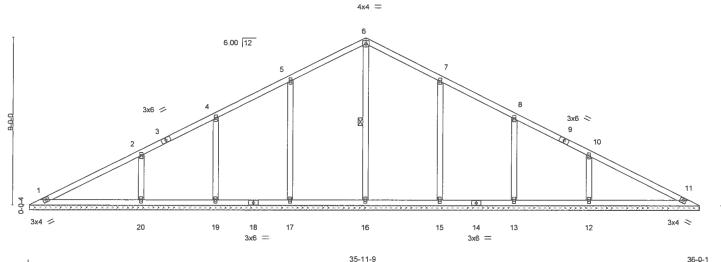
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTEN® connectors. This design is based only upon parameters and properly incorporate this design is based only upon parameters and properly incorporate this design into the overall building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see.

ASI/TPT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Surie 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence
					T17227316
1559185	V2	Valley	1	1	
					Job Reference (optional)
Buiders FirstSource,	Lake City, FL 8 240 s May 13 2019 MiTek Industries. Inc. Mon Jun. 3 13 31 37 2019 Page 1				
	ID.BVHjtU?4L8iDBBHQFc123Azju1B-b5NzpR?xssTPI388PS_CAV94Gy3L5N3idf2cAn				123Azlu1B-b5NzpR?xssTPI388PS_CAV94Gy3L5N3idf2cAhz9zzq
		18-0-0	1		36-0-1
		18-0-0			18-0-0

Scale = 1:59 6



	35-11-9						0-0-8					
LOADIN TCLL	G (psf) 20.0	SPACING- Plate Grip DOL	2-0-0 1.25	CSI.	0.32	DEFL. Vert(LL)	in n/a	(loc)	l/defl n/a	L/d 999	PLATES MT20	GRIP 244/190
TCDL BCLL	10.0 0.0 *	Lumber DOL Rep Stress Incr	1.25 YES	BC WB	0.23 0.23	Vert(CT) Horz(CT)	n/a 0.01	- 11	n/a n/a	999 n/a		
BCDL	10.0	Code FBC2017/TP		Matri	x-S	' '					Weight: 163 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

OTHERS 2x4 SP No.3

REACTIONS. All bearings 35-11-1. (lb) - Max Horz 1=-100(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 17, 19, 15, 13 except 20=-122(LC 12), 12=-122(LC 13)

All reactions 250 lb or less at joint(s) 1, 11 except 16=379(LC 22), 17=444(LC 19), 19=306(LC 19), 20=451(LC 23), 15=444(LC 20), 13=306(LC 20), 12=451(LC 24) Max Grav

TOP CHORD

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

5-6=-119/264, 6-7=-119/264 WEBS 5-17=-267/211, 2-20=-328/261, 7-15=-267/211, 10-12=-328/261

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 17, 19, 15, 13 except (jt=lb) 20=122, 12=122.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

6-16

Rigid ceiling directly applied or 10-0-0 oc bracing.

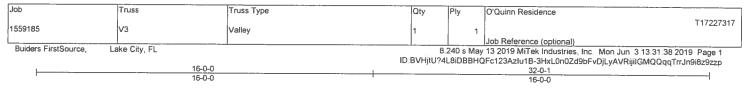
1 Row at midpt

Date:

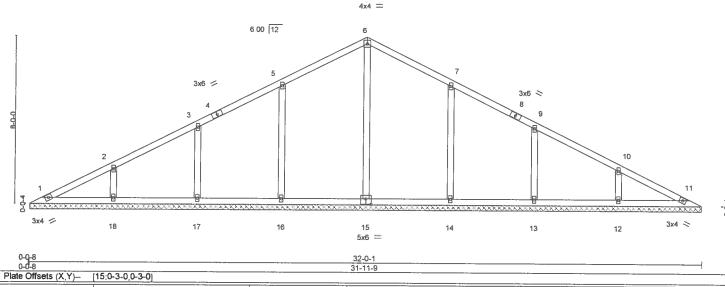
June 3.2019

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Scale = 1:53.0



LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) I/defl L/d **PLATES** GRIP TCLL 20 0 Plate Grip DOL 1.25 TC 0.18 Vert(LL) 999 n/a n/a MT20 244/190 TCDL 10.0 Lumber DOL 1 25 ВC 0.18 Vert(CT) n/a 999 n/a BCLL 0.0 Rep Stress Incr YES WB 0.22 Horz(CT) 0.00 11 n/a n/a Code FBC2017/TPI2014 BCDL 10.0 Matrix-S Weight: 140 lb FT = 20% BRACING-

TOP CHORD

BOT CHORD

LUMBER.

TOP CHORD 2x4 SP No.2 2x4 SP No.2

BOT CHORD

2x4 SP No.3 OTHERS

REACTIONS. All bearings 31-11-1.

(lb) -Max Horz 1=89(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 16, 17, 18, 14, 13, 12

All reactions 250 lb or less at joint(s) 1, 11 except 15=376(LC 22), 16=433(LC 19), 17=345(LC 19), Max Grav 18=330(LC 23), 14=433(LC 20), 13=345(LC 20), 12=330(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown WEBS 5-16=-260/206, 7-14=-260/206

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 16, 17, 18, 14, 13, 12.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Date:

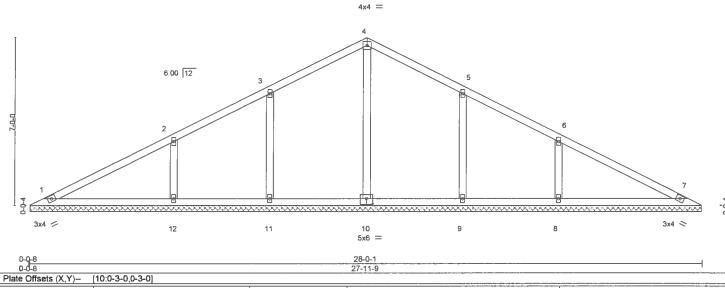
June 3,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information.



Truss Truss Type Qty O'Quinn Residence T17227318 1559185 V4 Valley Job Reference (optional) 8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 40 2019 Page 1 Buiders FirstSource Lake City, FL ID.BVHjtU?4L8iDBBHQFc123Azlu1B-?g35RT1p9nrz9Xtj4bYvn8nbW952lke8JdGGm0z9zzn 28-0-1 14-0-0 14-0-0 14-0-0

Scale = 1 46 4



PLATES GRIP LOADING SPACING-I/defl Ľ∕d (psf) (loc) TCLL 20.0 Plate Grip DOL 1.25 TC 0.32 Vert(LL) n/a n/a 999 MT20 244/190 RC. TCDL 10.0 Lumber DOL 1.25 0.23 Vert(CT) n/a n/a 999 **BCLL** WB 0.0 Rep Stress Incr YES 0.18 Horz(CT) 0.00 n/a n/a Code FBC2017/TPI2014 BCDL Weight: 116 lb FT = 20% 10.0 Matrix-S

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

OTHERS 2x4 SP No.3

REACTIONS. All bearings 27-11-1.

Max Horz 1=-77(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 11, 9 except 12=-120(LC 12), 8=-120(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=406(LC 19), 11=321(LC 19), 12=447(LC 1),

9=321(LC 20), 8=447(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-12=-323/259, 6-8=-323/259 WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 11, 9 except (jt=lb) 12=120, 8=120.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 3,2019

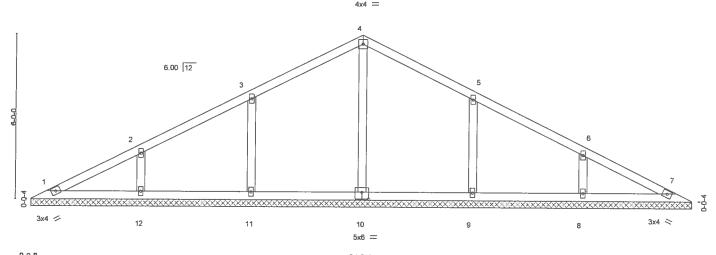
🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent tracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



| Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Truss | Trus

Scale = 1:40.4



24-0-1 Plate Offsets (X,Y)-[10:0-3-0,0-3-0] LOADING (psf) SPACING-2-0-0 CSI. **DEFL** in (loc) I/defl 1 /d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.18 Vert(LL) 999 n/a n/a MT20 244/190 TCDL 10.0 Lumber DOL 1.25 ВĊ 0.19 Vert(CT) n/a 999 n/a BCLL 0.0 Rep Stress Incr YES WB 0.12 Horz(CT) 0.00 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-S Weight: 96 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

OTHERS 2x4 SP No.3

ONS. All bearings 23-11-1.
(lb) - Max Horz 1=-66(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 11, 12, 9, 8

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=387(LC 19), 11=361(LC 19), 12=325(LC 1), 9=361(LC 20), 8=325(LC 1)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

WEBS 3-11=-260/207, 5-9=-260/207

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11, 12, 9, 8.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Bivd, Tampa FL 33610 Date:

June 3.2019



Job Truss Truss Type Qty O'Quinn Residence T17227320 1559185 V6 Valley | Job Reference (optional) 8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 42 2019 Page 1 Buiders FirstSource Lake City, FL ID BVHjtU?4L8iDBBHQFc123Azlu1B-x2Bss934hO5hOq06B0aNtZsxwzmWmffRmxlNrvz9zzi 10-0-0 10-0-0 20-0-1 10-0-0 Scale = 1:33.7 4x4 = 3 6.00 12 2x4 || 2x4 || 3x4 = 6 2x4 [] 5x6 = 2x4 || 0-0-8 0-0-8 19-11-9 Plate Offsets (X,Y)-[7:0-3-0,0-3-0] LOADING (psf) SPACING-2-0-0 CSI DEFL (loc) l/defi **PLATES** GRIP Ľd TCLL 20.0 Plate Grip DOL TC BC 1 25 0.33 Vert(LL) n/a n/a 999 MT20 244/190 TCDL 10.0 1.25 Lumber DOL 0.23 Vert(CT) n/a n/a 999 **BCLL** 0.0 Rep Stress Incr YES WB 0.08 Horz(CT) 0.00 5 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-S Weight: 75 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 **OTHERS** 2x4 SP No.3 **BRACING-**

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS.

All bearings 19-11-1.

Max Horz 1=54(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-128(LC 12), 6=-128(LC 13) All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=468(LC 23), 6=468(LC 24)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES.

WEBS

2-8=-343/271, 4-6=-343/271

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=128, 6=128.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610

June 3,2019

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Job Truss Truss Type Qty O'Quinn Residence T17227321 1559185 **V**7 Valley 1 Job Reference (optional) 8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 43 2019 Page 1 Buiders FirstSource Lake City, FL ID:BVHjtU?4L8iDBBHQFc123Azlu1B-PFIE3V3iSiDY0_bllj5cPmP9zN8eV7Aa?bVwNLz9zzk 16-0-1 8-0-0 Scale = 1.27.2 3 6.00 12 2x4 || 2x4 || В 6 3x4 🗢 2x4 || 2x4 || 2x4 || 15-11-9 LOADING (psf) SPACING-2-0-0 CSI. DEFL (loc) l/defl Ľ∕d **PLATES** GRIP TC BC TCLL 20.0 Plate Grip DOL 1.25 0.18 Vert(LL) n/a n/a 999 MT20 244/190 TCDL 10.0 Lumber DOL 1.25 0.11 Vert(CT) n/a n/a 999

LUMBER-

BCLL

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

0.0

10.0

OTHERS 2x4 SP No.3 BRACING.

Horz(CT)

0.00

5

n/a

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Weight: 58 lb

FT = 20%

Rigid ceiling directly applied or 10-0-0 oc bracing.

n/a

REACTIONS. All bearings 15-11-1.

(lb) -Max Horz 1=43(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 8, 6

Code FBC2017/TPI2014

Rep Stress Incr

All reactions 250 lb or less at joint(s) 1, 5 except 7=271(LC 1), 8=349(LC 23), 6=349(LC 24)

WB 0.06

Matrix-S

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown.

WEBS

2-8=-263/213, 4-6=-263/213

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10, Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

YES

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

June 3,2019

⚠ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design, Bracting indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312. Alexandria, VA 22314.



Job Truss Truss Type Qty O'Quinn Residence T17227322 1559185 V8 Valley Job Reference (optional)
8.240 s May 13 2019 MiTek Industries, Inc Mon Jun 3 13 31 44 2019 Page 1 Buiders FirstSource, Lake City, FL ID.BVHjtU?4L8iDBBHQFc123Azlu1B-tRlcHr4KD?LPd8AUJQcry_yHXnR5EZMjEFETvoz9zzj 12-0-1 Scale = 1:20 5 4x6 = 6.00 12 3x4 / 3x4 > 2x4 | 12-0-1 11-11-9 LOADING (psf) SPACING-CSI. **PLATES** GRIP DEFL 2-0-0 in I/defl L/d (loc) 20.0 Plate Grip DOL TC 244/190 TCLL 1.25 0.38 Vert(LL) 999 MT20 n/a n/a TCDL 10,0 Lumber DOL 1.25 ВC 0.29 Vert(CT) n/a n/a 999 **BCLL** 0.0 Rep Stress Incr YES WR 0.07 Horz(CT) 0.00 3 n/a n/a FT = 20% Code FBC2017/TPI2014 Weight: 39 lb BCDL 100 Matrix-S BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2

BOT CHORD 2x4 SP No.3 OTHERS

REACTIONS. (lb/size) 1=196/11-11-1, 3=196/11-11-1, 4=468/11-11-1

Max Horz 1=31(LC 9)

Max Uplift 1=-38(LC 12), 3=-44(LC 13), 4=-43(LC 12) Max Grav 1=198(LC 23), 3=198(LC 24), 4=468(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft, Cat. II; Exp C, Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4



Structural wood sheathing directly applied or 6-0-0 oc purlins

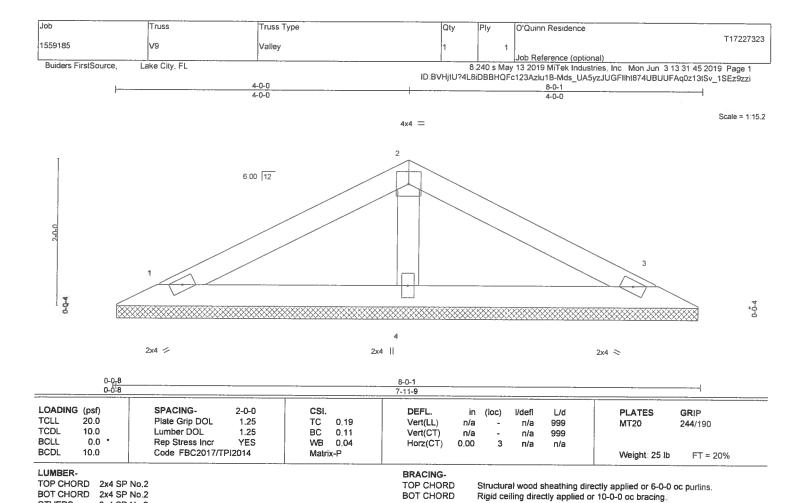
Rigid ceiling directly applied or 10-0-0 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

June 3,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-1473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314





REACTIONS.

OTHERS

(lb/size) 1=137/7-11-1, 3=137/7-11-1, 4=266/7-11-1

Max Horz 1=20(LC 9)

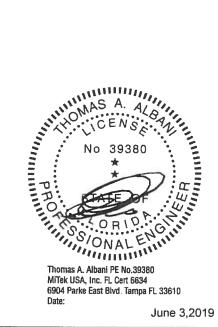
2x4 SP No.3

Max Uplift 1=-29(LC 12), 3=-33(LC 13), 4=-16(LC 12)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown,

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

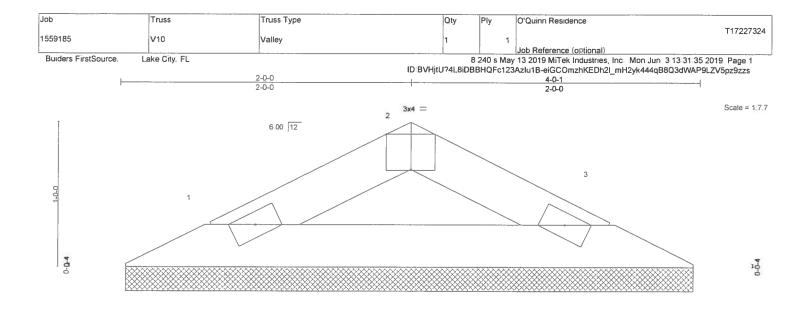


June 3,2019

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rey, 10/03/2015 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters and wown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria. VA 22314.





2x4 /

2x4 <

Structural wood sheathing directly applied or 4-0-1 oc purlins

Rigid ceiling directly applied or 10-0-0 oc bracing.

0-0-8 0-0-8 Plate Offsets (X,Y)— [2:0-2-0.Edge]								
le]								
CING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
Grip DOL 1.25	TC 0.04	Vert(LL)	n/a	` _	n/a	999	MT20	244/190
per DOL 1.25	BC 0.09	Vert(CT)	n/a	-	n/a	999		
Stress Incr YES	WB 0.00	Horz(CT)	0.00	3	n/a	n/a		
FBC2017/TPI2014	Matrix-P	, ,					Weight: 10 lb	FT = 20%
e ab	e Grip DOL 1.25 ber DOL 1.25	ACING- 2-0-0 CSI. e Grip DOL 1.25 TC 0.04 tiber DOL 1.25 BC 0.09 Stress Incr YES WB 0.00	ACING- 2-0-0 CSI. DEFL. e Grip DOL 1.25 TC 0.04 Vert(LL) tiber DOL 1.25 BC 0.09 Vert(CT) Stress Incr YES WB 0.00 Horz(CT)	3-11-9	3-11-9	3-11-9	3-11-9	3-11-9

BRACING-

TOP CHORD BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

(lb/size) 1=110/3-11-1, 3=110/3-11-1

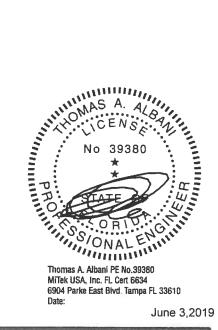
Max Horz 1=8(LC 11)

Max Uplift 1=-15(LC 12), 3=-15(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



June 3,2019

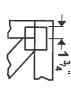
🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev., 10/03/2015 BEFORE USE Design valid for use only with MTEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Bratcing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see. ANS/TPF1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.



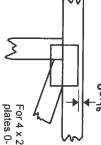
6904 Parke East Blvd Tampa, FL 36610

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y and fully embed teeth. Apply plates to both sides of truss offsets are indicated Dimensions are in ft-in-sixteenths



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

required direction of slots in This symbol indicates the

connector plates.

Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE



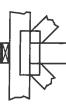
the length parallel to slots to slots. Second dimension is The first dimension is the plate width measured perpendicular

LATERAL BRACING LOCATION



by text in the bracing section of the output. Use T or I bracing if indicated. Indicated by symbol shown and/or

BEARING



Min size shown is for crushing only. reaction section indicates joint Indicates location where bearings number where bearings occur. (supports) occur. Icons vary but

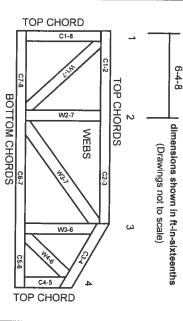
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.

DSB-89: Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Building Component Safety Information,

Connected Wood Trusses

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values established by others. Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer, For wide truss spacing, individual lateral braces themselves bracing should be considered. may require bracing, or alternative Tor I

.2

Never exceed the design loading shown and never stack materials on inadequately braced trusses.

ω

Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.

4

Cut members to bear tightly against each other.

6

G

6 Ċ

- Place plates on each face of truss at each locations are regulated by ANSI/TPI 1. joint and embed fully. Knots and wane at joint
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.

.7

- Unless otherwise noted, moisture content of tumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- 10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing. or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- 19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

ENGINEERED BY

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

1	Nailing Pattern	
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.

Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

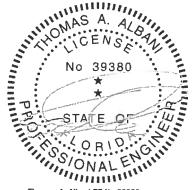
	for One-Ply Truss					
		Continuous iteral Bracing				
Web Size	1	2				
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace				
2x6	2x6 T-Brace	2x6 I-Brace				
2x8	2x8 T-Brace 2x8 I-Brace					

,, \	Nails
WEB	SPACING
	T-BRACE
Nails Section Detail T-Brace Web	

Nails	
Web	I-Brace
Nails	

		Brace Size for Two-Ply Truss					
		Continuous steral Bracing					
Web Size	1	2					
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace					
2x6	2x6 T-Brace	2x6 I-Brace					
2x8	2x8 T-Brace 2x8 I-Brace						

T-Brace / I-Brace must be same species and grade (or better) as web member.



Thomas A. Albani PE No. 39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

SCAB-BRACE DETAIL

MII-SCAB-BRACE

MiTek USA, Inc.

Page 1 of 1

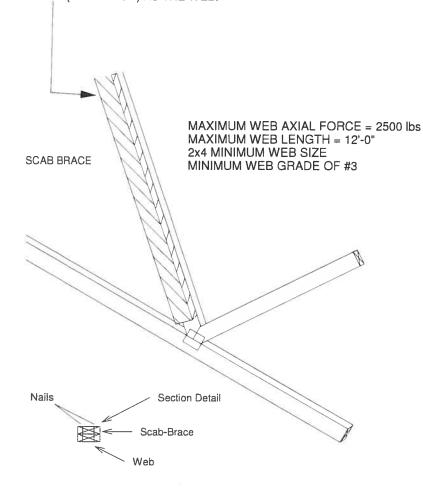


Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.

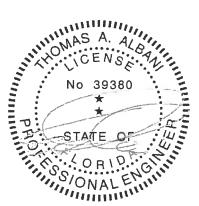
Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x____SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.

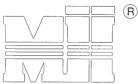


Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc. ENGINEERED BY

- 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION, THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
- THE LOADS INDICATED.

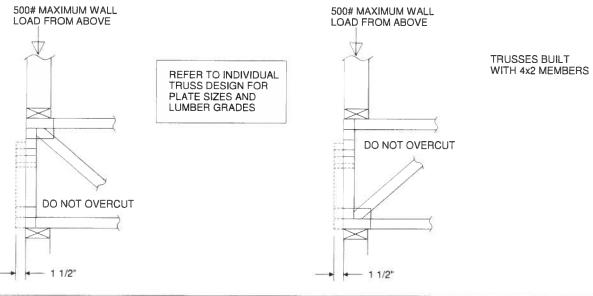
 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

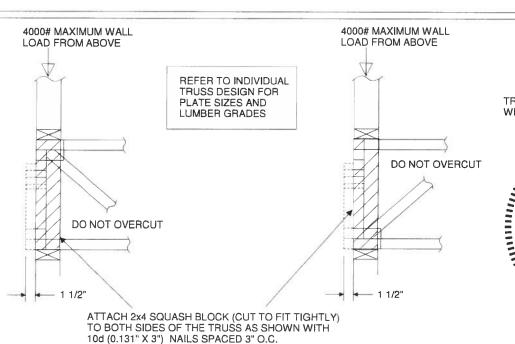
 3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.

 4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.

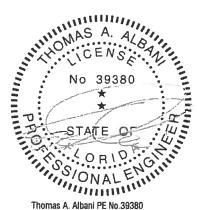
 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ORIENTATION ONLY.

 6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.

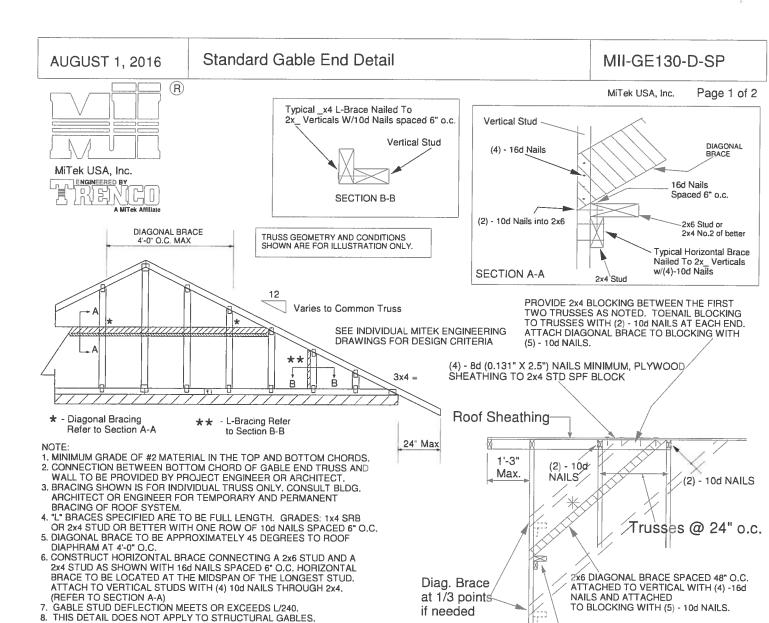




TRUSSES BUILT WITH 4x2 MEMBERS



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:



End Wall

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade			Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7			
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13			
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

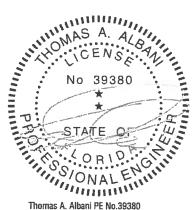
 DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
 SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH DURATION OF LOAD INCREASE : 1.60

06-01-13 BY SPIB/ALSC.

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



HORIZONTAL BRACE (SEE SECTION A-A)

Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

Standard Gable End Detail

MII-GE130-SP



Page 1 of 2

(2) - 10d NAILS

russes @ 24" o.c.



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DIAGONAL BRACE

Typical _x4 L-Brace Nailed To 2x_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

24

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY 4'-0" O.C. MAX Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA ** 3x4 = Ř B

Vertical Stud (4) - 16d Nails DIAGONAL BRACE 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A 2x4 Stud

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD

SHEATHING TO 2x4 STD SPF BLOCK

★ - Diagonal Bracing Refer to Section A-A

- L-Bracing Refer to Section B-B

NOTE

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
- CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
 BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.
- ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
 "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF
- DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4.
- (REFER TO SECTION A-A)
 GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE
- 06-01-13 BY SPIB/ALSC. 11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Roof Sheathi	ng—
4" Max 1'-3"	
Max.	(2) - 10d NAILS
	Trusses @ 24" o.
Diag. Brace at 1/3 points if needed	2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.
End Wall	HORIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade		Maximum Stud Length							
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6			
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1			
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15			

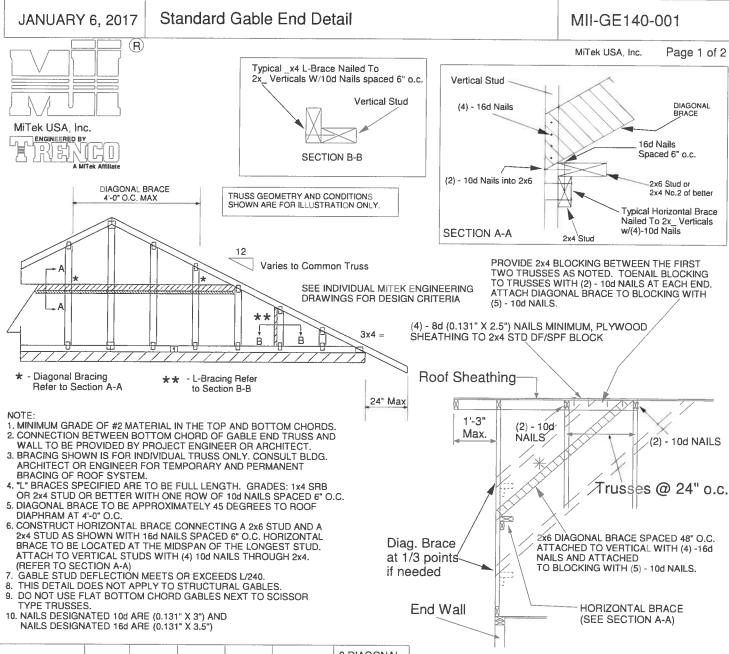
Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610



Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS	
and Grade			Maximum Stud Length				
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4	
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11	
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12	

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

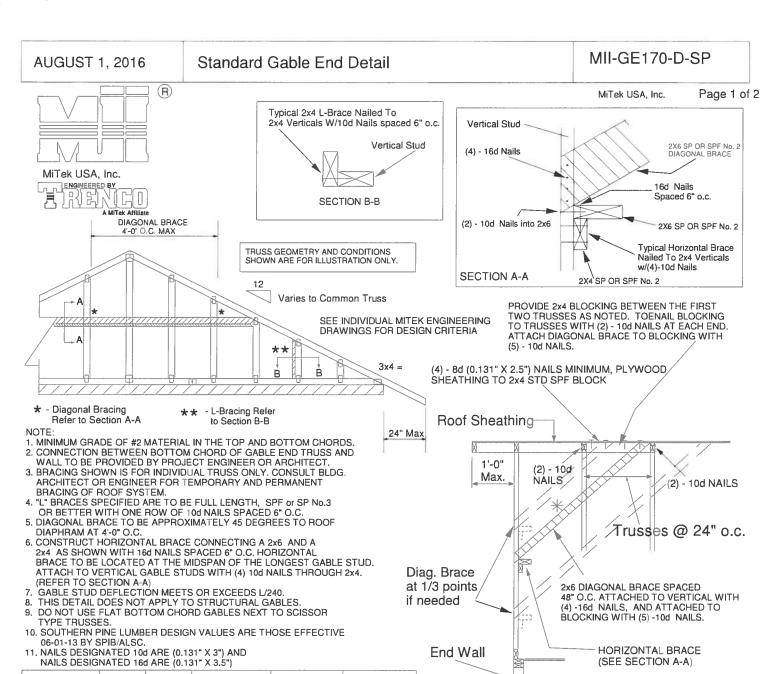
MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 19, 2018

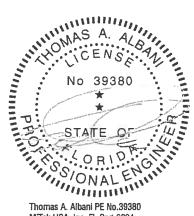


Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4			
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3			
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13			
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7			
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5			
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 170 MPH DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

AUGUST 1, 2016 Standard Gable End Detail (R) Typical 2x4 L-Brace Nailed To 2x4 Verticals W/10d Nails spaced 6" o.c. Vertical Stud MiTek USA, Inc. ENGINEERED BY 別點 SECTION B-B A MITEK Affiliate DIAGONAL BRACE 4'-0" O.C. MAX TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

MII-GE180-D-SP

MiTek USA, Inc. Page 1 of 2

w/(4)-10d Nails

(2) - 10d NAILS

Vertical Stud 2X6 SP OR SPF No. 2 DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2X6 SP OR SPF No. 2 Typical Horizontal Brace Nailed To 2x4 Verticals

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH

2X4 SP OB SPF No. 2

(5) - 10d NAILS.

(2) - 10d

NAILS

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

SECTION A-A

Roof Sheathing

1'-0'

Max.

3x4 =

24" Max

- Diagonal Bracing

Refer to Section A-A

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT

**

- L-Bracing Refer

to Section B-B

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT

BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3
OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

5. DIAGONAL BHACE TO BE APPHOXIMATELT 43 DEGREES TO TICO.
DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A
2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL
BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD.
ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3			
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7			
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4			
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1			
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7			
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

∕Trusses @ 24" o.c. Diag. Brace at 1/3 points 2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH if needed (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS. HORIZONTAL BRACE End Wall (SEE SECTION A-A)

No 39380

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Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

MiTek USA, Inc. Page 1 of 1

(R)

MiTek USA, Inc.

ENGINEERED BY RR

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C

DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

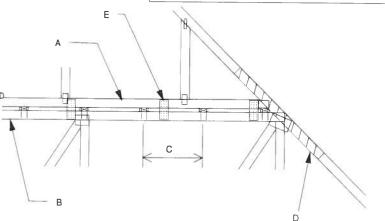
A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) (0.131" X 3.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - 2 X _ X 4"-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED.
ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND: DIRECTIONS AND:

DIRECTIONS AND:

1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

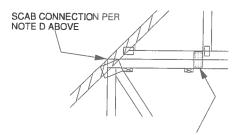
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM
PIGGYBACK SPAN OF 12 It.

E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH
MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT
72"O.C. W! (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS
FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE.
(MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

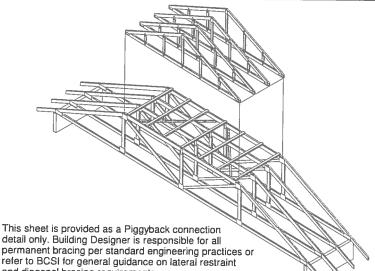


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-ON PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

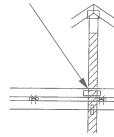


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48° O.C. W/ (4) (0.131° X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

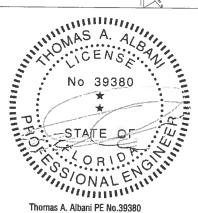
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP

MUST MATCH IN SIZE, GRADE, AND MUST LINE UP
AS SHOWN IN DETAIL.
ATTACH 2 x x 4°0° SCAB TO EACH FACE OF
TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131° X 3°) NAILS
SPACED 4°0.C, FROM EACH FACE. (SIZE AND GRADE TO MATCH
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)
(MINIMUM 2X4)
THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW
BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS
GREATER THAN 4000 LBS

GREATER THAN 4000 LBS

FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH

THE PIGGYBACK AND THE BASE TRUSS DESIGN.

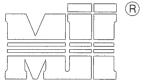


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STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT 7-10

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

ENGINEERED BY /[HI

A MTek Affiliate

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) (0(1.31" X 3.5") TOE-NAILED.

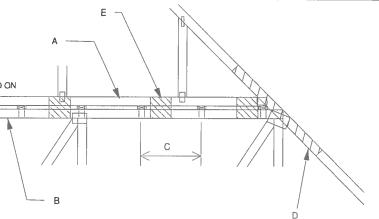
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - 2 X _ 4 -0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON
INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND:

1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM
PIGGYBACK SPAN OF 12 ft.
E - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH
ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET
EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH
3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM
EACH SIDE (TOTAL - 12 NAILS)

ASCE 7-10 **DURATION OF LOAD INCREASE: 1.60** DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING

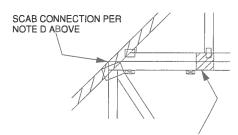
MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E



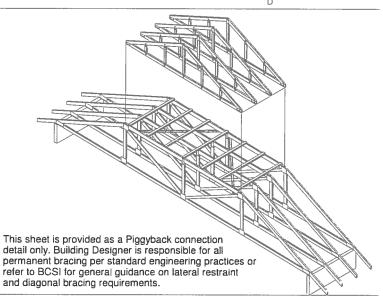
EXPOSURE B or C

WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS
 MUST MATCH IN SIZE, GRADE, AND MUST LINE UP
 AS SHOWN IN DETAIL.
 2) ATTACH 2 × 4'-0" SCAB TO EACH FACE OF
 TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
 SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
- VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)
 (MINIMUM 2X4)
 THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
 CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW
 BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS
 GREATER THAN 4000 LBS.

FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH

THE PIGGYBACK AND THE BASE TRUSS DESIGN.

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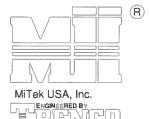
January 19, 2018

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

MiTek USA, Inc.

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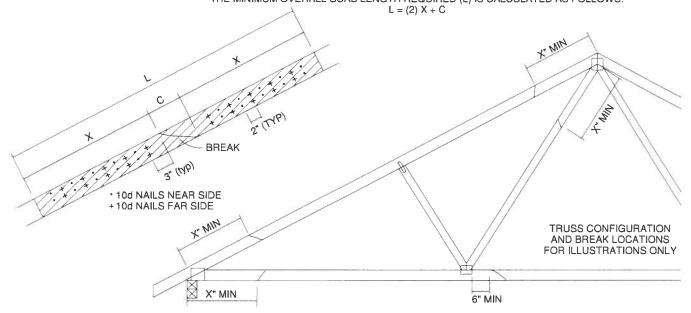


TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *			MAXIMUM FORCE (lbs) 15% LOAD DURATION							
		X INCHES	SP		DF		SPF		HF	
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_ SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN.
STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

NOTES:

- NOTES:

 1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

 3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID LINIUS AND TITING OF THE WOOD.

- THE END DISTANCE, EDGE DISTANCE AND SPACING OF MAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
 WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
 THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x_ ORIENTATION ONLY.
 THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



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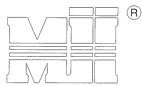
January 19, 2018

LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP

MiTek USA, Inc.

Page 1 of 1



MiTek USA, Inc.



NOTES:

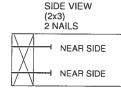
- 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.

 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

 3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES
- FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY



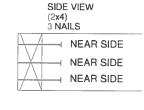
OE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail) DIAM. SP SPF-S .131 LONG 88.0 69.9 80.6 68.4 59.7 .135 93.5 85.6 74.2 63.4 72.6 108.8 99.6 86.4 84.5 73.8 3.5 .162 LONG .128 74.2 57.6 50.3 67.9 58.9 75.9 69.5 60.3 59.0 51.1 .131 3.25" 81.4 74.5 64.6 63.2 148 52.5

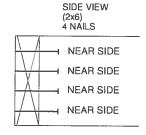
VALUES SHOWN ARE CAPACITY PER TOE-NAIL. APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

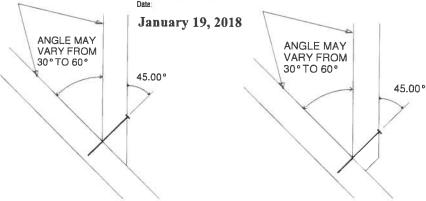
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

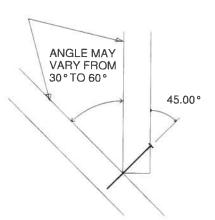






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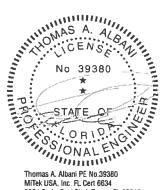


TRUSSED VALLEY SET DETAIL MII-VALLEY HIGH WIND1 AUGUST 1, 2016 R MiTek USA, Inc. Page 1 of 1 **GENERAL SPECIFICATIONS** 1. NAIL SIZE 10d (0.131" X 3") 2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
SECURE PER DETAIL A MiTek USA, Inc. ENGINEERED BY 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE H I GABLE END, COMMON TRUSS INDIVIDUAL DESIGN DRAWINGS. OR GIRDER TRUSS A MiTek Affiliate 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING. NAILING DONE PER NDS - 01 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C. **BASE TRUSSES** VALLEY TRUSS TYPICAL GABLE END, COMMON TRUSS VALLEY TRUSS TYPICAL 12 OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.) SECURE VALLEY TRUSS WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING W/ ONE ROW OF 10d NAILS 6" O.C. ATTACH 2x4 CONTINUOUS NO.2 SP EXPOSURE C TO THE ROOF W/TWO USP WS3 (1/4" X 3") WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF WOOD SCREWS INTO EACH BASE TRUSS. ON THE TRUSSES

DETAIL A

N.T.S.

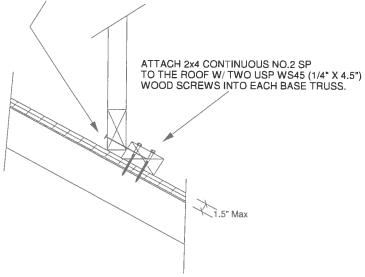
(NO SHEATHING)



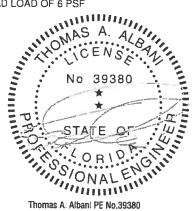
6904 Parke East Blvd. Tampa FL 33610

January 19, 2018

TRUSSED VALLEY SET DETAIL AUGUST 1, 2016 MII-VALLEY HIGH WIND2 R MiTek USA, Inc. Page 1 of 1 GENERAL SPECIFICATIONS 1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT
3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A
5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS. MiTek USA, Inc. GABLE END, COMMON TRUSS OR GIRDER TRUSS ENGINEERED BY 6. NAILING DONE PER NDS-01 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C. BASE TRUSSES VALLEY TRUSS TYPICAL GABLE END, COMMON TRUSS OR GIRDER TRUSS VALLEY TRUSS TYPICAL SEE DETAIL A BELOW (TYP.) SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C. WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/ TWO USP WS45 (1/4" X 4.5") WOOD SCREWS INTO EACH BASE TRUSS.



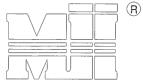
WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES



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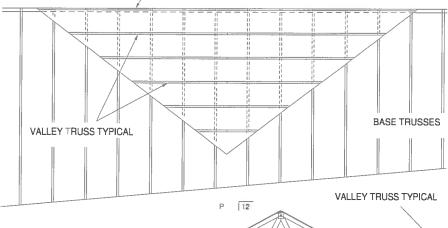
ENGINEERED BY

GABLE END, COMMON TRUSS OR GIRDER TRUSS

GENERAL SPECIFICATIONS

- 1. NAIL SIZE 16d (0.131" X 3.5") 2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.

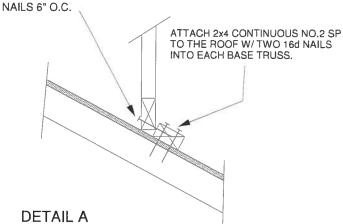
- 5. NAILING DONE PER NDS 01 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C. 7. ALL LUMBER SPECIES TO BE SP.



GABLE END, COMMON TRUSS OR GIRDER TRUSS

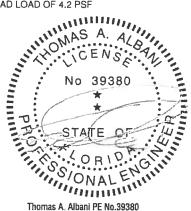
SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 16d



DETAIL A (MAXIMUM 1" SHEATHING) N.T.S.

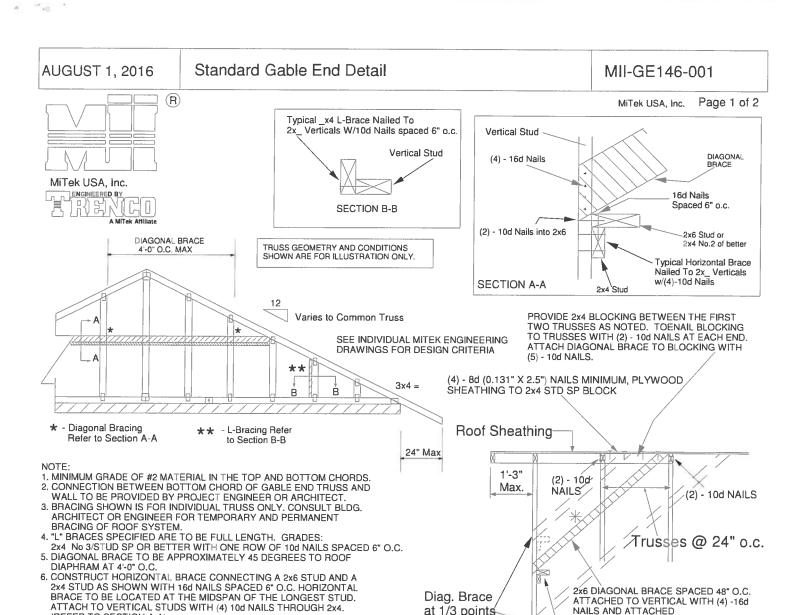
WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10 150 MPH MAX MEAN ROOF HEIGHT \pm 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES



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TRUSSED VALLEY SET DETAIL MII-VALLEY AUGUST 1, 2016 (HIGH WIND VELOCITY) NOTE: VALLEY STUD SPACING NOT R MiTek USA, Inc. Page 1 of 1 TO EXCEED 48" O.C. SPACING MiTek USA, Inc. ENGINEERED BY A MITek Affiliate FOR BEVELED BOTTOM CHORD, CLIP MAY BE APPLIED TO EITHER FACE CLIP MAY BE APPLIED TO THIS FACE UP TO A MAXIMUM 6/12 PITCH ATTACH VALLEY TRUSSES TO LOWER TRUSSES WITH USP RT7 OR EQUIVALENT WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING NON-BEVELED EXPOSURE B or C **BOTTOM CHORD** WIND DURATION OF LOAD INCREASE: 1.6 MAX TOP CHORD TOTAL LOAD = 50 PSF No 39380

STAILE OF THE STAIL O MAX SPACING = 24" O.C. (BASE AND VALLEY) SUPPORTING TRUSSES DIRECTLY UNDER VALLEY TRUSSES MUST BE DESIGNED WITH A MAXIMUM UNBRACED LENGTH OF NON-BEVELED 2'-10" ON AFFECTED TOP CHORDS. **BOTTOM CHORD** NOTES: - SHEATHING APPLIED AFTER **INSTALLATION OF VALLEY TRUSSES** - THIS DETAIL IS NOT APPLICABLE FOR SPF-S SPECIES LUMBER. CLIP MUST BE APPLIED Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 TO THIS FACE WHEN PITCH EXCEEDS 6/12. 6904 Parke East Bivd. Tampa FL 33610 (MAXIMUM 12/12 PITCH) January 19, 2018



at 1/3 points

End Wall

if needed

(REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES.

10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
		Maximum Stud Length					
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10		
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11		
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12-6" require 2x4 l-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS



ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED

HORIZONTAL BRACE

(SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

6904 Parke East Blvd. Tampa FL 33610

January 19, 2018

OCTOBER 5, 2016

REPLACE BROKEN OVERHANG

MII-REP13B

MiTek USA, Inc.

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R

MiTek USA, Inc.

ENGINEERED BY A MITEK Attiliate TRUSS CRITERIA:

LOADING: 40-10-0-10 **DURATION FACTOR: 1.15** SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12

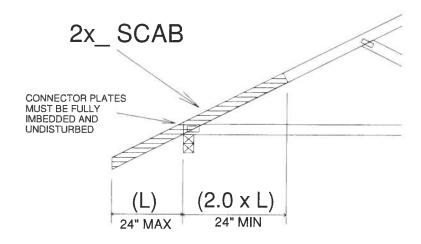
HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

END BEARING CONDITION

NOTES:

ATTACH 2x_SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
 THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
 WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED.

TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf. Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

 $V_{[p]}$

LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK

MiTek USA, Inc.

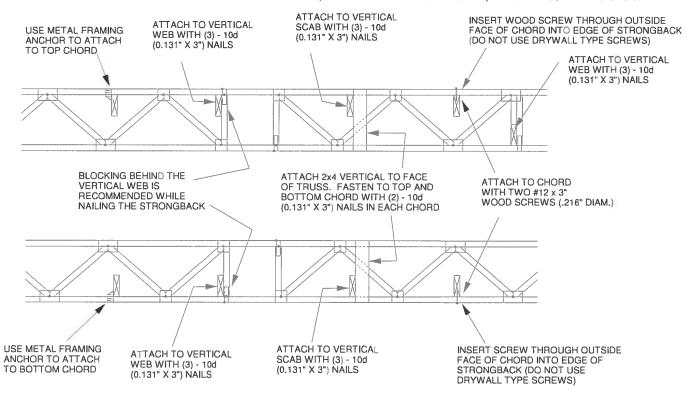
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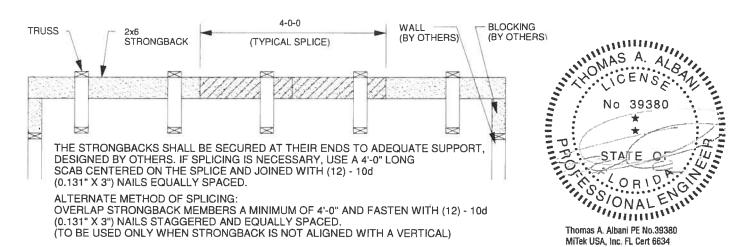


TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





6904 Parke East Blvd, Tampa FL 33610 Date:

