

#### ONE STORY WALL SECTION SCALE: 3/4" = 1'-0"

(2) 2X12 SYP #2 MIN. -SEE STRUCTURAL PLAN

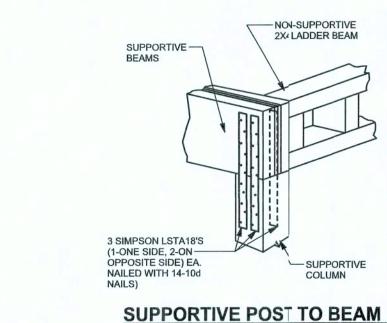
SIMPSON HUS412 MIN.

SEE STRUCTURAL PLAN

# EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS (1) 2x4 @ 16" OC TO 11'-9" STUD HEIGHT (1) 2x4 @ 12" OC TO 13'-0" STUD HEIGHT (1) 2x6 @ 16" OC TO 18'-10' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WECM 2001, TABLE 3,20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

TO 20.0' STUD HEIGHT

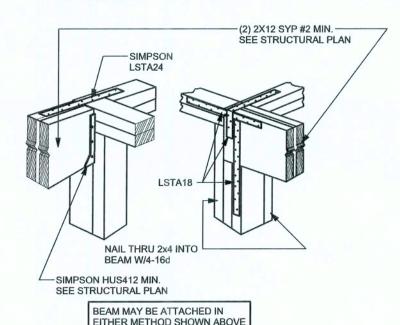


#### **BEAM MID-WALL CONNECTION DETAIL** SCALE: N.T.S.

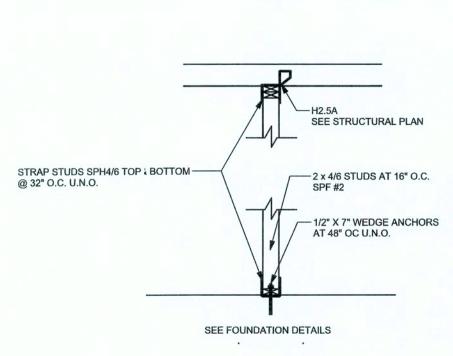
- (4)-2x4 SPF #2 NAILED

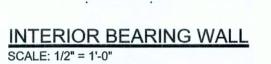
TOGETHER W/2-16d

MIN. (SEE STRUCTURAL PLAN)



**BEAM CORNER CONNECTION. DETAIL** SCALE: N.T.S.





-2X4 OUTRIGGER @ 24" O.C.

H3 EACH-

7/16" OSB 8d 6" O.C. --

EDGE & 12" O.C. FIELD

ATTACH RAT RUN TO-

BLOCKING w/ (4) 12dS

TOF NAIL TRUSS-TO TOP PLATE

w/ (8) -16d TO TRUSS AND (8) -16d TO WALL

INSTALL 2X4 SPF #2 DIAGONAL BRACE -

AND NAIL TO BLOCKING AT TOP CHORD & BOTTOM CHORD AND LAT RUN @ 6' O.O

- (8) 12dS

- 2X4 SPF #2 BLOCKING

SPACE RATRUN & DIAGONAL BRACE 6'-0" O.C.

GABLE 3RACING DETAIL

2X4 LADDER BEAM

COLUMN

-3-1/2" P.T.

SUPPORTIVE CENTER POST TO BEAM DETAIL

**DETAIL FOR SINGLE BEAM** 

SUPPORTIVE BEAM -

SCALE: N.T.S.

4-SIMPSON LSTA18 -

(2-ONE SIDE,2-ON OTHER SIDE)

IF BEAM JOINT IS AT ----

POST CONNECTION. INSTALL ONE SIMPSON

LSTA18 ON ONE SIDE

- H3 INSTALLED HORIZONTALLY

FOR GABLEHEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED

-7/16" OSB ROOF SHEATHING 8d 6" O.C.

EDGE, 12" O.C. FIELD, & 4" O.C. GABLES

BLOCKING REQUIRED BETWEEN OUT RIGGERS

DIAGONAL BRACE MUST BE NAILED

OVER 12' IT MAY BE "T" BRACED UP

TO 12' AND UNBRACED UP TO 7'

-2X4X8' RAT RUN NAIL EACH

12dS = 12d SINKER

OR .135" X 3.125"

OR .131 X 3.25"

SIMPSON H2.5A U.N.O. -

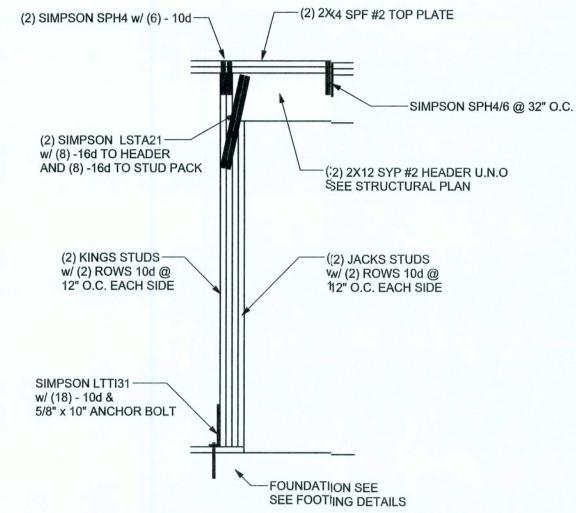
SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21-

w/ (8) -16d TO HEADER

AND (8) -16d TO POST

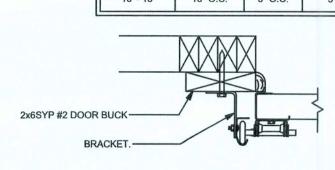
CONNECTION w/ (4) 12dS



TYPICAL GARAGE DOOR HEADER STRAPING DETAIL SCALE: 1/2" = 1'-0"

#### 2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD, CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN	
8' - 10'	24" O.C.	5" O.C.	5" O.C.	
11' - 15'	18" O.C.	4" O.C.	4" O.C.	
16' - 18'	16" O.C.	3" O.C.	3" O.C.	



**GARAGE DOOR BUCK INSTALLATION DETAIL** 

-(2) 2X12 SYP #2 U.N.O.

-6X6 / 4X4 SYP #2 POST

-SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

SEE FOOTING DETAILS

ANCHOR BOLT

4X4 / 6X6 PORCH POST DETAIL

SEE STRUCTURAL PLAN

# PRE ENGINEERED ROOF TRUSS -[DOUBLE 2x4 SPF TOP PLATE NAILED -TOGETHER W/2-16d NAILS AT 16" O.C. 44' MIN. LAP w/ (12) - 16d OR 4" LAP w/ INTERIOR CEILING AS ---SPECIFIED ON FLOOR PLAN TO Trop PLATE AT BOTTOM CHORD OF TRUSS

**GRADE & SPECIES TABLE** 

SYP #2

SYP #2

SYP #2

24F-V3 SP

MICROLAM

PARALAM

TIMBERSTRAND 1700

Fb (psi) | E (10<sup>6</sup> psi

1.6

1.6

1.6

1.8

1.7

1.9

1200

1050

975

2400

1600

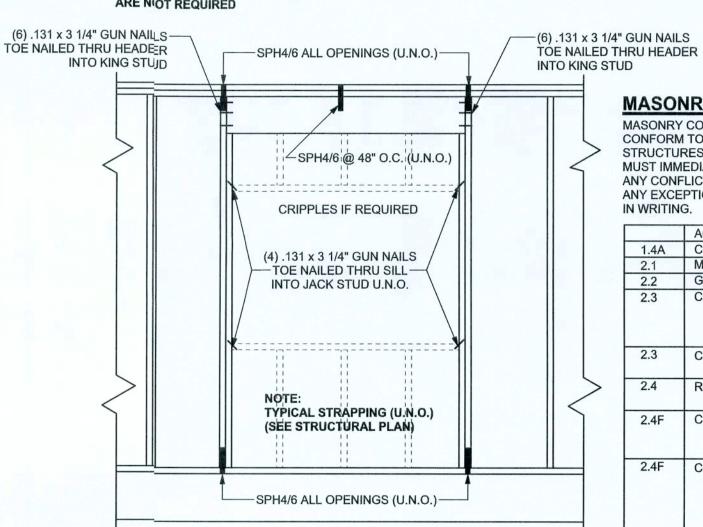
2900

ALL STUDS TO BE 2x4 ----SPF NAILED TO TOP AND BOTTOM PLATES WITH 2-16d NAILS

SCALE: N.T.S.

## CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

IF TRUISS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48" O.C. ARE NOT REQUIRED



(FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.) TYPICAL HEADER STRAPING DETAIL SCALE: 1/2" = 1'-0"

(1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O.

## **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER, IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR NTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

**WELDED WIRE REINFORCED SLAB:** 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. BER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

### **BUILDER'S RESPONSIBILITY**

ROOF SYSTEM DESIGN

SPECIFICALL	NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	DITIONS, FOUNDATION BEARING CAPACITY, GRADE AND WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	S AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 OR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE PL	UOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL GINEER IMMEDIATELY.
DESIGN, PLACE	MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS IT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL NS.

DESIGN DATA

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

#### MASONRY NOTES: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

	ACI530.1-02 Section	Specific Requirements		
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi		
2.1	Mortar	ASTM C 270, Type N, UNO		
2.2	Grout	ASTM C 476, admixtures require approval		
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block		
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"		
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)		
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS		
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS		
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.		
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.		

### **ANCHOR TABLE**

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED R 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED R 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED R 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED R 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
	0000	ADUICO	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-100		

Zone Effective Wind Area (ft2)

1 | 19.9 | -21.8 | 18.1 | -18.1

2 | 19.9 | -25.5 | 18.1 | -21.8

-40.6

3 O'hg -25.5 18.1 -21.8 -68.3 -42.4

4 | 21.8 | -23.6 | 18.5 | -20.4

5 | 21.8 | -29.1 | 18.5 | -22.6

Doors & Windows | 21.8 | -29.1

16x7 Garage Door | 18.5 | -21.0

3 O'hg

Worst Case

(Zone 5, 10 ft2)

8x7 Garage Door 19.5

MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT

ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10%

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

BASIC WIND SPEED = 110 MPH

3.) WIND IMPORTANCE FACTOR = 1.0

5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

2.) WIND EXPOSURE = B

BUILDING CATEGORY = II

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

30 PSF (SLEEPING ROOMS)

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

not to be reproduced, altered or copied in any form or manner without first the express written ermission and consent of Mark Disosway CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 portions of the plan, relating to wind engin comply with section R301.2.1, florida building (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS: code residential 2004, to the best of my

> LIMITATION: This design is valid for one ouilding, at specified location.

REVISIONS

SOFTPLAN

MARK DISOSWAY

INDLOAD ENGINEER: Mark Disosway,

PE No.53915, POB 868, Lake City, FL

Stated dimensions supercede scaled

dimensions. Refer all questions to

Mark Disosway, P.E. for resolution.

Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:

Mark Disosway, P.E. hereby expressly rese

its common law copyrights and property right i

hese instruments of service. This document

32056, 386-754-5419

DIMENSIONS:

Spec House Lot 69

ADDRESS: Columbia County, Florida

PRINTED DATE: November 28, 2007 David Disosway

FINALS DATE:

711271

OF 3 SHEETS

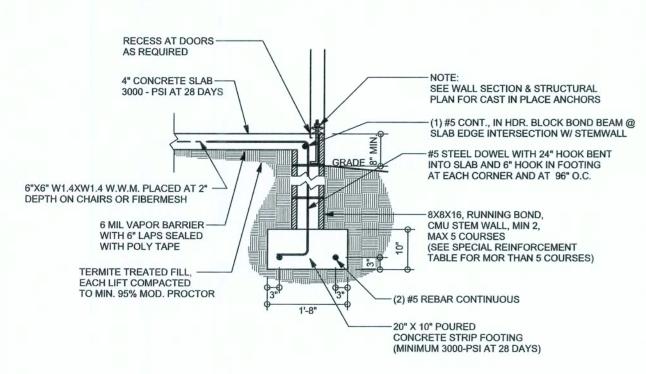
<u>ipscomb Eagle</u> Development

Emerald Cove S/D Lot 69 Emerald Cove S/D

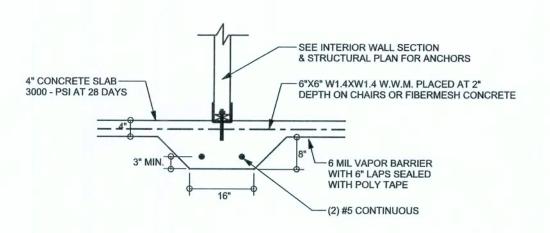
Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

DRAWN BY: CHECKED BY:

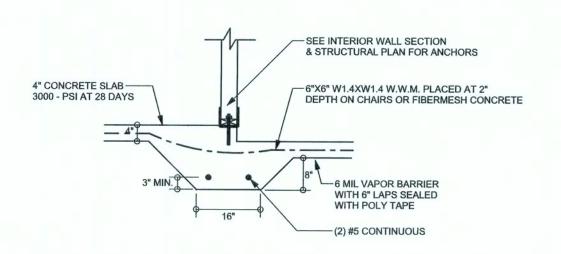
27 / Nov / 07 JOB NUMBER: PRAWING NUMBER



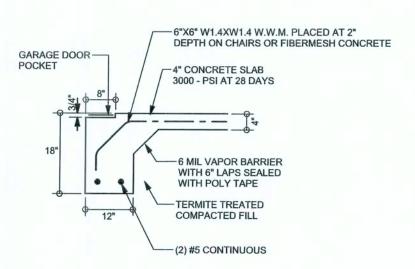
F9 STEM WALL FOOTING
S-2 SCALE: 1/2" = 1'-0"



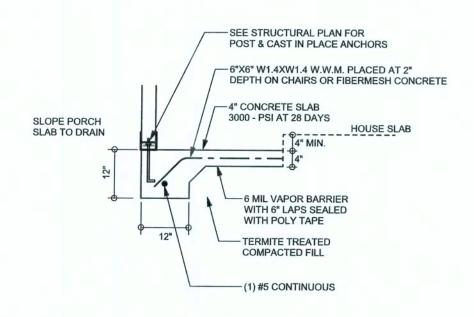
F2 INTERIOR BEARING FOOTING
S-2 SCALE: 1/2" = 1'-0"



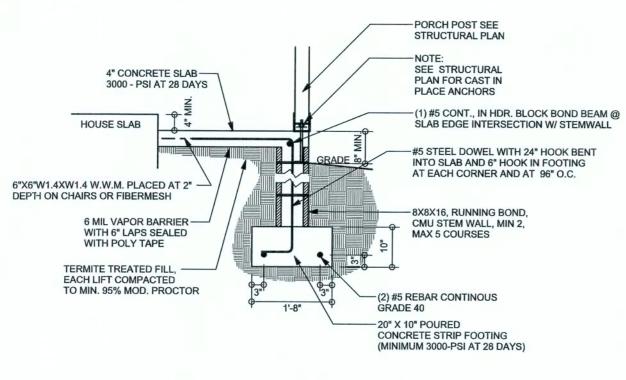
F3 INTERIOR BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"

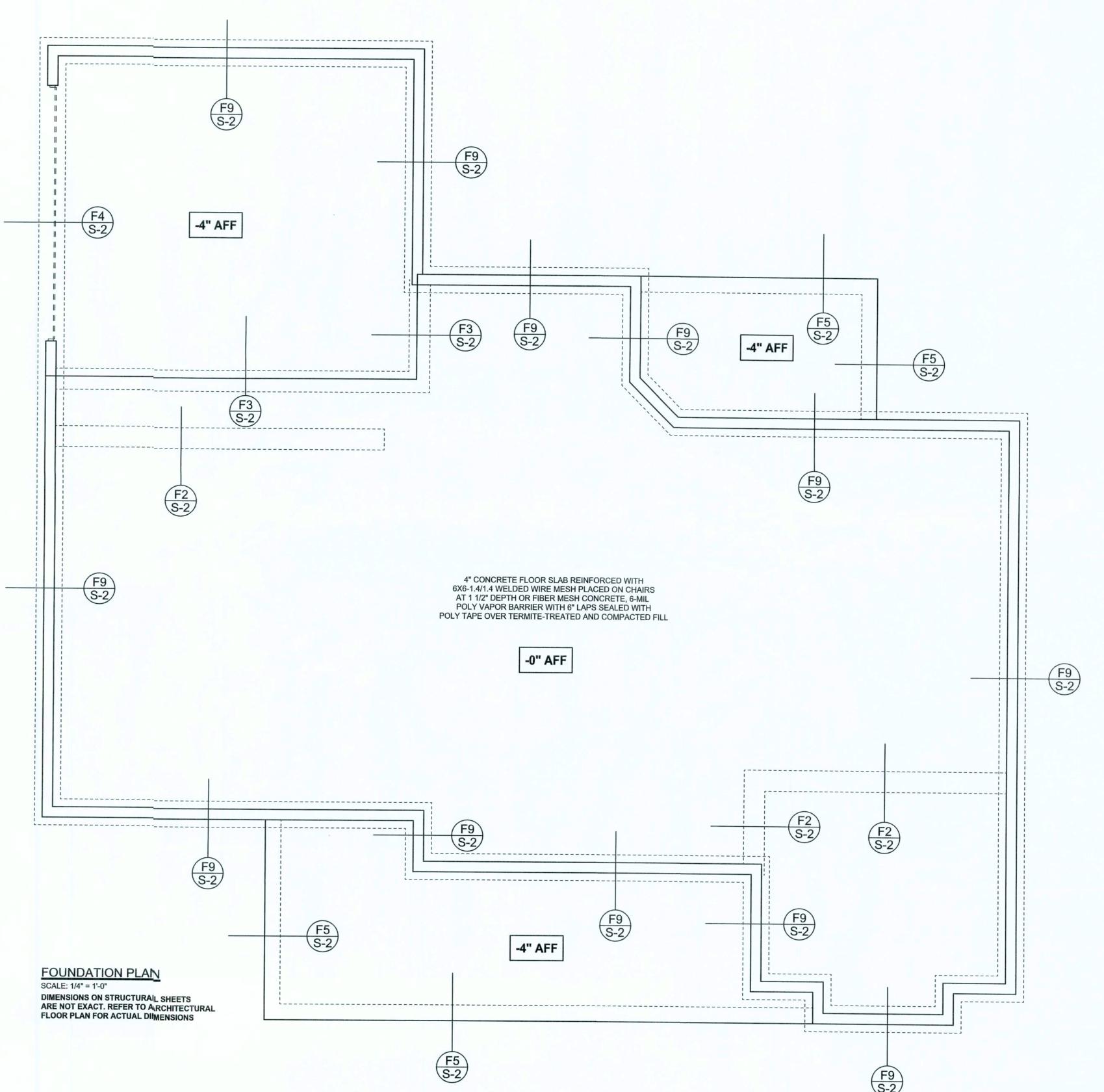


F12 ALT. STEM WALL PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	BACKFILL FOR 8" CN		AL REINFOR 8" CMU STEI (INCHES O.C	MWALL	VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



SOFTPIX

REVISIONS

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

examined this plan, and that the applicable portions of the plan, relating to wind engineeri comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

CERTIFICATION: I hereby certify that I have

SEAL

Lipscomb Eagle

Development

Spec House

Lot 69

Emerald Cove S/D

ADDRESS: Lot 69 Emerald Cove S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
November 27, 2007

DRAWN BY: CHECKED BY:
David Disosway

FINALS DATE: 27 / Nov / 07

JOB NUMBER: 711271 DRAWING NUMBER

> S-2 OF 3 SHEETS

