



Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: 1559185 - O'Quinn Residence

MiTek USA, Inc.

6904 Parke East Blvd.
Tampa, FL 33610-4115

Site Information:

Customer Info: Doug Edgley Construction Project Name: 1559185 Model: Custom
Lot/Block: 5 Subdivision: High Pointe Farms
Address:
City: Columbia County State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: unknown at time of seals License #: unknown at time of seals
Address: unknown at time of seals
City: unknown at time of seals State: unknown at time of seals

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014 Design Program: MiTek 20/20 8.2
Wind Code: ASCE 7-10 Wind Speed: 120 mph
Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 41 individual, Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

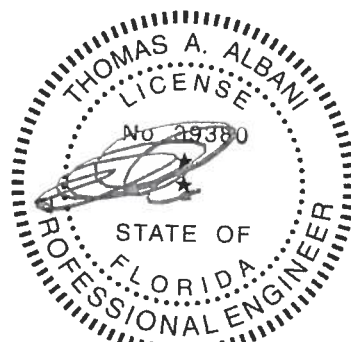
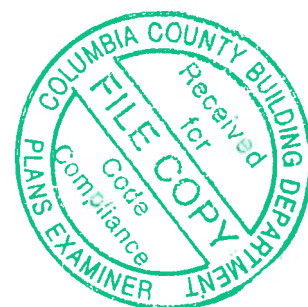
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T17227284	CJ1	6/3/19	23	T17227306	T13	6/3/19
2	T17227285	CJ3	6/3/19	24	T17227307	T14	6/3/19
3	T17227286	CJ5	6/3/19	25	T17227308	T15	6/3/19
4	T17227287	EJ7	6/3/19	26	T17227309	T15A	6/3/19
5	T17227288	EJ7A	6/3/19	27	T17227310	T16A	6/3/19
6	T17227289	HJ9	6/3/19	28	T17227311	T17A	6/3/19
7	T17227290	PB1	6/3/19	29	T17227312	T17G	6/3/19
8	T17227291	PB1G	6/3/19	30	T17227313	T18	6/3/19
9	T17227292	T01	6/3/19	31	T17227314	T18G	6/3/19
10	T17227293	T02	6/3/19	32	T17227315	V1A	6/3/19
11	T17227294	T03	6/3/19	33	T17227316	V2	6/3/19
12	T17227295	T04	6/3/19	34	T17227317	V3	6/3/19
13	T17227296	T05	6/3/19	35	T17227318	V4	6/3/19
14	T17227297	T06	6/3/19	36	T17227319	V5	6/3/19
15	T17227298	T07	6/3/19	37	T17227320	V6	6/3/19
16	T17227299	T08	6/3/19	38	T17227321	V7	6/3/19
17	T17227300	T09	6/3/19	39	T17227322	V8	6/3/19
18	T17227301	T10	6/3/19	40	T17227323	V9	6/3/19
19	T17227302	T10G	6/3/19	41	T17227324	V10	6/3/19
20	T17227303	T11	6/3/19				
21	T17227304	T11G	6/3/19				
22	T17227305	T12	6/3/19				

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Albani, Thomas

My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Thomas A. Albani PE No. 39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

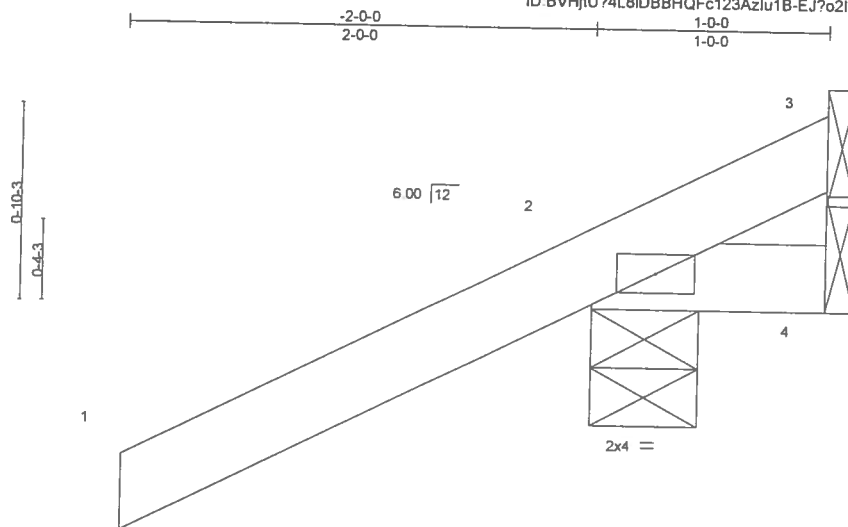
June 3, 2019

Albani, Thomas

1 of 1

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence
1559185	CJ1	Jack-Open	4	1	T17227284
Buiders FirstSource,	Lake City, FL				Job Reference (optional)

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 41 2019 Page 1
ID: BVHjtU74L8iDBBHQFc123Azlu1B-EJ?o2Ijp2eQLaSSN7i_4uirX6N?iz5pSlyUeCxzA_



Scale = 1/9.5

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.28	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.25	BC 0.01	Vert(LL) -0.00 2 >999 240		
BCLL 0.0	Lumber DOL 1.25	WB 0.00	Vert(CT) -0.00 2 >999 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Horz(CT) -0.00 3 n/a n/a		
	Code FBC2017/TPI2014			Weight: 7 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

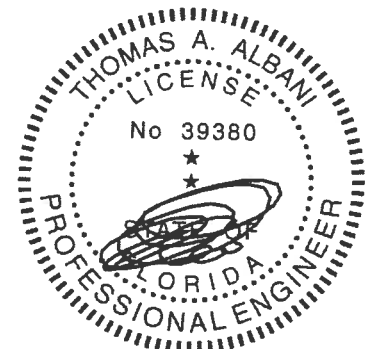
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=105/Mechanical, 2=294/0-5-8, 4=9/Mechanical
Max Horz 2=37(LC 12)
Max Uplift 3=105(LC 1), 2=76(LC 12)
Max Grav 3=42(LC 8), 2=294(LC 1), 4=19(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (it=lb) 3=105.



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Date:

June 3,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

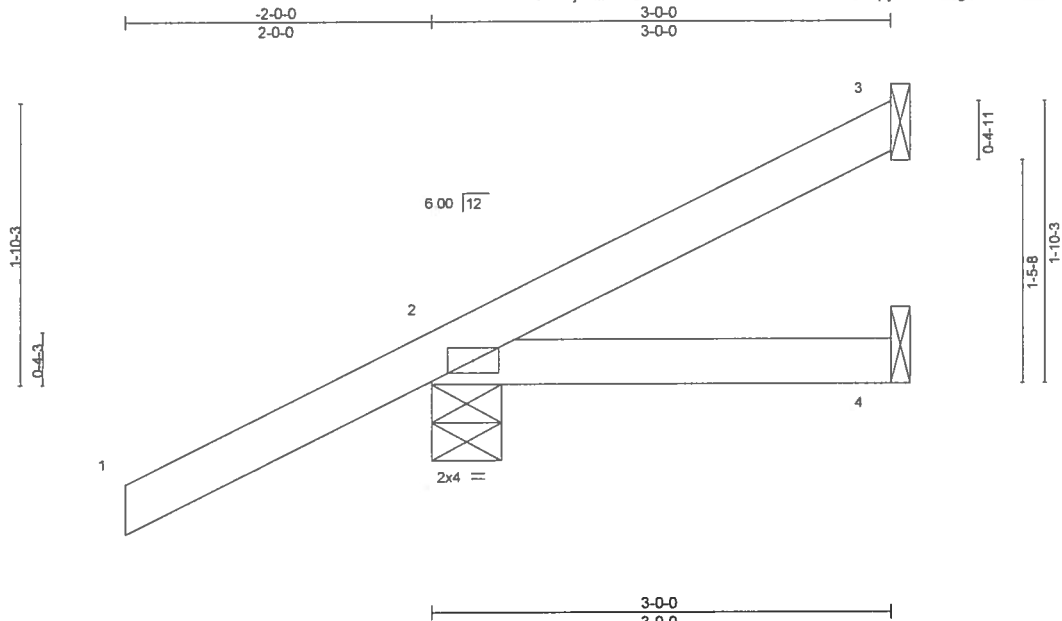
MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence
1559185	CJ3	Jack-Open	4	1	T17227285

Buiders FirstSource, Lake City, FL

8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 42 2019 Page 1
ID BVHjtU?4L8iDBBHQFc123Azlu1B-iWZAGeKRpyYCBcRagPVJRvNh4nKniY3bzcDBkOzA_h



Scale = 1/14.6

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.33	Vert(LL)	-0.00 2-4	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.08	Vert(CT)	-0.01 2-4	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	-0.00 3	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P					Weight 13 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=26/Mechanical, 2=297/0-5-8, 4=27/Mechanical
Max Horz 2=65(LC 12)
Max Uplift 3=23(LC 12), 2=57(LC 12)
Max Grav 3=28(LC 19), 2=297(LC 1), 4=54(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



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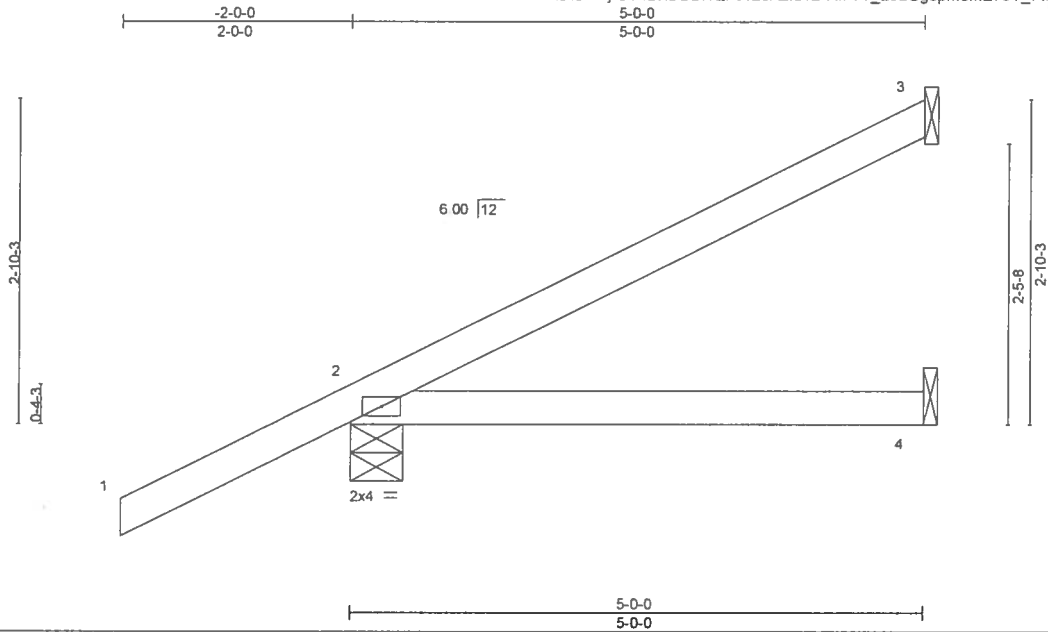
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Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227286
1559185	CJ5	Jack-Open	4	1	Job Reference (optional)	

Buildiers FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 43 2019 Page 1
ID:BVHjtU74L8iDBBHQFc123Azlu1B-Ai7YT_L3aGg3pm0mE70Y_7wsqBd1R?iICGzIGqZA_g



Scale = 1:19.5

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.33	Vert(LL)	-0.03 2-4	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL 1.25	BC 0.27	Vert(CT)	-0.06 2-4	>958	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	-0.00 3	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-P					Weight: 19 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=110/Mechanical, 2=354/0-5-8, 4=47/Mechanical
Max Horz 2=94(LC 12)
Max Uplift 3=58(LC 12), 2=57(LC 12)
Max Grav 3=110(LC 1), 2=354(LC 1), 4=94(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



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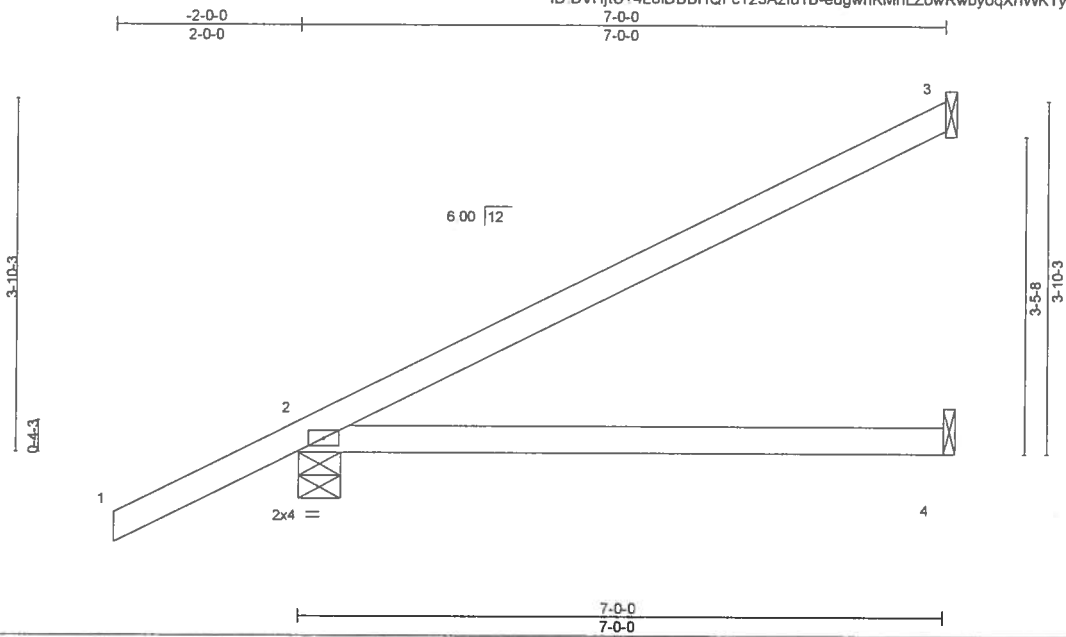


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Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227287
1559185	EJ7	Jack-Open	10	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL

8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 44 2019 Page 1
ID BVHjU74L8iDBBHQFc123Azlu1B-eugwhKMhLZowRwbyoqXnWKTy0buLASYuRwlpGzA_f



Scale 1/2"=1'

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.68	Vert(LL)	-0.12	2-4	>663	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.59	Vert(CT)	-0.24	2-4	>331	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						Weight: 26 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=179/Mechanical, 2=424/0-5-8, 4=67/Mechanical
Max Horz 2=123(LC 12)
Max Uplift 3=88(LC 12), 2=61(LC 12)
Max Grav 3=179(LC 1), 2=424(LC 1), 4=134(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



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MiTek

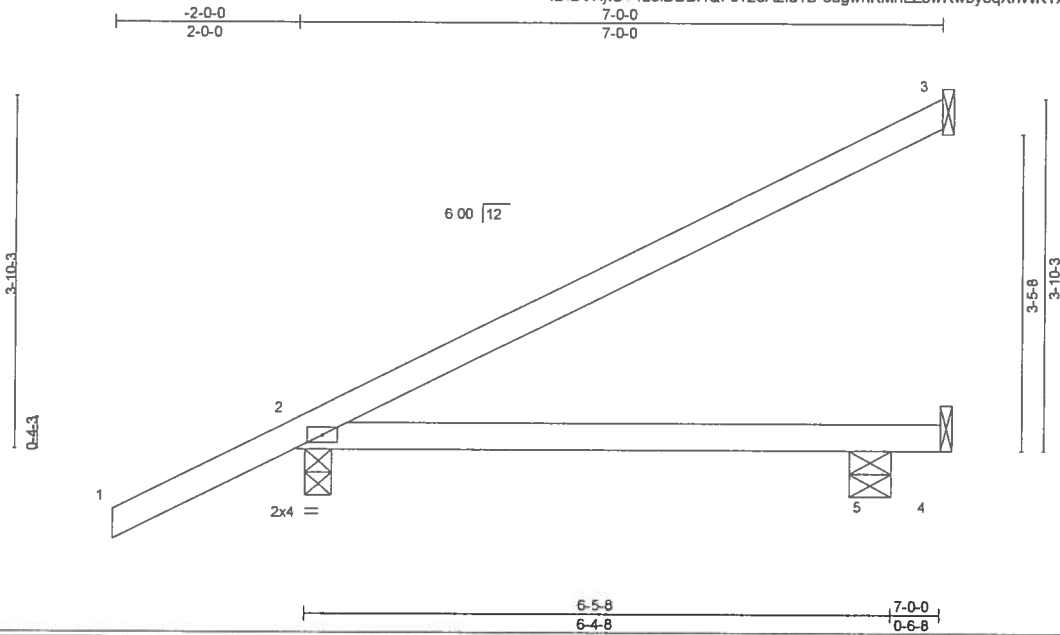
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Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227288
1559185	EJ7A	Jack-Open	5	1		
Job Reference (optional)						

Builds FirstSource, Lake City, FL

8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 44 2019 Page 1

ID BVHjtU74L8iDBBHQFc123Azlu1B-eugwhKMhLZowRwbyoqXnWKTxTbyLASyUrwilpGzA__f



Scale 1/2"=1'

LOADING (psf)	SPACING-	CSL	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.72	Vert(LL)	0.08	2-5	>961	MT20	244/190
TCDL 10.0	Lumber DOL 1.25	BC 0.33	Vert(CT)	-0.08	2-5	>938		
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(CT)	-0.00	3	n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-P						
							Weight: 26 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS.

All bearings Mechanical except (jt=length) 2=0-3-8, 5=0-5-8.

(lb) - Max Horz 2=123(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 3, 2 except 4=220(LC 3), 5=129(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 3, 4 except 2=400(LC 1), 5=397(LC 3)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, Encl., GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6-0 tall by 2'-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2 except (jt=lb) 4=220, 5=129.



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6904 Parke East Blvd
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227289
1559185	HJ9	Diagonal Hip Girder	2	1		

Builds FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 46 2019 Page 1
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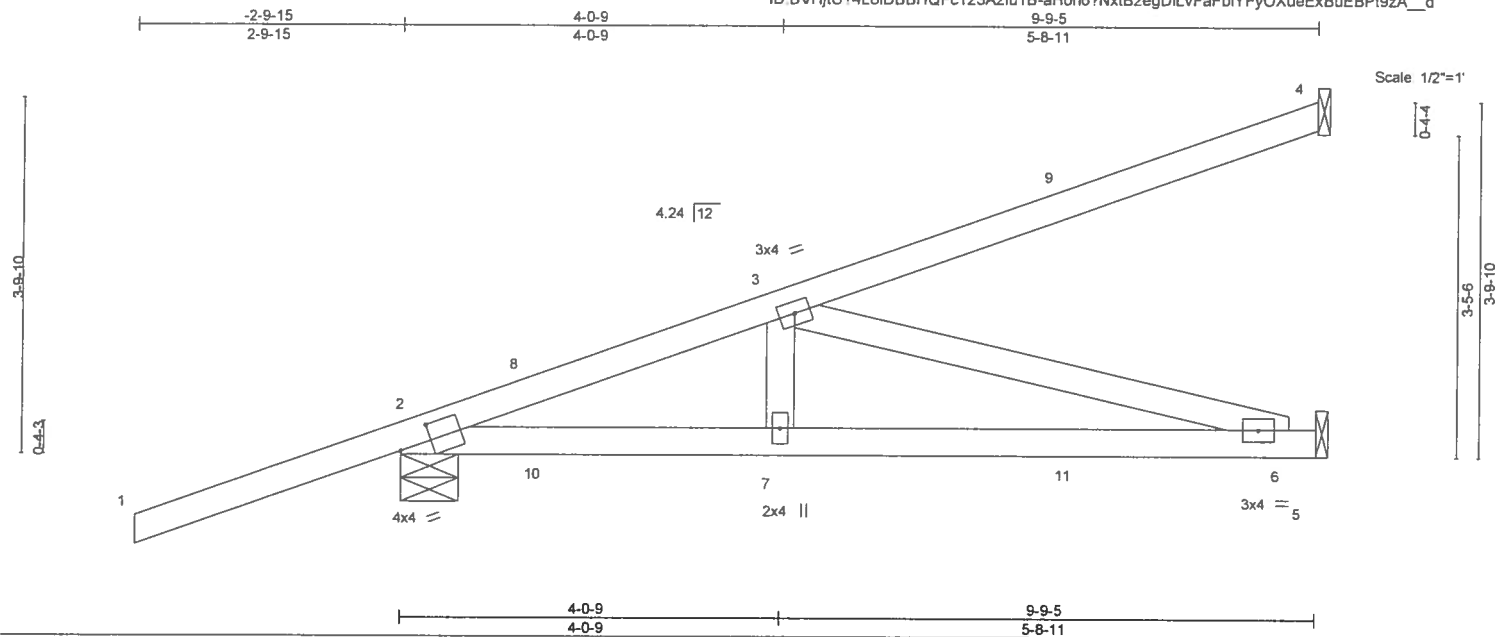


Plate Offsets (X,Y)- [2:0-4-1,0-2-2]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.85	Vert(LL)	-0.07	6-7	>999	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.25	BC 0.71	Vert(CT)	-0.15	6-7	>742		
BCLL 0.0	Lumber DOL 1.25	WB 0.46	Horz(CT)	0.01	5	n/a		
BCDL 10.0	Rep Stress Incr NO	Matrix-S						
	Code FBC2017/TPI2014						Weight: 44 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-2-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=173/Mechanical, 2=509/0-7-6, 5=219/Mechanical
Max Horz 2=141(LC 4)
Max Uplift 4=-85(LC 4), 2=-186(LC 4), 5=-30(LC 8)
Max Grav 4=173(LC 1), 2=548(LC 35), 5=260(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-834/111
BOT CHORD 2-7=-181/699, 6-7=-181/699
WEBS 3-7=0/277, 3-6=-724/188

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft, Cat. II; Exp C, Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5 except (jt=lb) 2=186.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 80 lb down and 190 lb up at 1-6-1, 80 lb down and 190 lb up at 1-6-1, 71 lb down and 19 lb up at 4-4-0, 71 lb down and 19 lb up at 4-4-0, and 88 lb down and 64 lb up at 7-1-15, and 88 lb down and 64 lb up at 7-1-15 on top chord, and 1 lb down at 1-6-1, 1 lb down at 1-6-1, 19 lb down at 4-4-0, 19 lb down at 4-4-0, and 39 lb down at 7-1-15, and 39 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-60, 2-5=-20
Concentrated Loads (lb)
Vert: 8=127(F=63, B=63) 9=-49(F=-25, B=-25) 11=-38(F=-19, B=-19)



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MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 3, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314

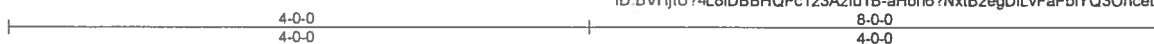
MiTek

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Tampa, FL 33610

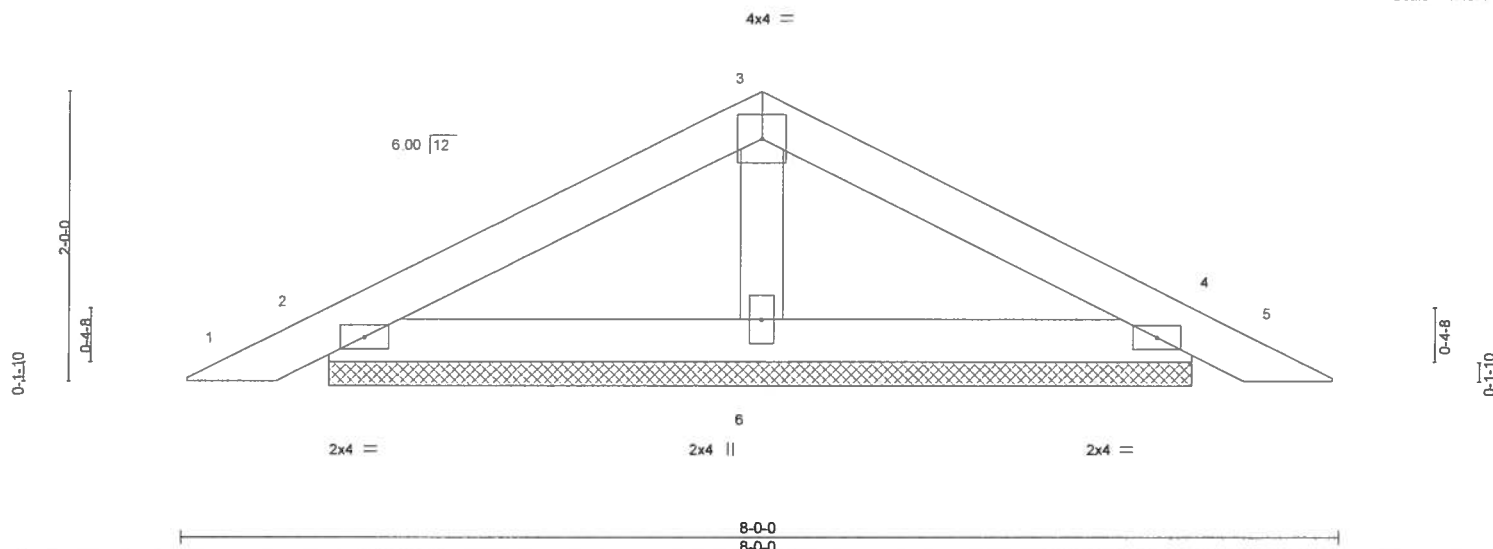
Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence
1559185	PB1	Piggyback	26	1	T17227290

Buildiers FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 46 2019 Page 1
ID: BVHjU?4L8iDBBHQFc123Azlu1B-aHoh6?NxtB2egDLvFaFbYQ3OheLYBuEBPt9zA_d



Scale = 1/15.4



LOADING (psf)	SPACING-		CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.13	Vert(LL)	0.00	5	n/r	120	MT20
TCDL 10.0	Lumber DOL	1.25	BC 0.09	Vert(CT)	0.01	5	n/r	120	244/190
BCLL 0.0	Rep Stress Incr	YES	WB 0.03	Horz(CT)	0.00	4	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						
								Weight: 24 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=166/5-11-10, 4=166/5-11-10, 6=227/5-11-10
Max Horz 2=-21(LC 10)
Max Uplift 2=-38(LC 12), 4=-41(LC 13), 6=-11(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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June 3, 2019

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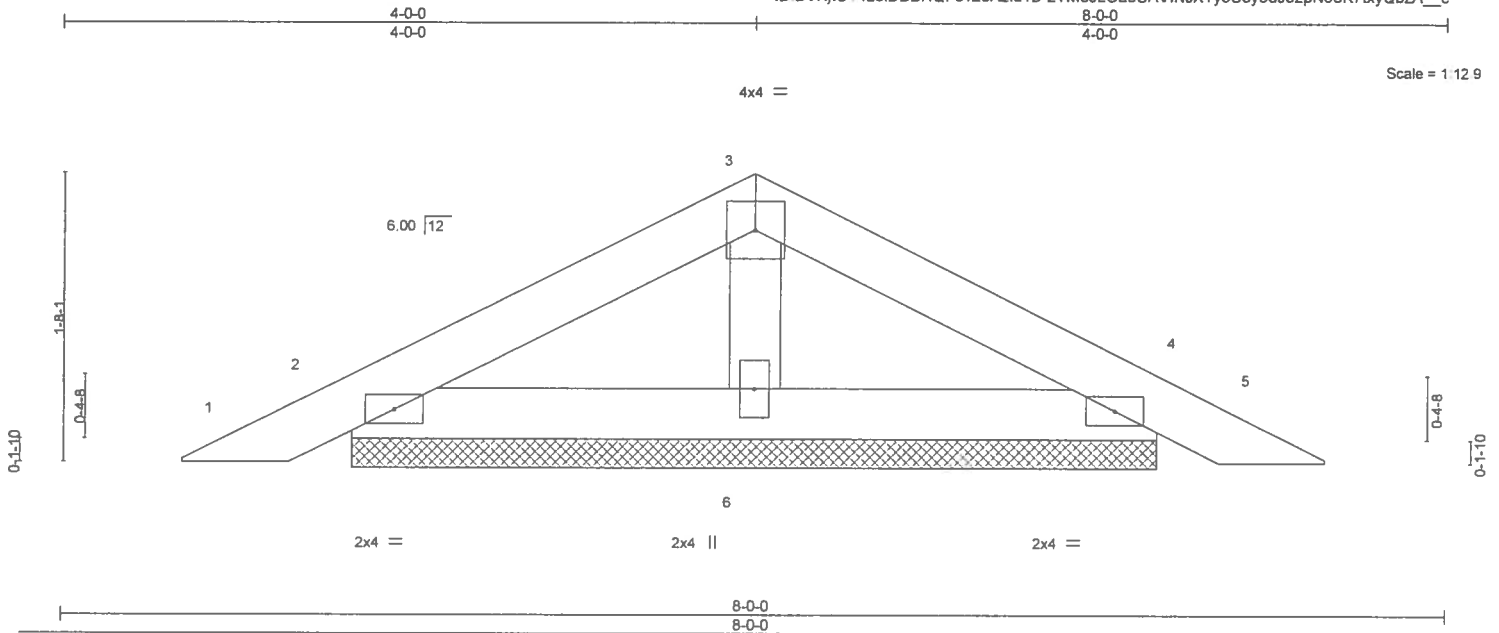
6904 Parke East Blvd
Tampa, FL 36610

Job 1559185	Truss PB1G	Truss Type Piggyback	Qty 2	Ply 2	O'Quinn Residence	T17227291
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Builds FirstSource,

Lake City, FL

8 240 s May 13 2019 MITek Industries, Inc. Mon Jun 3 13 30 47 2019 Page 1
ID BVHjtU74L8iDBBHQFc123Azlu1B-2TM3JLOaeUAVINJXTy5U8y5dJo2pNo6K7xyQbzA_c



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.04	Vert(LL)	0.00 4	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.03	Vert(CT)	0.00 5	n/r	120		
BCLL 0.0	Rep Stress Incr	YES	WB 0.01	Horz(CT)	0.00 4	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						
								Weight: 39 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=140/4-7-14, 4=140/4-7-14, 6=173/4-7-14
Max Horz 2=27(LC 17)
Max Uplift 2=57(LC 12), 4=62(LC 13), 6=24(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl.,
GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- * Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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June 3, 2019

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Design valid for use only with MITTEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information - available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314

MITTEK

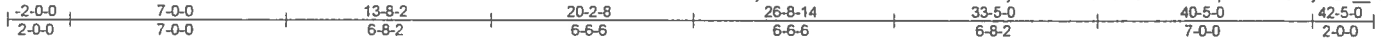
6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227292
1559185	T01	Hip Girder	1	2	Job Reference (optional)	

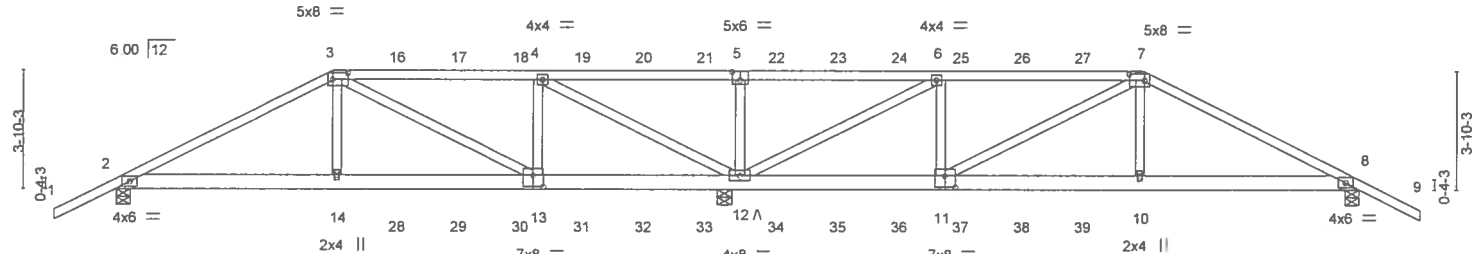
Buiders FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 52 2019 Page 1

ID BVHjIU74LBiDBBHQFc123Azlu1B-PR9yM3SiS1onO8CVGWhfr0oJmph62vC4G9ej5ozA_X



Scale = 1.72 4



	7-0-0	13-8-2	19-6-0	20-2-8	26-8-14	33-5-0	40-5-0
	7-0-0	6-8-2	5-9-14	0-8-8	6-6-6	6-8-2	7-0-0
Plate Offsets (X,Y)---	[3:0-6-0,0-2-8], [5:0-3-0,0-3-4], [7:0-6-0,0-2-8], [11:0-4-0,0-4-8], [13:0-4-0,0-4-8]						

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.70	Vert(LL)	0.26 11-12	>932	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.28	Vert(CT)	0.26 12-13	>940	180		
BCLL 0.0	Rep Stress Incr	NO	WB 0.66	Horz(CT)	-0.04 8	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S					Weight: 457 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

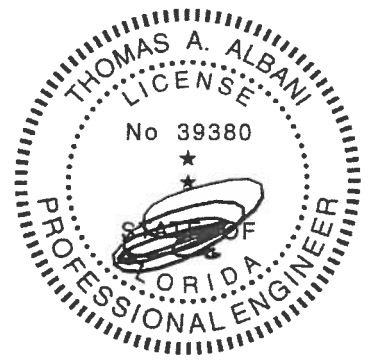
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 12-13.

REACTIONS. (lb/size) 2=1088/0-5-8, 12=3251/0-5-8, 8=1311/0-5-8
Max Horz 2=52(LC 7)
Max Uplift 2=895(LC 8), 12=REL, 8=837(LC 9)
Max Grav 2=1092(LC 19), 12=3274(LC 43), 8=1314(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=1708/1884, 3-4=960/2791, 4-5=0/3013, 5-6=0/3013, 6-7=1398/2585,
7-8=2126/1756
BOT CHORD 2-14=1646/1504, 13-14=1645/1525, 12-13=2734/1021, 11-12=2506/1398,
10-11=1499/1827, 8-10=1501/1808
WEBS 3-14=0/642, 3-13=1271/0, 4-13=44/523, 4-12=2368/0, 5-12=868/393, 6-12=2873/0,
6-11=0/776, 7-11=1168/0, 7-10=0/695

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10, Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II, Exp C, Encl.,
GCPi=0.18; MWFRS (envelope), Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
2=895, 8=837.
- "A" indicates Released bearing: allow for upward movement at joint(s) 12.



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MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 3, 2019

Continued on page 2

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314

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6904 Parke East Blvd
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227292
1559185	T01	Hip Girder	1	2	Job Reference (optional)	

Burders FirstSource, Lake City, FL

8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 52 2019 Page 2
ID BVHjtU74L8iDBBHQFc123Azlu1B-PR9yM3SIS1onO8CVGWhfr0oJmph62vC4G9ej5ozA__X

NOTES-

- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 254 lb down and 204 lb up at 7-0-0, 119 lb down and 102 lb up at 9-0-12, 123 lb down and 103 lb up at 11-0-12, 123 lb down and 103 lb up at 13-0-12, 123 lb down and 103 lb up at 15-0-12, 123 lb down and 103 lb up at 17-0-12, 123 lb down and 103 lb up at 19-0-12, 119 lb down and 102 lb up at 20-2-8, 119 lb down and 102 lb up at 21-4-4, 119 lb down and 102 lb up at 23-4-4, 119 lb down and 102 lb up at 25-4-4, 119 lb down and 102 lb up at 27-4-4, 119 lb down and 102 lb up at 29-4-4, and 119 lb down and 102 lb up at 31-4-4, and 254 lb down and 204 lb up at 33-5-0 on top chord, and 298 lb down and 58 lb up at 7-0-0, 94 lb down at 9-0-12, 76 lb down and 130 lb up at 11-0-12, 76 lb down and 130 lb up at 13-0-12, 76 lb down and 130 lb up at 15-0-12, 76 lb down and 130 lb up at 17-0-12, 76 lb down and 130 lb up at 19-0-12, 94 lb down at 20-2-8, 94 lb down at 21-4-4, 94 lb down at 23-4-4, 94 lb down at 25-4-4, 94 lb down at 27-4-4, 94 lb down at 29-4-4, and 94 lb down at 31-4-4, and 298 lb down and 58 lb up at 33-4-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=60, 3-7=60, 7-9=60, 2-8=20

Concentrated Loads (lb)

Vert: 3=207(F) 7=207(F) 14=238(F) 12=47(F) 5=119(F) 10=238(F) 16=119(F) 17=123(F) 18=123(F) 19=123(F) 20=123(F) 21=123(F) 22=119(F) 23=119(F) 24=119(F) 25=119(F) 26=119(F) 27=119(F) 28=47(F) 29=130(F) 30=130(F) 31=130(F) 32=130(F) 33=130(F) 34=47(F) 35=47(F) 36=47(F) 37=47(F) 38=47(F) 39=47(F)

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6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T1727293
1559185	T02	Hip	1	1	Job Reference (optional)	

Buiders FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc Mon Jun 3 13 30 54 2019 Page 1
ID: BVHjtU74L8iDBBHQFc123Azlu1B-LpHinkTz_e2VeSMtNwj7wRtiTdDcWuIMkT7q9hzA_V

-2-0-0	4-9-4	9-0-0	14-8-2	20-2-8	25-8-14	31-5-0	35-7-12	40-5-0	42-5-0
2-0-0	4-9-4	4-2-12	5-8-2	5-6-6	5-6-6	5-8-2	4-2-12	4-9-4	2-0-0

Scale = 1.72 4

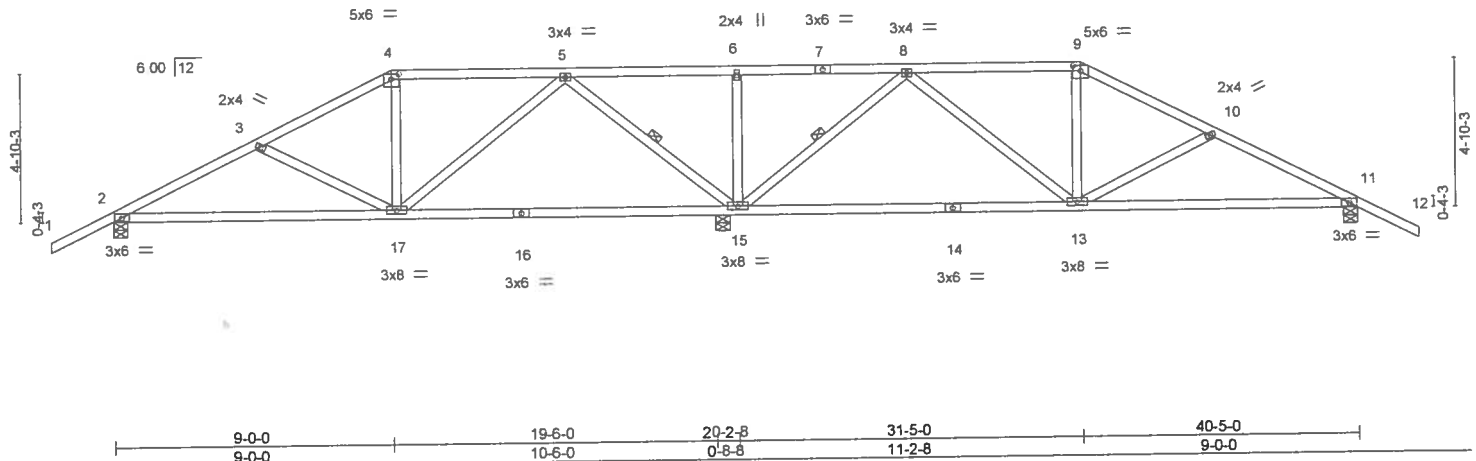


Plate Offsets (X,Y) -		[4:0-3-0,0-2-0], [9:0-3-0,0-2-0]		9-0-0		19-6-0		20-2-8		31-5-0		40-5-0	
LOADING (psf)		SPACING-		CSI.		DEFL.		in (loc)		I/defl		L/d	
TCLL	20.0	Plate Grip DOL	1.25	TC	0.49	Vert(LL)	-0.20	15-17	>999	240		PLATES	GRIP
TCDL	10.0	Lumber DOL	1.25	BC	0.91	Vert(CT)	-0.40	15-17	>594	180		MT20	244/190
BCLL	0.0	Rep Stress Incr	YES	WB	0.34	Horz(CT)	0.03	11	n/a	n/a		Weight: 205 lb FT = 20%	
BCDL	10.0	Code FBC2017/TP12014		Matrix-S									

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-10-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-7-8 oc bracing.
WEBS 1 Row at midpt 5-15, 8-15

REACTIONS.

(lb/size) 2=752/0-5-8, 15=1960/0-5-8, 11=752/0-5-8
Max Horz 2=64(LC 10)
Max Uplift 2=127(LC 12), 15=272(LC 9), 11=135(LC 13)
Max Grav 2=772(LC 23), 15=1960(LC 1), 11=772(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=977/400, 3-4=699/285, 4-5=570/294, 5-6=161/743, 6-8=161/743, 8-9=570/285,
9-10=699/275, 10-11=977/390
BOT CHORD 2-17=218/806, 11-13=241/806
WEBS 3-17=268/226, 5-17=123/543, 5-15=1107/442, 6-15=323/183, 8-15=1107/449,
8-13=130/543, 10-13=268/226

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II; Exp C, Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=127, 15=272, 11=135.



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Date:

June 3,201



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



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Tampa, FL 33610

Scale = 1.724

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Tampa, FL 36610

Job 1559185	Truss T05	Truss Type Hip	Qty 1	Ply 1	O'Quinn Residence	T17227296
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Builders FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc Mon Jun 3 13 30 58 2019 Page 1
ID BVHjiU74L8iDBBHQFc123Azlu1B-EbXdd6XT2LzX63ffcmo35H2IIeCzSgCyf552ISzA_R



Scale = 1.73 5

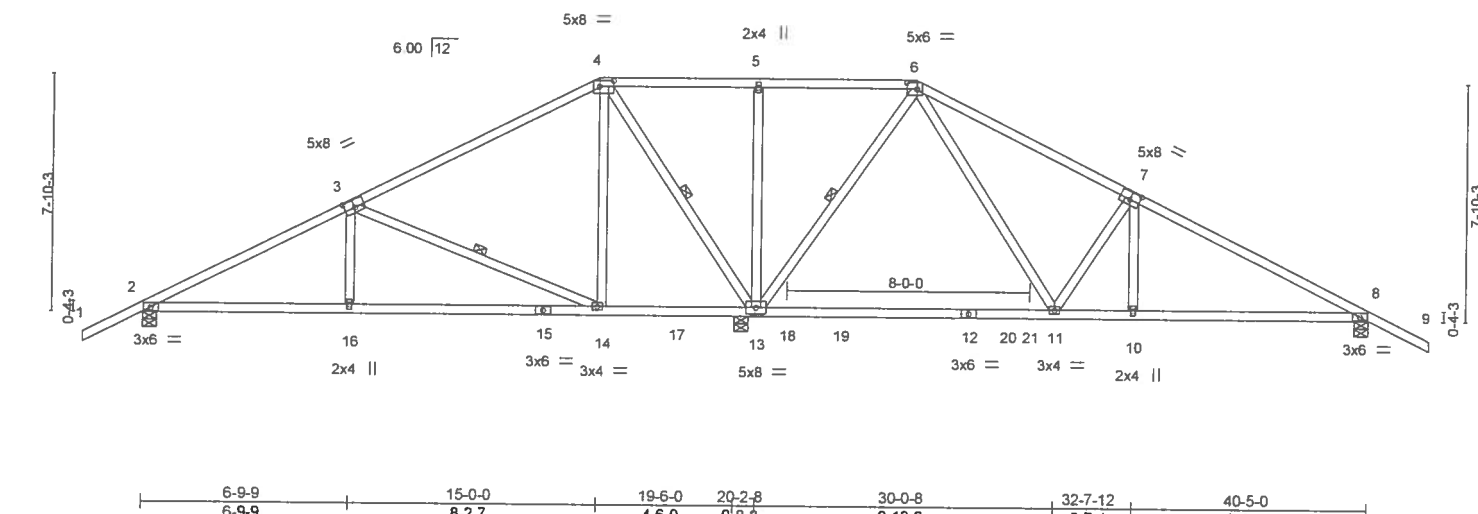


Plate Offsets (X,Y)=		[3:0-4-0,0-3-0], [4:0-5-8,0-2-4], [6:0-4-0,0-2-8], [7:0-4-0,0-3-0]											
LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP			
TCLL 20.0		Plate Grip DOL 1.25		TC 0.86		in (loc) l/defl L/d		MT20		244/190			
TCDL 10.0		Lumber DOL 1.25		BC 0.82		Vert(LL) -0.35 11-13 >690 240							
BCLL 0.0		Rep Stress Incr NO		WB 0.50		Vert(CT) -0.53 11-13 >454 180							
BCDL 10.0		Code FBC2017/TP12014		Matrix-S		Horz(CT) 0.02 8 n/a n/a							
								Weight .223 lb		FT = 20%			

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.1
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-3-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-14, 4-13, 6-13

REACTIONS.

(lb/size) 2=636/0-5-8, 13=2432/0-5-8, 8=716/0-5-8
Max Horz 2=98(LC 10)
Max Uplift 2=128(LC 12), 13=204(LC 12), 8=144(LC 13)
Max Grav 2=724(LC 23), 13=2432(LC 1), 8=724(LC 24)

FORCES.

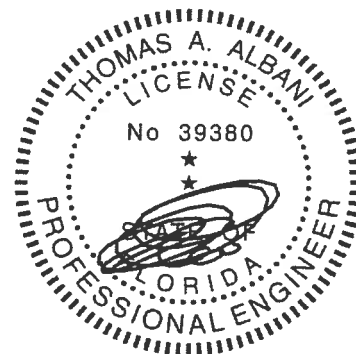
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=879/301, 3-4=120/297, 4-5=88/738, 5-6=88/738, 6-7=529/260, 7-8=795/264
BOT CHORD 2-16=132/710, 14-16=133/706, 13-14=171/266, 11-13=181/265, 10-11=98/619, 8-10=98/621
WEBS 3-16=0/341, 3-14=781/418, 4-14=125/461, 4-13=997/396, 5-13=347/168, 6-13=1116/407, 6-11=286/913, 7-11=527/354

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph, TCDL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCp=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=128, 13=204, 8=144.
- Load case(s) 1 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard Except:

- Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=60, 4-6=60, 6-9=60, 2-18=20, 18-21=60(F=40), 8-21=20



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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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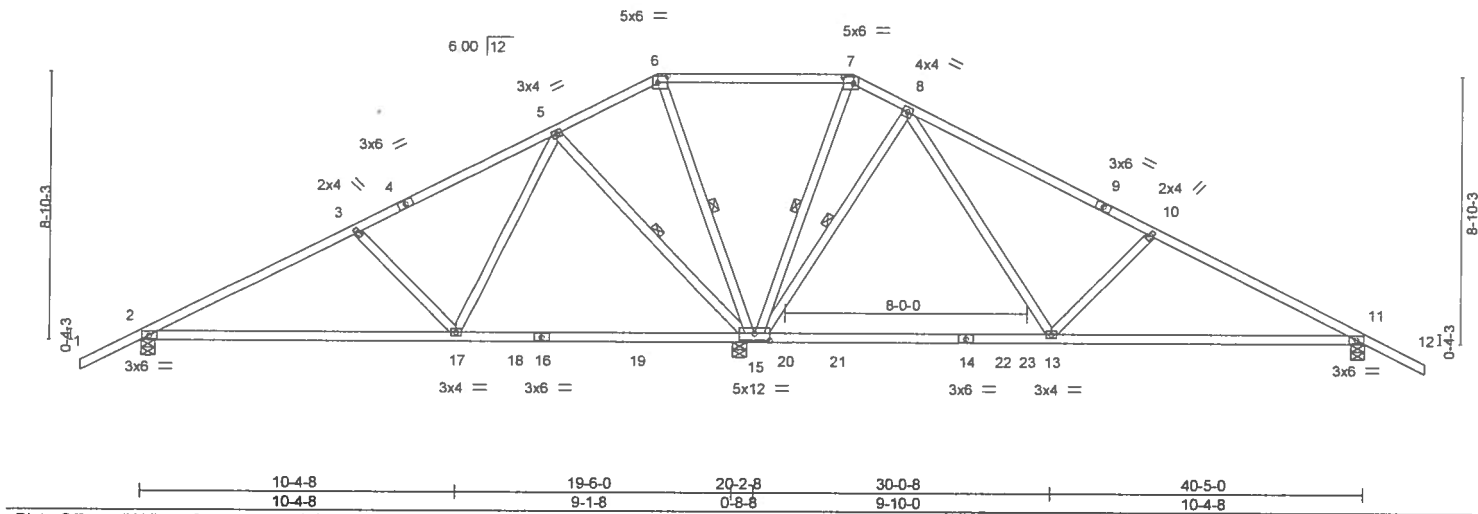
6904 Parke East Blvd.
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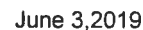
Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227297
1559185	T06	Hip	1	1	Job Reference (optional)	

Builds FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 30 59 2019 Page 1
ID BVHjtU74L8iDBBHQFc123Azlu1B-in4bqSX5pAhokDErAUJldUaUCeyyB9W6ttrbruzA Q

Scale = 1:73.5



Scale = 1.73.5

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Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227299
1559185	T08	Common	1	1		

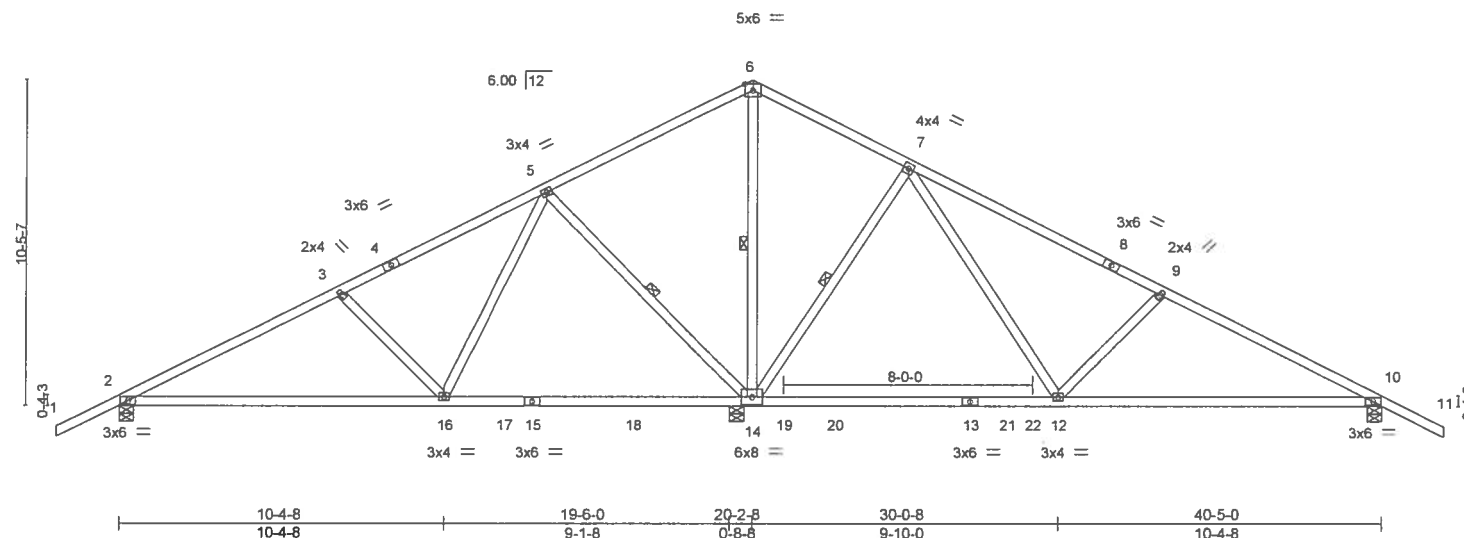
Builds FirstSource, Lake City, FL

8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 02 2019 Page 1

ID:BVHjU74L8iDBBHQFc123Azlu1B-6MmkSta_653MbhZQrcs?F7C0Er_fOUWYaj3FRDzA_N

-2-0-0	7-1-3	13-7-13	20-2-8	25-2-10	33-3-13	40-5-0	42-5-0
2-0-0	7-1-3	6-6-11	6-6-11	5-0-2	8-1-4	7-1-3	2-0-0

Scale = 1:71.2



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.76	in (loc) l/defl L/d	MT20	244/190
TCCL 10.0	Plate Grip DOL 1.25	BC 0.77	Vert(LL) -0.49 12-14 >492 240		
BCCL 0.0	Lumber DOL 1.25	WB 0.48	Vert(CT) -0.57 12-14 >421 180		
BCDL 10.0	Rep Stress Incr NO	Matrix-S	Horz(CT) 0.01 10 n/a n/a		
	Code FBC2017/TP12014			Weight: 214 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.1
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-9-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-14, 5-14, 7-14

REACTIONS.

(lb/size) 2=609/0-5-8, 14=2486/0-5-8, 10=689/0-5-8
Max Horz 2=128(LC 11)
Max Uplift 2=117(LC 12), 14=277(LC 12), 10=136(LC 13)
Max Grav 2=718(LC 23), 14=2486(LC 1), 10=713(LC 24)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-765/249, 3-5=-468/186, 5-6=-96/760, 6-7=-69/734, 7-9=-459/181, 9-10=-775/255
BOT CHORD 2-16=-135/602, 12-14=-240/296, 10-12=-103/618
WEBS 6-14=-893/237, 9-12=-445/361, 5-14=-752/439, 5-16=-178/603, 3-16=-388/307,
7-14=-831/436, 7-12=-233/851

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=117, 14=277, 10=136.
- Load case(s) 1 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard Except:

- Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-60, 6-11=-60, 2-19=-20, 19-22=-60(F=40), 10-22=-20



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June 3, 2019

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8 240 s May 13 2019 MiTek Industries, Inc Mon Jun 3 13 31 03 2019 Page 1
ID BVHjtU74L8IDBBHQFc123Azlu1B-aYK6gpacsPBDDrYcPJNENKIHuFK07rEhoNop qzA M

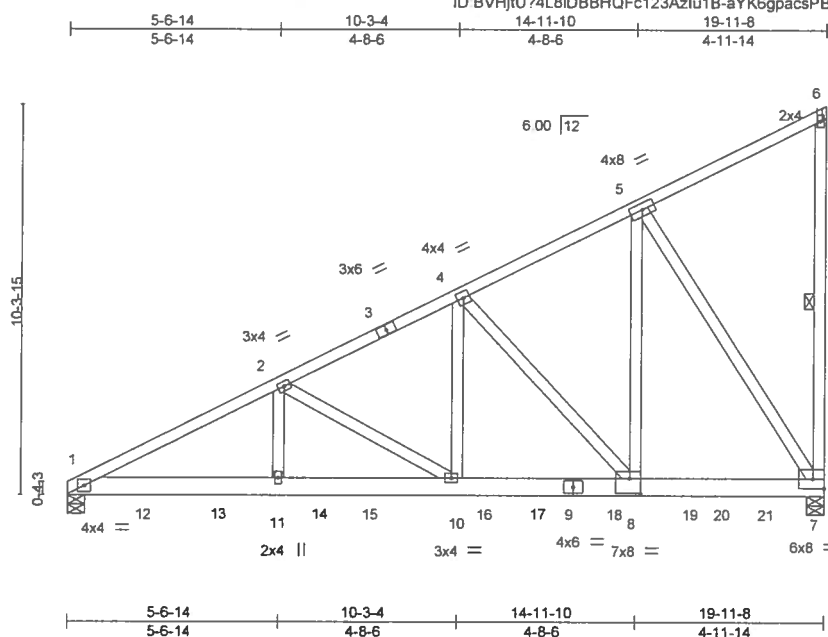


Plate Offsets (X,Y)— [8:0-3-8.0-4-12]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.38	Vert(LL)	-0.08 10-11	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.83	Vert(CT)	-0.13 10-11	>999	180		
BCLL 0.0	Rep Stress Incr	NO	WB 0.77	Horz(CT)	0.04 7	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S					Weight: 290 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3 *Except*
 5-7: 2x4 SP M 31

BRACING- TOP CHORD	Structural wood sheathing directly applied or 5-3-4 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 6-7

REACTIONS. (lb/size) 7=4318/0-5-8, 1=2652/0-5-8
Max Horz 1=285(LC 26)
Max Uplift 7=946(LC 8), 1=599(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.

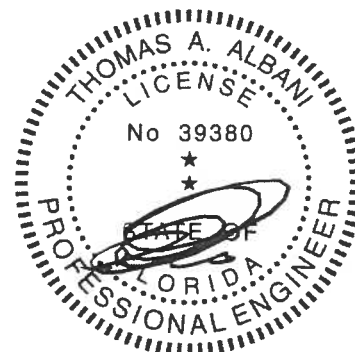
TOP CHORD	1-2=4953/1075, 2-4=3811/761, 4-5=2465/437
BOT CHORD	1-11=1195/4362, 10-11=1195/4362, 8-10=842/3348, 7-8=489/2155
WEBS	2-11=242/826, 2-10=1174/409, 4-10=441/1633, 4-8=1760/521, 5-8=840/4038, 5-7=3925/890

NOTES-

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Wind: ASCE 7-10, Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCPI=0.18; MVFRS (envelope), Lumber DOL=1.60 plate grip DOL=1.60
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=946, 1=599.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 275 lb up down and 116 lb up at 2-0-0, 275 lb down and 116 lb up at 4-0-0, 275 lb down and 116 lb up at 6-0-0, 275 lb down and 116 lb up at 8-0-0, 364 lb down and 104 lb up at 10-0-0, 364 lb down and 104 lb up at 11-0-0, 364 lb down and 104 lb up at 12-5-0, 1069 lb down and 208 lb up at 14-5-0, and 1069 lb down and 208 lb up at 16-5-0, and 1069 lb down and 208 lb up at 18-5-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25. Plate Increase=1.25



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June 3.2019

Continued on page 2

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Design valid for use only with Mitek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence
1559185	T09	Monopitch Girder	1	2	T17227300
Buiders FirstSource, Lake City, FL		Job Reference (optional)			

8.240 s May 13 2019 MiTek Industries, Inc Mon Jun 3 13:31:03 2019 Page 2
ID: BVHjtU?4L8iDBBHQFc123Azlu1B-aYK6gpacsPBDDrYcPJNEKIHuFK07rEhoNpp_gzA__M

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-6=60, 1-7=20

Concentrated Loads (lb)

Vert: 10=364(B) 12=275(B) 13=275(B) 14=275(B) 15=275(B) 16=364(B) 17=364(B) 18=1069(B) 19=1069(B) 21=1069(B)

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6904 Parke East Blvd
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227301
1559185	T10	ROOF SPECIAL	2	1	Job Reference (optional)	

Buiders FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 05 2019 Page 1
ID BVHjIU74L8iDBBHQFc123Azlu1B-XxSs5Vcs00RxS8h?XkQitlqYk33LbIG_Ghiv2YzA_K

Scale = 1/32" = 1' 3.94"

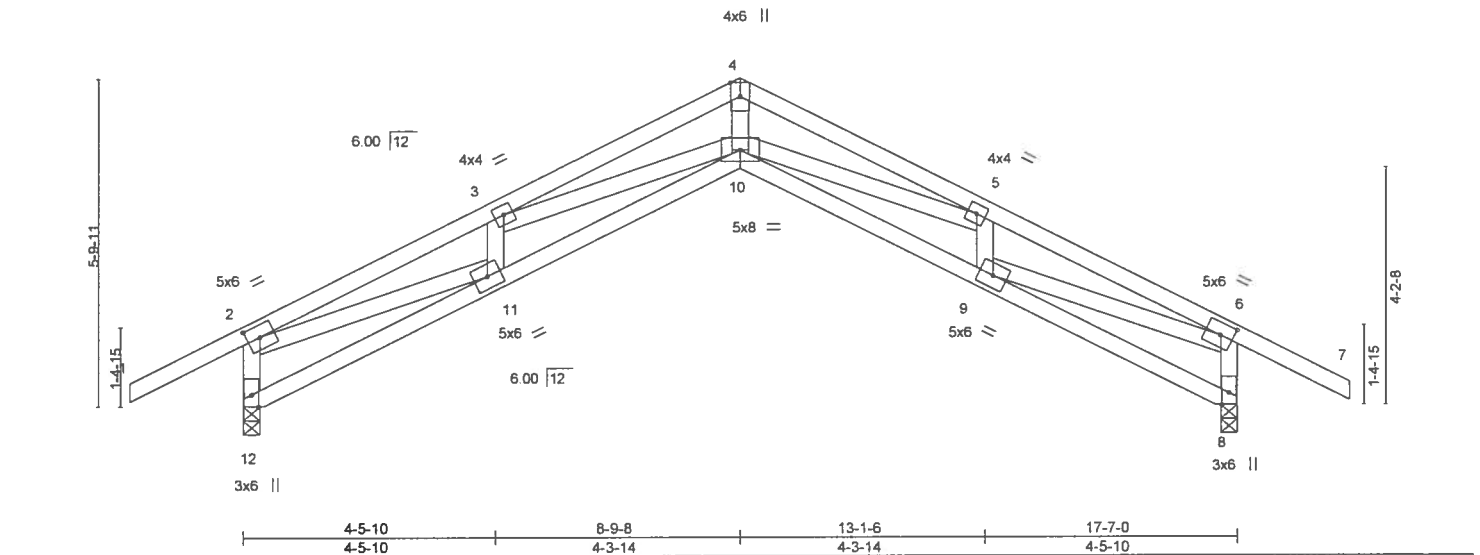


Plate Offsets (X,Y)--- [2:0-2-11,0-2-8], [6:0-2-11,0-2-8]										
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d		PLATES GRIP		
TCLL	20.0	Plate Grip DOL	1.25	TC	0.62	Vert(LL)	0.30 9-10	>691	240	MT20 244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.52	Vert(CT)	-0.34 10	>603	180	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.80	Horz(CT)	0.38 8	n/a	n/a	
BCDL	10.0	Code FBC2017/TPI2014		Matrix-S						
									Weight: 100 lb	FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 3-3-5 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 3-10-3 oc bracing.
WEBS	2x4 SP No.3		

REACTIONS. (lb/size) 12=820/0-3-8, 8=820/0-3-8
Max Horz 12=62(LC 11)
Max Uplift 12=204(LC 9), 8=204(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-12=783/904, 2-3=1957/2358, 3-4=2586/2479, 4-5=2586/2479, 5-6=1957/2381, 6-8=783/886
BOT CHORD 10-11=1967/1904, 9-10=1994/1904
WEBS 4-10=2038/2007, 5-10=31/602, 5-9=409/179, 6-9=1972/1696, 3-10=26/587, 3-11=409/175, 2-11=1964/1696

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Bearing at joint(s) 12, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=204, 8=204.



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June 3, 2019

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227302
1559185	T10G	ROOF SPECIAL	1	1		
Job Reference (optional)						

Builds FirstSource, Lake City, FL

8 240 s May 13 2019 Mitek Industries, Inc. Mon Jun 3 13 31 06 2019 Page 1
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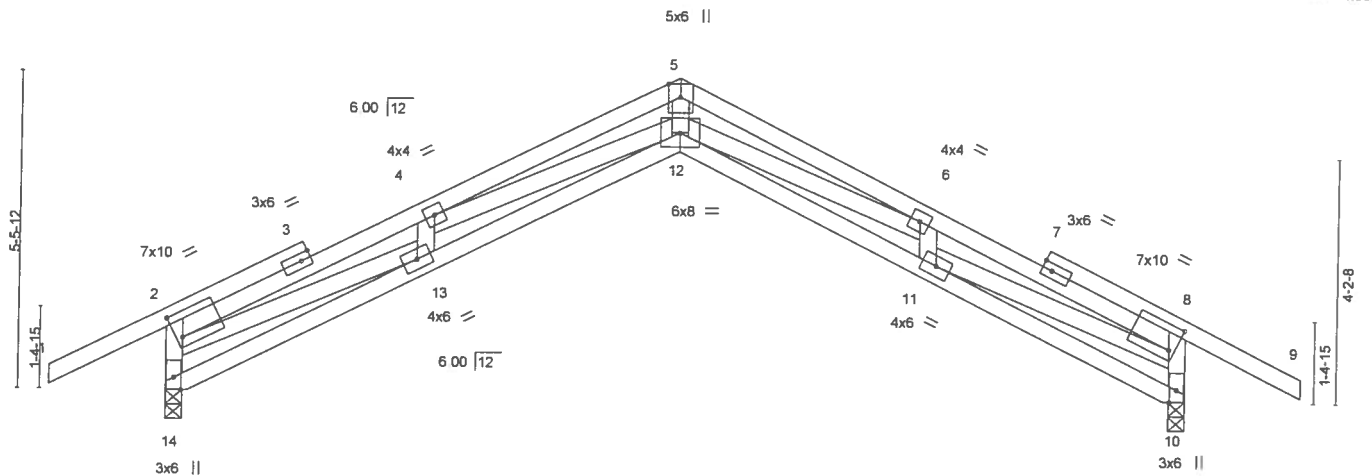
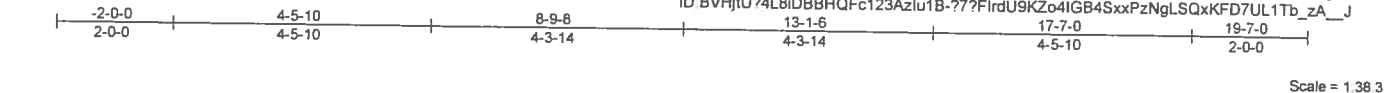


Plate Offsets (X,Y) -		[2:0-1-4, 0-5-0], [8:0-1-4, 0-5-0]									
LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL 20.0		2-0-0		TC 0.82		in (loc)		MT20		244/190	
TCDL 10.0		Plate Grip DOL 1.25		BC 0.50		Vert(LL) 0.44 11-12 >468 240		Weight: 106 lb		FT = 20%	
BCLL 0.0		Lumber DOL 1.25		WB 0.63		Vert(CT) -0.51 12 >403 180					
BCDL 10.0		Rep Stress Incr YES		Matrix-S		Horz(CT) 0.56 10 n/a n/a					
		Code FBC2017/TP12014									

LUMBER-
TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
WEBS 2x4 SP No.3 *Except*
5-12: 2x4 SP No.2, 6-12, 8-11, 4-12, 2-13: 2x4 SP M 31

BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-11-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 3-5-6 oc bracing.

REACTIONS. (lb/size) 14=820/0-3-8, 10=820/0-3-8
Max Horz 14=91(LC 13)
Max Uplift 14=250(LC 9), 10=250(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-14=834/974, 2-4=2544/3103, 4-5=3410/3289, 5-6=3410/3290, 6-8=2544/3129, 8-10=834/955
BOT CHORD 12-13=2810/2577, 11-12=2845/2577
WEBS 5-12=2797/2774, 6-12=132/820, 6-11=563/381, 8-11=2599/2183, 4-12=8/747, 4-13=563/367, 2-13=2559/2183

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10, Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Bearing at joint(s) 14, 10 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 14=250, 10=250.



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Date:

June 3, 2019

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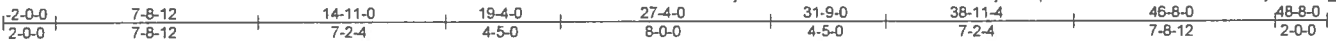
MITEK

6904 Parke East Blvd
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227303
1559185	T11	PIGGYBACK BASE	1	1	Job Reference (optional)	

Builds FirstSource, Lake City, FL

8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 08 2019 Page 1
ID BVHjU74L8iDBBHQCf123Azlu1B-xW7?jXelxpwJcQaCtzPUOS2UG6Eo55QyWafzA_H



Scale = 1.85.3

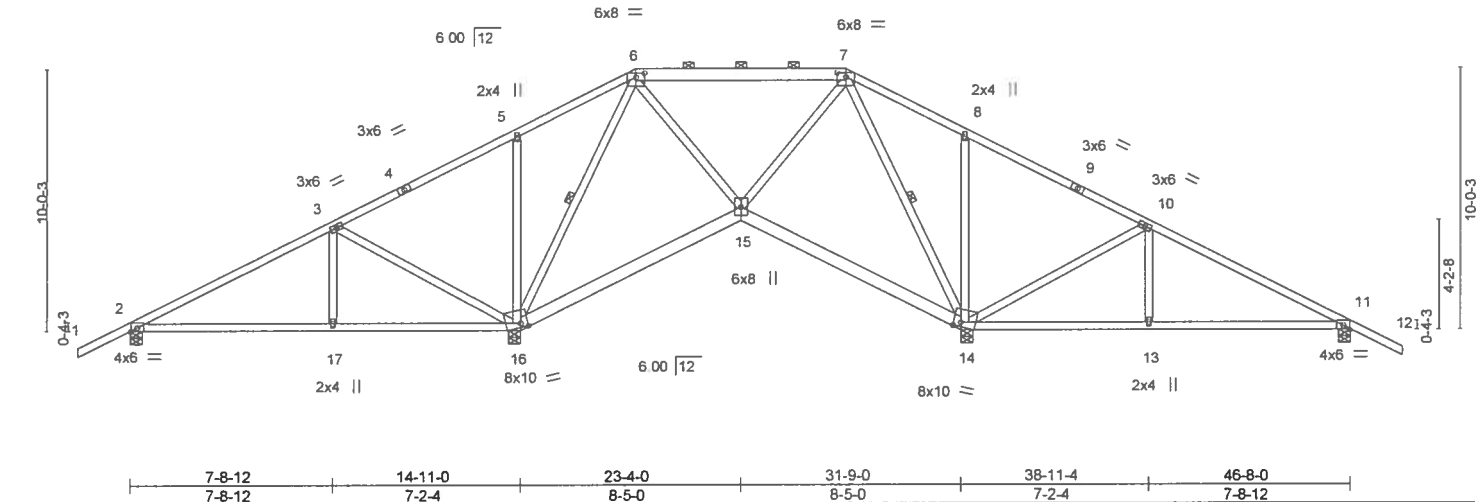


Plate Offsets (X,Y)– [6:0-4-0,0-1-15], [7:0-4-0,0-1-15], [14:0-3-6,0-1-15], [16:0-3-6,0-1-15]											
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d		PLATES GRIP			
TCLL	20.0	Plate Grip DOL	1.25	TC	0.65	Vert(LL)	0.13 11-13	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.51	Vert(CT)	-0.17 2-17	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.86	Horz(CT)	0.03 11	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-S						Weight 284 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*
6-7: 2x6 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
15-16, 14-15: 2x6 SP No.2
WEBS 2x4 SP No.3 *Except*
6-16, 7-14: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-16, 7-14

REACTIONS.

All bearings 0-5-8.
(lb) - Max Horz 2=123(LC 11)
Max Uplift All uplift 100 lb or less at joint(s) except 2=149(LC 8), 16=349(LC 9),
14=320(LC 8), 11=165(LC 8)
Max Grav All reactions 250 lb or less at joint(s) except 2=536(LC 23), 16=1477(LC 23), 14=1477(LC 24), 11=536(LC 24)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=397/408, 3-5=127/485, 5-6=40/456, 7-8=6/449, 8-10=134/485, 10-11=397/398
BOT CHORD 2-17=199/268, 16-17=199/268, 15-16=205/534, 14-15=203/531, 13-14=212/268,
11-13=212/268
WEBS 3-17=270/332, 3-16=696/714, 5-16=351/286, 6-16=689/49, 6-15=0/331, 7-15=0/336,
7-14=689/4, 8-14=351/286, 10-14=696/714, 10-13=270/332

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCCL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II, Exp C, Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 149 lb uplift at joint 2, 349 lb uplift at joint 16, 320 lb uplift at joint 14 and 165 lb uplift at joint 11.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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June 3, 2019

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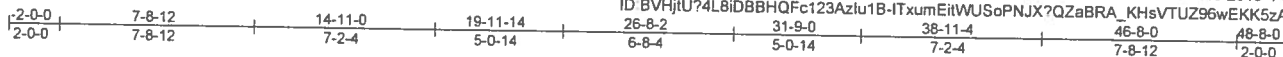
6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227304
1559185	T11G	GABLE II	1	2	Job Reference (optional)	

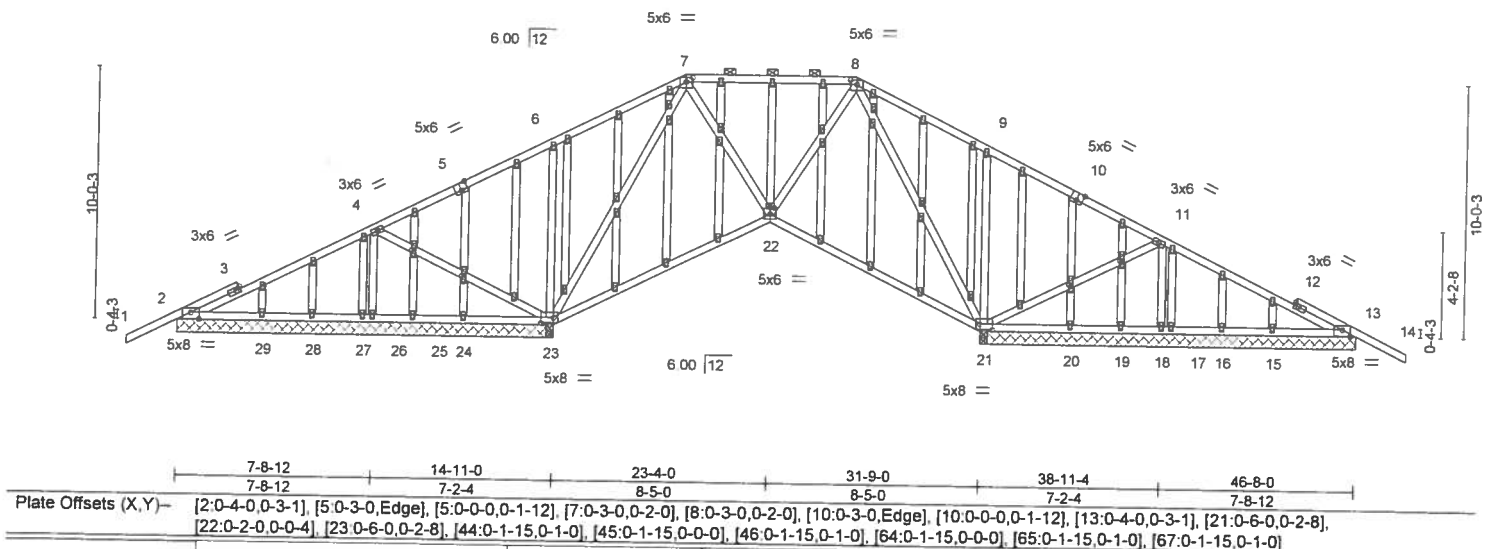
Builds FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 13 2019 Page 1

ID BVHjtU74L8iDBBHQFc123Azlu1B-ITxumEitWUSoPNJX7QZaBRA_KHsVTUZ96wEKK5zA_C



Scale = 1:88.2



LOADING (psf)	SPACING-	CSL	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.32	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.25	BC 0.31	Vert(LL) -0.09 21-22 >999 240		
BCLL 0.0	Lumber DOL 1.25	WB 0.33	Vert(CT) -0.19 21-22 >999 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.02 13 n/a n/a		
	Code FBC2017/TPI2014				

Weight: 824 lb FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins, except
BOT CHORD 2x4 SP No.2	2-0-0 oc purlins (6-0-0 max.): 7-8.
WEBS 2x4 SP No.3	Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
OTHERS 2x4 SP No.3	10-0-0 oc bracing: 22-23,21-22.

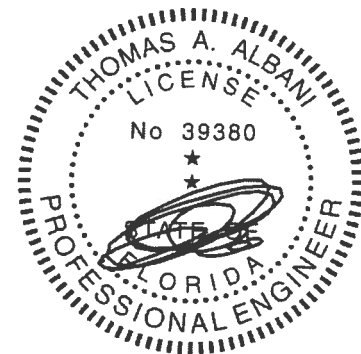
REACTIONS. All bearings 14-11-0.
(lb) - Max Horz 2=191(LC 13)
Max Uplift All uplift 100 lb or less at joint(s) 2, 13, 27, 29, 17, 15 except
23=259(LC 12), 21=197(LC 13), 26=345(LC 12), 18=359(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 13, 24, 25, 27, 28, 29, 20, 19,
17, 16, 15 except 23=1045(LC 1), 23=1045(LC 1), 21=1045(LC 1), 21=1045(LC 1),
1, 26=487(LC 23), 18=487(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=274/408, 4-6=65/401, 6-7=0/410, 8-9=0/398, 9-11=0/362, 11-13=190/408
BOT CHORD 2-29=272/283, 28-29=272/283, 27-28=272/283, 26-27=272/283, 25-26=272/283,
24-25=272/283, 23-24=272/283, 22-23=104/343, 21-22=101/338, 20-21=272/268,
19-20=272/268, 18-19=272/268, 17-18=272/268, 16-17=272/268, 15-16=272/268,
13-15=272/268
WEBS 4-26=496/396, 6-23=371/309, 7-23=665/96, 8-22=0/260, 8-21=665/70,
9-21=371/308, 11-18=496/394

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Provide adequate drainage to prevent water ponding.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2



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June 3,2019

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Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence
1559185	T11G	GABLE II	1	2	T17227304

Buiders FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc Mon Jun 3 13 31 13 2019 Page 2
ID BVHjtU74L8iDBBHQFc123Azlu1B-ITxumEitWUSoPNJX?QZaBRA_KHsVTUZ96wEKK5zA__C

NOTES-

- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 13, 27, 29, 17, 15 except (jt=lb) 23=259, 21=197, 26=345, 18=359.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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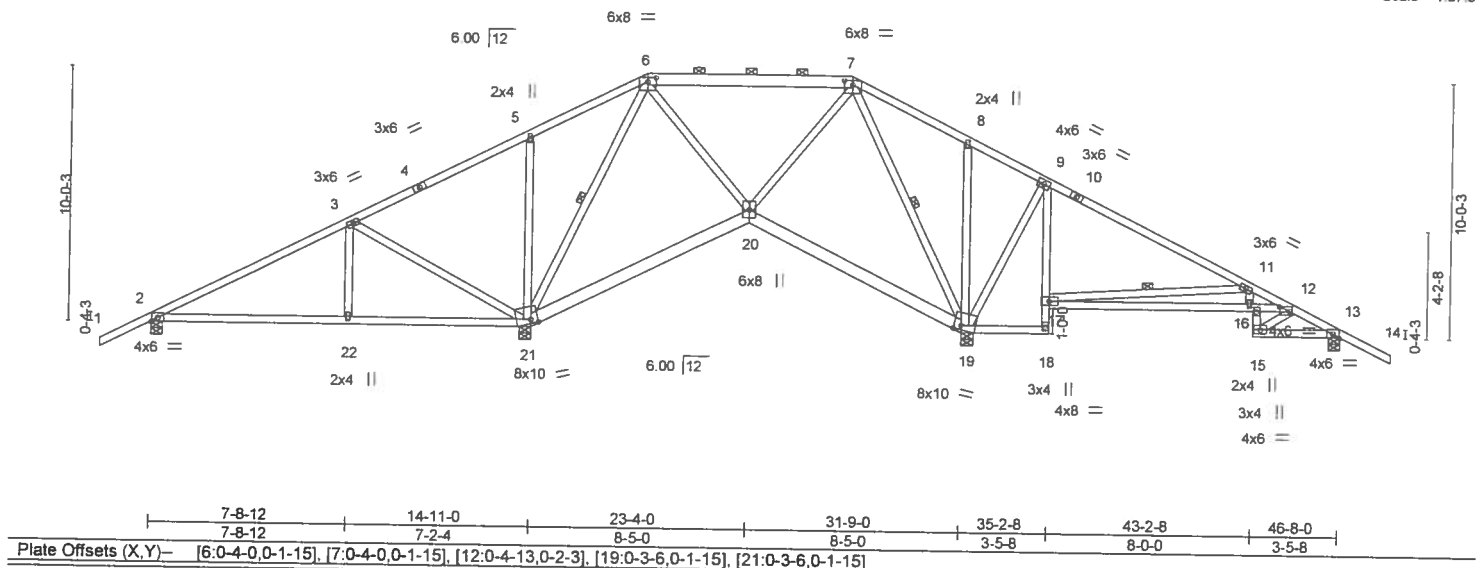
6904 Parke East Blvd
Tampa, FL 36610

Job 1559185	Truss T12	Truss Type PIGGYBACK BASE	Qty 2	Ply 1	O'Quinn Residence	T17227305
Buiders FirstSource, Lake City, FL						Job Reference (optional)

8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 16 2019 Page 1
ID BVHjTU74L8iDBBHQCf123Azlu1B-i2c0PGImpPqNGr16gY6Hp4nQfUpQgi4couS?xPzA_9

2-0-0	7-8-12	14-11-0	19-4-0	27-4-0	31-9-0	35-2-8	43-2-8	46-8-0	48-8-0	2-0-0
2-0-0	7-8-12	7-2-4	4-5-0	8-0-0	4-5-0	3-5-8	8-0-0	3-5-8	2-0-0	

Scale = 1:87.3



LOADING (psf)	SPACING-	CSL	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.70	Vert(LL) -0.12	16-17	>999	240	MT20	244/190
TCCL 10.0	Lumber DOL 1.25	BC 0.61	Vert(CT) -0.27	16-17	>654	180		
BCCL 0.0	Rep Stress Incr YES	WB 0.86	Horz(CT) 0.04	13	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-S						
Weight: 302 lb								FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 *Except* 6-7: 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 4-10-10 oc purlins, except 2-0-0 oc purlins (10-0-0 max.): 6-7.
BOT CHORD 2x4 SP No.2 *Except* 20-21,19-20: 2x6 SP No.2, 9-18,15-16: 2x4 SP No.3	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing:
WEBS 2x4 SP No.3 *Except* 6-21,7-19: 2x4 SP No.2	WEBS 1 Row at midpt 6-21, 7-19, 11-17

REACTIONS. All bearings 0-5-8.
(lb) - Max Horz 2=123(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 2 except 21=242(LC 12), 19=175(LC 13), 13=140(LC 13)
Max Grav All reactions 250 lb or less at joint(s) except 2=534(LC 23), 21=1464(LC 23), 19=1591(LC 24), 13=471(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=392/215, 3-5=33/498, 5-6=0/466, 7-8=0/612, 8-9=0/560, 9-11=0/364, 11-12=1091/557, 12-13=341/239
BOT CHORD 2-22=71/273, 21-22=71/273, 20-21=261/366, 19-20=290/328, 9-17=26/269, 16-17=481/939, 12-16=427/1031
WEBS 3-22=0/332, 3-21=696/370, 5-21=351/289, 6-21=666/85, 7-20=1/251, 7-19=807/0, 9-19=543/316, 11-16=0/526, 12-15=281/89, 11-17=1167/596

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 21=242, 19=175, 13=140.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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June 3,2019

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MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence
1559185	T13	PIGGYBACK BASE	2	1	T17227306

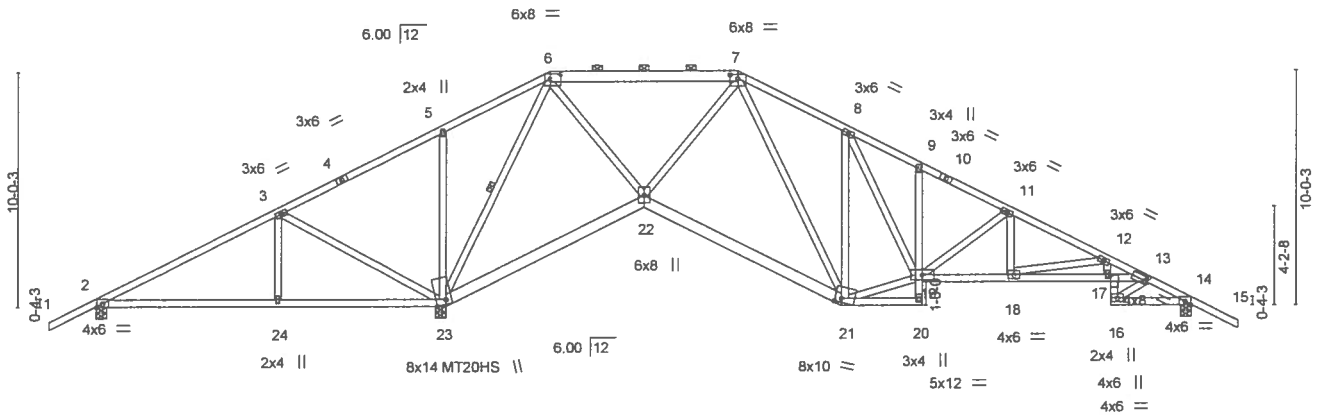
Buiders FirstSource, Lake City, FL

6 240 s May 13 2019 MiTek Industries, Inc Mon Jun 3 13 31 17 2019 Page 1

ID.BVHjU74L6iDBBHQFc123Azlu1B-AFAPccIOZiyEu_cIEGdWMHKYVv2zP8RI0YCYSza__8

2-0-0	7-8-12	14-11-0	19-4-0	27-4-0	31-9-0	35-2-8	38-11-4	43-2-8	46-8-0	48-8-0
2-0-0	7-8-12	7-2-4	4-5-0	8-0-0	4-5-0	3-5-8	3-8-12	4-3-4	3-5-8	2-0-0

Scale = 1:94.9



	7-8-12	14-11-0	23-4-0	31-9-0	35-2-8	38-11-4	43-2-8	46-8-0
	7-8-12	7-2-4	8-5-0	8-5-0	3-5-8	3-8-12	4-3-4	3-5-8

Plate Offsets (X,Y)-	[6:0-5-4,0-2-8], [7:0-4-0,0-1-15], [13:0-1-2,0-2-4], [17:0-0-8,0-1-12], [21:0-3-6,0-1-15], [23:0-2-6,Edge]
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LOADING (psf)	SPACING-		CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCCL 20.0	Plate Grip DOL	1.25	TC 0.89	Vert(LL)	-0.16	17-18	>999	240	MT20 244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.98	Vert(CT)	-0.32	17-18	>999	180	MT20HS 187/143
BCCL 0.0	Rep Stress Incr	YES	WB 0.98	Horz(CT)	0.14	14	n/a	n/a	
BCDL 10.0	Code FBC2017/TP12014		Matrix-S						
									Weight: 312 lb FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2 *Except* 6-7: 2x6 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD	2x4 SP No.2 *Except* 22-23,21-22: 2x6 SP No.2, 9-20,16-17: 2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	2x4 SP No.3 *Except* 6-23,7-21: 2x4 SP No.2	WEBS	1 Row at midpt 6-23

REACTIONS. (lb/size) 2=19/0-5-8, 23=2865/0-5-8, 14=1080/0-5-8
Max Horz 2=123(LC 10)
Max Uplift 2=304(LC 24), 23=287(LC 12), 14=201(LC 13)
Max Grav 2=239(LC 23), 23=2865(LC 1), 14=1097(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-241/1066, 3-5=-390/1598, 5-6=-233/1557, 7-8=-883/607, 8-9=-1328/682,
9-11=-1365/608, 11-12=-1955/772, 12-13=-3585/1299, 13-14=-1560/604
BOT CHORD 2-24=-911/328, 23-24=-911/328, 22-23=-630/544, 21-22=0/596, 18-19=-520/1724,
17-18=-992/2908, 13-17=-1017/3046, 16-17=-260/843, 14-16=-422/1260
WEBS 3-24=0/333, 3-23=-719/378, 5-23=-364/293, 6-23=-1928/405, 6-22=-81/928,
7-22=-733/512, 7-21=-492/603, 8-21=-864/446, 11-19=-696/315, 11-18=-92/414,
12-18=-1208/481, 8-19=-377/935, 19-21=-72/707, 12-17=-230/877, 13-16=-1375/461

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10, Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=304, 23=287, 14=201.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Date:

June 3,2019

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MiTek
6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227307
1559185	T14	PIGGYBACK BASE	10	1		

Builds FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 20 2019 Page 1

ID BVHjIU74L8iDBBHQFc123Azlu1B-aqsXEdoGsdKpSLivOBD_wy1F68jcWBjWQC4BZA_5

Job Reference (optional)

2-0-0	7-8-12	14-11-0	19-4-0	27-4-0	31-9-0	38-11-4	46-8-0
2-0-0	7-8-12	7-2-4	4-5-0	8-0-0	4-5-0	7-2-4	7-8-12

Scale = 1.84.3

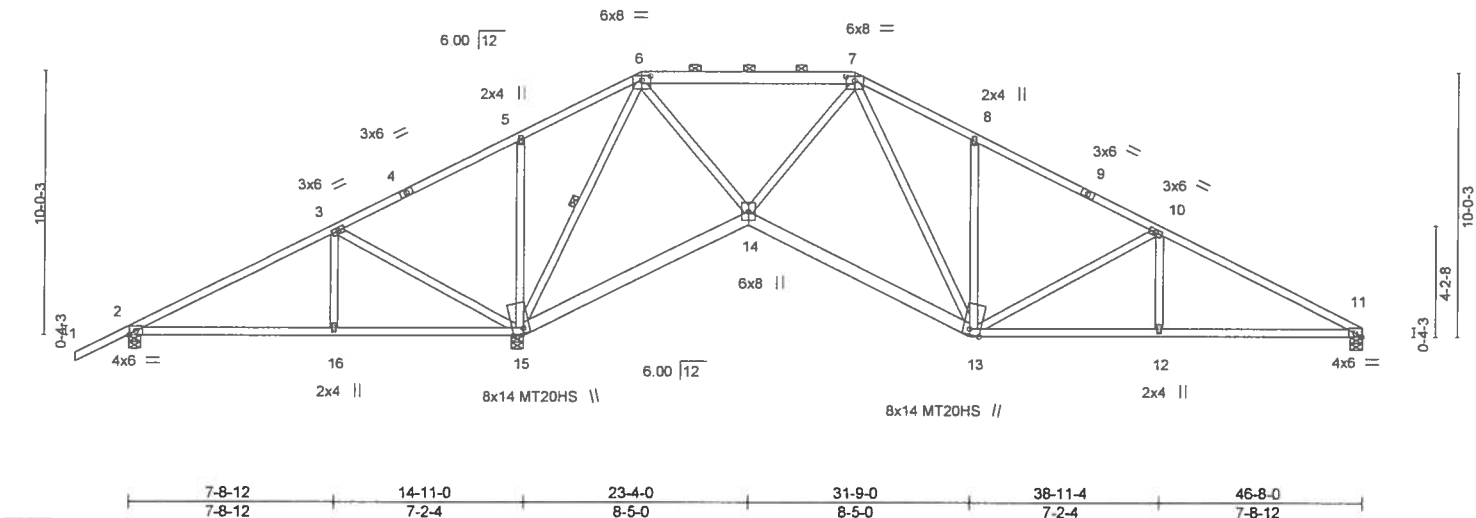


Plate Offsets (X,Y)	[6:0-4-0,0-1-15]	[7:0-4-0,0-1-15]	[9:0-0-0,0-0-0]	[13:0-2-6,Edge]	[15:0-2-6,Edge]
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LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.92	Vert(LL)	-0.10 11-12	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.72	Vert(CT)	-0.24 11-12	>999	180	MT20HS	187/143
BCLL 0.0	Rep Stress Incr	YES	WB 0.91	Horz(CT)	0.07 11	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						
								Weight 280 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 *Except* 6-7: 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied, except 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD 2x4 SP No.2 *Except* 14-15,13-14: 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2x4 SP No.3 *Except* 6-15,7-13: 2x4 SP No.2	WEBS 1 Row at midpt 6-15

REACTIONS. (lb/size) 2=252/0-5-8, 15=2530/0-5-8, 11=1049/0-5-8
Max Horz 2=126(LC 11)
Max Uplift 2=81(LC 12), 15=280(LC 12), 11=180(LC 13)
Max Grav 2=369(LC 23), 15=2530(LC 1), 11=1067(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=89/553, 3-5=242/1088, 5-6=87/1049, 6-7=559/238, 7-8=1157/690,
8-10=1200/552, 10-11=1858/721
BOT CHORD 2-16=456/154, 15-16=456/154, 14-15=264/348, 13-14=40/918, 12-13=549/1589,
11-12=549/1589
WEBS 3-16=0/332, 3-15=707/375, 5-15=360/292, 6-15=1787/397, 6-14=150/1048,
7-14=425/390, 7-13=444/494, 8-13=330/273, 10-13=699/382, 10-12=0/335

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCCL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II, Exp C, Encl., GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 15=280, 11=180.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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MiTek

6904 Parke East Blvd
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227308
1559185	T15	PIGGYBACK BASE	1	1		

Buiders FirstSource, Lake City, FL

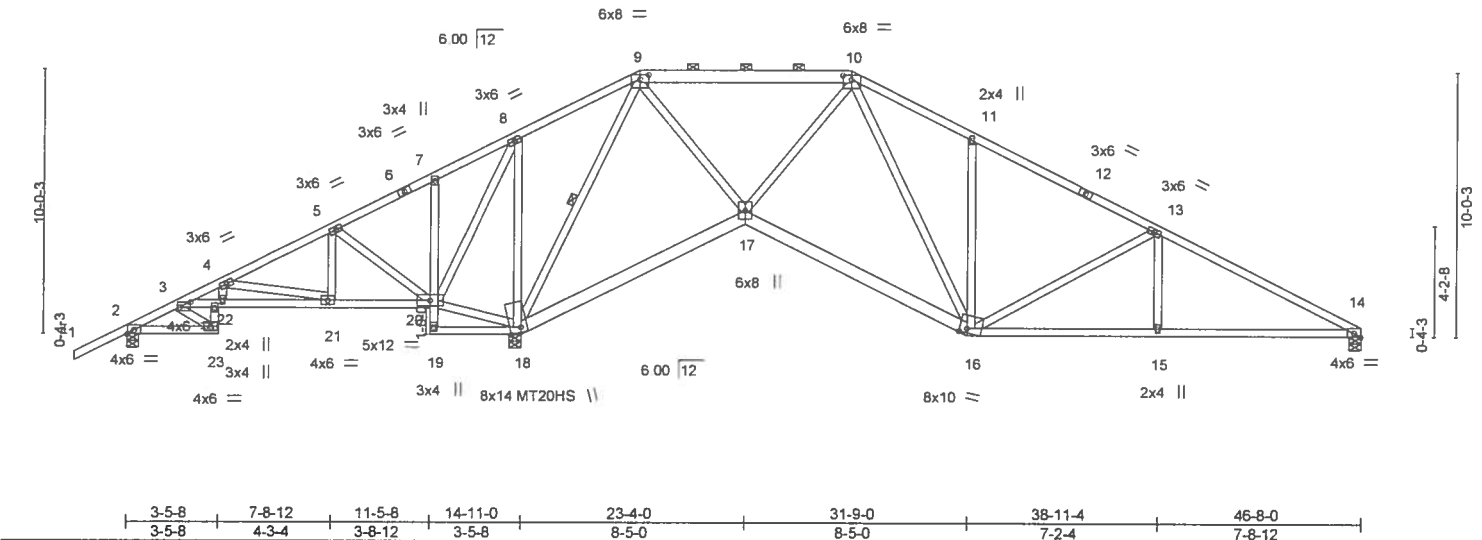
8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 22 2019 Page 1

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Job Reference (optional)

2-0-0	3-5-8	7-8-12	11-5-8	14-11-0	19-4-0	27-4-0	31-9-0	38-11-4	46-8-0
2-0-0	3-5-8	4-3-4	3-8-12	3-5-8	4-5-0	8-0-0	4-5-0	7-2-4	7-8-12

Scale = 1/84 3



LOADING (psf)	SPACING-	CSL	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.94	Vert(LL)	-0.10 14-15	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL 1.25	BC 0.72	Vert(CT)	-0.24 14-15	>999	180	MT20HS	187/143
BCLL 0.0 *	Rep Stress Incr YES	WB 0.91	Horz(CT)	0.06 14	n/a	n/a		
BCDL 10.0	Code FBC2017/TP12014	Matrix-S						
							Weight: 309 lb	FT = 20%

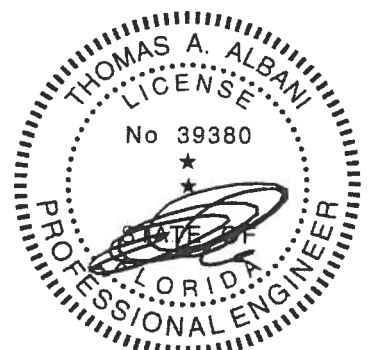
LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 *Except* 9-10: 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied, except 2-0-0 oc purlins (6-0-0 max.): 9-10.
BOT CHORD 2x4 SP No.2 *Except* 22-23,7-19: 2x4 SP No.3, 17-18,16-17: 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2x4 SP No.3 *Except* 9-18,10-16: 2x4 SP No.2	WEBS 1 Row at midpt 9-18

REACTIONS. (lb/size) 2=271/0-5-8, 18=2514/0-5-8, 14=1046/0-5-8
Max Horz 2=126(LC 11)
Max Uplift 2=-59(LC 12), 18=-313(LC 12), 14=-188(LC 13)
Max Grav 2=342(LC 23), 18=2514(LC 1), 14=1092(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=240/479, 4-5=122/558, 5-7=209/901, 7-8=128/905, 8-9=102/1047,
9-10=667/251, 10-11=1212/696, 11-13=1254/558, 13-14=1911/727
BOT CHORD 3-22=393/233, 21-22=406/210, 20-21=483/231, 17-18=287/352, 16-17=48/990,
15-16=554/1637, 14-15=554/1637
WEBS 4-21=427/186, 5-20=443/219, 8-18=457/291, 9-18=1793/416, 9-17=149/1072,
10-17=431/392, 10-16=447/490, 11-16=330/274, 13-16=699/382, 13-15=0/335,
18-20=881/481, 8-20=139/312

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10, Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 18=313, 14=188.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Date:

June 3, 2019

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MiTek

6904 Parke East Blvd
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227309
1559185	T15A	PIGGYBACK BASE	3	1		

Builders FirstSource, Lake City, FL

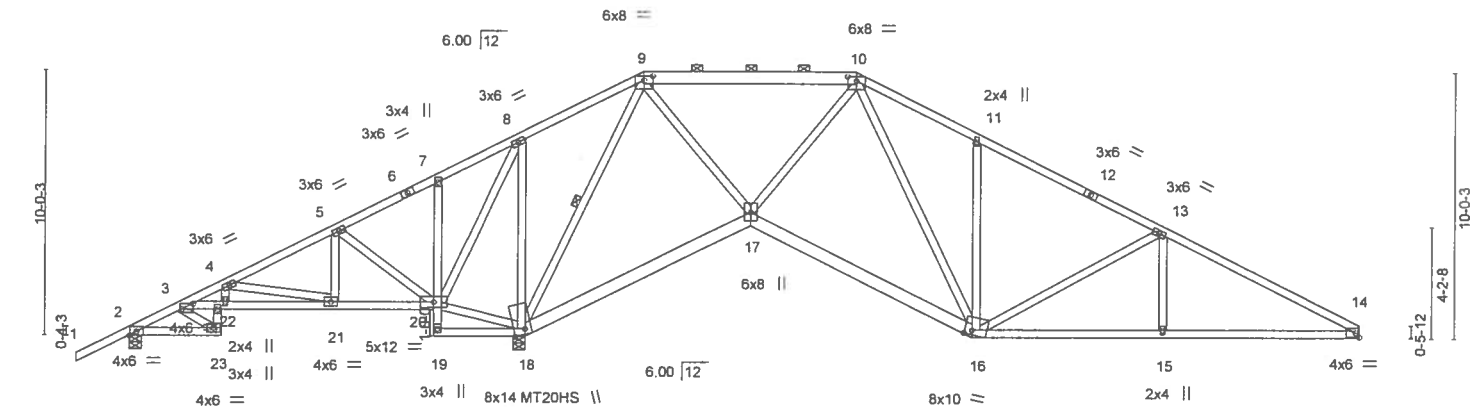
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Job Reference (optional)

2-0-0 3-5-8 7-8-12 11-5-8 14-11-0 19-4-0 27-4-0 31-9-0 38-11-4 46-4-14
2-0-0 3-5-8 4-3-4 3-8-12 3-5-8 4-5-0 8-0-0 4-5-0 7-2-4 7-5-10

Scale = 1/83.8



2-0-0 3-5-8 7-8-12 11-5-8 14-11-0 23-4-0 31-9-0 38-11-4 46-4-14	2-0-0 1-5-8 4-3-4 3-8-12 3-5-8 8-5-0 8-5-0 7-2-4 7-5-10
Plate Offsets (X,Y) =	[3:0-4-13,0-2-3], [9:0-4-0,0-1-15], [10:0-4-0,0-1-15], [16:0-3-6,0-1-15], [18:Edge,0-5-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.91	Vert(LL)	-0.09 14-15	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.70	Vert(CT)	-0.22 14-15	>999	180	MT20HS	187/143
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.91	Horz(CT)	0.06 14	n/a	n/a		
BCDL 10.0	Code	FBC2017/TPI2014	Matrix-S					Weight: 308 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD	TOP CHORD
2x4 SP No.2 *Except*	Structural wood sheathing directly applied, except
9-10: 2x6 SP No.2	2-0-0 oc purlins (6-0-0 max.): 9-10.
BOT CHORD	BOT CHORD
2x4 SP No.2 *Except*	Rigid ceiling directly applied or 6-0-0 oc bracing.
22-23,7-19: 2x4 SP No.3, 17-18,16-17: 2x6 SP No.2	WEBS
WEBS	1 Row at midpt
2x4 SP No.3 *Except*	9-18
9-18,10-16: 2x4 SP No.2	

REACTIONS. (lb/size) 2=274/0-5-8, 18=2506/0-5-8, 14=1043/Mechanical
Max Horz 2=126(LC 11)
Max Uplift 2=59(LC 12), 18=312(LC 12), 14=188(LC 13)
Max Grav 2=344(LC 23), 18=2506(LC 1), 14=1089(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=247/465, 4-5=119/549, 5-7=206/892, 7-8=125/897, 8-9=99/1040,
9-10=668/251, 10-11=1207/695, 11-13=1249/556, 13-14=1894/721
BOT CHORD 3-22=381/232, 21-22=395/208, 20-21=474/228, 17-18=285/350, 16-17=47/988,
15-16=546/1618, 14-15=546/1618
WEBS 4-21=429/188, 5-20=443/219, 8-18=458/292, 9-18=1787/414, 9-17=147/1069,
10-17=426/390, 10-16=445/485, 11-16=332/275, 13-16=682/375, 13-15=0/333,
18-20=875/479, 8-20=140/314

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph, TCDL=4.2psf, BCDL=3.0psf; h=18ft, Cat. II; Exp C, Encl., GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 18=312, 14=188.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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June 3,2019

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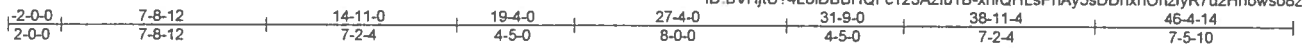
6904 Parke East Blvd
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227310
1559185	T16A	PIGGYBACK BASE	3	1		

Buiders FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 25 2019 Page 1

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Scale = 1/83.8

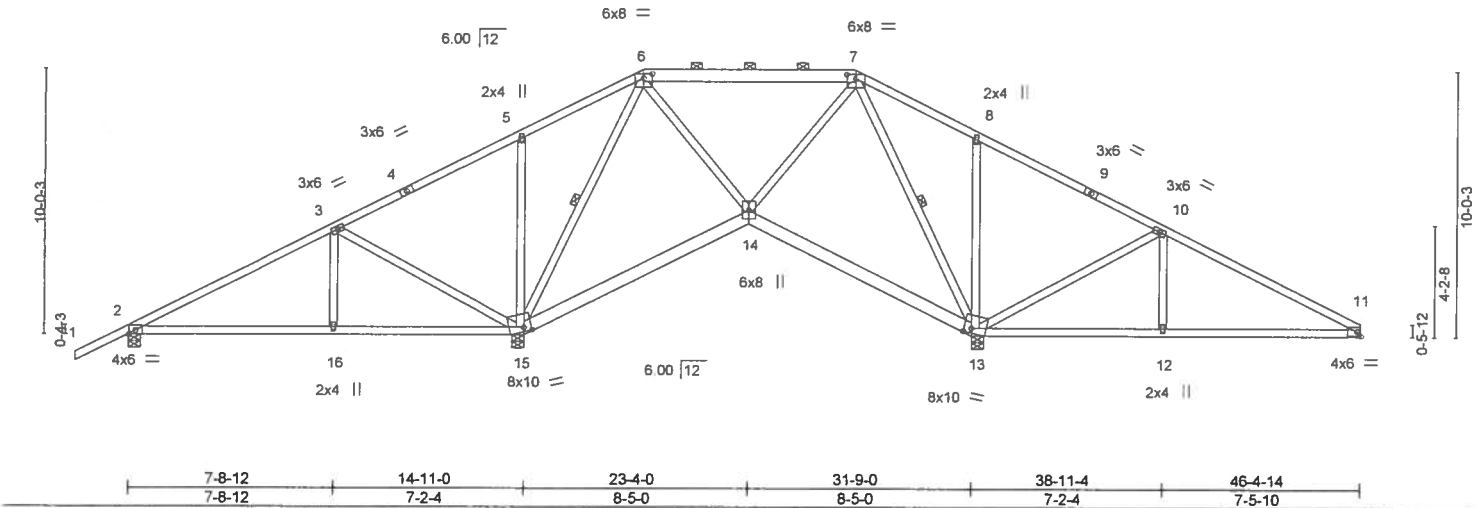


Plate Offsets (X,Y)--- [6:0-4-0,0-1-15], [7:0-4-0,0-1-15], [13:0-3-6,0-1-15], [15:0-3-6,0-1-15]											
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d		PLATES		GRIP	
TCLL	20.0	Plate Grip DOL 1.25		TC	0.70	Vert(LL)	-0.08 11-12 >999 240	MT20		244/190	
TCDL	10.0	Lumber DOL 1.25		BC	0.52	Vert(CT)	-0.18 11-12 >981 180	Weight: 279 lb FT = 20%			
BCLL	0.0 *	Rep Stress Incr YES		WB	0.89	Horz(CT)	0.03 11 n/a n/a				
BCDL	10.0	Code FBC2017/TPI2014		Matrix-S							

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2 *Except* 6-7: 2x6 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD	2x4 SP No.2 *Except* 14-15,13-14: 2x6 SP No.2	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	2x4 SP No.3 *Except* 6-15,7-13: 2x4 SP No.2	WEBS	1 Row at midpt 6-15, 7-13

REACTIONS. All bearings 0-5-8 except (jt=length) 11=Mechanical.
 (lb) - Max Horz 2=126(LC 9)
 Max Uplift All uplift 100 lb or less at joint(s) 2, 11 except 15=232(LC 12), 13=208(LC 13)
 Max Grav All reactions 250 lb or less at joint(s) except 2=536(LC 23), 15=1476(LC 23), 13=1489(LC 24), 11=384(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=397/172, 3-5=48/485, 5-6=0/450, 7-8=0/458, 8-10=21/490, 10-11=387/180
 BOT CHORD 2-16=76/278, 15-16=76/278, 14-15=226/327, 13-14=225/329, 12-13=68/285, 11-12=68/285
 WEBS 3-16=0/332, 3-15=696/371, 5-15=351/289, 6-15=688/69, 7-13=695/30, 8-13=344/279, 10-13=723/390, 10-12=0/335

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11 except (jt=lb) 15=232, 13=208.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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 Date:

June 3, 2019

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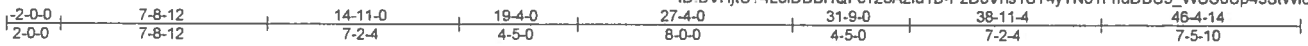
Job 1559185	Truss T17A	Truss Type PIGGYBACK BASE	Qty 4	Ply 1	O'Quinn Residence	T17227311
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Buiders FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 26 2019 Page 1

ID: BVHjIU74L8iDBBHQFc123Azlu1B-PzDoVhs1ST4yTN01FfIdDBC5_WCS0Cp45SiWlqzA_?

Job Reference (optional)



Scale = 1/83.1

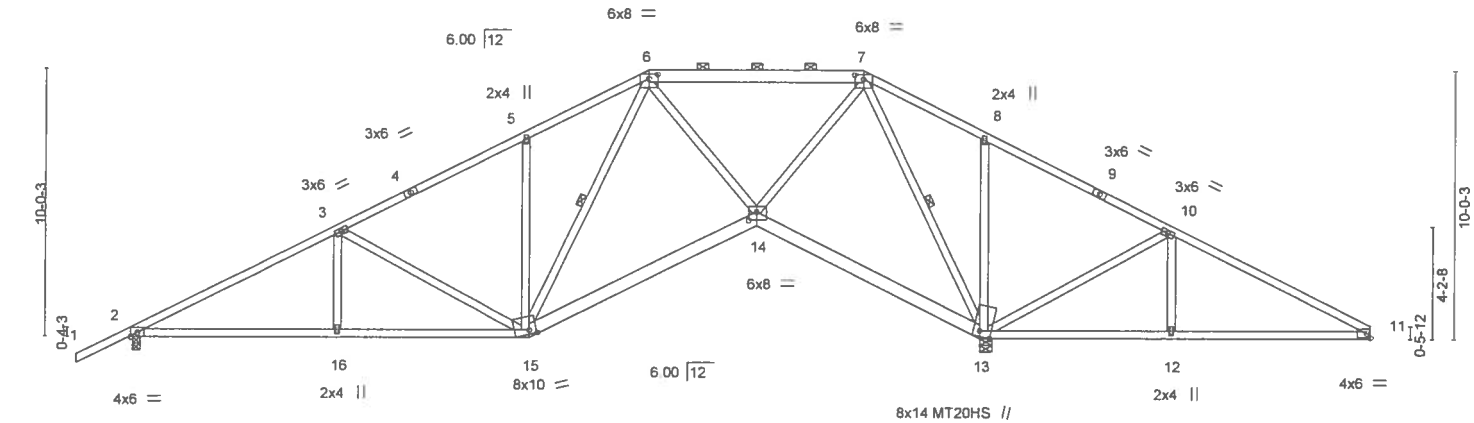


Plate Offsets (X,Y) =	[6:0-4-0,0-1-15], [7:0-4-0,0-1-15], [13:Edge,0-5-0], [14:0-3-8,0-3-12], [15:0-3-6,0-1-15]
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LOADING (psf)	SPACING-	CSL	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.77	in (loc) l/def L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.25	BC 0.70	Vert(LL) 0.27 14-15 >999 240	MT20HS	187/143
BCLL 0.0	Lumber DOL 1.25	WB 0.97	Vert(CT) 0.23 14-15 >999 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-MS	Horz(CT) 0.08 13 n/a n/a		
	Code FBC2017/TPI2014			Weight: 279 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 *Except*	TOP CHORD Structural wood sheathing directly applied or 3-4-11 oc purlins, except
6-7: 2x6 SP No.2	2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD 2x4 SP No.2 *Except*	Rigid ceiling directly applied or 4-2-2 oc bracing.
14-15,13-14: 2x6 SP No.2	1 Row at midpt 6-15, 7-13
WEBS 2x4 SP No.3 *Except*	
6-15,7-13: 2x4 SP No.2	

REACTIONS. (lb/size) 13=2441/0-5-8, 11=184/Mechanical, 2=1208/0-3-8
Max Horz 2=128(LC 12)
Max Uplift 13=549(LC 9), 11=96(LC 13), 2=376(LC 9)
Max Grav 13=2441(LC 1), 11=295(LC 24), 2=1215(LC 23)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=1910/1904, 3-5=1229/1415, 5-6=1187/1551, 6-7=618/935, 7-8=483/900,
8-10=633/926, 10-11=449/433
BOT CHORD 2-16=1614/1633, 15-16=1614/1633, 14-15=600/959, 13-14=157/396, 12-13=349/467,
11-12=349/467
WEBS 3-16=252/330, 3-15=712/704, 5-15=342/281, 6-15=777/453, 6-14=351/284,
7-14=1172/1064, 7-13=1737/1490, 8-13=345/286, 10-13=746/393, 10-12=0/338

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11 except (jt=lb) 13=549, 2=376.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227312
1559185	T17G	GABLE GABLE L GABLE	1	2	Job Reference (optional)	

Buders FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 32 2019 Page 2
ID: BVHjtU74L8iDBBHQFc123Azlu1B-E7a4lkxo1Jr6BIGBcvP1TSSCMxK4Qx5yTOKrUUz9zzv

NOTES-

- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17, 15, 13 except (jt=lb) 2=424, 19=963, 12=104, 16=466.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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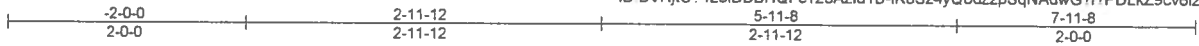


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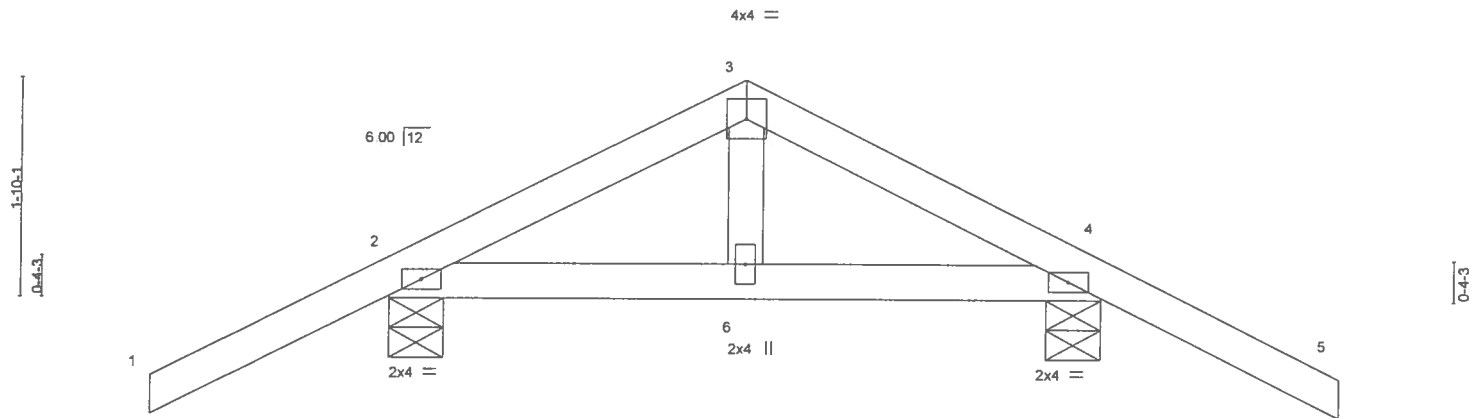
Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227313
1559185	T18	Common	1	1		

Buiders FirstSource, Lake City, FL

8.240 s May 13 2019 MiTek Industries, Inc Mon Jun 3 13 31.33 2019 Page 1
ID BVHjtU?4L8iDBBHQFc123Azlu1B-iK8Sz4yQodzzpSqNAdwG?7f7PDLkZ9cv6i24O1wz9zzu



Scale = 1:18.7



LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL	20.0	Plate Grip DOL	1.25	TC	0.33	Vert(LL)	-0.00 6 >999 240	MT20		244/190	
TCDL	10.0	Lumber DOL	1.25	BC	0.09	Vert(CT)	-0.00 4-6 >999 180				
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00 4 n/a n/a				
BCDL	10.0	Code FBC2017/TPI2014		Matrix-P							
								Weight	28 lb	FT = 20%	

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-11-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 2=354/0-5-8, 4=354/0-5-8
Max Horz 2=29(LC 10)
Max Uplift 2=68(LC 12), 4=68(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10, Vult=120mph (3-second gust) Vasd=93mph, TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



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Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227314
1559185	T18G	Common Supported Gable	1	1	Job Reference (optional)	

Buildings FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 33 2019 Page 1
ID: BVHjtU74L8iDBBHQFc123Azlu1B-iK8Sz4yQodzzpSqNAdwG7f7Q8Lkd9cN6i24O1wz9zzu

-2-0-0	2-11-12	5-11-8	7-11-8
2-0-0	2-11-12	2-11-12	2-0-0

Scale = 1:19.3

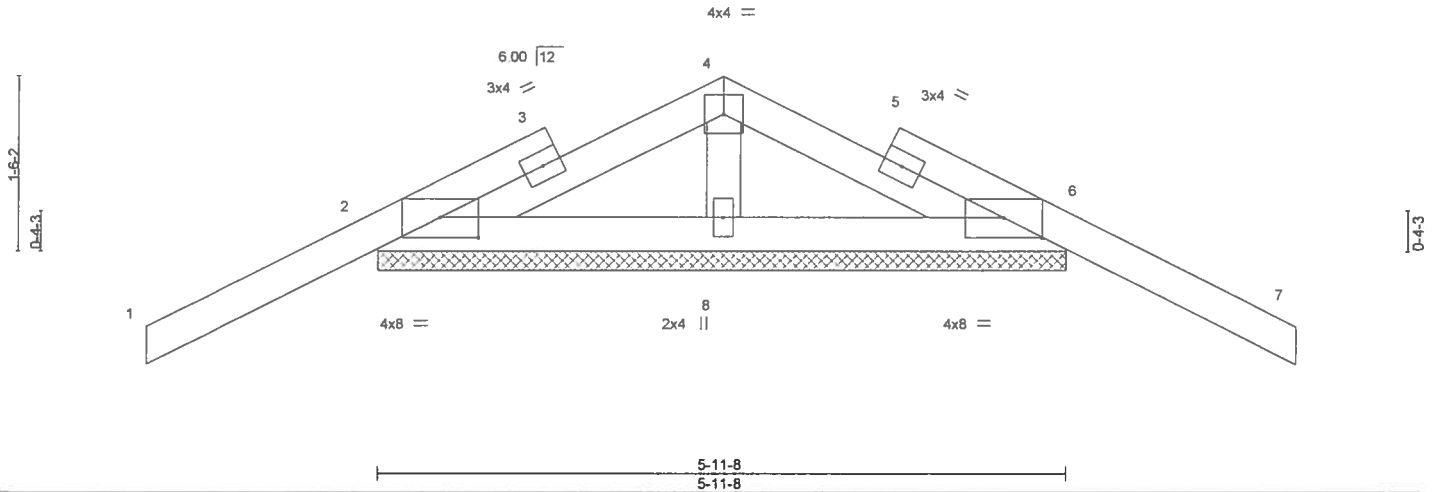


Plate Offsets (X,Y) - [2-0-4-0,0-2-1], [6-0-4-0,0-2-1]

LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	2-0-0	TC 0.27	Vert(LL)	-0.02	7	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.09	Vert(CT)	-0.03	7	n/r	120		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.02	Horz(CT)	0.00	6	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						Weight: 30 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-11-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS.

(lb/size) 2=265/5-11-8, 6=265/5-11-8, 8=187/5-11-8
Max Horz 2=39(LC 12)
Max Uplift 2=115(LC 12), 6=121(LC 13), 8=24(LC 12)
Max Grav 2=267(LC 23), 6=267(LC 24), 8=187(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (it=lb) 2=115, 6=121.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



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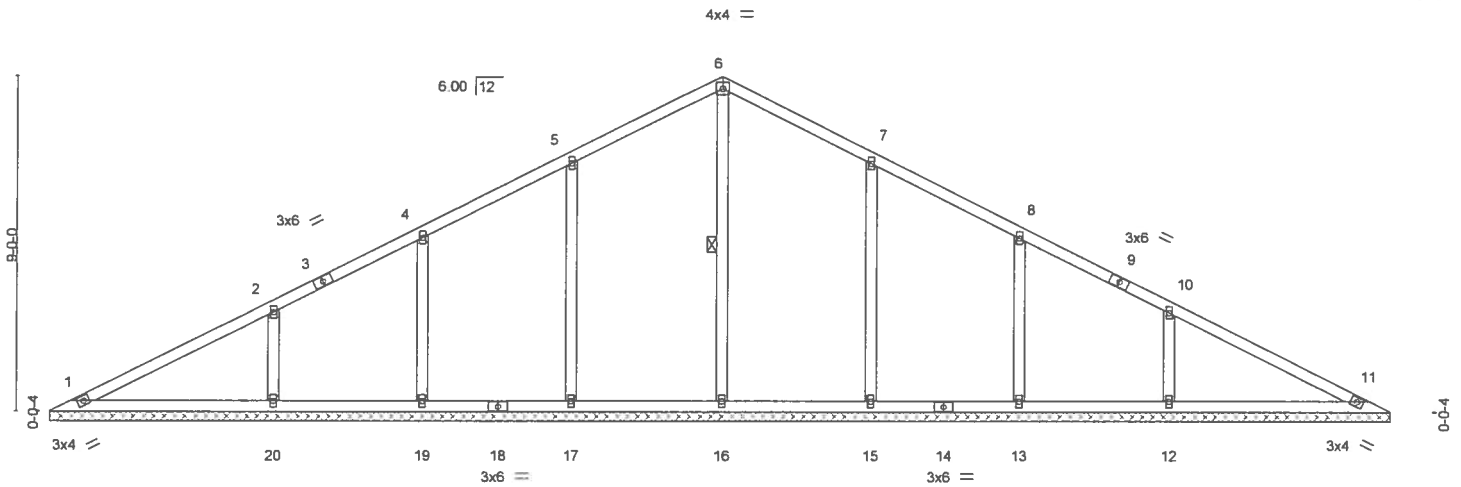
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8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 37 2019 Page 1
ID: BVHjtU?4L8iDBBHQFc123Azlu1B-b5Nzpr?xssTPI388PS_CAV94Gy3L5N3idf2cAhz9zzq

18-0-0	36-0-1
18-0-0	18-0-0

[illegible]

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 6-16

REACTIONS. All bearings 35-11-1.
(lb) - Max Horz 1=100(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 17, 19, 15, 13 except 20=122(LC 12), 12=122(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 11 except 16=379(LC 22), 17=444(LC 19), 19=306(LC 19),
20=451(LC 23), 15=444(LC 20), 13=306(LC 20), 12=451(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 5-6=119/264, 6-7=119/264
WEBS 5-17=267/211, 2-20=328/261, 7-15=267/211, 10-12=328/261

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft, Cat. II; Exp C; Encl., GCpf=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 17, 19, 15, 13 except (jt=lb) 20=122, 12=122.



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June 3.2019

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Job 1559185	Truss V3	Truss Type Valley	Qty 1	Ply 1	O'Quinn Residence	T17227317
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Builders FirstSource, Lake City, FL

8.240 s May 13 2019 MiTek Industries, Inc Mon Jun 3 13 31 38 2019 Page 1
ID BVHjtU74L8iDBBHQFc123Azlu1B-3HxL0n0Zd9bFvDjLyAVRijilGMQQqqTrrJn9i8z9zpp
32-0-1
16-0-0

Scale = 1/53 0

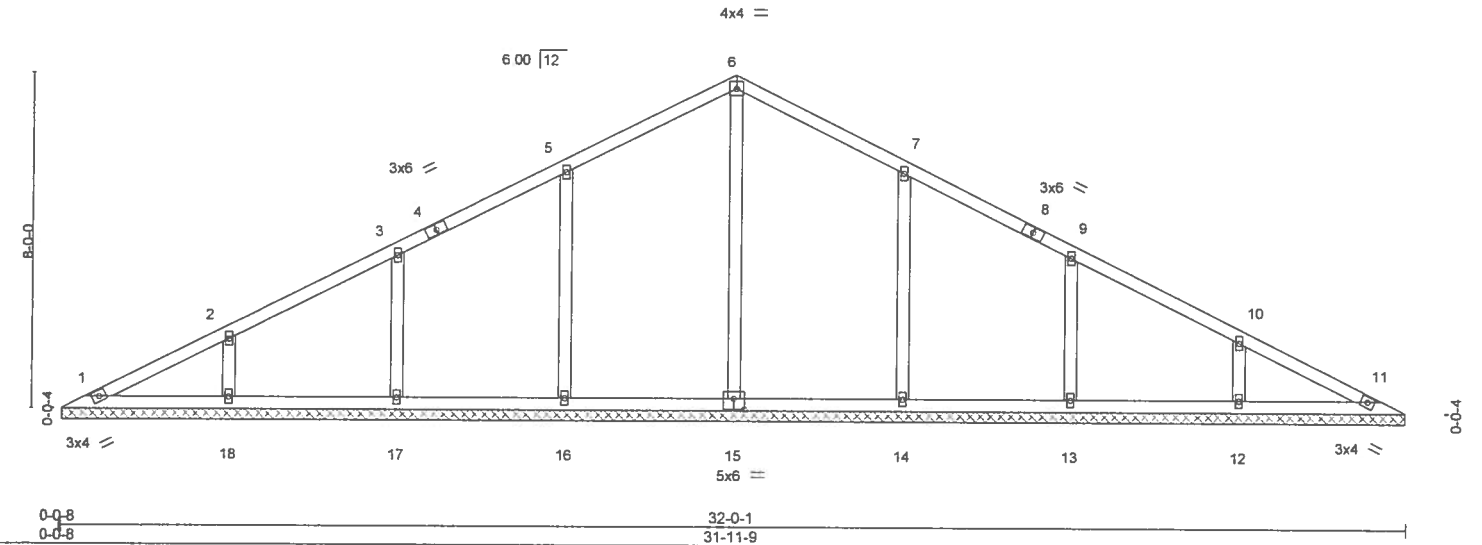


Plate Offsets (X,Y) - [15:0-3-0,0-3-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	L/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.18	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.25	BC 0.18	Vert(CT)	n/a	-	n/a		
BCLL 0.0	Lumber DOL 1.25	WB 0.22	Horz(CT)	0.00	11	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S						
	Code FBC2017/TPI2014						Weight: 140 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

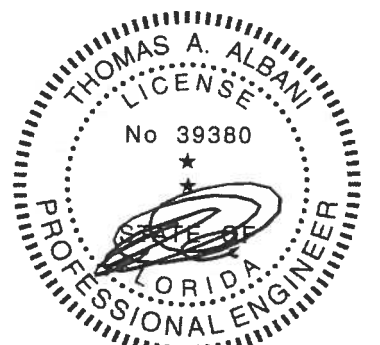
All bearings 31-11-1.
(lb) - Max Horz 1=89(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 1, 16, 17, 18, 14, 13, 12
Max Grav All reactions 250 lb or less at joint(s) 1, 11 except 15=376(LC 22), 16=433(LC 19), 17=345(LC 19), 18=330(LC 23), 14=433(LC 20), 13=345(LC 20), 12=330(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 5-16=260/206, 7-14=260/206

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCp=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 16, 17, 18, 14, 13, 12.



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June 3, 2019

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MiTek

6904 Parke East Blvd
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence
1559185	V4	Valley	1	1	

T17227318

Buiders FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13:31:40 2019 Page 1
ID BVHjtU74L8iDBBHQFc123Azlu1B-7g35RT1p9nrz9Xtj4bYvn8nbW952lke8JdGGm0z9zzn

Job Reference (optional)



Scale = 1/4" = 1'-0"

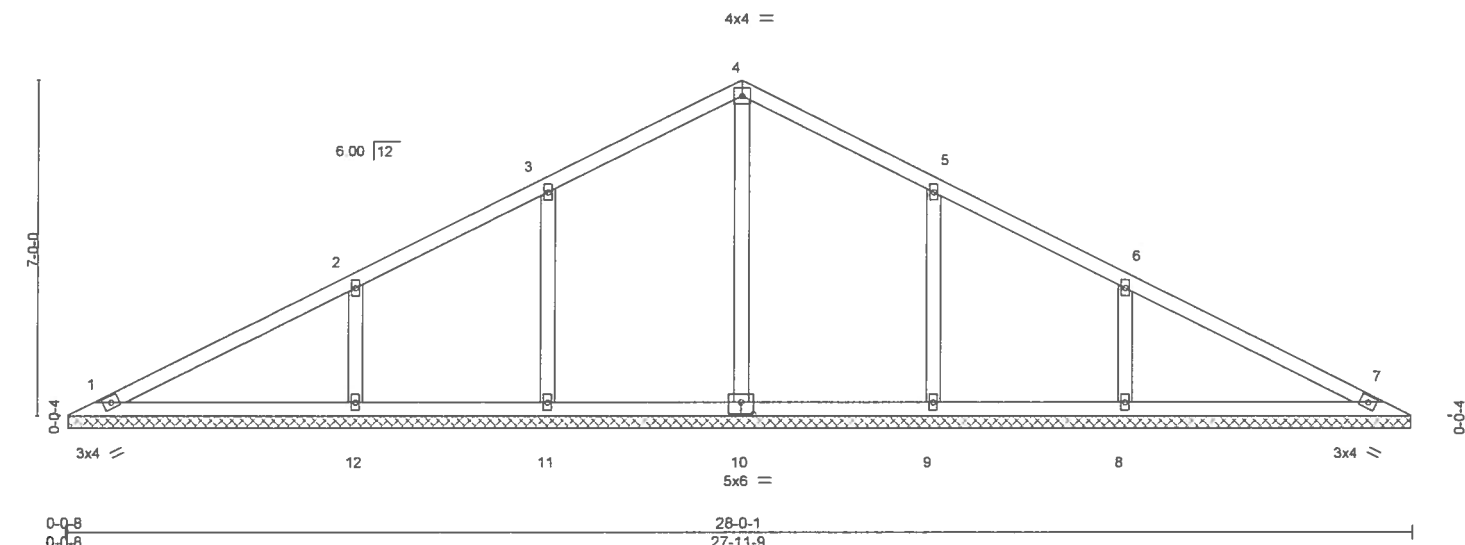


Plate Offsets (X,Y) = [10:0-3-0,0-3-0]

LOADING (psf)	SPACING-		CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.32	Vert(LL)	n/a	-	n/a	MT20	244/190
TCCL 10.0	Lumber DOL	1.25	BC 0.23	Vert(CT)	n/a	-	n/a		
BCCL 0.0 *	Rep Stress Incr	YES	WB 0.18	Horz(CT)	0.00	7	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						
								Weight: 116 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS.

All bearings 27-11-1.
 (lb) - Max Horz 1=77(LC 8)
 Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 11, 9 except 12=120(LC 12), 8=120(LC 13)
 Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=406(LC 19), 11=321(LC 19), 12=447(LC 1),
 9=321(LC 20), 8=447(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-12=323/259, 6-8=323/259

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCCL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 11, 9 except (jt=lb) 12=120, 8=120.



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 Date:

June 3, 2019

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6904 Parke East Blvd
 Tampa, FL 36610

Job 1559185	Truss V5	Truss Type Valley	Qty 1	Ply 1	O'Quinn Residence	T17227319
Buiders FirstSource, Lake City, FL						

8.240 s May 13 2019 MiTek Industries, Inc Mon Jun 3 13 31 41 2019 Page 1
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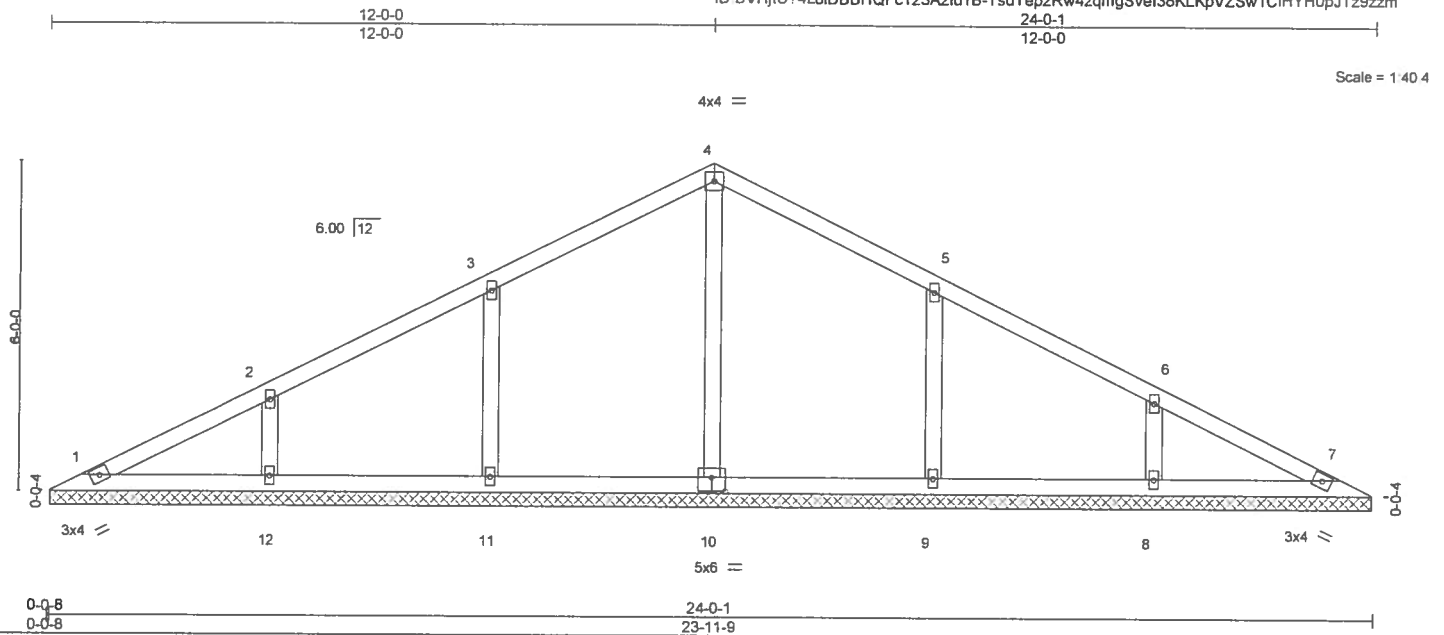


Plate Offsets (X,Y)-- [10:0-3-0-0-3-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.18	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.19	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.12	Horz(CT)	0.00	7	n/a		
BCDL 10.0	Code FBC2017/TP12014		Matrix-S					Weight: 96 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 23-11-1.
(lb) - Max Horz 1=66(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 11, 12, 9, 8
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=387(LC 19), 11=361(LC 19), 12=325(LC 1), 9=361(LC 20), 8=325(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-11=260/207, 5-9=260/207

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11, 12, 9, 8.



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June 3, 2019

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Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence
1559185	V6	Valley	1	1	

T17227320

Buiders FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 42 2019 Page 1
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Job Reference (optional)

10-0-0 10-0-0 20-0-1 10-0-0

Scale = 1/32.7

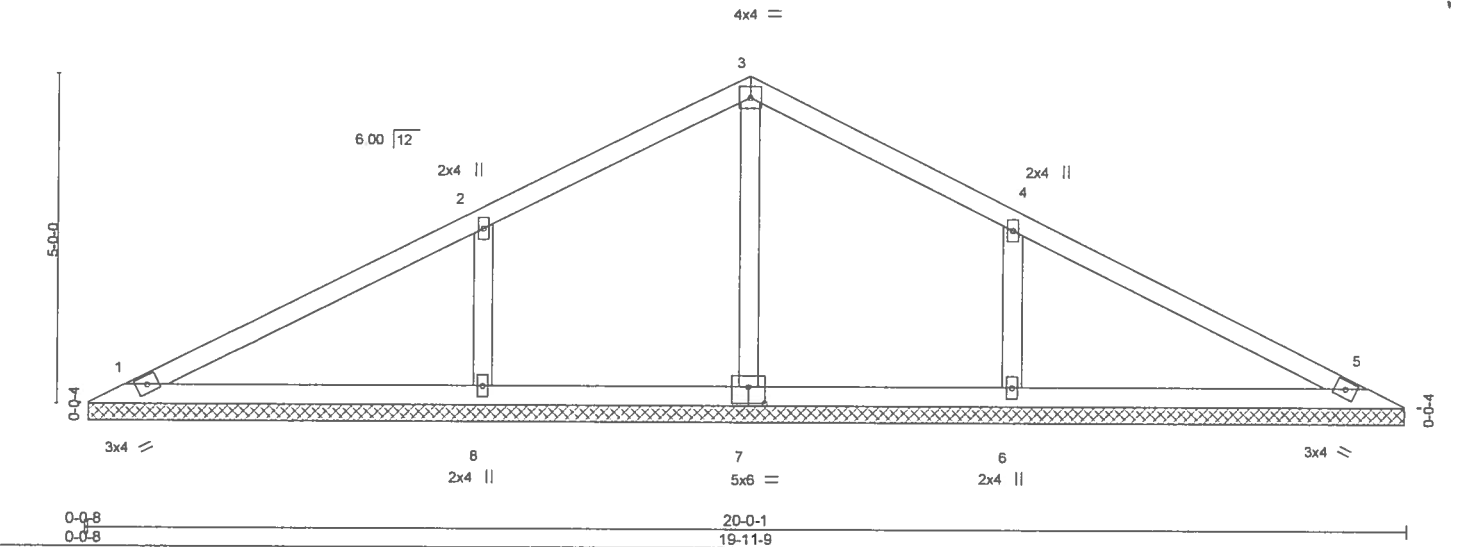


Plate Offsets (X,Y) - [7:0-3-0,0-3-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.33	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.25	BC 0.23	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.08	Horz(CT)	0.00	5	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S						
	Code FBC2017/TP12014						Weight: 75 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 19-11-1.
(lb) - Max Horz 1=54(LC 11)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=128(LC 12), 6=128(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=468(LC 23), 6=468(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.**WEBS** 2-8=343/271, 4-6=343/271**NOTES-**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=128, 6=128.



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June 3, 2019

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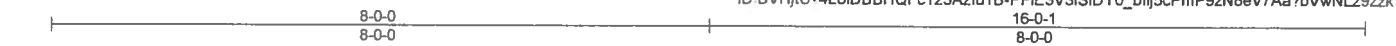
6904 Parke East Blvd
Tampa, FL 33610

Job 1559185	Truss V7	Truss Type Valley	Qty 1	Ply 1	O'Quinn Residence	T17227321
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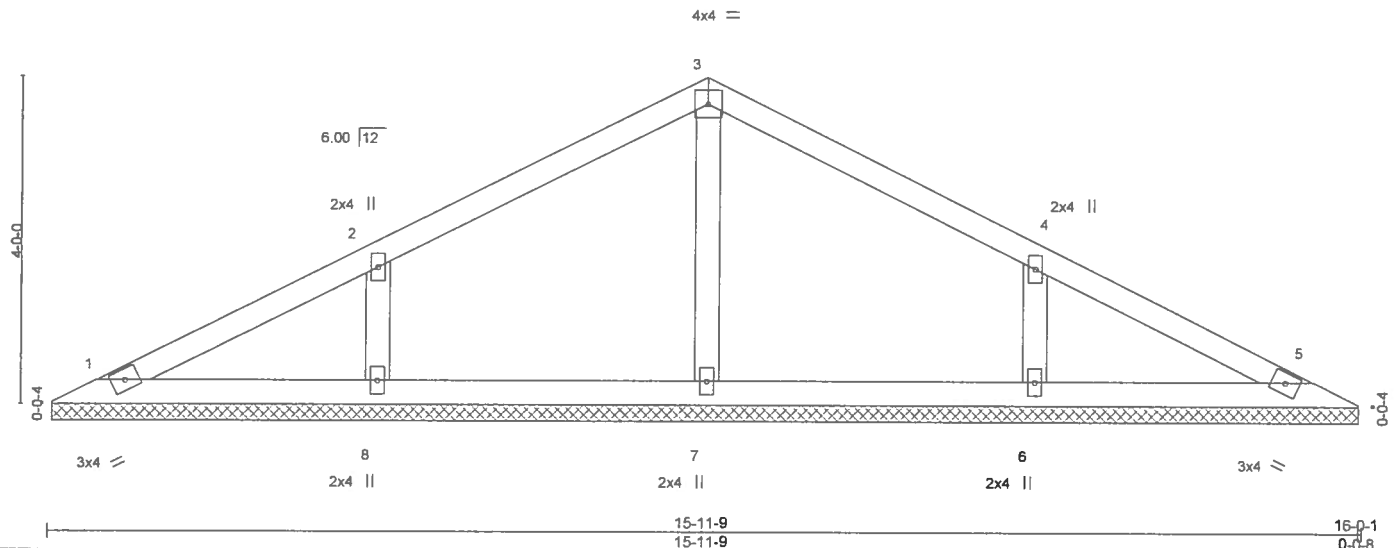
Builders FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 43 2019 Page 1
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Job Reference (optional)



Scale = 1/27.2



LOADING (psf)	SPACING-		CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25		TC 0.18	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Lumber DOL 1.25		BC 0.11	Vert(CT)	n/a	-	n/a		
BCLL 0.0	Rep Stress Incr YES		WB 0.06	Horz(CT)	0.00	5	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						
								Weight: 58 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 15-11-1.
(lb) - Max Horz 1=43(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 8, 6
Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=271(LC 1), 8=349(LC 23), 6=349(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-8=263/213, 4-6=263/213

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6.



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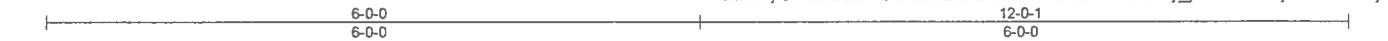
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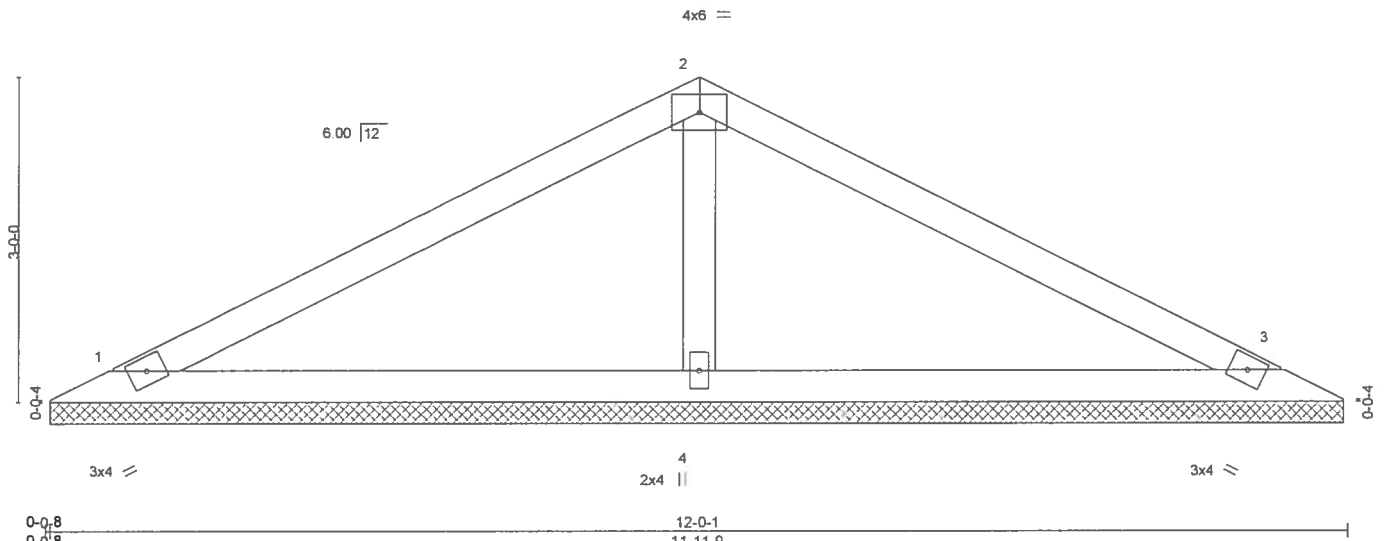
Job 1559185	Truss V8	Truss Type Valley	Qty 1	Ply 1	O'Quinn Residence	T17227322
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Buildiers FirstSource, Lake City, FL

8.240 s May 13 2019 MiTek Industries, Inc. Mon Jun '3 13 31 44 2019 Page 1
ID: BVHjtU74L8iDBBHQFc123Azlu1B-IRicHr4KD?LPd8AUJQcry_yHXnR5EZMjEFETvoz9zzj



Scale = 1/20.5



LOADING (psf)	SPACING-	CSI.	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.38	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL 1.25	BC 0.29	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.07	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2017/TP12014	Matrix-S						Weight: 39 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=196/11-11-1, 3=196/11-11-1, 4=468/11-11-1
Max Horz 1=31(LC 9)
Max Uplift 1=-38(LC 12), 3=-44(LC 13), 4=-43(LC 12)
Max Grav 1=198(LC 23), 3=198(LC 24), 4=468(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-4=-310/195

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft, Cat. II, Exp C, Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



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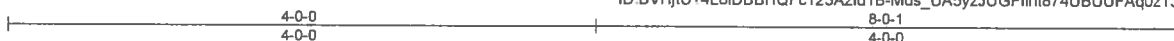
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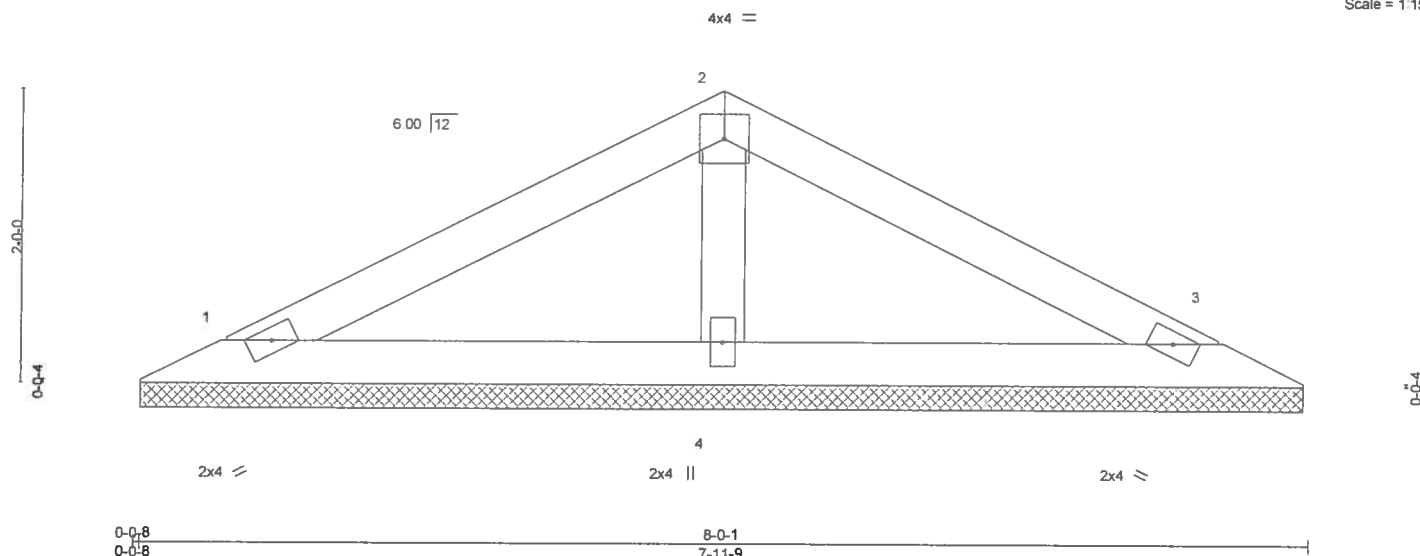
Job 1559185	Truss V9	Truss Type Valley	Qty 1	Ply 1	O'Quinn Residence	T17227323
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Buiders FirstSource, Lake City, FL

8 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 45 2019 Page 1
ID:BVHjtU74L8iDBBHQFc123Azlu1B-Mds_UA5yzJUGfllht874UBUUFaq0z13ISv_1SEz9zzi



Scale = 1:15.2



LOADING (psf)	SPACING-		CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.19	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.11	Vert(CT)	n/a	-	n/a		
BCLL 0.0	Rep Stress Incr	YES	WB 0.04	Horz(CT)	0.00	3	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P					Weight: 25 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=137/7-11-1, 3=137/7-11-1, 4=266/7-11-1
Max Horz 1=20(LC 9)
Max Uplift 1=29(LC 12), 3=33(LC 13), 4=16(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10, Vult=120mph (3-second gust) Vasd=93mph, TCDL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



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June 3, 2019

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6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	O'Quinn Residence	T17227324
1559185	V10	Valley	1	1		

Builders FirstSource.

Lake City, FL

B 240 s May 13 2019 MiTek Industries, Inc. Mon Jun 3 13 31 35 2019 Page 1
ID BVHjtU74L8iDBBHQFc123Azlu1B-eiGCOMzhKEDh2l_mH2yk444qB8Q3dWAP9LZV5pz9zss

2-0-0 2-0-0 4-0-1 2-0-0

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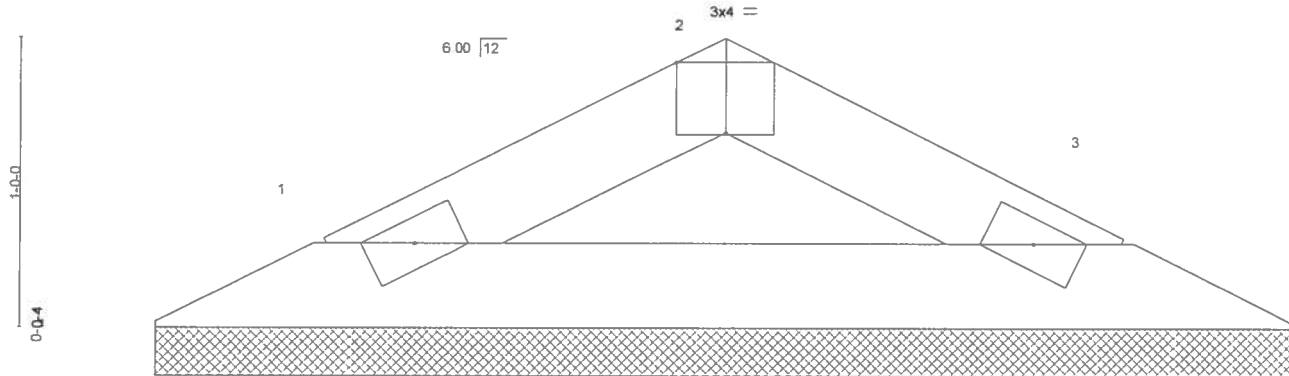


Plate Offsets (X,Y) -	[2:0-2-0,Edge]	4-0-1	3-11-9
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LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.04	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.09	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	0.00	3	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P					Weight: 10 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 4-0-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 1=110/3-11-1, 3=110/3-11-1
Max Horz 1=8(LC 11)
Max Uplift 1=15(LC 12), 3=15(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=93mph; TCDL=4.2psf, BCDL=3.0psf; h=18ft; Cat. II; Exp C, Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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6904 Parke East Blvd. Tampa FL 33610
Date:

June 3, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII.7473 rev. 10/03/2015 BEFORE USE.

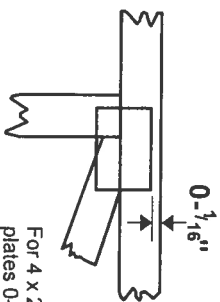
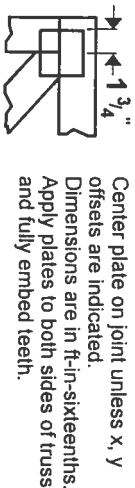
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.

MiTek

6904 Parke East Blvd
Tampa, FL 36610

Symbols

PLATE LOCATION AND ORIENTATION



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ " from outside edge of truss.

— This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MITek 20/20 software or upon request.

PLATE SIZE

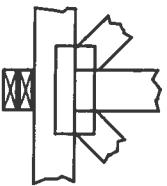
4 X 4
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



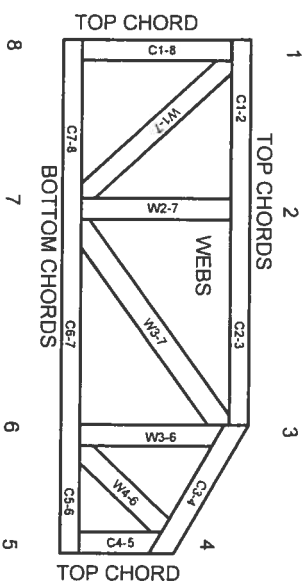
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft.-in.-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 1 section 6.3. These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: Mil-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

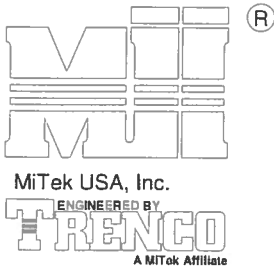
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and ware at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or punfs provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.

AUGUST 1, 2016

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

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Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

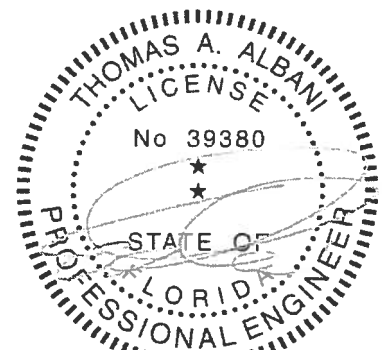
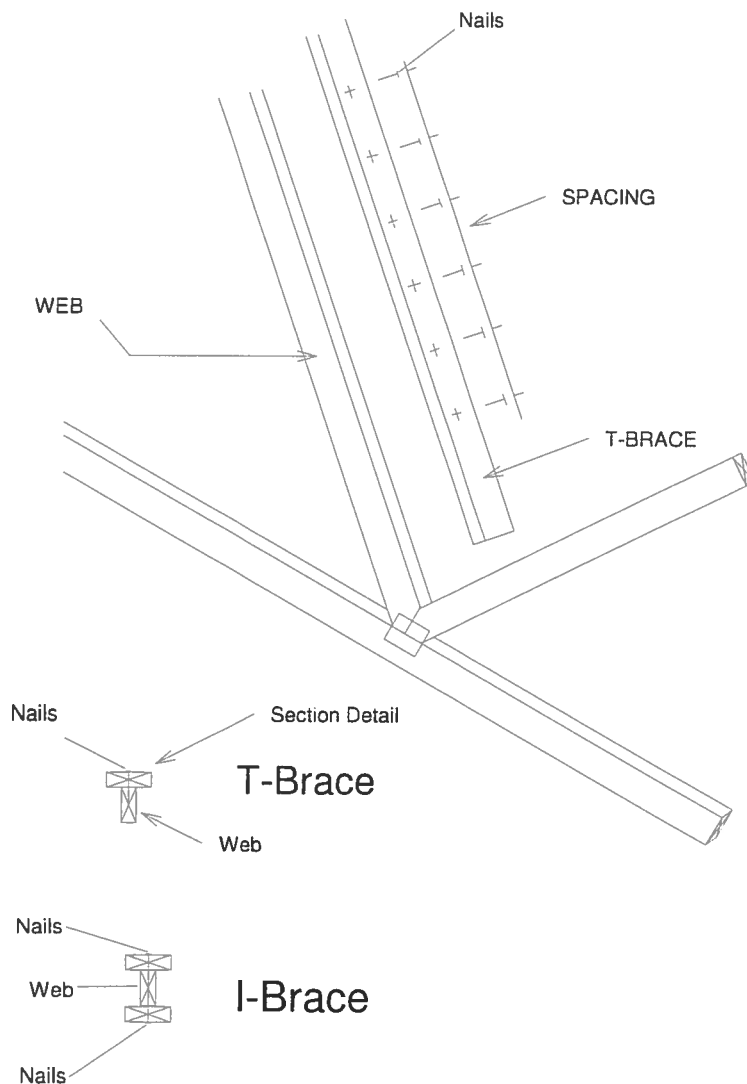
Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

Brace Size for One-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size for Two-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.



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February 12, 2018

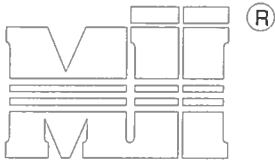
AUGUST 1, 2016

SCAB-BRACE DETAIL

MII-SCAB-BRACE

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Page 1 of 1



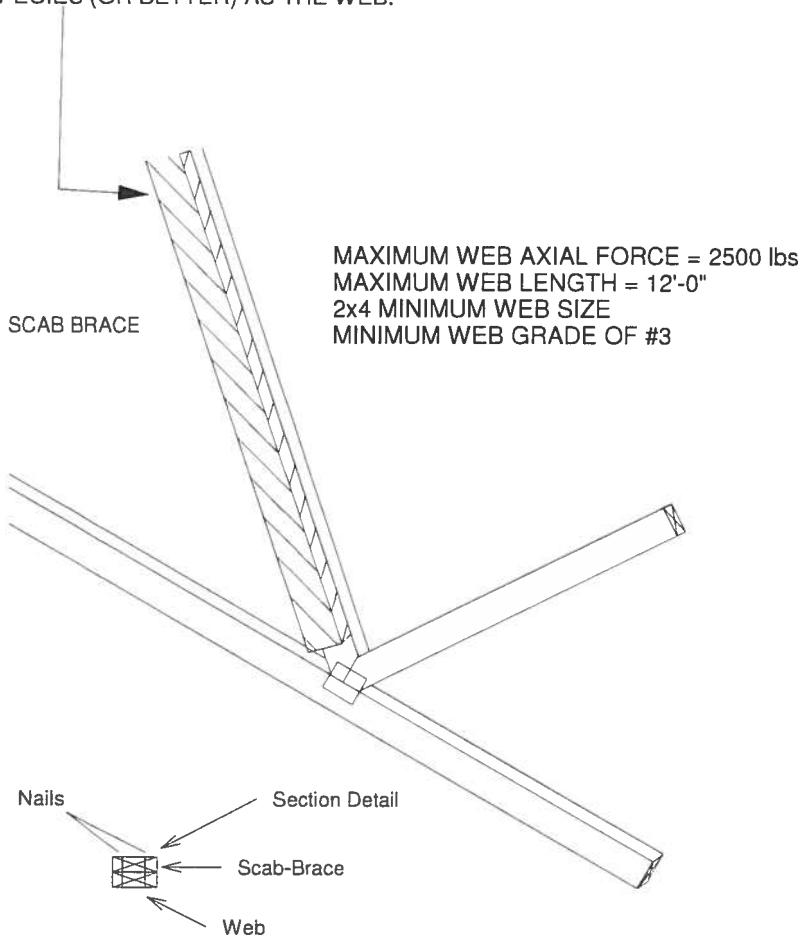
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ENGINEERED BY
TRENCO
A MiTek Affiliate

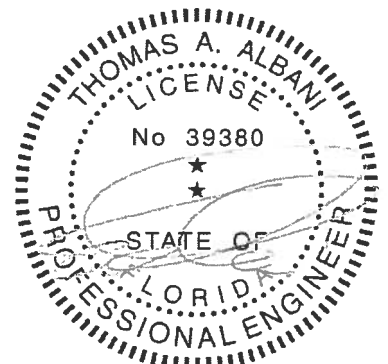
Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.
Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APPLICABLE WHEN BRACING IS REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x ____ SCAB TO ONE FACE OF WEB WITH
2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
SCAB MUST BE THE SAME GRADE, SIZE AND
SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.



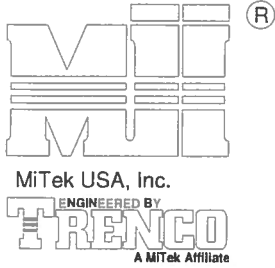
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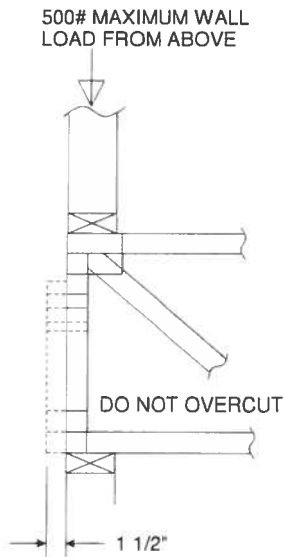
STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

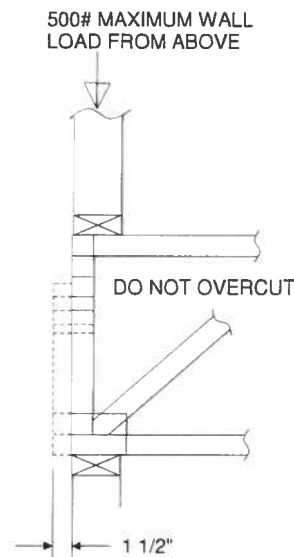


MiTek USA, Inc. Page 1 of 1

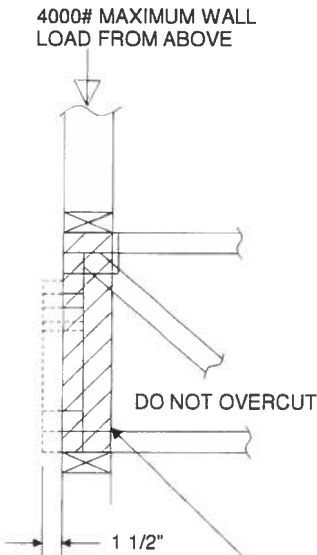
1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.
5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X ORIENTATION ONLY.
6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.



REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



TRUSSES BUILT WITH 4x2 MEMBERS

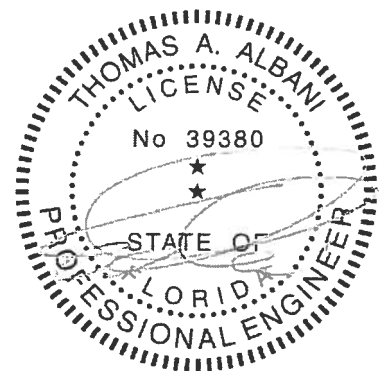


REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



TRUSSES BUILT WITH 4x2 MEMBERS

ATTACH 2x4 SQUASH BLOCK (CUT TO FIT TIGHTLY) TO BOTH SIDES OF THE TRUSS AS SHOWN WITH 10d (0.131" X 3") NAILS SPACED 3" O.C.



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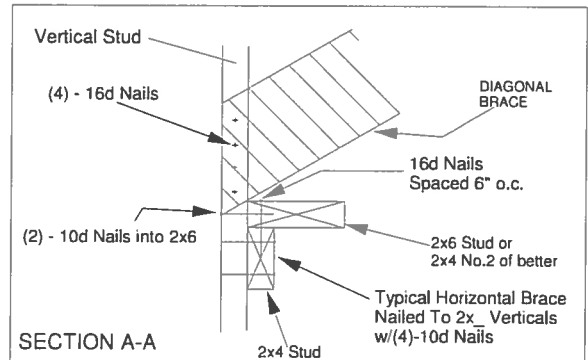
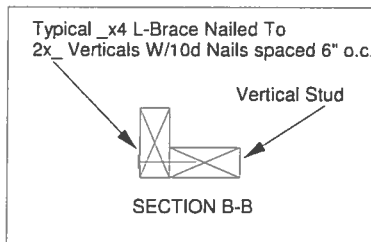
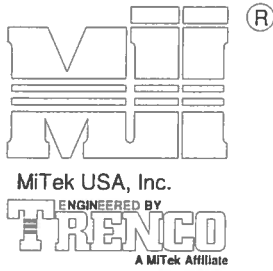
February 12, 2018

AUGUST 1, 2016

Standard Gable End Detail

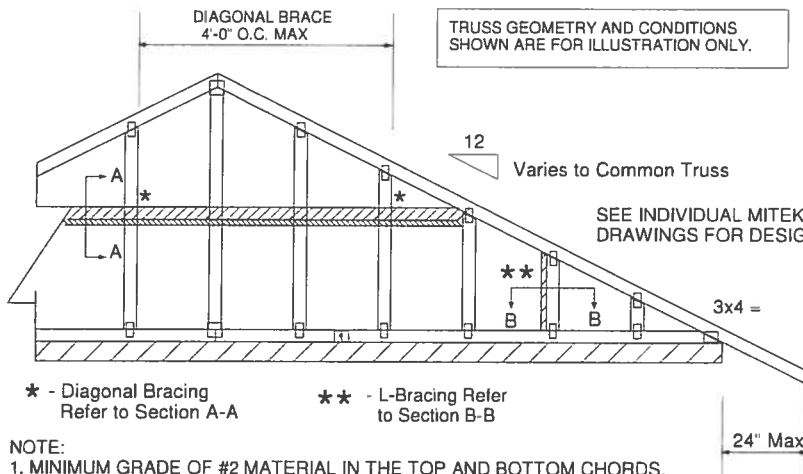
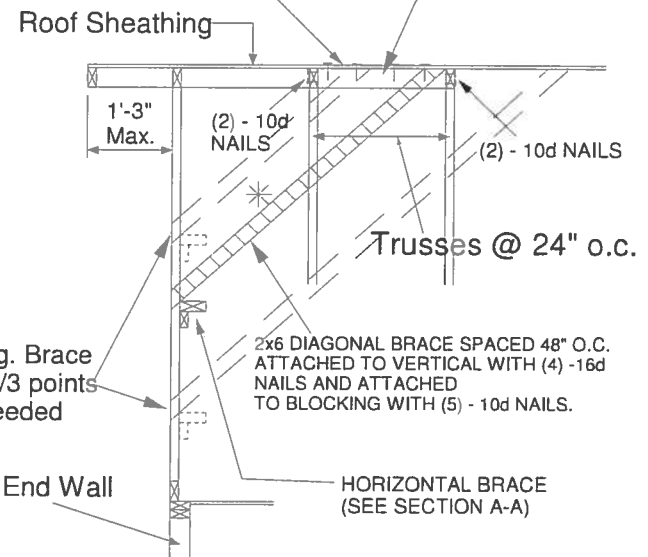
MII-GE130-D-SP

MiTek USA, Inc. Page 1 of 2



PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK



* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

NOTE:

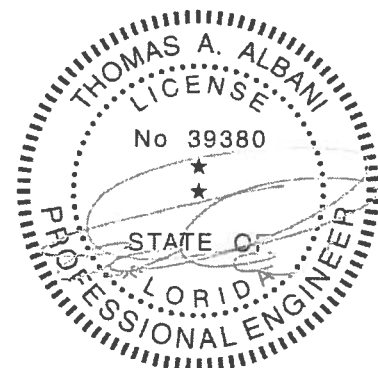
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
			Maximum Stud Length			
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE D
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



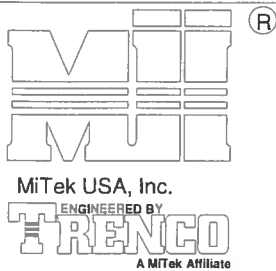
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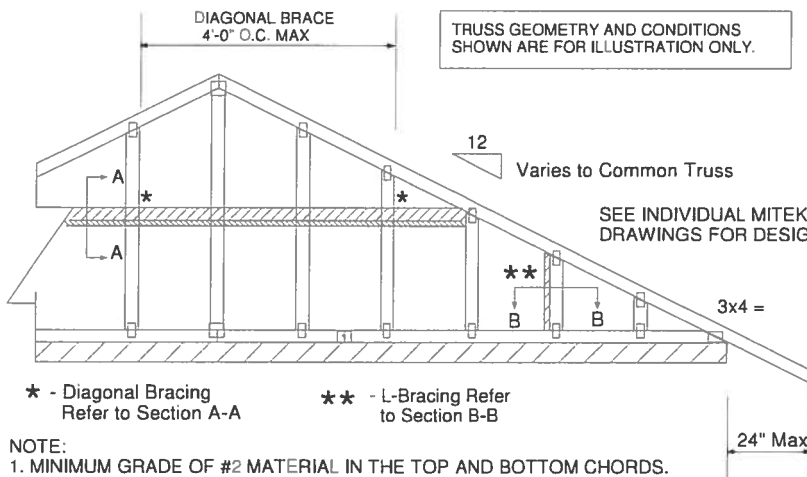
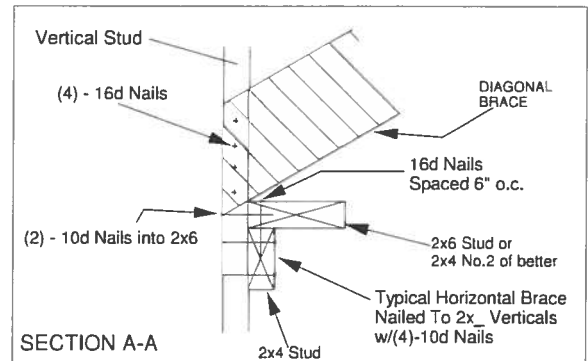
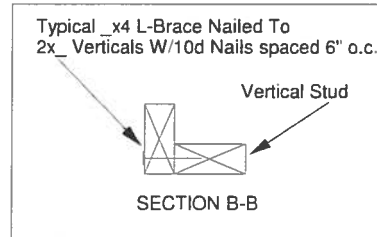
Standard Gable End Detail

MII-GE130-SP



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Page 1 of 2

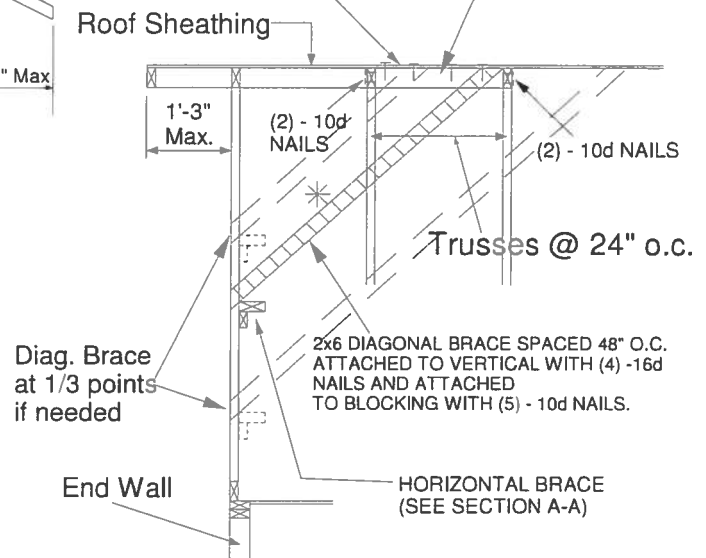


NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

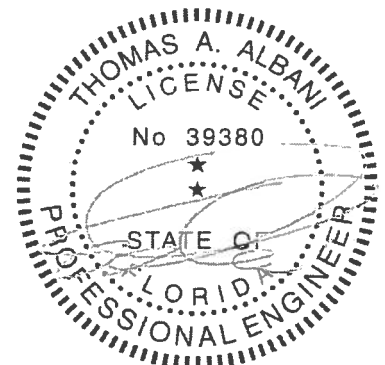


Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
			Maximum Stud Length			
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



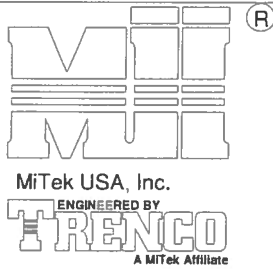
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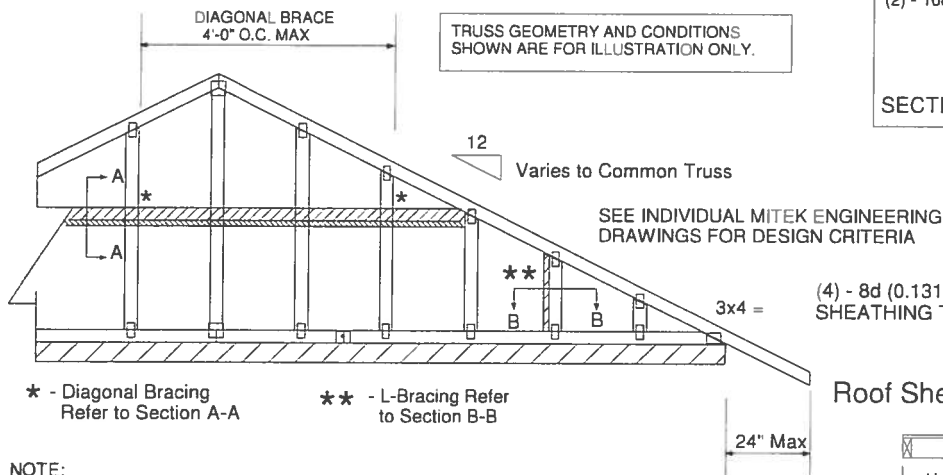
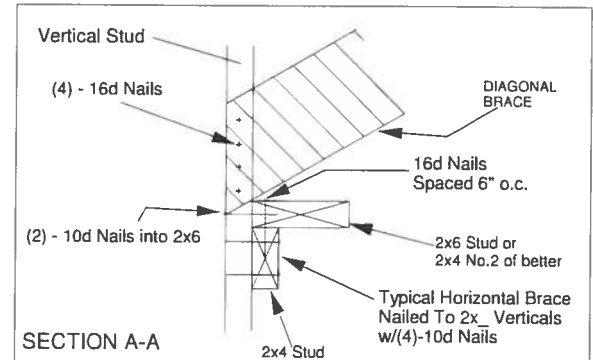
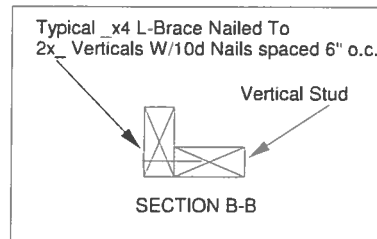
JANUARY 6, 2017

Standard Gable End Detail

MII-GE140-001



MiTek USA, Inc. Page 1 of 2

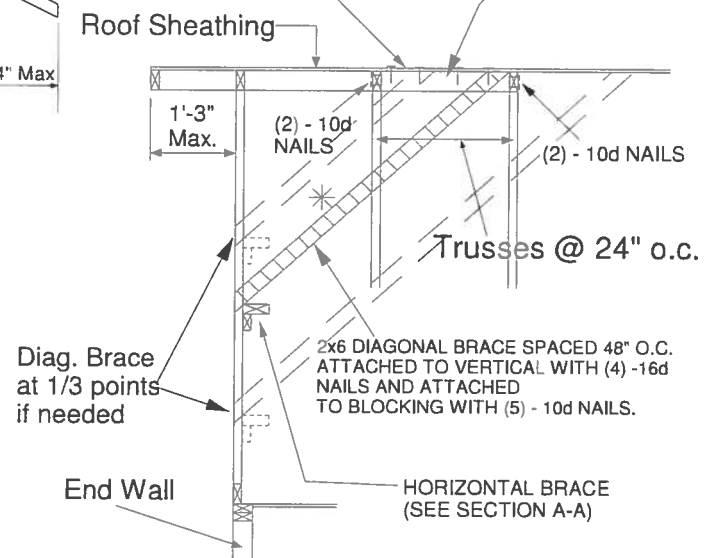


NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD DF/SPF BLOCK

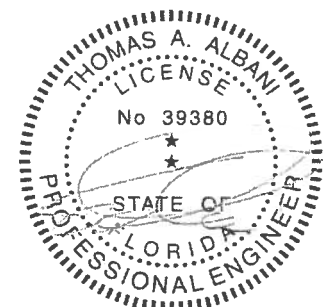


Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
			Maximum Stud Length			
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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Date:

January 19, 2018

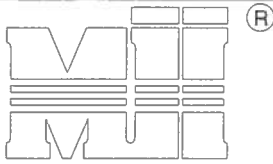
AUGUST 1, 2016

Standard Gable End Detail

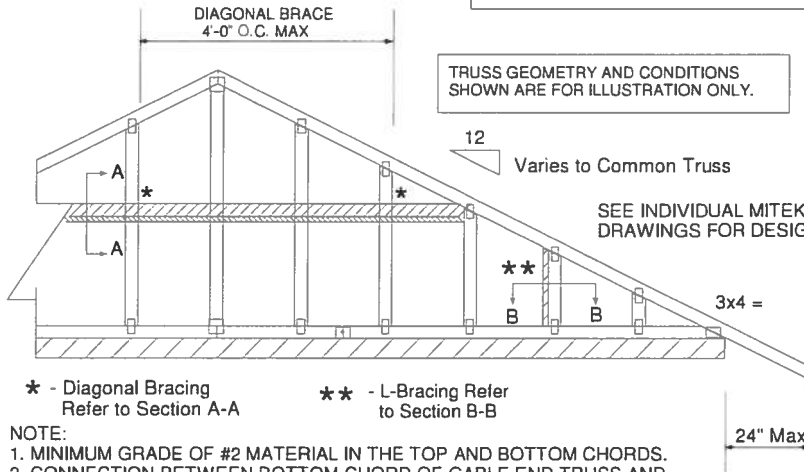
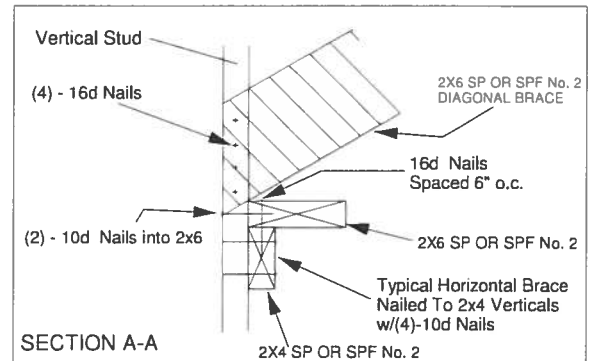
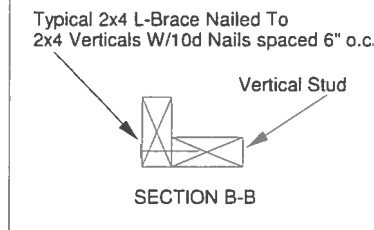
MII-GE170-D-SP

MiTek USA, Inc.

Page 1 of 2



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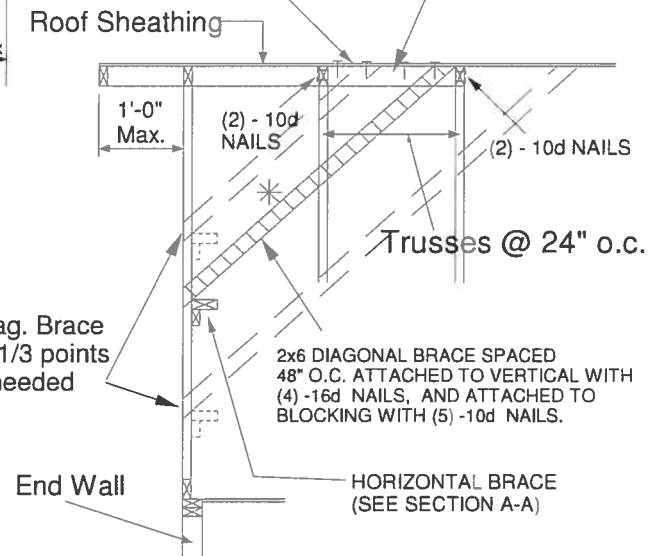
ENGINEERED BY
TRENCO
 A MiTek Affiliate


NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

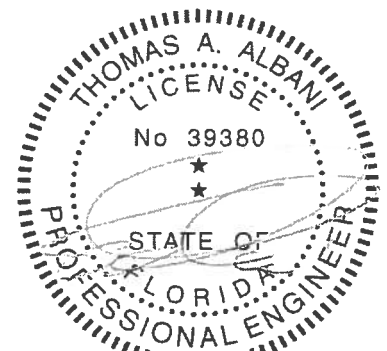


Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
Maximum Stud Length					
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 l-braces attached to both edges. Fasten T and l braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or l braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET
 EXPOSURE D
 ASCE 7-10 170 MPH
 DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
 CONNECTION OF BRACING IS BASED ON MWFRS.



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 6904 Parke East Blvd. Tampa FL 33610
 Date:

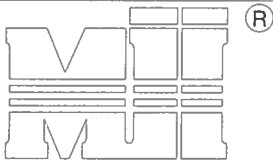
February 12, 2018

AUGUST 1, 2016

Standard Gable End Detail

MII-GE180-D-SP

MiTek USA, Inc. Page 1 of 2

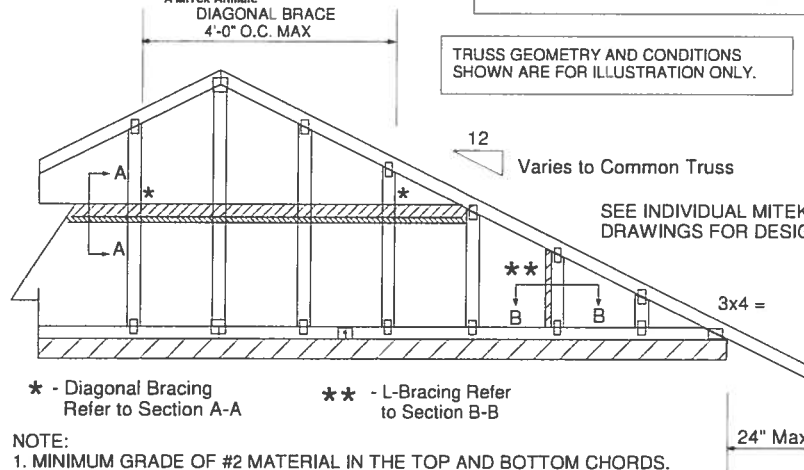


MiTek USA, Inc.

ENGINEERED BY
TRENCOA MiTek Affiliate
DIAGONAL BRACE
4'-0" O.C. MAXTypical 2x4 L-Brace Nailed To
2x4 Verticals W/10d Nails spaced 6" o.c.

Vertical Stud

SECTION B-B

TRUSS GEOMETRY AND CONDITIONS
SHOWN ARE FOR ILLUSTRATION ONLY.* - Diagonal Bracing
Refer to Section A-A** - L-Bracing Refer
to Section B-B

NOTE:

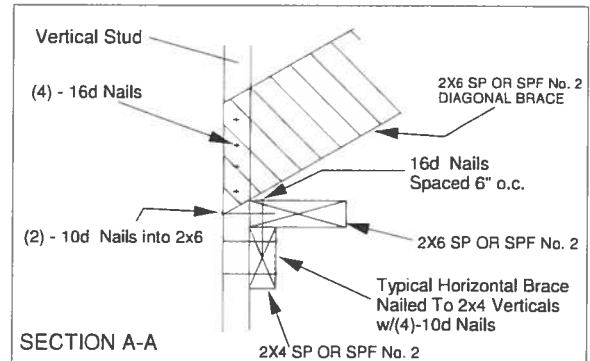
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 l-braces attached to both edges. Fasten T and l braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or l braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET
EXPOSURE D
ASCE 7-10 180 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



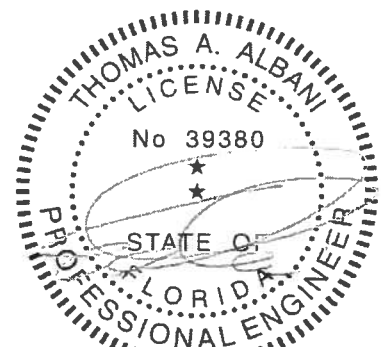
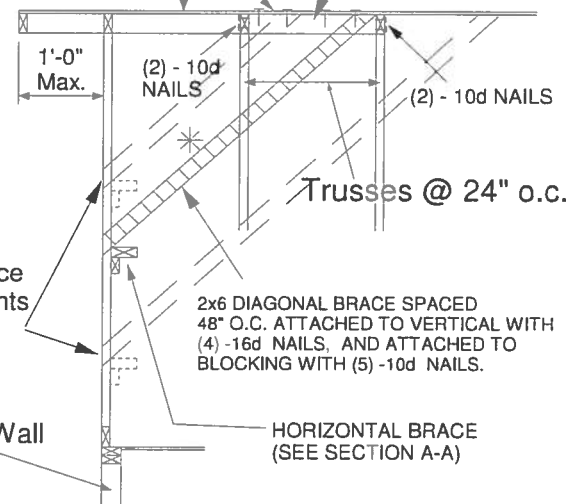
PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

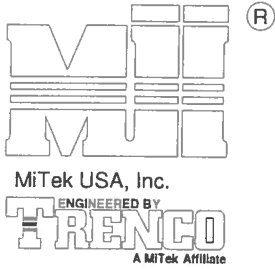
Diag. Brace at 1/3 points if needed

End Wall



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Date:

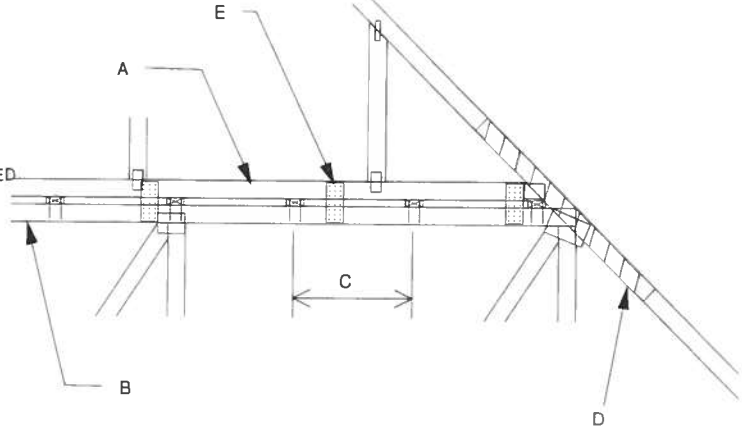
February 12, 2018



MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24" O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

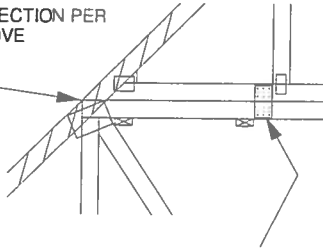
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) (0.131" X 3.5") TOE-NAILED.
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
- D - 2 X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION. WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



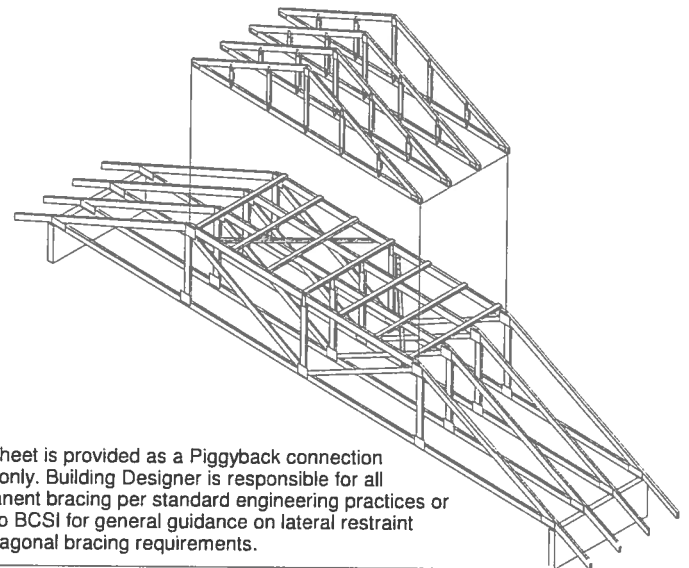
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER
NOTE D ABOVE

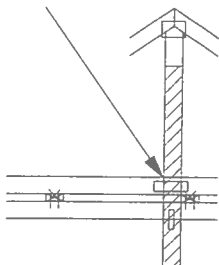


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



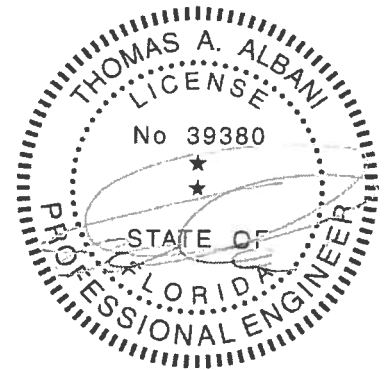
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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Date:

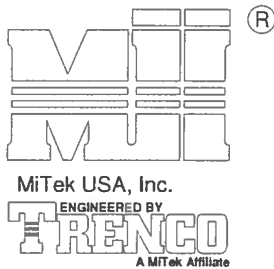
February 12, 2018

AUGUST 1, 2016

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT
7-10

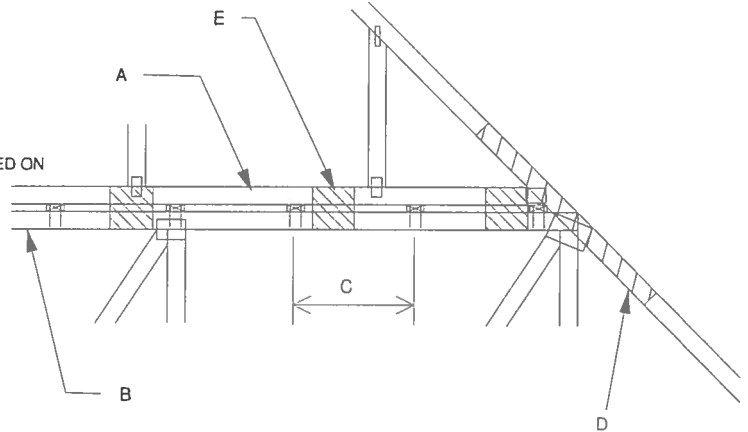
MiTek USA, Inc. Page 1 of 1



MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24" O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

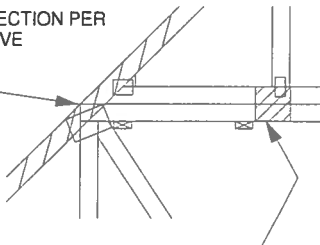
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0(0.131" X 3.5") TOE-NAILED.
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
- D - 2 X X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH ADD 9' x 9' x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



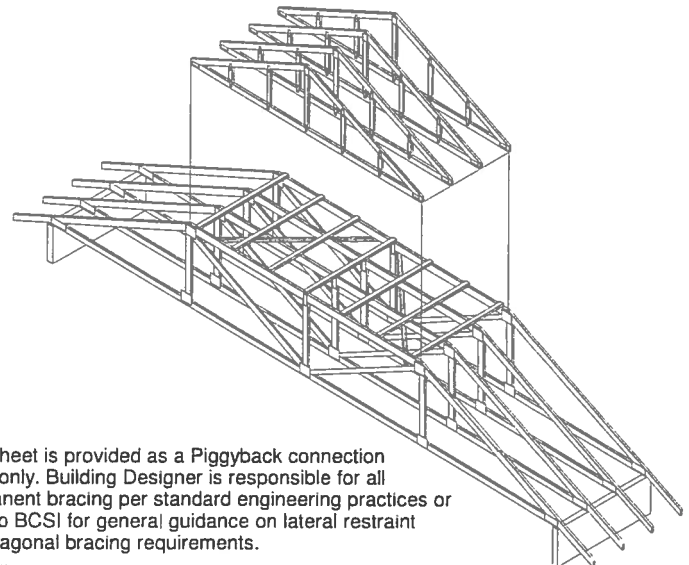
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER
NOTE D ABOVE

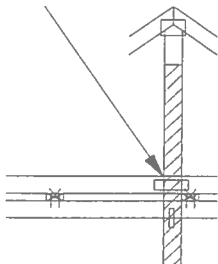


7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



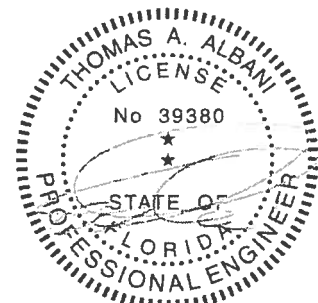
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED
TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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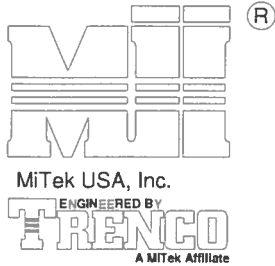
January 19, 2018

AUGUST 1, 2016

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS
AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

MiTek USA, Inc. Page 1 of 1

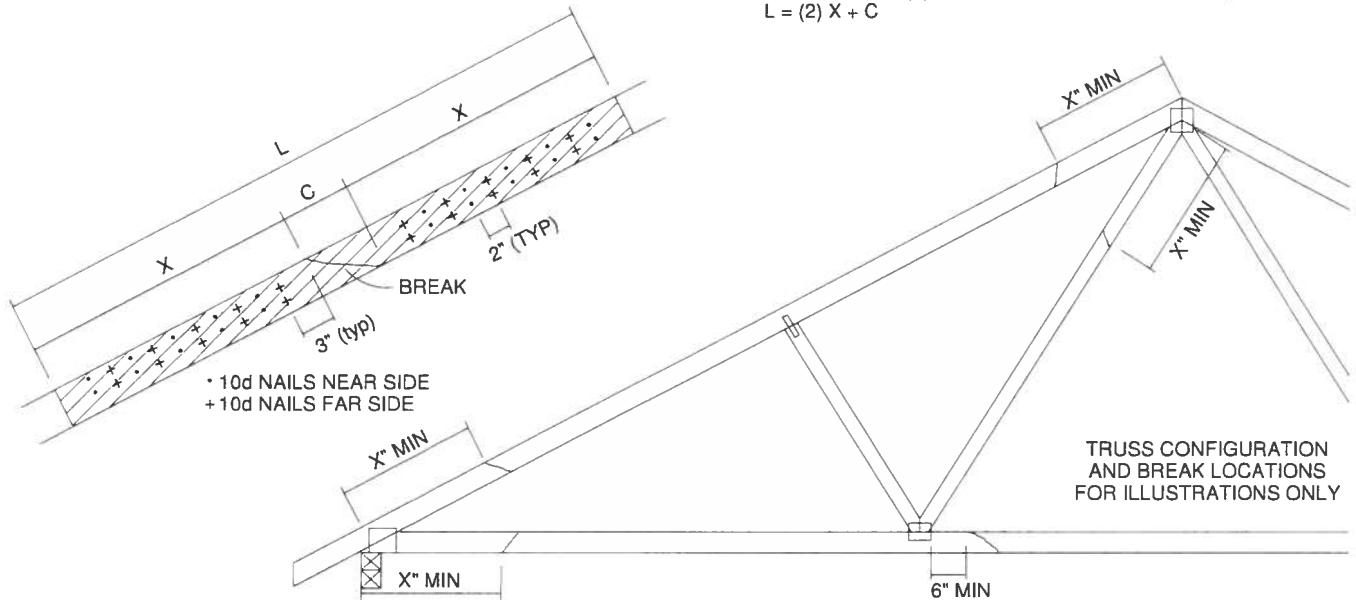


TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *		X INCHES	MAXIMUM FORCE (lbs) 15% LOAD DURATION							
			SP		DF		SPF		HF	
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH
FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS
(TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN.
STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C.
SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS)
THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:
 $L = (2) X + C$

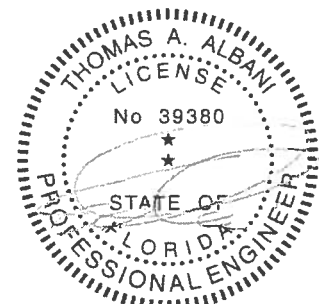


THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

NOTES:

1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x ORIENTATION ONLY.
6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



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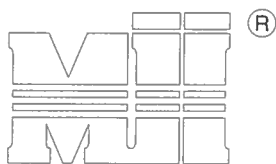
AUGUST 1, 2016

LATERAL TOE-NAIL DETAIL

MII-TOENAIL_SP

MiTek USA, Inc.

Page 1 of 1



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ENGINEERED BY
TRENCO
A MiTek Affiliate

NOTES:

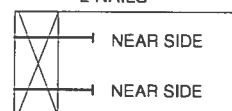
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.)
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE
THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR
ILLUSTRATION PURPOSES ONLY

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)						
	DIAM.	SP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

SIDE VIEW
(2x3)
2 NAILS



VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

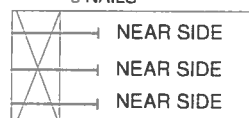
EXAMPLE:

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

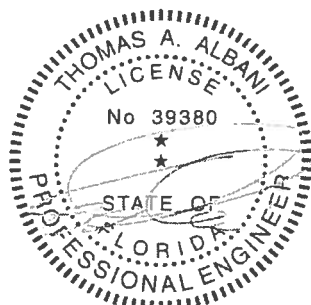
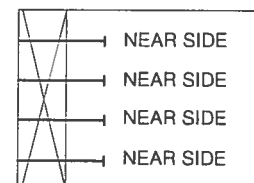
For load duration increase of 1.15:

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

SIDE VIEW
(2x4)
3 NAILS

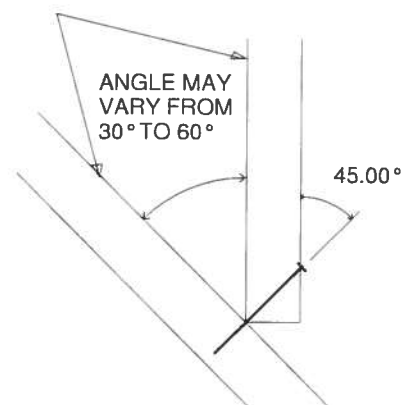
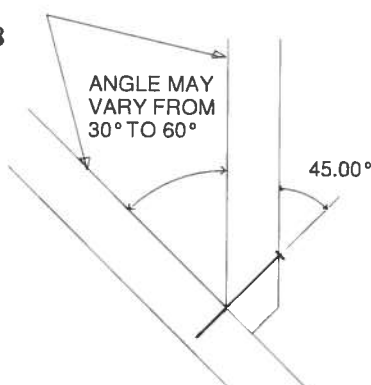
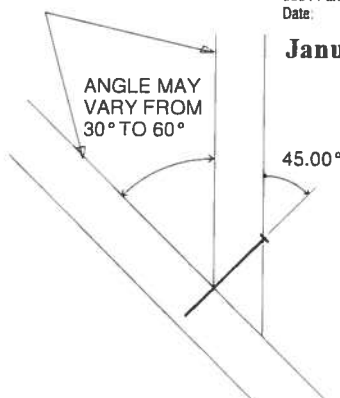


SIDE VIEW
(2x6)
4 NAILS



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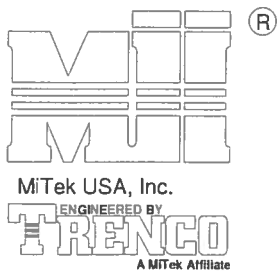
January 19, 2018



AUGUST 1, 2016

TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND1

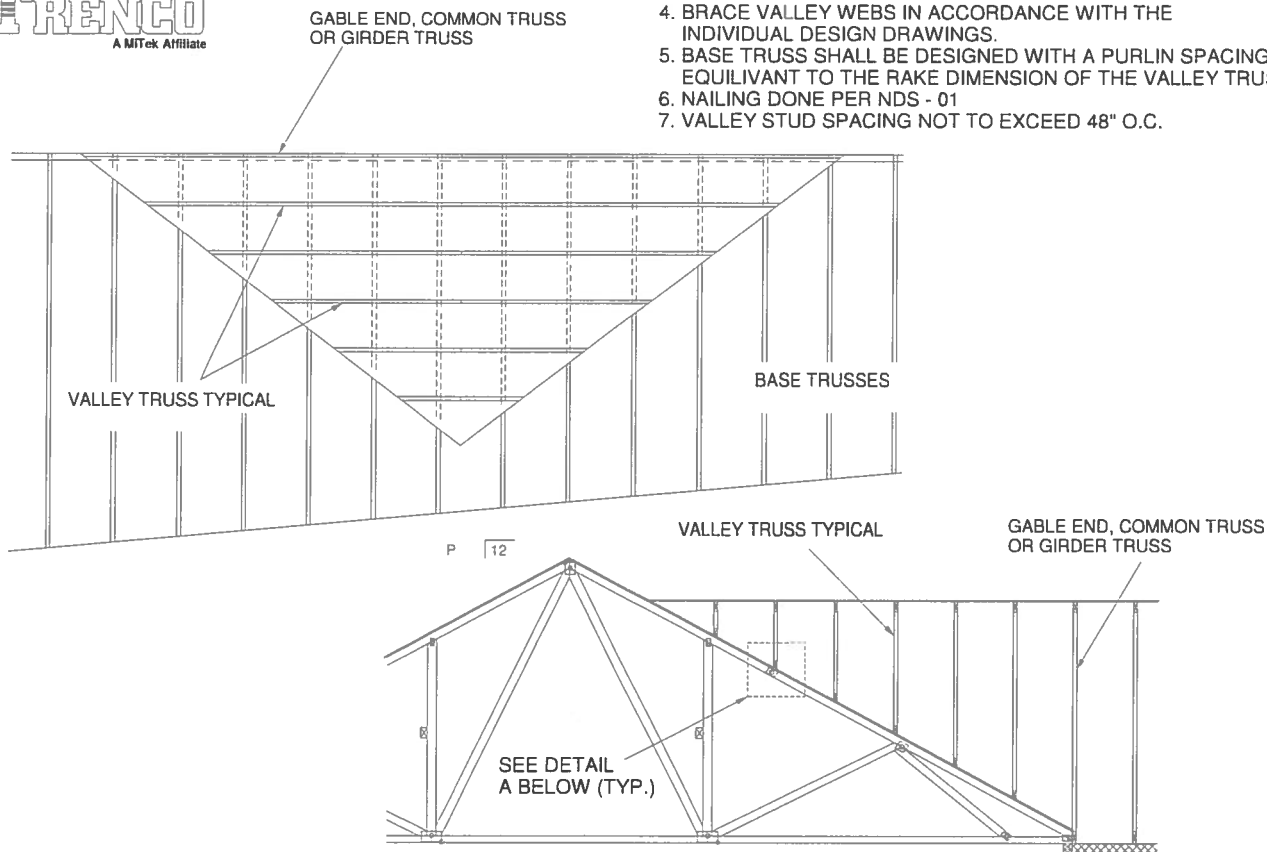


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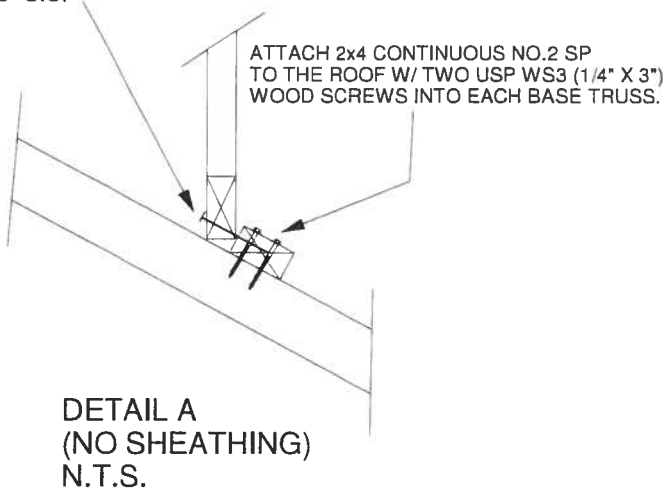
Page 1 of 1

GENERAL SPECIFICATIONS

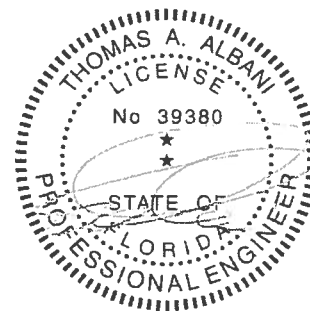
1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT
DO NOT USE DRYWALL OR DECKING TYPE SCREW
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUIVALENT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
6. NAILING DONE PER NDS - 01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



SECURE VALLEY TRUSS
W/ ONE ROW OF 10d
NAILS 6" O.C.

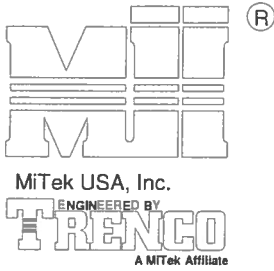


WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES



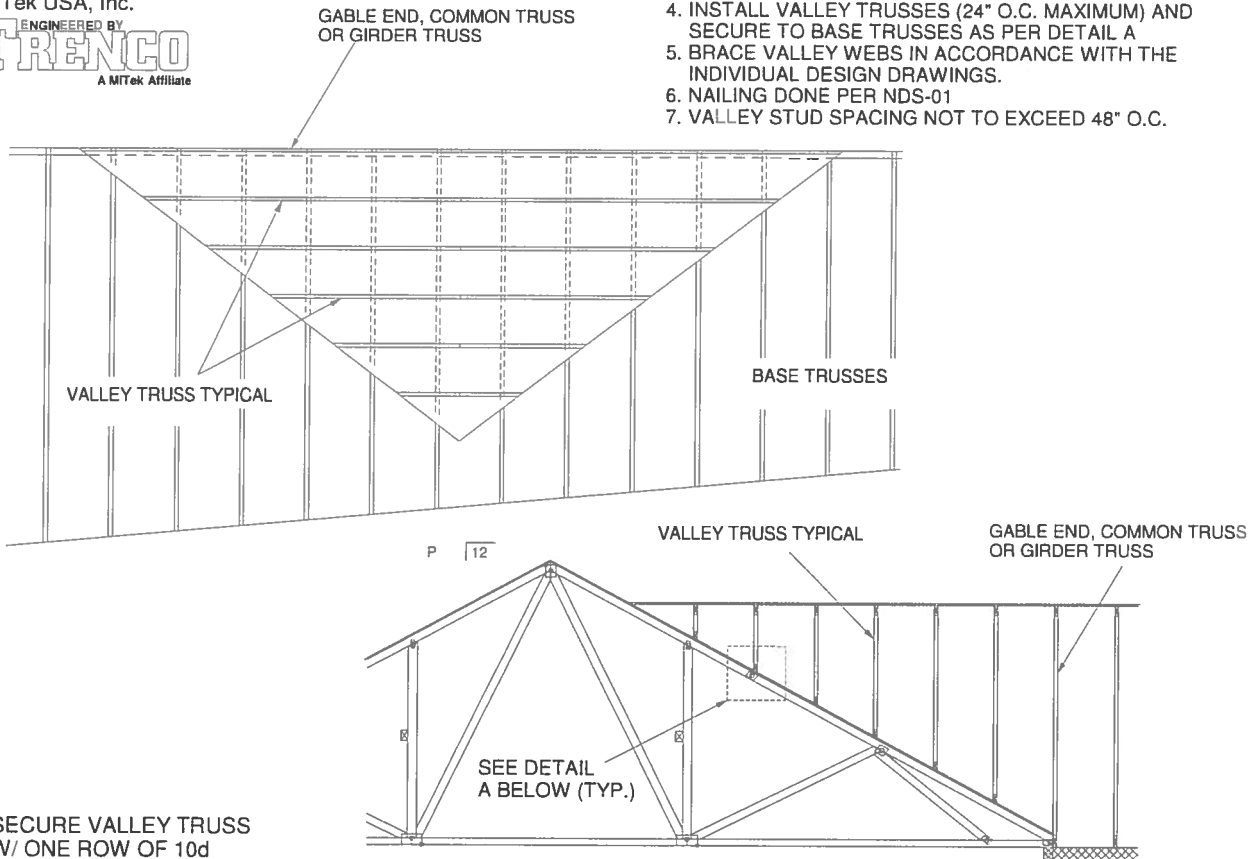
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January 19, 2018

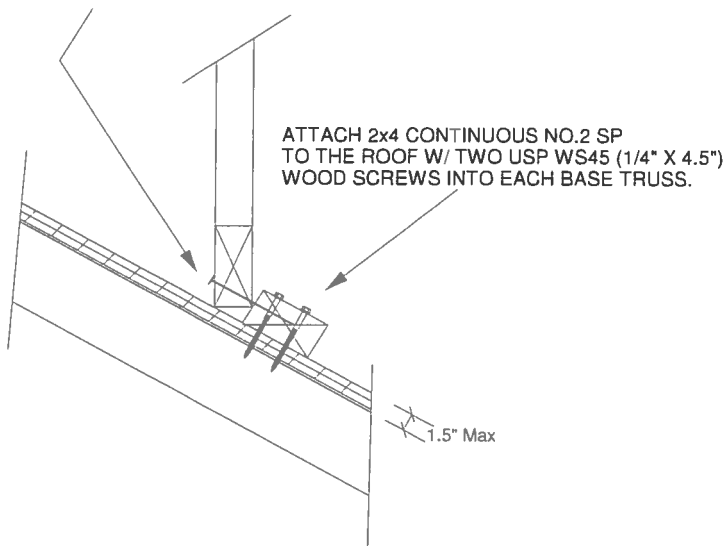


GENERAL SPECIFICATIONS

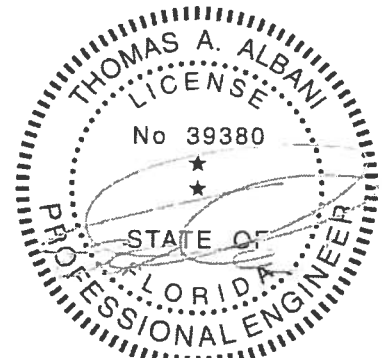
1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 4.5" WS45 USP OR EQUIVANT
3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A
5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
6. NAILING DONE PER NDS-01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



SECURE VALLEY TRUSS
W/ ONE ROW OF 10d
NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES



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February 12, 2018

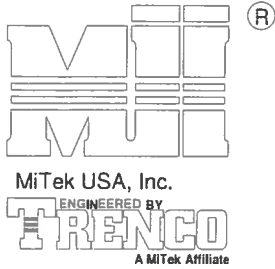
AUGUST 1, 2016

TRUSSED VALLEY SET DETAIL

MII-VALLEY SP

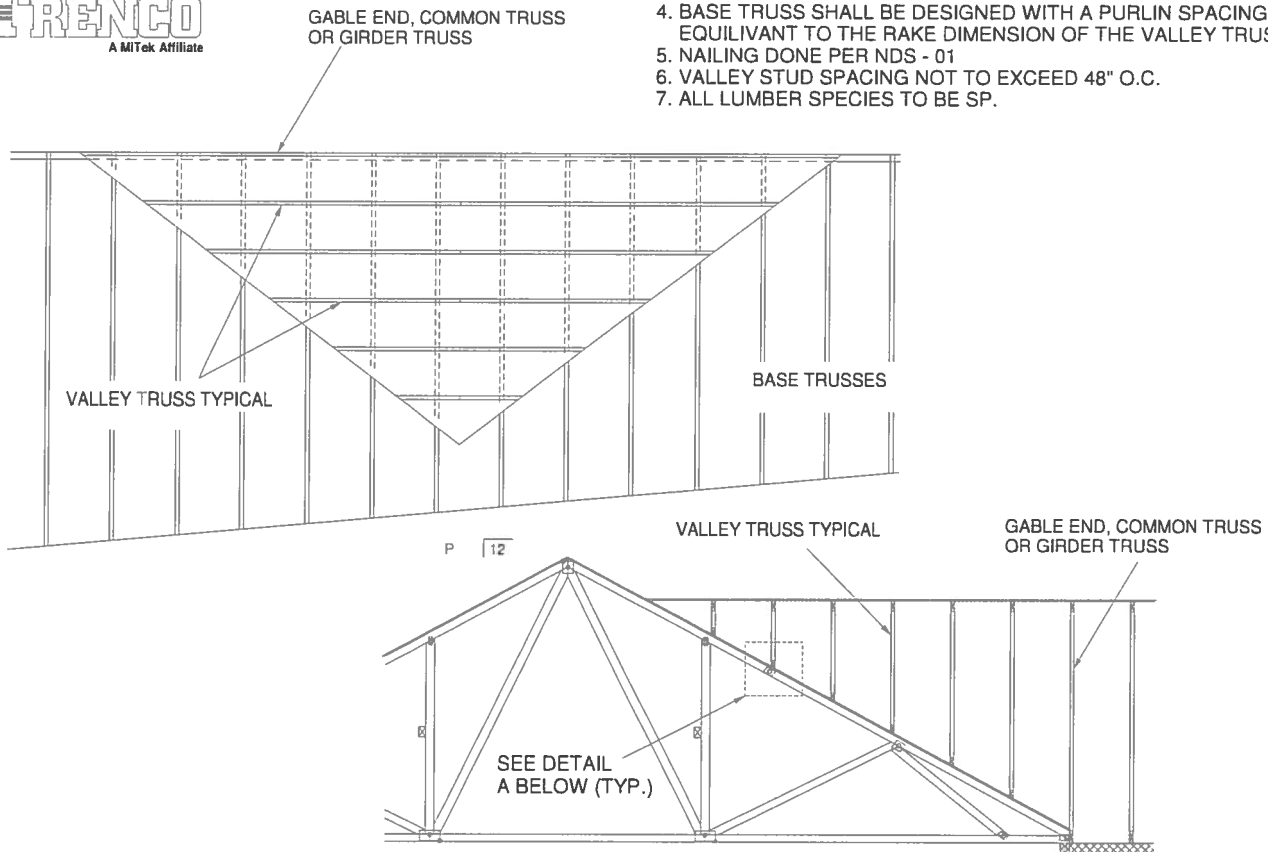
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Page 1 of 1

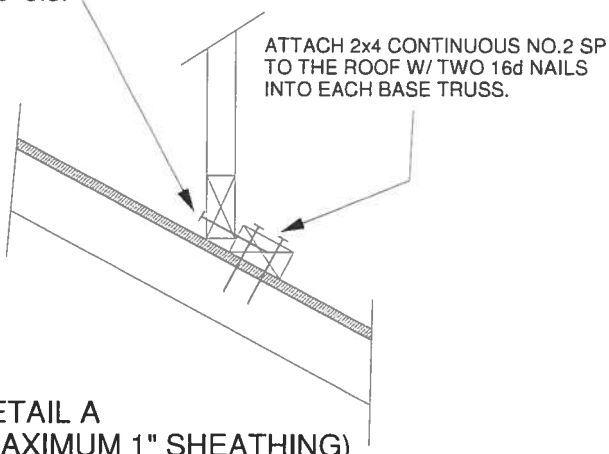


GENERAL SPECIFICATIONS

1. NAIL SIZE 16d (0.131" X 3.5")
2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUIVARIANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
5. NAILING DONE PER NDS - 01
6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
7. ALL LUMBER SPECIES TO BE SP.

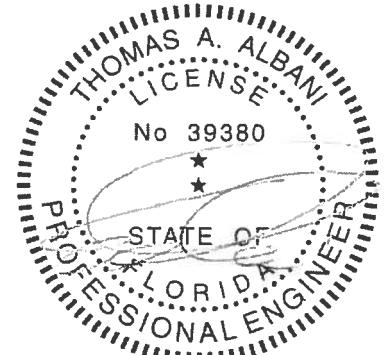


SECURE VALLEY TRUSS
W/ ONE ROW OF 16d
NAILS 6" O.C.



DETAIL A
(MAXIMUM 1" SHEATHING)
N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH
WIND DESIGN PER ASCE 7-10 150 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12
CATEGORY II BUILDING
EXPOSURE C OR B
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 60 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 4.2 PSF
ON THE TRUSSES



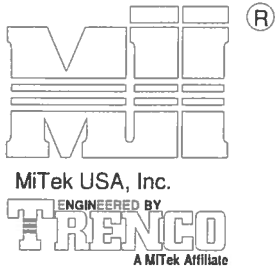
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February 12, 2018

AUGUST 1, 2016

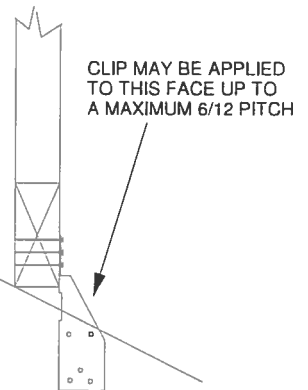
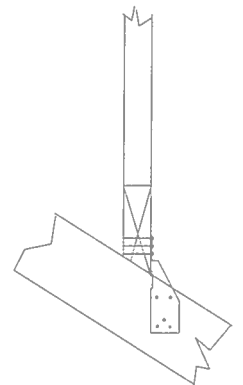
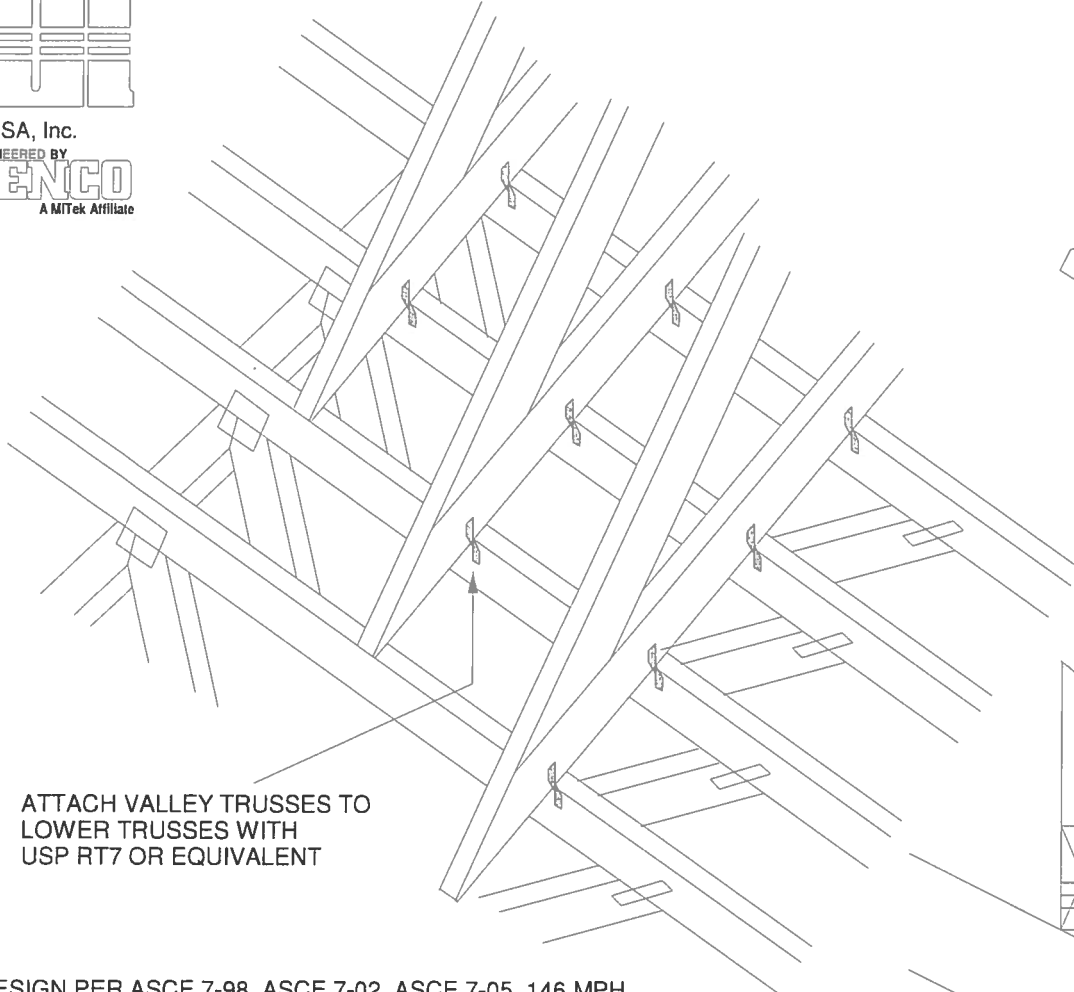
TRUSSED VALLEY SET DETAIL
(HIGH WIND VELOCITY)

MII-VALLEY

MiTek USA, Inc. Page 1 of 1



NOTE: VALLEY STUD SPACING NOT
TO EXCEED 48" O.C. SPACING

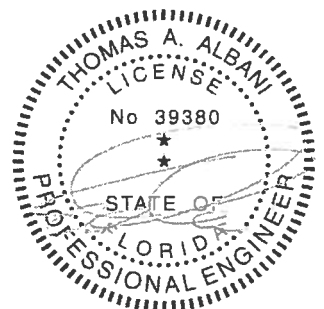
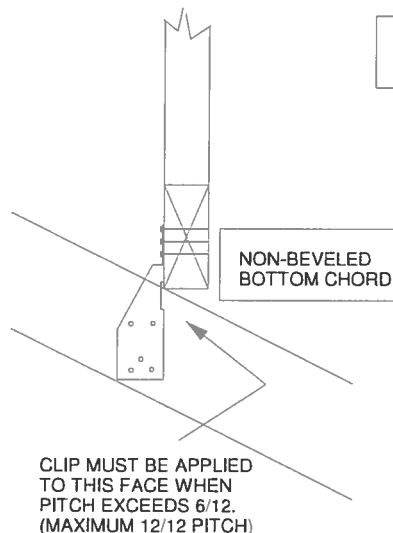


WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
WIND DURATION OF LOAD INCREASE : 1.6
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)

SUPPORTING TRUSSES DIRECTLY UNDER
VALLEY TRUSSES MUST BE DESIGNED
WITH A MAXIMUM UNBRACED LENGTH OF
2'-10" ON AFFECTED TOP CHORDS.

NOTES:

- SHEATHING APPLIED AFTER
INSTALLATION OF VALLEY TRUSSES
- THIS DETAIL IS NOT APPLICABLE FOR
SPF-S SPECIES LUMBER.



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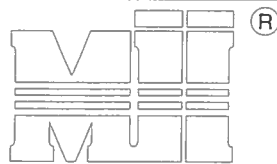
January 19, 2018

AUGUST 1, 2016

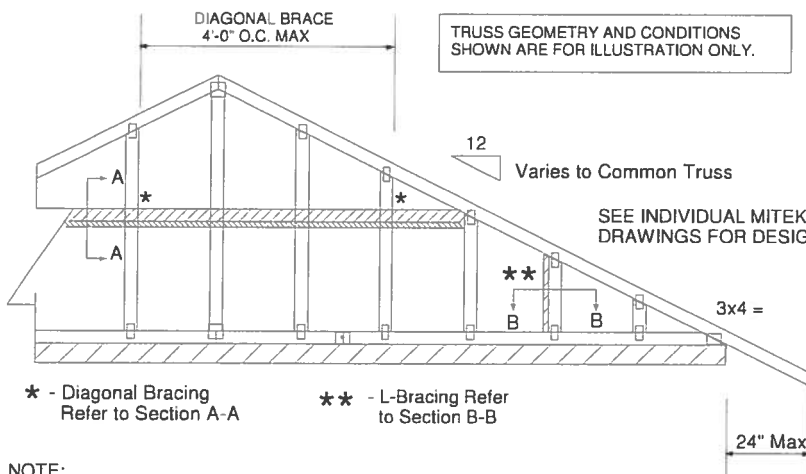
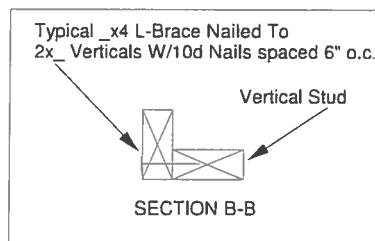
Standard Gable End Detail

MII-GE146-001

MiTek USA, Inc. Page 1 of 2



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NOTE:

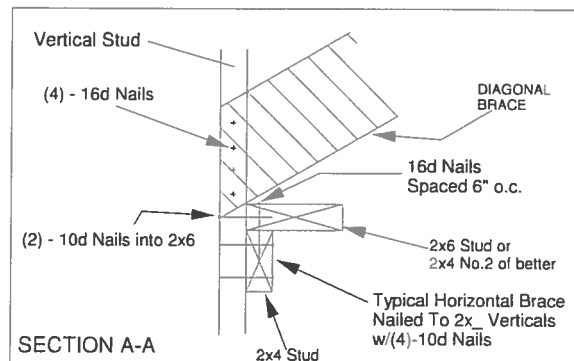
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length			
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 l-braces attached to both edges. Fasten T and l braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

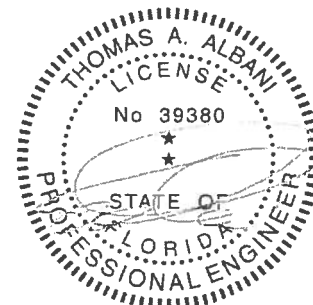
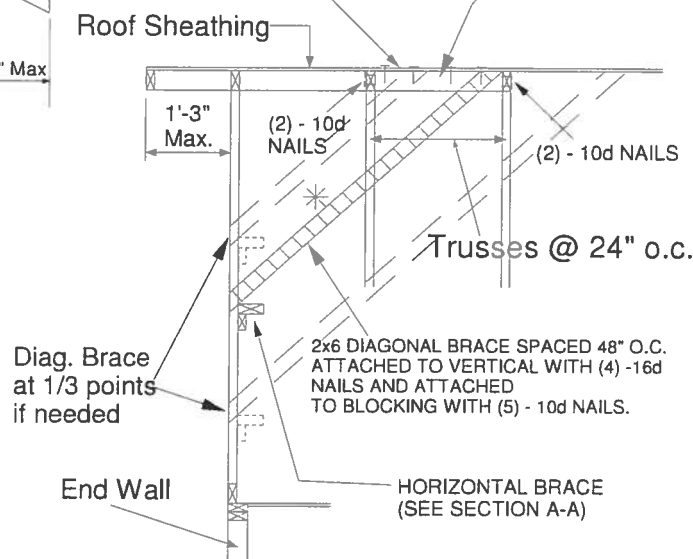
MAXIMUM WIND SPEED = 146 MPH
 MAX MEAN ROOF HEIGHT = 30 FEET
 CATEGORY II BUILDING
 EXPOSURE B or C
 ASCE 7-98, ASCE 7-02, ASCE 7-05
 DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
 CONNECTION OF BRACING IS BASED ON MWFRS.



PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK



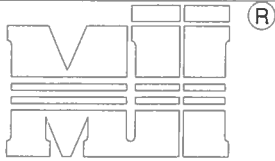
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 Date:

January 19, 2018

OCTOBER 5, 2016

REPLACE BROKEN OVERHANG

MII-REP13B



MiTek USA, Inc.



TRUSS CRITERIA:

LOADING: 40-10-0-10

DURATION FACTOR: 1.15

SPACING: 24" O.C.

TOP CHORD: 2x4 OR 2x6

PITCH: 4/12 - 12/12

HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

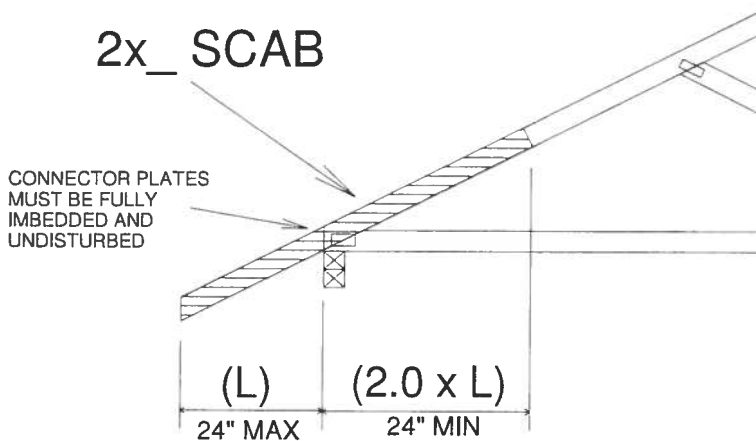
END BEARING CONDITION

MiTek USA, Inc.

Page 1 of 1

NOTES:

1. ATTACH 2x SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

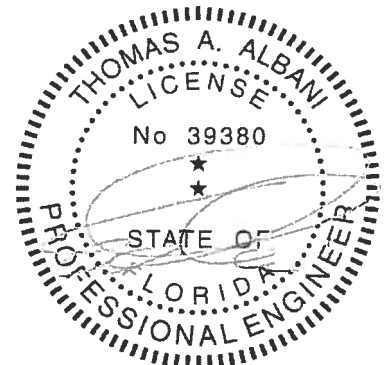


IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.

Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN
FOR PLATE SIZES AND LUMBER GRADES



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February 12, 2018

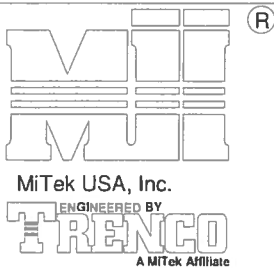
AUGUST 1, 2016

LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK

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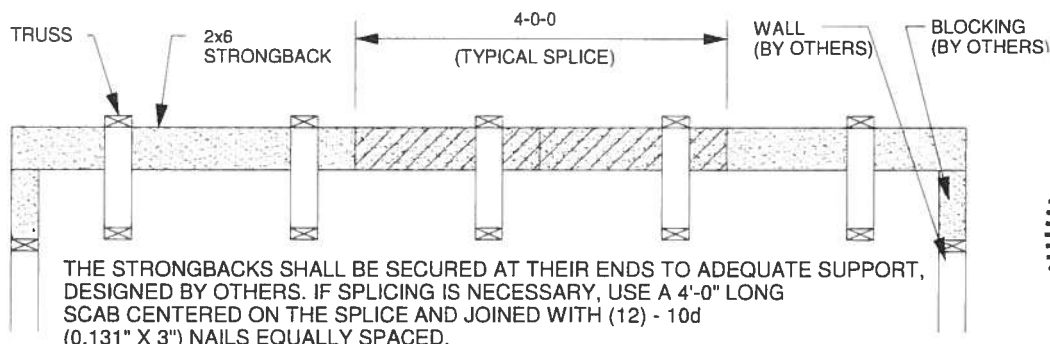
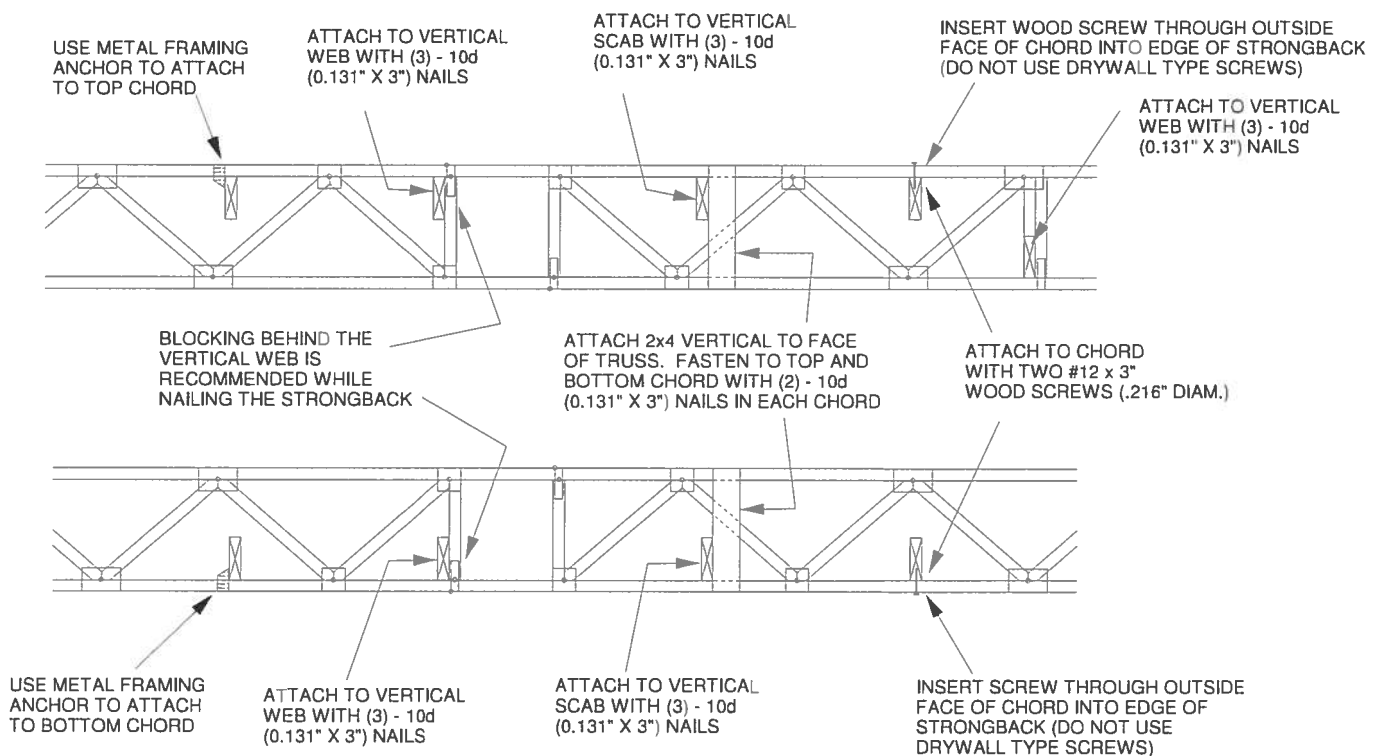
Page 1 of 1



TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

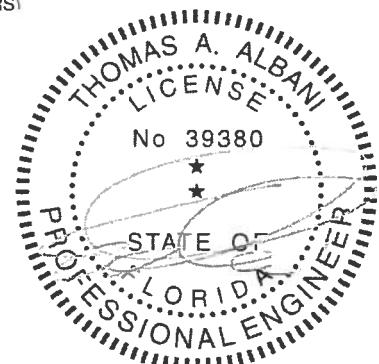
NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.



THE STRONGBACKS SHALL BE SECURED AT THEIR ENDS TO ADEQUATE SUPPORT, DESIGNED BY OTHERS. IF SPLICING IS NECESSARY, USE A 4'-0" LONG SCAB CENTERED ON THE SPLICE AND JOINED WITH (12) - 10d (0.131" X 3") NAILS EQUALLY SPACED.

ALTERNATE METHOD OF SPLICING: OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d (0.131" X 3") NAILS STAGGERED AND EQUALLY SPACED. (TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)



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