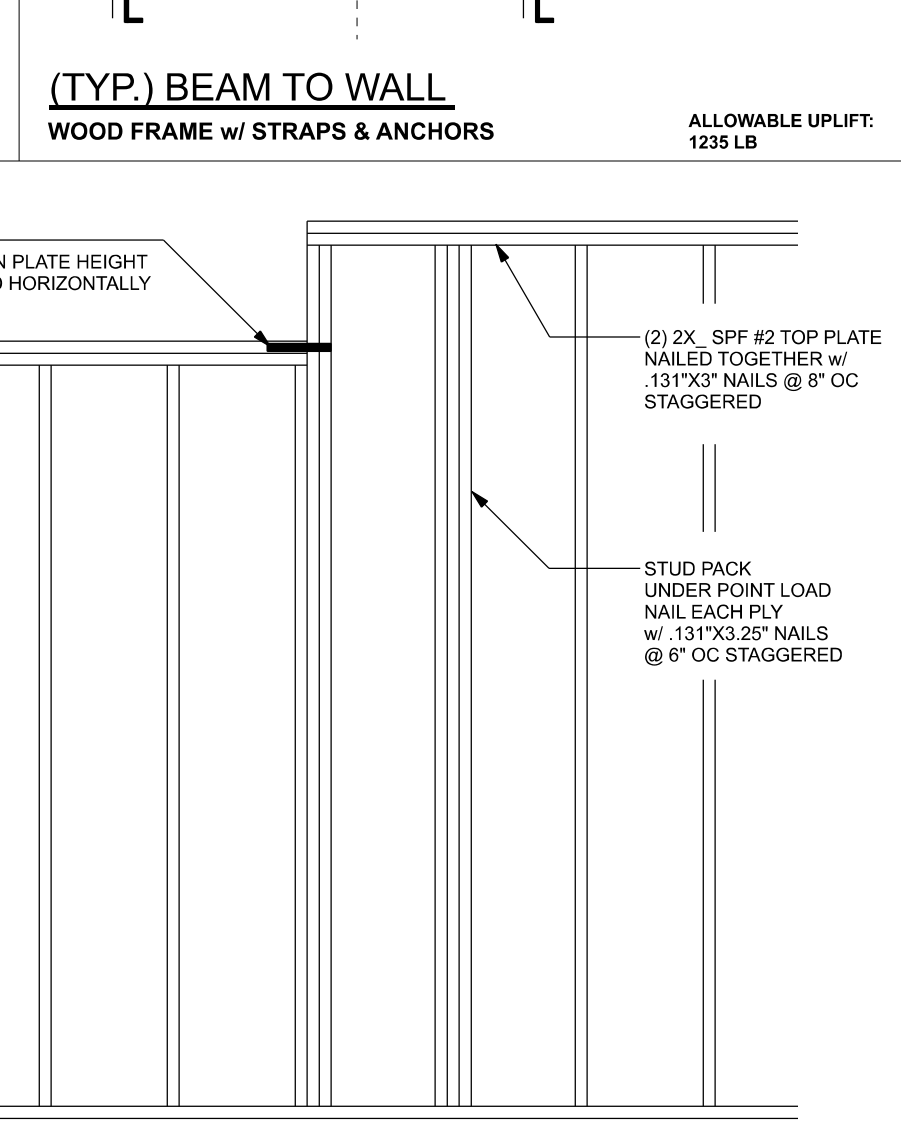
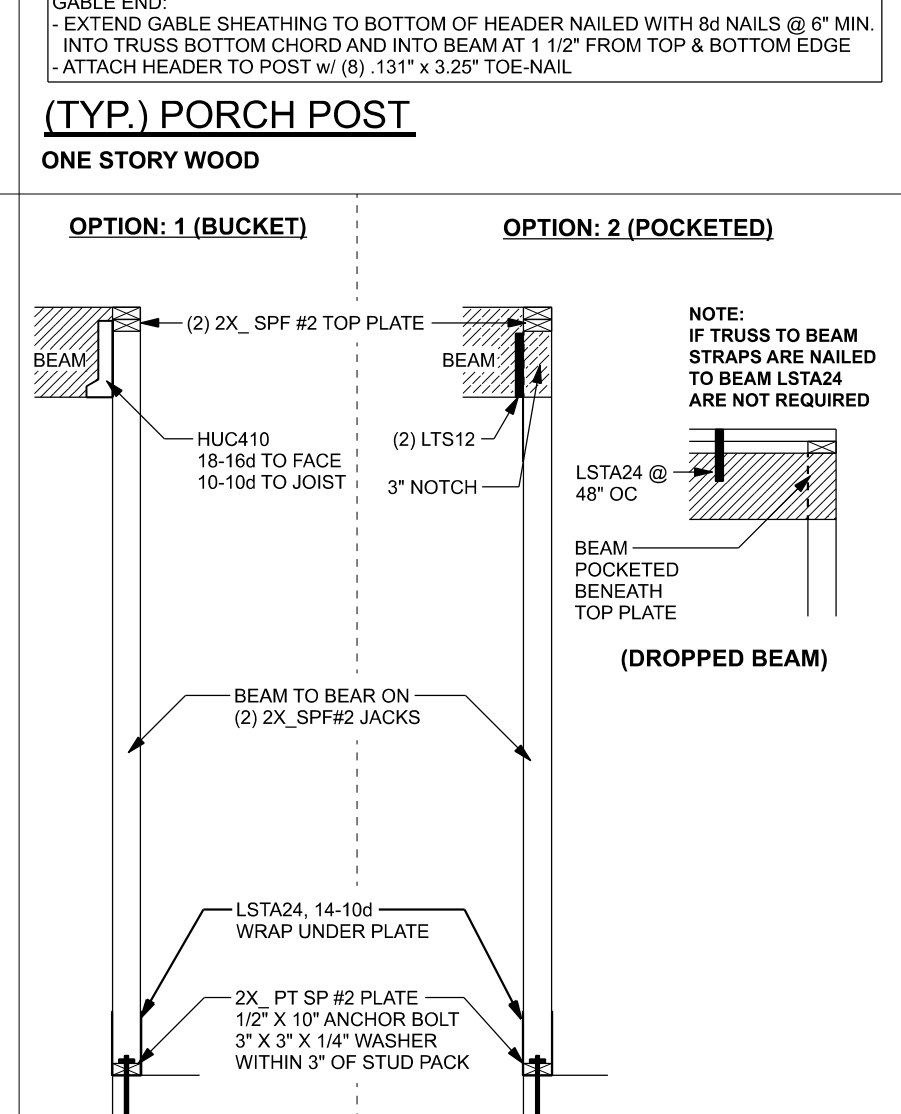
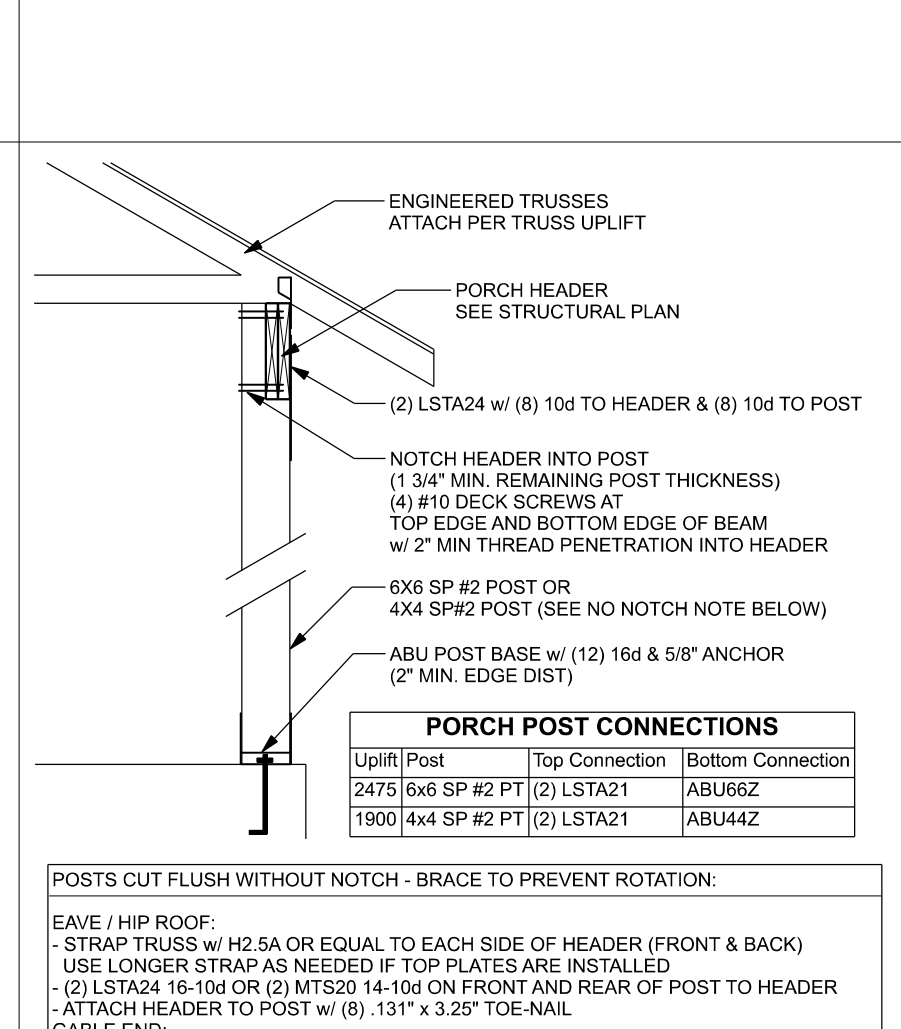
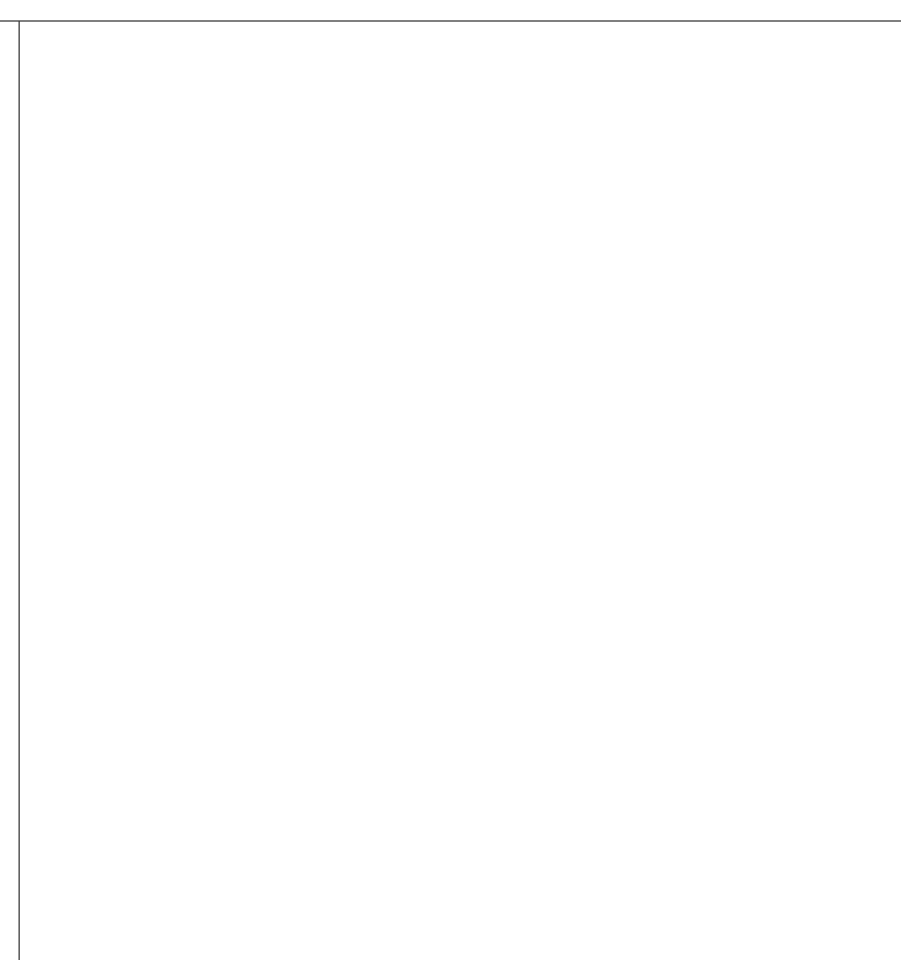
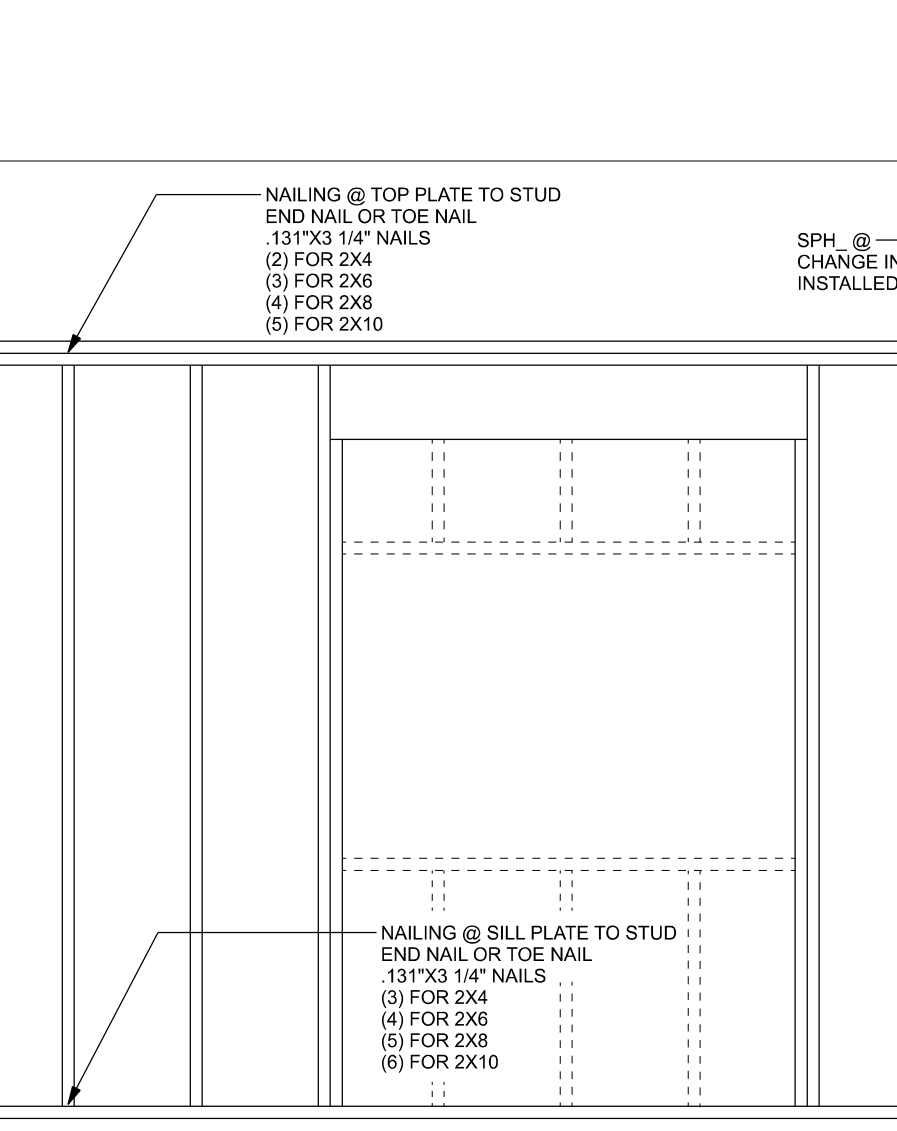
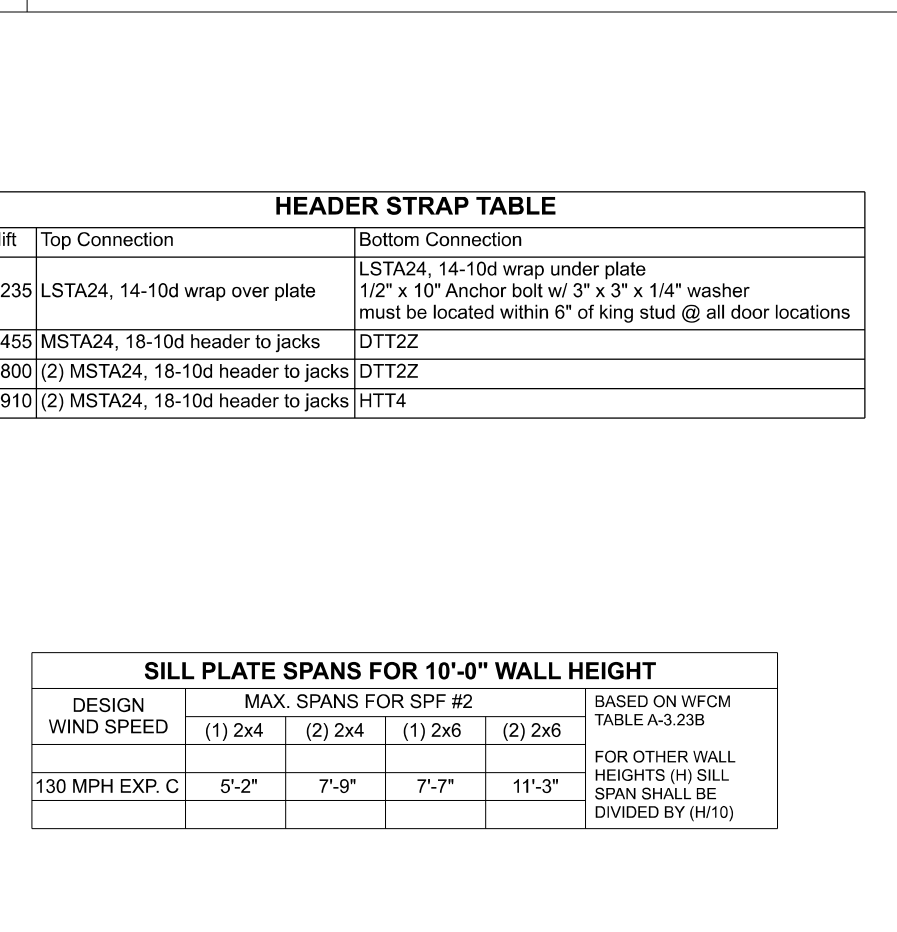
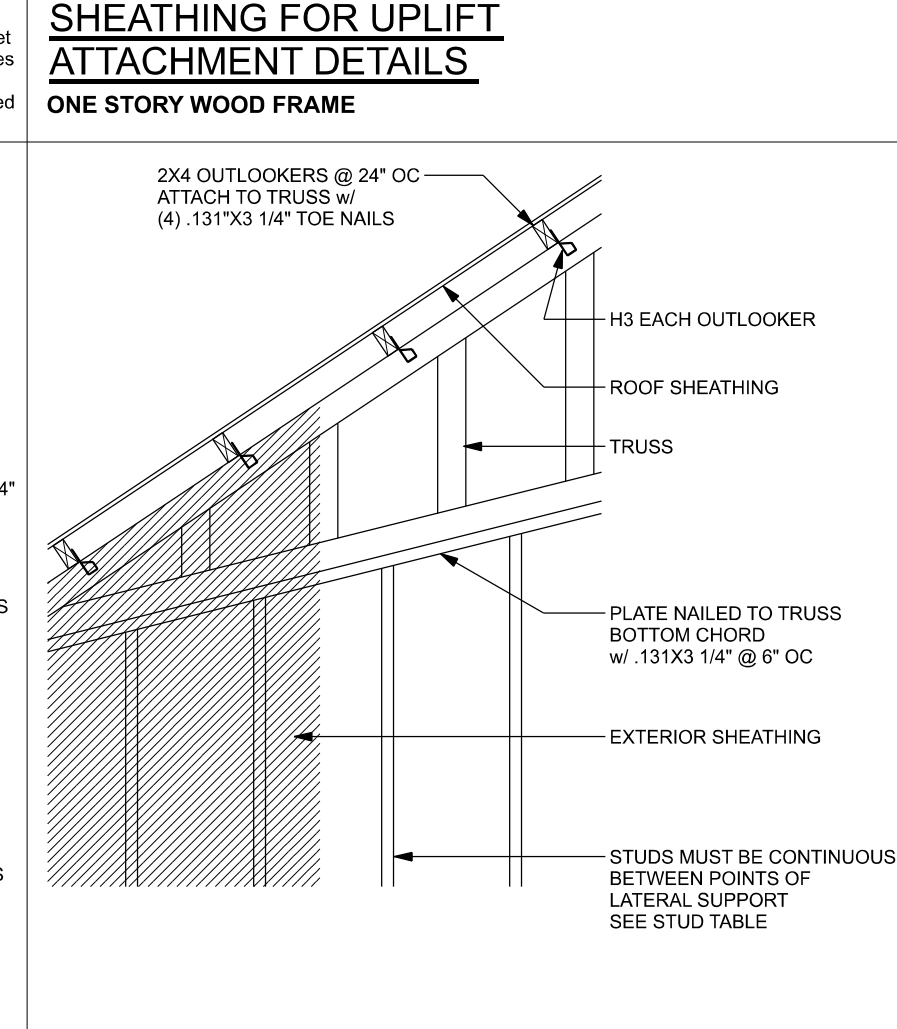
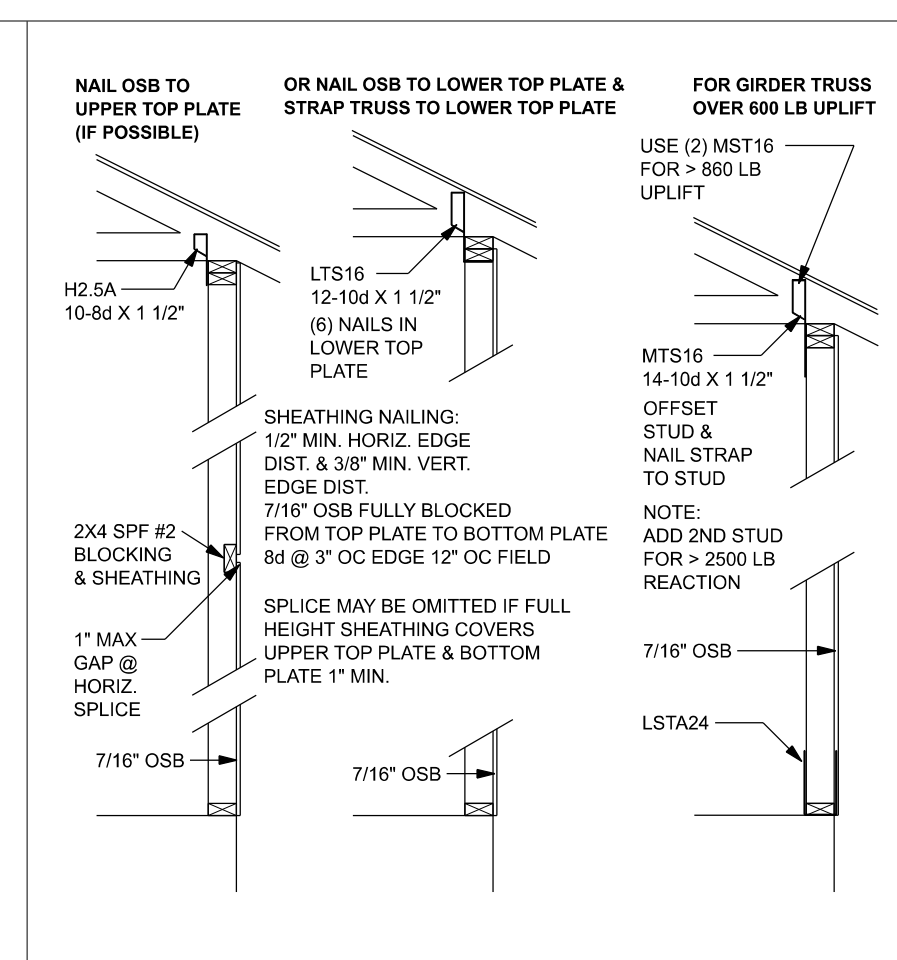
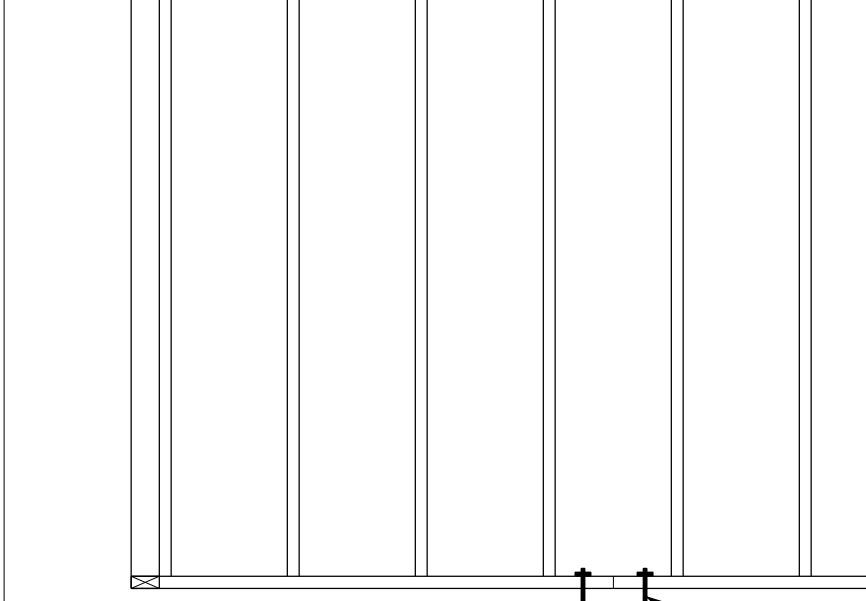
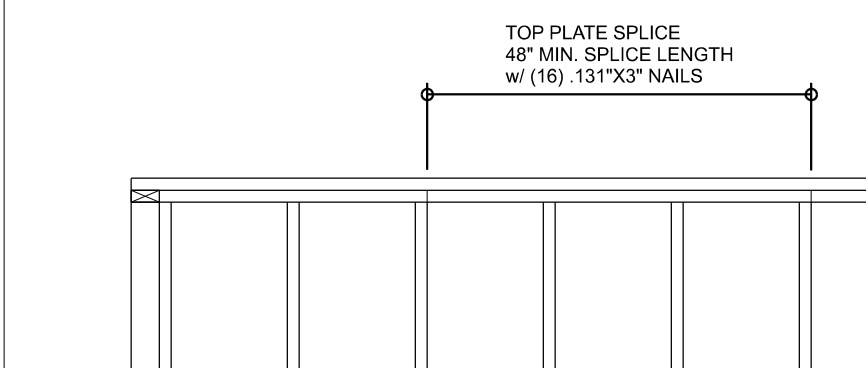
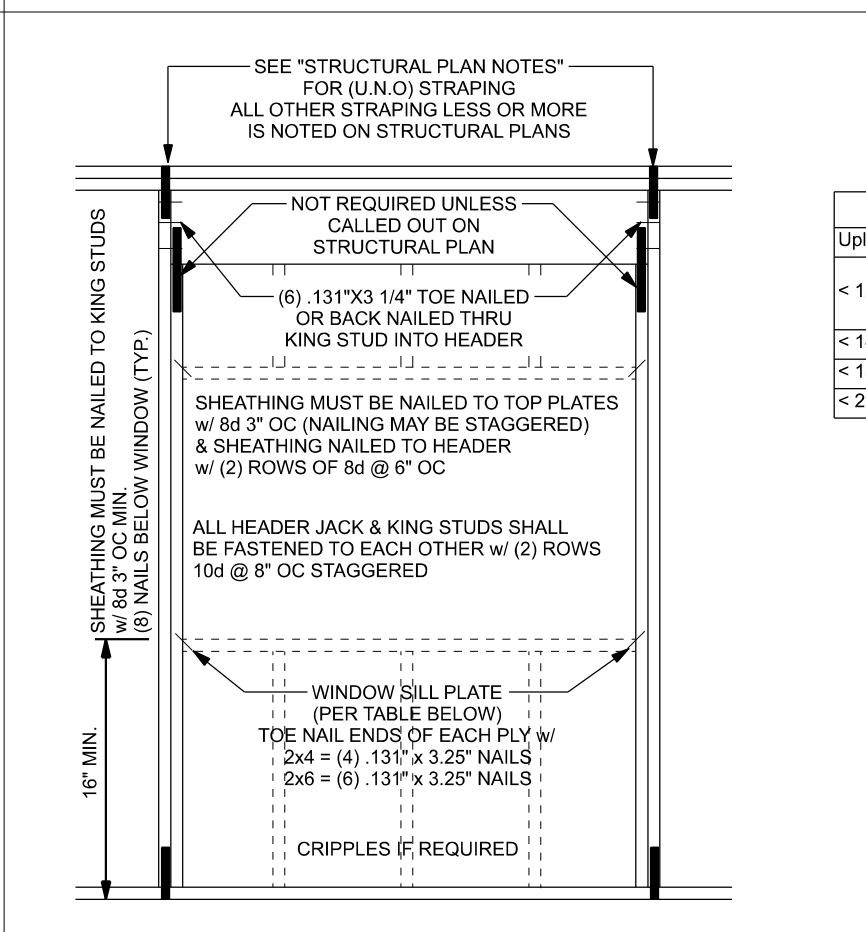
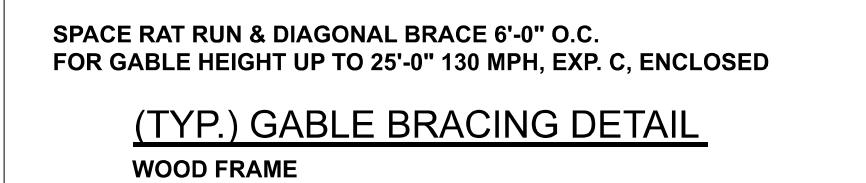
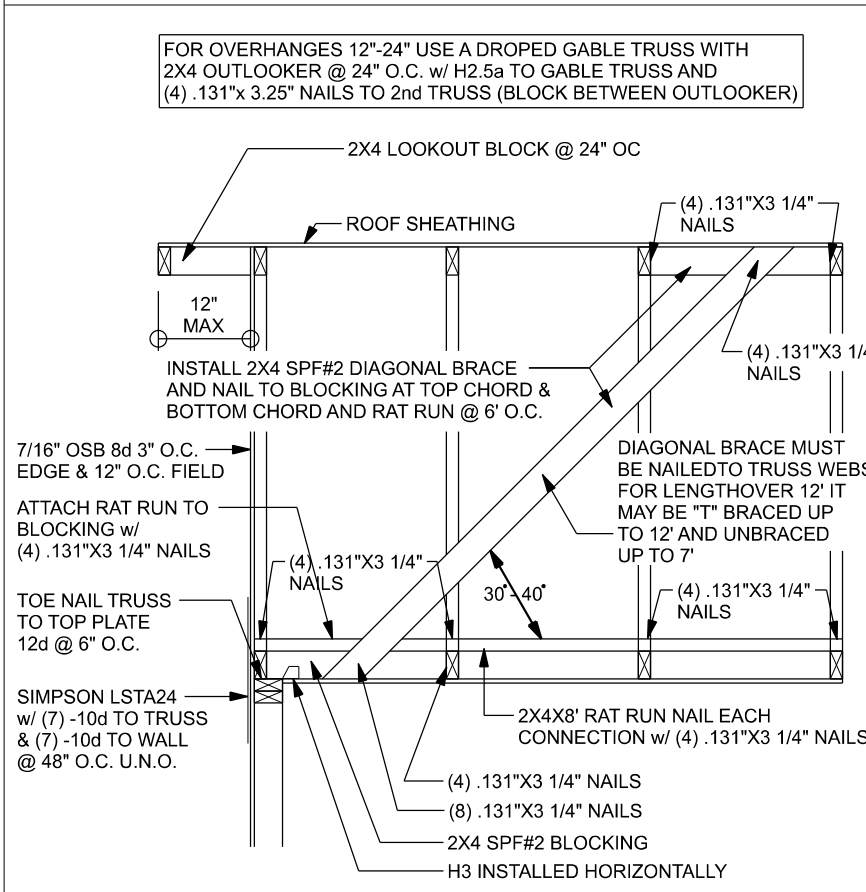


ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSR5-01 (2 3/8" x 0.131")	6" oc	6" oc
120 mph Exp. C	7/16"	ASTM F1667 RSR5-01 (2 3/8" x 0.131")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RSR5-03 (2 1/2" x 0.131") or ASTM F1667 RSR5-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSR5-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RSR5-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RSR5-03 (2 1/2" x 0.131") or ASTM F1667 RSR5-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSR5-01 (2 3/8" x 0.131")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSR5-03 (2 1/2" x 0.131") or ASTM F1667 RSR5-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RSR5-03 (2 1/2" x 0.131") or ASTM F1667 RSR5-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RSR5-03 (2 1/2" x 0.131") or ASTM F1667 RSR5-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RSR5-03 (2 1/2" x 0.131") or ASTM F1667 RSR5-04 (3" x 0.120")	4" oc	4" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



CONNECTOR TABLE

Uplift SP Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	
415	290	H3	4-8x1 1/2"
575	485	H2.5A	5-8x1 1/2"
1340	1015	H10A	9-10x1 1/2"
720	620	LTS12-20	6-10x1 1/2"
1000	860	MTS12-30	7-10x1 1/2"
1450	1245	HTS20-30	12-10x1 1/2"
Uplift SP Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA31	8-10x1
1640	1455	MSTA24	9-10x1
1030	1030	CS20	7-10x1
Uplift SP Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10x1
1085	865	SP2	6-10x1
771	771	LSTA24	10-10x1
1235	1235	LSTA24	14-10x1
Uplift SP Uplift SPF	Holdowns @ Stenwall	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"
4235	3640	HTT4	18-16x2 1/2"
Uplift SP Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"
4235	3640	HTT4	18-16x2 1/2"
Uplift SP Uplift SPF	Post Bases @ Stenwall	To Post	Anchor
1900	ABU44Z	12-16x1	5/8"x12" Drill & Epoxy
2475	ABU66Z	12-16x1	5/8"x12" Drill & Epoxy
Uplift SP Uplift SPF	Post Bases @ Mono	To Post	Anchor
1900	ABU44Z	12-16x1	5/8"x7" Drill & Epoxy
2475	ABU66Z	12-16x1	5/8"x7" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE

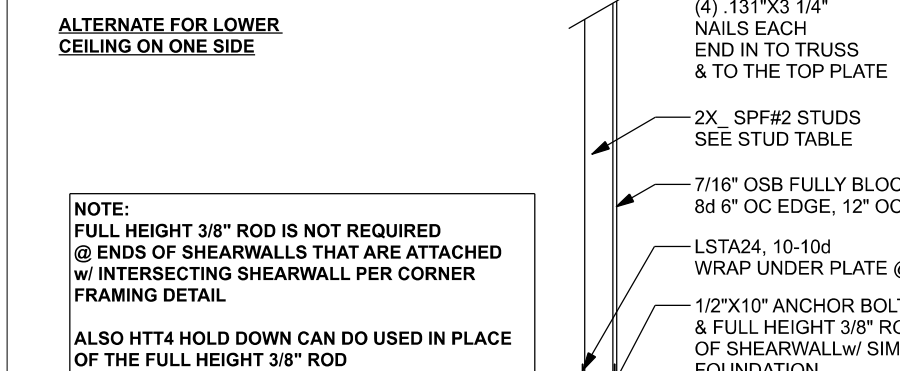
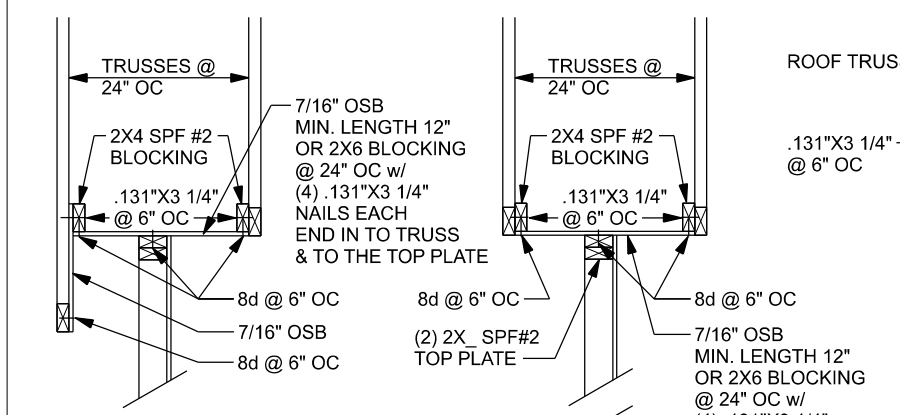
	Fb	E
2x8 SP #2	925	1.4
2x10 SP #2	800	1.4
2x12 SP #2	750	1.4
GLB 24F-V3 SP	2600	1.9
LSL TIMBERSTRAND	1700	1.7
LVL MICROLAM	2950	2.0
PSL PARALAM	2900	2.0

PORCH POST CONNECTIONS

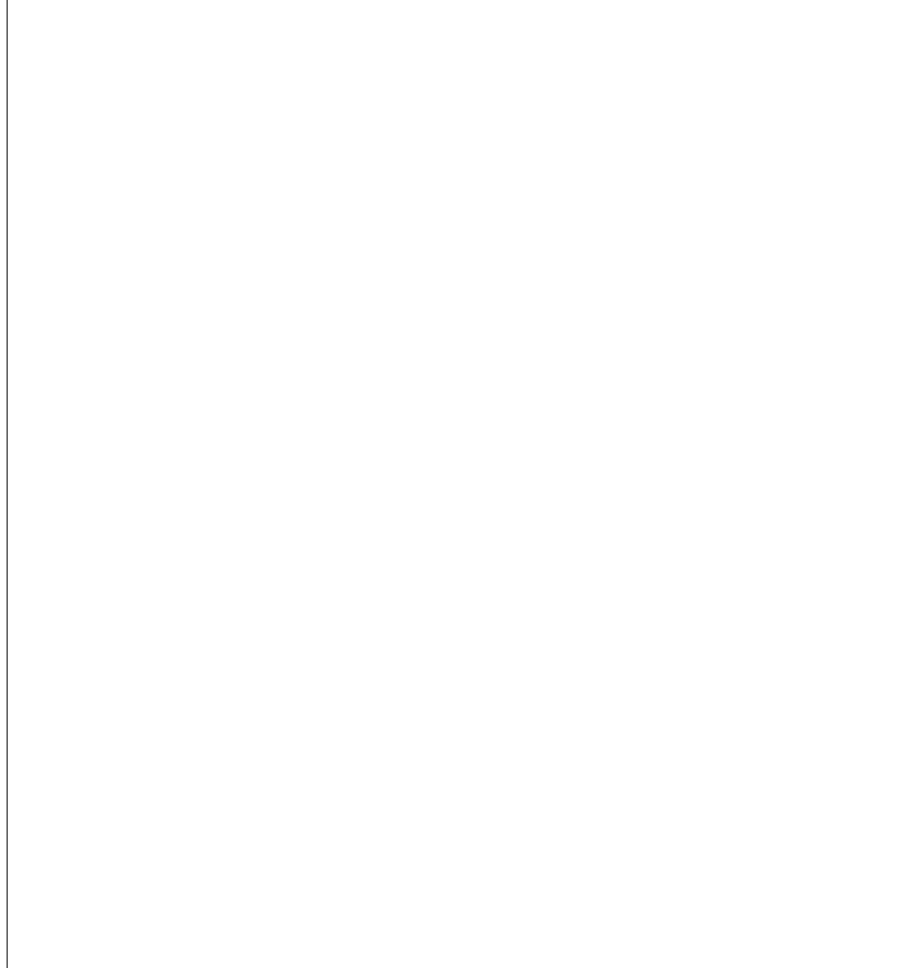
Uplift Post	Top Connection	Bottom Connection
2475 6x6 SP #2 PT (2)	LSTA31	ABU66Z
1900 4x4 SP #2 PT (2)	LSTA21	ABU44Z

OPTION: 1 (BUCKET)

OPTION: 2 (POCKETED)



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF THE TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X4 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB, 6" x 6" W1 x W1.4, FB = 89KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH & WIDTH OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL (RECOMMENDED) LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI. ALL LAP SPACES 40" DB (25" FOR 45 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILD AND PLACED IN ACCORDANCE WITH ACI 318.6, R11.0.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROUTED CMU.

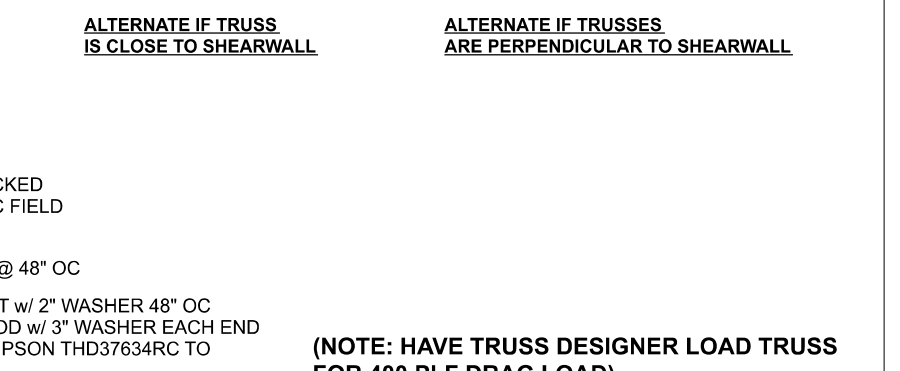
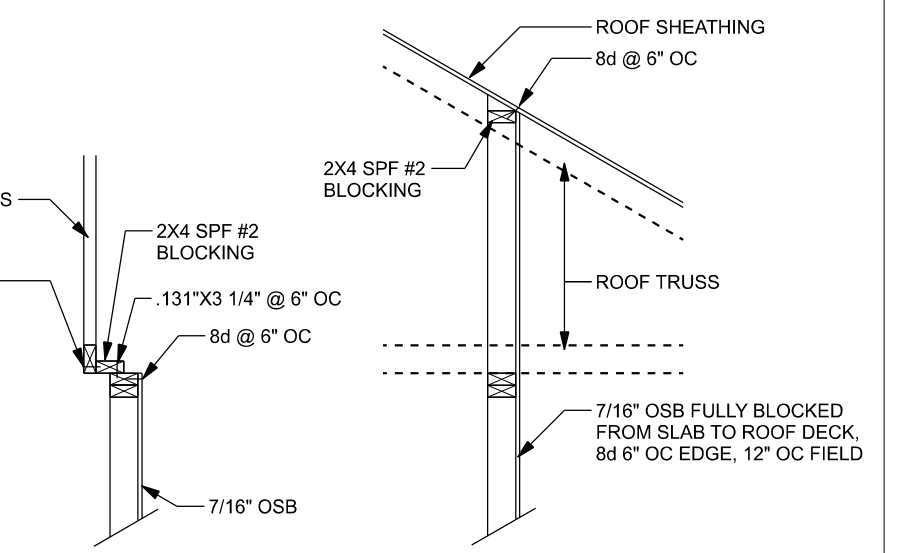
BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK:

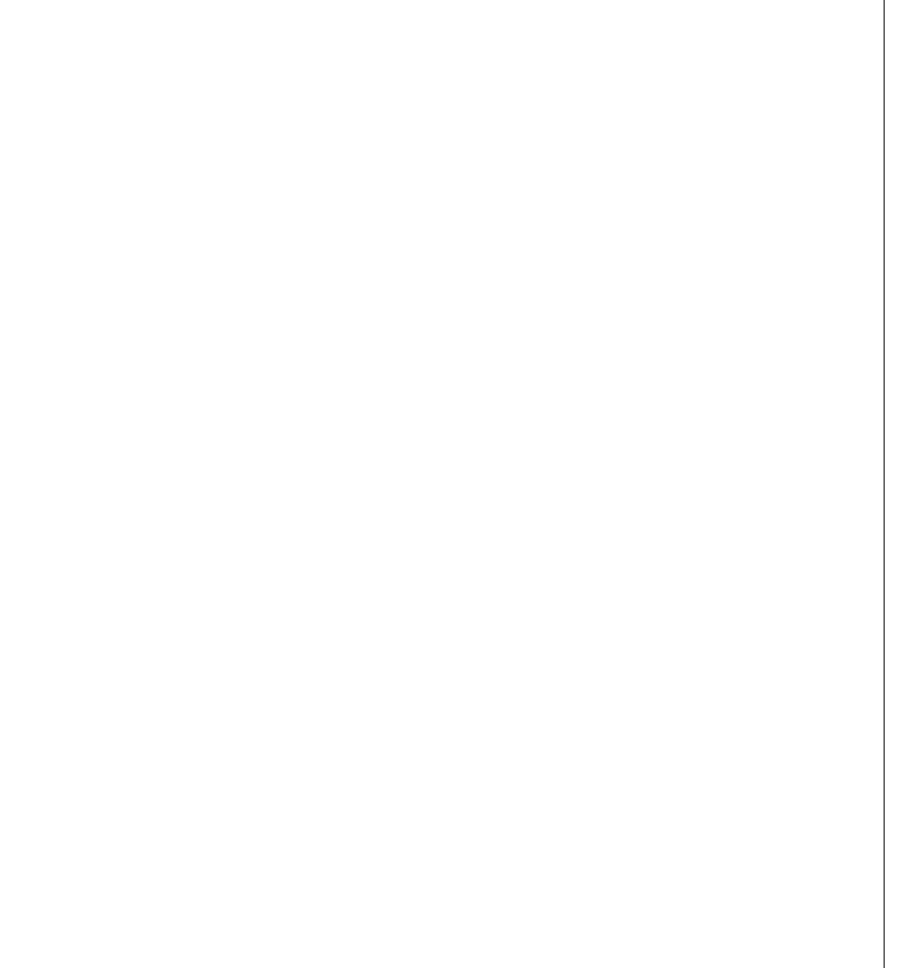
- CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND ELEVATION, HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
- PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THIS STATED WIND VELOCITY AND DESIGN PRESSURES.
- PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
- VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



DESIGN CRITERIA & LOADS:

BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	
BASIC WIND SPEED (BASED 7-16, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&G DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

EFFECTIVE WIND AREA (FT ²)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasd) -27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20	+42.6(Vult) -46.2(Vult)	+42.6(Vult) -57(Vult)

COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C)

Columbia County Building Department
Plans Reviewed for Code Compliance
State of Florida

BRYAN ZECHER CONSTRUCTION
PATTY & CHRIS HETRICK RES.

PROJECT ADDRESS: 3858 W. STATE ROAD LAKE CITY, FLORIDA

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

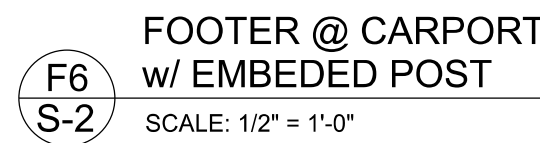
MARK DISOWAY P.E. 53915

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386.754.5419
disowaydesign@gmail.com

JOB NUMBER:

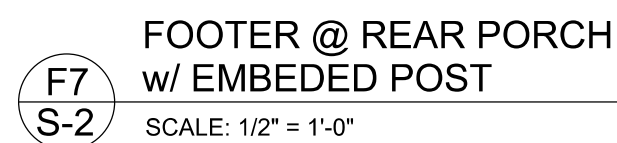
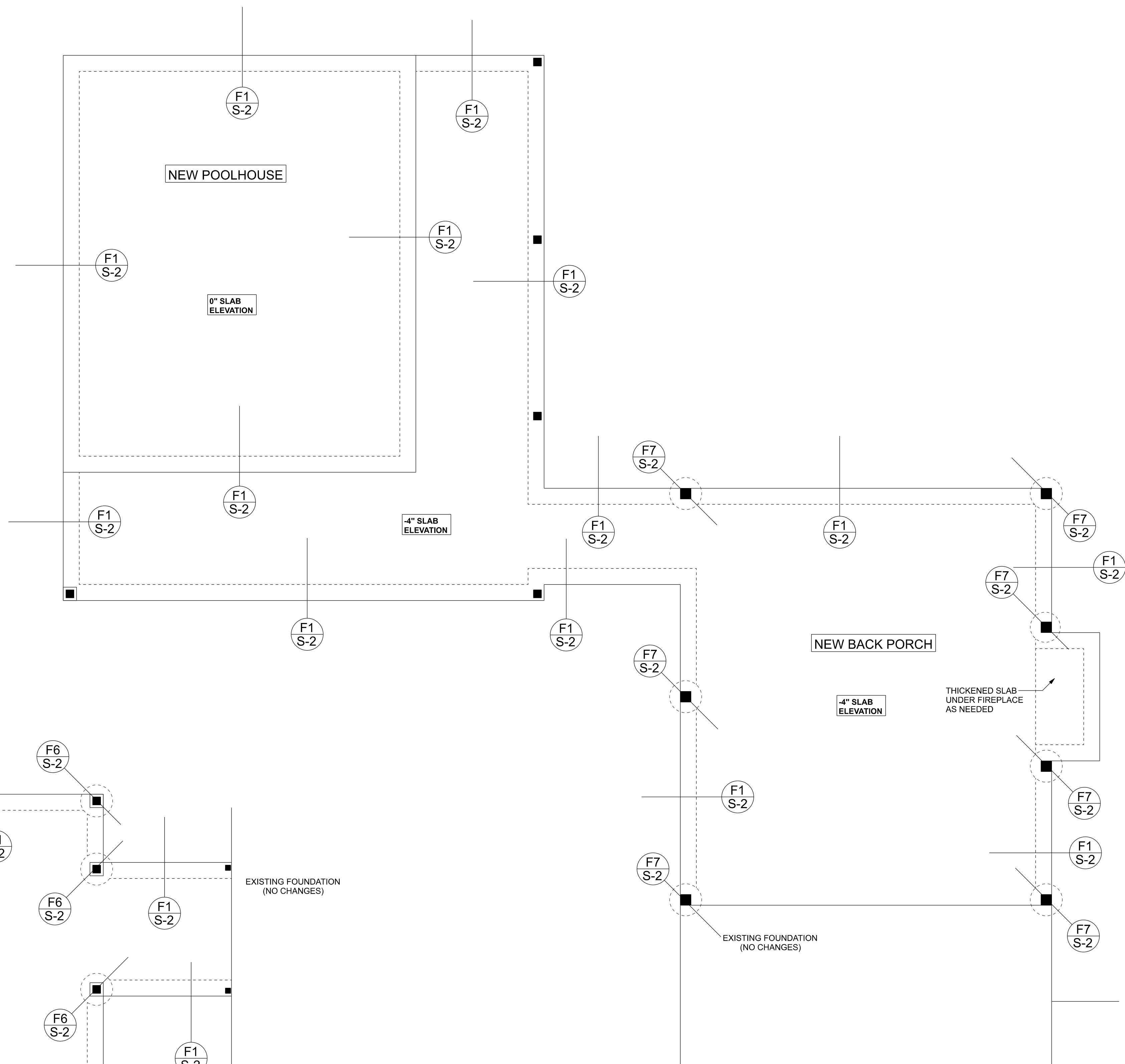
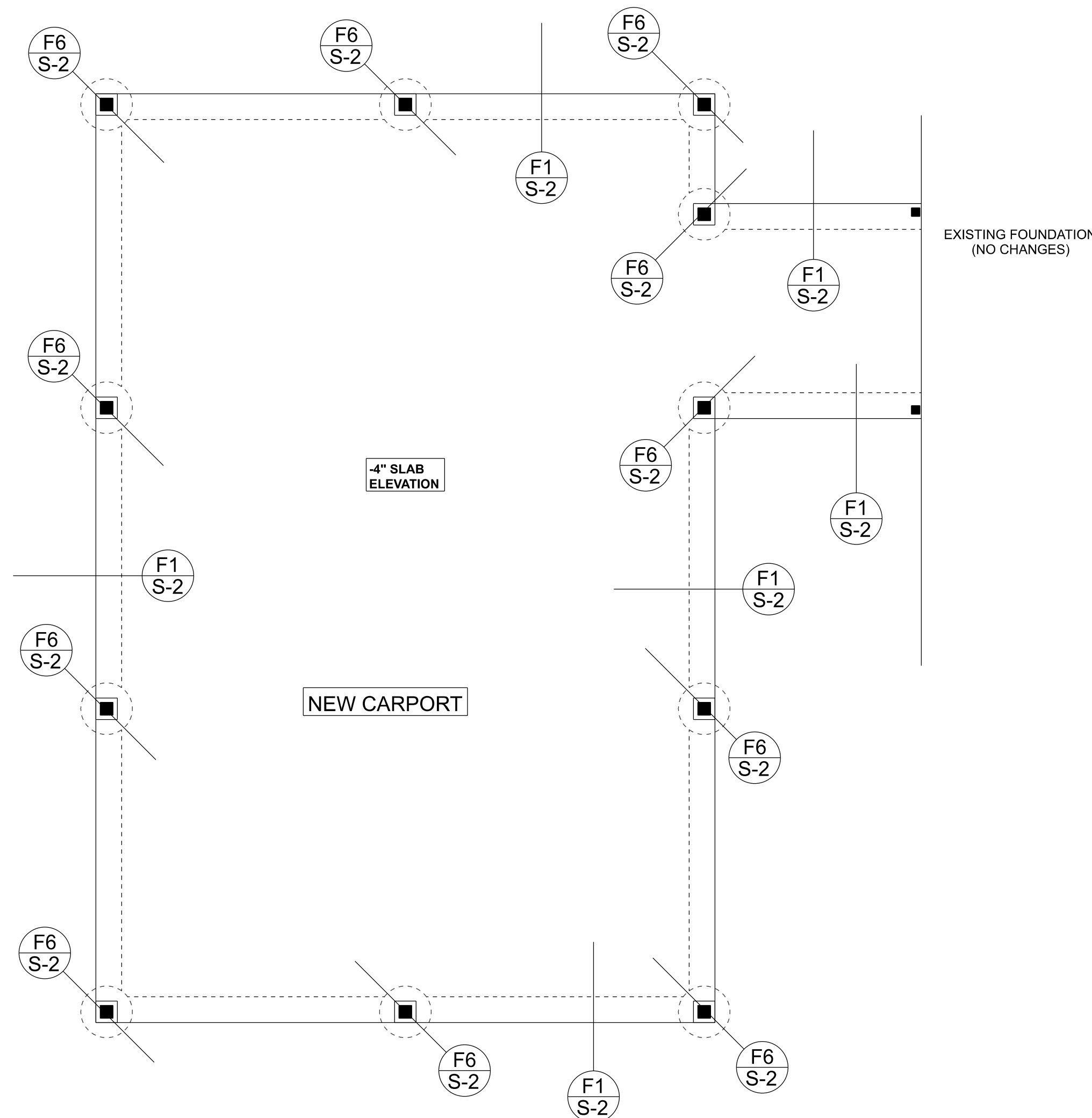
S-1
OF 3 SHEETS



STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

MASONRY NOTE:	
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/MS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE MASONRY OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN ORDINANCES. NO OTHER ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE MASONRY IN WRITING.	
	Specific Requirements
1.4A	ACI308-1.02 Section
2.1	Compressive strength ACI 530.1-02, Type N, UMO
2.1	Mortar ASTM C 270, Type N, UMO
2.2	Grout ASTM C 476, and jointing approval
2.3	CMU standard ASTM C 90-02, Normal weight, Hollow, and surface finish, 8" nominal thickness and 12"x12"x16" or 16"x16"x16" column block
2.3	Grout brick standard ASTM C 1126-02, Grade SW, Type FBS, 5.5"x2.5"x11.5"
2.4	Reinforcing bars, #3 - #11 ASTM 615, Grade 60, Fy = 40 ksi, Lap length min 40 dia bars
2.4F	Coating for corrosion protection Anchors, steel metal ties completely embedded in mortar or grout, ASTM A655, Class GB6, 0.60 oz/ft ² 304SS
2.4F	Coating for corrosion protection Joint reinforcement in walls exposed to moisture or water ties, anchors, steel metal ties not completely embedded in mortar or grout, ASTM A153, Class B, 1.50 oz/ft ² 304SS
3.3.E.2	Pipes, conduits, and accessories Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints Anchor movement responsibility for type and location of movement joints if not shown on project drawings.

**BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF
12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL**



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT REFERENCE TO ARCHITECTURAL PLANS. FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISPOWAY DESIGN GROUP OR MASTER DISPOWAY, INC. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
FN - 3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/ 6# WELDED WIRE MESH, 12" MIN. THICKNESS, 3/8" @ 12" DEPTH OR FIBER MESH CONCRETE, 6 MIL POLY VAPOR BARRIER W/ 6" EPS SEALED W/ POLY TAP OVER TERMITE-TREATMENT & COMPACTED FILL (ALL) (ANY OTHER UNAPPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD).

PATTY & CHRIS
HETRICK RES.

PROJECT ADDRESS:
1897 SW PINEMOUNT RD
LAKE CITY, FLORIDA

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

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LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915
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Thursday, October 28, 202

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JOB NUMBER:

S-2

OF 3 SHEETS



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. W.B. HOLLAND COMPANY
JOB #21-5823

S-3
OF 3 SHEET