

Daniel Shaheen  
Daniel Shaheen

2-06-03

D.D.S.  
Studios

ARCHITECTURAL  
DESIGN  
P.O. Box 273  
LAKE CITY FL. 32056  
(386) 754-0181

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MARK DISOSWAY, P.E.  
P.O. Box 888, LAKE CITY, FL 32056  
PH: (888) 754-5418  
FLORIDA P.E. 05915

ENGINEERED BY:

WILLIAM & BONNIE  
ROBBINS

DESIGNED FOR:

EXTERIOR ELEVATIONS

SHEET NUMBER  
1 of 4

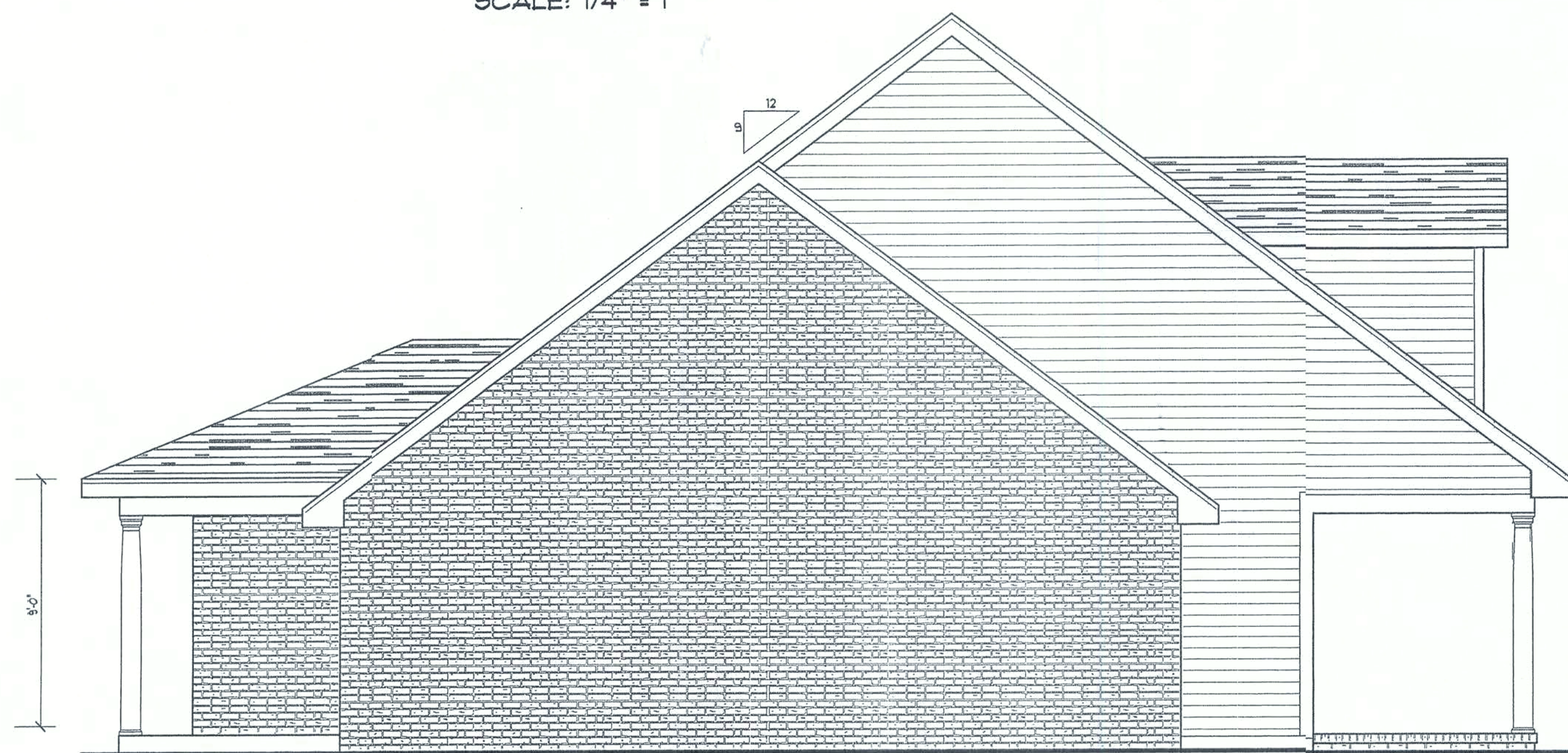
All work shall comply with  
the standard building code,  
and all applicable local  
codes and ordinances.  
Contractor shall verify all  
dimensions prior to  
commencing construction.



FRONT ELEVATION  
SCALE: 1/4" = 1'



REAR ELEVATION  
SCALE: 1/4" = 1'



LEFT ELEVATION SCALE: 1/4" = 1'

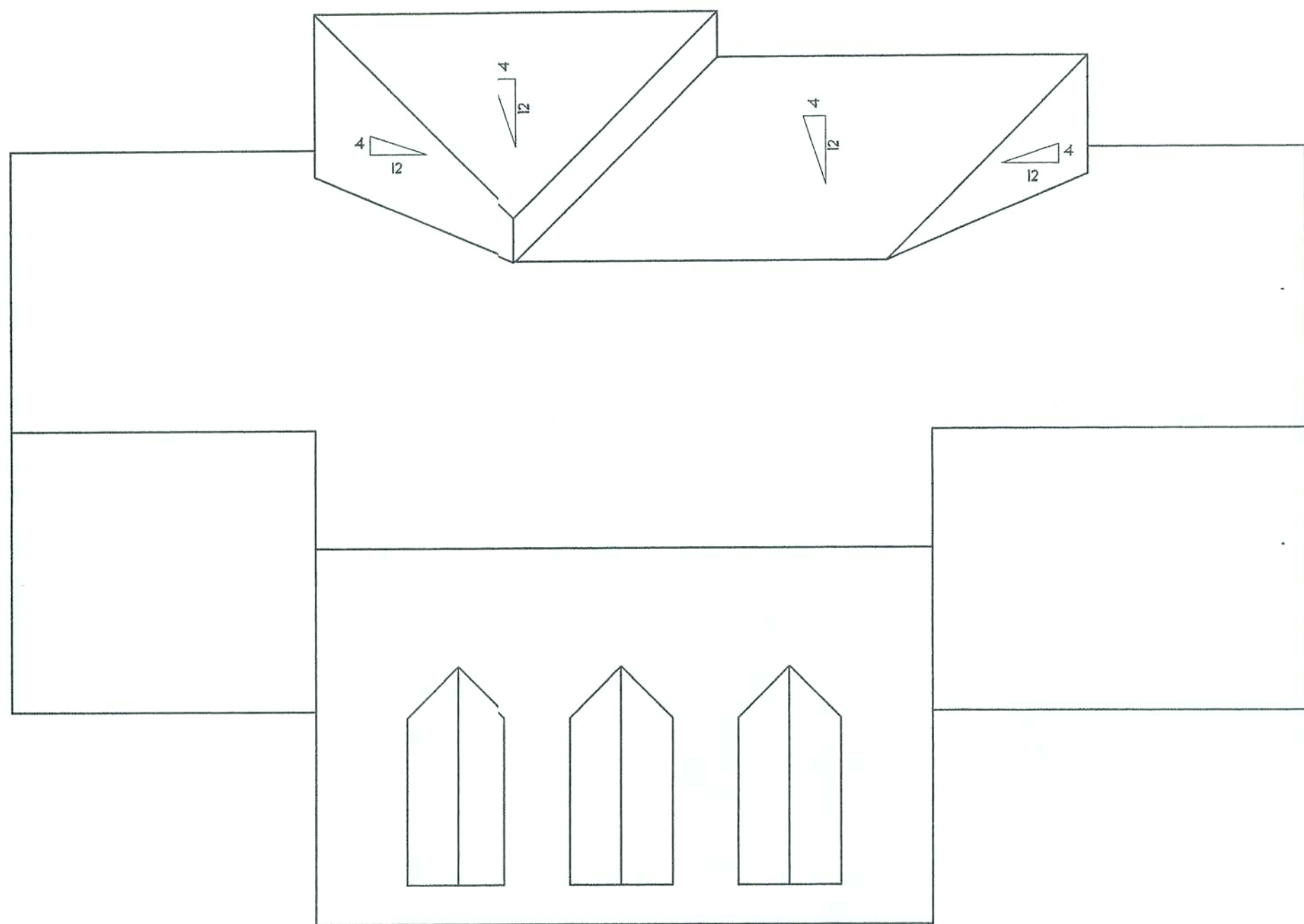


RIGHT ELEVATION SCALE: 1/4" = 1'

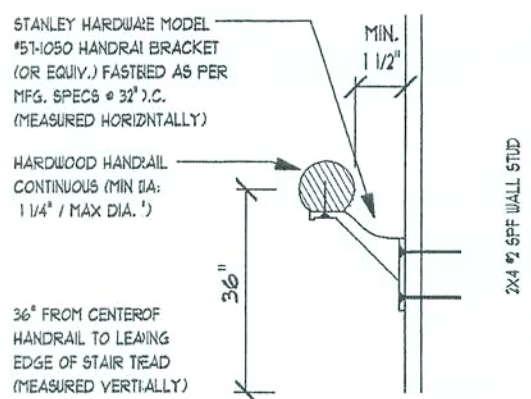
ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

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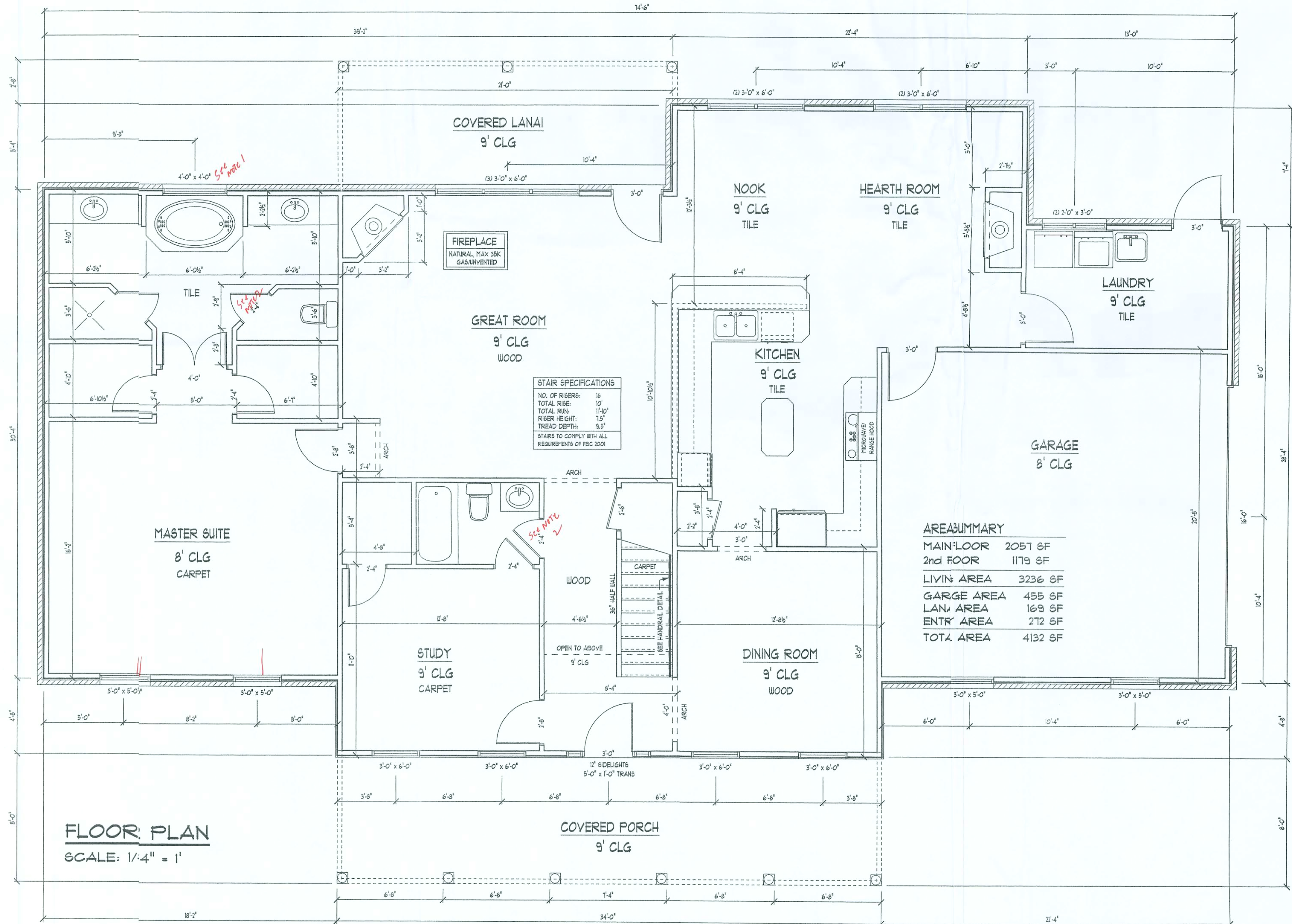
ROOF PLAN  
SCALE: 1/8" = 1'



HANDRAIL DETAIL  
SCALE: 2" = 1'

ROOF PLAN NOTES

- R-1 ALL ROOF PITCH 3/12 UNLESS OTHERWISE NOTED
- R-2 ALL OVERHANG 18" UNLESS OTHERWISE NOTED
- R-3 PROVIDE ATTIC VENTILATION IN ACCORDANCE WITH CODE REQUIREMENTS
- R-4 SEE EXTERIOR ELEVATIONS AND FLOOR PLANS TO VERIFY PLATE AND HESL HEIGHTS
- R-5 MOVE ALL VENTS AND OTHER ROOF PENETRATIONS TO REAR



STAIR SPECIFICATIONS

|                |        |
|----------------|--------|
| NO. OF RISERS: | 16     |
| TOTAL RISE:    | 10'    |
| TOTAL RUN:     | 17'-0" |
| RISER HEIGHT:  | 13"    |
| TREAD DEPTH:   | 11"    |

STAIRS TO COMPLY WITH ALL REQUIREMENTS OF IBC 2001

AREASUMMARY

|             |         |
|-------------|---------|
| MAIN FLOOR  | 2051 SF |
| 2nd FLOOR   | 1179 SF |
| LIVING AREA | 3236 SF |
| GARAGE AREA | 485 SF  |
| LANA AREA   | 169 SF  |
| ENTRY AREA  | 272 SF  |
| TOTAL AREA  | 4132 SF |

FLOOR PLAN  
SCALE: 1/4" = 1'

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ENGINEERED BY:

WILLIAM & BONNIE  
ROBBINS

DESIGNED FOR:

FLOOR PLAN  
ROOF PLAN

SHEET NUMBER  
2 of 4

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ENGINEERED BY:

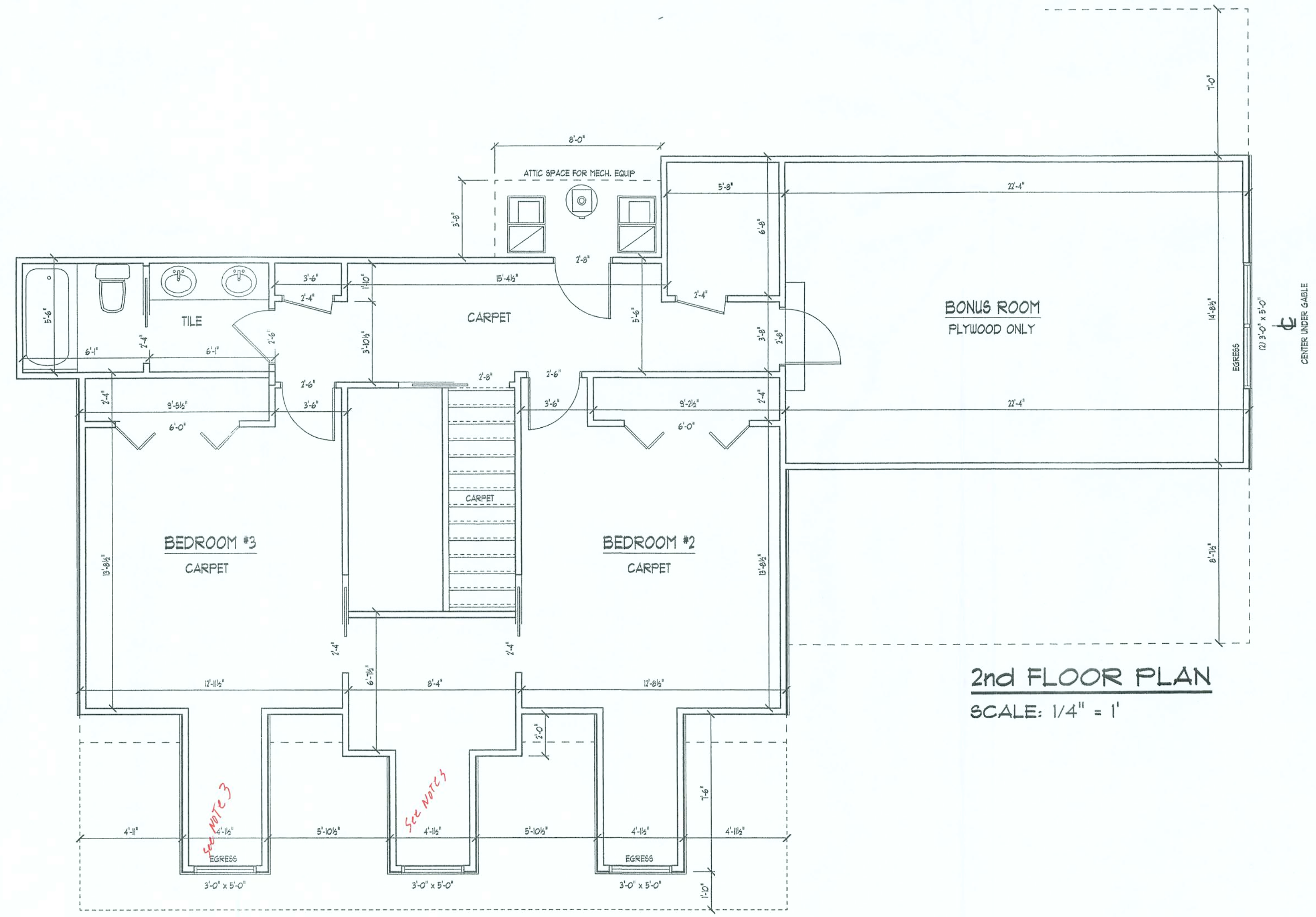
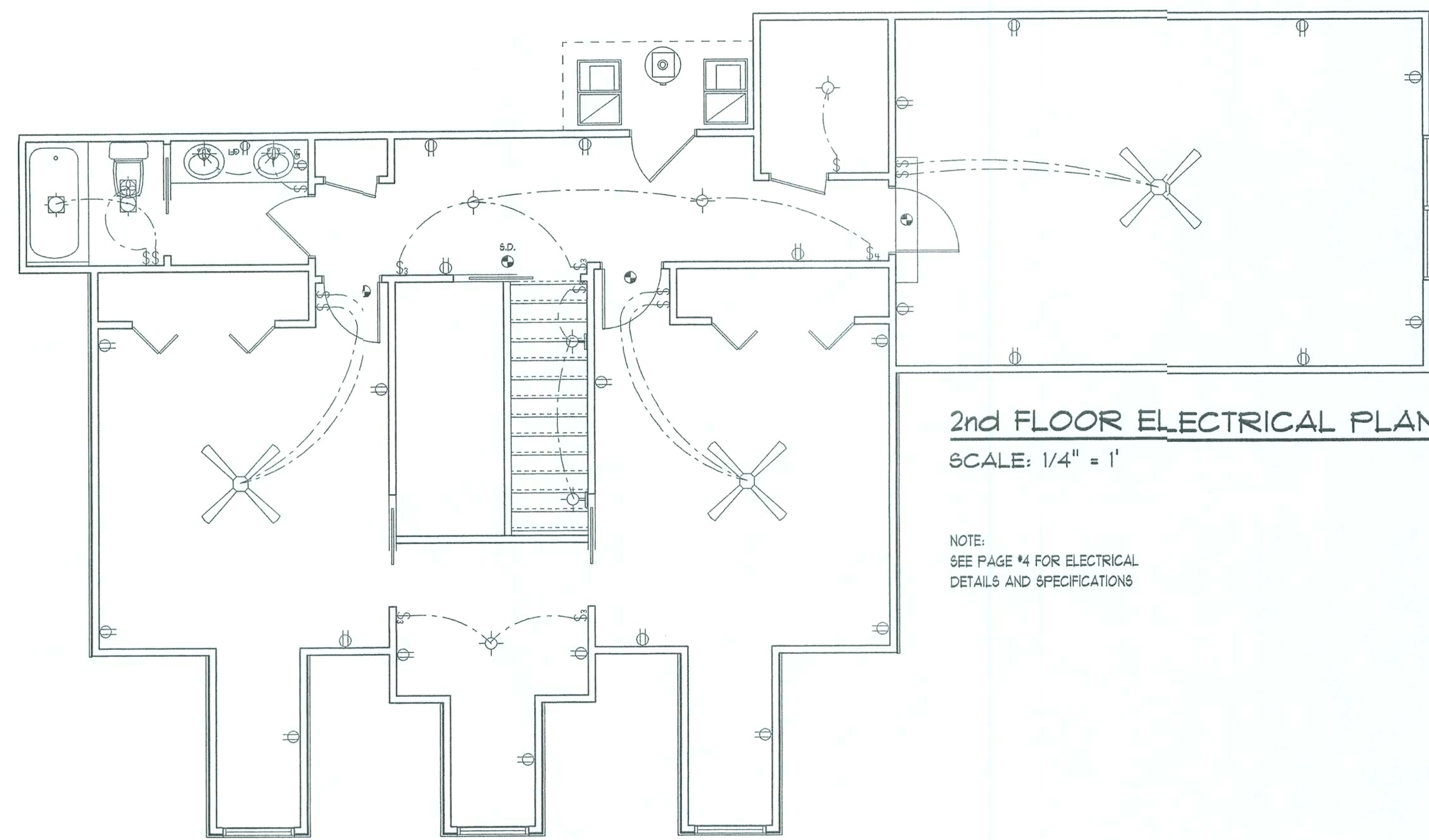
WILLIAM & BONNIE  
ROBBINS

DESIGNED FOR:

SECOND FLOOR PLAN  
SECOND FLOOR ELECTRICAL PLAN

SHEET NUMBER  
3 of 4

All work shall comply with the standard building codes, and all applicable local codes and ordinances.  
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ENGINEERED BY:

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**ROBBINS**

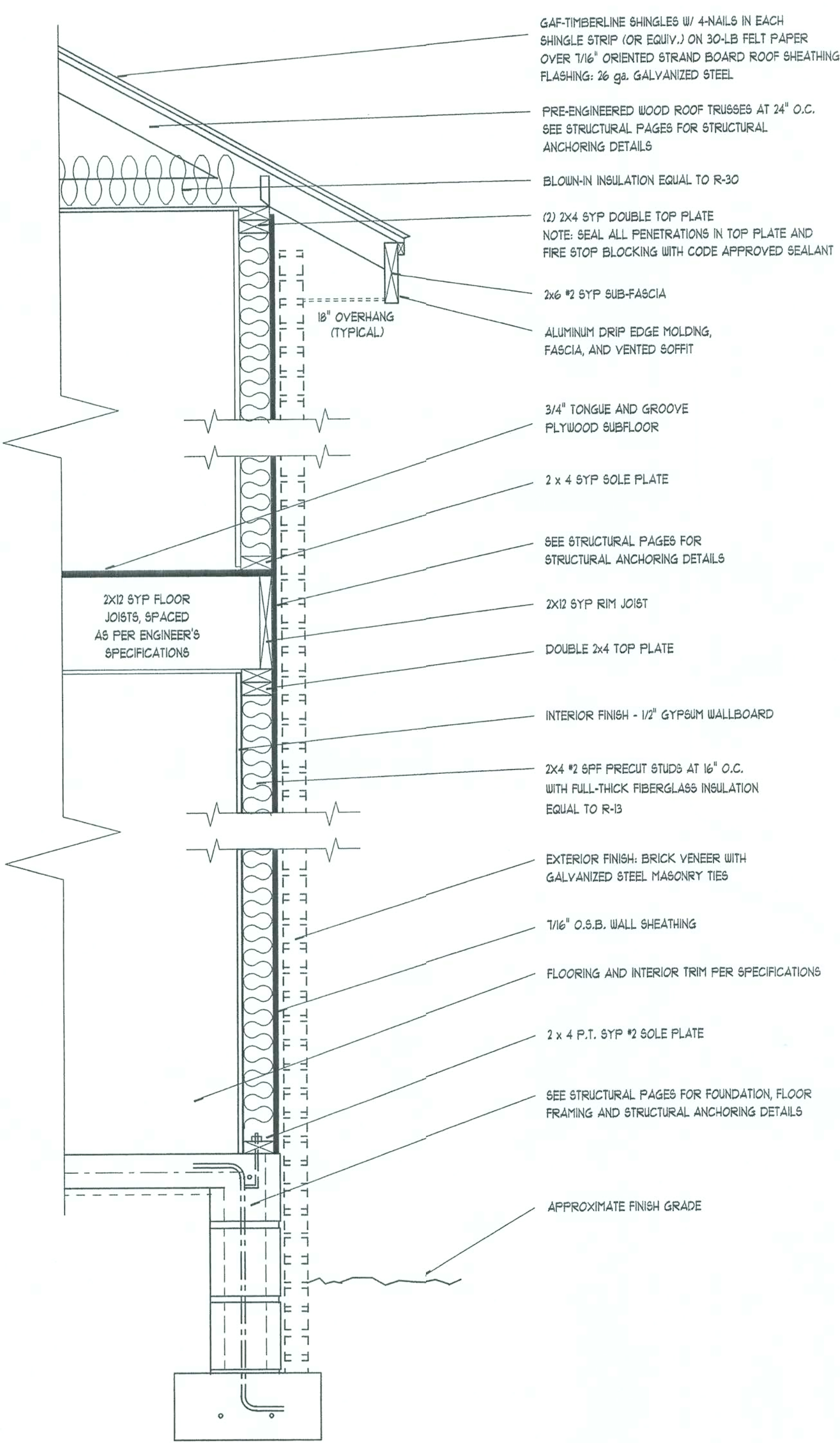
DESIGNED FOR:

**ELECTRICAL PLAN**  
**TYPICAL WALL SECTION**

SHEET NUMBER  
4 of 4

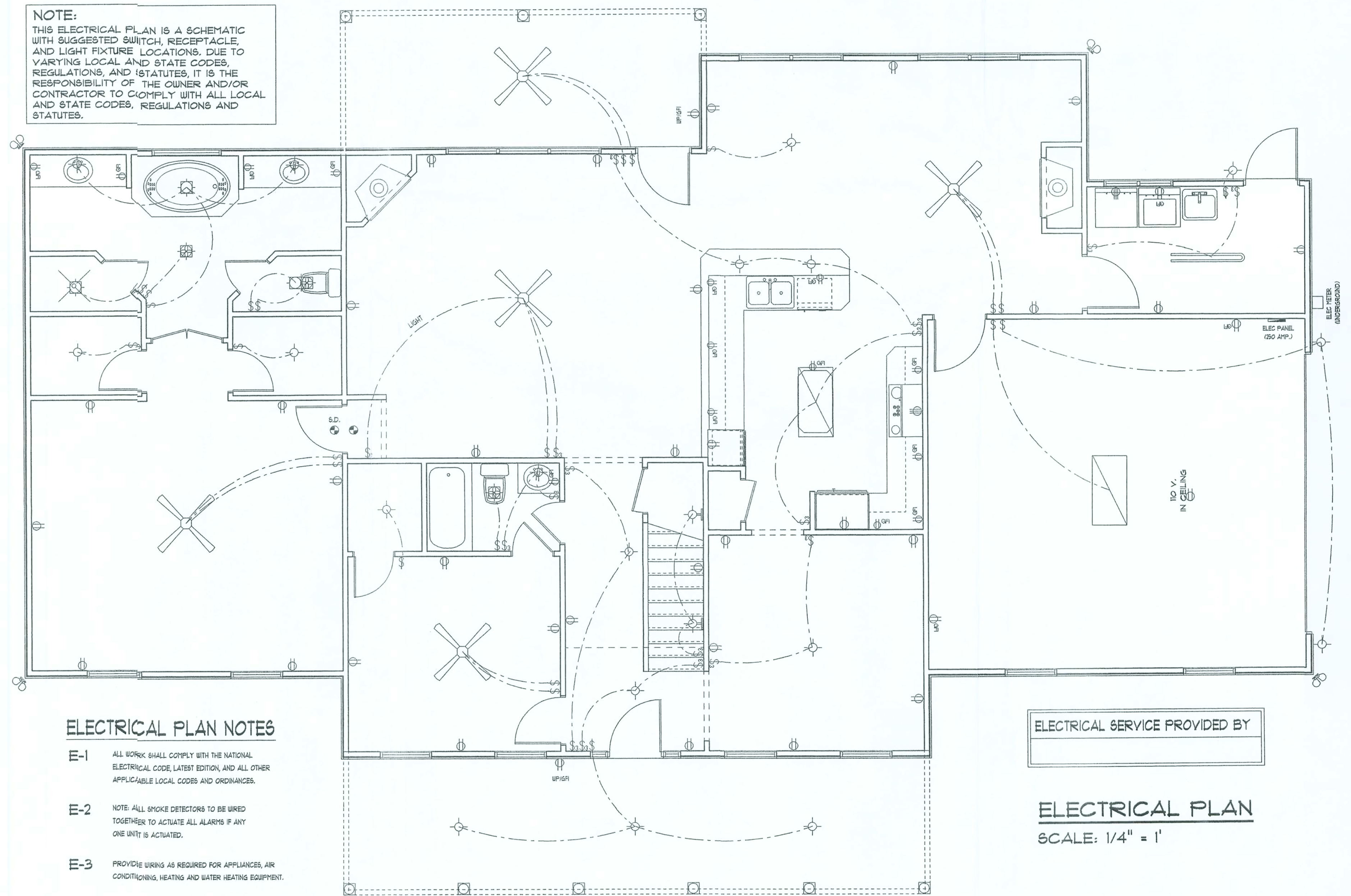
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Contractor shall verify all dimensions prior to commencing construction.

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**TWO-STORY TYPICAL WALL SECTION**  
SCALE: 1" = 1'  
REFER TO STRUCTURAL PAGE FOR STRUCTURAL SPECIFICATIONS

**NOTE:**  
THIS ELECTRICAL PLAN IS A SCHEMATIC WITH SUGGESTED SWITCH, RECEPTACLE, AND LIGHT FIXTURE LOCATIONS. DUE TO VARYING LOCAL AND STATE CODES, REGULATIONS, AND STATUTES, IT IS THE RESPONSIBILITY OF THE OWNER AND/OR CONTRACTOR TO COMPLY WITH ALL LOCAL AND STATE CODES, REGULATIONS AND STATUTES.

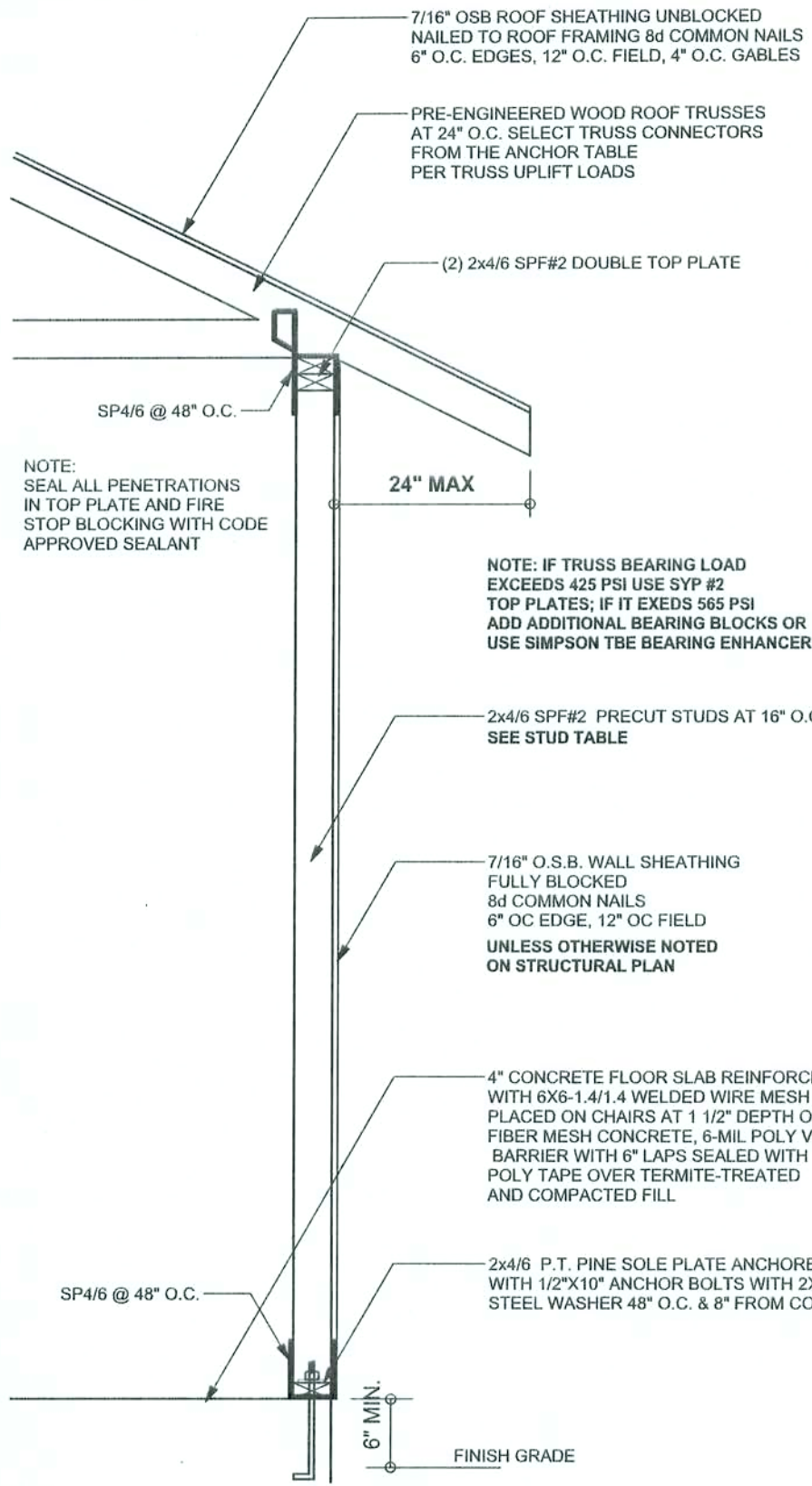


**ELECTRICAL PLAN NOTES**

- E-1 ALL WORK SHALL COMPLY WITH THE NATIONAL ELECTRICAL CODE, LATEST EDITION AND ALL OTHER APPLICABLE LOCAL CODES AND ORDINANCES.
- E-2 NOTE: ALL SMOKE DETECTORS TO BE WIRED TOGETHER TO ACTUATE ALL ALARMS IF ANY ONE UNIT IS ACTUATED.
- E-3 PROVIDE WIRING AS REQUIRED FOR APPLIANCES, AIR CONDITIONING, HEATING AND WATER HEATING EQUIPMENT.
- E-4 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)

ELECTRICAL SERVICE PROVIDED BY

**ELECTRICAL PLAN**  
SCALE: 1/4" = 1'

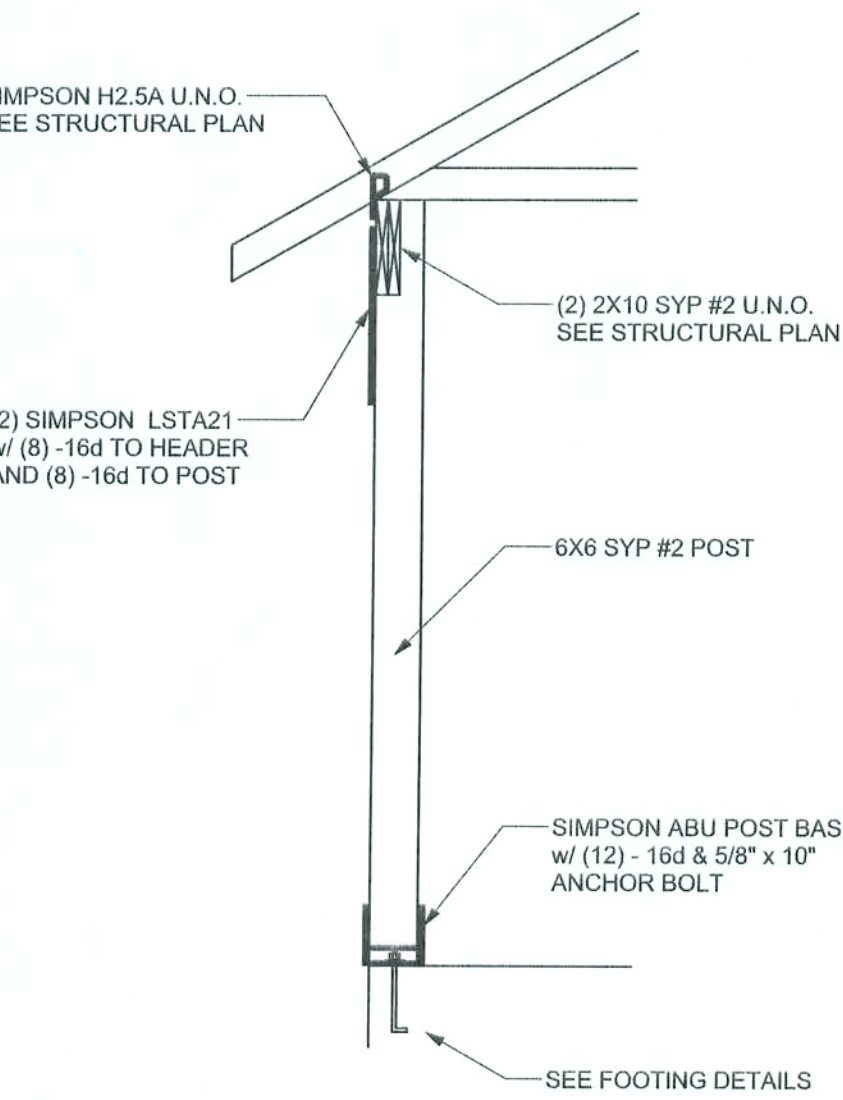


**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"

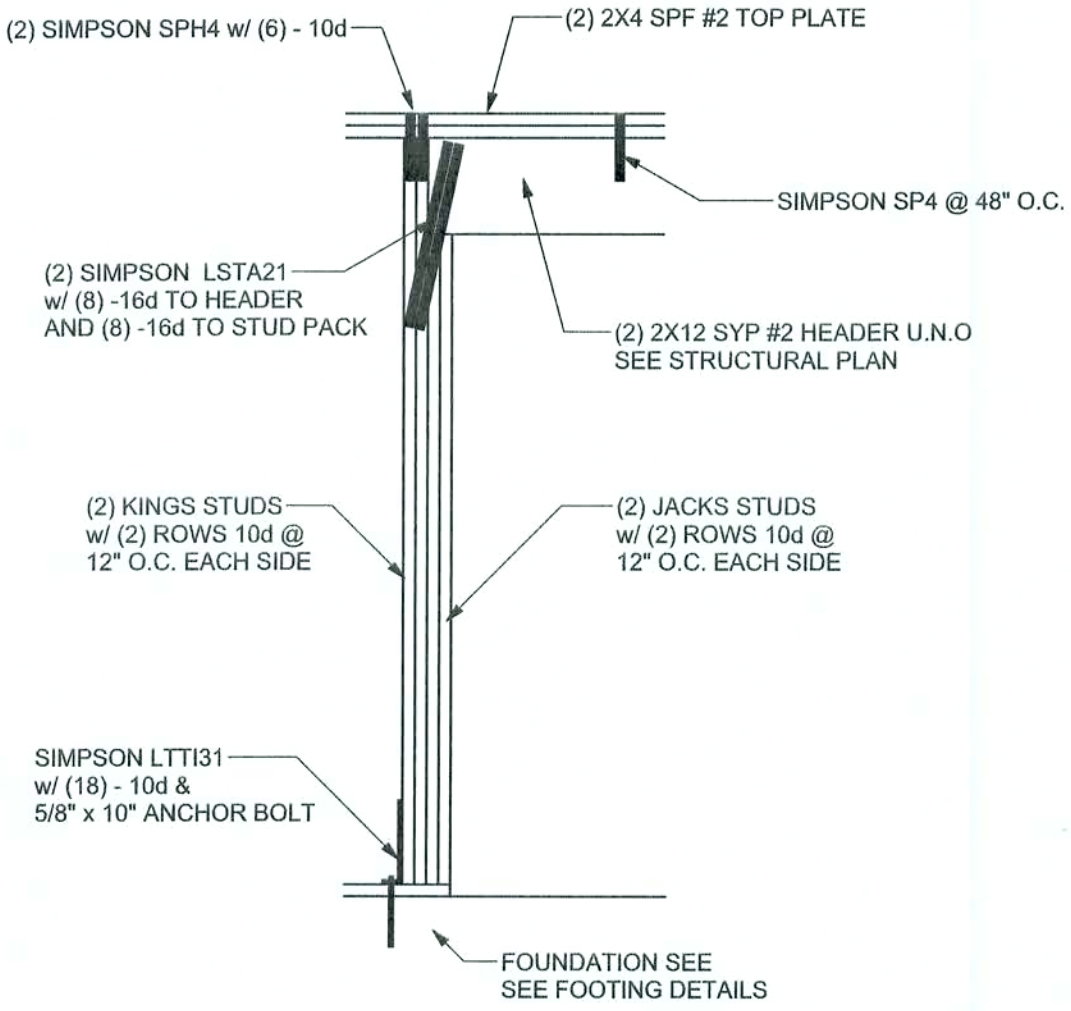
**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS**

|                  |                        |
|------------------|------------------------|
| (1) 2x4 @ 16" OC | TO 11'-9" STUD HEIGHT  |
| (1) 2x4 @ 12" OC | TO 13'-0" STUD HEIGHT  |
| (1) 2x6 @ 16" OC | TO 18'-10" STUD HEIGHT |
| (1) 2x6 @ 12" OC | TO 20'-0" STUD HEIGHT  |

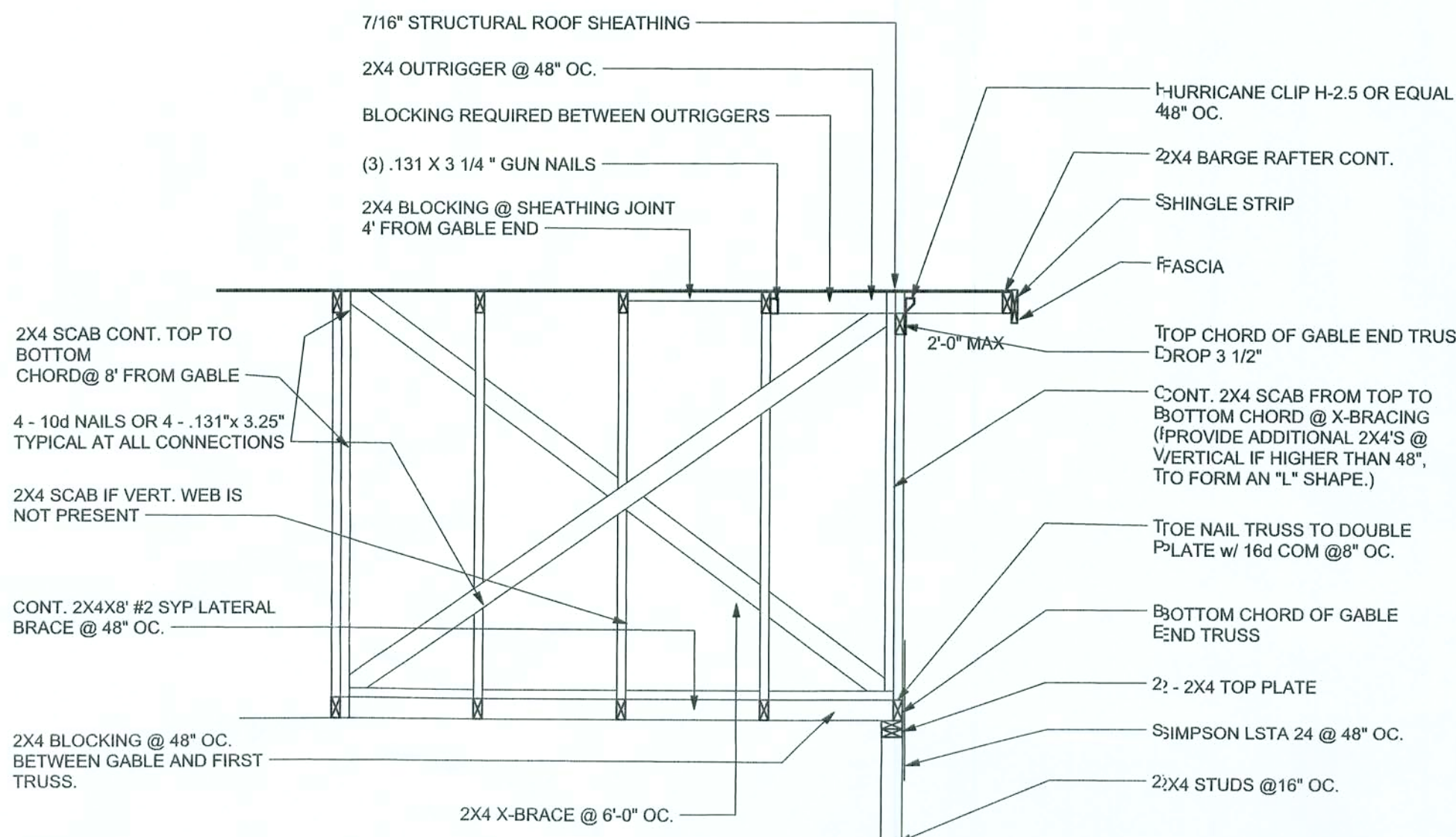
THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.206.  
EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS  
RESISTING INTERIOR ZONE WINDLOADS 10 MPH EXPOSURE B.  
STUD SPACINGS SHALL BE MAINTAINED BY 0.5% FOR FRAMING  
LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.  
EXAMPLE 16" O.C. x 0.15 = 13'-9" O.C.



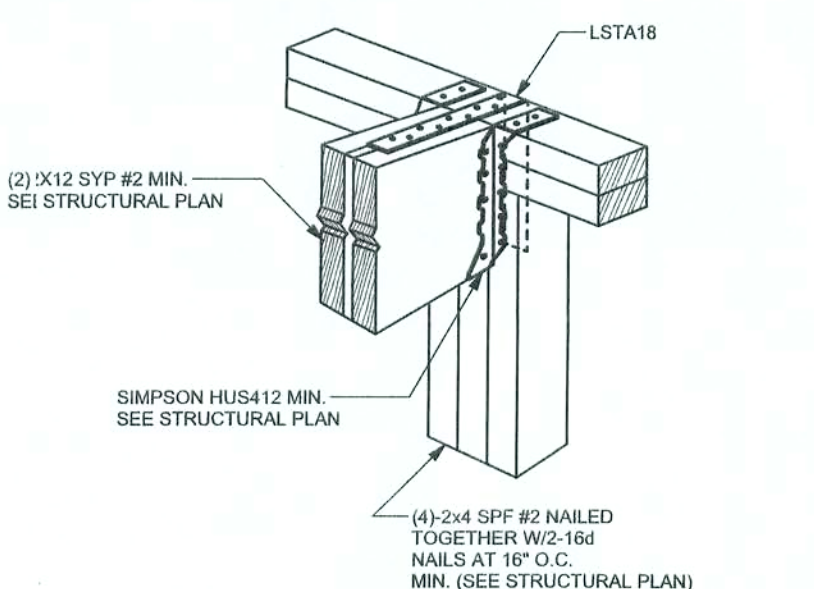
**TYPICAL PORCH POST DETAIL**  
SCALE: 1/2" = 1'-0"



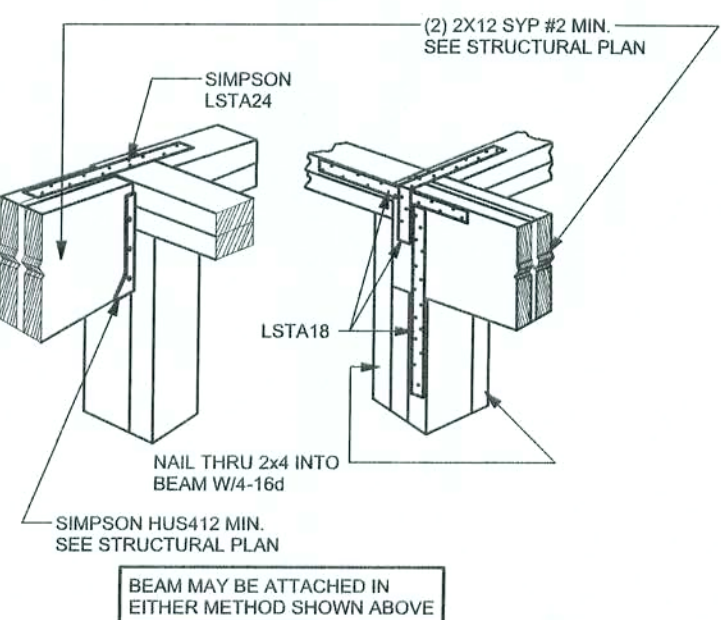
**TYPICAL GARAGE DOOR HEADER STRAPING DETAIL**  
SCALE: 1/2" = 1'-0"



**TYPICAL GABLE END (X-BRACING)**  
ALL MEMBERS SHALL BE SYP

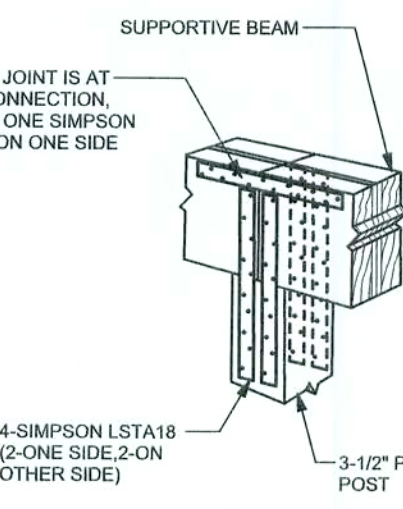


**BEAM MID-WALL CONNECTION DETAIL**  
SCALE: N.T.S.



**BEAM CORNER CONNECTION DETAIL**  
SCALE: N.T.S.

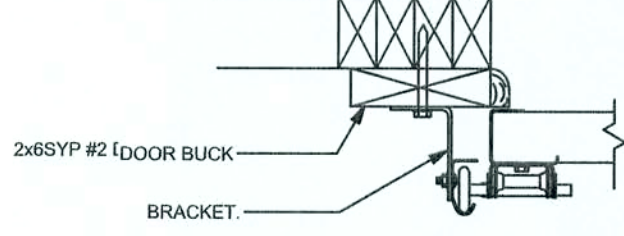
**SUPPORTIVE POST TO BEAM  
DETAIL FOR SINGLE BEAM**  
SCALE: N.T.S.



**SUPPORTIVE CENTER POST TO BEAM DETAIL**  
SCALE: N.T.S.

**2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT**  
ATTACH GARAGE DOOR BUCK TO STUD PACK AT  
EACH SIDE OF DOOR OPENING WITH 30"x4" LAG  
SCREWS w/ 1" WASHER LAG SCREWS MAY BE  
COUNTERBORN. HORIZONTAL JAMBS DO NOT  
TRANSFER LOAD. CENTER LAG SCREWS DO NOT  
STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"  
OR PER TABLE BELOW.

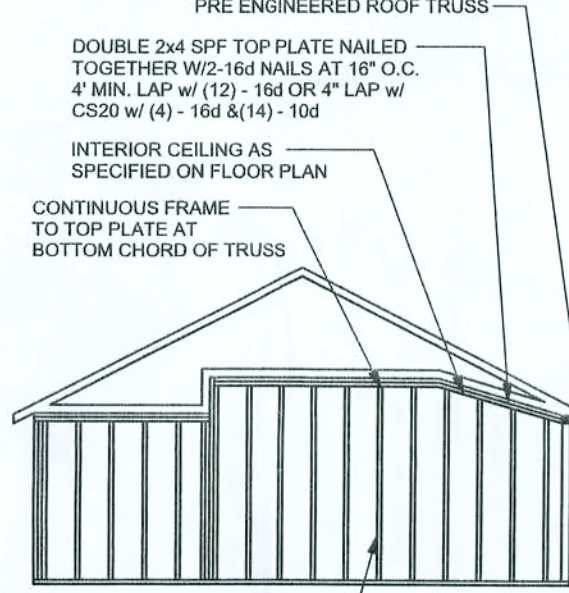
| DOOR WIDTH | 3/8" x 4" LAG | 16d<br>STAGGER | (2) ROWS OF<br>.131 x 3 1/4" GN |
|------------|---------------|----------------|---------------------------------|
| 8' - 10'   | 24" O.C.      | 5" O.C.        | 5" O.C.                         |
| 11' - 15'  | 18" O.C.      | 4" O.C.        | 4" O.C.                         |
| 16' - 18'  | 16" O.C.      | 3" O.C.        | 3" O.C.                         |



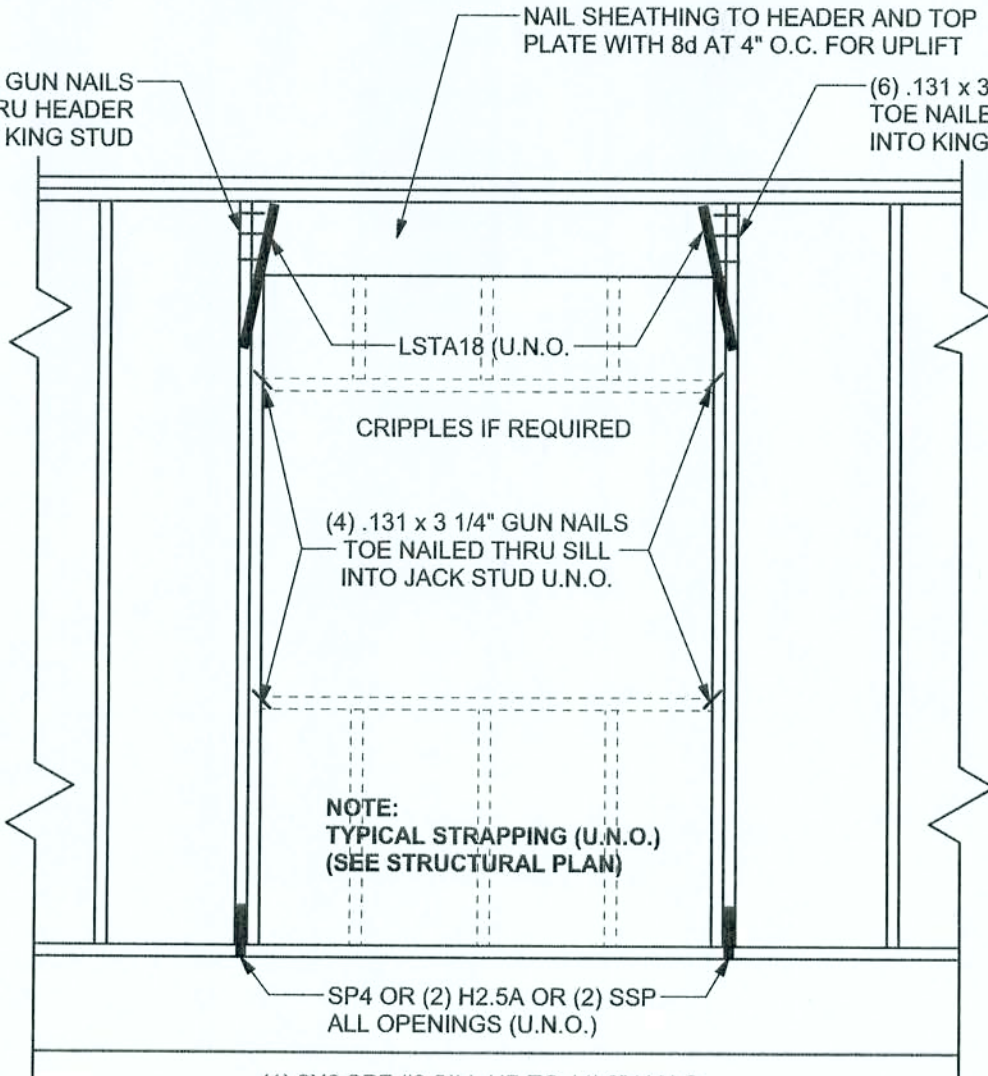
**GARAGE DOOR BUCK INSTALLATION DETAIL**  
SCALE: N.T.S.

**GRADE & SPECIES TABLE**

|      |              | Fb (psi) | E (10 <sup>6</sup> psi) |
|------|--------------|----------|-------------------------|
| 2x8  | SYP #2       | 1200     | 1.6                     |
| 2x10 | SYP #2       | 1050     | 1.6                     |
| 2x12 | SYP #2       | 975      | 1.6                     |
| GLB  | 24F-V3 SP    | 2400     | 1.8                     |
| LSL  | TIMBERSTRAND | 1700     | 1.7                     |
| LVL  | MICROLAM     | 1600     | 1.9                     |
| PSL  | PARALAM      | 2900     | 2.0                     |



**CONTINUOUS FRAME TO  
CEILING DIAPHRAGM DETAIL**  
SCALE: N.T.S.



**TYPICAL HEADER STRAPING DETAIL**  
SCALE: 1/2" = 1'-0"

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE  
FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND  
PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR  
ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER  
AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S  
RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO  
SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR  
INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR  
REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT  
CONNECTION 15LB EACH END, 2X6 RAFTERS 70 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET  
GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS  
VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 3000 PSI

WELDED WIRE REINFORCED SLAB: 8" x 8" W14 x W14, F<sub>y</sub> = 85KSI, WELDED WIRE REINFORCEMENT FABRIC  
(W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED  
MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.  
FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD  
PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER  
TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN  
ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH /  
WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 10 FT. DO NOT  
CUT WMM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO  
OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS  
BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F<sub>y</sub> = 60 KSI, ALL LAP SPICES 40" DB (25" FOR #5 BARS);  
UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F<sub>b</sub> = 2.4ksi, E = 1800ksi, UNO. SUPPLIER MAY SUPPLY AN  
ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.  
ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16" OSB SHEATHING, UNBLOCKED,  
APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES  
STAGGERED, FASTENED WITH 8" COMMON NAILS (.131), 6" O.C. PANEL EDGES, 12" O.C. INTERMEDIATE  
MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" O.C. UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS,  
AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT DEVICE OF THE  
SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE  
TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION  
INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: 4-30" ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO  
LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH  
3/4" BOLTS TO BE 3" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST  
REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

**BUILDER'S RESPONSIBILITY**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE  
SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND  
BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004  
REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU  
BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL  
THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS  
DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,  
TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL  
BEARING LOCATIONS.

**ROOF SYSTEM DESIGN**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION  
R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN  
TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS  
THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE  
COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS  
MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN  
PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED  
LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO  
BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF  
DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT  
RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE  
TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES  
RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED  
TRUSS SHEETS.

**MASONRY NOTES:**

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL  
CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY  
STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON  
MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF  
ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.  
ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER  
IN WRITING.

|         | ACI530.1-02 Section              | Specific Requirements                                                                                                                                                                     |
|---------|----------------------------------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| 1.4A    | Compressive strength             | 8" block bearing walls F'm = 1500 psi                                                                                                                                                     |
| 2.1     | Mortar                           | ASTM C 270, Type N, UNO                                                                                                                                                                   |
| 2.2     | Grout                            | ASTM C 476, admixtures require approval                                                                                                                                                   |
| 2.3     | CMU standard                     | ASTM C 90-02, Normal weight, hollow,<br>medium surface finish, 8"x8"x16" running<br>bond and 12"x12" or 16"x16" column<br>block                                                           |
| 2.3     | Clay brick standard              | ASTM C 216-02, Grade SW, Type FBS,<br>5.5"x2.75"x11.5"                                                                                                                                    |
| 2.4     | Reinforcing bars, #3 - #11       | ASTM 615, Grade 60, F <sub>y</sub> = 60 ksi, Lap<br>splices min 48 bar dia, (30" for #5)                                                                                                  |
| 2.4F    | Coating for corrosion protection | Anchors, sheet metal ties completely<br>embedded in mortar or grout, ASTM<br>A525, Class 60, 0.60 oz/lb or 30MS                                                                           |
| 2.4F    | Coating for corrosion protection | Joint reinforcement in walls exposed to<br>moisture or wire ties, anchors, sheet metal<br>ties not completely embedded in mortar or<br>grout, ASTM A153, Class 62, 1.50 oz/lb2<br>or 30MS |
| 3.3.E.2 | Pipes, conduits, and accessories | Any not shown on the project drawings<br>require engineering approval.                                                                                                                    |
| 3.3.E.7 | Movement joints                  | Contractor assumes responsibility for type<br>and location of movement joints if not<br>detailed on project drawings.                                                                     |

**ANCHOR TABLE**

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS  
MANUFACTURER'S ENGINEERING

| UPLIFT LBS. SYP        | UPLIFT LBS. SPF | TRUSS CONNECTOR*      | TO PLATES      | TO RAFTER/TRUSS | TO STUDS                             |
|------------------------|-----------------|-----------------------|----------------|-----------------|--------------------------------------|
| < 420                  | < 245           | H5A                   | 3-8d           | 3-8d            |                                      |
| < 455                  | < 265           | H5                    | 4-8d           | 4-8d            |                                      |
| < 360                  | < 235           | H4                    | 4-8d           | 4-8d            |                                      |
| < 455                  | < 320           | H3                    | 4-8d           | 4-8d            |                                      |
| < 415                  | < 365           | H2.5                  | 5-8d           | 5-8d            |                                      |
| < 600                  | < 535           | H2.5A                 | 5-8d           | 5-8d            |                                      |
| < 950                  | < 820           | H6                    | 8-8d           | 8-8d            |                                      |
| < 745                  | < 565           | H8                    | 5-10d, 1 1/2"  | 5-10d, 1 1/2"   |                                      |
| < 1465                 | < 1050          | H14-1                 | 13-8d          | 12-8d, 1 1/2"   |                                      |
| < 1465                 | < 1050          | H14-2                 | 15-8d          | 12-8d, 1 1/2"   |                                      |
| < 990                  | < 850           | H10-1                 | 8-8d, 1 1/2"   | 8-8d, 1 1/2"    |                                      |
| < 760                  | < 655           | H10-2                 | 6-10d          | 6-10d           |                                      |
| < 1470                 | < 1265          | H16-1                 | 10-10d, 1 1/2" | 2-10d, 1 1/2"   |                                      |
| < 1470                 | < 1265          | H16-2                 | 10-10d, 1 1/2" | 2-10d, 1 1/2"   |                                      |
| < 1000                 | < 860           | MTS24C                | 7-10d 1 1/2"   | 7-10d 1 1/2"    |                                      |
| < 1450                 | < 1245          | HTS24                 | 12-10d 1 1/2"  | 12-10d 1 1/2"   |                                      |
| < 2900                 | < 2490          | 2 - HTS24             |                |                 |                                      |
| < 2050                 | < 1785          | LG12                  | 14 -16d        | 14 -16d         |                                      |
| HEAVY GIRDER TIEWINDS* |                 |                       | TO FOUNDATION  |                 |                                      |
| < 3965                 | < 3330          | MGT                   |                | 22 -10d         | 1-5/8" THREADED ROD<br>12" EMBEDMENT |
| < 10980                | < 6485          | HGT-2                 |                | 16 -10d         | 2-5/8" THREADED ROD<br>12" EMBEDMENT |
| < 10530                | < 9035          | HGT-3                 |                | 16 -10d         | 2-5/8" THREADED ROD<br>12" EMBEDMENT |
| < 9250                 | < 9250          | HGT-4                 |                | 16 -10d         | 2-5/8" THREADED ROD<br>12" EMBEDMENT |
| STUD STRAP CONNECTOR*  |                 |                       | TO STUDS       |                 |                                      |
| < 435                  | < 435           | SSP DOUBLE TOP PLATE  | 3-10d          |                 | 4 -10d                               |
| < 455                  | < 420           | SSP SINGLE SILL PLATE | 1-10d          |                 | 4 -10d                               |
| < 825                  | < 825           | DSP DOUBLE TOP PLATE  | 6-10d          |                 | 8 -10d                               |
| < 825                  | < 600           | DSP SINGLE SILL PLATE | 2-10d          |                 | 8 -10d                               |
| < 885                  | < 760           | SP4                   |                |                 | 6-10d, 1 1/2"                        |
| < 1240                 | < 1065          | SPH4                  |                |                 | 10-10d, 1 1/2"                       |
| < 885                  | < 760           | SP6                   |                |                 | 6-10d, 1 1/2"                        |
| < 1240                 | < 1065          | SPH6                  |                |                 | 10-10d, 1 1/2"                       |
| < 1235                 | < 1165          | LSTA18                | 14-10d         |                 |                                      |
| < 1235                 | < 1235          | LSTA21                | 16-10d         |                 |                                      |
| < 1030                 | < 1030          | CS20                  | 18-8d          |                 |                                      |
| < 1705                 | < 1705          | CS16                  | 28-8d          |                 |                                      |
| STUD ANCHORS*          |                 |                       | TO STUDS       |                 |                                      |
| < 1310                 | < 1305          | LTT19                 | 8-16d          |                 | 1/2" AB                              |
| < 2350                 | < 2310          | LTT31                 | 18-10d, 1 1/2" |                 | 1/2" AB                              |
| < 2775                 | < 2570          | HD2A                  | 2-5/8" BOLTS   |                 | 5/8" AB                              |
| < 4175                 | < 3695          | HTT16                 | 18 -16d        |                 | 5/8" AB                              |
| < 1400                 | < 1400          | PAHD42                | 16-16d         |                 |                                      |
| < 3335                 | < 3335          | HPAHD22               | 16-16d         |                 |                                      |
| < 2200                 | < 2200          | ABU44                 | 12-16d         |                 | 1/2" AB                              |
| < 2300                 | < 2300          | ABU66                 | 12-16d         |                 | 1/2" AB                              |
| < 2320                 | < 2320          | ABU88                 | 18 -16d        |                 | 2-5/8" AB                            |

**DESIGN DATA**

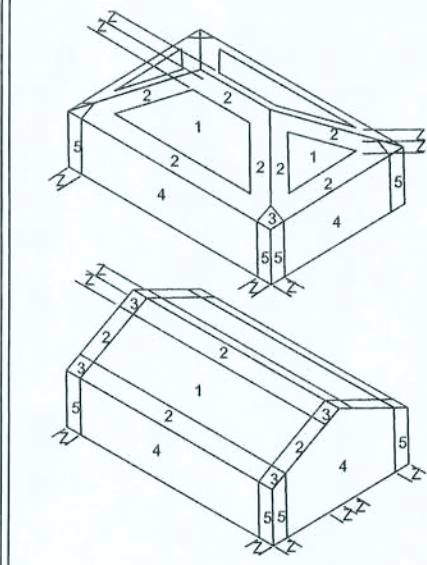
**WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1**

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;  
MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT  
ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10%  
SLOPE AND UNOBSERVED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

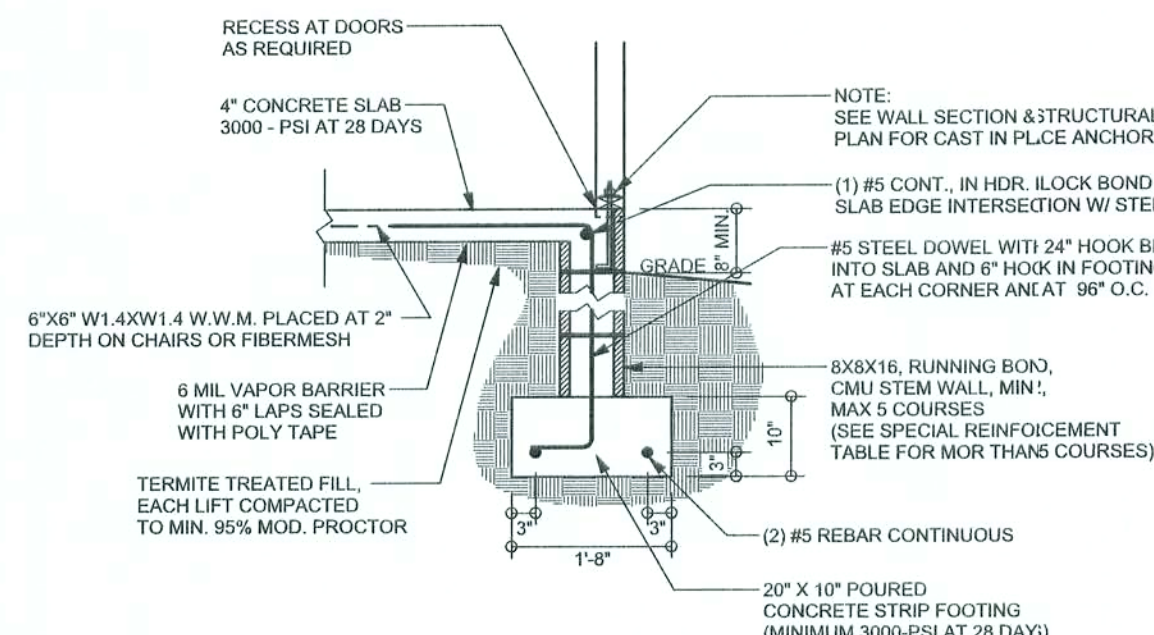


| Zone   | Effective Wind Area (ft <sup>2</sup> ) | 10   | 100   |
|--------|----------------------------------------|------|-------|
| 1      | 19.9 -21.1                             | 18.1 | -18.1 |
| 2      | 19.9 -25.1                             | 18.1 | -21.8 |
| 2 0thg | -40.1                                  |      | -40.6 |
| 3      | 19.9 -25.1                             | 18.1 | -21.8 |
| 3 0thg | -26.1                                  |      | -42.4 |
| 4      | 21.8 -23.4                             | 18.5 | -20.4 |
| 5      | 21.8 -29                               | 18.5 | -22   |

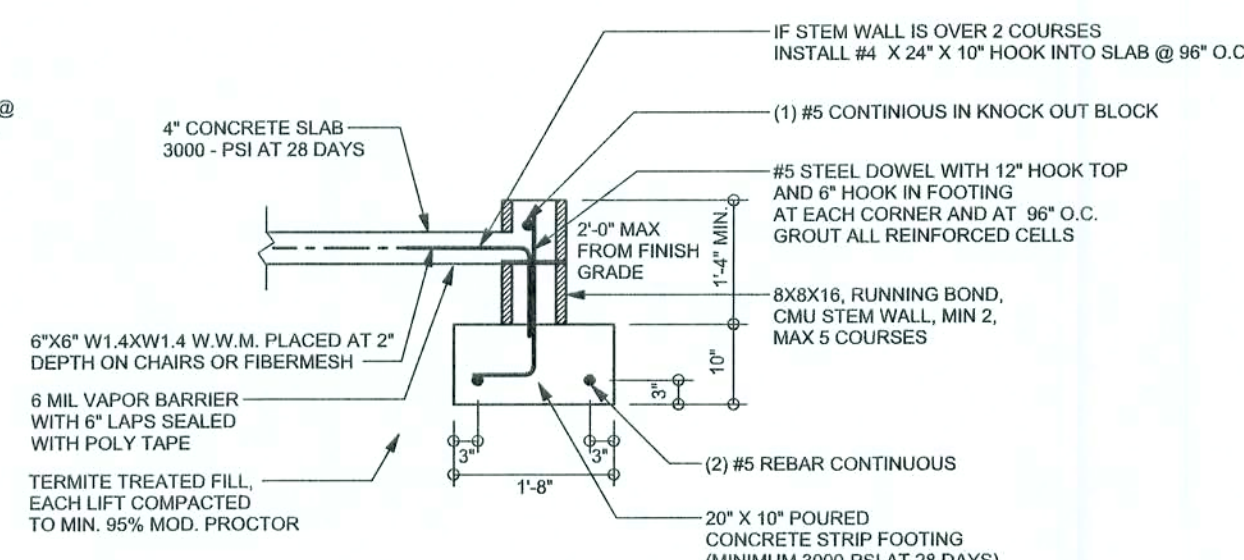
REVISIONS

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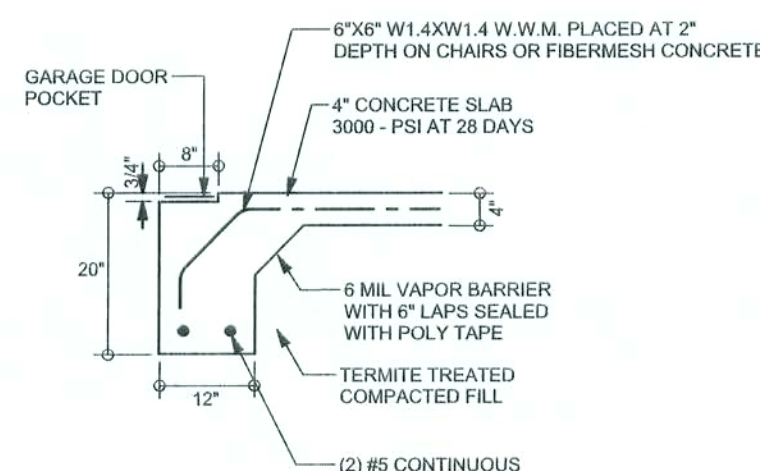
SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



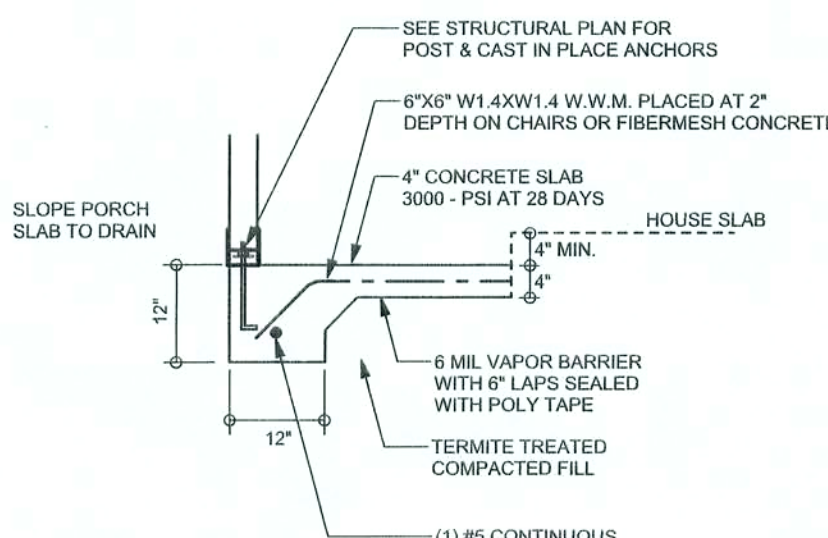
**F9 S-2 STEM WALL FOOTING**  
SCALE: 1/2" = 1'-0"



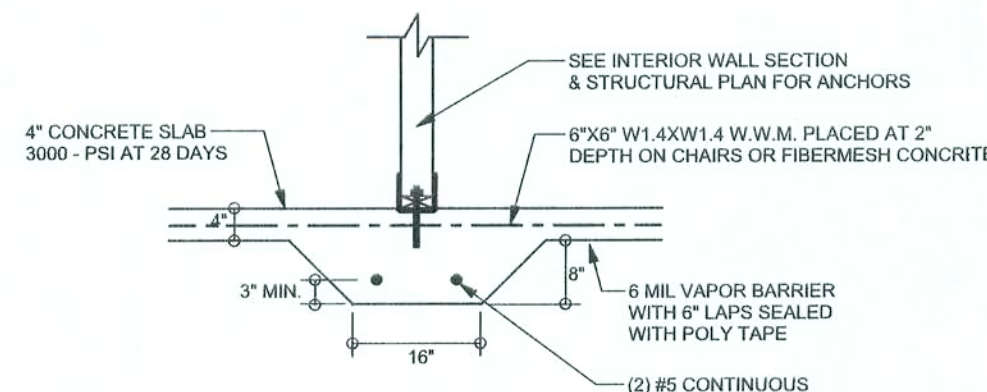
**F18 S-2 STEM WALL CURB FOOTING**  
SCALE: 1/2" = 1'-0"



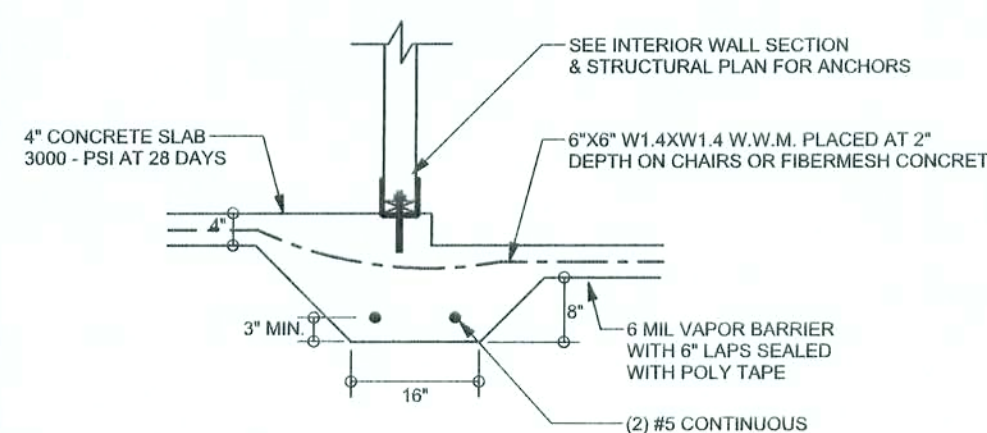
**F4 S-2 GARAGE DOOR FOOTING**  
SCALE: 1/2" = 1'-0"



**F5 S-2 PORCH FOOTING**  
SCALE: 1/2" = 1'-0"



**F2 S-2 INTERIOR BEARING FOOTING**  
SCALE: 1/2" = 1'-0"



**F3 S-2 INTERIOR BEARING STEP FOOTING**  
SCALE: 1/2" = 1'-0"

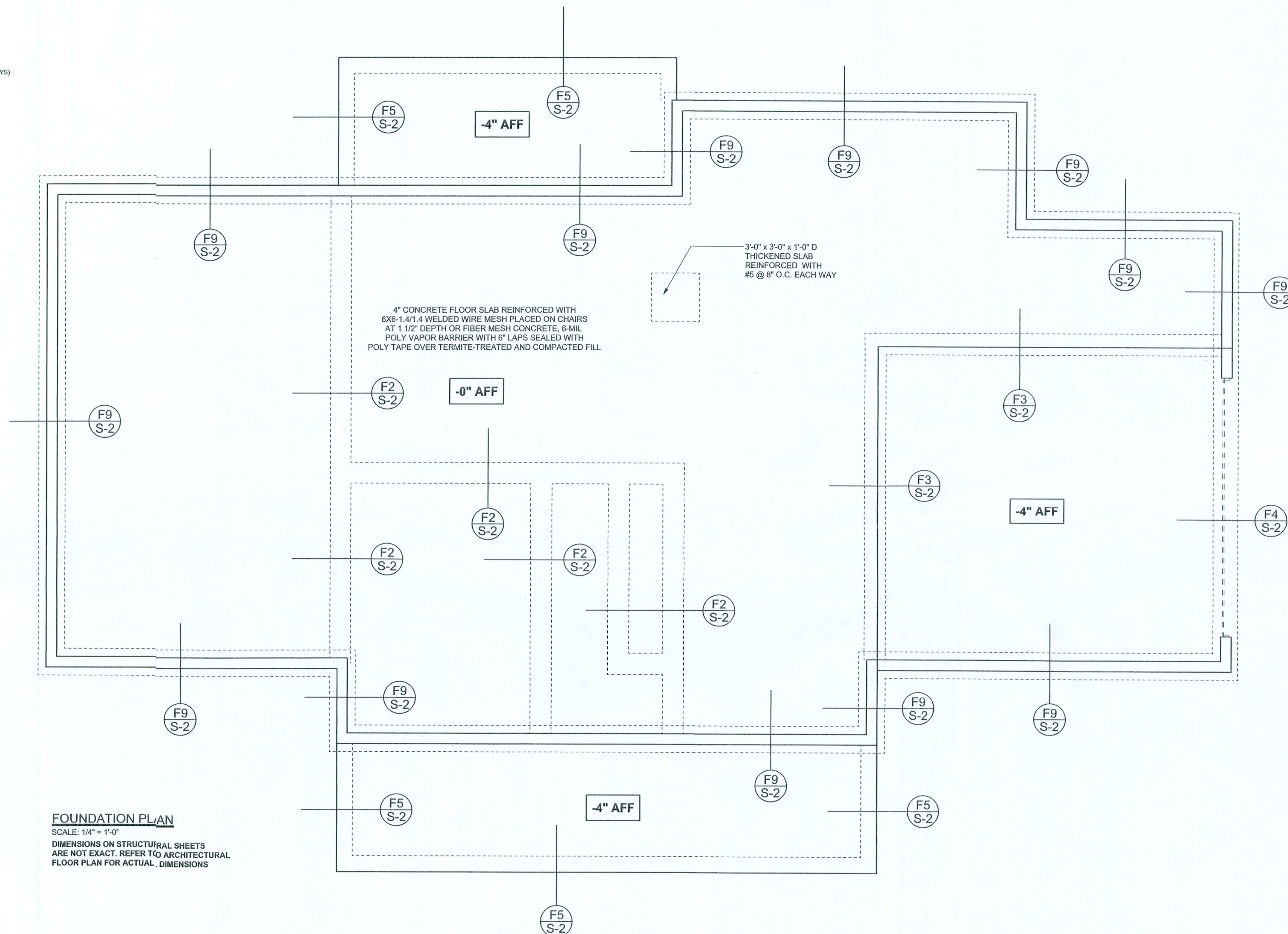
**TALL STEM WALL TABLE**

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 165 continuous at mid-height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

| STEM WALL HEIGHT (FEET) | UNBALANCED BACKFILL HEIGHT | VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.) |    |    | VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.) |    |    |
|-------------------------|----------------------------|-----------------------------------------------------------|----|----|------------------------------------------------------------|----|----|
|                         |                            | #5                                                        | #7 | #8 | #5                                                         | #7 | #8 |
| 3.3                     | 3.0                        | 96                                                        | 96 | 96 | 96                                                         | 96 | 96 |
| 4.0                     | 3.7                        | 96                                                        | 96 | 96 | 96                                                         | 96 | 96 |
| 4.7                     | 4.3                        | 88                                                        | 96 | 96 | 96                                                         | 96 | 96 |
| 5.3                     | 5.0                        | 56                                                        | 96 | 96 | 96                                                         | 96 | 96 |
| 6.0                     | 5.7                        | 40                                                        | 80 | 96 | 80                                                         | 96 | 96 |
| 6.7                     | 6.3                        | 32                                                        | 56 | 80 | 56                                                         | 96 | 96 |
| 7.3                     | 7.0                        | 24                                                        | 40 | 56 | 40                                                         | 80 | 96 |
| 8.0                     | 7.7                        | 16                                                        | 32 | 48 | 32                                                         | 64 | 80 |
| 8.7                     | 8.3                        | 8                                                         | 24 | 32 | 24                                                         | 48 | 64 |
| 9.3                     | 9.0                        | 8                                                         | 16 | 24 | 16                                                         | 40 | 48 |

**FOUNDATION PLAN**

SCALE: 1/4" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS



WINDLOAD ENGINEER: Mark Disoway,  
PE No. 53915, P.O. Box 868, Lake City, FL  
32056, 386-754-5419

**DIMENSIONS:**  
Scaled dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section FDOT 2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53915

*Mark Disoway*  
29 DEC 05  
SEAL

**Don Reed**  
Construction

William & Bonnie  
Robbins Residence

ADDRESS:  
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Fax: (386) 269 - 4871

PRINTED DATE:  
December 29, 2005

DRAWN BY: David Disoway

CHECKED BY:

FINALS DATE:  
29 / Dec / 05

JOB NUMBER:  
512228

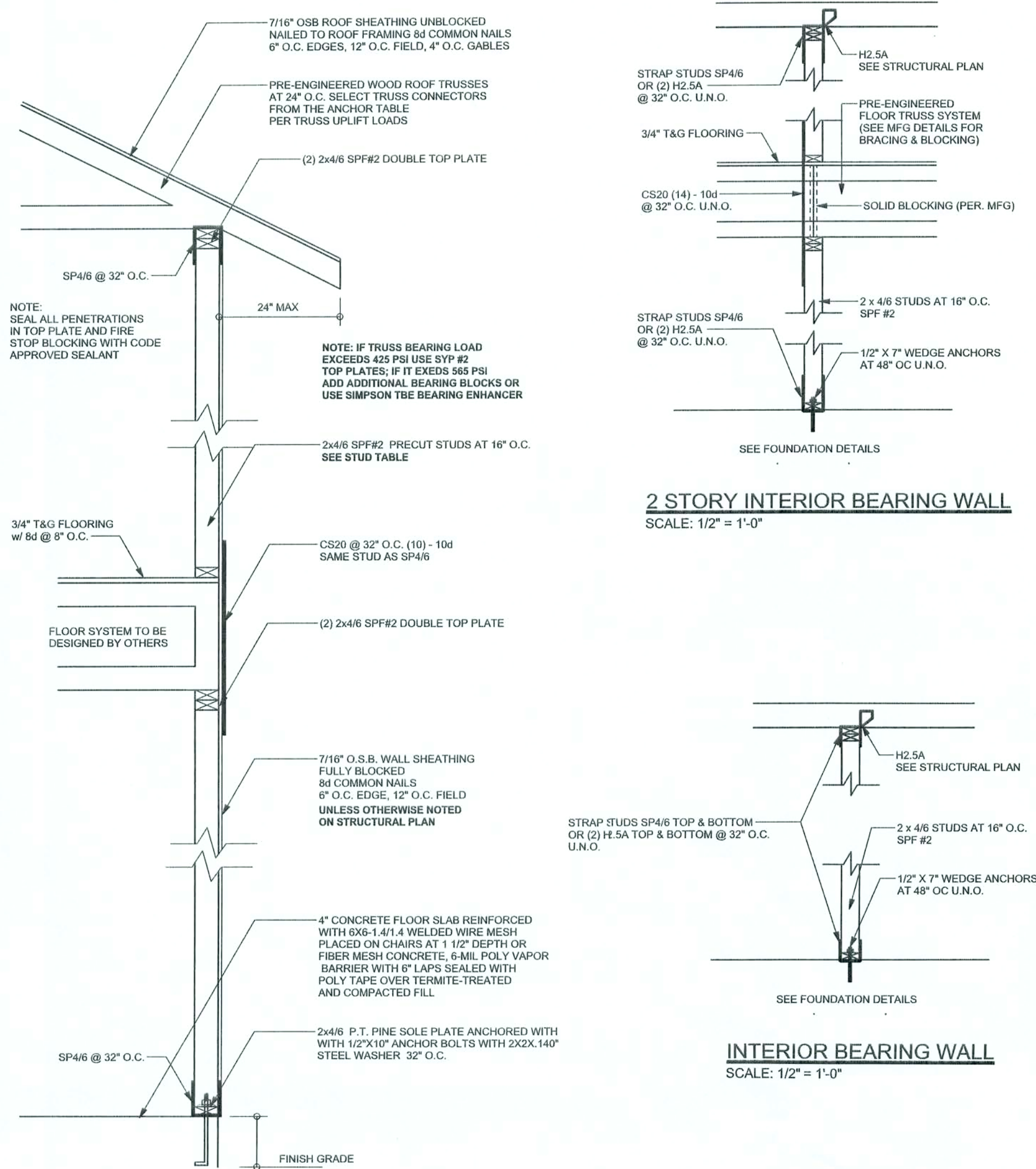
DRAWING NUMBER

**S-2**

OF 4 SHEETS

| REVISIONS |  |
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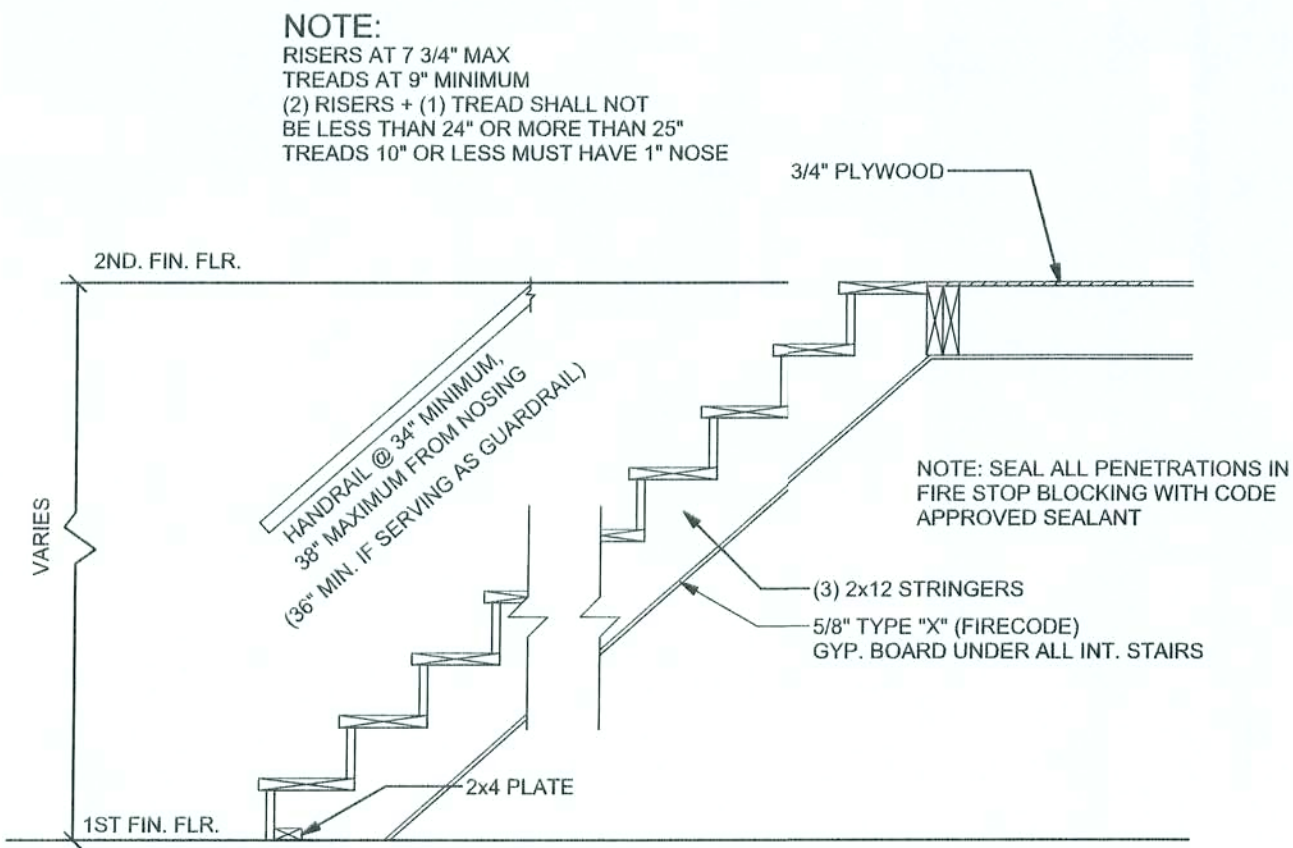
SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



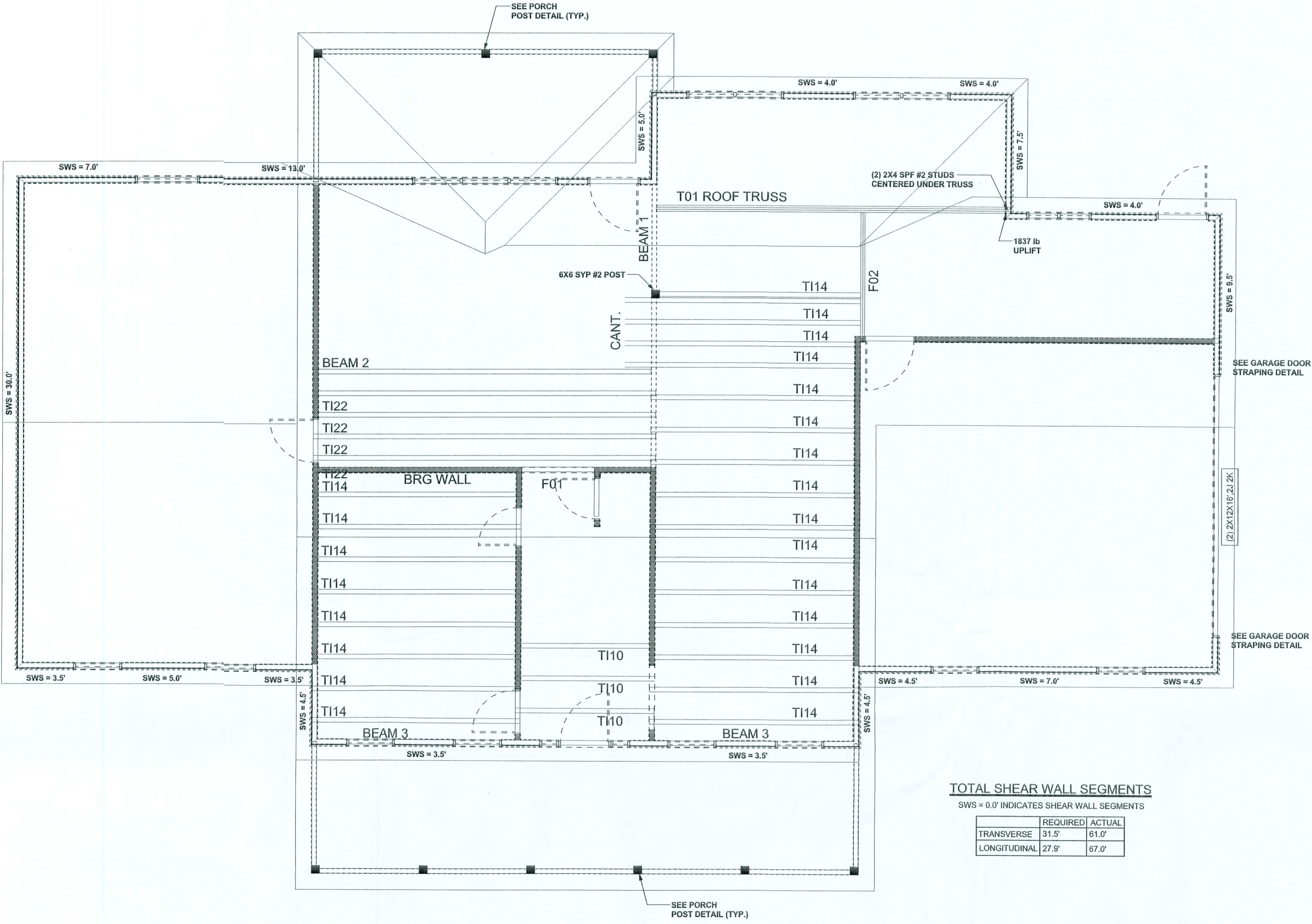
**TWO STORY WALL SECTION**  
SCALE: 3/4\" = 1'-0"

**2 STORY INTERIOR BEARING WALL**  
SCALE: 1/2\" = 1'-0"

**INTERIOR BEARING WALL**  
SCALE: 1/2\" = 1'-0"



**1403 STAIR DETAIL**  
SCALE: NTS



**STRUCTURAL PLAN**  
SCALE: 1/4\" = 1'-0"

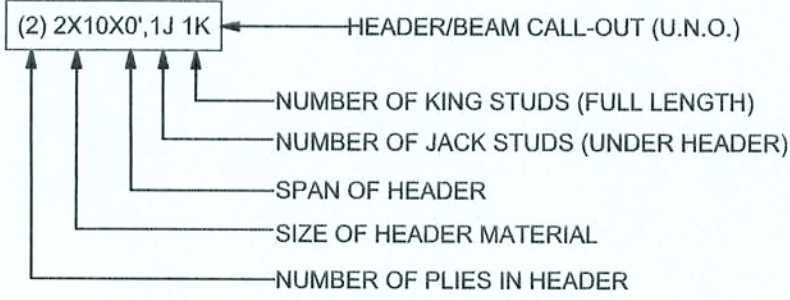
**STRUCTURAL PLAN NOTES**

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS11-03, BCS1-B1, BCS1-B2, & BCS1-B3. BCS1-B1, BCS1-B2, & BCS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

**WALL LEGEND**

|                   |                                                                                                                                 |
|-------------------|---------------------------------------------------------------------------------------------------------------------------------|
| <b>SWS = 0.0'</b> | 1ST FLOOR EXTERIOR WALL WITH 7/16\" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6\" O.C. EDGE, 12\" O.C. FIELD (U.N.O.) |
| <b>SWS = 0.0'</b> | 2ND FLOOR EXTERIOR WALL WITH 7/16\" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6\" O.C. EDGE, 12\" O.C. FIELD (U.N.O.) |
| <b>IBW</b>        | 1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1                                                                       |
| <b>IBW</b>        | 2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1                                                                       |

**HEADER LEGEND**



**TOTAL SHEAR WALL SEGMENTS**

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

|              | REQUIRED | ACTUAL |
|--------------|----------|--------|
| TRANSVERSE   | 31.5'    | 61.0'  |
| LONGITUDINAL | 27.9'    | 67.0'  |

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #132305

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53915

*Mark Disoway*  
3/10/2005

SEAL

**Don Reed Construction**

**William & Bonnie Robbins Residence**

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PRINTED DATE:  
December 29, 2005

DRAWN BY: David Disoway

CHECKED BY:

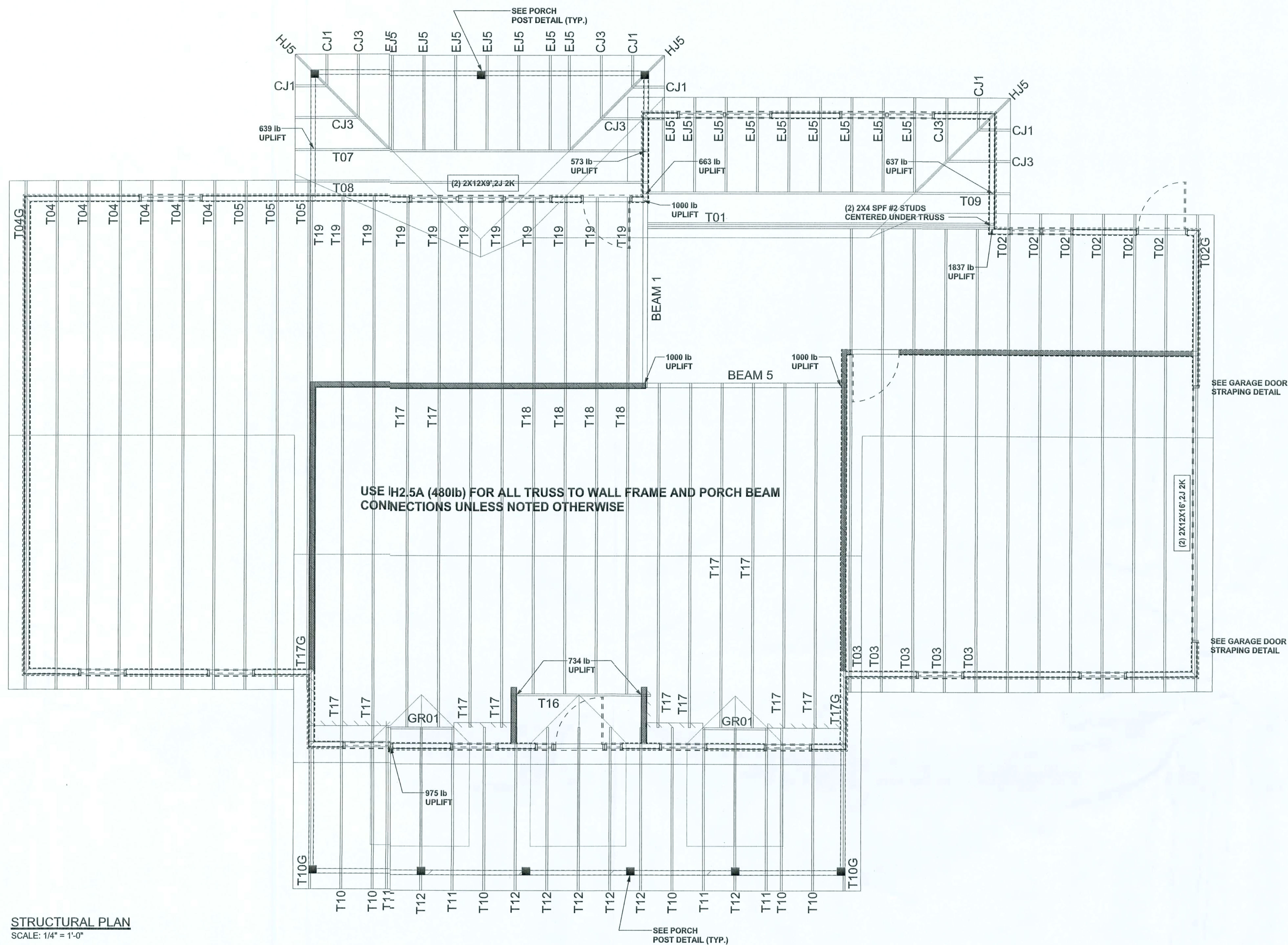
FINALS DATE:  
29 / Dec / 05

JOB NUMBER:  
512228

DRAWING NUMBER  
**S-3**  
OF 4 SHEETS

# REVISIONS

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



## STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

## STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

## WALL LEGEND

|                    |                                                                                                                              |
|--------------------|------------------------------------------------------------------------------------------------------------------------------|
| <b>SWS</b> = 10.0' | 1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.) |
| <b>SWS</b> = 10.0' | 2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.) |
| <b>IBW</b>         | 1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1                                                                    |
| <b>IBW</b>         | 2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1                                                                    |

## HEADER LEGEND

|                    |                                     |
|--------------------|-------------------------------------|
| (2) 2X10X0', 1J 1K | HEADER/BEAM CALL-OUT (U.N.O.)       |
| ↑                  | NUMBER OF KING STUDS (FULL LENGTH)  |
| ↑                  | NUMBER OF JACK STUDS (UNDER HEADER) |
| ↑                  | SPAN OF HEADER                      |
| ↑                  | SIZE OF HEADER MATERIAL             |
| ↑                  | NUMBER OF PLIES IN HEADER           |

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L32305

WINDLOAD ENGINEER: Mark Disosway, P.E. No. 53915, FOS 866, Lake City, FL 33506, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY  
P.E. 53915  
8 Dec 05  
SEAL

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Construction

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David Disosway

CHECKED BY:

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512228

DRAWING NUMBER  
S-4  
OF 4 SHEETS