



COMPONENT DESIGN DRAWINGS & DETAILS

Simpson Strong-Tie
Company, Inc.

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Prepared for:	Century Truss Systems
Job:	P-23-228 - 421 SE Alfred Markham Rd HOUSE
Date:	1/6/2025 12:16 PM
Ref. Number:	258827

Notes:

1. The component design drawings referenced below have been prepared based on design criteria and requirements set forth in the Construction Documents, as communicated by the Component Manufacturer.
2. The engineer's signature on these drawings indicates professional engineering responsibility solely for the individual components to be able to resist the design loads indicated, utilizing all the design parameter and materials indicated or referenced on each individual design.
3. It is the Building Designer's responsibility to review the component design drawings to insure compatibility with the Building design, Refer to all notes on the individual component design drawings.

21 Component Design Drawing(s):

1-GDR01: SID 3323675	7-GE3: SID 3323681	13-T04: SID 3323687	19-T10: SID 3323693
2-GDR02: SID 3323676	8-PB01: SID 3323682	14-T05: SID 3323688	20-T11: SID 3323694
3-GDR03: SID 3323677	9-T01: SID 3323683	15-T06: SID 3323689	21-T12: SID 3323695
4-GDR04: SID 3323678	10-T02: SID 3323684	16-T07: SID 3323690	
5-GE1: SID 3323679	11-T03: SID 3323685	17-T08: SID 3323691	
6-GE2: SID 3323680	12-T03A: SID 3323686	18-T09: SID 3323692	

General Notes

1. Each Truss Design Drawing (TDD) provided with this sheet has been prepared in conformance with ANSI/TPI 1. Refer to ANSI/TPI 1 Chapter 2 for the responsibilities of all parties involved, which include but are not limited to the responsibilities listed on this sheet, and for the definitions of all capitalized terms referenced in this document.
2. TDDs should not be assumed to be to scale.
3. The Contractor and Building Designer shall review and approve the Truss Submittal Package.
4. The suitability and use of the component depicted on the TDD for any particular building design is the responsibility of the Building Designer.
5. The Building Designer is responsible for the anchorage of the truss at all bearing locations as required to resist uplift, gravity and lateral loads, and for all Truss-to-Structural Element connections except Truss-to-Truss connections.
6. The Building Designer shall ensure that the supporting structure can accommodate the vertical and/or horizontal truss deflections.
7. Unless specifically stated otherwise, each Design assumes trusses will be adequately protected from the environment and will not be used in corrosive environments unless protected using an approved method. This includes not being used in locations where the sustained temperature is greater than 150°F.
8. Trusses are designed to carry loads within their plane. Any out-of-plane loads must be resisted by the Permanent Building Stability Bracing.
9. Design dead loads must account for all materials, including self-weight. The TDD notes will indicate the min. pitch above which the dead loads are automatically increased for pitch effects.
10. Trusses installed with roof slopes less than 0.25/12 may experience (but are not designed for) ponding. The Building Designer must ensure that adequate drainage is provided to prevent ponding.
11. Camber is a non-structural consideration and is the responsibility of truss fabricator.

Handling, Installing, Restraint & Bracing

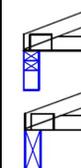
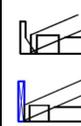
1. The Contractor is responsible for the proper handling, erection, restraint and bracing of the Trusses. In lieu of job-specific details, refer to BCSI.
2. ANSI/TPI 1 stipulates that for trusses spanning 60' or greater, the Owner shall contract with any Registered Design Professional for the design and inspection of the temporary and permanent truss restraint and bracing. Simpson Strong-Tie is not responsible for providing these services.
3. Trusses require permanent lateral restraint to be applied to chords and certain web members (when indicated) at the locations or intervals indicated on the TDD. Web restraints are to be located at mid points, or third points of the member and chord purlins are not to exceed the spacing specified by the TDD. Chords shown without bracing indicated are assumed to be continuously braced by sheathing or drywall. Permanent lateral restraint shall be accomplished in accordance with: standard industry lateral restraint/bracing details in BCSI-B3 or BCSI-B7, supplemental bracing details referenced on the TDD, or as specified in a project-specific truss permanent bracing plan provided by the Building Designer.
4. Additional building stability permanent bracing shall be installed as specified in the Construction Documents.
5. Special end wall bracing design considerations may be required if a flat gable end frame is used with adjacent trusses that have sloped bottom chords (see BCSI-B3).
6. Do not cut, drill, trim, or otherwise alter truss members or plates without prior written approval of an engineer, unless specifically noted on the TDD.
7. Piggyback assemblies shall be braced as per BCSI-B3 unless otherwise specified in the Construction Documents.
8. For floor trusses, when specified, Strongbacking shall be installed per BCSI-B7 unless otherwise specified in the Construction Documents.
9. For IBC 2021 and newer, truss chords without a diaphragm require a project specific bracing design prepared by a registered design professional.

Referenced Standards

ANSI/TPI 1: National Design Standard for Metal Plate Connected Wood Truss Construction, a Truss Plate Institute publication (www.tpinst.org).

BCSI: Guide to Good Practice for Handling, Installing, Restraining & Bracing Metal Plate Connected Wood Trusses, a joint publication of the Truss Plate Institute (www.tpinst.org) and the Structural Building Components Association (www.sbcindustry.com).

Symbols and Nomenclature

- 5x7** Plate size; the first digit is the plate width (perp. to the slots) and the second digit is the plate length (parallel to the slots).
- 5x7-18** -18, -18S5, or -18S6 following the plate size indicates different 18 gauge plate types.
- || = \sphericalangle** These symbols following the plate size indicate the direction of the plate length (and tooth slots) for square and nearly square plates.
- 10-3-14** Dimensions are shown in feet-inches-sixteenths (for this example, the dimension is 10'-3 14/16").
-  Joints are numbered left to right, first along the top chord and then along the bottom chord. Mid-panel splice joint numbers are not shown on the drawing. Members are identified using their end joint numbers (e.g., TC 2-3).
-  When this symbol is shown, permanent lateral restraint is required. Lateral restraint may be applied to either edge of the member. See Note 3 under Handling, Installing, Restraint & Bracing for more information.
-  Bearing supports (wall, beam, etc.), locations at which the truss is required to have full bearing. Minimum required bearing width for the given reactions are reported on the TDD. Required bearing widths are based on the truss material and indicated PSI of the support material. The Building Designer is responsible for verifying that the capacity of the support material exceeds the indicated PSI, and for all other bearing design considerations.
-  Truss-to-Truss or Truss-to-Structural Element connection, which require a hanger or other structural connection (e.g., toe-nail) that has adequate capacity to resist the maximum reactions specified in the Reaction Summary. Structural connection type is not limited by type shown on TDD. Toe-nails may be used where hanger type shown where allowed by detail or other connection design information. Design of the Structural Element and the connection of the Truss to a Structural Element is by others.

Note: These symbols are for graphical interpretation only; they are not intended to give any indication of the geometry requirements of the actual item that is represented.

Materials and Fabrication

1. Design assumes truss is manufactured in accordance with the TDD and the quality criteria in ANSI/TPI 1 Chapter 3, unless more restrictive criteria are part of the contract specifications.
2. Unless specifically stated, lumber shall not exceed 19% moisture content at time of fabrication or in service.
3. Design is not applicable for use with fire retardant, preservative treated or green lumber unless specifically stated on the TDD.
4. Plate type, size, orientation and location indicated are based on the specified design parameters. Larger plate sizes may be substituted in accordance with ANSI/TPI, Section 3.6.3. Plates shall be embedded within ANSI/TPI 1 tolerances on both faces of the truss at each joint, unless noted otherwise.
5. Truss plates shall be centered on the joint unless otherwise specified.

DSB-89 Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses, a Truss Plate Institute publication (www.tpinst.org).

NDS: National Design Specification for Wood Construction published by American Forest & Paper Association and American Wood Council.

ESR-2762 Simpson Strong-Tie® AS Truss Plates are covered under ESR-2762 published by the International Code Council Evaluation Service (www.icc-es.org).

Customer: Valued Customer

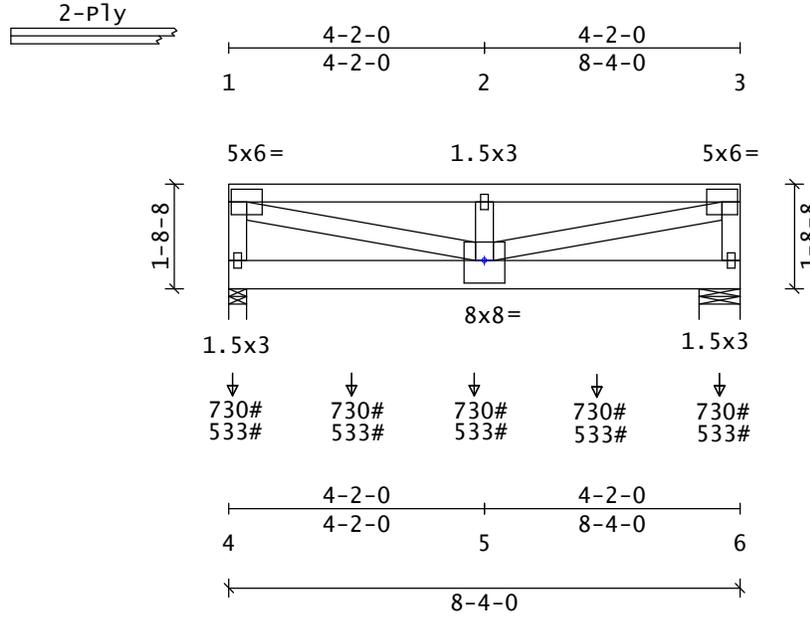
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Truss Mfr. Contact: Chris Wallington

TID: 258827

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Truss Weight = 109.1 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 2
 Repetitive Member Increase: No
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x6	SP (ALSC6-2013)	SS
Webs	2x4	SP (ALSC6-2013)	#1

Member Forces Summary

Max CSI in TC PANEL	1 - 2	0.27
Max CSI in BC PANEL	4 - 5	0.37
Max CSI in Web	1 - 5	0.37

...	Mem...	Ten	Comp	.CSI.
TC	OH- 1	0	0	0.00
	1- 2	1392	4594	0.27
	2- 3	1392	4594	0.27
	3-OH	0	0	0.00
BC	OH- 4	0	0	0.00
	4- 5	0	78	0.37
	5- 6	0	24	0.37
	6-OH	0	0	0.00
Web	1- 4	531	1662	0.07
	1- 5	4840	1462	0.37
	2- 5	619	285	0.03
	3- 5	4840	1462	0.37
	3- 6	531	1662	0.07

Reaction Summary

Jnt	--X-Loc	React	-Up-	--Width-	-Regd	-Mat	PSI
4	01-12	3598	1078	03-08	02-08	SPF	478
6	8-02-04	3382	1014	08-00	02-10	SPF	425

Max Horiz = -51 / +51 at Joint 4

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [4-02-00] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for load combinations and additional details.

Loads based on maximum and minimum reactions from tie-in spans.

Mbr	Max	Min	Location	Dir	Description
BC	730	-115	00-12	Vert	T02 @ 90 Deg
BC	533	-259	00-12	Vert	T01 @ -90 Deg
BC	730	-115	2-00-00	Vert	T02 @ 90 Deg
BC	533	-259	2-00-00	Vert	T01 @ -90 Deg
BC	730	-115	4-00-00	Vert	T02 @ 90 Deg
BC	533	-259	4-00-00	Vert	T01 @ -90 Deg
BC	730	-115	6-00-00	Vert	T02 @ 90 Deg
BC	533	-259	6-00-00	Vert	T01 @ -90 Deg
BC	730	-115	8-00-00	Vert	T02 @ 90 Deg
BC	533	-259	8-00-00	Vert	T01 @ -90 Deg

2-PLY TRUSS Fastener Spacing

Fasten each ply to the adjacent ply as follows (rows staggered):
 TC 2x4, 1-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 12.0" o.c.
 BC 2x6, 2-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 12.0" o.c.**
 WB 2x4, 1-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 9.0" o.c.
 ** Use additional fasteners of the same type (u.n.o.) within +/-12" of the location(s) indicated (except where approved hangers are used with fasteners that transfer the load to all plies):

BC:2-00-00, 2, BC:6-00-00, 2

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Less than 0.25/12 pitch requires adequate drainage to prevent ponding. Lumber and plating have been applied symmetrically.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.05)	5- 6
Vert DL	L/120	L/999(-0.05)	5- 6
Vert CR	L/180	L/999(-0.10)	5- 6
Horz LL	0.75in	(0.00) @Jt 6	
Horz CR	1.25in	(0.00) @Jt 6	

Bracing Data Summary

-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:

	--oc--	--From--	--To--	#Bays
TC	4-02-00	0	8-04-00	2
BC	8-04-00	0	8-04-00	1

Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)
 Jnt5(0, -00-06)



NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



Component Solutions
 Truss Studio V
 2024.3.2.1

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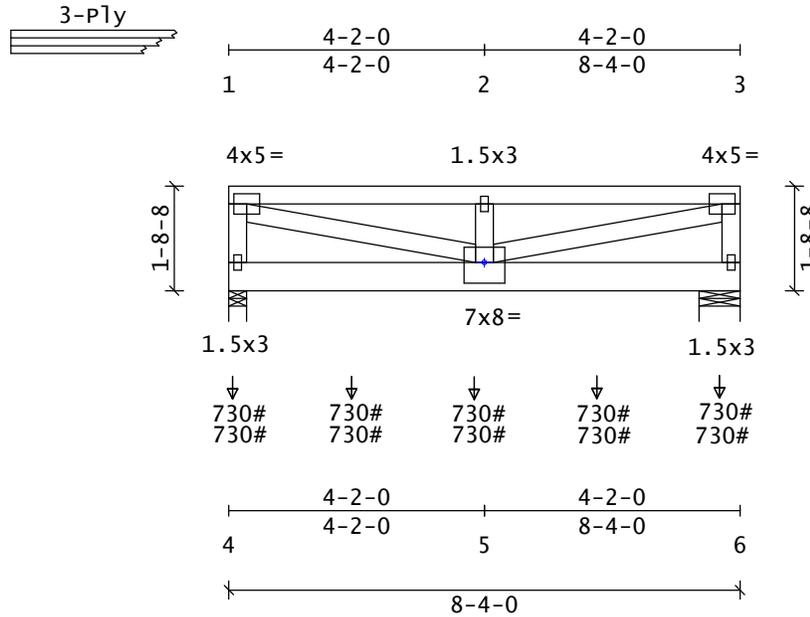
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Truss Mfr. Contact: Chris Wallington

TID: 258827

Date: 01 / 07 / 25

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Truss Weight = 156.8 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 3
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x6	SP (ALSC6-2013)	SS
Webs	2x4	SP (ALSC6-2013)	#1

Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Regd	-Mat	PSI
4	01-12	4108	1926	03-08	02-00	SPF	460
6	8-02-04	3858	1806	08-00	02-00	SPF	425

Max Horiz = -51 / +51 at Joint 4

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.04)	5- 6
Vert DL	L/120	L/999(-0.04)	5- 6
Vert CR	L/180	L/999(-0.07)	5- 6
Horz LL	0.75in	(0.00)	@Jt 6
Horz CR	1.25in	(0.00)	@Jt 6

Member Forces Summary

...	Mem...	Ten	Comp	.CSI.
TC	OH- 1	0	0	0.00
	1- 2	2448	5228	0.13
	2- 3	2448	5228	0.13
	3-OH	0	0	0.00
BC	OH- 4	0	0	0.00
	4- 5	0	78	0.25
	5- 6	0	24	0.25
	6-OH	0	0	0.00
Web	1- 4	890	1877	0.06
	1- 5	5508	2574	0.28
	2- 5	622	279	0.02
	3- 5	5508	2574	0.28
	3- 6	890	1877	0.06

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [4-02-00] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for load combinations and additional details.

Loads based on maximum and minimum reactions from tie-in spans.

Mbr	Max	Min	Location	Dir	Description
Transfer loads:					
BC	730	-351	00-12	Vert	T02 @ -90 Deg
BC	730	-351	00-12	Vert	T02 @ 90 Deg
BC	730	-351	2-00-00	Vert	T02 @ -90 Deg
BC	730	-351	2-00-00	Vert	T02 @ 90 Deg
BC	730	-351	4-00-00	Vert	T02 @ -90 Deg
BC	730	-351	4-00-00	Vert	T02 @ 90 Deg
BC	730	-351	6-00-00	Vert	T02 @ -90 Deg
BC	730	-351	6-00-00	Vert	T02 @ 90 Deg
BC	730	-351	8-00-00	Vert	T02 @ -90 Deg
BC	730	-351	8-00-00	Vert	T02 @ 90 Deg

3-PLY TRUSS Fastener Spacing

Fasten each ply to the adjacent ply as follows(rows staggered):
 TC 2x4, 1-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 12.0" o.c.
 BC 2x6, 2-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 12.0" o.c.**
 WB 2x4, 1-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 9.0" o.c.
 ** Use additional fasteners of the same type (u.n.o.) within +/-12" of the location(s) indicated (except where approved hangers are used with fasteners that transfer the load to all plies):

BC:2-00-00, 5, BC:6-00-00, 5

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
 Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
 Less than 0.25/12 pitch requires adequate drainage to prevent ponding.
 Lumber and plating have been applied symmetrically.



NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



Component Solutions
 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

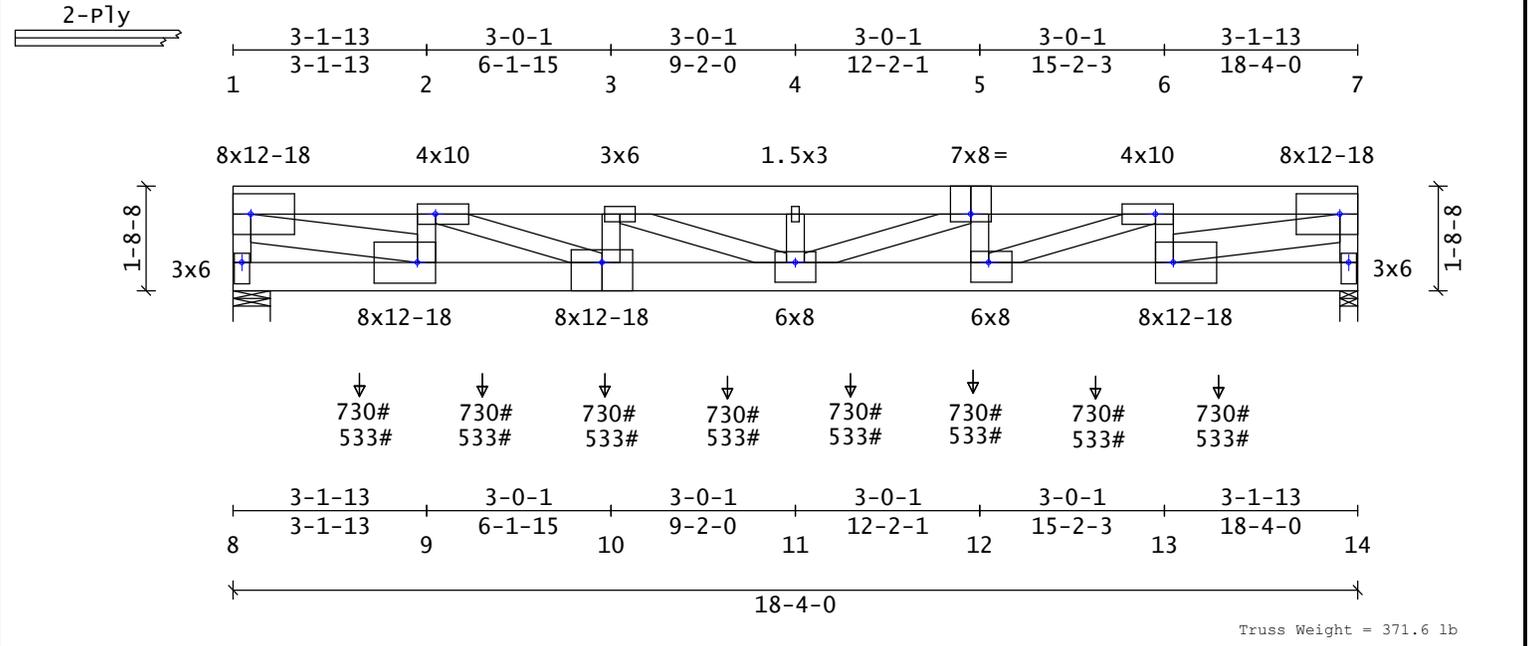
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Truss Mfr. Contact: Chris Wallington



Truss Weight = 371.6 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 2
 Repetitive Member Increase: No
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x6	SP (ALSC6-2013)	SS
BC	2x6	SP (ALSC6-2013)	SS
Webs	2x4	SP (ALSC6-2013)	#1
	2x6	SP (ALSC6-2013)	SS 1-9

Reaction Summary

Jnt	--X-Loc	React	-Up-	--Width-	-Reqd	-Mat	PSI
8	01-12	5839	1749	07-04	04-09	SPF	425
14	18-02-04	5731	1717	03-08	04-00**	SPF	478

Max Horiz = -42 / +42 at Joint 8
 (**) indicates Req'd Width > actual Width; enhancement may be required.
 Building Designer to provide adequate bearing size or enhancement.
 If applicable, bearing enhancement detail TD-BRG-0002A may be used.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/703(-0.31)	11-12
Vert DL	L/120	L/708(-0.31)	11-12
Vert CR	L/180	L/353(-0.61)	11-12
Horz LL	0.75in	(0.04)	@Jt14
Horz CR	1.25in	(0.08)	@Jt14

Member Forces Summary

Max CSI in TC PANEL	3	4	0.99
Max CSI in BC PANEL	10	11	0.95
Max CSI in Web	12	6	0.58

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [9-02-00] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for load combinations and additional details.

Loads based on maximum and minimum reactions from tie-in spans.

Mbr	Max	Min	Location	Dir	Description
BC	730	-115	2-00-12	Vert	T02 @ 90 Deg
BC	533	-259	2-00-12	Vert	T01 @ -90 Deg
BC	730	-115	4-00-12	Vert	T02 @ 90 Deg
BC	533	-259	4-00-12	Vert	T01 @ -90 Deg
BC	730	-115	6-00-12	Vert	T02 @ 90 Deg
BC	533	-259	6-00-12	Vert	T01 @ -90 Deg
BC	730	-115	8-00-12	Vert	T02 @ 90 Deg
BC	533	-259	8-00-12	Vert	T01 @ -90 Deg
BC	730	-115	10-00-12	Vert	T02 @ 90 Deg
BC	533	-259	10-00-12	Vert	T01 @ -90 Deg
BC	730	-115	12-00-12	Vert	T02 @ 90 Deg
BC	533	-259	12-00-12	Vert	T01 @ -90 Deg
BC	730	-115	14-00-12	Vert	T02 @ 90 Deg
BC	533	-259	14-00-12	Vert	T01 @ -90 Deg
BC	730	-115	16-00-12	Vert	T02 @ 90 Deg
BC	533	-259	16-00-12	Vert	T01 @ -90 Deg

2-PLY TRUSS Fastener Spacing

Fasten each ply to the adjacent ply as follows(rows staggered):

- TC 2x6, 2-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 12.0" o.c.
- BC 2x6, 2-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 12.0" o.c.**
- WB 2x4, 1-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 9.0" o.c.
- WB 2x6, 2-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 9.0" o.c.

** Use additional fasteners of the same type (u.n.o.) within +/-12" of the location(s) indicated (except where approved hangers are used with fasteners that transfer the load to all plies):

- BC:2-00-12, 2, BC:4-00-12, 2, BC:8-00-12, 2
- BC:10-00-12, 2, BC:14-00-12, 2, BC:16-00-12, 2

Bracing Data Summary

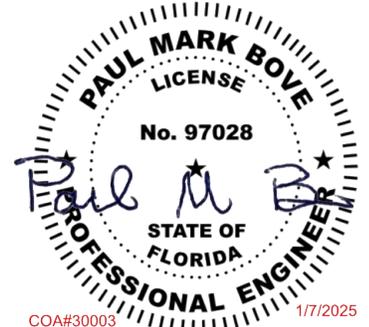
-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:

TC	3-11-00	--From--	0	18-04-00	5
BC	9-02-00		0	18-04-00	2

Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)
 Jnt1(02-08,0), Jnt2(01-08,0),
 Jnt5(0,02-00), Jnt6(-01-08,0),
 Jnt7(-02-08,0), Jnt8(0,-01-02),
 Jnt9(-02-08,0), Jnt10(0,-01-08),
 Jnt11(0,-00-14), Jnt12(00-08,-00-13),
 Jnt13(02-08,0), Jnt14(0,-01-02)



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Component Solutions
 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

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Truss Mfr. Contact: Chris Wallington

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
Less than 0.25/12 pitch requires adequate drainage to prevent ponding.
This truss is not symmetric - proper orientation is critical.



COA#30003

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Component Solutions
Truss Studio V
2024.3.2.1

Customer: Valued Customer

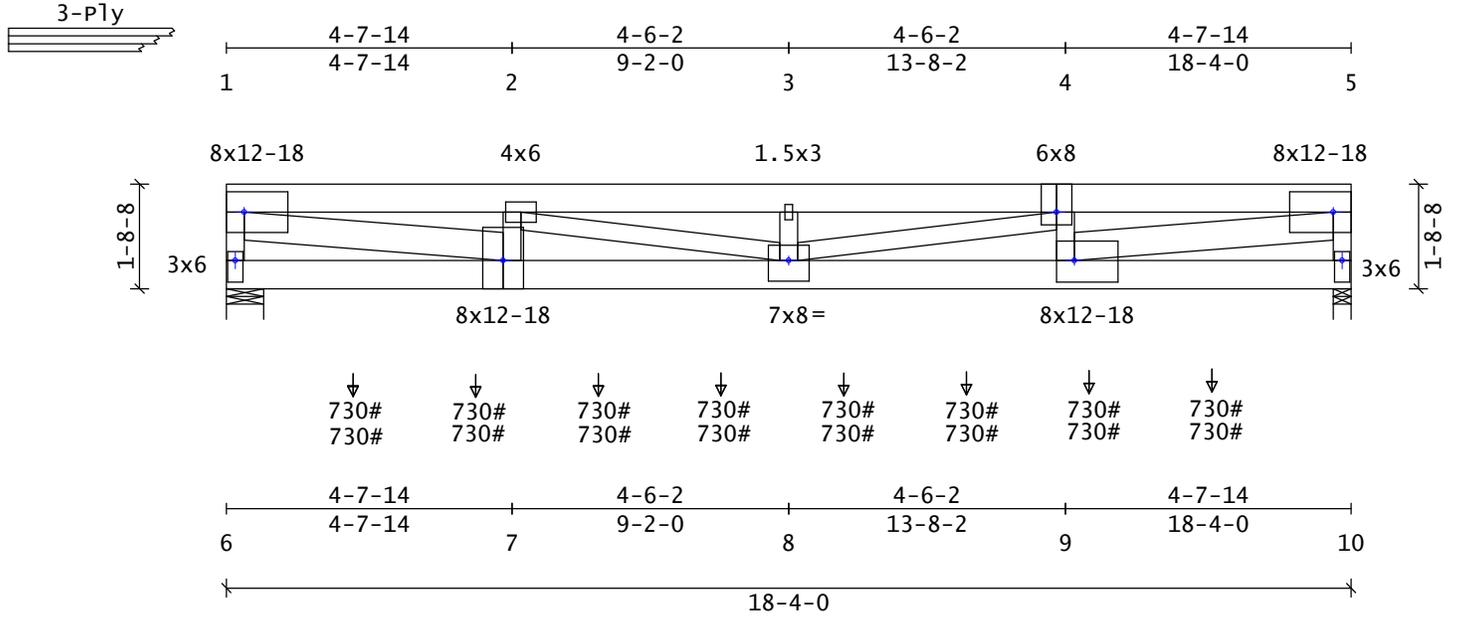
SID: 0003323678

TID: 258827

Date: 01 / 07 / 25

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Truss Mfr. Contact: Chris Wallington



Truss Weight = 503.0 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 3
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary
 TC 2x6 SP (ALSC6-2013) SS
 BC 2x6 SP (ALSC6-2013) SS
 Webs 2x4 SP (ALSC6-2013) #1
 2x6 SP (ALSC6-2013) SS 1-7
 9-5

Reaction Summary
 -----Reaction Summary(Lbs)-----
 Jnt --X-Loc- React -Up- --Width- -Regd -Mat PSI
 6 01-12 6640 3077 07-04 03-08 SPF 425
 10 18-02-04 6505 3012 03-08 03-02 SPF 460
 Max Horiz = -42 / +42 at Joint 6

Deflection Summary
 TrussSpan Limit Actual(in) Location
 Vert LL L/240 L/796(-0.27) 8- 9
 Vert DL L/120 L/795(-0.27) 8- 9
 Vert CR L/180 L/397(-0.54) 8- 9
 Horiz LL 0.75in (0.02) @Jt10
 Horiz CR 1.25in (0.05) @Jt10

Member Forces Summary
 Max CSI in TC PANEL 2 - 3 0.46
 Max CSI in BC PANEL 8 - 9 0.64
 Max CSI in Web 9 - 5 0.45

...	Mem...	Ten	Comp	.CSI.
TC	OH- 1	0	0	0.00
	1- 2	8992	19314	0.25
	2- 3	11820	25393	0.46
	3- 4	11820	25393	0.46
	4- 5	9113	19577	0.27
	5-OH	0	0	0.00
BC	OH- 6	0	0	0.00
	6- 7	0	54	0.25
	7- 8	19533	9108	0.63
	8- 9	19791	9217	0.64
	9-10	0	19	0.27
	10-OH	0	0	0.00
Web	1- 6	2715	5805	0.18
	1- 7	20091	9345	0.44
	2- 7	917	1885	0.06
	2- 8	6356	2828	0.32
	3- 8	365	191	0.01
	4- 8	6091	2713	0.31
	4- 9	901	1850	0.05
	5- 9	20315	9446	0.45
	5-10	2671	5710	0.17

Loads Summary
 This truss has been designed for the effects of an unbalanced top chord live load occurring at [9-02-00] using a 1.00 Full and 0.00 Reduced load factor.
 See Loadcase Report for load combinations and additional details.
 Loads based on maximum and minimum reactions from tie-in spans.

Mbr	Max	Min	Location	Dir	Description
Transfer loads:					
BC	730	-351	2-00-12	Vert	T02 @ -90 Deg
BC	730	-351	2-00-12	Vert	T02 @ 90 Deg
BC	730	-351	4-00-12	Vert	T02 @ -90 Deg
BC	730	-351	4-00-12	Vert	T02 @ 90 Deg
BC	730	-351	6-00-12	Vert	T02 @ -90 Deg
BC	730	-351	6-00-12	Vert	T02 @ 90 Deg
BC	730	-351	8-00-12	Vert	T02 @ -90 Deg
BC	730	-351	8-00-12	Vert	T02 @ 90 Deg
BC	730	-351	10-00-12	Vert	T02 @ -90 Deg
BC	730	-351	10-00-12	Vert	T02 @ 90 Deg
BC	730	-351	12-00-12	Vert	T02 @ -90 Deg
BC	730	-351	12-00-12	Vert	T02 @ 90 Deg
BC	730	-351	14-00-12	Vert	T02 @ -90 Deg
BC	730	-351	14-00-12	Vert	T02 @ 90 Deg
BC	730	-351	16-00-12	Vert	T02 @ -90 Deg
BC	730	-351	16-00-12	Vert	T02 @ 90 Deg

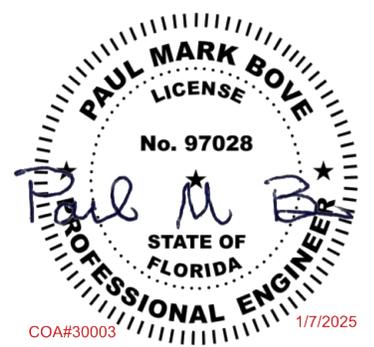
Bracing Data Summary
 -----Bracing Data-----
 Chords; Sheathing required or bracing indicated:
 -----Purlins-----
 ---oc--- --From--- --To--- #Bays
 TC 4-07-00 0 18-04-00 4
 BC 9-02-00 0 18-04-00 2
 Web Bracing -- None

Plate offsets (X, Y):
 (None unless indicated below)
 Jnt1(02-08,0), Jnt4(0,01-08),
 Jnt5(-02-08,0), Jnt6(0,-01-04),
 Jnt7(0,00-08), Jnt8(0,-00-08),
 Jnt9(02-08,-00-03), Jnt10(0,-01-03)

3-PLY TRUSS Fastener Spacing
 Fasten each ply to the adjacent ply as follows (rows staggered):
 TC 2x6, 2-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 12.0" o.c.
 BC 2x6, 2-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 12.0" o.c.**
 WB 2x4, 1-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 9.0" o.c.
 WB 2x6, 2-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 9.0" o.c.
 ** Use additional fasteners of the same type (u.n.o.) within +/-12" of the location(s) indicated (except where approved hangers are used with fasteners that transfer the load to all plies):

BC:2-00-12, 5, BC:4-00-12, 5, BC:6-00-12, 5
 BC:8-00-12, 5, BC:10-00-12, 5, BC:12-00-12, 5
 BC:14-00-12, 5, BC:16-00-12, 5

Notes
 Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
 Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.



NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.

SIMPSON Strong-Tie Component Solutions
 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

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TID: 258827

Date: 01 / 07 / 25

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Truss Mfr. Contact: Chris Wallington

Notes

Less than 0.25/12 pitch requires adequate drainage to prevent ponding.
This truss is not symmetric - proper orientation is critical.



COA#30003

1/7/2025

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Component Solutions
Truss Studio V
2024.3.2.1

Customer: Valued Customer

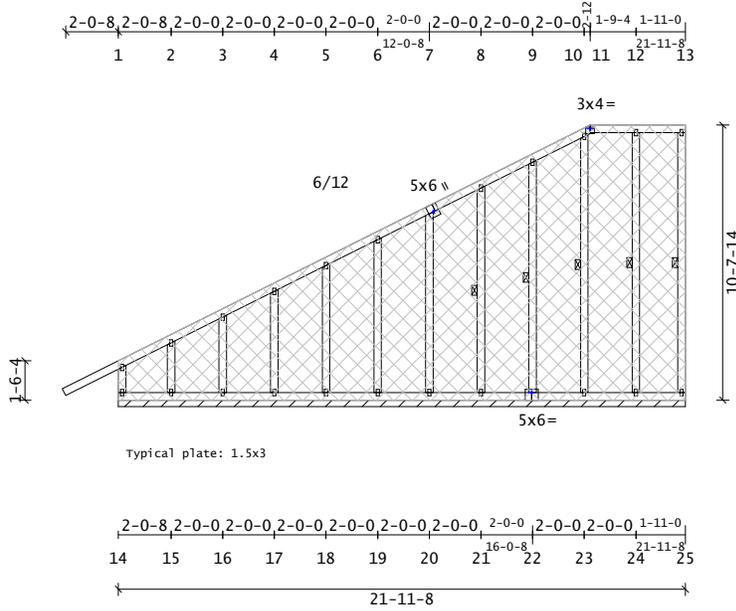
SID: 0003323680

TID: 258827

Date: 01 / 06 / 25

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Truss Mfr. Contact: Chris Wallington



Truss Weight = 181.1 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1

Member Forces Summary

Max CSI in TC PANEL	12 - 13	0.66
Max CSI in BC PANEL	14 - 15	0.88
Max CSI in Web	14 - 1	0.92

...	Mem...	Ten	Comp	.CSI.
TC	1-7	124	0	0.63
	7-11	264	295	0.07
	11-13	276	224	0.66
BC	14-22	882	237	0.88
	22-25	272	239	0.65
Web	1-14	166	452	0.92
	2-15	458	194	0.05
	3-16	136	228	0.04
	4-17	156	221	0.06
	5-18	142	221	0.09
	6-19	178	225	0.13
	7-20	151	220	0.17
	8-21	169	211	0.06
	9-22	153	224	0.08
	10-23	200	205	0.08
	12-24	365	218	0.09
	13-25	355	156	0.76

Reaction Summary

Jnt	--X-Loc	React	-Up-	--Width-	-Reqd	-Mat	PSI
14	01-12	524	227	21-11-08			
15	2-00-08	229	446	21-11-08			
Max Horiz = -314 / +603 at Joint 19							
Reactions not shown: down < 400 and up < 150							
Jnt	Jnt	React	-Up-	--Width-			
14	25	52	0	21-11-08	(reduced)		

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [20-01-06] using a 1.00 Full and 0.00 Reduced load factor.
 See Loadcase Report for load combinations and additional details.

Notes

If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
 The maximum rake overhang length is 12.0".
 Gable requires 7/16" OSB sheathing on front from 0-00-00 to 21-11-08; connection details to be provided by the Building Designer.
 Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.
 Less than 0.25/12 pitch requires adequate drainage to prevent ponding.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(0.00)	14-15
Vert DL	L/120	L/999(0.00)	14-15
Vert CR	L/180	L/999(0.00)	14-15
Horz LL	0.75in	(0.01)	@Jt14
Horz CR	1.25in	(0.01)	@Jt14
Ohng CR	2L/180	2L/801(0.07)	1- 1

Bracing Data Summary

-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:
 -----Purlins-----

TC	--oc--	--From--	--To--	#Bays
TC	5-11-00	-2-02-01	19-02-13	5
TC	2-00-00	19-02-13	20-11-15	1
TC	11-00	20-11-15	21-11-08	2
BC	7-03-00	0	21-11-08	4

 ----- Web Bracing ----- CLR -----
 Single: 21- 8 22- 9 23-10 24-12 25-13
 Continuous Restraint Bracing Req'd
 See BCSI-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)
 Jnt7(-00-04,00-07), Jnt11(0,-00-12), Jnt22(0,-01-00)



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Component Solutions
 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

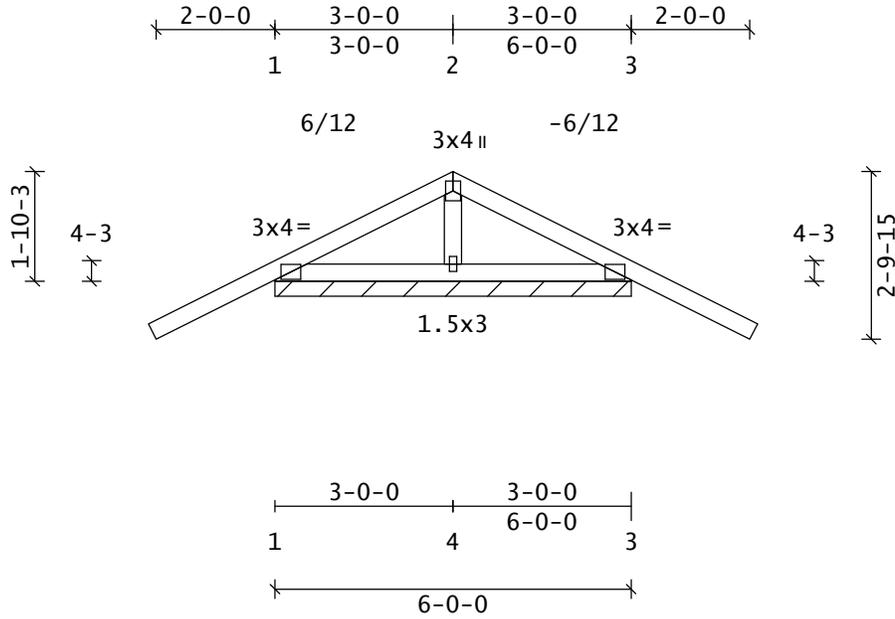
SID: 0003323681

TID: 258827

Date: 01 / 06 / 25

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Truss Mfr. Contact: Chris Wallington



Truss Weight = 27.8 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1

Member Forces Summary

Max CSI in TC PANEL	1 - 1	0.53
Max CSI in BC PANEL	1 - 4	0.28
Max CSI in Web	4 - 2	0.03

...	Mem...	Ten	Comp	.CSI.
TC	1-2	117	0	0.53
	2-3	117	0	0.50
BC	1-3	398	182	0.28
Web	2-4	311	207	0.03

Reaction Summary

Max Horiz = -48 / +48 at Joint 4
 Reactions not shown: down < 400 and up < 150
 ---- Reaction Summary (plf) ----
 Jnt-Jnt React -Up- --Width-
 1- 3 125 49 6-00-00

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [3-00-00] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for load combinations and additional details.

Notes

If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
 The maximum rake overhang length is 12.0".
 Plates designed for Cg at 0.80 and Rotational Tolerance of 10.0 degrees.
 Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
 Lumber and plating have been applied symmetrically.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(0.00)	4- 3
Vert DL	L/120	L/999(0.00)	4- 3
Vert CR	L/180	L/999(0.00)	4- 3
Horz LL	0.75in	(0.01)	@Jt 3
Horz CR	1.25in	(0.01)	@Jt 3
Ohng CR	2L/180	2L/676(0.08)	1- 1
Ohng CR	2L/180	2L/676(0.08)	3- 3

Bracing Data Summary

-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:
 -----Purlins-----

	--oc--	--From--	--To--	#Bays
TC	5-08-00	-2-01-09	8-01-09	3
BC	6-00-00	0	6-00-00	1

 Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)



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Component Solutions
 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

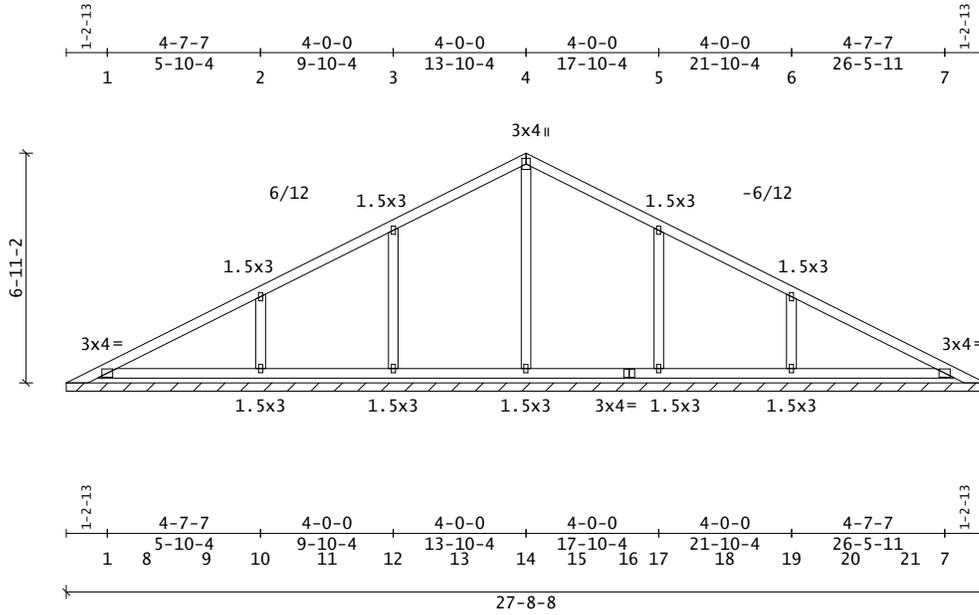
SID: 0003323682

Truss Mfr. Contact: Chris Wallington

TID: 258827

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Truss Weight = 108.4 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 1.0 Lum 1.25 1.60 N/A
 Total 31.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums (Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 1.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1

Member Forces Summary

Max CSI in TC PANEL	1 - 2	0.24
Max CSI in BC PANEL	1 - 8	0.17
Max CSI in Web	14 - 4	0.16

...	Mem...	Ten	Comp	.CSI.
TC	1-2	86	130	0.24
	2-3	103	70	0.19
	3-4	253	95	0.24
	4-5	253	95	0.24
	5-6	103	70	0.19
	6-7	86	130	0.23
BC	1-8	180	74	0.17
	7-21	180	74	0.17
	8-9	180	74	0.09
	9-10	180	74	0.02
	10-11	180	74	0.02
	11-12	180	74	0.02
	12-13	180	74	0.03
	13-14	180	74	0.03
	14-15	180	74	0.02
	15-16	180	74	0.02
	16-17	180	74	0.02
	17-18	180	74	0.02
	18-19	180	74	0.02
	19-20	180	74	0.02
	20-21	180	74	0.09
Web	2-10	349	315	0.04
	3-12	354	306	0.09
	4-14	19	255	0.16
	5-17	354	306	0.09
	6-19	349	315	0.04

Reaction Summary

Jnt	--X-Loc	React	-Up-	--Width-	-Reqd	-Mat	PSI
10	5-10-04	321	211	27-08-08			
12	9-10-04	311	188	27-08-08			
17	17-10-04	310	189	27-08-08			
19	21-10-04	321	211	27-08-08			

Max Horiz = -223 / +223 at Joint 14
 Reactions not shown: down < 400 and up < 150
 ---- Reaction Summary (plf) ----
 Jnt-Jnt React -Up- --Width-
 1- 7 16 5 27-08-08 (reduced)

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [13-10-04] using a 1.00 Full and 0.00 Reduced load factor.
 See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
 Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
 This truss is not symmetric - proper orientation is critical.

Deflection Summary

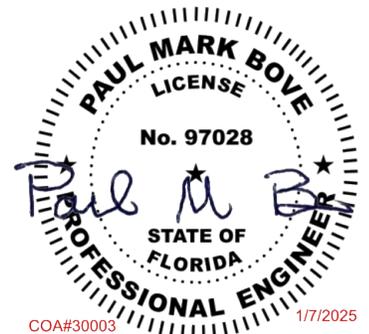
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.00)	1- 8
Vert DL	L/120	L/999(-0.00)	21- 7
Vert CR	L/180	L/999(-0.01)	21- 7
Horz LL	0.75in	(0.01)	@Jt 1
Horz CR	1.25in	(0.01)	@Jt 1

Bracing Data Summary

-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:
 -----Purlins-----
 --oc-- --From-- --To-- #Bays
 TC 5-01-00 0 27-08-08 7
 BC 8-07-00 11-09 26-08-15 4
 Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)



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Component Solutions
 Truss Studio V
 2024.3.2.1

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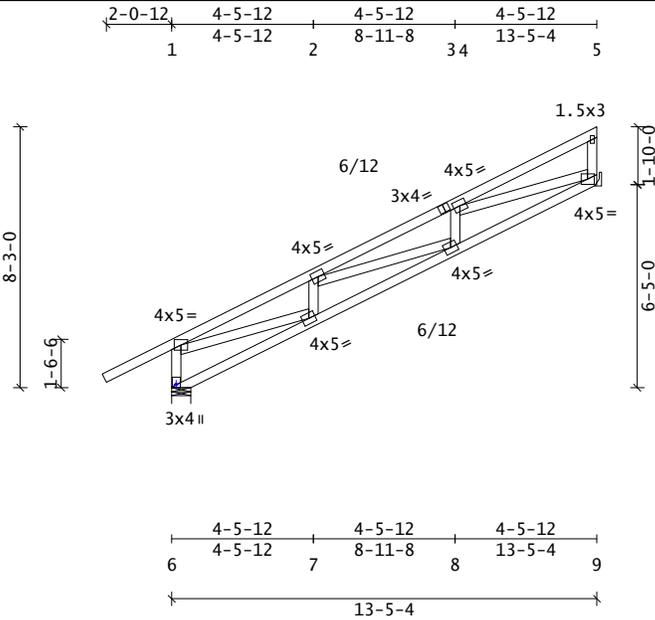
SID: 0003323683

TID: 258827

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Truss Mfr. Contact: Chris Wallington



Truss Weight = 79.5 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1

Member Forces Summary

Max CSI in TC PANEL	1 - 2	1.00
Max CSI in BC PANEL	7 - 8	0.98
Max CSI in Web	4 - 9	0.34

...	Mem...	Ten	Comp	.CSI.
TC	OH- 1	125	0	0.64
	1- 2	1106	1230	1.00
	2- 3	1144	1261	0.99
	3- 4	1146	1150	0.91
	4- 5	84	121	0.32
	5-OH	0	7	0.00
BC	OH- 6	5	0	0.00
	6- 7	232	783	0.26
	7- 8	1192	1646	0.98
	8- 9	1220	1488	0.79
	9-OH	0	5	0.00
Web	1- 6	762	649	0.10
	1- 7	1061	777	0.16
	2- 7	320	271	0.04
	2- 8	235	181	0.06
	4- 8	188	0	0.03
	4- 9	1317	1088	0.34
	5- 9	184	205	0.04

Reaction Summary

Jnt	--X-Loc	React	-Up-	--Width-	-Reqd	-Mat	PSI
6	01-12	691	156	07-04	01-08	SPF	425
9	13-03-08	532	259	01-11	HGR	SPF	565

Max Horiz = -56 / +356 at Joint 6

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [13-05-04] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Deflection Summary

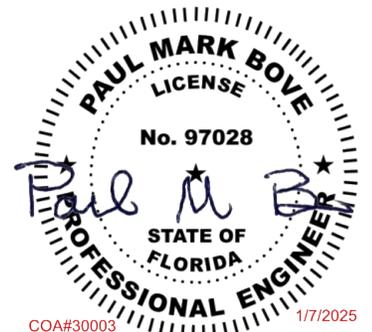
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.04)	7- 8
Vert DL	L/120	L/999(-0.04)	7- 8
Vert CR	L/180	L/999(-0.09)	7- 8
Horz LL	0.75in	(0.02)	@Jt 9
Horz CR	1.25in	(0.03)	@Jt 9
Ohng CR	2L/180	2L/676(0.08)	1- 1

Bracing Data Summary

-----Bracing Data-----			
Chords; Sheathing required or bracing indicated:			
-----Purlins-----			
--oc---	--From--	--To---	#Bays
TC	5-08-00	-2-02-05	13-05-04 4
BC	7-06-00	0	13-05-04 3
Web Bracing -- None			

Plate offsets (X, Y):

(None unless indicated below)
 Jnt6(0,00-13)



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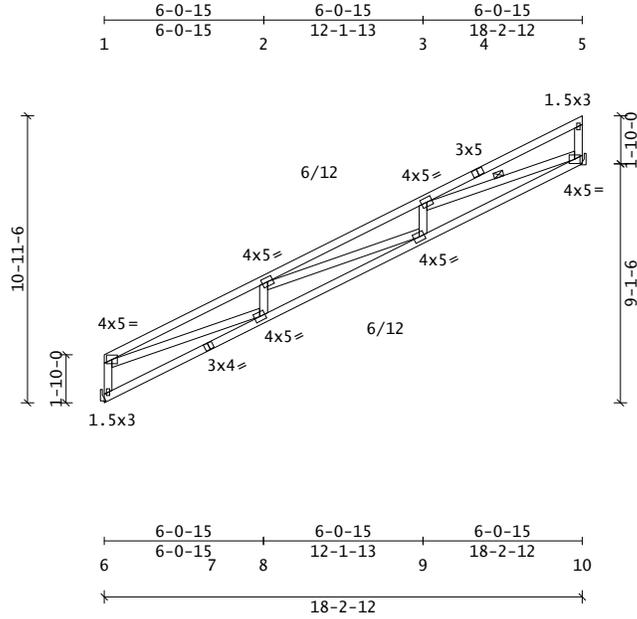
SID: 0003323684

Truss Mfr. Contact: Chris Wallington

TID: 258827

Date: 01 / 07 / 25

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Truss Weight = 99.1 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums (Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1

Member Forces Summary

Max CSI in TC PANEL	1 - 2	0.97
Max CSI in BC PANEL	8 - 9	1.00
Max CSI in Web	1 - 8	0.49

...	Mem...	Ten	Comp	.CSI.
TC	OH- 1	7	0	0.00
	1- 2	1695	2333	0.97
	2- 3	1717	2348	0.84
	3- 4	21	134	0.39
	4- 5	99	124	0.53
	5-OH	0	7	0.00
BC	OH- 6	5	0	0.00
	6- 7	246	761	0.42
	7- 8	265	744	0.29
	8- 9	2294	2317	1.00
	9-10	2291	2077	0.80
	10-OH	0	5	0.00
Web	1- 6	552	672	0.08
	1- 8	2088	1465	0.49
	2- 8	433	389	0.05
	2- 9	297	297	0.19
	3- 9	258	0	0.04
	3-10	1879	2088	0.37
	5-10	199	238	0.04

Reaction Summary

Jnt	--X-Loc	React	-Up-	--Width-	-Reqd	-Mat	PSI
6	01-12	729	114	01-11	HGR	SPF	565
10	18-01-00	729	350	01-11	HGR	SPF	565

Max Horiz = -85 / +415 at Joint 6

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [18-02-12] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.

Deflection Summary

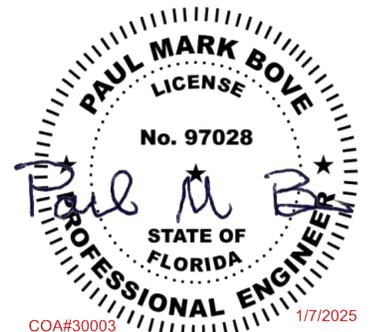
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.15)	8- 9
Vert DL	L/120	L/999(-0.16)	8- 9
Vert CR	L/180	L/686(-0.31)	8- 9
Horz LL	0.75in	(0.03)	@Jt10
Horz CR	1.25in	(0.05)	@Jt10

Bracing Data Summary

-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:
 -----Purlins-----
 ---oc--- --From--- --To--- #Bays
 TC 3-07-00 0 18-02-12 6
 BC 5-06-00 0 18-02-12 4
 ----- Web Bracing -- CLR -----
 Single: 3-10
 Continuous Restraint Bracing Req'd
 See BC31-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)



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Customer: Valued Customer

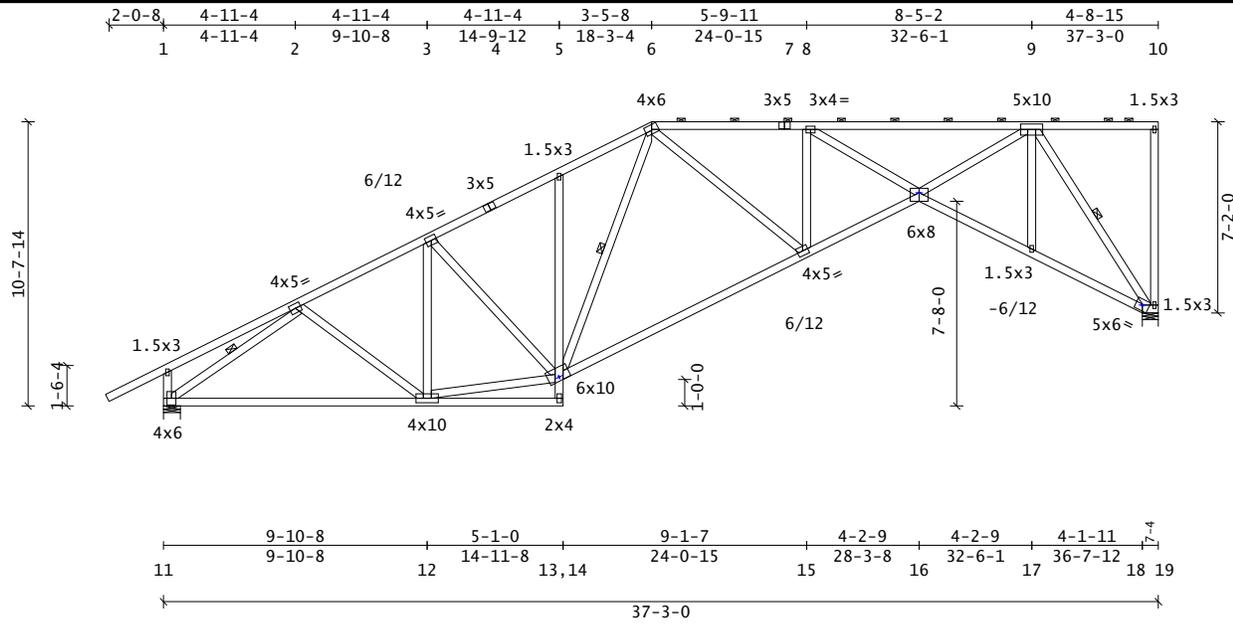
SID: 0003323685

Truss Mfr. Contact: Chris Wallington

TID: 258827

Date: 01 / 07 / 25

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Truss Weight = 262.3 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1

Member Forces Summary

Max CSI in TC PANEL	6 - 7	0.97
Max CSI in BC PANEL	15 - 16	0.96
Max CSI in Web	9 - 18	0.61

...	Mem...	Ten	Comp	.CSI.
TC	OH-1	124	0	0.61
	1-2	218	76	0.60
	2-3	1344	2014	0.60
	3-4	1392	1955	0.58
	4-5	1404	1893	0.53
	5-6	1588	1961	0.57
	6-7	2125	2629	0.97
	7-8	2125	2629	0.67
	8-9	2903	3524	0.93
	9-10	156	140	0.50
	10-OH	0	0	0.00
BC	OH-11	0	0	0.00
	11-12	1640	1719	0.89
	12-13	26	24	0.67
	14-15	1975	1823	0.86
	15-16	2946	2585	0.96
	16-17	1376	1244	0.40
	17-18	1342	1198	0.55
	18-19	156	140	0.10
	19-OH	0	0	0.00
Web	1-11	433	417	0.05
	2-11	1185	2064	0.35
	2-12	219	39	0.03
	3-12	330	223	0.13
	3-14	182	191	0.15
	5-14	411	329	0.32
	6-14	120	349	0.11
	6-15	1155	861	0.35
	8-15	983	998	0.35
	8-16	1053	914	0.17
	9-16	2714	2316	0.41
	9-17	198	4	0.03
	9-18	1877	2247	0.61
	10-19	93	200	0.46
	12-14	1748	1622	0.26
	13-14	85	22	0.03

Reaction Summary

Jnt	--X-Loc	React	-Up-	--Width-	-Regd	-Mat	PSI
11	01-12	1622	552	07-08	02-09	SPF	425
19	36-10-01	1499	507	07-04	02-06	SPF	425

Max Horiz = -212 / +519 at Joint 11
 Max Horiz = -212 / +519 at Joint 18

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [27-09-02] using a 1.00 Full and 0.00 Reduced load factor.
 See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
 Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
 Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.
 Less than 0.25/12 pitch requires adequate drainage to prevent ponding.

Deflection Summary

TrussSpan	Limit	Actual (in)	Location
Vert LL	L/240	L/999(-0.35)	14-15
Vert DL	L/120	L/988(-0.45)	14-15
Vert CR	L/180	L/556(-0.79)	14-15
Horz LL	0.75in	(0.19)	@Jt19
Horz CR	1.25in	(0.38)	@Jt19
Ohng CR	2L/180	2L/801(0.07)	1- 1

Bracing Data Summary

-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:
 -----Purlins-----

--oc--	--From--	--To--	#Bays
TC	3-09-00	-2-02-01	19-02-13 7
TC	2-00-00	19-02-13	36-03-07 9
TC	11-00	36-03-07	37-03-00 2
BC	5-10-00	0	37-03-00 7

 ----- Web Bracing ----- CLR -----
 Single: 11- 2 14- 6 9-18
 Continuous Restraint Bracing Req'd
 See BCSI-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)
 Jnt14(-00-09,01-02), Jnt16(0,-00-15),
 Jnt18(00-01,00-01)



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 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

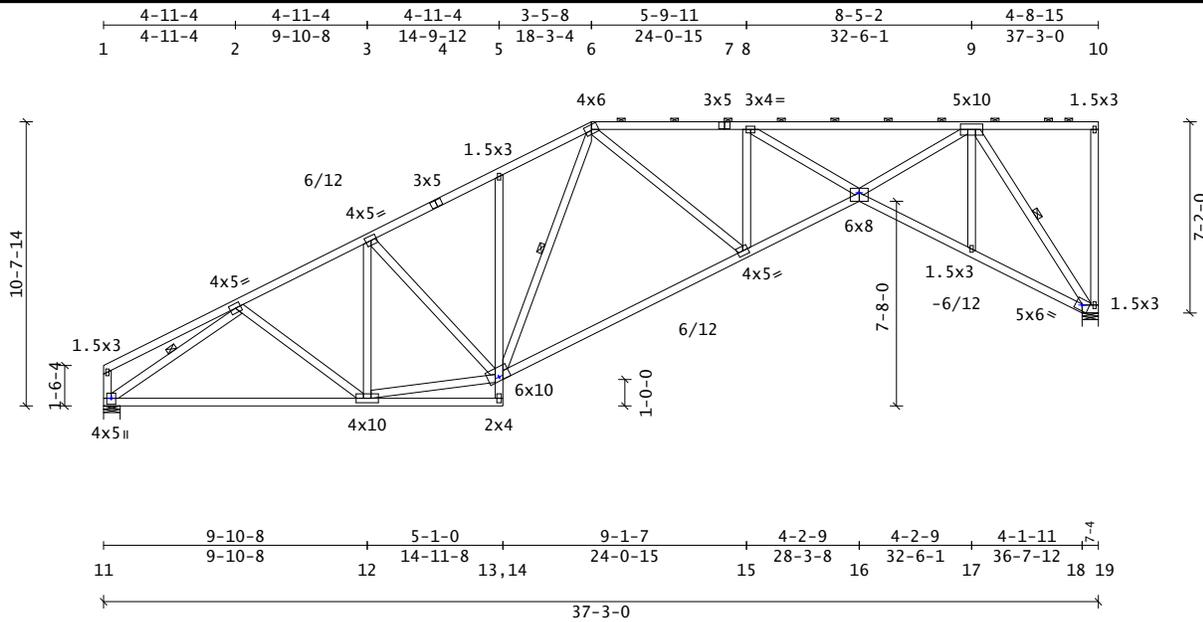
SID: 0003323686

Truss Mfr. Contact: Chris Wallington

TID: 258827

Date: 01 / 07 / 25

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Truss Weight = 258.8 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1

Reaction Summary

Jnt	--X-Loc	React	-Up-	--Width-	-Reqd	-Mat	PSI
11	01-12	1479	474	07-08	02-05	SPF	425
19	36-10-01	1500	507	07-04	02-06	SPF	425

Max Horiz = -236 / +492 at Joint 11
 Max Horiz = -236 / +492 at Joint 18

Deflection Summary

TrussSpan	Limit	Actual (in)	Location
Vert LL	L/240	L/999(-0.35)	14-15
Vert DL	L/120	L/986(-0.45)	14-15
Vert CR	L/180	L/555(-0.79)	14-15
Horz LL	0.75in	(0.19)	@Jt19
Horz CR	1.25in	(0.38)	@Jt19

Member Forces Summary

Max CSI in TC PANEL	6 - 7	0.98
Max CSI in BC PANEL	15 - 16	0.96
Max CSI in Web	9 - 18	0.61

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [27-09-02] using a 1.00 Full and 0.00 Reduced load factor.
 See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
 Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
 Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.
 Less than 0.25/12 pitch requires adequate drainage to prevent ponding.

Bracing Data Summary

-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:
 -----Purlins-----

TC	--oc--	--From--	--To--	#Bays
TC	3-09-00	0	19-02-13	6
TC	2-00-00	19-02-13	36-03-07	9
TC	11-00	36-03-07	37-03-00	2
BC	5-10-00	0	37-03-00	7

----- Web Bracing -- CLR -----
 Single: 11- 2 14- 6 9-18
 Continuous Restraint Bracing Req'd
 See BCSI-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)
 Jnt11(0,-00-02), Jnt14(-00-09,01-02),
 Jnt16(0,-00-15), Jnt18(00-01,00-01)

...	Mem...	Ten	Comp	.CSI.
TC	OH-1	7	0	0.00
	1- 2	112	89	0.38
	2- 3	1352	2024	0.63
	3- 4	1407	1961	0.57
	4- 5	1418	1891	0.53
	5- 6	1600	1967	0.57
	6- 7	2149	2633	0.98
	7- 8	2149	2633	0.67
	8- 9	2936	3530	0.93
	9-10	157	141	0.50
	10-OH	0	0	0.00
BC	OH-11	0	0	0.00
	11-12	1659	1752	0.89
	12-13	26	24	0.67
	14-15	1979	1840	0.86
	15-16	2951	2614	0.96
	16-17	1378	1259	0.40
	17-18	1344	1212	0.55
	18-19	157	141	0.10
	19-OH	0	0	0.00
Web	1-11	160	216	0.03
	2-11	1318	2088	0.35
	2-12	203	55	0.03
	3-12	318	202	0.11
	3-14	173	193	0.15
	5-14	415	331	0.32
	6-14	122	318	0.11
	6-15	1156	874	0.35
	8-15	996	999	0.35
	8-16	1054	925	0.17
	9-16	2719	2342	0.41
	9-17	198	0	0.03
	9-18	1900	2250	0.61
	10-19	94	200	0.47
	12-14	1755	1637	0.26
	13-14	85	22	0.03



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Component Solutions
 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

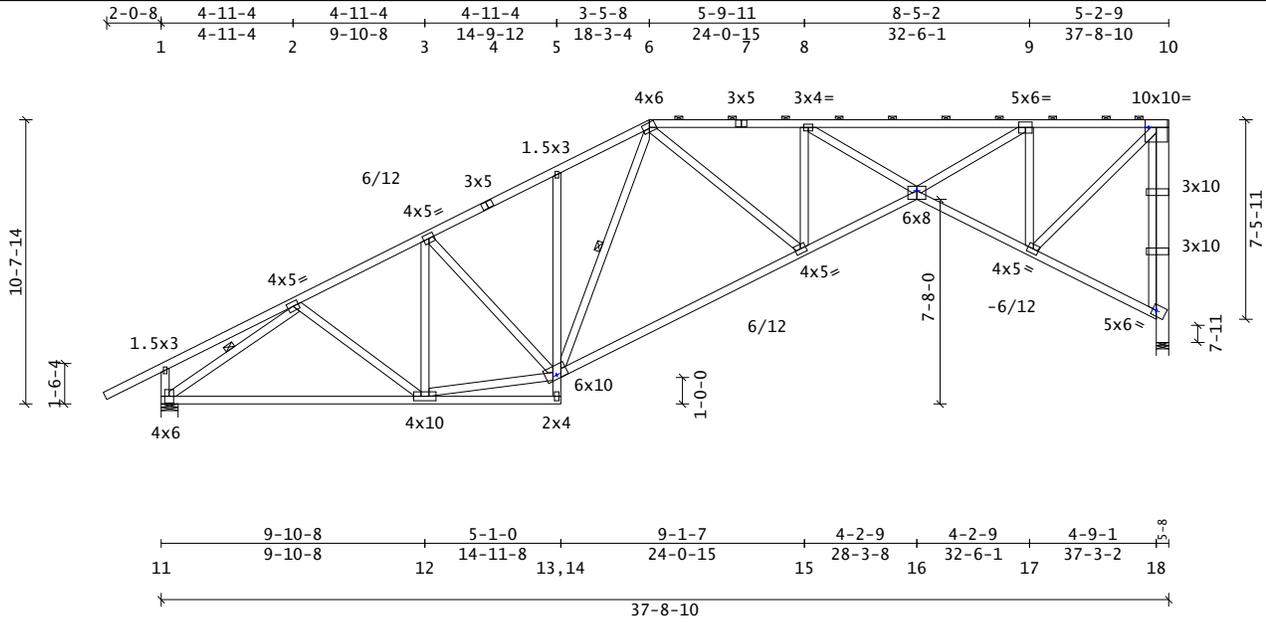
SID: 0003323687

Truss Mfr. Contact: Chris Wallington

TID: 258827

Date: 01 / 07 / 25

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Truss Weight = 290.1 lb

Code/Design: FBC-2023/TPI-2014

PSF Live	Dead	Dur	Factors
TC 20.0	10.0	Live	Wind Snow
BC 0.0	10.0	Lum	1.25 1.60 N/A
Total	40.0	Plt	1.25 1.60 N/A
Spacing:	2-00-00	o.c.	Plies: 1
Repetitive Member Increase:	Yes		
Green Lumber:	No	Wet Service:	No
Fab Tolerance:	20% Creep	(Kcr) =	2.0
OH Soffit Load:	2.0	psf	

-----Snow Load Specs-----

ASCE7-22 Ground Snow (Pg) = N/A

Risk Cat: II Terrain Cat: C

Roof Exposure: Sheltered

Thermal Condition: All Others(1.0)

Unobstructed Slippery Roof: No

Low-Slope Minimums(Pfmin): No

Unbalanced Snow Loads: No

Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----

ASCE7-22 Wind Speed (V) = 140 mph

Risk Cat: II Exposure Cat: C

Bldg Dims: L = 97.2 ft B = 78.3 ft

M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00

Bldg Enclosure: Enclosed

Wind DL(psf): TC = 5.0 BC = 5.0

End Vertical Exposed: L = Yes R = Yes

Wind Uplift Reporting: ASCE7 MWFRS

Hurricane Prone Region

C&C End Zone: 7-10-00

-----Additional Design Checks-----

10 psf Non-Concurrent BCLL:	Yes
20 psf BC Limited Storage:	Yes
200 lb BC Accessible Ceiling:	Yes
300 lb TC Maintenance Load:	Yes
2000 lb TC Safe Load:	No
Unbalanced TCLL:	Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1
BB	2x6	SP (ALSC6-2013)	SS

Reaction Summary

-----Reaction Summary(Lbs)-----

Jnt	--X-Loc	React	-Up-	--Width-	-Regd	-Mat	PSI
11	01-12	1648	552	07-08	02-09	SPF	425
18	37-05-14	1501	517	05-08	01-14	SPF	531

Max Horiz = -255 / +554 at Joint 11

Max Horiz = -255 / +554 at Joint 18

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.35)	14-15
Vert DL	L/120	L/991(-0.45)	14-15
Vert CR	L/180	L/558(-0.80)	14-15
Horz LL	0.75in	(0.21)	@Jt10
Horz CR	1.25in	(0.41)	@Jt10
Ohng CR	2L/180	2L/801(0.07)	1- 1
DPL LL	L/120	L/152(-0.08)	19-19

Bracing Data Summary

-----Bracing Data-----

Chords; Sheathing required or bracing indicated:

-----Purlins-----

TC	3-08-00	-2-02-01	19-02-13	7
TC	2-00-00	19-02-13	36-09-01	9
TC	11-00	36-09-01	37-08-10	2
BC	5-07-00	0	37-03-02	8

-----Web Bracing -----CLR-----

Single: 11- 2 14- 6

Continuous Restraint Bracing Req'd

See BCSI-B3 3.0

Member Forces Summary

Max CSI in TC PANEL	6 - 7	1.00
Max CSI in BC PANEL	15 - 16	0.98
Max CSI in Web	17 - 9	0.68

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [27-11-15] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.

Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.

Less than 0.25/12 pitch requires adequate drainage to prevent ponding.

Lateral forces from the bearing wall, if applicable, have not been considered in the design of the web(s) extended down to a bearing below the bottom chord. This condition may require additional design considerations and must be approved by the Building Designer.

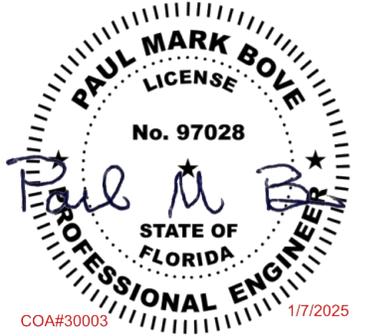
Plate offsets (X, Y):

(None unless indicated below)

Jnt10(03-08,-01-08), Jnt14(-00-09,01-02), Jnt16(0,-00-15), Jnt18(01-02,-00-09)

...Mem... Ten Comp .CSI.

TC OH- 1	124	0	0.61
1- 2	218	76	0.60
2- 3	1333	2060	0.59
3- 4	1401	2011	0.57
4- 5	1413	1949	0.52
5- 6	1581	2019	0.56
6- 7	2261	2759	1.00
7- 8	2261	2759	0.67
8- 9	3206	3796	0.97
9-10	1125	1422	0.59
BC OH-11	0	0	0.00
11-12	1674	1784	0.89
12-13	27	25	0.67
14-15	2044	1932	0.87
15-16	3089	2782	0.98
16-17	1592	1494	0.38
17-18	1073	285	0.23
Web 1-11	433	417	0.05
2-11	1156	2107	0.35
2-12	225	34	0.03
3-12	346	237	0.13
3-14	191	184	0.14
5-14	416	331	0.32
6-14	184	370	0.12
6-15	1243	964	0.40
8-15	1081	1081	0.38
8-16	1219	1111	0.21
9-16	2791	2446	0.42
9-17	1808	1943	0.68
10-17	1967	1679	0.54
10-18	718	661	0.46
12-14	1789	1692	0.27
13-14	85	22	0.04
BBK 10-18	1196	1501	0.52
10-18	842	1053	0.37



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SIMPSON Strong-Tie Component Solutions
Truss Studio V
2024.3.2.1

Customer: Valued Customer

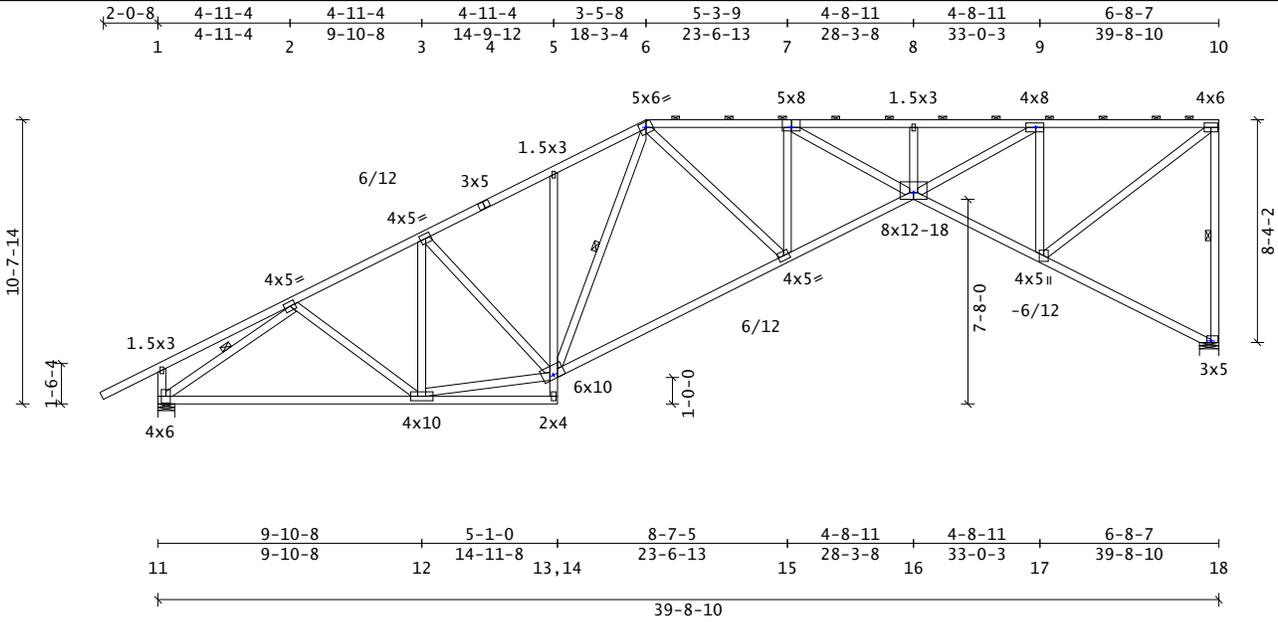
SID: 000323688

Truss Mfr. Contact: Chris Wallington

TID: 258827

Date: 01 / 07 / 25

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Truss Weight = 283.5 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1

Member Forces Summary

Max CSI in TC PANEL	6 - 7	0.95
Max CSI in BC PANEL	15 - 16	1.00
Max CSI in Web	17 - 10	0.87

...	Mem...	Ten	Comp	.CSI.
TC	OH-1	124	0	0.61
	1-2	218	76	0.60
	2-3	1432	2208	0.52
	3-4	1525	2189	0.53
	4-5	1537	2126	0.48
	5-6	1720	2197	0.52
	6-7	2440	3009	0.95
	7-8	4004	4786	0.62
	8-9	4004	4786	0.80
	9-10	1370	1773	0.65
	10-OH	0	0	0.00
BC	OH-11	0	0	0.00
	11-12	1783	1864	0.90
	12-13	29	27	0.67
	14-15	2264	2097	0.84
	15-16	3417	3016	1.00
	16-17	1986	1765	0.45
	17-18	973	288	0.45
Web	1-11	433	417	0.05
	2-11	1253	2244	0.38
	2-12	247	20	0.03
	3-12	379	279	0.16
	3-14	163	162	0.12
	5-14	413	333	0.33
	6-14	249	256	0.12
	6-15	1390	1059	0.41
	7-15	1223	1300	0.50
	7-16	2001	1758	0.37
	8-16	240	294	0.04
	9-16	3441	3009	0.59
	9-17	1920	2086	0.81
	10-17	2237	1856	0.87
	10-18	1318	1533	0.79
	12-14	1920	1791	0.29
	13-14	86	22	0.04

Reaction Summary

Jnt	--X-Loc	React	-Up-	--Width-	-Reqd	-Mat	PSI
11	01-12	1732	578	07-08	02-11	SPF	425
18	39-06-14	1584	537	05-02	02-08	SPF	425

Max Horiz = -255 / +554 at Joint 11
 Max Horiz = -255 / +554 at Joint 18

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [28-11-15] using a 1.00 Full and 0.00 Reduced load factor.
 See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE. Less than 0.25/12 pitch requires adequate drainage to prevent ponding.

Deflection Summary

TrussSpan	Limit	Actual (in)	Location
Vert LL	L/240	L/999(-0.33)	14-15
Vert DL	L/120	L/999(-0.46)	14-15
Vert CR	L/180	L/598(-0.79)	14-15
Horz LL	0.75in	(0.26)	@Jt18
Horz CR	1.25in	(0.52)	@Jt18
Ohng CR	2L/180	2L/801(0.07)	1- 1

Bracing Data Summary

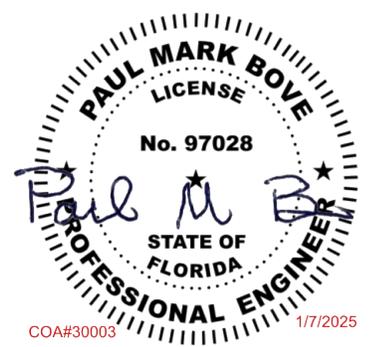
-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:
 -----Purlins-----

--oc--	--From--	--To--	#Bays
TC	3-05-00	-2-02-01	19-02-13 7
TC	2-00-00	19-02-13	38-09-01 10
TC	11-00	38-09-01	39-08-10 2
BC	5-05-00	0	39-05-02 8

 ----- Web Bracing ----- CLR -----
 Single: 11- 2 14- 6 18-10
 Continuous Restraint Bracing Req'd
 See BCSI-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)
 Jnt6(00-01,-00-01), Jnt7(0,01-00),
 Jnt9(-00-08,0), Jnt14(-00-09,01-02),
 Jnt16(0,00-15), Jnt18(00-09,00-13)



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SIMPSON Strong-Tie Component Solutions
 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

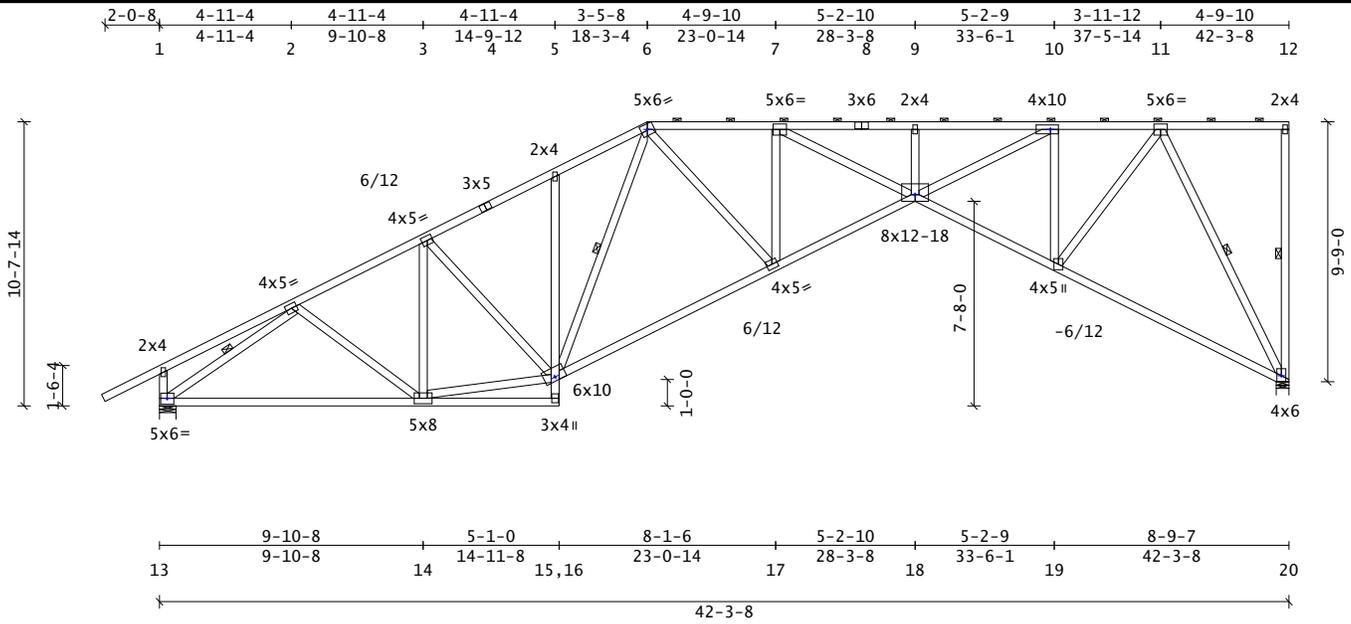
SID: 0003323689

Truss Mfr. Contact: Chris Wallington

TID: 258827

Date: 01 / 07 / 25

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Truss Weight = 311.0 lb

Code/Design: FBC-2023/TPI-2014

PSF Live	Dead	Dur Factors
TC 20.0	10.0	Live Wind Snow
BC 0.0	10.0	Lum 1.25 1.60 N/A
Total	40.0	Plt 1.25 1.60 N/A
Spacing:	2-00-00 o.c.	Plies: 1
Repetitive Member Increase:	Yes	
Green Lumber:	No	Wet Service: No
Fab Tolerance:	20% Creep (Kcr) = 2.0	
OH Soffit Load:	2.0 psf	

-----Snow Load Specs-----

ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----

ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----

10 psf Non-Concurrent BCLL:	Yes
20 psf BC Limited Storage:	Yes
200 lb BC Accessible Ceiling:	Yes
300 lb TC Maintenance Load:	Yes
2000 lb TC Safe Load:	No
Unbalanced TCLL:	Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1

Member Forces Summary

Max CSI in TC PANEL	6 - 7	0.99
Max CSI in BC PANEL	17 - 18	0.97
Max CSI in Web	11 - 20	0.99

...	Mem...	Ten	Comp	.CSI.
TC	OH-1	124	0	0.61
	1-2	218	76	0.60
	2-3	1542	2389	0.51
	3-4	1678	2408	0.54
	4-5	1690	2346	0.51
	5-6	1898	2422	0.55
	6-7	2722	3339	0.99
	7-8	4968	5906	0.87
	8-9	4968	5906	0.82
	9-10	4968	5906	0.85
	10-11	1711	2226	0.35
	11-12	220	198	0.35
	12-OH	0	0	0.00
BC	OH-13	0	0	0.00
	13-14	1917	2000	0.91
	14-15	32	29	0.67
	16-17	2534	2351	0.81
	17-18	3745	3325	0.97
	18-19	2490	2187	0.75
	19-20	1114	1053	0.72
	20-OH	5	0	0.00
Web	1-13	433	417	0.05
	2-13	1340	2413	0.41
	2-14	273	10	0.04
	3-14	425	331	0.19
	3-16	148	135	0.10
	5-16	424	338	0.33
	6-16	346	365	0.14
	6-17	1621	1266	0.47
	7-17	1447	1566	0.67
	7-18	2870	2510	0.62
	9-18	348	325	0.04
	10-18	4115	3641	0.86
	10-19	1936	2121	0.91
	11-19	2112	1694	0.51
	11-20	1854	2190	0.99
	12-20	138	211	0.90
	14-16	2083	1948	0.31
	15-16	86	22	0.05

Reaction Summary

-----Reaction Summary(Lbs)-----

Jnt	--X-Loc	React	-Up-	--Width-	-Reqd	-Mat	PSI
13	01-12	1834	606	07-08	02-14	SPF	425
20	42-01-12	1690	568	05-12	02-02	SPF	531

Max Horiz = -294 / +586 at Joint 13
 Max Horiz = -294 / +586 at Joint 20

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [30-03-06] using a 1.00 Full and 0.00 Reduced load factor.
 See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
 Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
 Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRAE. Alternatively, see D-WEBREINFORCE.
 Less than 0.25/12 pitch requires adequate drainage to prevent ponding.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.43)	18-19
Vert DL	L/120	L/999(-0.50)	16-17
Vert CR	L/180	L/589(-0.86)	16-17
Horz LL	0.75in	(0.38)	@Jt20
Horz CR	1.25in	(0.75)	@Jt20
Ohng CR	2L/180	2L/801(0.07)	1- 1

Bracing Data Summary

-----Bracing Data-----

Chords; Sheathing required or bracing indicated:

-----Purlins-----

oc---	From--	To---	#Bays
TC	3-03-00	-2-02-01	19-02-13 8
TC	2-00-00	19-02-13	41-03-15 12
TC	11-00	41-03-15	42-03-08 2
BC	5-01-00	0	42-03-08 9

----- Web Bracing --- CLR -----

Single: 13- 2 16- 6 11-20 20-12
 Continuous Restraint Bracing Req'd
 See BCSI-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)
 Jnt6(00-01,-00-01), Jnt10(-01-08,0),
 Jnt13(0,-00-04), Jnt16(-00-09,01-02),
 Jnt18(0,00-15), Jnt20(0,00-03)



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Component Solutions
 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

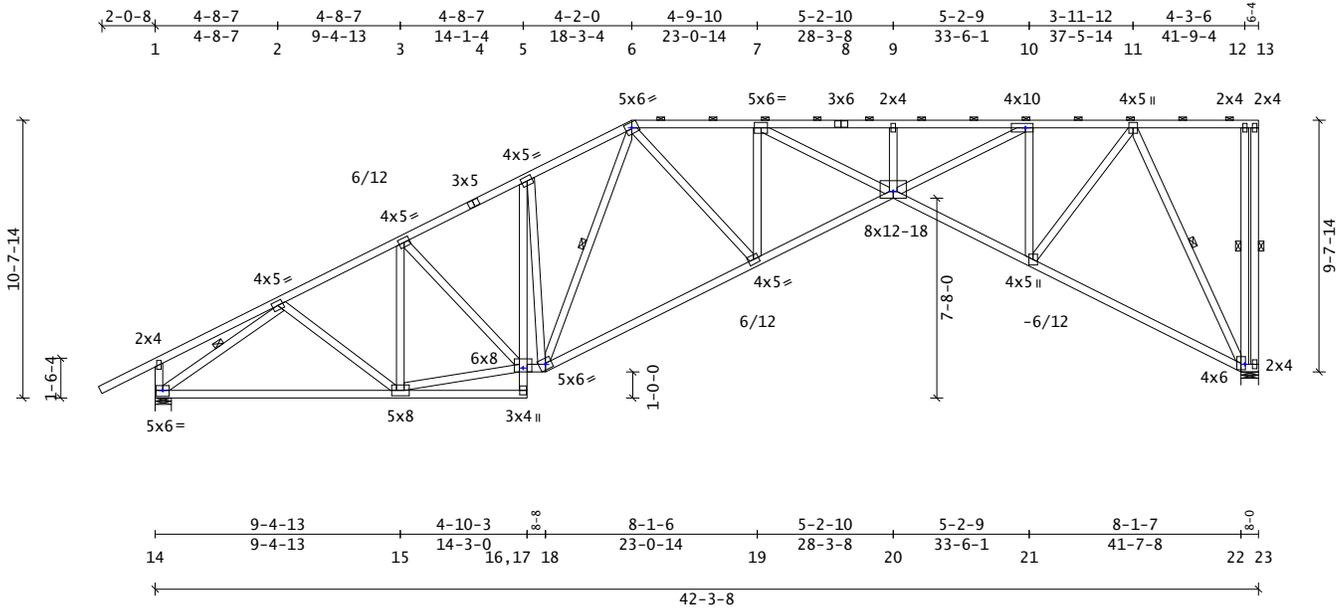
SID: 0003323690

Truss Mfr. Contact: Chris Wallington

TID: 258827

Date: 01 / 07 / 25

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Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums (Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1

...Mem... Ten Comp .CSI.

13-23	0	200	0.86
15-17	2072	1941	0.31
16-17	85	22	0.04

Deflection Summary

TrussSpan	Limit	Actual (in)	Location
Vert LL	L/240	L/999(-0.42)	20-21
Vert DL	L/120	L/999(-0.41)	20-21
Vert CR	L/180	L/605(-0.83)	20-21
Horz LL	0.75in	(0.37)	@Jt23
Horz CR	1.25in	(0.73)	@Jt23
Ohng CR	2L/180	2L/801(0.07)	1- 1

Member Forces Summary

Max CSI in TC PANEL	6 - 7	0.96
Max CSI in BC PANEL	19 - 20	0.98
Max CSI in Web	21 - 10	0.90

Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat	PSI
14	01-12	1819	602	07-08	02-14	SPF	425
23	41-09-04	1705	573	08-00	02-11	SPF	425

Max Horiz = -285 / +579 at Joint 14
 Max Horiz = -285 / +579 at Joint 22

Bracing Data Summary

Chords; Sheathing required or bracing indicated:

...Mem... Ten Comp .CSI.

TC OH-1	124	0	0.61	
1- 2	216	76	0.60	
2- 3	1523	2364	0.49	
3- 4	1702	2445	0.52	
4- 5	1710	2370	0.53	
5- 6	1837	2393	0.58	
6- 7	2662	3266	0.96	
7- 8	4823	5749	0.84	
8- 9	4823	5749	0.78	
9-10	4823	5749	0.81	
10-11	1633	2124	0.33	
11-12	213	194	0.32	
12-13	215	193	0.10	
13-OH	0	0	0.00	
BC OH-14	0	0	0.00	
14-15	1876	1964	0.86	
15-16	31	28	0.62	
17-18	2138	2008	0.37	
18-19	2495	2310	0.69	
19-20	3667	3251	0.98	
20-21	2380	2090	0.57	
21-22	1022	967	0.60	
22-23	215	193	0.12	
23-OH	0	0	0.00	
Web	1-14	429	419	0.05
2-14	1314	2376	0.38	
2-15	289	14	0.04	
3-15	472	400	0.21	
3-17	132	120	0.06	
5-17	317	346	0.14	
5-18	751	613	0.52	
6-18	368	368	0.14	
6-19	1556	1218	0.45	
7-19	1401	1521	0.65	
7-20	2776	2415	0.60	
9-20	349	325	0.04	
10-20	4053	3567	0.84	
10-21	1913	2098	0.90	
11-21	2069	1677	0.51	
11-22	1779	2113	0.83	
12-22	262	159	0.05	

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [30-03-06] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for load combinations and additional details.

Notes

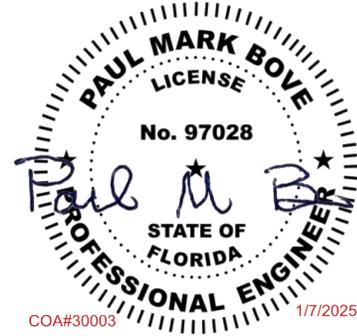
Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE. Less than 0.25/12 pitch requires adequate drainage to prevent ponding.

-----Purlins-----

TC	3-03-00	-2-02-01	19-02-13	8
TC	2-00-00	19-02-13	41-03-15	12
TC	11-00	41-03-15	42-03-08	2
BC	5-02-00	0	42-03-08	9

----- Web Bracing --- CLR -----
 Single: 14- 2 18- 6 11-22 22-12 23-13
 Continuous Restraint Bracing Req'd
 See BCSI-B3 3.0

Plate offsets (X, Y):
 (None unless indicated below)
 Jnt6(00-01,-00-01), Jnt10(-01-08,0),
 Jnt14(0,-00-04), Jnt17(0,01-04),
 Jnt18(-00-01,00-01), Jnt20(0,00-15),
 Jnt22(-01-12,00-08)



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SIMPSON Strong-Tie Component Solutions
 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

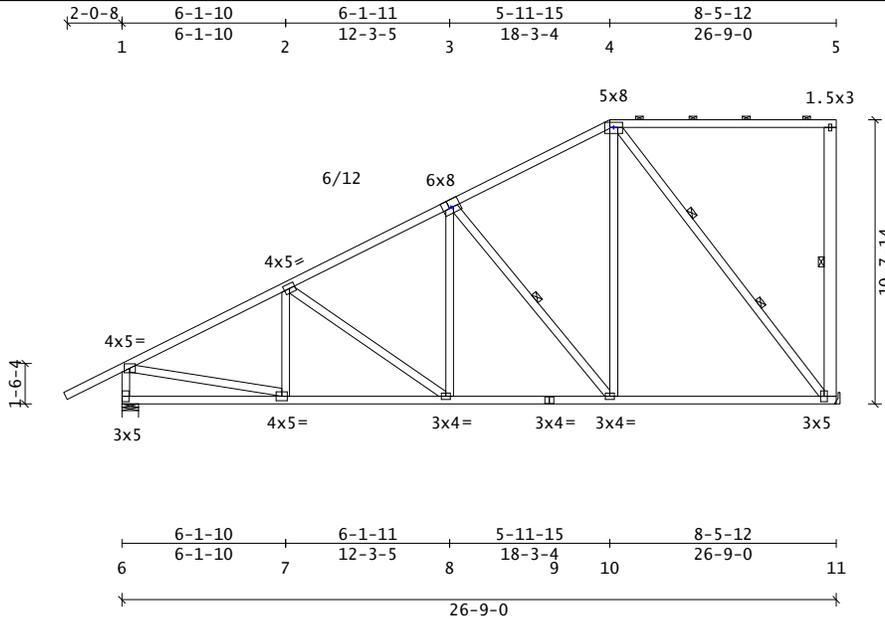
SID: 0003323691

Truss Mfr. Contact: Chris Wallington

TID: 258827

Date: 01 / 07 / 25

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Truss Weight = 195.0 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1
	2x6	SP (ALSC6-2013)	SS 11-5

Member Forces Summary

Max CSI in TC PANEL	1 - 2	0.98
Max CSI in BC PANEL	6 - 7	1.00
Max CSI in Web	4 - 11	0.51

...	Mem...	Ten	Comp	.CSI.
TC	OH- 1	124	0	0.62
	1- 2	807	1404	0.98
	2- 3	792	1163	0.79
	3- 4	660	761	0.53
	4- 5	260	225	0.96
	5-OH	0	0	0.00
BC	OH- 6	0	0	0.00
	6- 7	302	1017	1.00
	7- 8	1193	1353	0.98
	8- 9	966	1093	0.62
	9-10	966	1093	0.69
	10-11	624	718	0.70
	11-OH	0	0	0.00
Web	1- 6	818	1160	0.11
	1- 7	1220	527	0.18
	2- 7	182	119	0.03
	2- 8	319	316	0.27
	3- 8	331	132	0.05
	3-10	578	566	0.21
	4-10	610	354	0.24
	4-11	936	1005	0.51
	5-11	310	286	0.33

Reaction Summary

Jnt	--X-Loc	React	-Up-	--Width-	-Reqd	-Mat	PSI
6	01-12	1212	396	07-08	01-14	SPF	425
11	26-06-04	1092	385	01-08	HGR	SPF	565

Max Horiz = -314 / +603 at Joint 6
 Max Horiz = -314 / +603 at Joint 11

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [22-06-02] using a 1.00 Full and 0.00 Reduced load factor.
 See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE. Less than 0.25/12 pitch requires adequate drainage to prevent ponding.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.19)	10-11
Vert DL	L/120	L/999(-0.15)	10-11
Vert CR	L/180	L/937(-0.34)	10-11
Horz LL	0.75in	(0.03)	@Jt11
Horz CR	1.25in	(0.04)	@Jt11
Ohng CR	2L/180	2L/801(0.07)	1- 1

Bracing Data Summary

-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:
 -----Purlins-----

TC	--oc--	--From--	--To--	#Bays
TC	5-02-00	-2-02-01	19-02-13	5
TC	2-00-00	19-02-13	25-09-07	4
TC	11-00	25-09-07	26-09-00	2
BC	8-05-00	0	26-09-00	4

 ----- Web Bracing ----- CLR -----
 Single: 3-10 11- 5
 ----- Web Bracing ----- CLR -----
 Double: 4-11
 Continuous Restraint Bracing Req'd
 See BCSI-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)
 Jnt3(-00-04,00-07), Jnt4(0,-00-02)



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 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

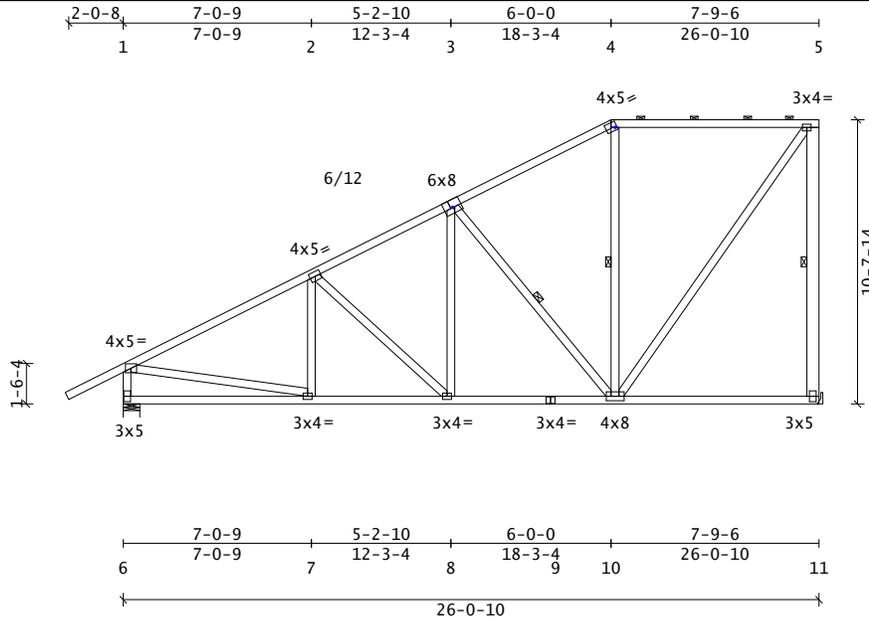
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TID: 258827

Date: 01 / 07 / 25

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Truss Mfr. Contact: Chris Wallington



Truss Weight = 193.7 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1
	2x6	SP (ALSC6-2013)	SS 11-5

Member Forces Summary

Max CSI in TC PANEL	1	-	2	0.99
Max CSI in BC PANEL	6	-	7	0.99
Max CSI in Web	10	-	5	0.86

...	Mem...	Ten	Comp	.CSI.
TC	OH- 1	124	0	0.62
	1- 2	777	1367	0.99
	2- 3	772	1099	0.66
	3- 4	618	690	0.53
	4- 5	649	545	0.90
	5-OH	0	0	0.00
BC	OH- 6	0	0	0.00
	6- 7	302	1019	0.99
	7- 8	1149	1294	0.87
	8- 9	916	1050	0.56
	9-10	916	1050	0.62
	10-11	897	237	0.62
	11-OH	0	0	0.00
Web	1- 6	804	1123	0.10
	1- 7	1169	482	0.18
	2- 7	161	72	0.02
	2- 8	332	332	0.25
	3- 8	343	178	0.06
	3-10	587	575	0.21
	4-10	378	175	0.07
	5-10	938	913	0.86
	5-11	1004	978	0.37

Reaction Summary

Jnt	--X-	-Loc-	React	-Up-	--Width-	-Reqd	-Mat	PSI
6	01-12	1184	386	07-08	01-14	SPF	425	
11	25-09-14	1073	377	01-08	HGR	SPF	565	

Max Horiz = -314 / +603 at Joint 6
 Max Horiz = -314 / +603 at Joint 11

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [22-01-15] using a 1.00 Full and 0.00 Reduced load factor.
 See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
 Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
 Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.
 Less than 0.25/12 pitch requires adequate drainage to prevent ponding.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.15)	10-11
Vert DL	L/120	L/999(-0.10)	10-11
Vert CR	L/180	L/999(-0.25)	10-11
Horz LL	0.75in	(0.02)	@Jt11
Horz CR	1.25in	(0.03)	@Jt11
Ohng CR	2L/180	2L/801(0.07)	1- 1

Bracing Data Summary

-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:

---	oc---	From--	To---	#Bays
TC	4-10-00	-2-02-01	19-02-13	5
TC	2-00-00	19-02-13	25-01-01	3
TC	11-00	25-01-01	26-00-10	2
BC	8-04-00	0	26-00-10	4

----- Web Bracing --- CLR -----
 Single: 3-10 10- 4 11- 5
 Continuous Restraint Bracing Req'd
 See BCSI-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)
 Jnt3(-00-04,00-07), Jnt4(-01-12,0)



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 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

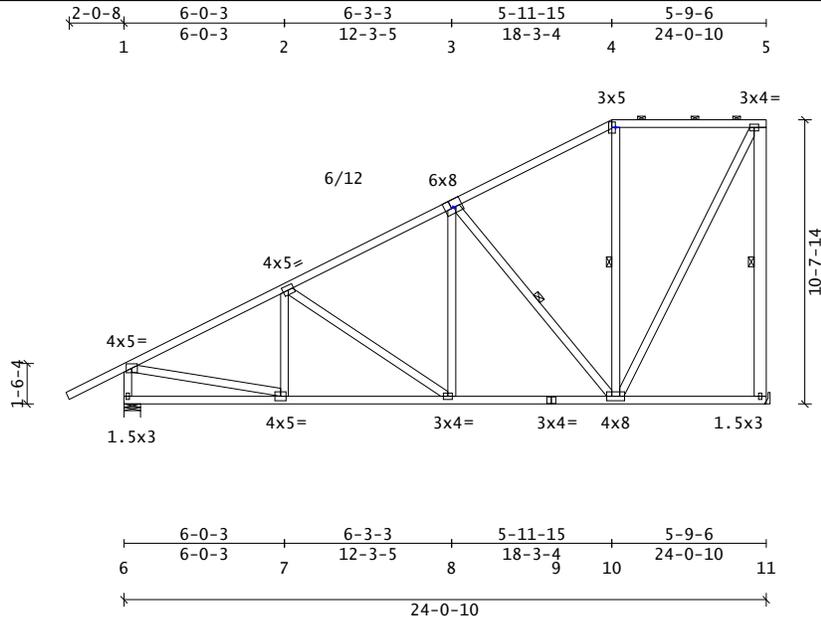
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Truss Mfr. Contact: Chris Wallington



Truss Weight = 184.0 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1
	2x6	SP (ALSC6-2013)	SS 11-5

Member Forces Summary

Max CSI in TC PANEL	1	-	2	0.97
Max CSI in BC PANEL	6	-	7	0.82
Max CSI in Web	10	-	5	0.67

...	Mem...	Ten	Comp	.CSI.
TC	OH- 1	124	0	0.62
	1- 2	688	1232	0.97
	2- 3	654	971	0.71
	3- 4	514	531	0.53
	4- 5	545	394	0.57
	5-OH	0	0	0.00
BC	OH- 6	0	0	0.00
	6- 7	302	1016	0.82
	7- 8	1041	1210	0.68
	8- 9	798	927	0.40
	9-10	798	927	0.41
	10-11	892	237	0.35
	11-OH	0	0	0.00
Web	1- 6	743	1054	0.10
	1- 7	1065	421	0.16
	2- 7	142	94	0.02
	2- 8	343	325	0.28
	3- 8	365	132	0.05
	3-10	619	622	0.23
	4-10	332	194	0.08
	5-10	853	832	0.67
	5-11	935	915	0.37

Reaction Summary

Jnt	--X-Loc	React	-Up-	--Width-	-Reqd	-Mat	PSI
6	01-12	1104	359	07-08	01-12	SPF	425
11	23-09-14	973	355	01-08	HGR	SPF	565

Max Horiz = -314 / +603 at Joint 6
 Max Horiz = -314 / +603 at Joint 11

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [21-01-15] using a 1.00 Full and 0.00 Reduced load factor.
 See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
 Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
 Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.
 Less than 0.25/12 pitch requires adequate drainage to prevent ponding.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.04)	8-10
Vert DL	L/120	L/999(-0.05)	6- 7
Vert CR	L/180	L/999(-0.09)	6- 7
Horz LL	0.75in	(0.02)	@Jt11
Horz CR	1.25in	(0.03)	@Jt11
Ohng CR	2L/180	2L/801(0.07)	1- 1

Bracing Data Summary

-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:
 -----Purlins-----

TC	--oc--	--From--	--To--	#Bays
TC	5-06-00	-2-02-01	19-02-13	5
TC	2-00-00	19-02-13	23-01-01	2
TC	11-00	23-01-01	24-00-10	2
BC	8-00-00	0	24-00-10	4

 ----- Web Bracing ----- CLR -----
 Single: 3-10 10- 4 11- 5
 Continuous Restraint Bracing Req'd
 See BCSI-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)
 Jnt3(-00-04,00-07), Jnt4(-01-12,0)



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Component Solutions
 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

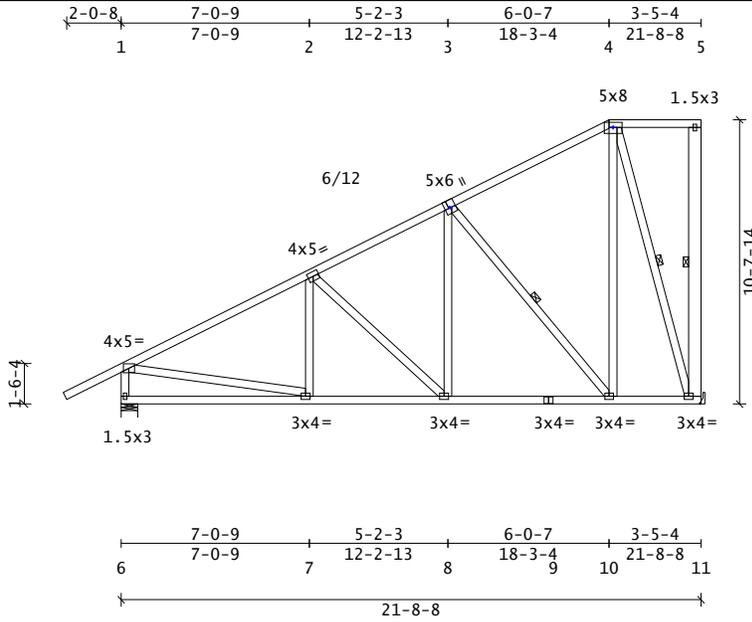
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Truss Mfr. Contact: Chris Wallington

TID: 258827

Date: 01 / 07 / 25

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Truss Weight = 176.0 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. - Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1
	2x6	SP (ALSC6-2013)	SS 11-5

Member Forces Summary

Max CSI in TC PANEL	1 - 2	0.99
Max CSI in BC PANEL	6 - 7	0.57
Max CSI in Web	4 - 11	0.37

...	Mem...	Ten	Comp	.CSI.
TC	OH- 1	124	0	0.62
	1- 2	580	1083	0.99
	2- 3	551	785	0.59
	3- 4	383	342	0.50
	4- 5	274	232	0.20
	5-OH	0	0	0.00
BC	OH- 6	0	0	0.00
	6- 7	302	1013	0.57
	7- 8	896	1038	0.44
	8- 9	636	761	0.36
	9-10	636	761	0.31
	10-11	666	401	0.28
	11-OH	0	0	0.00
Web	1- 6	683	951	0.09
	1- 7	911	303	0.14
	2- 7	183	29	0.02
	2- 8	376	352	0.26
	3- 8	391	202	0.06
	3-10	613	631	0.23
	4-10	595	409	0.31
	4-11	767	793	0.37
	5-11	130	187	0.34

Reaction Summary

Jnt	--X-Loc	React	-Up-	--Width-	-Regd	-Mat	PSI
6	01-12	1011	326	07-08	01-09	SPF	425
11	21-05-12	868	329	01-08	HGR	SPF	565

Max Horiz = -314 / +603 at Joint 6
 Max Horiz = -314 / +603 at Joint 11

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [19-11-14] using a 1.00 Full and 0.00 Reduced load factor.
 See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
 Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
 Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.
 Less than 0.25/12 pitch requires adequate drainage to prevent ponding.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.08)	6- 7
Vert DL	L/120	L/999(-0.09)	6- 7
Vert CR	L/180	L/999(-0.17)	6- 7
Horz LL	0.75in	(0.02)	@Jt11
Horz CR	1.25in	(0.03)	@Jt11
Ohng CR	2L/180	2L/801(0.07)	1- 1

Bracing Data Summary

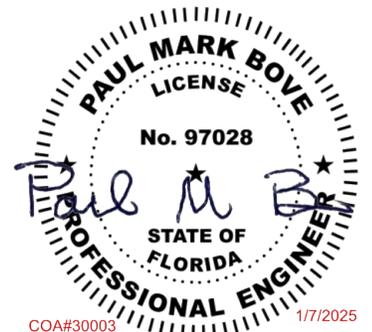
-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:

---	oc---	From--	To---	#Bays
TC	5-06-00	-2-02-01	19-02-13	5
TC	2-00-00	19-02-13	20-08-15	1
TC	11-00	20-08-15	21-08-08	2
BC	7-02-00	0	21-08-08	4

----- Web Bracing --- CLR -----
 Single: 3-10 4-11 11- 5
 Continuous Restraint Bracing Req'd
 See BCSI-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)
 Jnt3(-00-04,00-07), Jnt4(0,-00-02)



NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



Component Solutions
 Truss Studio V
 2024.3.2.1

Customer: Valued Customer

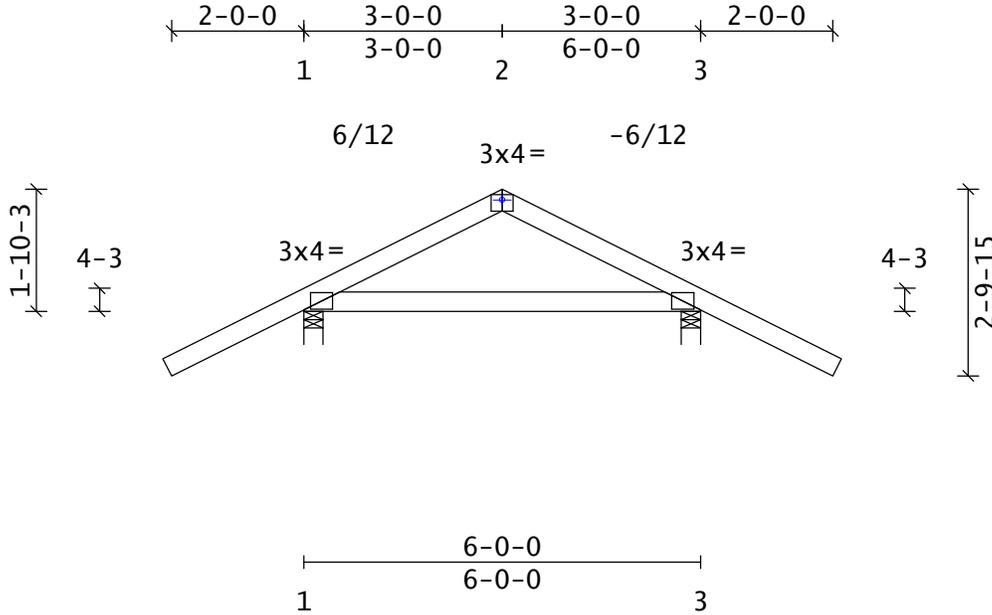
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Date: 01 / 06 / 25

Page: 1 of 1

Truss Mfr. Contact: Chris Wallington



Truss Weight = 25.8 lb

Code/Design: FBC-2023/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.25 1.60 N/A
 Total 40.0 Plt 1.25 1.60 N/A
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-22 Ground Snow (Pg) = N/A
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----
 ASCE7-22 Wind Speed (V) = 140 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 97.2 ft B = 78.3 ft
 M.R.H(h) = 25.0ft Kzt = 1.0 Ke = 1.00
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 MWFRS
 Hurricane Prone Region
 C&C End Zone: 7-10-00

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: Yes
 300 lb TC Maintenance Load: Yes
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1

Member Forces Summary

Max CSI in TC PANEL	1 - 1	0.53
Max CSI in BC PANEL	1 - 3	0.24

...	Mem...	Ten	Comp	.CSI.
TC	OH- 1	117	0	0.53
	1- 2	152	286	0.24
	2- 3	152	286	0.24
	3-OH	117	0	0.50
BC	1- 3	235	23	0.24

Reaction Summary

-----Reaction Summary(Lbs)-----
 Jnt --X-Loc- React -Up- --Width- -Reqd -Mat PSI
 1 01-12 454 147 03-08 01-08 SPF 531
 3 5-10-04 454 147 03-08 01-08 SPF 531
 Max Horiz = -48 / +48 at Joint 1

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [3-00-00] using a 1.00 Full and 0.00 Reduced load factor.
 See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Lumber and plating have been applied symmetrically.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.02)	1- 3
Vert DL	L/120	L/999(-0.01)	1- 3
Vert CR	L/180	L/999(-0.04)	1- 3
Horz LL	0.75in	(0.01)	@Jt 1
Horz CR	1.25in	(0.01)	@Jt 1
Ohng CR	2L/180	2L/676(0.08)	1- 1
Ohng CR	2L/180	2L/676(0.08)	3- 3

Bracing Data Summary

-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:
 -----Purlins-----
 ---oc--- --From-- ---To--- #Bays
 TC 5-08-00 -2-01-09 8-01-09 3
 BC 6-00-00 0 6-00-00 1
 Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)
 Jnt2(0, -00-09)



COA#30003

1/7/2025

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



Component Solutions
 Truss Studio V
 2024.3.2.1

NOTES:

1. This detail provides information for attaching a bearing block to one face of a 2-ply truss to increase the allowable reaction of the truss.
2. Refer to the individual truss design drawing for the bottom chord size and other design information.
3. The bearing blocks must be min. 16" long and the same size, species and grade as the truss bottom chord.
4. If the truss bottom chord species is different than the bearing material, the lower of the listed reactions for Details A-C shall apply. Values listed for SP do not apply to Non-Dense (ND) grades.

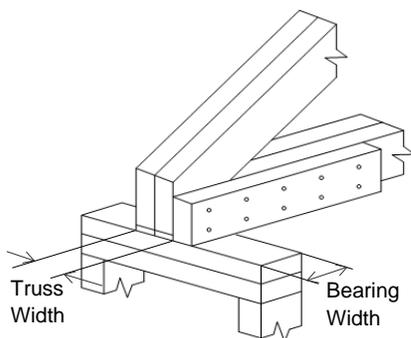


Fig. 1 Bearing Block at End of Truss

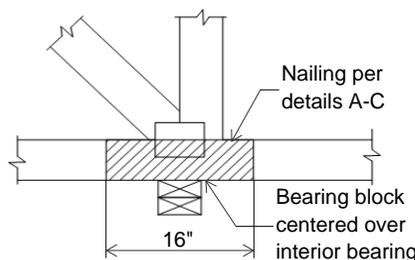
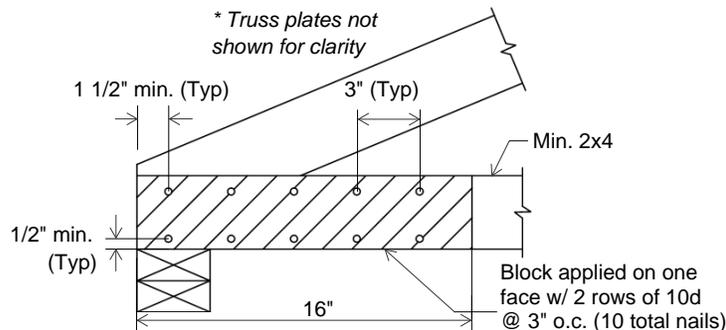
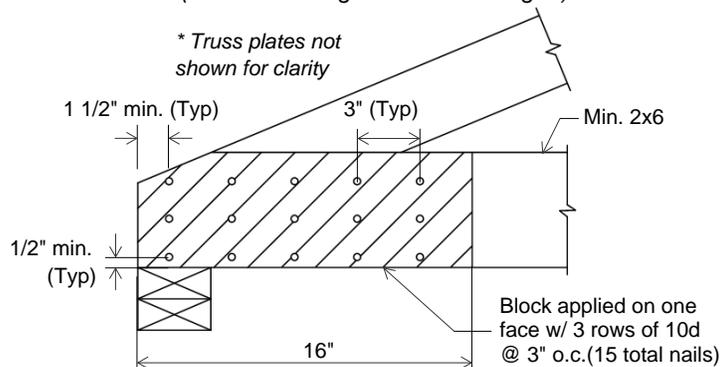


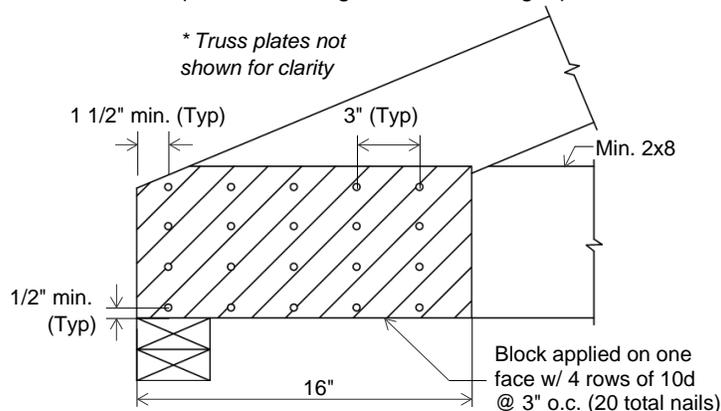
Fig. 2 Bearing Block at Interior Bearing



Detail A - Bearing Block Detail for 2x4 Bottom Chord (Interior Bearing Similar - See Fig. 2)



Detail B - Bearing Block Detail for 2x6 Bottom Chord (Interior Bearing Similar - See Fig. 2)



Detail C - Bearing Block Detail for 2x8 Bottom Chord (Interior Bearing Similar - See Fig. 2)

Maximum Reactions for Bearing Block Details A-C (2-Ply Truss with Bearing Block on One Face)

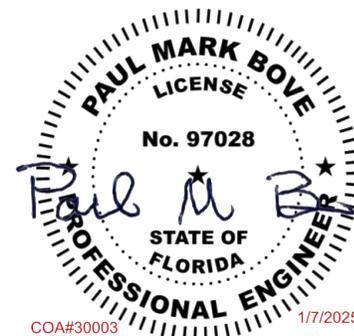
Detail	Bottom Chord (BC) Size	Lumber Species	Max. Reaction	
			3.5" Brg Width	5.5" Brg Width
A	2x4	SP	7150 lbs.	10540 lbs.
		DF	7680 lbs.	11430 lbs.
		HF	5220 lbs.	7650 lbs.
		SPF	5405 lbs.	7955 lbs.
B	2x6	SP	7760 lbs.	11150 lbs.
		DF	8235 lbs.	11985 lbs.
		HF	5700 lbs.	8130 lbs.
		SPF	5875 lbs.	8425 lbs.
C	2x8 or larger	SP	8370 lbs.	11760 lbs.
		DF	8795 lbs.	12545 lbs.
		HF	6185 lbs.	8615 lbs.
		SPF	6350 lbs.	8900 lbs.

DETAIL LIMITATIONS:

Load Duration Factor: 1.15

Nail Dimension

10d = 3" x 0.131"



COA#30003

1/7/2025

This detail provides minimum connection requirements between a cap truss and a base truss of a piggyback assembly for assemblies that meet the following conditions and design requirements:

- the cap truss has continuous bearing or bearings at 2' o.c. max.;
- the cap truss contains vertical web members at 4' o.c. max.;
- the cap trusses are spaced no greater than 24" o.c.;
- the pitch does not exceed 12/12;
- the cap truss span does not exceed 100'; for spans > 36', see Table 2 Footnote 5
- the cap truss supports no point loads or drag loads (see detail TD-CPS-0002 for drag load connection requirements)

Design Requirements:

Max. Wind Speed: 140 mph (nominal) Load Duration Factor: 1.6
 Max. Mean Roof Height: 30' Lumber: SPF or Better
 Exp. Category: B or C (Min. SG = 0.42)

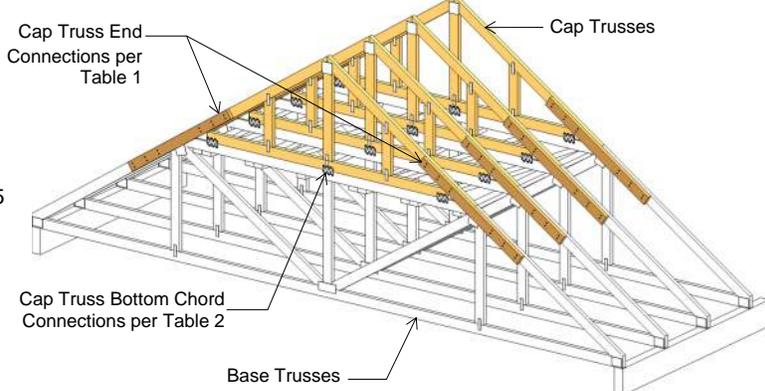


TABLE 1 - Connection Requirements At Each End of Cap Truss (One Face Only)

Cap Truss Span	Max. Wind Speed (mph)		End Connection Options	Fasteners ¹
	Nominal	Ultimate		
Up to 18'	100	125	Sheathing - See Note 2	---
	140	180	4' 2x Scab LSTA18 or CS20 Strap	16-10d nails 14-10dx1.5 nails
18' < Span <= 100'	120	150	4' 2x Scab	16-10d nails
	130	160	LSTA18 or CS20 Strap	14-10dx1.5 nails
	140	180	6' 2x Scab LSTA30 or CS18 Strap	24-10d nails 10-SDS 1/4x3 20-10dx1.5

See footnotes below

TABLE 2 - Connection Requirements Along Cap Truss Bottom Chord

Condition	Max. Wind Speed (mph)		Connection Options Along BC	Fasteners
	Nominal	Ultimate		
With 1.5" Gap (See Fig 1)	140	180	Nails into purlins installed on base truss at 24" o.c. (max)	Purlin-to-Base: 2-10d nails (ea purlin) Cap-to-purlin: 2-10d toe-nails (ea purlin)
			(3) 7"x7"x7/16" Plywood/OSB Gussets (each face)	6-6d nails per Gusset (see note 1 & 5)
			(3) LTP5 Framing Angles (one face)	8-8dx1.5 nails per LTP5 (see note 1 & 5)
No Gap (See Fig 2)	130	160	(2) LTP4 Framing Angles (one face)	12-8dx1.5 nails per LTP4 (see note 1 & 5)
	140	180	Toe-Nails @ 24" o.c.	2-10d toe-nails along BC @ 24" o.c.
			(3) 7"x7"x7/16" Plywood/OSB Gussets (each face) (3) LTP4 Framing Angles (one face)	6-6d nails per Gusset (see note 1 & 5) 12-8dx1.5 nails per LTP4 (see note 1 & 5)

1. Where noted, install half of the specified fasteners in each member being connected.
2. An additional end connection is not required for wind speeds up to 100 mph (nominal) and cap truss spans up to 18' if the sheathing is continuous and extends at least 12" beyond the intersection.
3. When using (2) connectors along the cap truss BC, install one at each end of the bottom chord; when using (3) connectors, install one on each end and one in the center of the bottom chord.
4. NAILS: 10d=0.148" dia.x3" long, 10dx1.5=0.148" dia.x1 1/2" long, 8dx1.5=0.131" dia.x1 1/2" long, 6d=0.113" dia.x2" long
5. For piggyback truss spans greater than 36' and less than or equal to 100', where there is a 1.5" gap between BC of cap truss and TC of base truss, attach BC of cap truss to the TC of base truss with 7"x7"x7/16" Plywood/OSB Gussets (each face) with 6-6d nails per gusset or LTP5 Framing Angles (one face) with 8-8dx1.5" nails per LTP5 under each vertical web at 4' on center. Where there is no gap between BC of cap truss and TC of base truss, attach BC of cap truss to the TC of base truss with 7"x7"x7/16" Plywood/OSB Gussets (each face) with 6-6d nails per gusset or LTP4 Framing Angles (one face) with 12-8dx1.5" nails per LTP4 under each vertical web at 4' on center. Note: spacing for vertical webs must not exceed 4' on center.

Bracing for the Base Truss Flat Top Chord in a Piggyback Assembly

The flat top chords of the supporting base trusses must be adequately braced to prevent them from buckling out from under the cap trusses. One option for accomplishing this is with flat 2x4 purlins in combination with diagonal bracing that gets repeated at max. 10' intervals. Other methods may be required as specified in the Construction Documents. See BCSI-B3 for additional information.

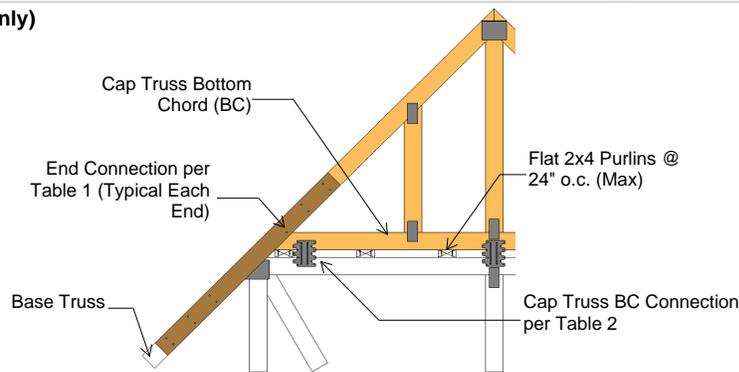
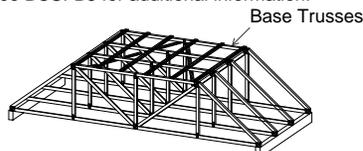


Figure 1 - Typical Piggyback Assembly with 1 1/2" Gap Between Cap and Base

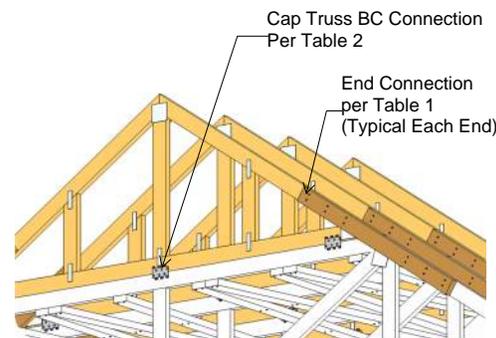
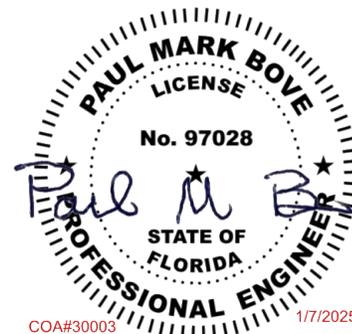
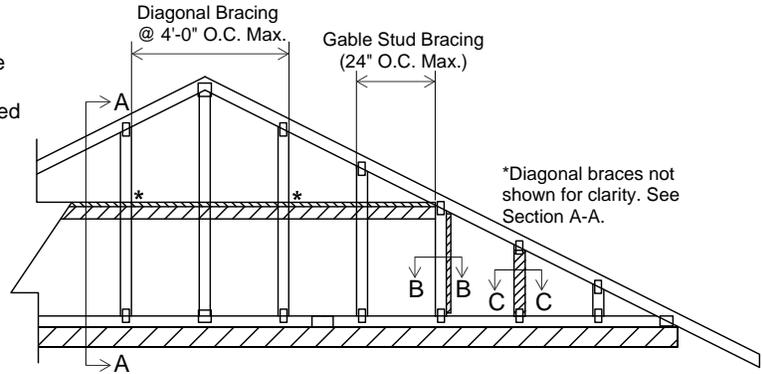


Figure 2 - Typical Piggyback Assembly with No Gap Between Cap and Base



NOTES:

1. This detail provides bracing/reinforcement options for the gable studs to resist the out-of-plane wind loading. Refer to the individual truss design drawing for bracing/reinforcement requirements for resisting the vertical (in-plane) loads assumed in the design of the gable end frame. Additional bracing/reinforcement at the end of the building and/or at the gable end wall may be required. Refer to the Building Designer/Construction Documents for all gable end frame and roof system bracing requirements. For additional information, see BCSI-B3.
2. This detail does not apply to structural gables.
3. Connection requirements between the gable end frame and the wall to be specified by the Building Designer.
4. The gable end frame must match the profile of the adjacent trusses. Do not use a gable end frame with a flat bottom chord next to trusses with sloped bottom chords, such as scissor or vaulted trusses.



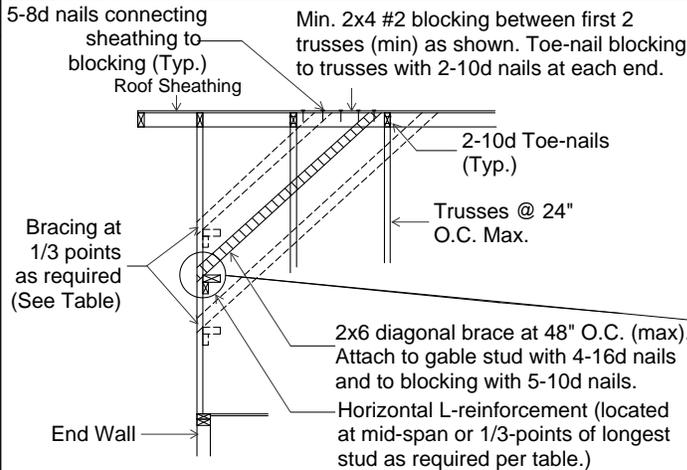
GABLE END WITH STUD BRACING/REINFORCEMENT

MINIMUM GABLE STUD SIZE, SPECIES & GRADE	MAX. GABLE STUD SPACING	WITHOUT BRACE	L-REINFORCEMENT	SCAB REINFORCEMENT	DIAGONAL BRACING @ MID-SPAN ²	DIAGONAL BRACING @ 1/3 POINTS ²	MAXIMUM STUD LENGTH ³	
2X4 SPF STUD or STANDARD	12" O.C.	4-6-0	7-11-4	9-0-4	9-0-4	13-6-8		
	16" O.C.	4-1-0	7-0-4	8-2-4	8-2-4	12-3-8		
	24" O.C.	3-5-8	5-8-12	6-11-0	6-11-0	10-4-8		

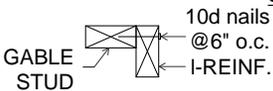
DETAIL LIMITATIONS:

Max. Mean Roof Height: 30'
Category: II
Exposure: B or C
Load Duration Factor: 1.6
Wind Speed: 110 mph Nominal
(140 mph Ultimate)

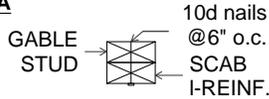
1. L- and Scab Reinforcements shall be minimum 2x4 stud grade and must be a minimum of 90% of the gable stud length. Fasten the reinforcement member to the gable stud with 10d nails @ 6" o.c.
2. Attach horizontal reinforcing member at mid-span (or 1/3 points as required) of the longest stud and install diagonal bracing @ 4' o.c. (max) as shown in Section A-A.
3. Tabulated maximum stud lengths are based on components and cladding wind pressures using the wind design parameters listed in the detail limitations. Gable stud deflection criteria is L/240.



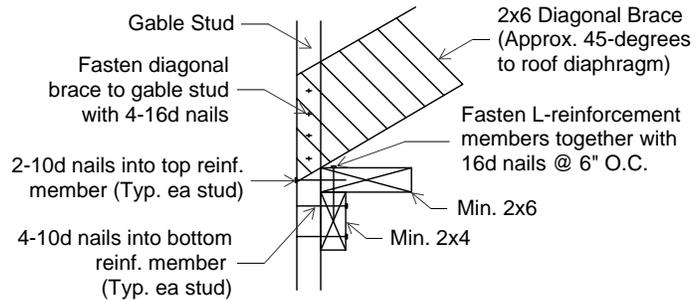
Section A-A



Section B-B



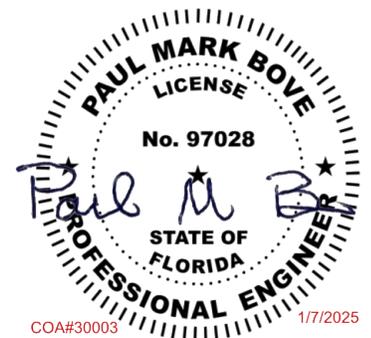
Section C-C



NOTE: Diagonal braces over 6'-3" require a 2x4 T-brace attached to one narrow edge. Diagonal braces over 12'-6" require 2x4s attached on both narrow edges. The braces must cover 90% of the diagonal brace and shall be fastened to the narrow edge with 10d nails at 6" o.c. (min. 3" end distance). When attached on both narrow edges, stagger the nails on each side by 3".

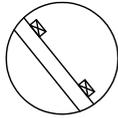
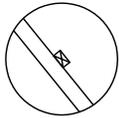
Nail Dimension

16d = 3.5" x 0.162"
10d = 3" x 0.148"
8d = 2.5" x 0.131"

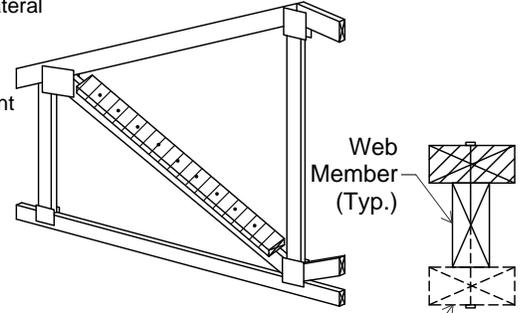


NOTES:

1. This detail provides web reinforcement options that may be used as an alternative to continuous lateral restraint (CLR) when installing CLR's in combination with diagonal bracing is not practical or desired.
2. Refer to the truss design drawing for web lateral restraint requirements. A  on the truss design drawing indicates that continuous lateral restraint is required at the locations shown (either at the midpoint or 1/3-points of the web member). Refer to the tables below for acceptable web reinforcement options that may be used in place of one or two rows of CLR.
3. T-, L-, I- and scab web reinforcements must be the same or better species and grade of the web member as indicated on the truss design drawing.
4. All reinforcements must extend to within 6" of each end of the web member.
5. This detail does not apply to single-ply webs that exceed 14' in length.

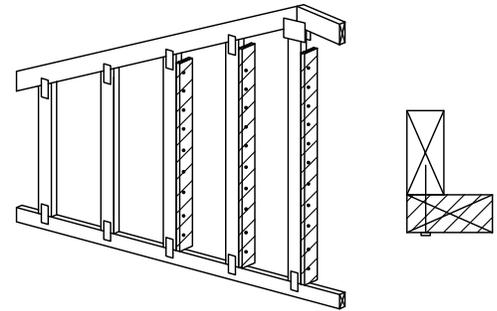


1 Row of CLR @ Web Mid-point 2 Rows of CLR's @ Web 1/3 points

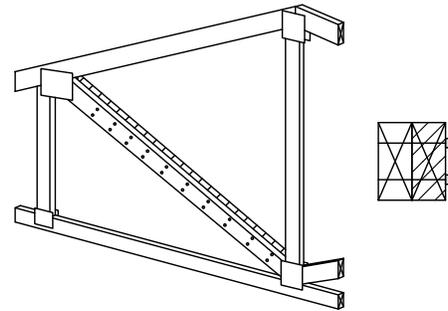


Add member to both edges for I-Reinforcement

T- Reinforcement
(I-Reinforcement similar)



L- Reinforcement



Scab Reinforcement

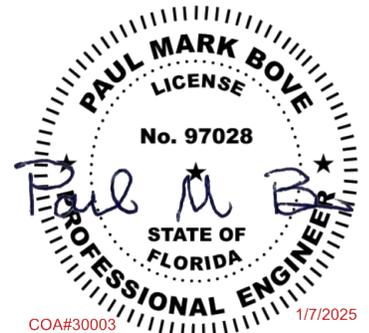
WEB REINFORCEMENT OPTIONS FOR SINGLE-PLY TRUSSES ¹						
Specified Web Member Lateral Restraint (CLR's)	Web Member Size	Acceptable Web Reinforcement Substitutions - Type & Size				Reinforcement-to-Web Connection Requirements
		T-	L-	Scab	I-	
1 Row @ Mid-point	2x4	2x4	2x4	2x4	---	16d gun nails @ 6" on-center
	2x6	2x6	2x6	2x6	---	
	2x8	2x8	2x8	2x8	---	
2 Row @ 1/3-points	2x4	No substitutions allowed			2-2x4	
	2x6	No substitutions allowed			2-2x6	
	2x8	No substitutions allowed			2-2x8	

WEB REINFORCEMENT OPTIONS FOR 2-PLY TRUSSES ²						
Specified Web Member Lateral Restraint (CLR's)	Web Member Size	Acceptable Web Reinforcement Substitutions - Type & Size				Reinforcement-to-Web Connection Requirements
		T-	L-	Scab	I-	
1 Row @ Mid-point	2x4	2x4	2x4	---	---	16d gun nails @ 6" on-center
	2x6	2x6	2x6	---	---	
	2x8	2x8	2x8	---	---	
2 Row @ 1/3-points	2x4	No substitutions allowed			2-2x4	
	2x6	No substitutions allowed			2-2x6	
	2x8	No substitutions allowed			2-2x8	

1. The maximum allowable web length for single-ply trusses is 14'.
2. For 2-ply trusses, the reinforcement must be nailed to both plies of the web with the nailing pattern specified in the table.
3. For the scab reinforcement, 2 rows of 10d gun nails @ 6" o.c may be used in place of 16d gun nails for attaching the reinforcement to the web.
4. For I-reinforcement, attach each 2x_member to opposite edges of the web using the nailing pattern specified in the table.

Nail Dimension

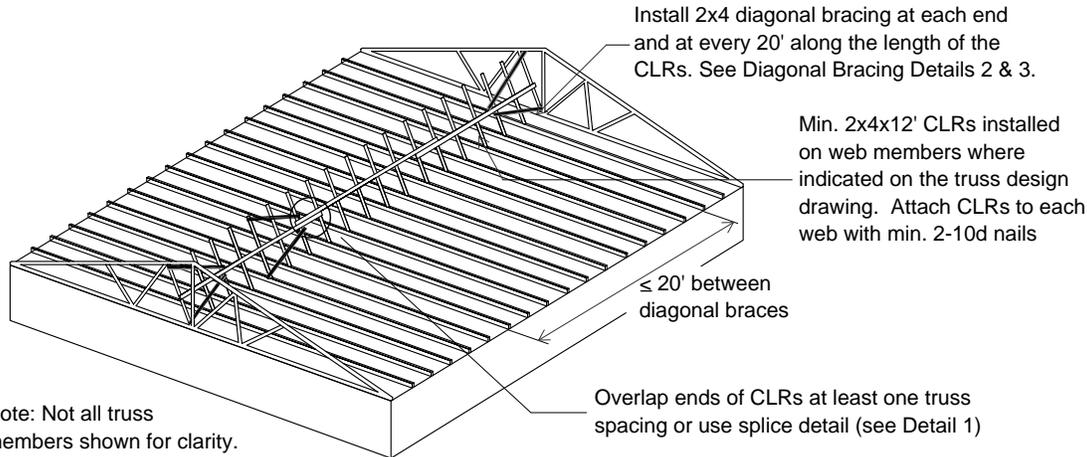
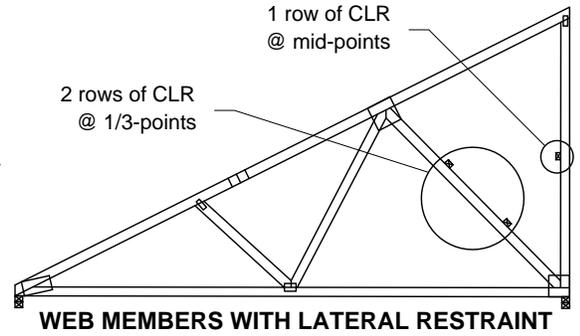
16d = 3.5" x 0.131"
10d = 3" x 0.120"



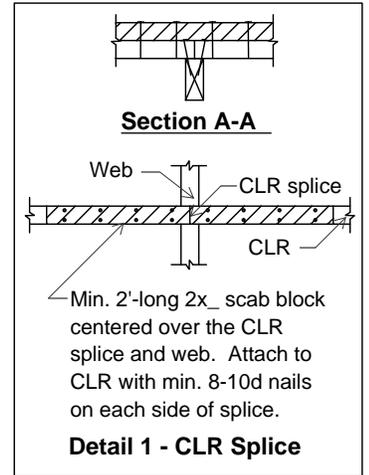
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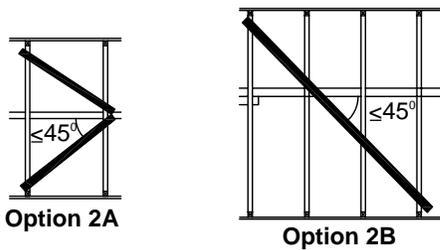
This detail provides information for laterally restraining and bracing web members to prevent lateral buckling using continuous lateral restraints (CLRs) in combination with diagonal bracing. In addition to the CLRs indicated on the truss design drawing, diagonal bracing must be installed as indicated in this detail and BCSI-B3. See WEBREINFORCE for web reinforcement options that may be used as an alternative to this detail when installing CLRs and diagonal bracing is not practical or desired. Properly attached full-length sheathing satisfies (may replace) any bracing requirements specified for end vertical webs. Refer to the Construction Documents for additional bracing requirements. For trusses with spacing greater than 2' o.c. refer to BCSI-B10



Note: Not all truss members shown for clarity.



For webs with one row of CLRs, diagonal bracing shall be installed using Option 2A or 2B. Attach diagonal braces to each truss with min. 2-10d nails.



Detail 2 - Diagonal Bracing for 1 Row of CLRs

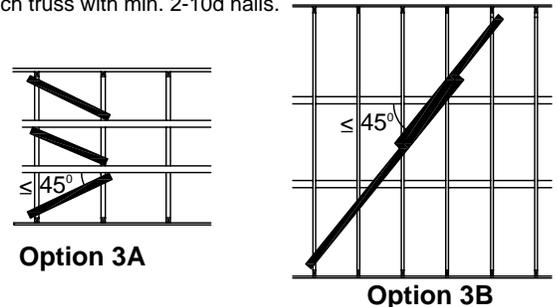
DETAIL LIMITATIONS:

1. Restraint and Bracing Material min. 2x4 stress graded lumber.
2. This detail does not address permanent building stability bracing to resist lateral forces acting on the building.
3. This detail shall not supersede any project-specific truss member permanent bracing design for the roof framing structural system.
4. This detail is not applicable for trusses with spacing greater than 2' o.c.

Nail Dimensions:

10d = 3" x 0.128"

For webs with 2 rows of CLRs, diagonal bracing shall be installed using Option 3A or 3B. Attach diagonal braces to each truss with min. 2-10d nails.



Detail 3 - Diagonal Bracing for 2 Rows of CLRs

