32639 73

Alpine, an ITW Company

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1VFB9114Z0101165421

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/01/2015

Douglas Fleming
-Truss Design Engineer-

2400 Lake Orange Dr, Suite 150
Orlando FL, 32837

Truss Fabricator:	**Anderson Truss Company**
Job Identification:	**REPAIR / 14-215J -B /RES FOR (REPAIR / 14-215J-)**
Truss Count:	**1**
Model Code:	**Florida Building Code 2014 or 2010**
Truss Criteria:	**FBC2010Res/TPI-2007(STD)**
Engineering Software:	**Alpine Software,Version 14.03.**
Structural Engineer of Record:	**The identity of the structural EOR did not exist as of**
Address:	**the seal date per section 61G15-31.003(5a) of the FAC**
Minimum Design Loads:	**Roof - 37.0 PSF @ 1.25 Duration**
	Floor - N/A
	Wind - 130 MPH ASCE 7-10 -Closed

Notes:

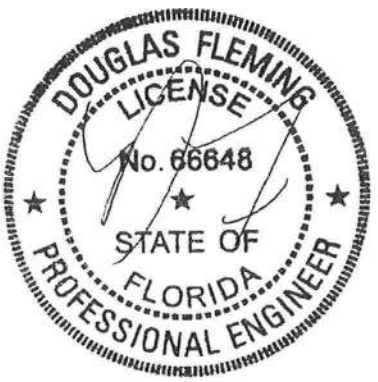
1. **Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1**
2. **The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.**
3. **As shown on attached drawings; the drawing number is preceded by: HCUSR9114**

Details: -

#	Ref	Description	Drawing#	Date
1	04668--B	39' Common	15091001	04/01/15

Alpine, an ITW Company

2400 Lake Orange Drive suite 150 Orlando FL 32837
 Document ID:1VFB9114Z0101165421



Truss Fabricator:	**Anderson Truss Company**
Job Identification:	**REPAIR / 14-215J -B /RES FOR (REPAIR / 14-215J-)**
Truss Count:	**1**
Model Code:	**Florida Building Code 2014 or 2010**
Truss Criteria:	**FBC2010Res/TPI-2007(STD)**
Engineering Software:	**Alpine Software,Version 14.03.**
Structural Engineer of Record:	
Address:	
Minimum Design Loads:	**Roof - 37.0 PSF @ 1.25 Duration** **Floor - N/A** **Wind - 130 MPH ASCE 7-10 -Closed**

04/01/2015

Notes:

1. **Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1**
2. **The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.**
3. **As shown on attached drawings; the drawing number is preceded by: HCUSR9114**

-Truss Design Engineer-
Douglas Fleming

1950 Marley Drive
Haines City, FL 33844

Revised Trusses

#	Ref	Description	Drawing#	Date
1	04668--B	39' Common	15091001	04/01/15

(REPAIR / 14-215J -B /RES FOR - B 39' Common)

This truss is repaired for the addition of a 1-9-4 flat Bottom Chord section at the left and right ends of truss and to change the bearing elevation to 23-2-0 as shown..

Refer to drawing HCUSR9114 15057005 for plates and other data not given here.

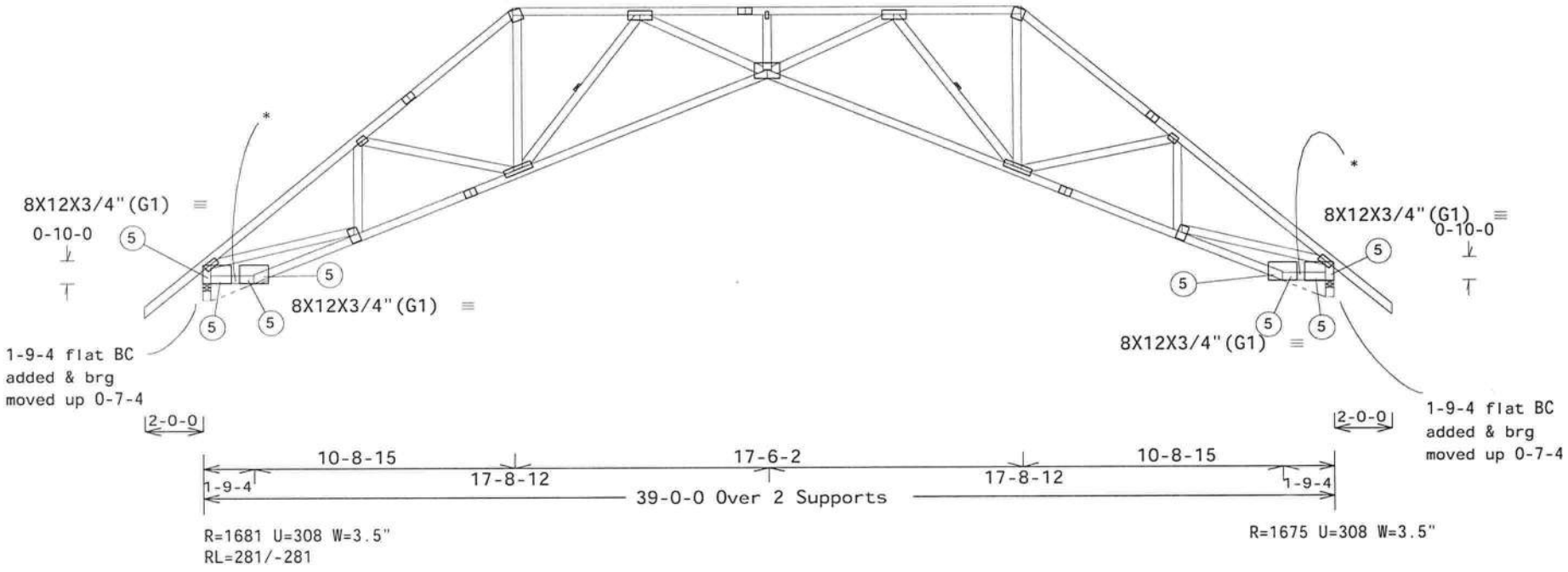
Repair(s) must comply with Alpine designs & specifications

Shore Truss and any supported spans in proper position as repair is being made.

* 2x4 SP_#1_13B FIELD-INSTALLED CUT-TO-FIT MEMBER REQ'D. (2 new member(s) required.)

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(G1) Gusset Plates are 3/4" APA RATED SHEATHING, 48/24, EXP 1 or 2. Apply gusset to each face of truss and attach with evenly distributed 0.099"x2.0" Nails specified in circles. Hatched lines indicate portions on gussets protruding outside of the perimeter of the truss that may be trimmed flush with the truss profile.
Minimum Nail/Screw Spacing Requirements Based on ANSI/AF&PA NDS-2001:
End Distance 1-1/2" Edge Distance 1/2"
Spacing Between Rows 1/2" Spacing in a Row 1-1/2"
Maximum Number of Rows for Member Size:
2x4 5 Rows



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 14.03.0 QTY:1 FL/-/3/-/-/R/- Scale =.1875"/Ft.

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2400 Lake Orange Dr., Suite 150
Orlando, FL 32837
FL COA #0 278

TRUSS REPAIR

DAMAGED TRUSSES MUST BE CAREFULLY EVALUATED TO DETERMINE THE EXTENT OF DAMAGE AND THE FEASIBILITY OF REPAIR. IN SOME CASES THE PRUDENT SOLUTION IS TO SCRAP THE DAMAGED TRUSSES AND REBUILD. INTERNAL WOOD FIBER DAMAGE AND EXCESSIVE CONNECTOR STRESS FROM BENDING OR SHOCK CANNOT BE READILY DETECTED. THEREFORE, IT IS VITAL THAT THE TRUSS FABRICATOR AND BUILDING CONTRACTOR CONSIDER THE CAUSE OF THE DAMAGE IN THEIR DECISION WHETHER TO REPAIR OR REBUILD.

REPAIR WORK SHOWN ON THIS DRAWING APPLIES ONLY TO THOSE SECTIONS OF THE TRUSS REPORTED BY THE TRUSS MANUFACTURER TO HAVE BEEN DAMAGED. A QUALIFIED THIRD PARTY INSPECTOR SHALL CHECK TRUSSES TO DETERMINE THE EXTENT OF ANY FURTHER DAMAGE, IF ANY, AND VERIFY THAT REPAIRS HAVE BEEN PERFORMED AS INDICATED ON THIS DRAWING.

DOUGLAS FLEMING LICENSE No. 56848 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/01/2015

TC LL	20.0 PSF	REF	R9114- 4668
TC DL	7.0 PSF	DATE	04/01/15
BC DL	10.0 PSF	DRW	HCUSR9114 15091001
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	37.0 PSF	SEQN-	1520 REV
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VFB9114Z01

32639

Alpine, an ITW Company

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1VFG487-Z0107100218

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

Truss Fabricator:	Anderson Truss Company
Job Identification:	14-215L--Skyline Construction /Res for Rimrock Developme -- 1629 SW Paquita Lake (1629 SW Man
Truss Count:	5
Model Code:	Florida Building Code 2014 or 2010
Truss Criteria:	FBC2010Res/TPI-2007(STD)
Engineering Software:	Alpine Software,Version 14.03.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 130 MPH ASCE 7-10 -Closed

04/07/2015

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Walter P. Finn
-Truss Design Engineer-

2400 Lake Orange Dr, Suite 150
Orlando FL, 32837

Details: BRCLBSUB-

#	Ref	Description	Drawing#	Date
1	46101--A	29' Attic	15096030	04/06/15
2	46102-A1	29' Attic Gir	15097002	04/07/15
3	46103--B1	39' Attic	15097005	04/07/15
4	46104--B2	39' Attic	15097003	04/07/15
5	46105-B3	39' Attic Gir	15097004	04/07/15

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-215L--Skyline Construction /Res for Rimrock Developme -- 1629 SW Mandiba Lake - A 29' Attic)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP M-30 :T2, T4 2x6 SP SS: :T3 2x4 SP #1:
Bot chord 2x6 SP #1 Dense :B2, B4 2x8 SP 2400f-2.0E:
:B3 2x4 SP #1:
Webs 2x4 SP #3
:Lt Wedge 2x4 SP #3::Rt Wedge 2x4 SP #3:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Calculated horizontal deflection is 0.14" due to live load and 0.23" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 19.10 ft. from roof edge.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	63 PLF	at -2.00 to	63 PLF	at 0.00
TC - From	58 PLF	at 0.00 to	58 PLF	at 7.76
TC - From	184 PLF	at 7.76 to	184 PLF	at 9.32
TC - From	158 PLF	at 9.32 to	158 PLF	at 9.62
TC - From	178 PLF	at 9.62 to	178 PLF	at 19.38
TC - From	158 PLF	at 19.38 to	158 PLF	at 19.68
TC - From	184 PLF	at 19.68 to	184 PLF	at 21.24
TC - From	58 PLF	at 21.24 to	58 PLF	at 29.00
TC - From	63 PLF	at 29.00 to	63 PLF	at 31.00
BC - From	20 PLF	at 0.00 to	20 PLF	at 29.00
BC -	122 LB	Conc. Load at 7.76, 21.24		

Collar-tie braced with continuous lateral bracing at 24" O.C. including chord ends, or rigid ceiling.

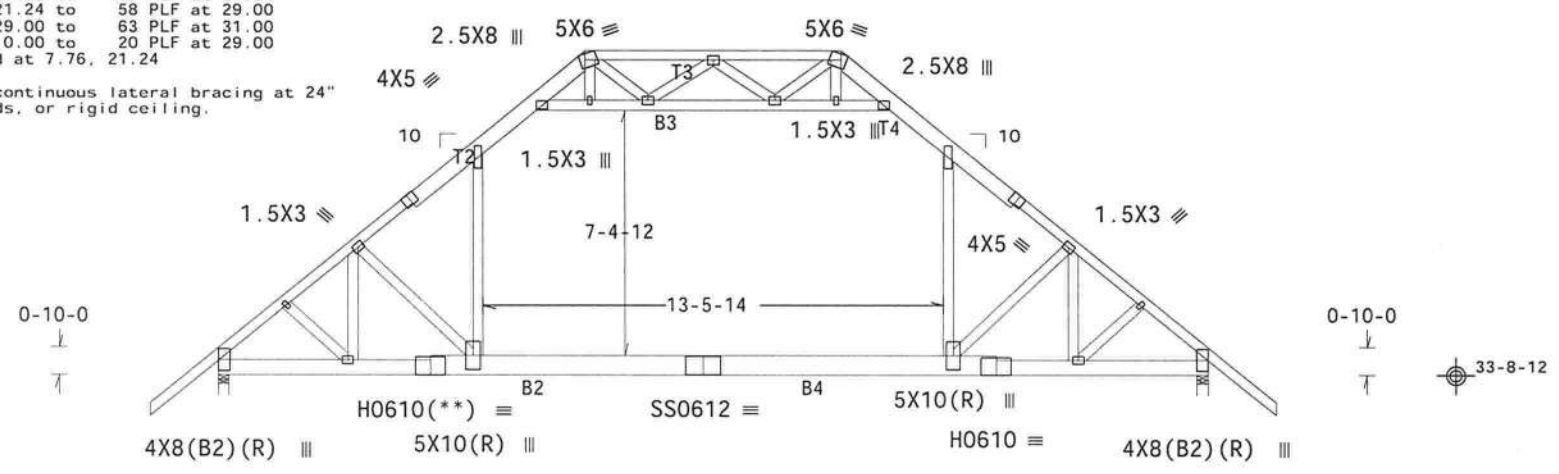
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

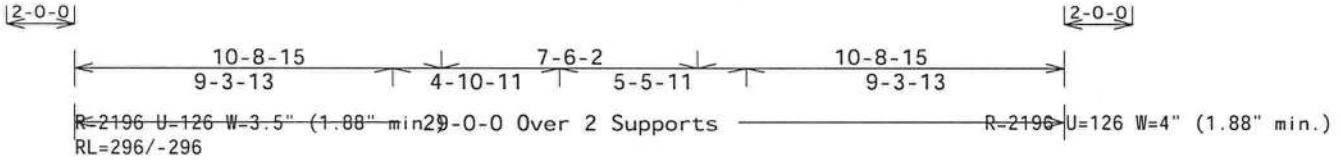
130 mph wind, 38.21 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Note: All Plates Are 3X4 Except As Shown.

PLT TYP. 20 Gauge HS,18 Gauge HS, Wave Design Crit: FBC2010Res/TPI-2007(STD)

FT/RT=10%(0%)/0(0) 14.03.01.0123.00 QTY:9 FL/-/5/-/-/R/- Scale =.1875"/Ft.

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AN ITW COMPANY

2400 Lake Orange Dr., Suite 150
Orlando, FL 32837
FL COA #0 278

WARNING! READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices [illegible] to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpinoitw.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/07/2015

TC LL	20.0 PSF	REF	R9114- 46101
TC DL	7.0 PSF	DATE	04/06/15
BC DL	10.0 PSF	DRW	HCUSR9114 15096030
BC LL	0.0 PSF	HC-ENG	GA/WPF
TOT.LD.	37.0 PSF	SEQN-	405022
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VFG487_Z01

(14-215L--Skyline Construction /Res for Rimrock Developme -- 1629 SW Mandiba Lake - A1 29' Attic Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1 :T2 2x6 SP SS:
:T4 2x6 SP #1 Dense: :T5 2x4 SP M-30:
Bot chord 2x8 SP 2400f-2.0E :B1 2x6 SP #1 Dense:
:B3 2x4 SP #1: :B5 2x6 SP #2:
Webs 2x4 SP #3
:Lt Wedge 2x4 SP #3::Rt Wedge 2x4 SP #3:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 58 plf at -2.00 to 58 plf at 0.00
TC- From 58 plf at 0.00 to 58 plf at 5.50
TC- From 58 plf at 5.50 to 58 plf at 7.76
TC- From 84 plf at 7.76 to 84 plf at 9.32
TC- From 58 plf at 9.32 to 58 plf at 10.74
TC- From 58 plf at 10.74 to 58 plf at 18.26
TC- From 58 plf at 18.26 to 58 plf at 19.68
TC- From 84 plf at 19.68 to 84 plf at 21.24
TC- From 58 plf at 21.24 to 58 plf at 23.50
TC- From 58 plf at 23.50 to 58 plf at 29.00
TC- From 58 plf at 29.00 to 58 plf at 31.00
PLT- From 100 plf at 7.76 to 100 plf at 9.62
PLT- From 120 plf at 14.21 to 120 plf at 19.38
BC- From 5 plf at -2.00 to 5 plf at 0.00
BC- From 5 plf at 29.00 to 5 plf at 31.00
BC- 1506.00 lb Conc. Load at 7.31
BC- 121.90 lb Conc. Load at 7.76,21.24
BC- 756.00 lb Conc. Load at 11.29,13.29,15.29,17.29
19.29,21.29
BC- 1570.00 lb Conc. Load at 21.69

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 27.64 ft. from roof edge.

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FABRICATOR TO REVIEW THIS DWG PRIOR TO CUTTING LUMBER TO VERIFY THAT ALL DATA, INCLUDING DIMENSIONS AND LOADS, CONFORM TO THE ARCHITECTURAL PLANS/SPECIFICATIONS AND FABRICATOR'S TRUSS LAYOUT.

Collar-tie braced with continuous lateral bracing at 24" O.C. including chord ends, or rigid ceiling.

4 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @11.50" o.c.
Bot Chord: 1 Row @ 5.50" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting. In addition, apply (1) 0.22"-0.25" min/max dia. X 6.0" length wood screw at each joint location.
4" o.c. spacing of nails perpendicular and parallel to grain required in area over bearings greater than 4"

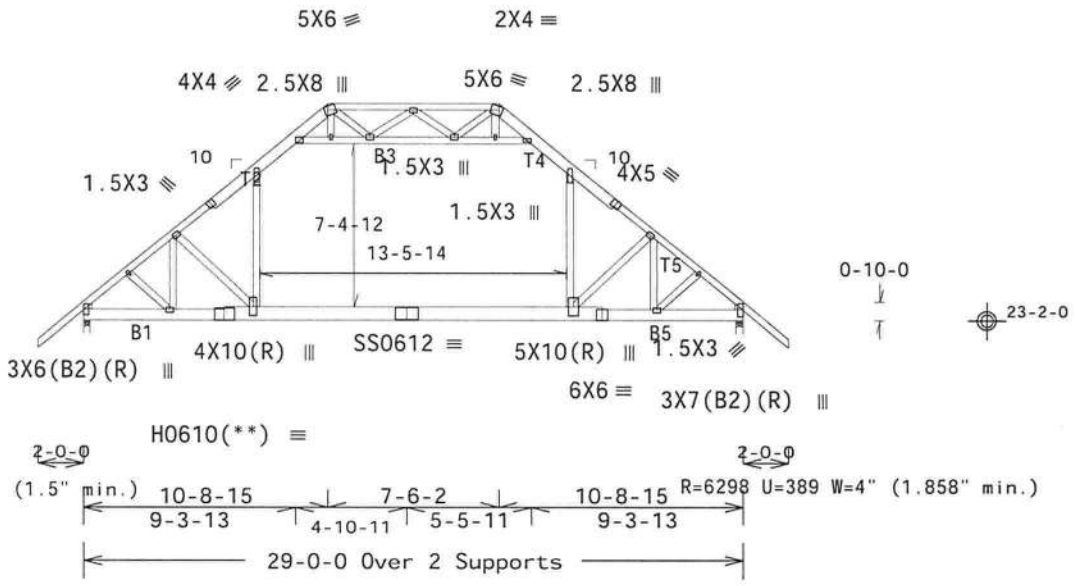
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

130 mph wind, 27.64 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Calculated horizontal deflection is 0.15" due to live load and 0.19" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



Note: All Plates Are 3X4 Except As Shown.
PLT TYP. 20 Gauge HS,18 Gauge HS, Design Crit: FBC2010Res/TPI-2007(STD)
Wave FT/RT=10%(0%)/0(0) 14.03.01 0123.00 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE™ AN ITW COMPANY
2400 Lake Orange Dr., Suite 150
Orlando, FL 32837
FL COA #0 278

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/07/2015

TC LL	20.0 PSF	REF	R9114- 46102
TC DL	7.0 PSF	DATE	04/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15097002
BC LL	0.0 PSF	HC-ENG	GA/WPF
TOT.LD.	37.0 PSF	SEQN-	405013
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VFG487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-215L--Skyline Construction /Res for Rimrock Developme -- 1629 SW Mandiba Lake - B1 39' Attic)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x6 SP #2 :B2, B4 2x4 SP #1:
:B3, B5 2x8 SP SS:
Webs 2x4 SP #3
:Lt Wedge 2x4 SP #3::Rt Wedge 2x4 SP #3:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	63 PLF at	-2.00 to	63 PLF at	0.00
TC - From	58 PLF at	0.00 to	58 PLF at	9.36
TC - From	78 PLF at	9.36 to	78 PLF at	12.76
TC - From	178 PLF at	12.76 to	178 PLF at	26.24
TC - From	78 PLF at	26.24 to	78 PLF at	28.26
TC - From	38 PLF at	28.26 to	38 PLF at	28.61
TC - From	78 PLF at	28.61 to	78 PLF at	29.64
TC - From	58 PLF at	29.64 to	58 PLF at	39.00
TC - From	63 PLF at	39.00 to	63 PLF at	41.00
BC - From	20 PLF at	0.00 to	20 PLF at	39.00
BC -	148 LB Conc. Load at 12.76, 26.24			

(++) - This plate works for both joints covered.

130 mph wind, 82.37 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

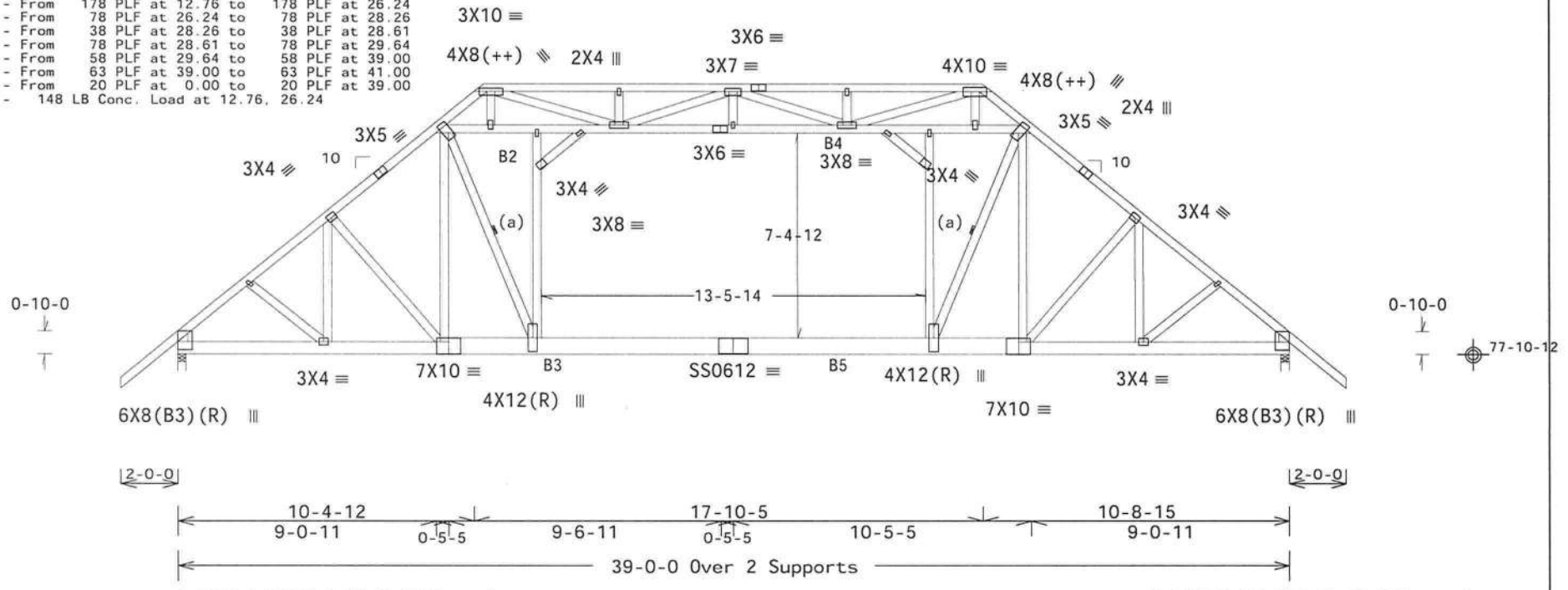
Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 41.19 ft. from roof edge.

Collar-tie braced with continuous lateral bracing at 24" O.C. including chord ends, or rigid ceiling.



R=2674 U=164 W=3.5" (2.674" min.)
RL=363/-363

R=2667 U=231 W=3.5" (2.667" min.)

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 14.03.01 01/20.00 QTY:6 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE™ AN ITW COMPANY
2400 Lake Orange Dr., Suite 150
Orlando, FL 32837
FL COA #0 278

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/07/2015

TC LL	20.0 PSF	REF	R9114- 46103
TC DL	7.0 PSF	DATE	04/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15097005
BC LL	0.0 PSF	HC-ENG	GA/WPF
TOT.LD.	37.0 PSF	SEQN-	404929
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VFG487_Z01

(14-215L--Skyline Construction /Res for Rimrock Developme -- 1629 SW Mandiba Lake - B2 39' Attic)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x6 SP #2 :B2, B4 2x4 SP M-30:
:B3 2x8 SP 2400f-2.0E: :B5 2x8 SP #1 Dense:
Webs 2x4 SP #3 :W5 2x4 SP #2:
:Lt Wedge 2x4 SP #3::Rt Wedge 2x4 SP #3:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Calculated horizontal deflection is 0.12" due to live load and 0.23" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 63 PLF at -2.00 to 63 PLF at 0.00
TC - From 58 PLF at 0.00 to 58 PLF at 9.36
TC - From 78 PLF at 9.36 to 78 PLF at 12.76
TC - From 178 PLF at 12.76 to 178 PLF at 26.24
TC - From 78 PLF at 26.24 to 78 PLF at 28.26
TC - From 38 PLF at 28.26 to 38 PLF at 28.61
TC - From 78 PLF at 28.61 to 78 PLF at 29.64
TC - From 58 PLF at 29.64 to 58 PLF at 39.00
TC - From 63 PLF at 39.00 to 63 PLF at 41.00
BC - From 20 PLF at 0.00 to 20 PLF at 39.00
BC - 148 LB Conc. Load at 12.76, 26.24

(++) - This plate works for both joints covered.

130 mph wind, 27.64 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Left cantilever is exposed to wind

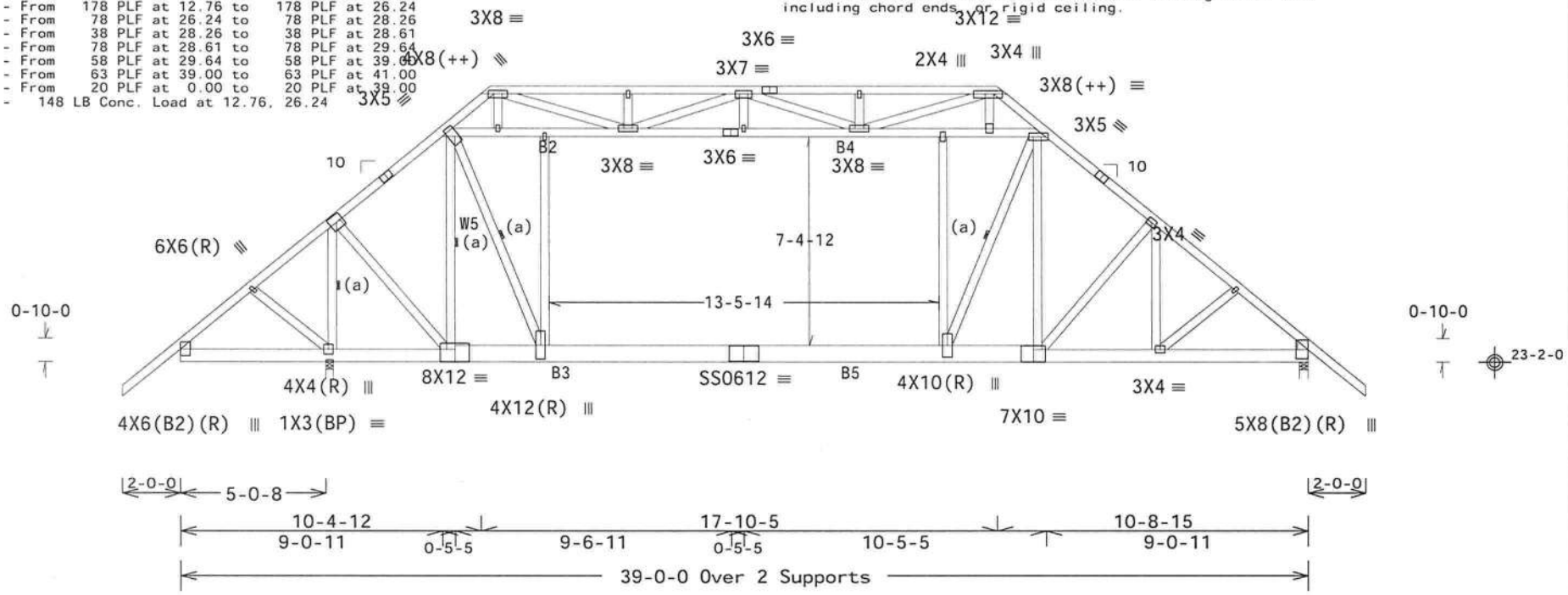
(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 27.64 ft. from roof edge.

Collar-tie braced with continuous lateral bracing at 24" O.C. including chord ends, or rigid ceiling.



R=3077 W=3" (2.76" min.)
RL=279/-278

R=2264 U=12 W=3.5" (2.264" min.)

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 14.03.01 [illegible]00 QTY:7 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE™ AN ITW COMPANY

2400 Lake Orange Dr., Suite 150
Orlando, FL 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/07/2015

TC LL	20.0 PSF	REF	R9114- 46104
TC DL	7.0 PSF	DATE	04/07/15
BC DL	10.0 PSF	DRW	HCUSR9114 15097003
BC LL	0.0 PSF	HC-ENG	GA/WPF
TOT.LD.	37.0 PSF	SEQN-	404933
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VFG487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(14-215L--Skyline Construction /Res for Rimrock Developme -- 1629 SW Mandiba Lake - B3 39' Attic Girder)

Value Set: 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x6 SP #2 :B2 2x4 SP #1:
:B3, B5 2x8 SP 2400f-2.0E: :B4 2x4 SP M-30:
Webs 2x4 SP #3 :W5, W8, W17 2x4 SP #2:
:Lt Wedge 2x4 SP #3::Rt Wedge 2x4 SP #3:

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 58 plf at -2.00 to 58 plf at 0.00
TC- From 58 plf at 0.00 to 58 plf at 7.03
TC- From 58 plf at 7.03 to 58 plf at 10.39
TC- From 58 plf at 10.39 to 58 plf at 20.39
TC- From 58 plf at 20.39 to 58 plf at 28.26
TC- From 18 plf at 28.26 to 18 plf at 28.61
TC- From 58 plf at 28.61 to 58 plf at 31.97
TC- From 58 plf at 31.97 to 58 plf at 39.00
TC- From 58 plf at 39.00 to 58 plf at 41.00
PLT- From 20 plf at 9.36 to 20 plf at 29.64 (CEILING)
BC- From 5 plf at -2.00 to 5 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 39.00
BC- From 5 plf at 39.00 to 5 plf at 41.00
BC- 1506.00 lb Conc. Load at 12.31
BC- 147.92 lb Conc. Load at 12.76,26.24
BC- 756.00 lb Conc. Load at 14.31,16.31,18.31,20.31
22.31,24.31
BC- 2556.00 lb Conc. Load at 24.71
19.29,21.29
BC- 1570.00 lb Conc. Load at 21.69

Collar-tie braced with continuous lateral bracing at 24" O.C. including chord ends, or rigid ceiling.

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FABRICATOR TO REVIEW THIS DWG PRIOR TO CUTTING LUMBER TO VERIFY THAT ALL DATA, INCLUDING DIMENSIONS AND LOADS, CONFORM TO THE ARCHITECTURAL PLANS/SPECIFICATIONS AND FABRICATOR'S TRUSS LAYOUT.

4 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 6.50" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting. In addition, apply (1) 0.22"-0.25" min/max dia. X 6.0" length wood screw at each joint location.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

130 mph wind, 27.64 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

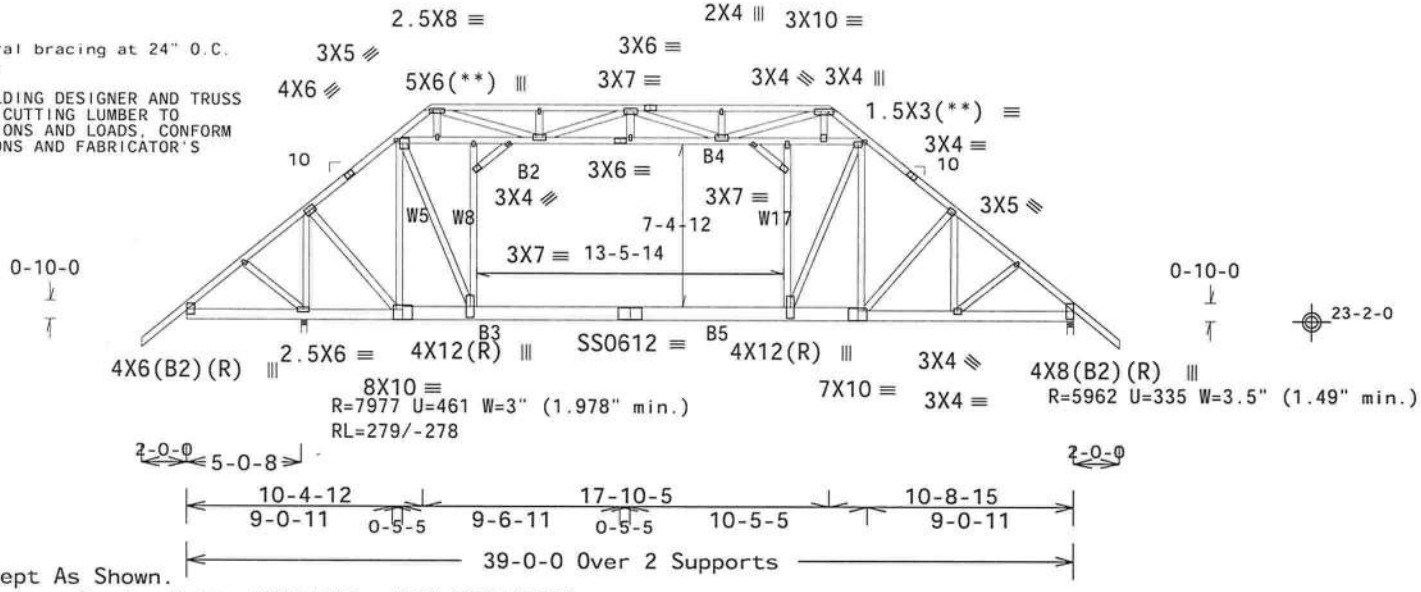
Left cantilever is exposed to wind

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 27.64 ft. from roof edge.



Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 14.03.01.0120.00 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE AN ITW COMPANY

2400 Lake Orange Dr., Suite 150
Orlando, FL 32837
FL COA #0 278

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BC LL	0.0 PSF		HC-ENG GA/WPF
TOT.LD.	37.0 PSF	SEQN-	404989
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VFG487_Z01

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

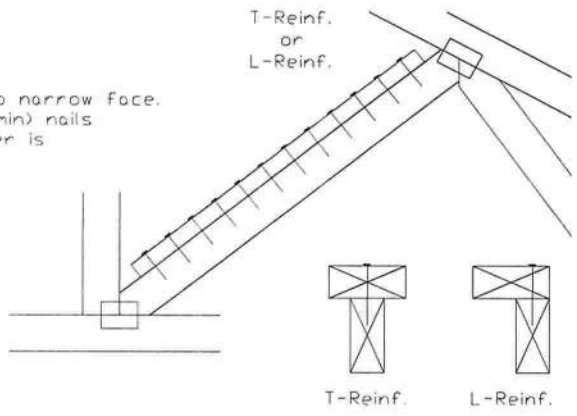
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

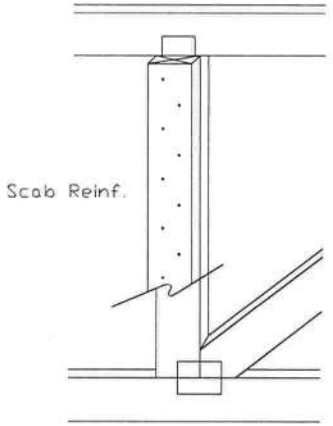
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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TC DL	PSF	DATE 10/01/14
BC DL	PSF	DRWG BRCLBSUB1014
BC LL	PSF	
TOT. L.D.	PSF	
DUR. FAC.		
SPACING		