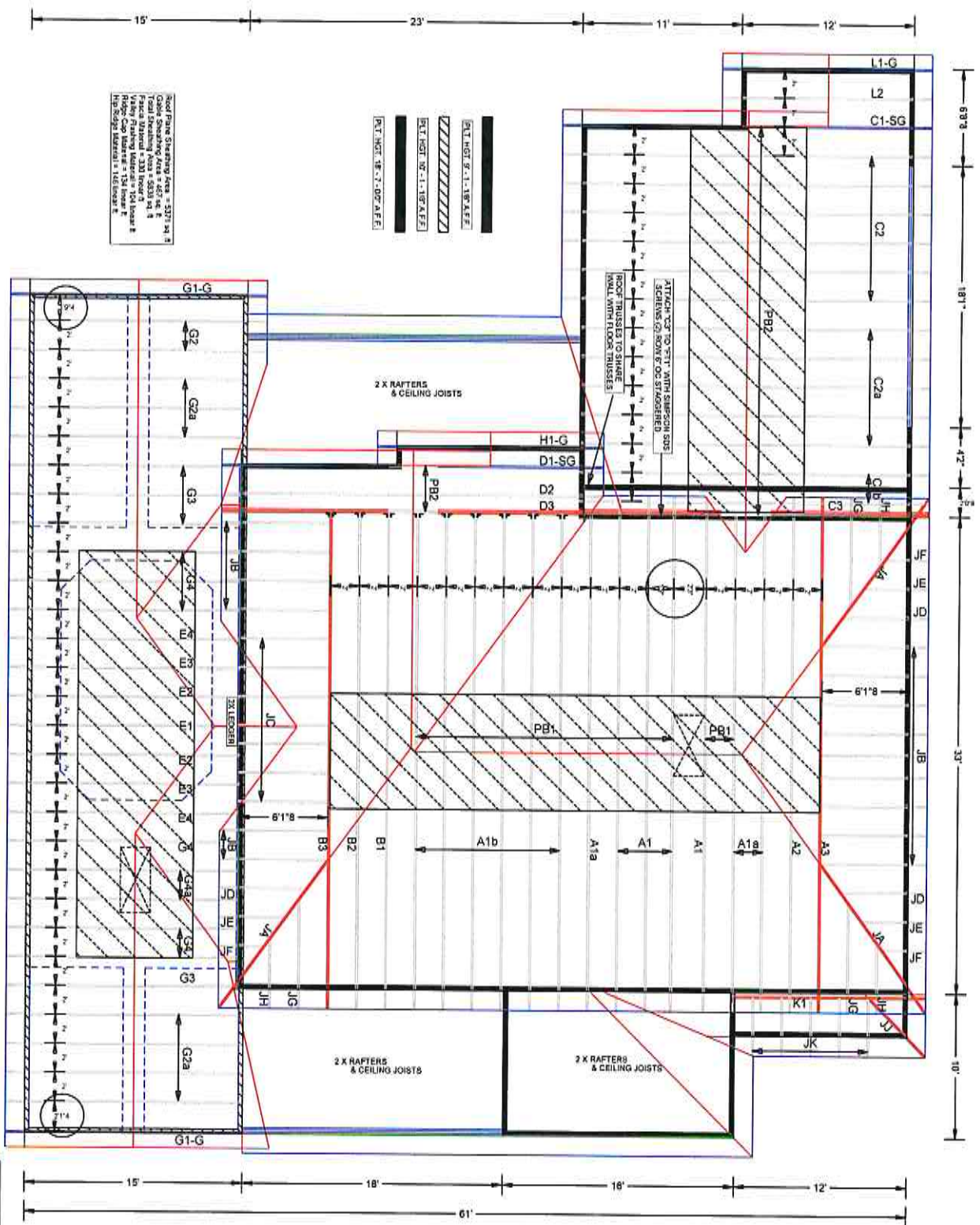


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1 OF 1

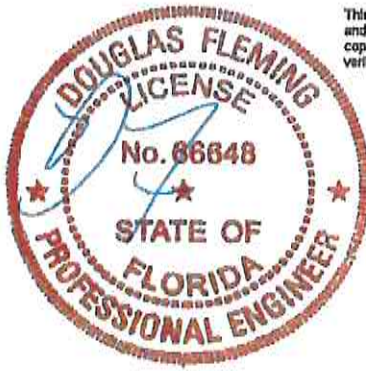


SEAINOLE TRUSSES INC.
30726 Bueslar Memorial Hwy.
MIDWAY FL 32343
Phone (850) 575-0102
Fax (850) 575-4413
Design By Robert J. Little

Job Name: RIOS/FIDEL RESIDENCE
Customer: America's Home Place
Designer: ROBERT J. LITTLE
PlanName: LEXINGTON-MFH
Created : 01-10-2022
SemRef#: B54908a

JOB NO:
B54908a

PAGE NO:
1 OF 1



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Alpine, an ITW Company
6750 Forum Drive, Suite 305
Orlando, FL 32821
Phone: (800)755-6001
www.alpineitw.com

COA #0 278
01/10/2022



Site Information:	Page 1:
Customer: Seminole Trusses, Inc.	Job Number: B54908b
Job Description: RIOS/FIDEL RESIDENCE	
Address: 7313 WEST US HWY 90, LAKE CITY, FL 32055	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.01.03A
	JRef #: 1Xc38570002
Wind Standard: ASCE710 Wind Speed (mph): 0	Design Loading (psf): 55.00
Building Type:	

This package contains general notes pages, 9 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss
1	010.22.0914.17083	F100 12'3"4 Floor Truss
3	010.22.0914.14450	F102 10'11"12 Floor Truss
5	010.22.0914.10403	F104 15'11"12 Floor Truss
7	010.22.0914.11883	F106 12'11"4 Floor Truss
9	010.22.0914.36747	FT1 22'11"12 Flat Girder
11	PB180160118	
13	STRBRIBR1014	

Item	Drawing Number	Truss
2	010.22.0914.06653	F101 12'3"4 Floor Truss
4	010.22.0914.08980	F103 15'11"12 Floor Truss
6	010.22.0914.13140	F105 12'11"4 Gable
8	010.22.0914.15713	F107 12'11"4 Floor Truss
10	PB160160118	
12	REPCHRD1014	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

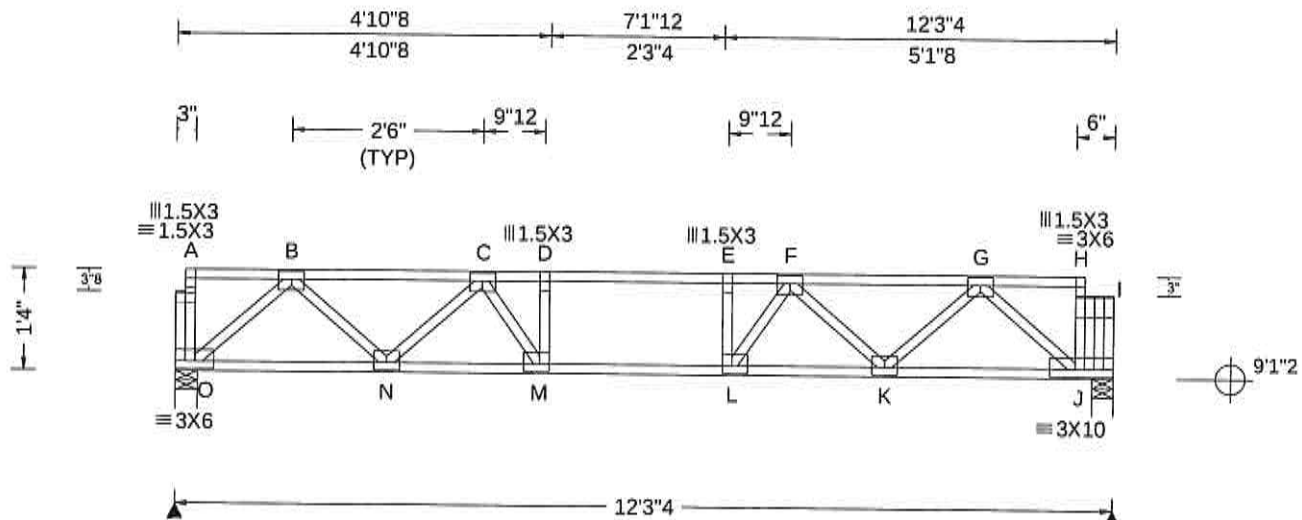
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 97375 FROM: RJL	SY42	Ply: 1 Qty: 12	Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F100 12'3"4 Floor Truss	Cust: R 857 JRef: 1Xc38570002 T8 DrwNo: 010.22.0914.17083 SSB / DF 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2"	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.053 E 999 360 VERT(CL): 0.086 E 999 240 HORZ(LL): 0.012 B - - HORZ(TL): 0.020 B - - Creep Factor: 2.0 Max TC CSI: 0.392 Max BC CSI: 0.324 Max Web CSI: 0.214 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL O 525 - / - / - / - / - J 514 - / - / - / - / - O Brg Wid = 3.4 Min Req = 1.5 J Brg Wid = 3.4 Min Req = 1.5 Bearings O & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 - 836 E - F 0 - 1228 C - D 0 - 1227 F - G 0 - 884 D - E 0 - 1234

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Plating Notes

All plates are 3X4 except as noted.

Additional Notes

See detail STRBRIBR1014 for bracing and bridging recommendations.

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

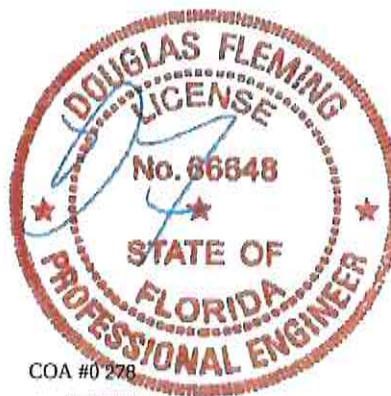
Truss must be installed as shown with top chord up.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
O - N	524	L - K	1154
N - M	1131	K - J	562
M - L	1234		0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
O - B	0	K - G	449
B - N	435	G - J	0
N - C	0	I - J	191



COA #0 278

01/10/2022

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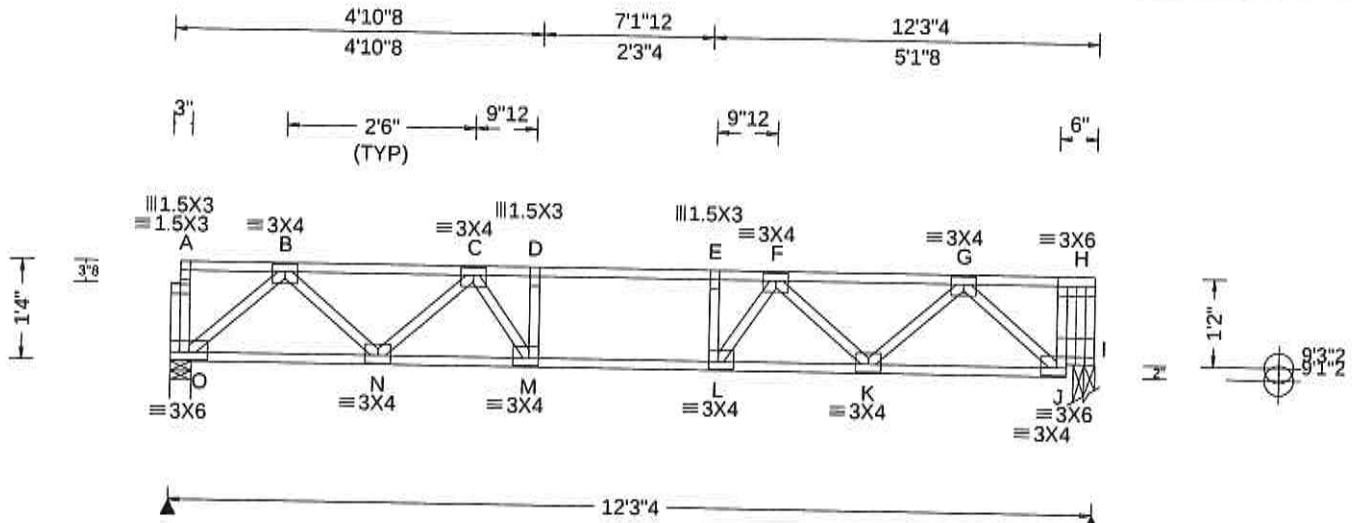
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCEA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCEA: sbceacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97346 FROM: RJL	SY42 Qty: 8	Ply: 1	Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F101 12'3"4 Floor Truss	Cust: R 857 JRef: 1Xc38570002 T1 DrwNo: 010.22.0914.06653 SSB / DF 01/10/2022
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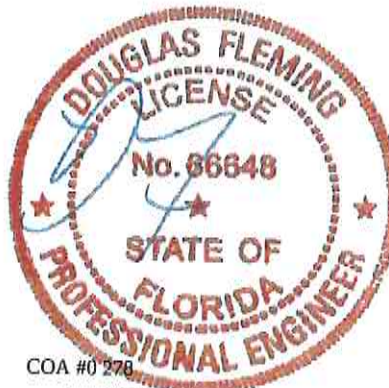
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, PI in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpl: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.054 E 999 360 VERT(CL): 0.088 E 999 240 HORZ(LL): 0.012 B - - HORZ(TL): 0.019 B - - Creep Factor: 2.0 Max TC CSI: 0.420 Max BC CSI: 0.332 Max Web CSI: 0.440 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL O 530 /- /- /- /- /- I 536 /- /- /- /- /- O Brg Wid = 3.4 Min Req = 1.5 I Brg Wid = 3.4 Min Req = 1.5 Bearings O & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 - 845 E - F 0 - 1251 C - D 0 - 1249 F - G 0 - 908 D - E 0 - 1257

Lumber
Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;
Rt Bearing Leg: 4x2 SP #3;

Additional Notes
See detail STRBR1014 for bracing and bridging recommendations.
Truss must be installed as shown with top chord up.

Chords	Tens.Comp.	Chords	Tens. Comp.
O - N	528 0	L - K	1180 0
N - M	1145 0	K - J	612 0
M - L	1257 0		

Webs	Tens.Comp.	Webs	Tens. Comp.
O - B	0 - 719	K - G	412 0
B - N	441 0	G - J	0 - 701
N - C	0 - 417	I - J	472 0
F - K	0 - 377	H - I	472 - 540



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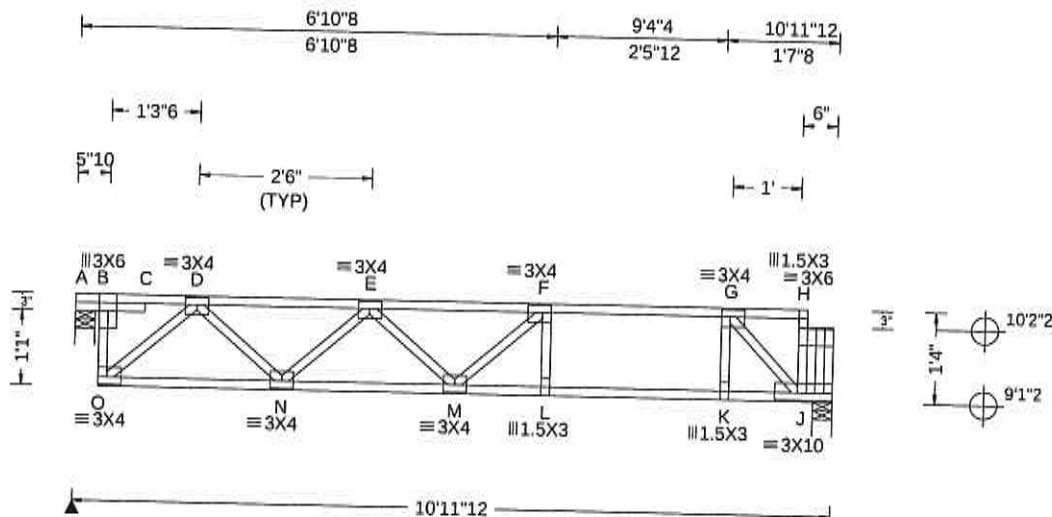
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCE) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to Job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCE: sbcecomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97373 FROM: RJL	SY42	Ply: 1 Qty: 7	Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F102 10'11"12 Floor Truss	Cust: R 857 JRef: 1Xc38570002 T6 DrwNo: 010.22.0914.14450 SSB / DF 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)																																												
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2"	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.234 L 540 360 VERT(CL): 0.328 L 385 240 HORZ(LL): -0.040 O - - HORZ(TL): 0.056 O - - Creep Factor: 2.0 Max TC CSI: 0.715 Max BC CSI: 0.976 Max Web CSI: 0.250 VIEW Ver: 21.01.03A.0805.15	<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="2">Gravity</th> <th colspan="2">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>R-</th> <th>Rw</th> <th>RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>475</td> <td>-</td> <td>-</td> <td>-</td> </tr> <tr> <td>J</td> <td>458</td> <td>-</td> <td>-</td> <td>-</td> </tr> <tr> <td colspan="5">A Brg Wid = 3.4 Min Req = 1.5 J Brg Wid = 3.4 Min Req = 1.5 Bearings A & J are a rigid surface. Members not listed have forces less than 375#</td> </tr> <tr> <th colspan="5">Maximum Top Chord Forces Per Ply (lbs)</th> </tr> <tr> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> <tr> <td>D - E</td> <td>0</td> <td>-784</td> <td>F - G</td> <td>0 -729</td> </tr> <tr> <td>E - F</td> <td>0</td> <td>-974</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity		Non-Gravity		R+	R-	Rw	RL	A	475	-	-	-	J	458	-	-	-	A Brg Wid = 3.4 Min Req = 1.5 J Brg Wid = 3.4 Min Req = 1.5 Bearings A & J are a rigid surface. Members not listed have forces less than 375#					Maximum Top Chord Forces Per Ply (lbs)					Chords	Tens.	Comp.	Chords	Tens. Comp.	D - E	0	-784	F - G	0 -729	E - F	0	-974		
Loc	Gravity		Non-Gravity																																													
	R+	R-	Rw	RL																																												
A	475	-	-	-																																												
J	458	-	-	-																																												
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Maximum Top Chord Forces Per Ply (lbs)																																																
Chords	Tens.	Comp.	Chords	Tens. Comp.																																												
D - E	0	-784	F - G	0 -729																																												
E - F	0	-974																																														

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Additional Notes

See detail STRBRIBR1014 for bracing and bridging recommendations.

Truss must be installed as shown with top chord up.

Maximum Bot Chord Forces Per Ply (lbs)				
Chords	Tens.	Comp.	Chords	Tens. Comp.
O - N	541	0	L - K	729 0
N - M	1020	0	K - J	710 0
M - L	743	0		

Maximum Web Forces Per Ply (lbs)				
Webs	Tens.	Comp.	Webs	Tens. Comp.
O - B	492	0	G - J	0 -1065
O - D	0	-737		



01/10/2022

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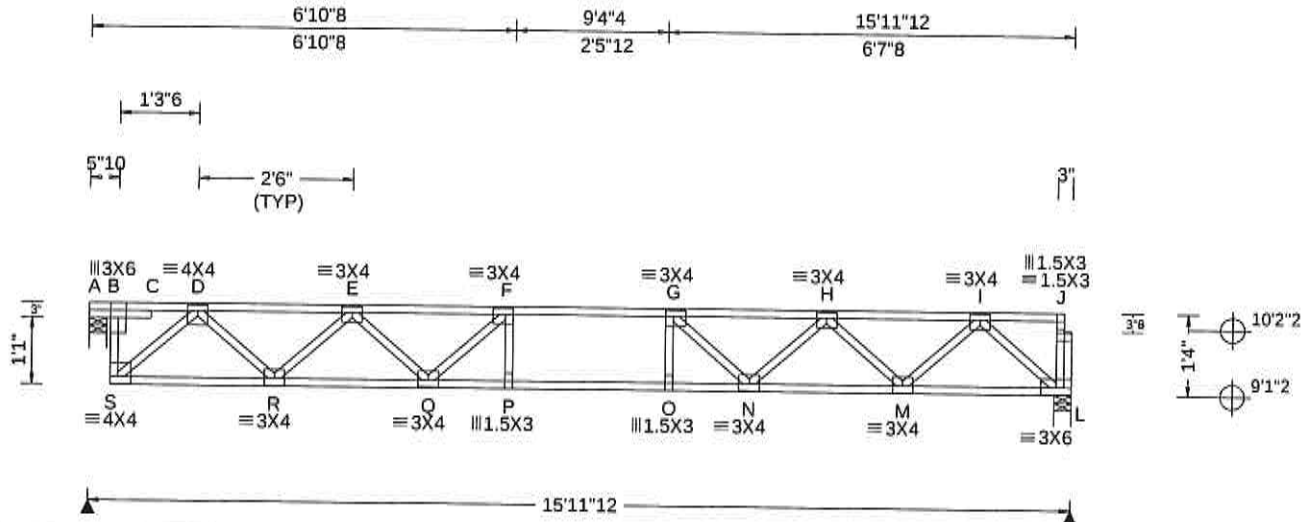
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. Refer to Job's General Notes page for additional information.

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpiinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
ANTHONY COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97369 FROM: RJL	SY42 Qty: 5	Ply: 1	Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F103 15'11"12 Floor Truss	Cust: R 857 JRef: 1Xc38570002 T2 DrwNo: 010.22.0914.08980 SSB / DF 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg. Pl in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpl: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.136 P 999 360 VERT(CL): 0.223 P 841 240 HORZ(LL): -0.022 S - - HORZ(TL): 0.037 S - - Creep Factor: 2.0 Max TC CSI: 0.577 Max BC CSI: 0.682 Max Web CSI: 0.365 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 699 /- /- /- /- /- L 694 /- /- /- /- /- A Brg Wid = 3.4 Min Req = 1.5 L Brg Wid = 3.4 Min Req = 1.5 Bearings A & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. D - E 0 - 1308 G - H 0 - 1927 E - F 0 - 1977 H - I 0 - 1197 F - G 0 - 2188

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Additional Notes

See detail STRBRIBR1014 for bracing and bridging recommendations.

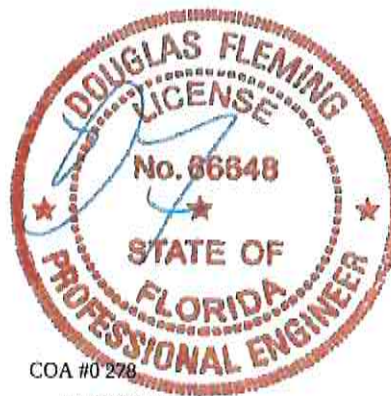
Truss must be installed as shown with top chord up.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
S - R	851	O - N	2186
R - Q	1742	N - M	1664
Q - P	2188	M - L	704
P - O	2188		0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
S - B	766	G - N	0 - 495
S - D	0 - 1159	N - H	403
D - R	635	H - M	0 - 651
R - E	0 - 604	M - I	685
E - Q	375	I - L	0 - 958
Q - F	0 - 448		



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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

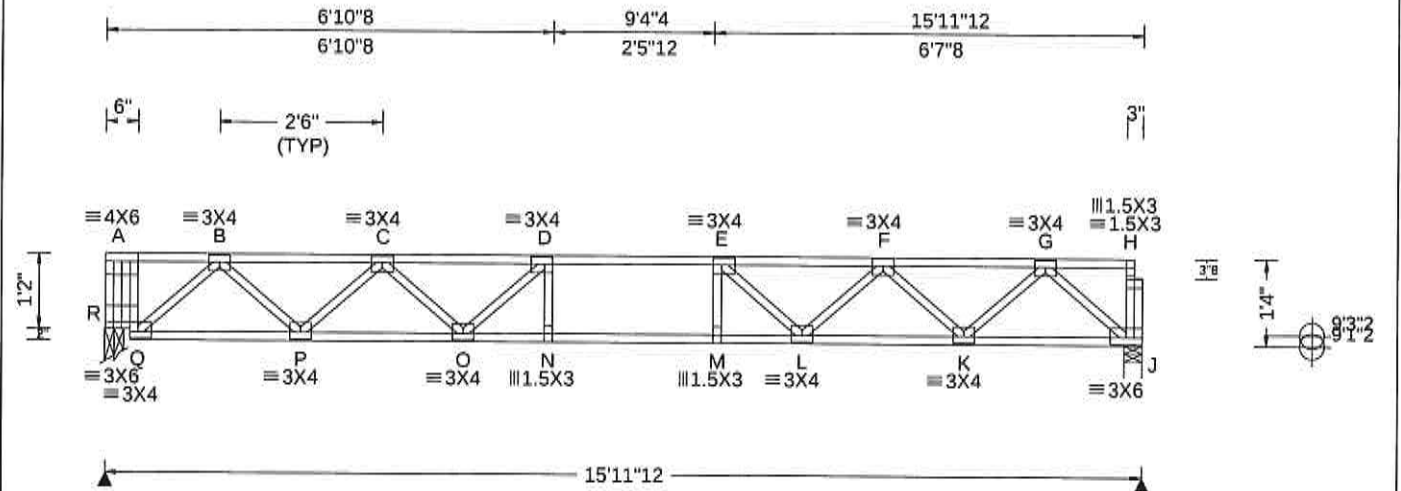
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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ALPINE
A DIVISION OF ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97366 FROM: RJL	SY42	Ply: 1 Qty: 8	Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F104 15'11"12 Floor Truss	Cust: R 857 JRef: 1Xc38570002 T3 DrwNo: 010.22.0914.10403 SSB / DF 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 40.00	Wind Std: NA	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity						
TCDL: 10.00	Speed: NA mph	Pf: NA Ce: NA	VERT(LL): 0.126 N 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: NA	Lu: NA Cs: NA	VERT(CL): 0.207 N 905 240	R	699	-	-	-	-	-
BCDL: 5.00	Category: NA	Snow Duration: NA	HORZ(LL): 0.025 A - -	J	693	-	-	-	-	-
Des Ld: 55.00	EXP: NA Kzt: NA	Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	HORZ(TL): 0.039 A - -	R	Brg Wid = 3.4 Min Req = 1.5					
NCBCLL: 10.00	Mean Height: NA ft		Creep Factor: 2.0	J	Brg Wid = 3.4 Min Req = 1.5					
Soffit: 0.00	TCDL: NA psf		Max TC CSI: 0.429	Bearings R & J are a rigid surface.						
Load Duration: 1.00	BCDL: NA psf		Max BC CSI: 0.676	Members not listed have forces less than 375#						
Spacing: 19.2 "	MWFRS Parallel Dist: NA		Max Web CSI: 0.586	Maximum Top Chord Forces Per Ply (lbs)						
	C&C Dist a: NA ft			Chords	Tens.	Comp.	Chords	Tens.	Comp.	
	Loc. from endwall: NA			B - C	0	-1290	E - F	0	-1923	
	I: NA GCpi: NA		VIEW Ver: 21.01.03A.0805.15	C - D	0	-1971	F - G	0	-1195	
	Wind Duration: NA			D - E	0	-2181				
Lumber										

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;
Lt Bearing Leg: 4x2 SP #3;

Additional Notes

See detail STRBRIBR1014 for bracing and bridging recommendations.

Truss must be installed as shown with top chord up.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
Q - P	820	0	M - L	2180	0
P - O	1738	0	L - K	1662	0
O - N	2181	0	K - J	703	0
N - M	2181	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - R	631	-4	O - D	0	-447
R - A	0	-711	E - L	0	-492
R - Q	631	0	L - F	401	0
Q - B	0	-940	F - K	0	-650
B - P	654	0	K - G	683	0
P - C	0	-623	G - J	0	-956



COA #0 278

01/10/2022

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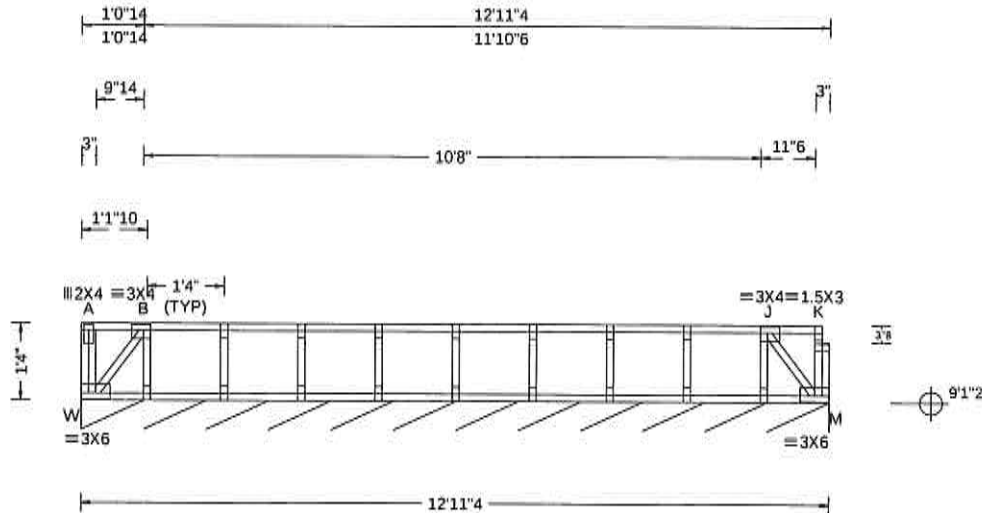
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCEA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97357 FROM: RJL	SY42	Ply: 1 Qty: 2	Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F105 12'11"4 Gable	Cust: R 857 JRef: 1Xc38570002 T5 DrwNo: 010.22.0914.13140 SSB / DF 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 C 999 360 VERT(CL): 0.000 C 999 240 HORZ(LL): -0.000 J - - HORZ(TL): 0.000 J - - Creep Factor: 2.0 Max TC CSI: 0.048 Max BC CSI: 0.011 Max Web CSI: 0.026 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL W* 87 /- /- /- /- /- W Brg Wid = 155 Min Req = - Bearing W is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Bracing

Sheathing is required for any longitudinal(drag) forces. All connections to be designed by the building designer.

Plating Notes

All plates are 1.5X3 except as noted.

Additional Notes

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

Truss must be installed as shown with top chord up.



COA #0278

01/10/2022

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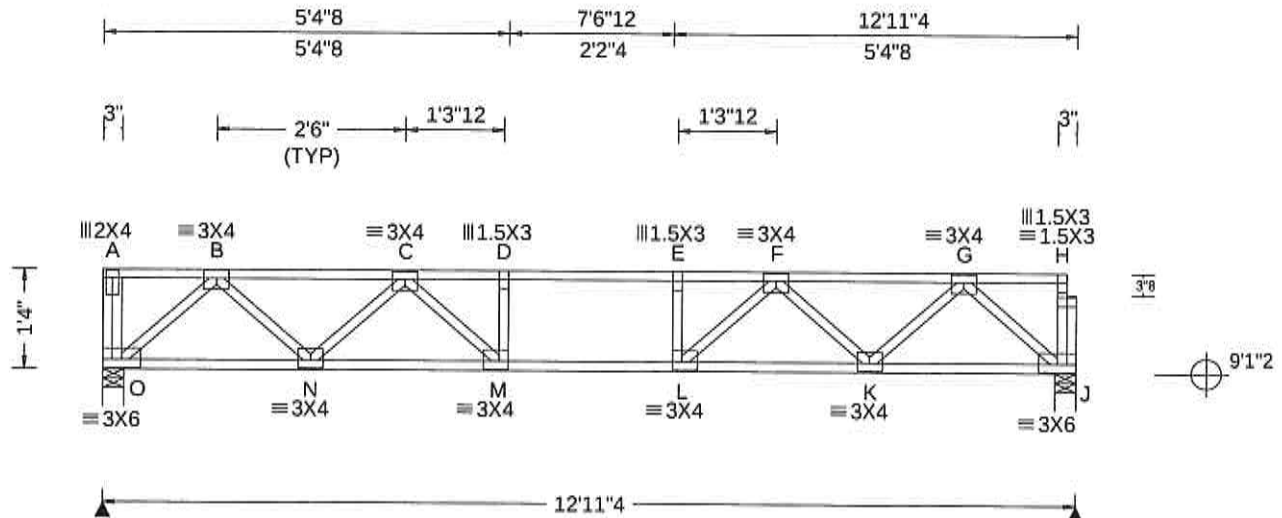
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AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97363 FROM: RJL	SY42 Qty: 3	Ply: 1	Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F106 12'11"4 Floor Truss	Cust: R 857 JRef: 1Xc38570002 T4 DrwNo: 010.22.0914.11883 SSB / DF 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpl: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.059 E 999 360 VERT(CL): 0.097 E 999 240 HORZ(LL): 0.014 B - - HORZ(TL): 0.023 B - - Creep Factor: 2.0 Max TC CSI: 0.386 Max BC CSI: 0.342 Max Web CSI: 0.232 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL O 572 -/-/-/-/- J 556 -/-/-/-/- O Brg Wid = 3.4 Min Req = 1.5 J Brg Wid = 3.4 Min Req = 1.5 Bearings O & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 - 883 E - F 0 - 1382 C - D 0 - 1382 F - G 0 - 902 D - E 0 - 1389

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Additional Notes

See detail STRBRIBR1014 for bracing and bridging recommendations.

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

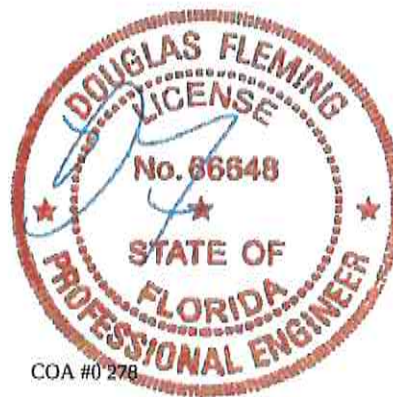
Truss must be installed as shown with top chord up.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
O - N	533 0	L - K	1223 0
N - M	1211 0	K - J	557 0
M - L	1389 0		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
O - B	0 - 745	F - K	0 - 446
B - N	487 0	K - G	479 0
N - C	0 - 456	G - J	0 - 758
C - M	379 0		



COA #0 278
01/10/2022

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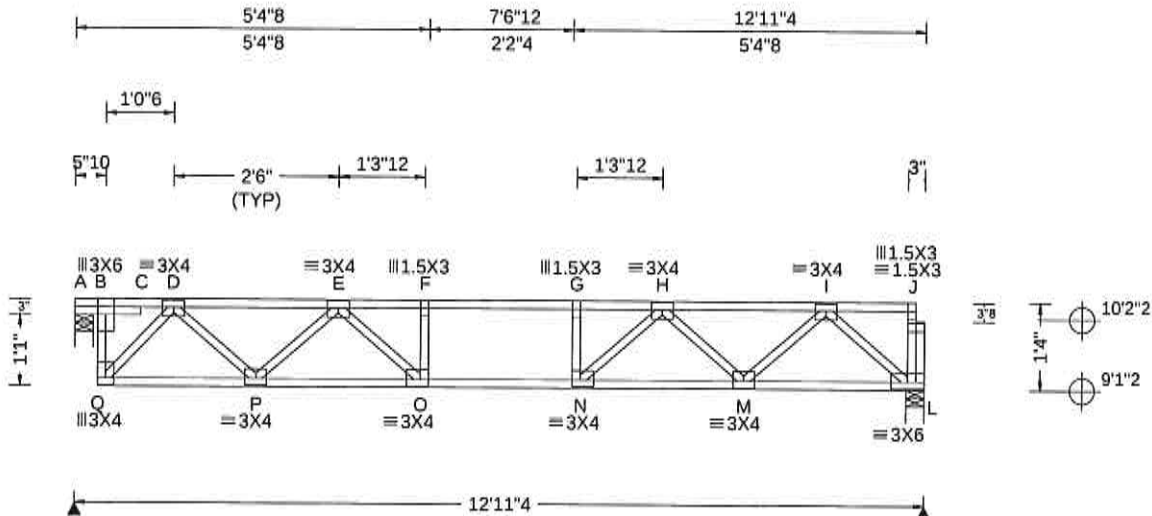
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97361 FROM: RJL	SY42	Ply: 1 Qty: 7	Job Number: B54908b RIOS/FIDEL RESIDENCE Truss Label: F107 12'11"4 Floor Truss	Cust: R 857 JRef: 1Xc38570002 T7 DrwNo: 010.22.0914.15713 SSB / DF 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 40.00 TCCL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2"	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpl: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.067 F 999 360 VERT(CL): 0.109 F 999 240 HORZ(LL): -0.012 Q - - HORZ(TL): 0.020 Q - - Creep Factor: 2.0 Max TC CSI: 0.463 Max BC CSI: 0.344 Max Web CSI: 0.307 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 565 - / - / - / - / - L 560 - / - / - / - / - A Brg Wid = 3.4 Min Req = 1.5 L Brg Wid = 3.4 Min Req = 1.5 Bearings A & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. D - E 0 -922 G - H 0 -1404 E - F 0 -1404 H - I 0 -910 F - G 0 -1412

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Additional Notes

See detail STRBRI1014 for bracing and bridging recommendations.

Truss must be installed as shown with top chord up.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
Q - P	585 0	N - M	1236 0
P - O	1237 0	M - L	562 0
O - N	1412 0		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
Q - B	645 0	N - H	379 0
Q - D	0 -880	H - M	0 -453
D - P	469 0	M - I	485 0
P - E	0 -438	I - L	0 -764
E - O	376 0		



COA #0278

01/10/2022

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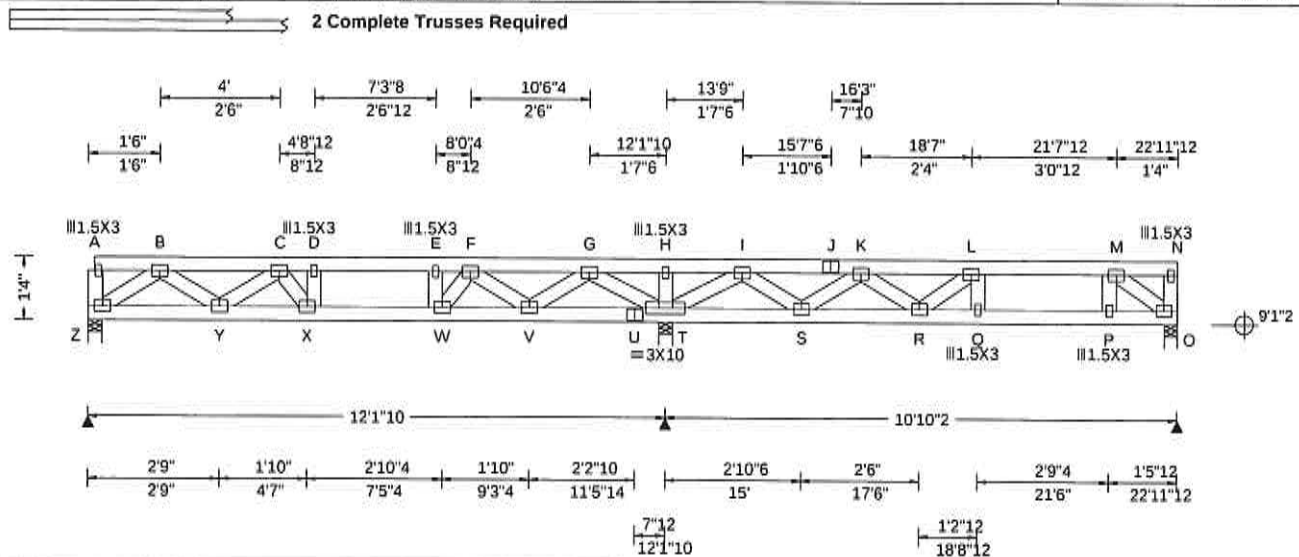
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ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97377 FROM: RJL	FLAT Ply: 2 Qty: 1	Job Number: B54908b RÍOS/FIDEL RESIDENCE Truss Label: FT1 22'11"12 Flat Girder	Cust: R 857 JRef:1xc38570002 T11 DrwNo: 010.22.0914.36747 SSB / DF 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpl: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.022 Q 999 360 VERT(CL): 0.036 Q 999 240 HORZ(LL): 0.005 A - - HORZ(TL): 0.008 A - - Creep Factor: 2.0 Max TC CSI: 0.165 Max BC CSI: 0.167 Max Web CSI: 0.146 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL Z 465 -/-/-/-/- T 1259 -/-/-/-/- O 386 -/-/-/-/- Z Brg Wid = 3.4 Min Req = 1.5 T Brg Wid = 3.5 Min Req = 1.5 O Brg Wid = 3.4 Min Req = 1.5 Bearings Z, T, & O are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 -429 E - F 37 -530 C - D 19 -544 G - H 632 0 D - E 26 -546 H - I 632 0 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. Y - X 543 0 W - V 448 -100 X - W 546 -26 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. G - T 0 -492 M - O 0 -398 T - I 0 -455

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Nailnote

Nail Schedule: 0.128"x3" nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Plating Notes

All plates are 3X4 except as noted.

Additional Notes

Truss must be installed as shown with top chord up.



COA #0278

01/10/2022

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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBICA: sbicacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

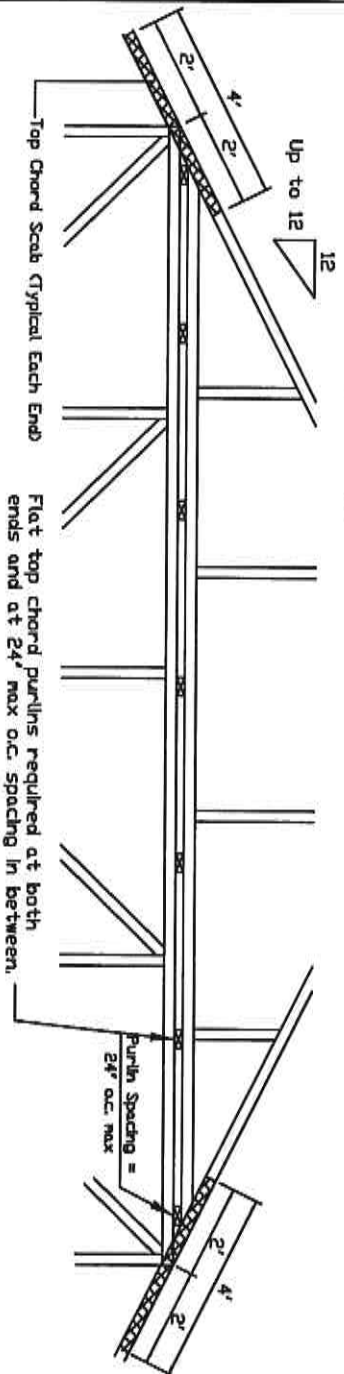
160 mph Wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp C, Wind DL = 5.0 psf (min), Kzt=1.0, Dr 160 mph Wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp D, Wind DL = 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

See Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

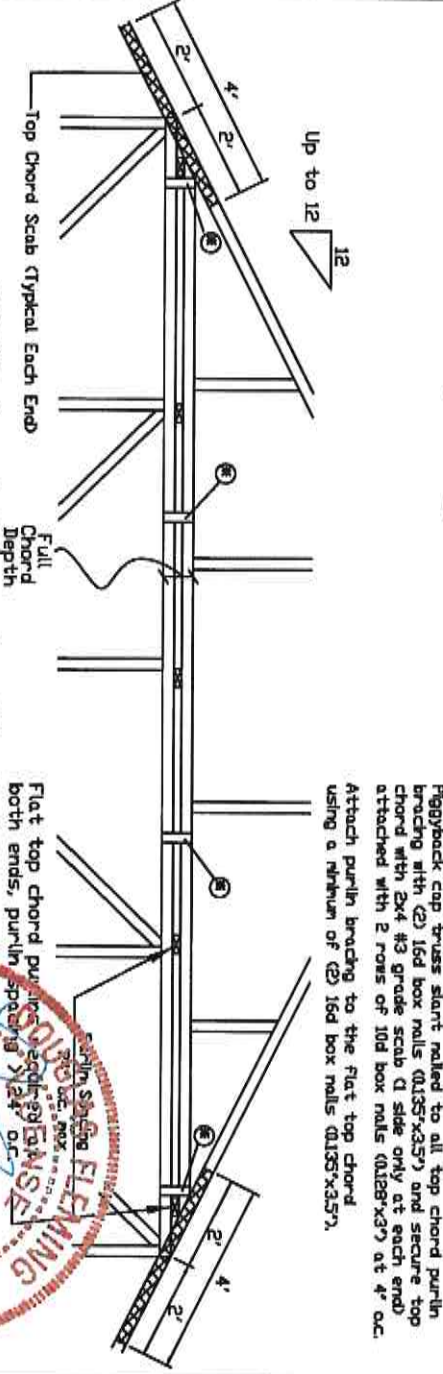


Piggyback cap truss shall nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following (1) 3X8 Tru-lux plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (2) 2BPB wave piggyback plate attached to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss shall nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

■ In addition, provide connection with one of the following methods:

Tru-lux
Use 3X8 Tru-lux plates for 2x4 chord member, and 3X10 Tru-lux plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Tru-lux plates may be staggered 4' o.c. front to back faces.

APA Rated Gussset
8"x8"x7/16" (min) APA rated sheathing gusssets (each face). Attach @ 8' o.c. with (8) 5d common (0.113"x2") nails per gussset; (4) in cap bottom chord and (4) in base truss top chord. Gusssets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab; (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

2BPB Wave Piggyback Plate
One 2BPB wave piggyback plate to each face @ 8' o.c. Attach teeth to supporting truss of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.



ALPINE COMPANY

514 East Cuy Expressway
Suite 212
East Cuy, MO 63045

No. 06648

STATE OF FLORIDA

PROFESSIONAL ENGINEER

0416204-HO 278

REF PIGGYBACK

DATE 01/02/2018

DRWG PB160160118

SPACING 24.0"

Piggyback Detail - ASCE 7-16: 180 mph, 30' Mean Hgt, Partially Enclosed, Exp. C, Kzt=1.00

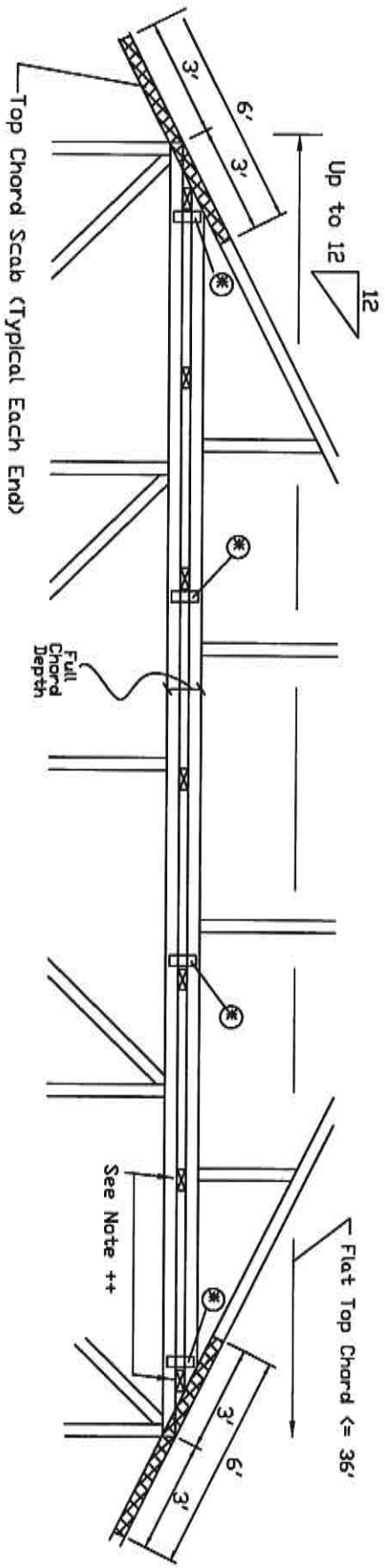
180 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Part Enclosed Bldg, located anywhere in roof, Exp C, Wind DL = 5.0 psf (wh), Kzt=1.0.
 Or 160 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Part Enclosed Bldg, located anywhere in roof, Exp D, Wind DL = 5.0 psf (wh), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.
 Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3.7") at 4' o.c.

Flat top chord purlins required at both ends and at a maximum of 24' intervals unless otherwise noted on base truss design drawing. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



In addition, provide connection with one of the following methods:	
Trulox. Use 3x8 Trulox plates for 2x4 chord member, and 3x10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.	28P3 Wave Piggyback Plate. One 28P3 wave piggyback plate to each face @ 8' o.c. Attach each to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.
APA Rated Gussset. 8"x8"x7/16" APA APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2.7) nails per gusset. (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.	2x4 Vertical Scabs. 2x4 SPF #2. Full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3.7) per scab. Cover cap bottom chord and (2) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

514 Earth Cn Expressway
 Suite 202
 Earth City, MO 63045

UNDESIGNED BEAMS AND PURLINS ALL NOTED ON THIS DRAWING
 UNDESIGNED BEAMS AND PURLINS ALL NOTED ON THIS DRAWING
 UNDESIGNED BEAMS AND PURLINS ALL NOTED ON THIS DRAWING

Trusses require advance care in fabricating, handling, bracing, and bracing. All trusses must be braced prior to erection. Trusses shall be braced by the fabricator. Trusses shall be braced by the fabricator. Trusses shall be braced by the fabricator.

Alpine, a division of IIV Building Components Group Inc. shall not be responsible for any deviation from the design, any failure to build the truss in accordance with ASCE 7-16, or for handling, shipping, or bracing of trusses.

A seal on this drawing or cover page listing the drawing indicates acceptance of professional engineering services by the Engineer of Record. The seal shall be placed on the drawing and the seal shall be the responsibility of the Engineer of Record. The seal shall be placed on the drawing and the seal shall be the responsibility of the Engineer of Record.

For more information see the job's general notes page and the following:

ALPINE: unalpine.com TRS: unalpine.com TRS: unalpine.com TRS: unalpine.com

STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 No. 66648
 01/02/2018
 01/02/2018

SPACING 24.0'

REF PIGGYBACK
 DATE 01/02/2018
 DRWG PB180160118

This drawing specifies repairs for a truss with broken chord or web member.

This design is valid only for single ply trusses with 2x4 or 2x6 broken members. No more than one break per chord panel and no more than two breaks per truss are allowed. Contact the truss manufacturer for any repairs that do not comply with this detail.

(B) = Damaged area, 12" max length of damaged section

(D) = Minimum nailing distance on each side of damaged area (B)

Species as damaged member. Apply one scab per face.
Minimum slide member length(s) = (2X1) + (CB)

Scab member length (S) must be within the broken panel.

Nail into 2x4 members using two (2) rows at 4" o.c., rows staggered.
 Nail into 2x6 members using three (3) rows at 4" o.c., rows staggered

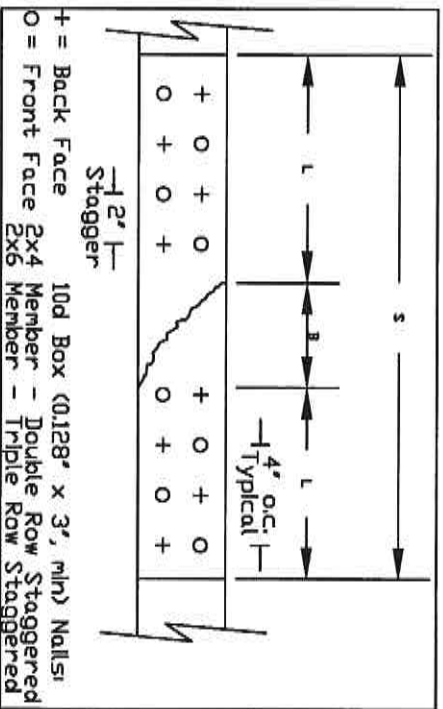
Nail using 10d box or gun nails (0.128"x3", min) into each side member.

The maximum permitted lumber grade for use with this detail is limited to Visual grade #1 and MSR grade 1650F.

This repair detail may be used for broken connector plate at mid-panel splices.

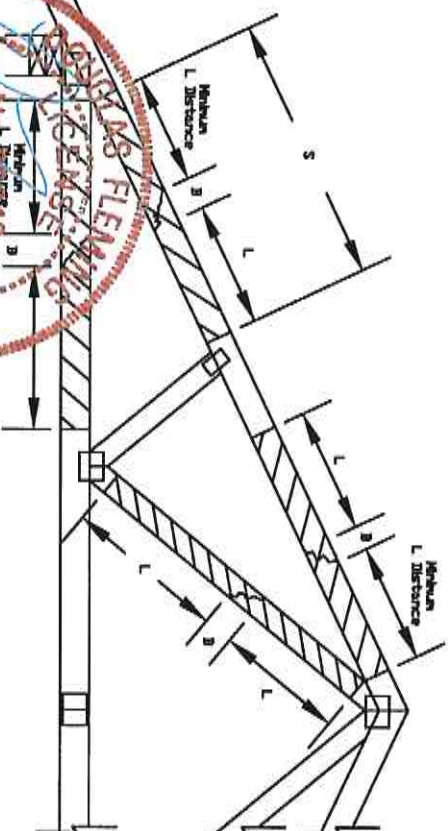
This repair detail may not be used for damaged chord or web sections occurring within the connector plate area.

Broken chord may not support any tie-in loads.



		Maximum Member Axial Force				
Member	Size	L	SPF-C	HF	DF-L	SYP
Web Only	2x4	12'	620#	635#	730#	800#
Web Only	2x4	18'	975#	1055#	1295#	1415#
Web or Chord	2x4	24'	975#	1055#	1495#	1745#
Web or Chord	2x6		1465#	1585#	2245#	2620#
Web or Chord	2x4	30'	1910#	1960#	2315#	2555#
Web or Chord	2x6		2230#	2365#	3125#	3575#
Web or Chord	2x4	36'	2470#	2530#	2930#	3210#
Web or Chord	2x6		3535#	3635#	4295#	4745#
Web or Chord	2x4	42'	2975#	3045#	3505#	3835#
Web or Chord	2x6		4395#	4500#	5225#	5725#
Web or Chord	2x4	48'	3460#	3540#	4070#	4445#
Web or Chord	2x6		5165#	5280#	6095#	6660#

Load Duration = 0%
Member forces may be increased for Duration of Load



REF MEMBER REPAIR

DATE 10/01/14

DRWG REPCHRD1014



514 Earth City Expressway
Suite 242
Earth City, MO 63045

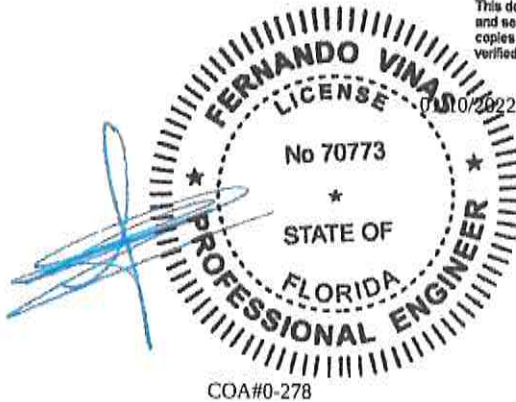
MANAGEMENT RECORD AND FUTURE ALL NOTES IN THIS DRAWING
IMPORTANTLY FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE DETAILER.

These negative active-care in Fabricating, handling, shipping, handling and packing. Be a
the latest edition of BCI Building Components Safety Information by TPI and SCSO for
to perform these operations. Installers shall provide temporary bracing for
a properly attached rigid cavity. Locations shown for permanent lateral bracing
bracing installed per BCI sections 33, 37, or 38, as applicable. Apply plates to re-
and position as shown above and on the Joint Details, unless noted otherwise.

to a division of TPI Building Components Group Inc. shall not be responsible for any deviations
any failure to comply with the stated in conferences with AISC/TPI 1, or for handling, shipping
on the drawing or to cause any injury by the drawing indicates acceptance of professional
responsibility solely for the design shown. The authority and use of the drawing
structure is the responsibility of the Building Designer per AISC/TPI 1, Item 4.

For more information see the Job's general notes page and those with asterisk
UNITED WALLSTEEL/CONCRETE TPI wallsteel.org SCSO steelconcrete.com/CONCRETE/IDC wallsteel.org

SPACING 24.0' MAX



This document has been electronically signed and sealed using a Digital Signature. Printed copies without an original signature must be verified using the original electronic signature.



Alpine, an ITW Company
6750 Forum Drive, Suite 305
Orlando, FL 32821
Phone: (800)755-6001
www.alpineitw.com

Site Information:	Page 1:
Customer: Seminole Trusses, Inc.	Job Number: B54908a
Job Description: RIOS/FIDEL RESIDENCE	
Address: 7313 WEST US HWY 90, Lake City, FL 32055	

Job Engineering Criteria:			
Design Code: FBC 7th Ed. 2020 Res.		IntelliVIEW Version: 21.01.03A	
		JRef #: 1Xc38570001	
Wind Standard: ASCE 7-16	Wind Speed (mph): 130	Design Loading (psf): 37.00	
Building Type: Closed			

This package contains general notes pages, 42 truss drawing(s) and 8 detail(s).

Item	Drawing Number	Truss
1	010.22.1204.21447	A1 33' Stepdown Hip
3	010.22.1203.00323	A1b 33' Stepdown Hip
5	010.22.1202.42607	A3 33' Stepdown Hip Girder
7	010.22.1202.44117	B2 33' Mono Hip
9	010.22.1203.51050	C1-SG 23' Gable
11	010.22.1203.37440	C2a 23' Common
13	010.22.1202.58910	C3 23' Stepdown Hip Girder
15	010.22.1202.39363	D2 23'8" Common
17	010.22.1204.04047	E1 14'7"8 Mono Hip
19	010.22.1204.05293	E3 14'7"8 Mono Hip
21	010.22.1200.08073	G1-G 15' Gable
23	010.22.1202.41057	G2a 15' Common
25	010.22.1204.24477	G4 14'7"8 Special
27	010.22.1202.55960	H1-G 13' Gable
29	010.22.1200.04670	L1-G 12' Gable
31	010.22.1203.26433	PB1 4'1"14 Common
33	010.22.1202.20423	JA 10'3"15 Hip Jack Girder
35	010.22.1202.37880	JC 6'1"8 End Jack
37	010.22.1204.35673	JE 3'2"3 Jack
39	010.22.1204.22800	JG 6'1"13 Jack
41	010.22.1204.13997	JJ 3'8"8 Hip Jack Girder
43	PB160160118	
45	REPCHRD1014	
47	GBLLETIN0118	
49	CNNAILSP1014	

Item	Drawing Number	Truss
2	010.22.1204.19820	A1a 33' Stepdown Hip
4	010.22.1202.28893	A2 33' Stepdown Hip
6	010.22.1200.06490	B1 33' Mono Hip
8	010.22.1200.09770	B3 33' Mono Hip Girder
10	010.22.1203.45450	C2 23' Common
12	010.22.1202.05937	C2b 23' Stepdown Hip
14	010.22.1203.13683	D1-SG 23'8"8 Gable
16	010.22.1202.36440	D3 23'8"8 Common Girder
18	010.22.1203.53943	E2 14'7"8 Mono Hip
20	010.22.1203.59653	E4 14'7"8 Mono Hip
22	010.22.1204.12160	G2 15' Common
24	010.22.1204.01473	G3 14'7"8 Special
26	010.22.1200.03050	G4a 14'7"8 Special
28	010.22.1204.17843	K1 12'3"8 Mono Hip Girder
30	010.22.1202.57447	L2 12' Common
32	010.22.1204.06487	PB2 3'4"8 Common
34	010.22.1204.38830	JB 6'1"8 End Jack
36	010.22.1204.34600	JD 4'7"11 Jack
38	010.22.1204.36707	JF 1'8"12 Jack
40	010.22.1204.33473	JH 3'4"13 Jack
42	010.22.1204.15687	JK 2'8" End Jack
44	PB180160118	
46	A14015ENC160118	
48	BRCLBSUB0119	
50	A14030ENC160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

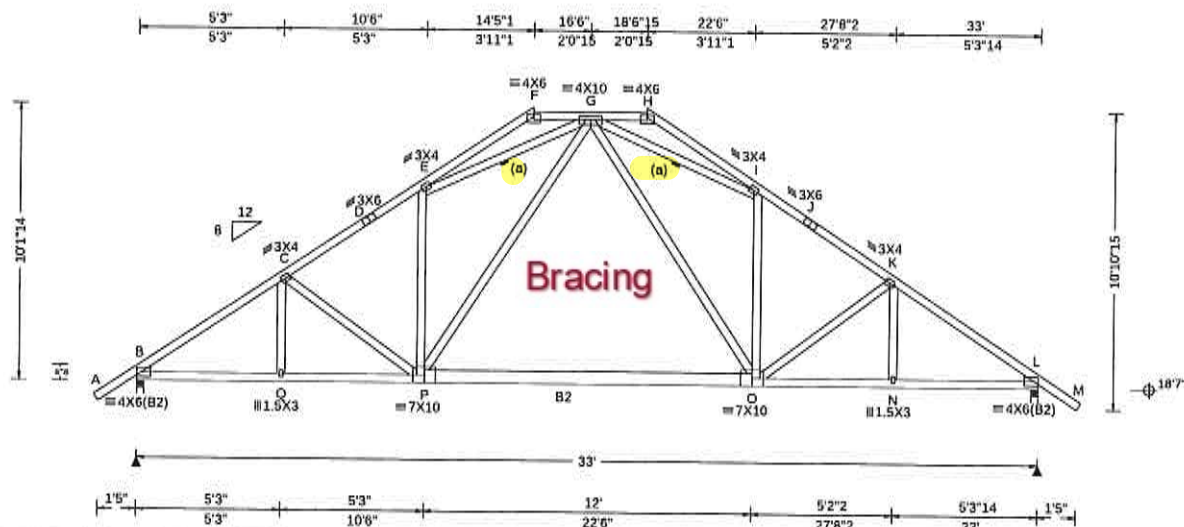
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 97562 FROM: RJL	HIPS Qty: 4	Ply: 1 Qty: 4	Job Number: 854908a RIOS/FIDEL RESIDENCE Truss Label: A1 33' Stepdown Hip	Cust: R 857 JRef: 1Xc38570001 T37 DrwNo: 010.22.1204.21447 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.41 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.30 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.095 P 999 360 VERT(CL): 0.165 P 999 240 HORZ(LL): 0.049 L - - HORZ(TL): 0.084 L - - Creep Factor: 2.0 Max TC CSI: 0.432 Max BC CSI: 0.676 Max Web CSI: 0.724 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1495 - / - / 736 / 94 / 239 L 1495 - / - / 736 / 94 / - Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.8 L Brg Wid = 3.0 Min Req = 1.8 Bearings B & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 280 - 2090 H - I 173 - 443 C - D 294 - 1886 I - J 305 - 1789 D - E 305 - 1788 J - K 294 - 1884 E - F 173 - 441 K - L 280 - 2089

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with Bd Box or Gun (0.113"x2.5", min.) nails @ 6" oc.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

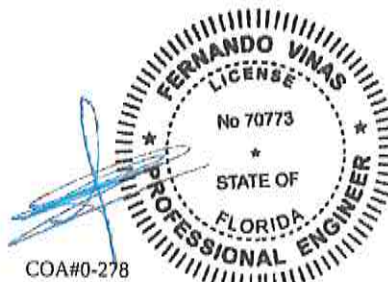
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - Q	1651 - 134	O - N	1651 - 141
Q - P	1653 - 135	N - L	1649 - 140
P - O	1209 - 33		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
E - G	230 - 1291	G - O	561 - 57
P - G	562 - 57	G - I	228 - 1288



COA#0-278

01/10/2022

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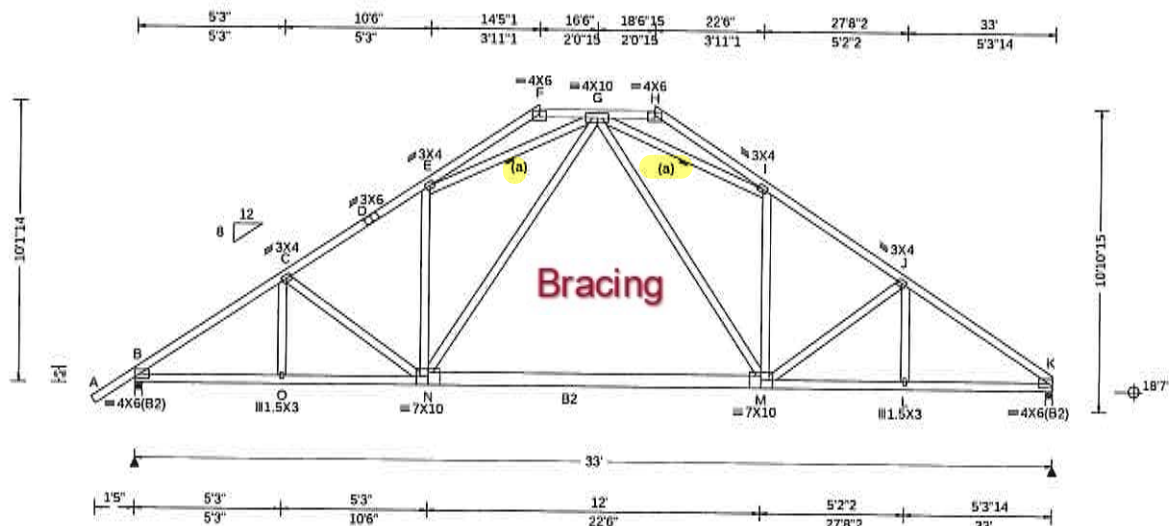
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97565 FROM: RJL	HIPS Qty: 3	Ply: 1 Qty: 3	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: A1a 33' Stepdown Hip	Cust: R 857 JRef: 1Xc38570001 T36 DrwNo: 010.22.1204.19820 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.41 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.30 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in Loc L/defl L/# VERT(LL): 0.094 N 999 360 VERT(CL): 0.164 N 999 240 HORZ(LL): 0.047 K - - HORZ(TL): 0.082 K - - Creep Factor: 2.0 Max TC CSI: 0.432 Max BC CSI: 0.677 Max Web CSI: 0.363 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1497 - / - / - / 736 / 95 / 224 K 1408 - / - / - / 669 / 77 / - Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.8 K Brg Wid = 3.0 Min Req = 1.7 Bearings B & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

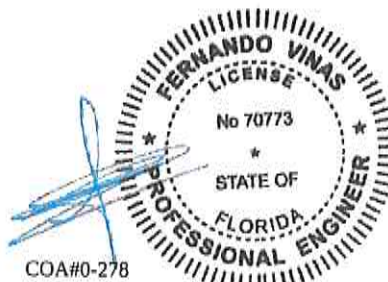
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - O	1654 -172	M - L	1671 -181
O - N	1656 -173	L - K	1670 -180
N - M	1212 -66		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
E - G	232 -1294	G - M	565 -59
N - G	561 -56	G - I	231 -1291



COA#0-278

01/10/2022

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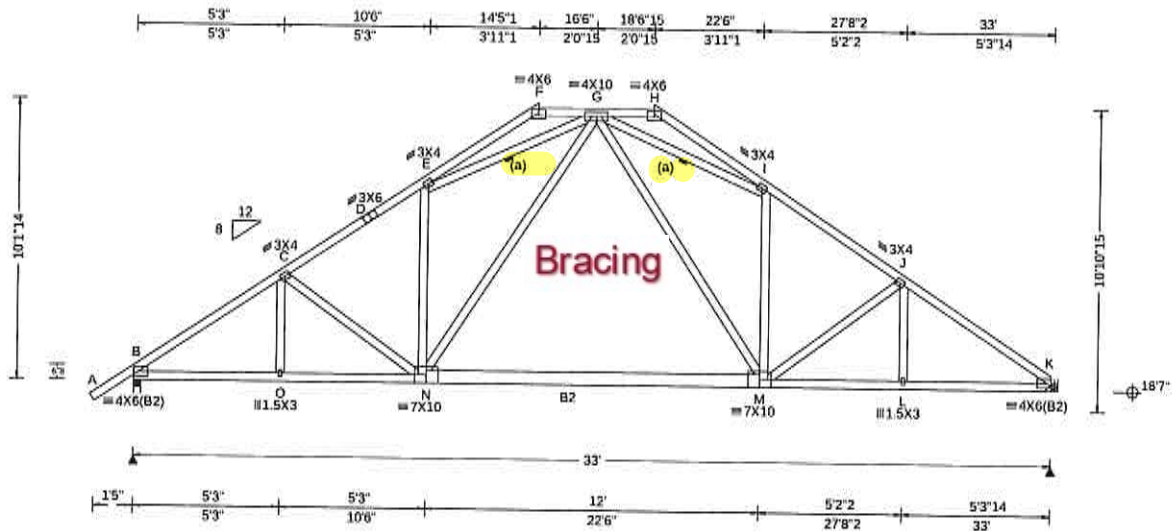
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97568 FROM: RJL	HIPS Qty: 6	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: A1b 33' Stepdown Hip	Cust: R 857 JRef: 1Xc38570001 T18 DrwNo: 010.22.1203.00323 SSB / FV 01/10/2022
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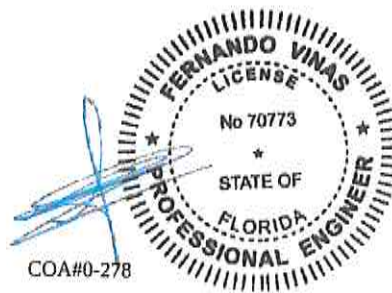


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.41 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.30 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.094 N 999 360 VERT(CL): 0.164 N 999 240 HORZ(LL): 0.047 K - - HORZ(TL): 0.082 K - - Creep Factor: 2.0 Max TC CSI: 0.432 Max BC CSI: 0.677 Max Web CSI: 0.363 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1497 /- /- /736 /95 /224 K 1408 /- /- /669 /77 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.8 K Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber	Special loads
Top chord: 2x4 SP #1; Bot chord: 2x4 SP #1; B2 2x6 SP #1; Webs: 2x4 SP #3;	----- (Lumber Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25) TC: From 57 plf at -1.58 to 57 plf at 33.00 PLB: From 40 plf at 12.95 to 40 plf at 20.05 BC: From 20 plf at 0.00 to 20 plf at 33.00

Bracing	Hangers / Ties
(a) Continuous lateral restraint equally spaced on member.	(J) Hanger Support Required, by others

Loading	Wind
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.	Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.

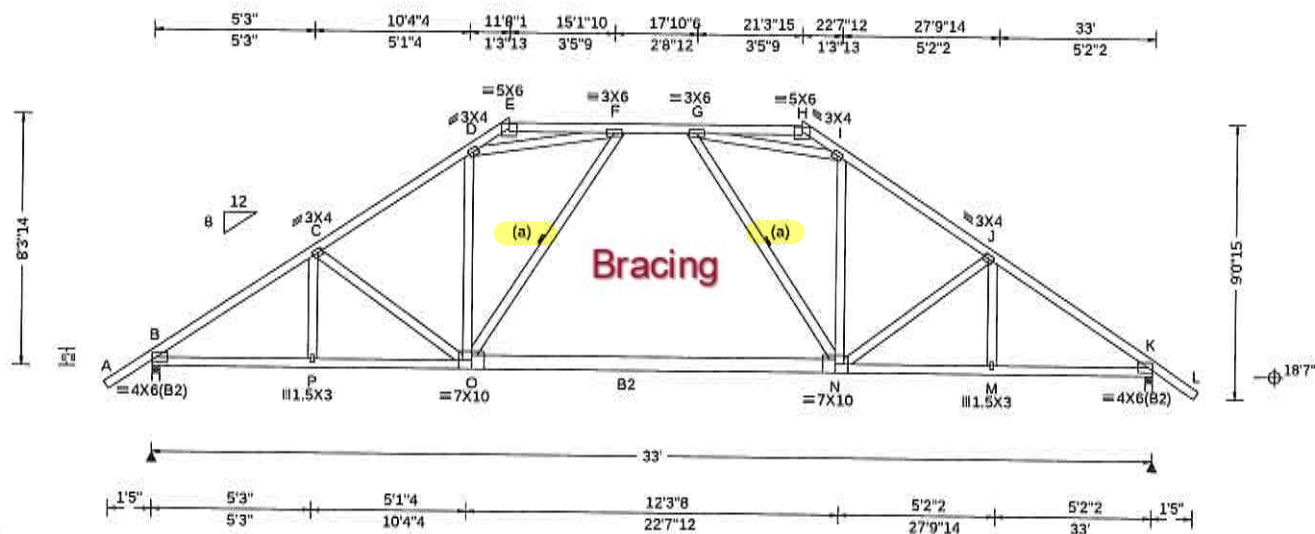


COA#0-278

01/10/2022

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information. Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org	ALPINE AN ITW COMPANY 6750 Forum Drive Suite 305 Orlando FL, 32821
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SEQN: 97573 FROM: RJL	HIPS Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: A2 33' Stepdwn Hip	Cust: R 857 JRef: 1Xc38570001 T8 DrwNo: 010.22.1202.28893 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 22.49 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.30 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.105 F 999 360 VERT(CL): 0.223 F 999 240 HORZ(LL): 0.051 K - - HORZ(TL): 0.088 K - - Creep Factor: 2.0 Max TC CSI: 0.433 Max BC CSI: 0.713 Max Web CSI: 0.659 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1499 /- /- /732 /193 /200 K 1499 /- /- /732 /193 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.8 K Brg Wid = 3.0 Min Req = 1.8 Bearings B & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with Bd Box or Gun (0.113"x2.5", min.) nails @ 6" oc.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Special loads

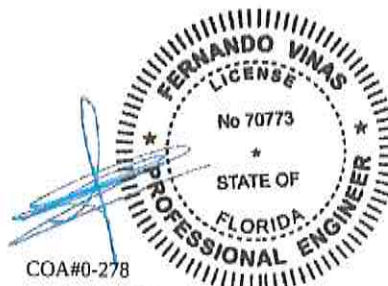
----- (Lumber Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25)
TC: From 57 plf at -1.58 to 57 plf at 34.58
PLB: From 40 plf at 12.84 to 40 plf at 20.16
BC: From 20 plf at 0.00 to 20 plf at 33.00

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - P	1657 -214	N - M	1657 -222
P - O	1659 -215	M - K	1655 -220
O - N	1501 -203		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - O	531 -50	G - I	289 -1328
D - F	292 -1334	N - I	532 -51



01/10/2022

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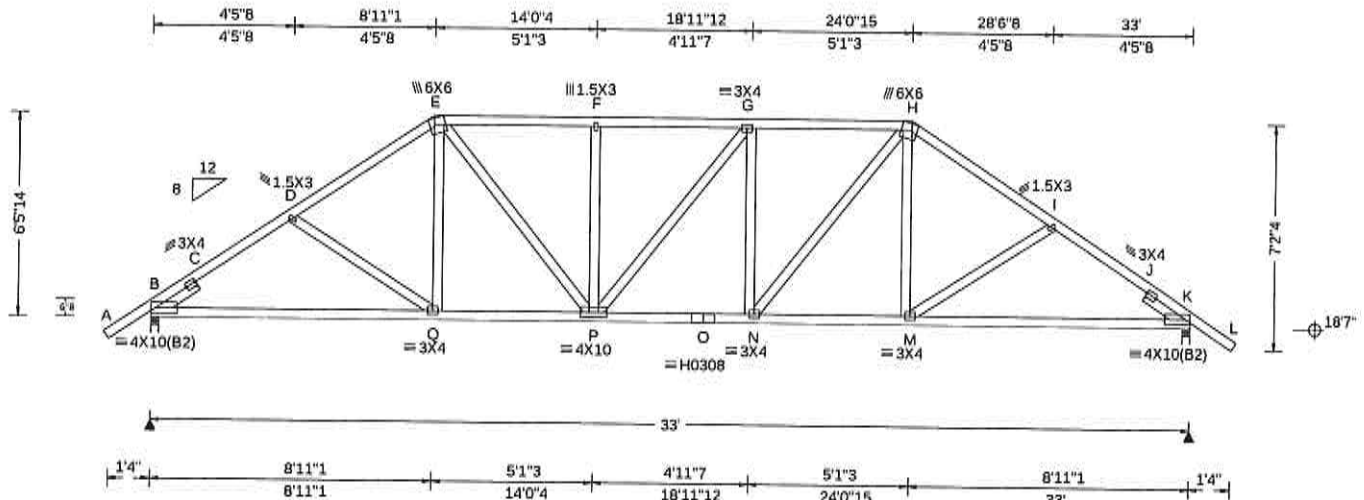
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97521 FROM: RJL	HIPS Qty: 1	Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: A3 33' Steppedown Hip Girder	Cust: R 857 JRef: 1Xc38570001 T13 DrwNo: 010.22.1202.42607 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.60 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.30 ft Loc. from endwall: NA GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.164 F 999 360 VERT(CL): 0.321 F 999 240 HORZ(LL): 0.072 K - - HORZ(TL): 0.140 K - - Creep Factor: 2.0 Max TC CSI: 0.535 Max BC CSI: 0.725 Max Web CSI: 0.416 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 2496 /- /- /- /241 /- K 2496 /- /- /- /241 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 2.9 K Brg Wid = 3.0 Min Req = 2.9 Bearings B & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 458 -3806 G - H 296 -3574 C - D 361 -3757 H - I 312 -3628 D - E 312 -3628 I - J 361 -3756 E - F 299 -3593 J - K 459 -3805 F - G 299 -3592

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'
Rt Slider: 2x4 SP #3; block length = 1.500'

Loading

#1 hip supports 6-1-8 jacks with no webs.
Left side jacks have 8-11-1 setback with 0-0-0 cant and 1-5-15 overhang. End jacks have 6-1-8 setback with 0-0-0 cant and 1-5-15 overhang. Right side jacks have 8-11-1 setback with 0-0-0 cant and 1-5-15 overhang.

Wind

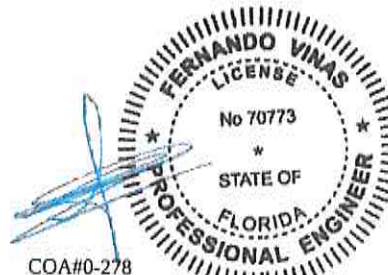
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - Q	3044 -282	O - N	3600 -303
Q - P	2972 -243	N - M	2972 -244
P - O	3600 -303	M - K	3043 -282

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
E - Q	789 0	G - N	160 -555
E - P	975 -88	N - H	961 -84
F - P	161 -546	H - M	785 0



01/10/2022

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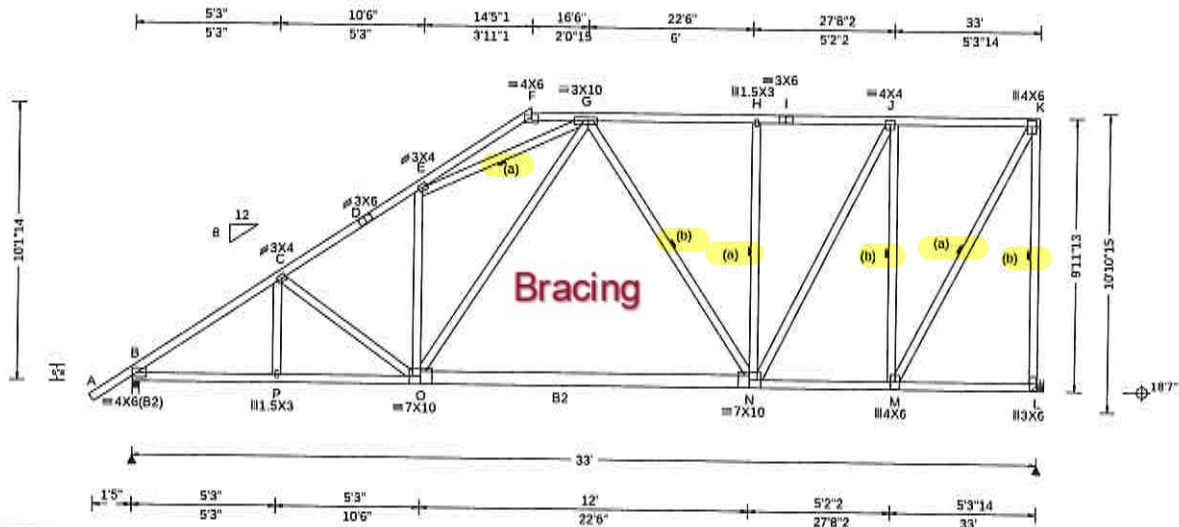
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97570 FROM: RJL	HIPM Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: B1 33' Mono Hip	Cust: R 857 JRef: 1Xc38570001 T3 DrwNo: 010.22.1200.06490 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCCL: 7.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.41 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.30 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.100 O 999 360 VERT(CL): 0.166 O 999 240 HORZ(LL): 0.033 C - - HORZ(TL): 0.055 C - - Creep Factor: 2.0 Max TC CSI: 0.442 Max BC CSI: 0.687 Max Web CSI: 0.961 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1530 /- /- /774 /36 /205 L 1603 /- /- /628 /193 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.8 L Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 318 -2148 H - I 344 -1206 C - D 347 -1946 I - J 344 -1206 D - E 359 -1849 J - K 214 -727 G - H 341 -1201

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement, 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.
(b) Continuous lateral restraint equally spaced on member. Or 2x6 #3 or better "T" reinforcement, 80% length of web member. Attached with 10d Box or Gun (0.128"x3", min.) nails @ 6" oc.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA#0-278

01/10/2022

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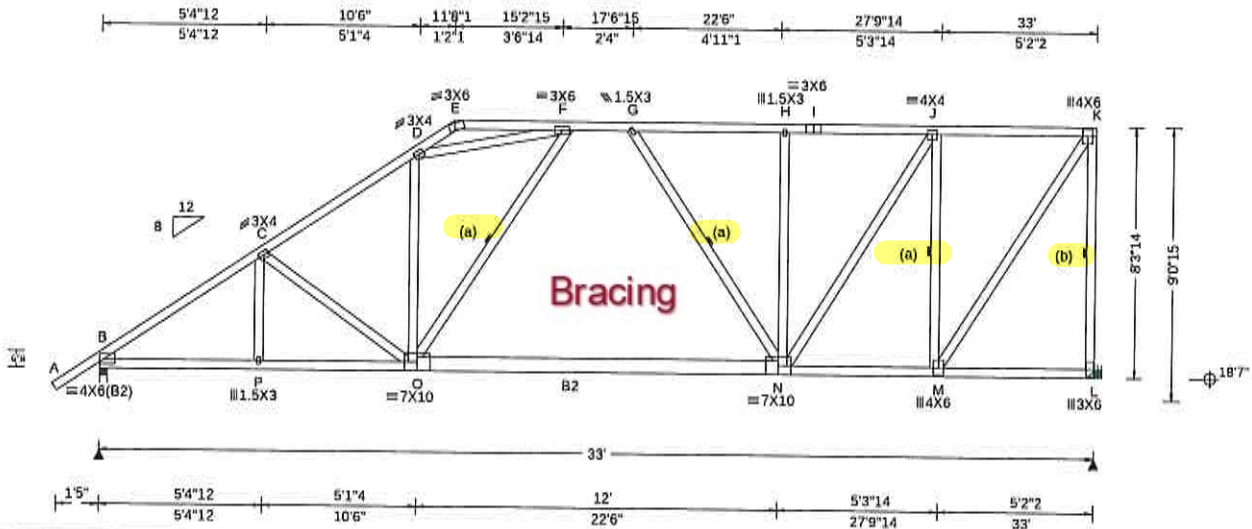
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97575 FROM: RJL	HIPM Qty: 1	Job Number: 854908a RIOS/FIDEL RESIDENCE Truss Label: B2 33' Mono Hip	Cust: R 857 JRef: 1Xc38570001 T14 DrwNo: 010.22.1202.44117 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 22.49 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.30 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Cc: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.117 G 999 360 VERT(CL): 0.233 G 999 240 HORZ(LL): 0.033 M - - HORZ(TL): 0.062 D - - Creep Factor: 2.0 Max TC CSI: 0.439 Max BC CSI: 0.716 Max Web CSI: 0.970 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1525 - / - / - / 757 / 45 / 167 L 1577 - / - / - / 608 / 173 / - Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.8 L Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.
(b) Continuous lateral restraint equally spaced on member. Or 2x4 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3", min.) nails @ 6" oc.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Special loads

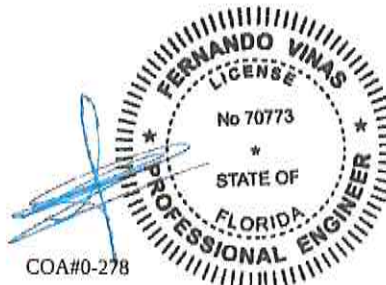
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 57 plf at -1.58 to 57 plf at 33.00
PLB: From 40 plf at 12.95 to 40 plf at 20.05
PLB: From 40 plf at 25.10 to 40 plf at 27.53
PLB: From 40 plf at 30.20 to 40 plf at 32.71
BC: From 20 plf at 0.00 to 20 plf at 33.00

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - P	1691 - 479	O - N	1529 - 483
P - O	1693 - 480	N - M	908 - 277

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - O	521 - 62	J - M	454 - 1170
D - F	353 - 1431	M - K	1595 - 480
N - J	992 - 266	K - L	494 - 1468



COA#0-278

01/10/2022

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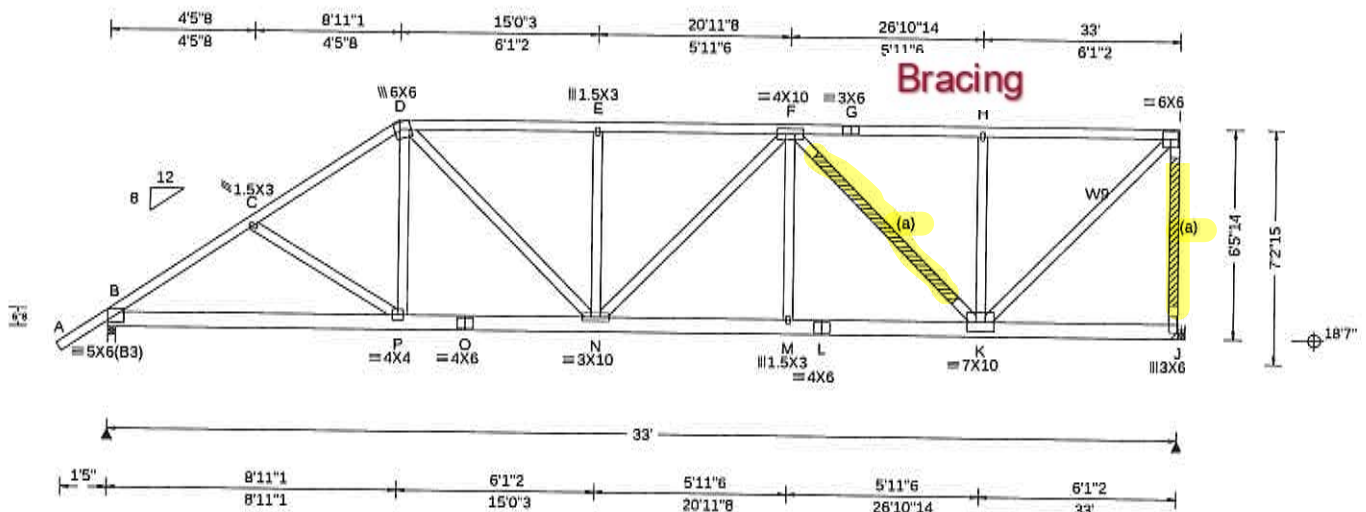
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97534 FROM: RJL	HIPM Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: B3 33' Mono Hip Girder	Cust: R 857 JRef: 1Xc38570001 T5 DrwNo: 010.22.1200.09770 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.57 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.30 ft Loc. from endwall: NA GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.149 E 999 360 VERT(CL): 0.296 E 999 240 HORZ(LL): 0.042 D - - HORZ(TL): 0.083 D - - Creep Factor: 2.0 Max TC CSI: 0.952 Max BC CSI: 0.696 Max Web CSI: 0.616 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 2433 /- /- /- /232 /- J 2526 /- /- /- /197 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 2.9 J Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

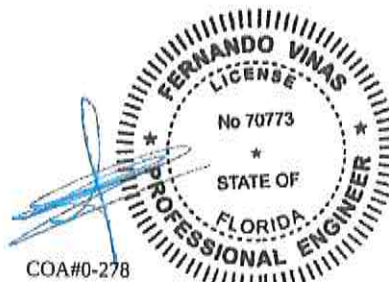
Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3; W9 2x4 SP #1;

Bracing
(a) #3 or better scab reinforcement. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" oc.

Hangers / Ties
(J) Hanger Support Required, by others

Loading
#1 hip supports 6-1-8 jacks with no webs.
Left side jacks have 8-11-1 setback with 0-0-0 cant and 1-6-15 overhang. End jacks have 6-1-8 setback with 0-0-0 cant and 1-6-15 overhang.

Wind
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

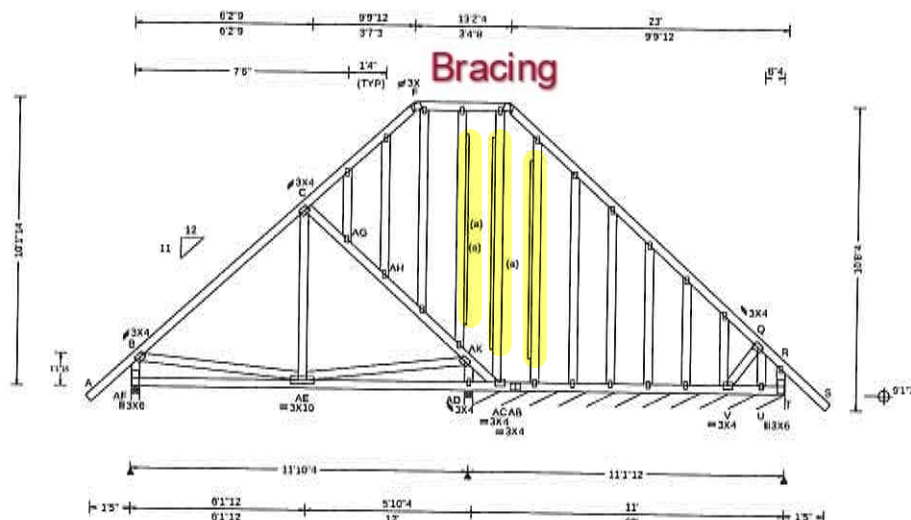


01/10/2022

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCEA: sbceacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97483 FROM: RJL	GABL Qty: 1	Ply: 1 Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: C1-SG 23' Gable	Cust: R 857 JRef: 1Xc38570001 T24 DrwNo: 010.22.1203.51050 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg. P1 in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.020 E 999 360 VERT(CL): 0.045 E 999 240 HORZ(LL): -0.015 AH - - HORZ(TL): 0.034 AH - - Creep Factor: 2.0 Max TC CSI: 0.466 Max BC CSI: 0.213 Max Web CSI: 0.178 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL AF 843 /- /- /371 /19 /275 AD 700 /- /- /408 /21 /- T* 104 /- /- /61 /17 /- V /-114 Wind reactions based on MWFRS AF Brg Wid = 3.5 Min Req = 1.5 AD Brg Wid = 3.5 Min Req = 1.5 T Brg Wid = 132 Min Req = - Bearings AF, AD, & AD are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) 1x4 #3SRB SPF-S or better "L" reinforcement.
80% length of web member. Attach with 8d Box or
Gun (0.113"x2.5", min.) nails @ 6" oc.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers
and cladding load not to exceed 5.00 PSF one face
and 24.0" span opposite face. Top chord must not be
cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C
member design.

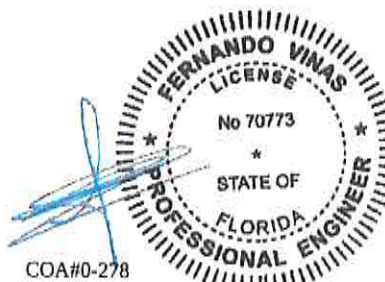
End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for
gable wind bracing and other requirements.

Laterally brace top chord below filler and bottom chord
above filler at 24" o.c., including a lateral brace
at chord ends (if no rigid diaphragm exists at that point).



COA#0-278

01/10/2022

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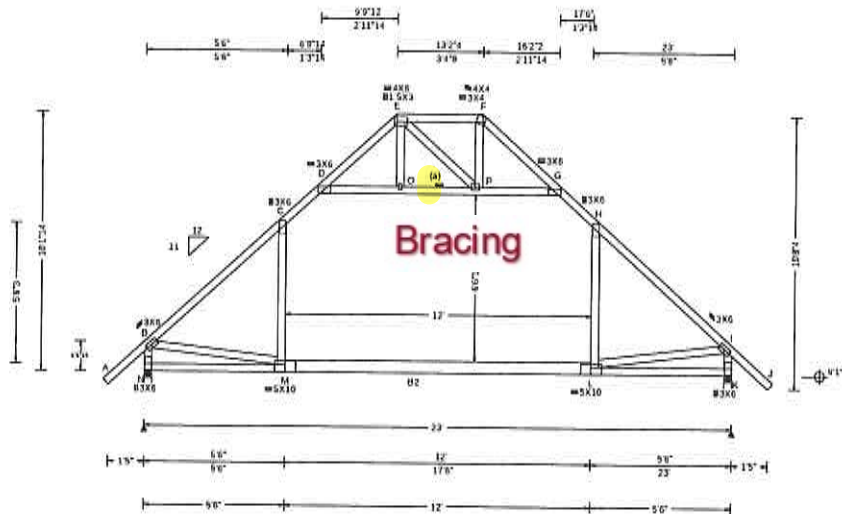
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AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97474 FROM: RJL	COMN	Ply: 1 Qty: 6	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: C2 23' Common	Cust: R 857 JRef: 1Xc38570001 T23 DrwNo: 010.22.1203.45450 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.348 L 792 360 VERT(CL): 0.647 L 426 240 HORZ(LL): -0.337 H - - HORZ(TL): 0.627 H - - Creep Factor: 2.0 Max TC CSI: 0.956 Max BC CSI: 0.772 Max Web CSI: 0.310 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL N 1238 - / - / - / 547 / 42 / 244 K 1238 - / - / - / 547 / 42 / - Wind reactions based on MWFRS N Brg Wid = 3.5 Min Req = 1.5 K Brg Wid = 3.5 Min Req = 1.5 Bearings N & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 199 -1332 G - H 272 -898 C - D 272 -898 H - I 199 -1332

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Collar-Tie braced with continuous lateral bracing @24" OC, or rigid sheathing

Special loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 40 plf at -1.61 to 40 plf at 24.61
TC: From 19 plf at -1.61 to 19 plf at 24.61
PLB: From 40 plf at 5.50 to 40 plf at 17.50
BC: From 20 plf at 0.00 to 20 plf at 23.00

Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens.Comp.

M - L 881 -2

Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp. Webs Tens. Comp.

B - N 239 -1230 P - G 196 -876
B - M 813 -35 L - H 440 0
C - M 450 0 L - I 813 -39
D - O 196 -874 I - K 239 -1230
O - P 196 -874



COA#0-278

01/10/2022

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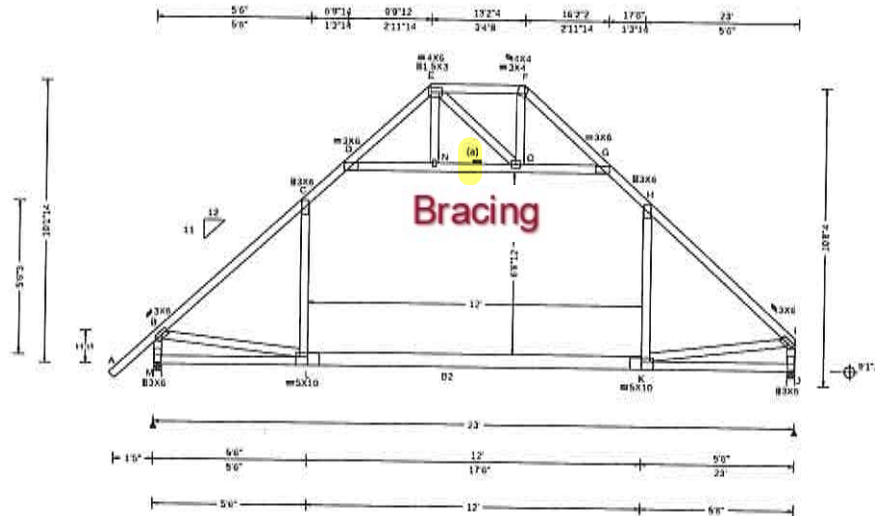
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97477 FROM: RJL	COMN Qty: 5	Ply: 1 Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: C2a 23' Common	Cust: R 857 JRef: 1Xc38570001 T22 DrwNo: 010.22.1203.37440 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pl in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed, 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in Loc L/defl L/H VERT(LL): 0.361 K 765 360 VERT(CL): 0.679 K 406 240 HORZ(LL): -0.350 H - - HORZ(TL): 0.659 H - - Creep Factor: 2.0 Max TC CSI: 0.985 Max BC CSI: 0.774 Max Web CSI: 0.312 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 1241 /- /- /548 /42 /225 J 1145 /- /- /476 /28 /- Wind reactions based on MWFRS M Brg Wid = 3.5 Min Req = 1.5 J Brg Wid = 3.5 Min Req = 1.5 Bearings M & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 202 -1340 G - H 275 -902 C - D 273 -903 H - I 194 -1334

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Collar-Tie braced with continuous lateral bracing @24" OC, or rigid sheathing

Special loads
----- (Lumber Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25)
TC: From 59 plf at -1.61 to 59 plf at 23.00
PLB: From 40 plf at 5.50 to 40 plf at 17.50
BC: From 20 plf at 0.00 to 20 plf at 23.00

Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp.
L - K 887 -53

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp. Webs Tens. Comp.
B - M 241 -1236 O - G 205 -886
B - L 820 -36 K - H 436 0
C - L 454 0 K - I 803 -28
D - N 200 -884 I - J 169 -1135
N - O 200 -884



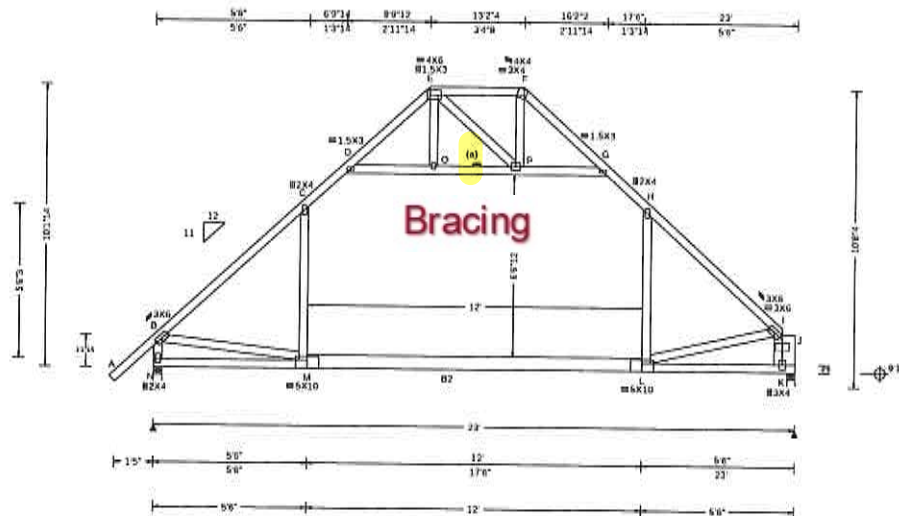
COA#0-278

01/10/2022

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AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97486 FROM: RJL	COMN Ply: 1 Qty: 2	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: C2b 23' Stepdown Hip	Cust: R 857 JRef: 1Xc38570001 T6 DrwNo: 010.22.1202.05937 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/H VERT(LL): 0.347 L 795 360 VERT(CL): 0.655 L 421 240 HORZ(LL): -0.337 H - - HORZ(TL): 0.637 H - - Creep Factor: 2.0 Max TC CSI: 0.967 Max BC CSI: 0.771 Max Web CSI: 0.534 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL N 1240 /- /- /548 /- /221 K 1119 /- /- /462 /- /- Wind reactions based on MWFRS N Brg Wid = 3.5 Min Req = 1.5 K Brg Wid = 3.5 Min Req = 1.5 Bearings N & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x6 SP #1;

Special loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 59 plf at -1.61 to 59 plf at 22.54
PLB: From 40 plf at 5.50 to 40 plf at 17.50
BC: From 20 plf at 0.00 to 20 plf at 23.00

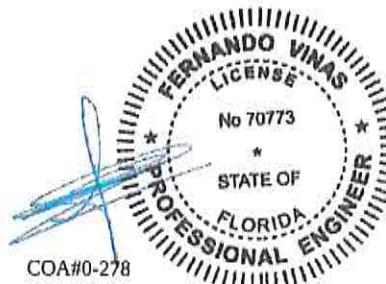
Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.
M - L 885 -28 L - K 413 -30

Bracing
(a) Continuous lateral restraint equally spaced on member.

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp. Webs Tens. Comp.
B - N 149 -1235 P - G 110 -880
B - M 818 -34 L - H 440 0
C - M 452 0 L - I 485 -24
D - O 109 -880 I - J 97 -1112
O - P 109 -880 J - K 170 -1225

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Collar-Tie braced with continuous lateral bracing @24" OC, or rigid sheathing
This truss has not been designed for habitation.
Truss not designed to be used as floor.
This truss must never be served by a fixed stairway.

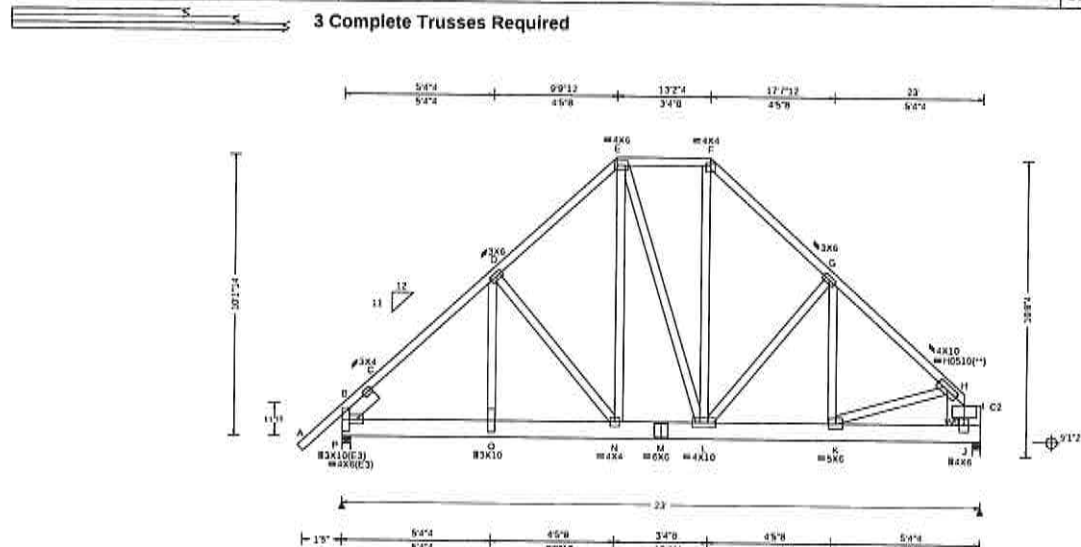


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AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97502 FROM: RJL	HIPS Qty: 1	Ply: 3 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: C3 23' Stepped Hip Girder	Cust: R 857 JRef: 1Xc39570001 T17 DrwNo: 010.22.1202.58910 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.058 N 999 360 VERT(CL): 0.111 N 999 240 HORZ(LL): 0.021 D - - HORZ(TL): 0.040 D - - Creep Factor: 2.0 Max TC CSI: 0.283 Max BC CSI: 0.217 Max Web CSI: 0.502 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL P 7758 -/- /- /366 -/ J 7653 -/- /- /352 -/ Wind reactions based on MWFRS P Brg Wid = 3.5 Min Req = 2.6 J Brg Wid = 3.5 Min Req = 2.6 Bearings P & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 129 -2728 E - F 69 -1533 C - D 126 -2710 F - G 100 -2086 D - E 99 -2062 G - H 125 -2657

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x8 SP SS Dense;
Webs: 2x4 SP #3; W9,C2 2x8 SP SS Dense;
Lt Slider: 2x6 SP #1; block length = 1.50'

Nailnote

Nail Schedule: 0.128"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 3.50" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Girder supports 33-0-0 span to BC one face and 2-0-0 span to TC/BC split opposite face.

Wind

Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Blocking

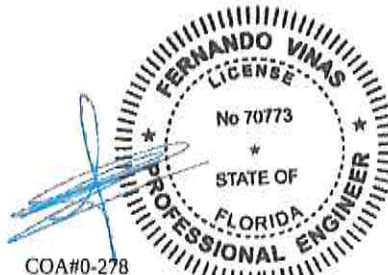
Apply additional nailing over the following bearings with fasteners at 4" oc both perpendicular and parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 5.50" if used)

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - O	1955 -88	M - L	1500 -68
O - N	1917 -86	L - K	1911 -87
N - M	1500 -68	K - J	1236 -56

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
O - D	939 -24	G - K	780 -17
D - N	31 -675	K - H	727 -32
E - N	1200 -42	H - I	97 -2003
L - F	1318 -47	I - J	612 -2859
L - G	30 -633		



COA#0-278

01/10/2022

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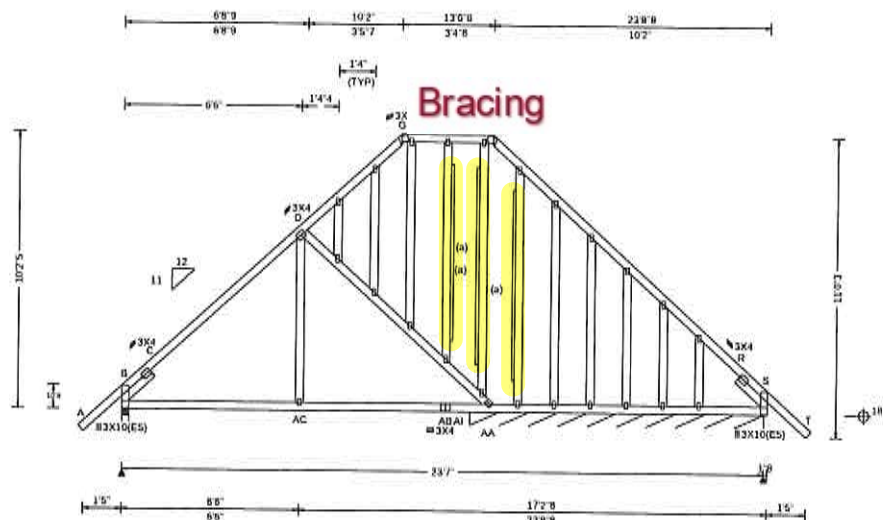
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97515 FROM: RJL	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: D1-SG 23'8" Gable	Cust: R 857 JRef: 1Xc38570001 T20 DrwNo: 010.22.1203.13683 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.38 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 10.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): -0.043 C 999 360 VERT(CL): 0.075 F 999 240 HORZ(LL): -0.050 C - - HORZ(TL): 0.062 C - - Creep Factor: 2.0 Max TC CSI: 0.658 Max BC CSI: 0.415 Max Web CSI: 0.190 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1103 /- /- /425 /103 /304 AI* 169 /- /- /76 /30 /- Non-Gravity Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 AI Brg Wid = 130 Min Req = - Bearings B & AI are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 290 -1268 G - K 191 -384 C - D 134 -1163 K - R 211 -613 D - G 206 -714 R - S 344 -693

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'
Rt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) 1x4 #3SRB SPF-S or better "L" reinforcement.
80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

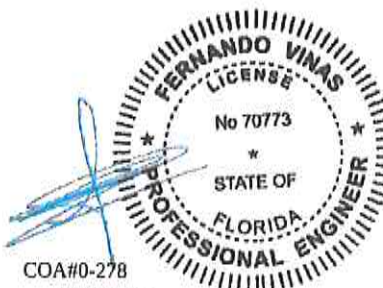
Wind loads based on MWFRS with additional C&C member design.

Right cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.



COA#0-278
01/10/2022

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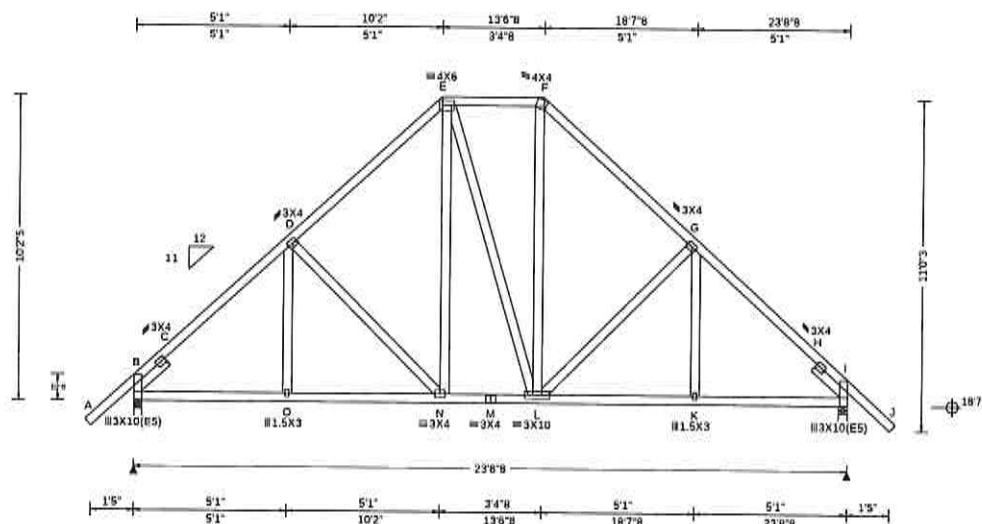
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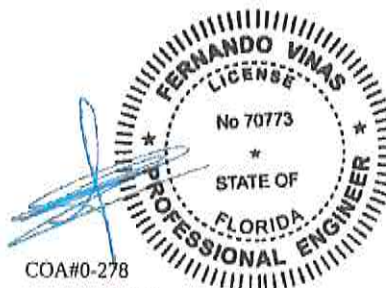
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Orlando FL, 32821



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.025 N 999 360	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.049 N 999 240	B 1026 /- /- /568 /49 /270
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.014 I - -	I 1026 /- /- /568 /49 /-
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.026 I - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 23.38 ft	Building Code:	Creep Factor: 2.0	B Brg Wid = 3.0 Min Req = 1.5
Soffit: 0.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.178	I Brg Wid = 3.5 Min Req = 1.5
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.217	Bearings B & I are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.270	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT: 20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18			
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.03A.0805.15	B - C 224 - 1138 F - G 264 - 856
Lumber				C - D 193 - 1093 G - H 192 - 1093
Top chord: 2x4 SP #1;				D - E 266 - 859 H - I 223 - 1138
Rel chord: 2x4 SP #1				E - F 242 - 556

Wind

Maximum Bot Chord Forces Per Ply (lbs)						
Chords		Tens.	Comp.	Chords		
		Tens.	Comp.			
B - O	738	-92		M - L	555	-39
O - N	736	-92		L - K	735	-24
N - M	555	-39		K - I	738	-24



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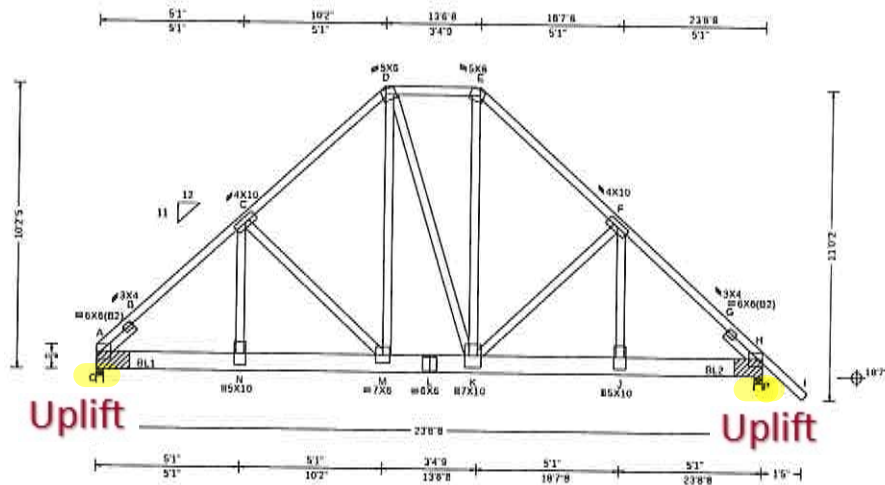
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6750 Forum Drive
Suite 305
Orlando FL, 32821

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.38 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Cs: NA Cg: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in Loc L/defl L/# VERT(LL): 0.106 M 999 360 VERT(CL): 0.195 M 999 240 HORZ(LL): 0.044 C - - HORZ(TL): 0.080 C - - Creep Factor: 2.0 Max TC CSI: 0.499 Max BC CSI: 0.469 Max Web CSI: 0.930 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL O 8801 /- /- /- /706 /- P 7020 /- /- /- /670 /- Wind reactions based on MWFRS O Brg Wid = 3.0 Min Req = - P Brg Wid = 3.5 Min Req = - Bearings O & P are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 425 -5064 E - F 349 -3790 B - C 415 -5035 F - G 427 -4637 C - D 334 -3747 G - H 437 -4668 D - E 250 -2807

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x8 SP SS Dense;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'
Rt Slider: 2x4 SP #3; block length = 1.500'

Nailnote

Nail Schedule: 0.126"x3" nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 2 Rows @ 5.00" o.c. (Each Row)
Webs: 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 59 plf at 0.00 to 59 plf at 13.54
TC: From 29 plf at 13.54 to 29 plf at 17.52
TC: From 59 plf at 17.52 to 59 plf at 25.32
BC: From 10 plf at 0.00 to 10 plf at 17.52
BC: From 20 plf at 17.52 to 20 plf at 23.71
BC: 1408 lb Conc. Load at 1.52, 3.52, 5.52, 7.52, 9.52, 11.52
BC: 1603 lb Conc. Load at 13.52
BC: 1577 lb Conc. Load at 15.52
BC: 2526 lb Conc. Load at 17.52

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Bearing Block(s)

Brg blocks: 0.126"x3" nails
brg x-loc #blocks length/blk #nails/blk wall plate
1 0.000' 1 14" 23 Rigid Surface
2 23.417' 1 12" 4 Rigid Surface
Brg block to be same size and species as chord.
Refer to drawing CNNAILSP1014 for more information.

Blocking

Apply additional nailing over the following bearings with fasteners at 4" oc both perpendicular and parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 3.50" if used)
Bearing 2 located at 23.4' (blocking >= 3.50" if used)

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - N	3668 -296	L - K	2722 -231
N - M	3617 -293	K - J	3321 -299
M - L	2722 -231	J - H	3337 -301

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
N - C	1697 -69	K - E	2442 -196
C - M	88 -1271	K - F	74 -771
D - M	2125 -111	F - J	1064 -65

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



01/10/2022

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

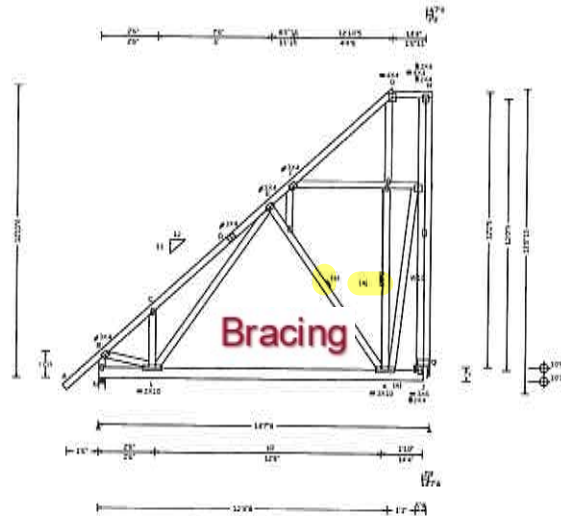
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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SEQN: 97450 FROM: RJL	SPEC	Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: E1 147"8 Mono Hip	Cust: R 857 JRef: 1Xc38570001 T29 DrwNo: 010.22.1204.04047 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.41 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.018 G 999 360 VERT(CL): 0.031 G 999 240 HORZ(LL): 0.014 C - - HORZ(TL): 0.023 C - - Creep Factor: 2.0 Max TC CSI: 0.202 Max BC CSI: 0.300 Max Web CSI: 0.320 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 762 /- /- /379 /- /229 Q 668 /- /- /411 /88 /- Wind reactions based on MWFRS M Brg Wid = 3.5 Min Req = 1.5 Q Brg Wid = 1.5 Min Req = 1.5 Bearings M & Q are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 -829 D - E 26 -735 C - D 14 -876

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3; W10 2x4 SP #1;
Rt Bearing Leg: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

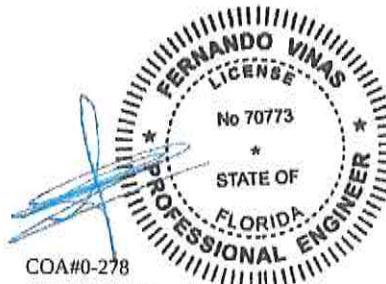
All plates are 1.5X3 except as noted.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



01/10/2022

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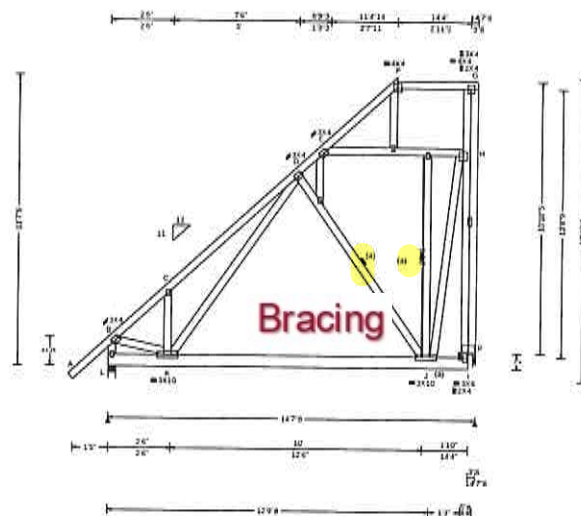
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AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97447 FROM: RJL	SPEC	Ply: 1 Qty: 2	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: E2 147"8 Mono Hip	Cust: R 857 JRef: 1Xc38570001 T25 DrwNo: 010.22.1203.53943 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.74 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.031 F 999 360 VERT(CL): 0.053 F 999 240 HORZ(LL): 0.022 G - - HORZ(TL): 0.040 G - - Creep Factor: 2.0 Max TC CSI: 0.188 Max BC CSI: 0.300 Max Web CSI: 0.298 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L 762 /- /- /389 /- /203 P 668 /- /- /376 /77 /- Wind reactions based on MWFRS L Brg Wid = 3.5 Min Req = 1.5 P Brg Wid = 1.5 Min Req = 1.5 Bearings L & P are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 -829 C - D 67 -876

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).



COA#0-278

01/10/2022

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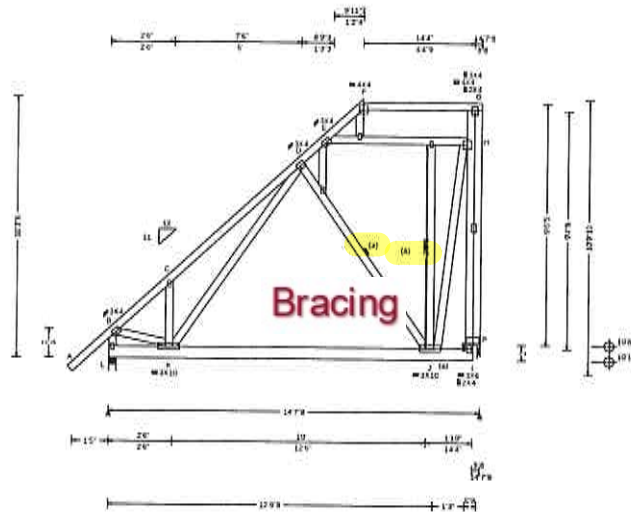
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97444 FROM: RJL	SPEC Qty: 2	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: E3 14'7"8 Mono Hip	Cust: R 857 JRef: 1Xc38570001 T30 DrwNo: 010.22.1204.05293 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.08 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.031 F 999 360 VERT(CL): 0.059 F 999 240 HORZ(LL): 0.021 H - - HORZ(TL): 0.044 H - - Creep Factor: 2.0 Max TC CSI: 0.213 Max BC CSI: 0.299 Max Web CSI: 0.313 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L 762 /- /- /394 /- /177 P 668 /- /- /345 /67 /- Wind reactions based on MWFRS L Brg Wid = 3.5 Min Req = 1.5 P Brg Wid = 1.5 Min Req = 1.5 Bearings L & P are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 -828 C - D 103 -873

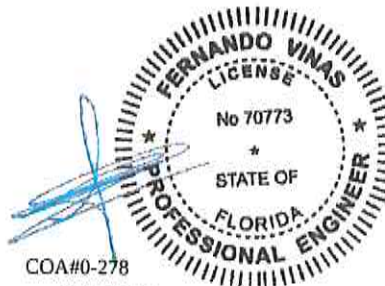
Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 1.5X3 except as noted.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)

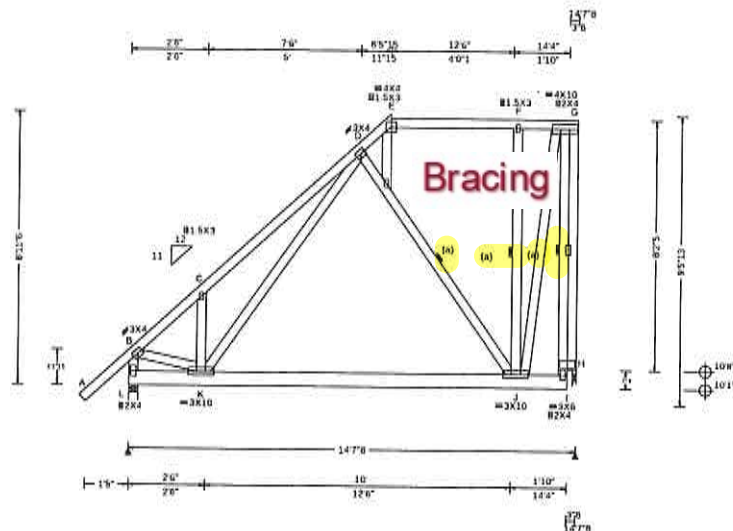


01/10/2022

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Suite 305
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SEQN: 97434 FROM: RJL	SPEC	Ply: 1 Qty: 2	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: E4 14'7" Mono Hip	Cust: R 657 JRef: 1Xc36570001 T26 DrwNo: 010.22.1203.59653 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.015 C 999 360 VERT(CL): 0.026 C 999 240 HORZ(LL): 0.013 C - - HORZ(TL): 0.022 C - - Creep Factor: 2.0 Max TC CSI: 0.199 Max BC CSI: 0.302 Max Web CSI: 0.371 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L 762 - / - / 395 - / 153 H 668 - / - / 319 / 60 - Wind reactions based on MWFRS L Brg Wid = 3.5 Min Req = 1.5 H Brg Wid = 1.5 Min Req = 1.5 Bearings L & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 18 -831 C - D 157 -882

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

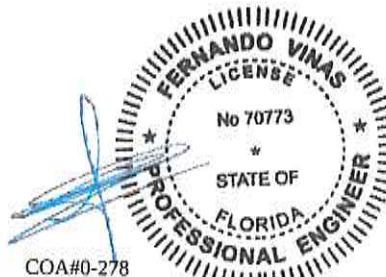
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA#0-278

01/10/2022

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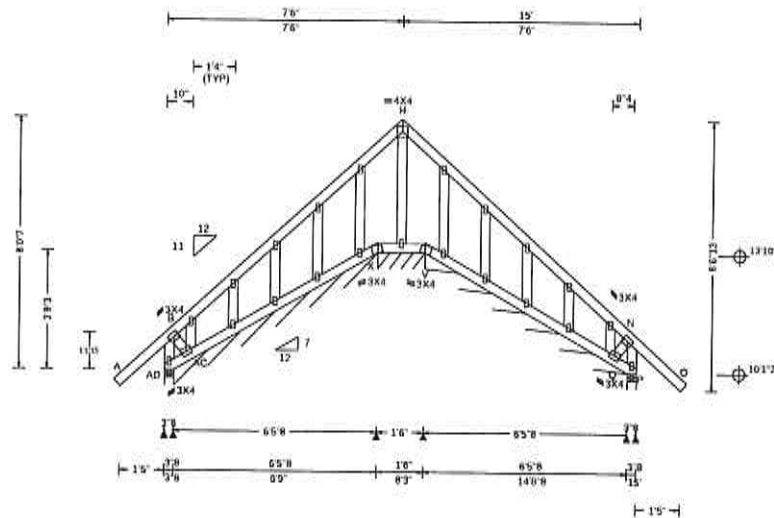
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97459 FROM: RJL	GABL Qty: 2	Ply: 1 Qty: 2	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: G1-G 15' Gable	Cust: R 857 JRef: 1Xc38570001 T4 DrwNo: 010.22.1200.08073 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCCL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCPl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/H VERT(LL): 0.001 H 999 360 VERT(CL): 0.001 H 999 240 HORZ(LL): 0.002 K - - HORZ(TL): 0.003 J - - Creep Factor: 2.0 Max TC CSI: 0.199 Max BC CSI: 0.032 Max Web CSI: 0.089 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL AD 225 - / - /333 /162 /224 AD*76 - / - /84 /36 - X* 88 - / - /194 /93 - V* 76 - / - /72 /36 - P 225 - / - /115 /60 - Wind reactions based on MWFRS AD Brg Wid = 3.5 Min Req = 1.5 AD Brg Wid = 77.5 Min Req = - X Brg Wid = 18.0 Min Req = - V Brg Wid = 77.5 Min Req = - P Brg Wid = 3.5 Min Req = 1.5 Bearings AD, AD, X, V, & P are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

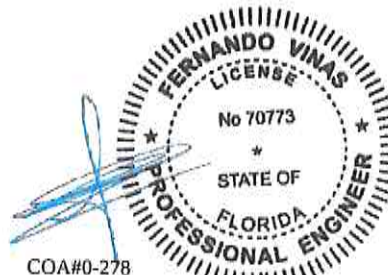
End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Shim all supports to solid bearing.



COA#0-278

01/10/2022

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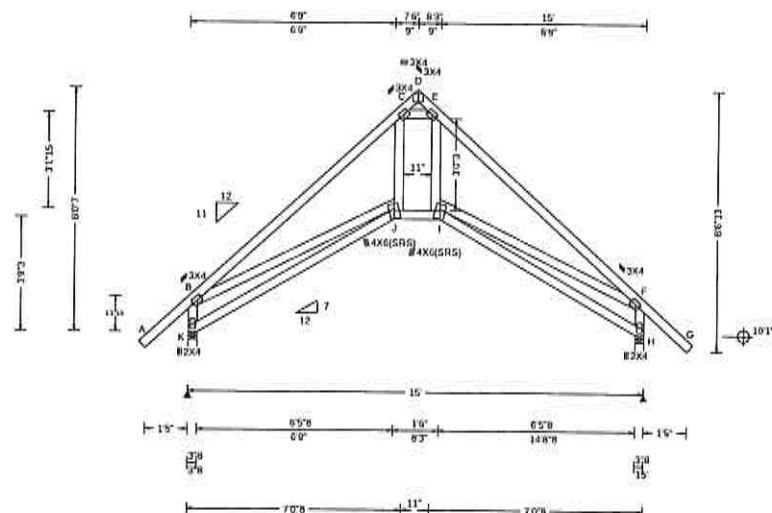
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97453 FROM: RJL	COMN	Ply: 1 Qty: 2	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: G2 15' Common	Cust: R 857 JRef: 1Xc38570001 T32 DrwNo: 010.22.1204.12160 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pl in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/H VERT(LL): 0.037 J 999 360 VERT(CL): 0.072 J 999 240 HORZ(LL): 0.045 H - - HORZ(TL): 0.087 H - - Creep Factor: 2.0 Max TC CSI: 0.393 Max BC CSI: 0.306 Max Web CSI: 0.293 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL K 703 /- /- /390 /20 /199 H 703 /- /- /390 /20 /- Wind reactions based on MWFRS K Brg Wid = 3.5 Min Req = 1.5 H Brg Wid = 3.5 Min Req = 1.5 Bearings K & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 49 -1165 E - F 38 -1165

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

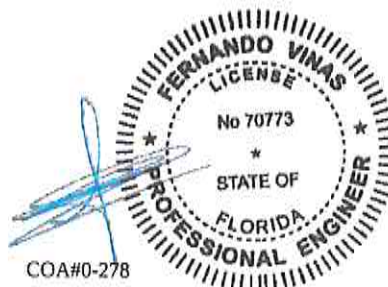
Chords Tens.Comp.

J - I 742 -60

Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp. Webs Tens. Comp.

B - K 261 -668 E - I 455 -54
B - J 768 0 I - F 768 -96
J - C 455 0 H - F 252 -668
C - E 619 -1144



01/10/2022

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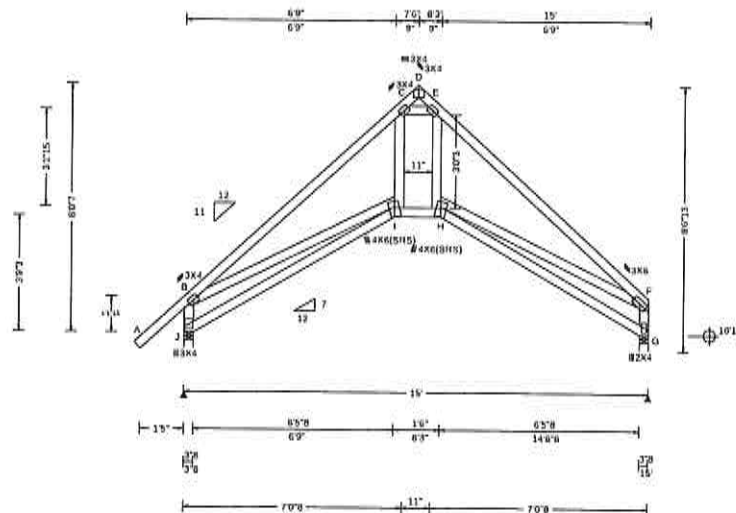
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97461 FROM: RJL	COMN	Ply: 1 Qty: 7	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: G2a 15' Common	Cust: R 057 JRef: 1Xc38570001 T12 DrwNo: 010.22.1202.41057 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.038 H 999 360 VERT(CL): 0.076 H 999 240 HORZ(LL): 0.045 G - - HORZ(TL): 0.089 G - - Creep Factor: 2.0 Max TC CSI: 0.426 Max BC CSI: 0.309 Max Web CSI: 0.299 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J 708 - / - / 391 / 20 / 180 G 609 - / - / 324 / 6 / - Wind reactions based on MWFRS J Brg Wid = 3.5 Min Req = 1.5 G Brg Wid = 3.5 Min Req = 1.5 Bearings J & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 158 - 1183 E - F 173 - 1176

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

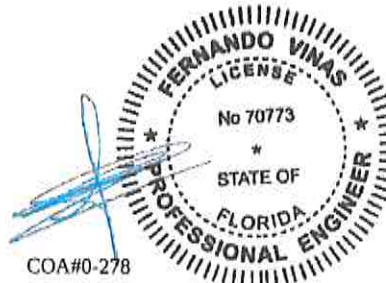
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens.Comp.
I - H 755 - 10

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - J	294 - 672	E - H	455 - 22
B - I	786 0	H - F	766 - 15
I - C	461 0	G - F	182 - 582
C - E	744 - 1178		



COA#0-278

01/10/2022

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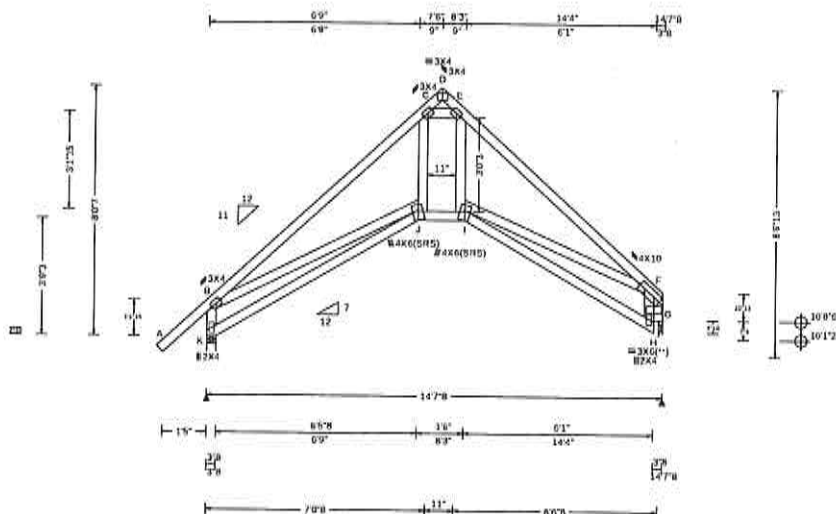
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97465 FROM: RJL	SPEC	Ply: 1 Qty: 4	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: G3 14'7"8 Special	Cust: R 857 JRef: 1Xc38570001 T27 DrwNo: 010.22.1204.01473 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCPI: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.037 J 999 360 VERT(CL): 0.073 J 999 240 HORZ(LL): 0.038 H - - HORZ(TL): 0.075 H - - Creep Factor: 2.0 Max TC CSI: 0.418 Max BC CSI: 0.315 Max Web CSI: 0.273 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL K 690 /- /- /382 /20 /177 G 589 /- /- /321 /7 /- Wind reactions based on MWFRS K Brg Wid = 3.5 Min Req = 1.5 G Brg Wid = 1.5 Min Req = 1.5 Bearings K & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 56 -1109 E - F 95 -1102

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

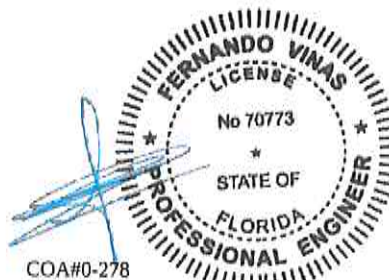
Chords	Tens.Comp.
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J - I 701 0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
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B - K 174 -654 E - I 442 -9
B - J 716 0 I - F 523 -88
J - C 443 0 F - G 478 -1131
C - E 427 -1044



COA#0-278

01/10/2022

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

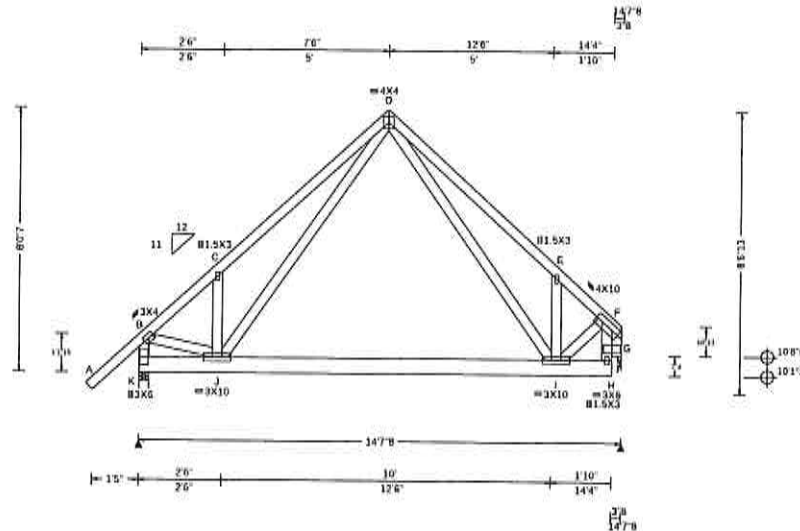
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.013 C 999 360	Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL	
	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.023 C 999 240	K	762	/-	/-	/371	/27	/177
	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.011 C - -	G	668	/-	/-	/298	/17	/-
	EXP: B Kzt: NA		HORZ(TL): 0.019 C - -	Wind reactions based on MWFRS						
	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	K	Brig Wid = 3.5 Min Req = 1.5					
	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.210	G	Brig Wid = 1.5 Min Req = 1.5					
	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.298	Bearings K & G are a rigid surface.						
	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.235	Members not listed have forces less than 375#						
C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)							
Loc. from endwall: not in 9.00 ft	Plate Type(s):		Chords	Tens.	Comp.	Chords	Tens.	Comp.		
GCpi: 0.18			B - C	57	- 831	D - E	192	- 823		
Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.03A.0805.15	C - D	193	- 876	E - F	30	- 719		

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

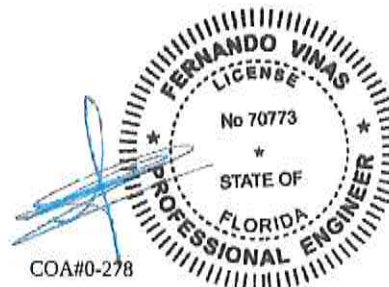
Special loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	59 plf at	-1.61 to	59 plf at	14.62
PLB: From	40 plf at	5.11 to	40 plf at	9.89
BC: From	20 plf at	0.00 to	20 plf at	14.33

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - K	83 - 867	D - I	377 - 85
B - J	616 0	I - F	579 - 19
J - D	449 - 90	F - G	31 - 769



COA#0-278

01/10/2022

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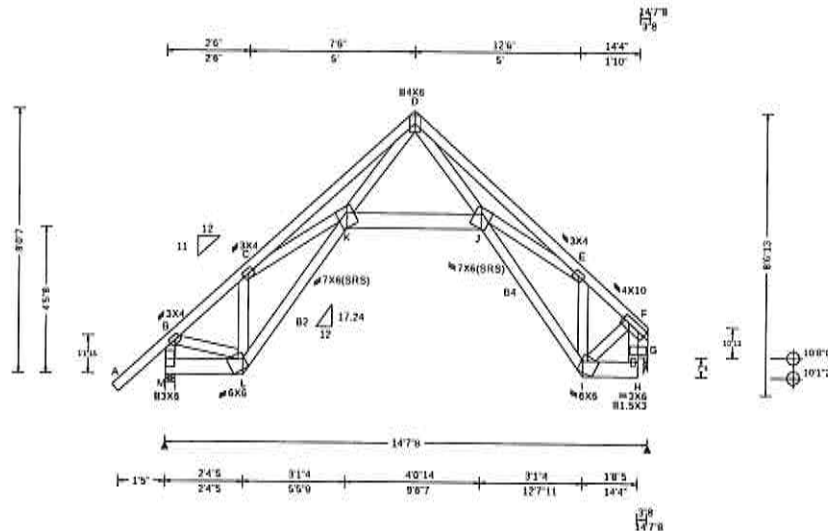
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97429 FROM: RJL	SPEC Qty: 2	Ply: 1 Qty: 2	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: G4a 14'7"8 Special	Cust: R 857 JRef: 1Xc38570001 T1 DrwNo: 010.22.1200.03050 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.101 K 999 360 VERT(CL): 0.208 K 841 240 HORZ(LL): 0.176 H - - HORZ(TL): 0.362 H - - Creep Factor: 2.0 Max TC CSI: 0.304 Max BC CSI: 0.226 Max Web CSI: 0.572 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 714 /- /- /416 /4 /177 G 618 /- /- /346 /- /- Wind reactions based on MWFRS M Brg Wid = 3.5 Min Req = 1.5 G Brg Wid = 1.5 Min Req = 1.5 Bearings M & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 61 -657 D - E 0 -2120 C - D 0 -2174 E - F 41 -544

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1; B2, B4 2x4 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Wind

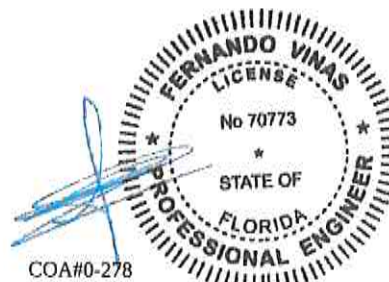
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
L - K	821 -105	J - I	740 -64
K - J	714 0		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - M	87 -710	D - J	1427 0
B - L	473 0	J - E	1206 0
L - C	105 -667	E - I	98 -707
C - K	1200 0	I - F	398 -29
K - D	1501 0	F - G	9 -577



01/10/2022

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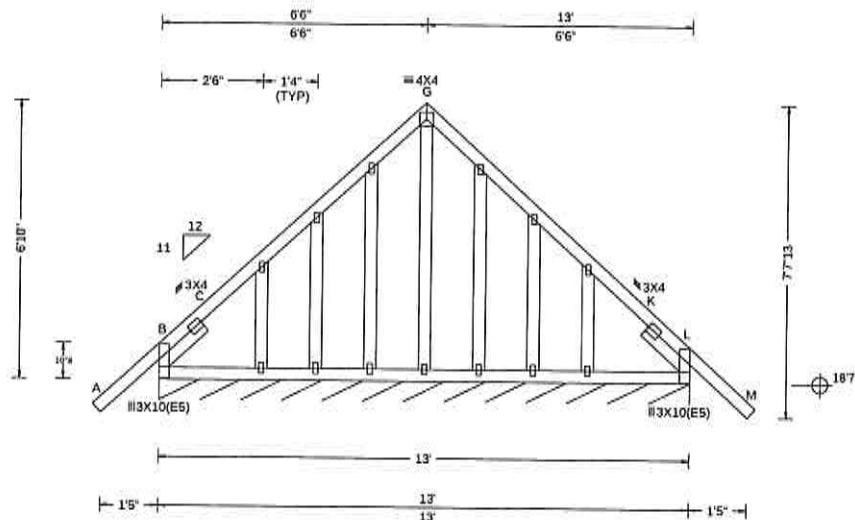
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97512 FROM: RJL	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: H1-G 13' Gable	Cust: R 857 JRef: 1Xc38570001 T15 DrwNo: 010.22.1202.55960 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pl in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.70 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 5.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): -0.002 K 999 360 VERT(CL): 0.004 K 999 240 HORZ(LL): -0.005 C - - HORZ(TL): 0.006 C - - Creep Factor: 2.0 Max TC CSI: 0.140 Max BC CSI: 0.030 Max Web CSI: 0.152 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L* 111 /- /- /50 /18 /17 Wind reactions based on MWFRS L Brg Wid = 156 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'
Rt Slider: 2x4 SP #3; block length = 1.500'

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

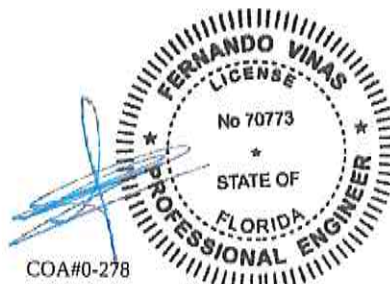
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.



COA#0-278

01/10/2022

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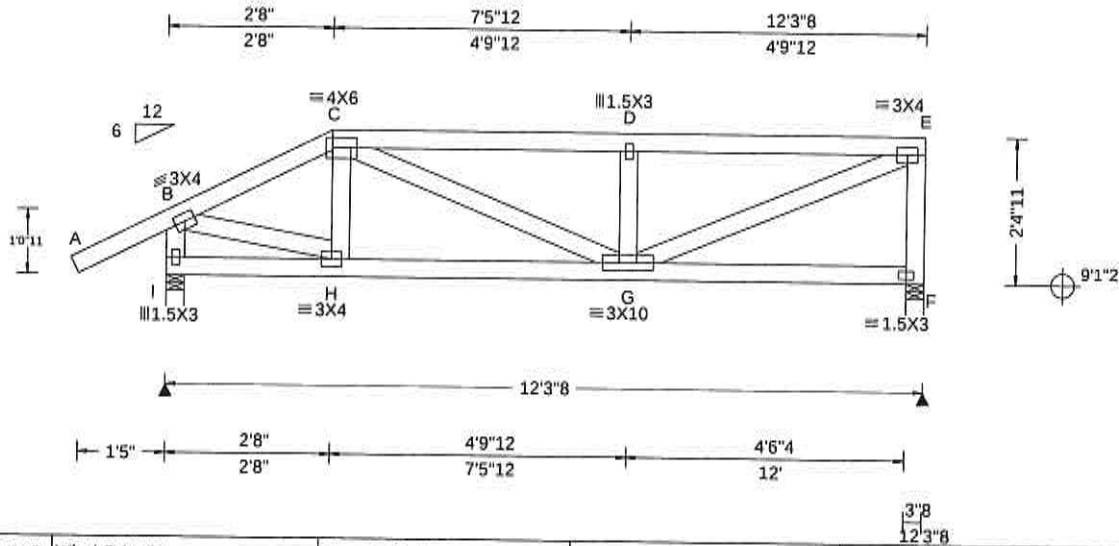
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97506 FROM: RJL	HIPM Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: K1 12'3" Mono Hip Girder	Cust: R 857 JRef: 1Xc38570001 T35 DrwNo: 010.22.1204.17843 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,PI in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 10.43 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.016 D 999 360 VERT(CL): 0.030 D 999 240 HORZ(LL): 0.004 C - - HORZ(TL): 0.007 C - - Creep Factor: 2.0 Max TC CSI: 0.175 Max BC CSI: 0.132 Max Web CSI: 0.268 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL I 531 - / - / - / 32 - F 401 - / - / - / 4 - Wind reactions based on MWFRS I Brg Wid = 3.5 Min Req = 1.5 F Brg Wid = 3.5 Min Req = 1.5 Bearings I & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 6 -538 D - E 5 -654 C - D 5 -654

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Loading

#1 hip supports 2-8-0 jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens.Comp.

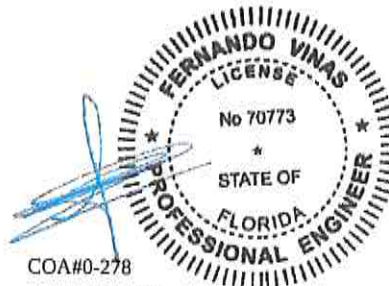
H - G 448 0

Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp. Webs Tens. Comp.

B - I 40 -514 G - E 703 -6

B - H 465 0



01/10/2022

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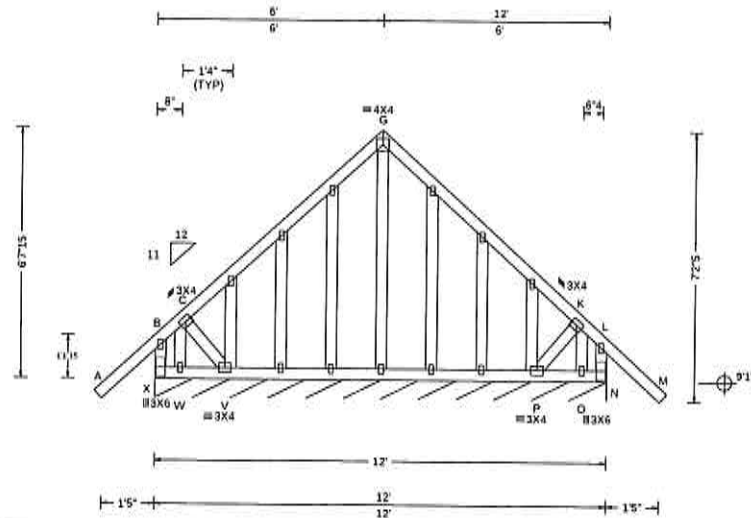
Alpine, a division of ITW Building Components Group Inc, shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97470 FROM: RJL	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: L1-G 12' Gable	Cust: R 857 JRef: 1Xc38570001 T2 DrwNo: 010.22.1200.04670 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 G 999 360 VERT(CL): 0.001 F 999 240 HORZ(LL): -0.003 B - - HORZ(TL): 0.003 B - - Creep Factor: 2.0 Max TC CSI: 0.179 Max BC CSI: 0.024 Max Web CSI: 0.195 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL X* 113 /- /- /51 /16 /16 Wind reactions based on MWFRS X Brg Wid = 144 Min Req = - Bearing X is a rigid surface. Members not listed have forces less than 375# Maximum Gable Forces Per Ply (lbs) Gables Tens. Comp. Gables Tens. Comp. X - B 398 - 239 K - O 277 - 435 W - C 242 - 433 L - N 397 - 236

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

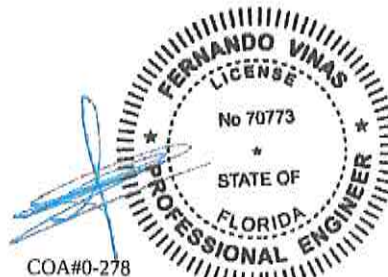
Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.



COA#0-278

01/10/2022

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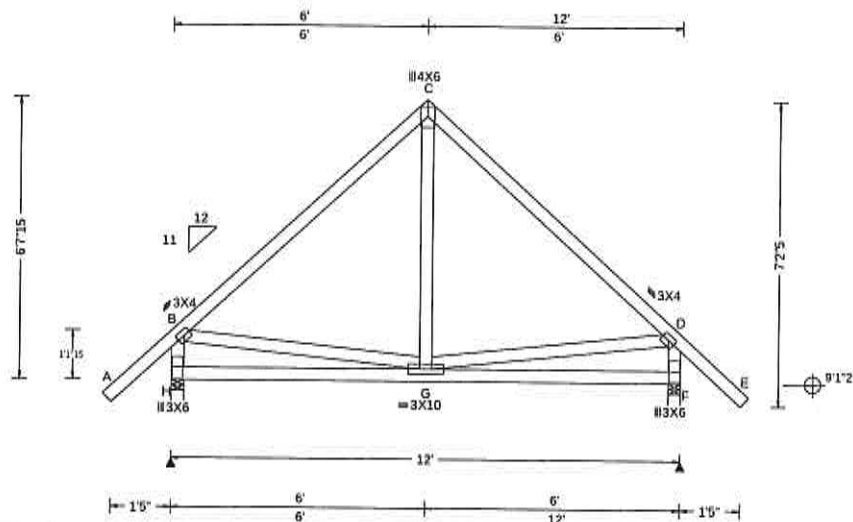
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97467 FROM: RJL	COMN Qty: 1	Ply: 1 Qty: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: L2 12' Common	Cust: R 857 JRef: 1Xc38570001 T16 DrwNo: 010.22.1202.57447 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 G 999 360 VERT(CL): 0.009 G 999 240 HORZ(LL): 0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.285 Max BC CSI: 0.237 Max Web CSI: 0.137 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 563 - / - / 315 / 25 / 170 F 563 - / - / 315 / 25 / - Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 F Brg Wid = 3.5 Min Req = 1.5 Bearings H & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 166 -458 C - D 166 -458

Lumber

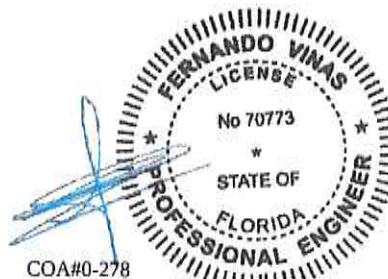
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - H	251 -515	D - F	251 -515



COA#0-278

01/10/2022

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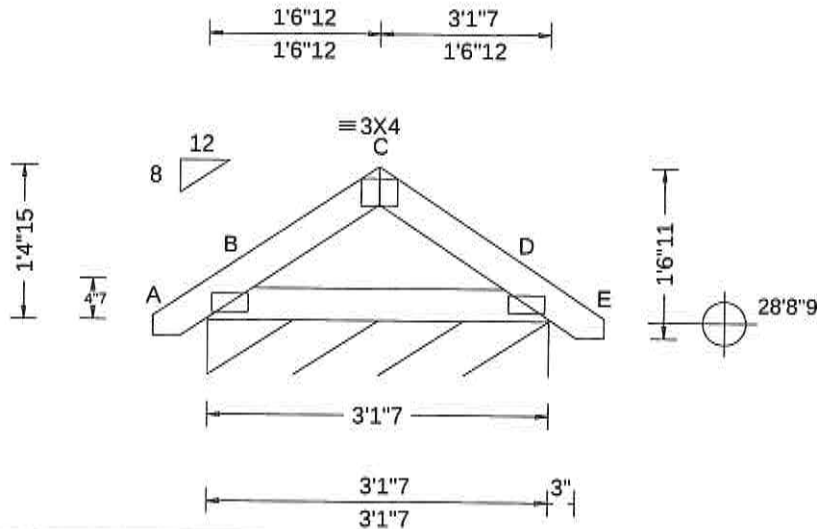
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97580 FROM: RJL	GABL Ply: 1 Qty: 12	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: PB1 4"1"14 Common	Cust: R 857 JRef: 1Xc38570001 T21 DrwNo: 010.22.1203.26433 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCCL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 29.44 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 360 VERT(CL): 0.001 B 999 240 HORZ(LL): 0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.015 Max BC CSI: 0.040 Max Web CSI: 0.000 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B* 95 /- /- /44 /24 /11 Wind reactions based on MWFRS B Brg Wid = 37.4 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

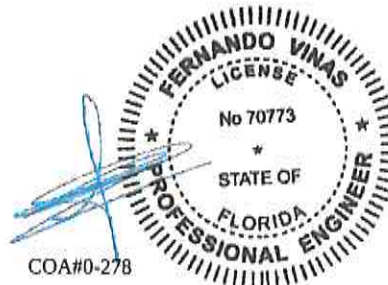
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;

Plating Notes

All plates are 2X4(A1) except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.
Refer to drawing PB160160118 or piggyback detail.



COA#0-278

01/10/2022

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

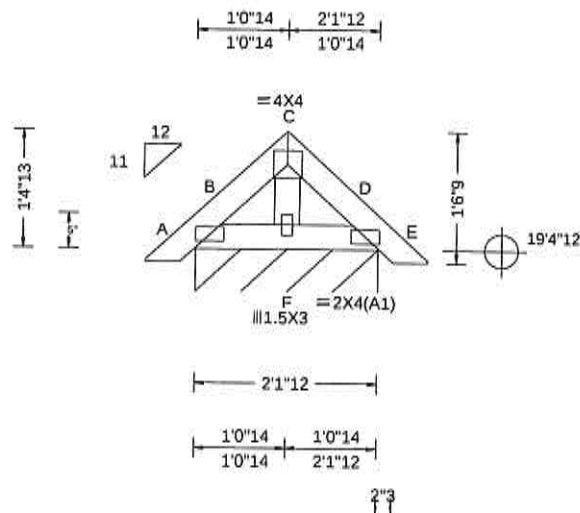
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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6750 Forum Drive
Suite 305
Orlando FL, 32821



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg. Pl in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/def L/#	Gravity Non-Gravity
TCDL: 7.00	Speed: 130 mph	Pl: NA Ce: NA	VERT(LL): 0.000 C 999 360	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.000 C 999 240	B* 105 /- /- /55 /30 /20
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.000 B - -	Wind reactions based on MWFRS
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.000 B - -	B Brg Wid = 25.8 Min Req = -
NCBCLL: 10.00	Mean Height: 20.03 ft	Building Code:	Creep Factor: 2.0	Bearing B is a rigid surface.
Soffit: 0.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.015	Members not listed have forces less than 375#
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.005	
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Varies by Ld Case	Max Web CSI: 0.008	
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		
	Loc. from endwall: not in 10.00 ft	Plate Type(s):		
	GCpl: 0.18	WAVE	VIEW Ver: 21.01.03A.0805.15	
	Wind Duration: 1.60			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3:

Plating Notes

All plates are 2X4(B1) except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Refer to drawing PB160160118 or piggyback detail.



COA#0-278

01/10/2022

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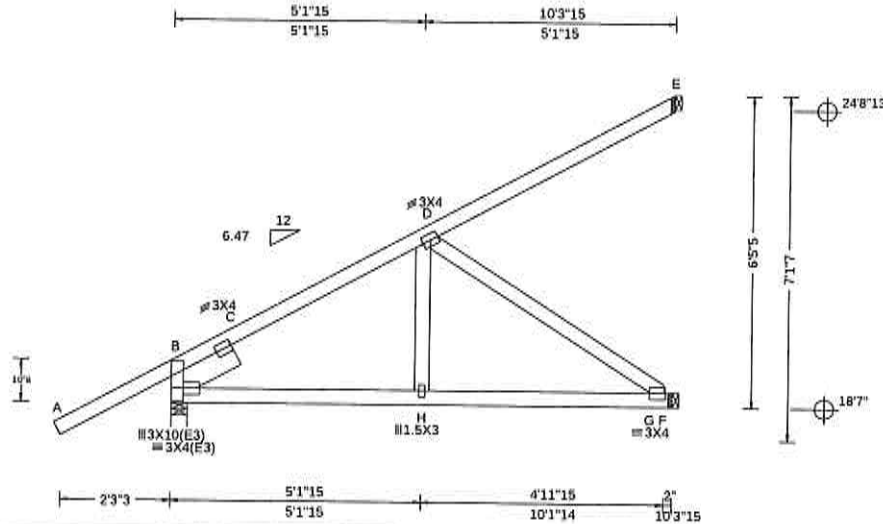
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97536 FROM: RJL	HIP_	Ply: 1 Qty: 3	Job Number: 854908a RIOS/FIDEL RESIDENCE Truss Label: JA 10'3"15 Hip Jack Girder	Cust: R 857 JRef: 1Xc38570001 T7 DrwNo: 010.22.1202.20423 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCDL: 7.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.59 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): -0.023 C 999 360 VERT(CL): -0.043 C 999 240 HORZ(LL): -0.020 C - - HORZ(TL): 0.038 C - - Creep Factor: 2.0 Max TC CSI: 0.554 Max BC CSI: 0.340 Max Web CSI: 0.369 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 449 /- /- /- /71 /- F 423 /- /- /- /4 /- E 225 /- /- /- /77 /- Wind reactions based on MWFRS B Brg Wid = 4.1 Min Req = 1.5 F Brg Wid = 1.5 E Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x6 SP #1; block length = 1.50'

Loading

Hipjack supports 7-3-10 setback jacks. Jacks up to 7' have no webs. Longer jacks supported to BC.

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

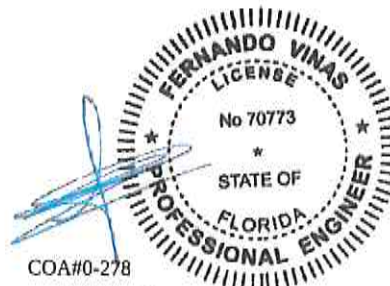
B - C 66 - 530 C - D 64 - 523

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - H	429 - 51	H - G	425 - 52

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
D - G	63 - 516



01/10/2022

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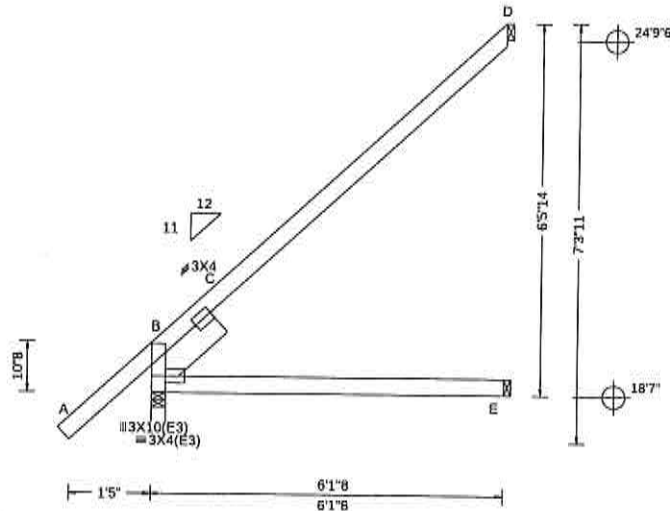
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCE) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. Refer to job's General Notes page for additional information.

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCE: sbcecomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97538 FROM: RJL	EJAC Qty: 15	Ply: 1 Qty: 15	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: JB 6'1"8 End Jack	Cust: R 857 JRef: 1Xc38570001 T57 DrwNo: 010.22.1204.38830 SSB / FV 01/10/2022
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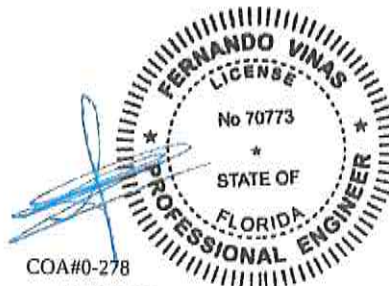
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, PI in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.53 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.058 C - - HORZ(TL): 0.107 C - - Creep Factor: 2.0 Max TC CSI: 0.487 Max BC CSI: 0.310 Max Web CSI: 0.070 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 342 /- /- /203 /- /208 E 117 /- /- /68 /- /- D 158 /- /- /118 /126 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 373 - 672

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x6 SP #1; block length = 1.500'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



01/10/2022

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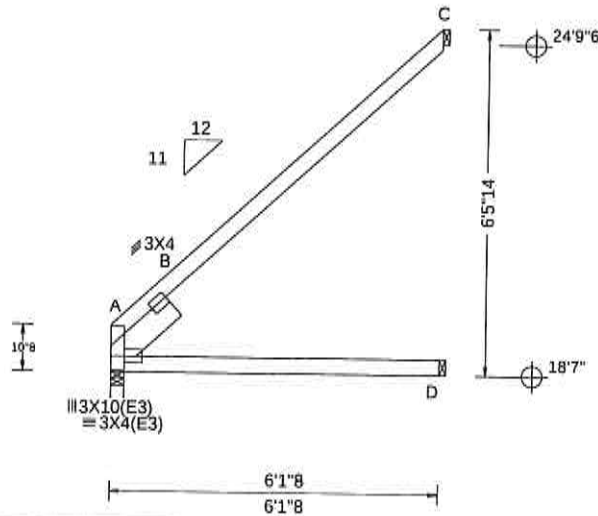
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97540 FROM: RJL	EJAC	Ply: 1 Qty: 7	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: JC 6'1"8 End Jack	Cust: R 857 JRef: 1Xc38570001 T10 DrwNo: 010.22.1202.37880 SSB / FV 01/10/2022
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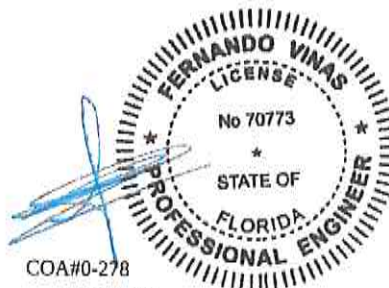
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCCL: 7.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 22.27 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCPl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.066 B - - HORZ(TL): 0.130 B - - Creep Factor: 2.0 Max TC CSI: 0.523 Max BC CSI: 0.317 Max Web CSI: 0.093 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 242 /- /- /120 /- /135 D 119 /- /- /72 /- /- C 165 /- /- /125 /96 /- Wind reactions based on MWFRS A Brg Wid = 3.0 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. A - B 611 -911

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x6 SP #1; block length = 1.500'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



01/10/2022

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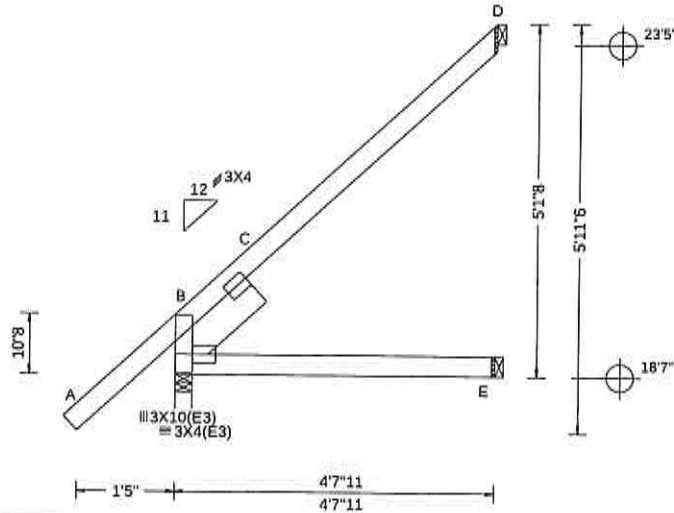
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97542 FROM: RJL	JACK Qty: 3	Ply: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: JD 4'7"11 Jack	Cust: R 857 JRef: 1Xc38570001 T47 DrwNo: 010.22.1204.34600 SSB / FV 01/10/2022
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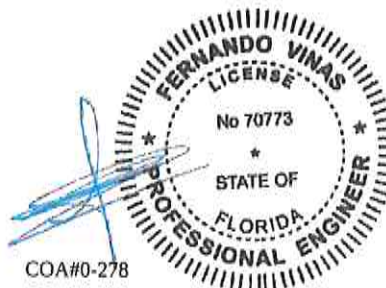
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.84 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.026 C - - HORZ(TL): 0.042 C - - Creep Factor: 2.0 Max TC CSI: 0.281 Max BC CSI: 0.168 Max Web CSI: 0.026 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 287 /- /- /176 /- /162 E 89 /- /- /50 /- /- D 116 /- /- /87 /94 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x6 SP #1; block length = 1.500'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



01/10/2022

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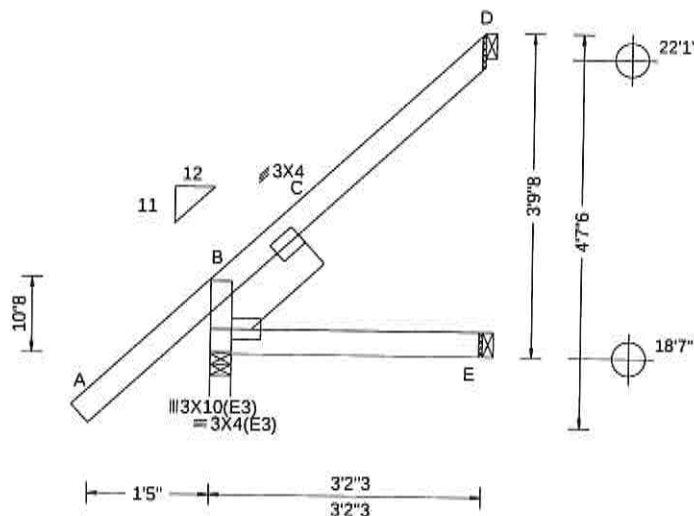
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Orlando FL, 32821

SEQN: 97544 FROM: RJL	JACK	Ply: 1 Qty: 3	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: JE 3'2"3 Jack	Cust: R 857 JRef: 1Xc38570001 T48 DrwNo: 010.22.1204.35673 SSB / FV 01/10/2022
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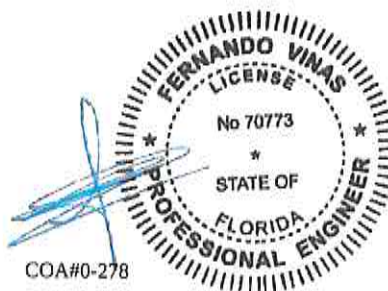
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.18 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.009 C - - HORZ(TL): 0.011 C - - Creep Factor: 2.0 Max TC CSI: 0.173 Max BC CSI: 0.073 Max Web CSI: 0.010 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 236 /- /- /153 /- /119 E 60 /- /- /34 /- /- D 71 /- /- /53 /63 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x6 SP #1; block length = 1.50'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



COA#0-278

01/10/2022

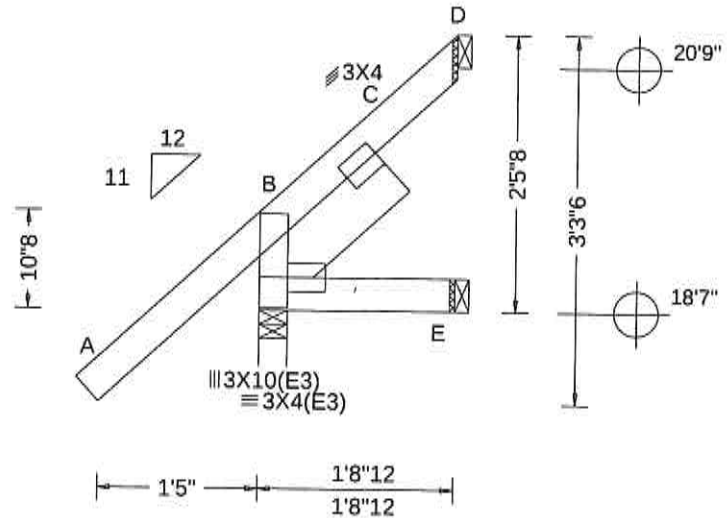
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
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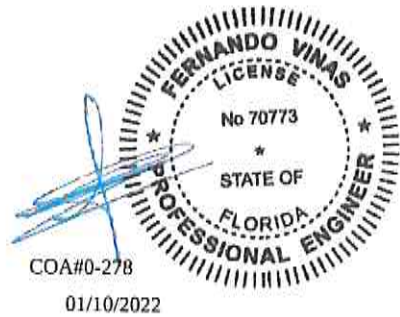
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AN ITW COMPANY
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Suite 305
Orlando FL, 32821



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.51 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed, 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.213 Max BC CSI: 0.019 Max Web CSI: 0.008 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 197 /- /- /140 /10 /78 E 33 /- /- /18 /- /- D 12 /- /- /28 /30 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x6 SP #1; block length = 1.500'

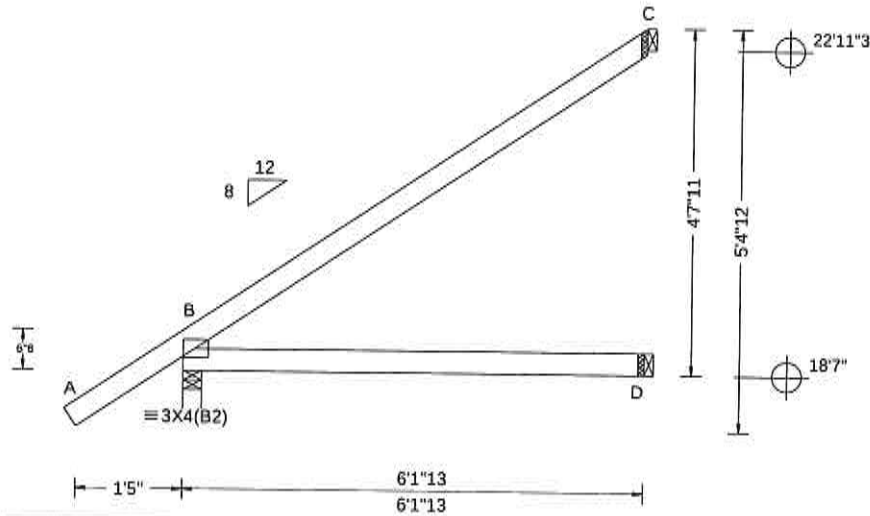
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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SEQN: 97548 FROM: RJL	JACK Qty: 3	Ply: 1 Qty: 3	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: JG 6'1"13 Jack	Cust: R 857 JRef: 1Xc39570001 T43 DrwNo: 010.22.1204.22800 SSB / FV 01/10/2022
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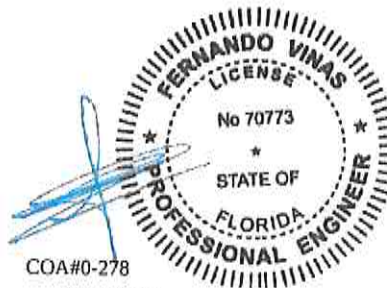
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.65 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.007 B - - HORZ(TL): 0.012 B - - Creep Factor: 2.0 Max TC CSI: 0.393 Max BC CSI: 0.304 Max Web CSI: 0.000 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 336 /- /- /206 /11 /147 D 116 /- /- /62 /- /- C 155 /- /- /89 /89 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



01/10/2022

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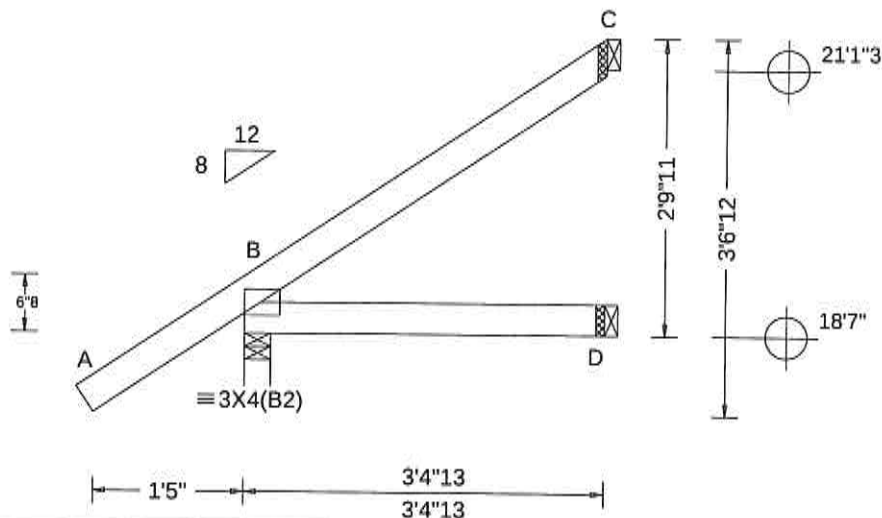
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 97550 FROM: RJL	JACK Qty: 3	Ply: 1	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: JH 3'4"13 Jack	Cust: R 857 JRef: 1Xc38570001 T46 DrwNo: 010.22.1204.33473 SSB / FV 01/10/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg.Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.73 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.001 C - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.155 Max BC CSI: 0.081 Max Web CSI: 0.000 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 240 /- /- /155 /18 /89 D 63 /- /- /35 /- /- C 75 /- /- /40 /45 /- Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#

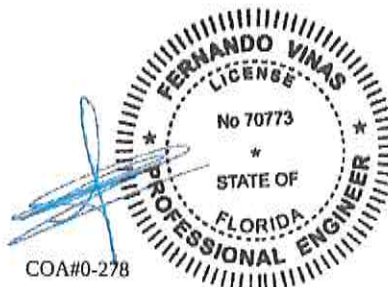
Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA#0-278

01/10/2022

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

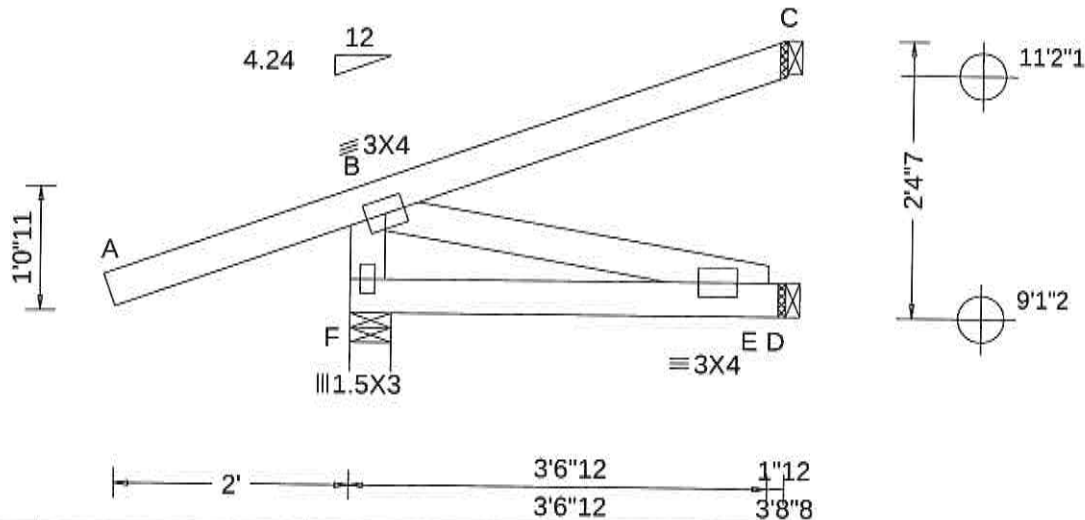
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCEA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCEA: sbceacomponents.com; ICC: iccsafe.org; AWC: awc.org

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Suite 305
Orlando FL, 32821



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 E 999 360 VERT(CL): 0.001 E 999 240 HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.064 Max BC CSI: 0.025 Max Web CSI: 0.007 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 94 /- /- /- /11 /- D 20 /- /- /10 /- /- C 48 /- /- /- /13 /- Wind reactions based on MWFRS F Brg Wid = 4.2 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing F is a rigid surface. Members not listed have forces less than 375#

Lumber

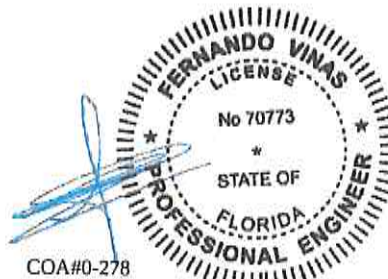
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Loading

Hipjack supports 2-7-8 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



01/10/2022

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

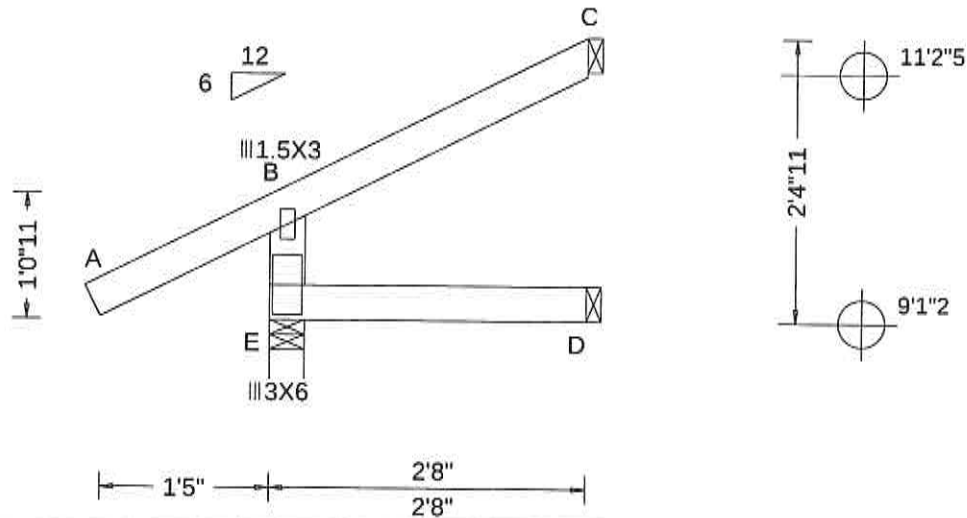
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to Job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

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Suite 305
Orlando FL, 32821

SEQN: 97510 FROM: RJL	EJAC Qty: 5	Ply: 1 Qty: 5	Job Number: B54908a RIOS/FIDEL RESIDENCE Truss Label: JK 2'8" End Jack	Cust: R 857 JRRef: 1Xc38570001 T34 DrwNo: 010.22.1204.15687 SSB / FV 01/10/2022
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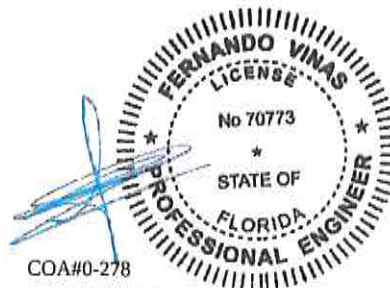
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 360 VERT(CL): 0.000 B 999 240 HORZ(LL): -0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.144 Max BC CSI: 0.055 Max Web CSI: 0.090 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 206 /- /- /144 /41 /- D 53 /- /- /27 /- /- C 51 /- /- /28 /11 /46 Wind reactions based on MWFRS E Brg Wid = 3.5 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing E is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA#0-278

01/10/2022

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc, shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

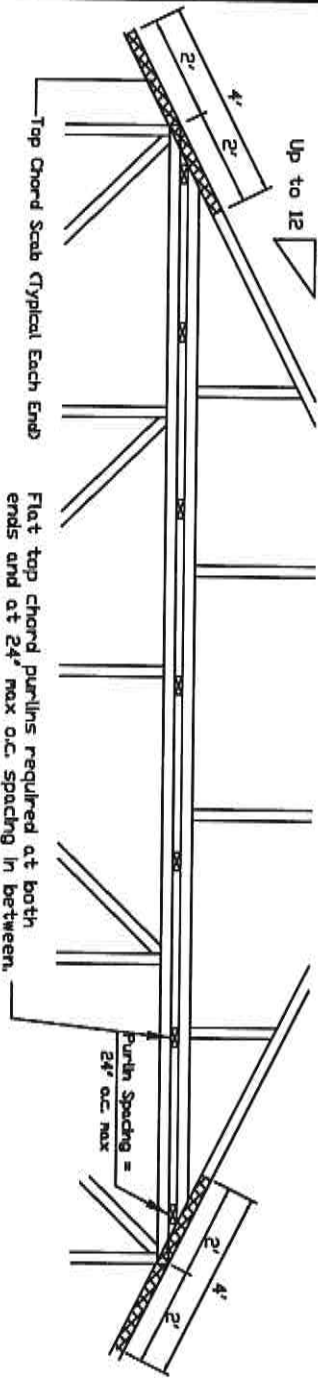
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Suite 305
Orlando FL, 32821

150 mph Wind, 30,000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp C, Wind D1 = 5.0 psf (q_h), Kz=1.0, Exp D1
140 mph Wind, 30,000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp D, Wind D1 = 5.0 psf (q_h), Kz=1.0

Note: Top chords of trusses supporting pidgeon/cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

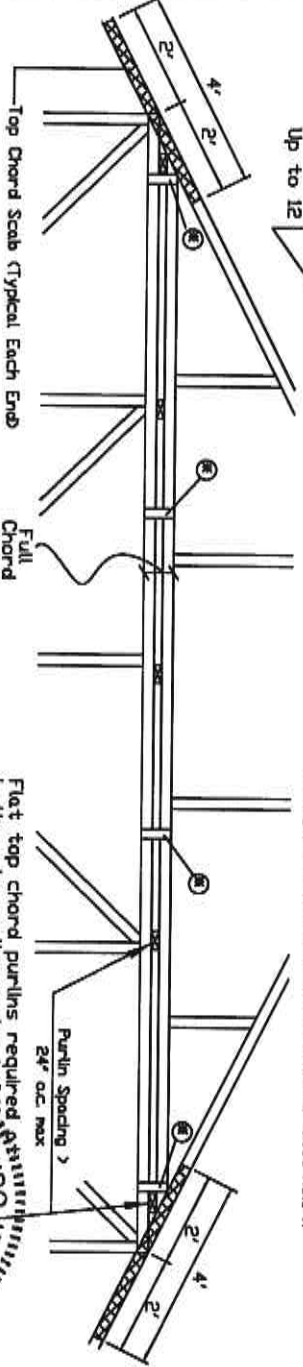
Up to 12



Pygmyback cap truss slant nailed to all top chord
purlin bracing with (2) 16d box nails (0.135"x3.5")
and secure top chord with 2x4 #3 grade scab
(1 side only at each end) attached with
2 rows of 10d box nails (0.128"x3") at 4" o.c.

The top chord #3 grade 2x4 scab was replaced with either of the following (d) 3x8 Tredox plate attached with (8) 0.120"x1.375" nails, (d) into cap TC & (4) into base truss TC or (d) 2B9B wave plywoodback plate plated to the plywoodback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note! Nailing thru holes of reave plate is acceptable.

Up to 12



Attach pulvin brocing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

■ In addition, provide connection with one of the following methods:

Use 3X6 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8" o.c. with (4) 0.125"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

8"x8"x7/16" (4N) APA rated sheathing gussets (each face). Attach @ 8" o.c. with (3) sd common (dallas #2) nails per gusset; (4) in cup bottom chord and (4) in base flange top chord. Gussets may be staggered 4" o.c. front to back faces.

2x4 S/F #2, full chord depth scabs (each face), Attach @ 8" o.c. with (6) 10d hex nails (3/16"x3") per scab, @ 3" in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4" o.c. front to back faces.

The 2BR3 wave playback plate to each face of the 8" o.c. attach teeth to playback at time of fabrication. Attach to supporting truss with (4) 0.125"x1.375" nolls per face per ply. Playback plates may be staggered 4' o.c. front to back faces.

Typical Each End	Full Chord Depth	Flat top chord purlins both ends, purlin splices
Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24" on center and sheathing at 16" on center.		

READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE DISTALLER.



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Suite 242
Earth City, MO 63045

[illegible]

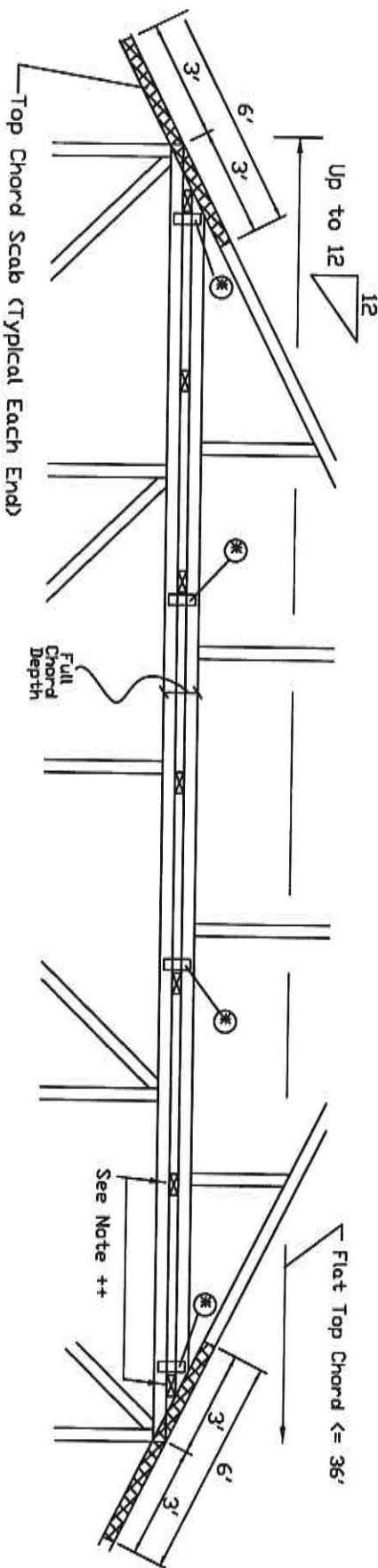
COA#0-278

SPACING

24.0"

Piggyback Detail - ASCE 7-16: 180 mph, 30' Mean Hgt, Partially Enclosed, Exp. C, Kzt=1.00

180 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Part: Enclosed Bldg, located anywhere in roof, Exp C, Wind II = 5.0 psf (min), Kzt=1.0.
 Dr 180 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Part: Enclosed Bldg, located anywhere in roof, Exp II, Wind II = 5.0 psf (min), Kzt=1.0.
 Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.
 Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.
 Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

++ Flat top chord purlins required at both ends and at a maximum of 24' intervals unless otherwise noted on base truss design drawing. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

In addition, provide connection with one of the following methods:

Trulox Use 3x8 Trulox plates for 2x4 chord member, and 3x10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.	28P3 Wave Piggyback Plate One 28P3 wave piggyback plate to each face @ 8' o.c. Attach each to piggyback at top of (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.
APA Rated Gusset 8"x8"x1/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.	2x4 Vertical Scabs 2x4 SPF #2, Full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of ICC Building Component Safety Information, by ITI and ICCA for safety instructions prior to performing these functions. Installers shall provide temporary bracing per ICCA. Trusses shall have a properly attached and properly attached structural sheathing and bottom chord shall have bracing installed per ICCA sections 301.17 or 301.18. Trusses shall be installed in accordance with the manufacturer's instructions and on the joint details, unless noted otherwise. Refer to drawings 100-2 for standard plate positions.

Alpine, a division of ITI Building Components Group, Inc. shall not be responsible for any deviation from these instructions. Any failure to build the truss in accordance with these instructions, or for handling, shipping, bracing, or any other deviation from these instructions, shall be the responsibility of the building designer per ICCA/ITI 1.3.3.3. For more information see the Job's general notes page and those with other drawings. www.alpine.com ITI www.itibuilding.com ICCA www.iccbuildingcomponents.com ITI www.alpine.com



514 Earth City Expressway
 Suite 242
 Earth City, MO 63045



REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB180160118

COA#0-278

SPACING 24.0"

Cracked or Broken Member Repair Detail

This drawing specifies repairs for a truss with broken chord or web member.

This design is valid only for single ply trusses with 2x4 or 2x6 broken members. No more than one break per chord panel and no more than two breaks per truss are allowed. Contact the truss manufacturer for any repairs that do not comply with this detail.

- (B) = Damaged area, 12" max length of damaged section
- (L) = Minimum nailing distance on each side of damaged area
- (S) = Two 2x4 or two 2x6 side members, same size, grade, and species as damaged member. Apply one scab per face. Minimum side member lengths = $(2 \times L) + (B)$

Scab member length (S) must be within the broken panel.

Nail into 2x4 members using two (2) rows at 4" o.c., rows staggered. Nail into 2x6 members using three (3) rows at 4" o.c., rows staggered.

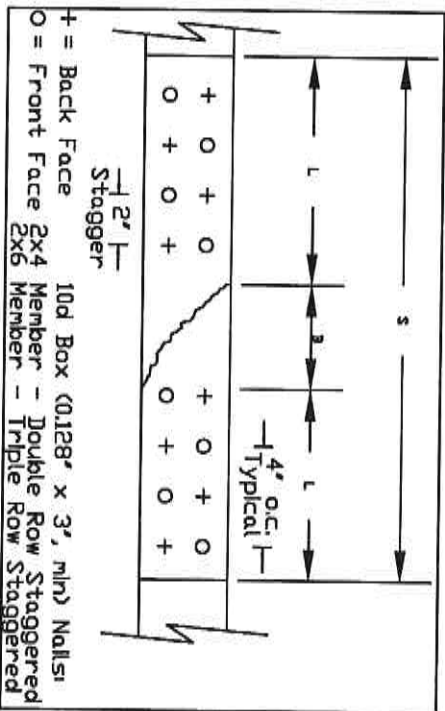
Nail using 10d box or gun nails (0.128"x3", min) into each side member.

The maximum permitted lumber grade for use with this detail is limited to Visual grade #1 and MSR grade 1650F.

This repair detail may be used for broken connector plate at mid-panel splices.

This repair detail may not be used for damaged chord or web sections occurring within the connector plate area.

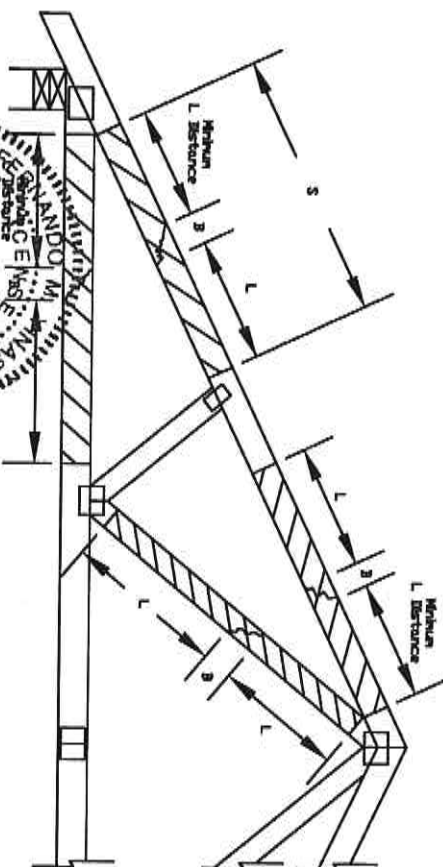
Broken chord may not support any tie-in loads.



Nail Spacing Detail

Member	Size	L	Maximum Member Axial Force			
			SPF-C	HF	DF-L	SY
Web Only	2x4	12'	620#	635#	730#	800#
Web Only	2x4	18'	975#	1055#	1295#	1415#
Web or Chord	2x4	24'	975#	1055#	1495#	1745#
Web or Chord	2x6	24'	1465#	1585#	2245#	2620#
Web or Chord	2x4	30'	1910#	1960#	2315#	2555#
Web or Chord	2x6	30'	2230#	2365#	3125#	3575#
Web or Chord	2x4	36'	2470#	2530#	2930#	3210#
Web or Chord	2x6	36'	3535#	3635#	4295#	4745#
Web or Chord	2x4	42'	2975#	3045#	3505#	3835#
Web or Chord	2x6	42'	4395#	4500#	5225#	5725#
Web or Chord	2x4	48'	3460#	3540#	4070#	4445#
Web or Chord	2x6	48'	5165#	5280#	6095#	6660#

Load Duration = 0%
Member forces may be increased for Duration of Load



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MEMBER REPAIR AND FOLLOWING ALL NOTES ON THIS DRAWING.

Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and follow the latest edition of SC21 Building Component Safety Information, by ITI and SC20 for safety practices prior to performing these functions. Installers shall provide temporary bracing per SC21. Trusses shall have a properly attached structural sheathing and bottom chord shall have bracing installed per SC21 section 32.37 or 32.38 as shown above and on the short details unless noted otherwise. Refer to drawings 1004-2 for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from these drawings. Any failure to build the truss in accordance with AWS/P1.1, or for handling, shipping, installing or bracing, is the responsibility of the truss manufacturer. The availability and use of the drawing for any structure is the responsibility of the Building Designer per AWS/P1.1 Section 2.1.1.

For more information see the job's general notes page and these web sites:
ALPINE: www.alpine.com ITI: www.iti.com SC21: www.sc21.com



COA#0-278	SPACING	24.0" MAX
REF MEMBER REPAIR		
DATE 10/01/14		
DRWG REPCHRD1014		

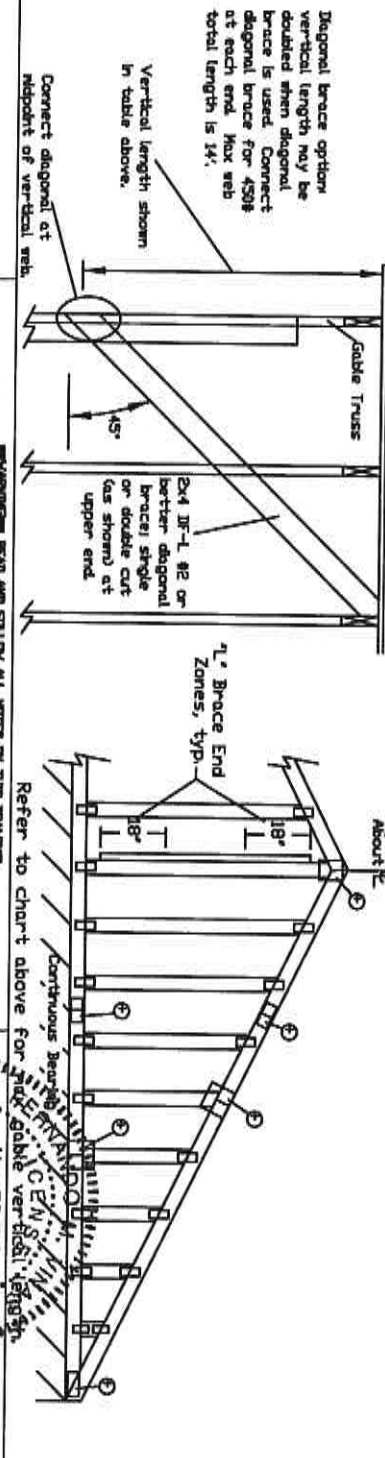
Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
 Or 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00
 Or 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
 Or 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Gable Vertical Length		Brace		No		CD 1x4 1" L ¹ Brace		CD 2x4 1" L ¹ Brace		CD 2x4 1" L ¹ Brace		CD 2x6 1" L ¹ Brace		CD 2x6 1" L ¹ Brace	
Spacing	Species	Grade	Braces	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
12" o.c.	SPF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	4' 1"	5' 8"	6' 0"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	SPF	#1 / #2	4' 3"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 3"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
24" o.c.	SPF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 11"	10' 4"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF	#1 / #2	5' 1"	8' 0"	8' 4"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	5' 1"	8' 0"	8' 4"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	SPF	#1 / #2	5' 1"	8' 0"	8' 4"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	5' 1"	8' 0"	8' 4"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
24" o.c.	SPF	#1 / #2	5' 1"	8' 0"	8' 4"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	5' 1"	8' 0"	8' 4"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades		Group A ₁		Group B ₁	
Spruce-Pine-Fir	#1 / #2 Standard	Hem-Fir		Hem-Fir	
	#3 Stud	#2 Stud		#1 & 2 Stud	
Douglas Fir-Larch	#3 Standard	Southern Pine		Southern Pine	
	Stud	#3 Stud		#3 Stud	

Gable Truss Detail Notes:
 Wind load deflection criterion is L/240.
 Provide uplift connections for SS pif over continuous bearing (5 psf TC Dead Load).
 Gable end supports load from 4' 0" overhangs with 2' 0" overhang, or 12" plywood overhang.



Gable Vertical Plate Sizes		No Sills	
Vertical Length	Less than 4' 0"	1X4 or 2X3	
	Greater than 4' 0"	3X4	

+ Refer to common truss design for purl, splice, and heel plates.
 Refer to the Building Designer for conditions not addressed by this detail.



Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of 2021 Gable Component Safety Information, by 171 and 3500 for safety instructions prior to performing these functions. Installers shall provide temporary bracing per 2021. Trusses shall have a properly attached and secured structural steeling and bottom chord. Trusses shall have bracing installed per 2021 sections 21.27 or 21.28 as applicable. Trusses shall be braced to existing 100A-2 for standard plate positions. Refer to drawings 100A-2 for standard plate positions. Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from these instructions. Alpine shall not be responsible for any deviation from these instructions. Alpine shall not be responsible for any deviation from these instructions. Alpine shall not be responsible for any deviation from these instructions.

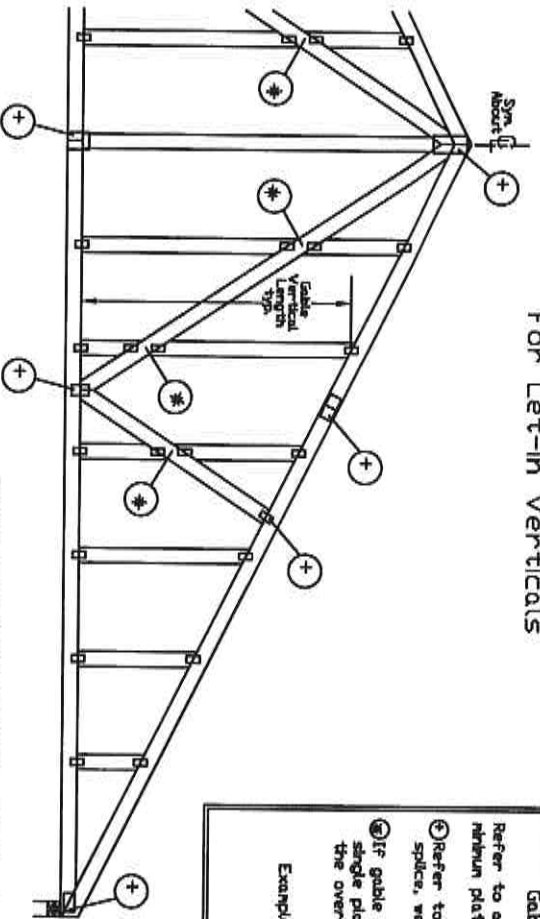


MAX. TOT. L.D. 60 PSF
 MAX. SPACING 24.0'

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 Suite 242
 South City, MO 63005

For more information see the job's general notes page and those with other ALPINE email@alpinesystems.com 771 554-1111 3300 554-1111 3300 554-1111 3300

Gable Detail For Let-In Verticals



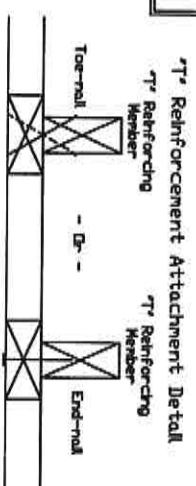
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

Refer to Engineered truss design for peak, splice, web, and heel plates.

If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



To convert from 1" to 1" reinforcing members, multiply 1" increase by length (based on appropriate Alpine gable detail).

Maximum allowable 1" reinforced gable vertical length is 14' from top to bottom chord.

1" reinforcing member material must match size, grade, and grade of the 1" reinforcing member.

Web Length Increase w/ 1" Brace

1" Reinf. Mem. Size	1" Increase
2x4	30 %
2x6	20 %

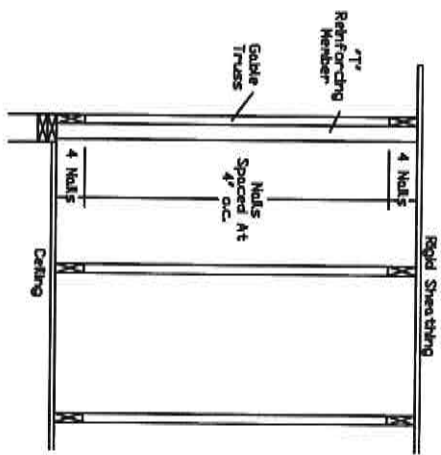
Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24' o.c. Sp #3

1" Reinforcing Member Size = 2x4

1" Brace Increase (from Above) = 30% = 1.30

1" Brace Length = 8' 7"

Maximum 1" Reinforced Gable Vertical Length = 1.30 x 8' 7" = 11' 2"



Provide connections for uplift specified on the engineered truss design.
Attach each 1" reinforcing member with:
End Driven Nails
10d Common (0.148"x3.75") Nails at 4' o.c. plus
(4) nails in the top and bottom chords.
Toenailed Nails
10d Common (0.148"x3.75") Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.
This detail to be used with the appropriate Alpine gable detail for ASCE

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

MANUFACTURE, INSTALL AND FINISH ALL WORK IN THIS DRAWING

INSTALLER SHALL BE RESPONSIBLE FOR THE QUALITY OF THE WORKMANSHIP AND THE COMPLETION OF THE WORK. THE INSTALLER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE EXISTING STRUCTURE AND THE ADJACENT AREAS. THE INSTALLER SHALL BE RESPONSIBLE FOR THE REMOVAL OF ALL DEBRIS AND WASTE MATERIAL FROM THE WORK AREA. THE INSTALLER SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF THE WORK AREA AND THE ADJACENT AREAS. THE INSTALLER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE EXISTING STRUCTURE AND THE ADJACENT AREAS. THE INSTALLER SHALL BE RESPONSIBLE FOR THE REMOVAL OF ALL DEBRIS AND WASTE MATERIAL FROM THE WORK AREA. THE INSTALLER SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF THE WORK AREA AND THE ADJACENT AREAS.



514 Earth City Expressway
Suite 202
Earth City, MO 63045



MAX. TOT. LD. 60 PSF	REF LET-IN VERT
DUR. FAC. ANY	DATE 01/02/2018
MAX. SPACING 24.0'	DRWG GBLETTN011B

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

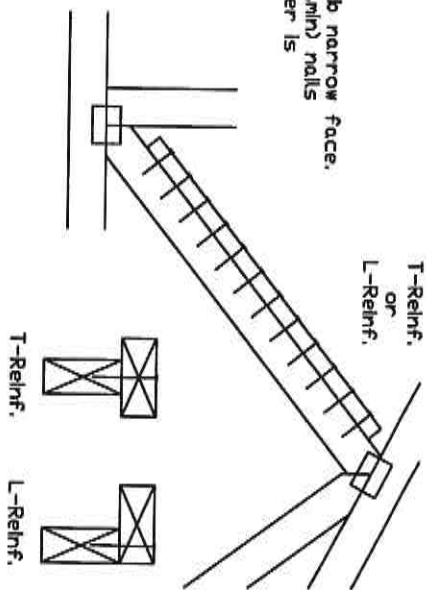
Use scabs instead of L- or T-reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4GD
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6GD

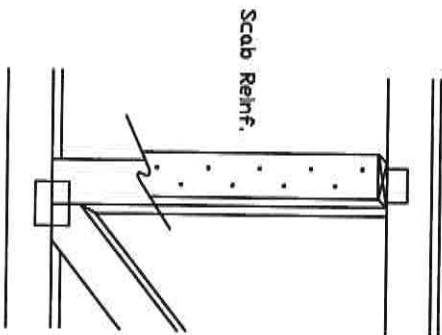
T-reinforcement, L-reinforcement, or slab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

- ③ Center scab on wide face of web. Apply (1) scab to each face of web.

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0" min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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MANUALLY READ AND FOLLOW ALL NOTES ON THIS DRAWING. IMPORTANT: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE

¹Insurance related activities are in fact-finding, health, shipping, handling and brokering. Refer to and follow the latest edition of ECRI Quality Component Safety Information, by IPI and SICO for safety practices prior to performing these functions. Installers shall provide temporary brokering per ECRI release notes otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rafter cord. Locations shown for permanent lateral restraint of ribs shall have bracing installed per ECRI sections 8.0, 9.7 or 9.8, as applicable. Apply plates to each face of all steel joists and position as shown above and on the joint details, unless noted otherwise. Refer to drawings ISM-2 for standard plate portions.

Notwithstanding to whom the design is submitted, the design engineer shall be responsible for any deviation from the design. The design engineer shall be responsible for the design being submitted to the building department for any structure to be constructed in accordance with the design. The design engineer shall be responsible for the design being submitted to the building department for any structure to be constructed in accordance with the design. The design engineer shall be responsible for the design being submitted to the building department for any structure to be constructed in accordance with the design.

For more information see this job's general notes page and these web sites:
ALPINE www.alpinethr.com TPI www.tpiusa.org SICA www.sicad.com/products.htm
ALPINE www.alpinethr.com TPI www.tpiusa.org SICA www.sicad.com/products.htm



STC LL	PSF	REF	CLR Subst.
TC DL	PSF	DATE	01/02/19
BC DL	PSF	DRWG	BRCLBSUB0119
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCE THIS DETAIL.

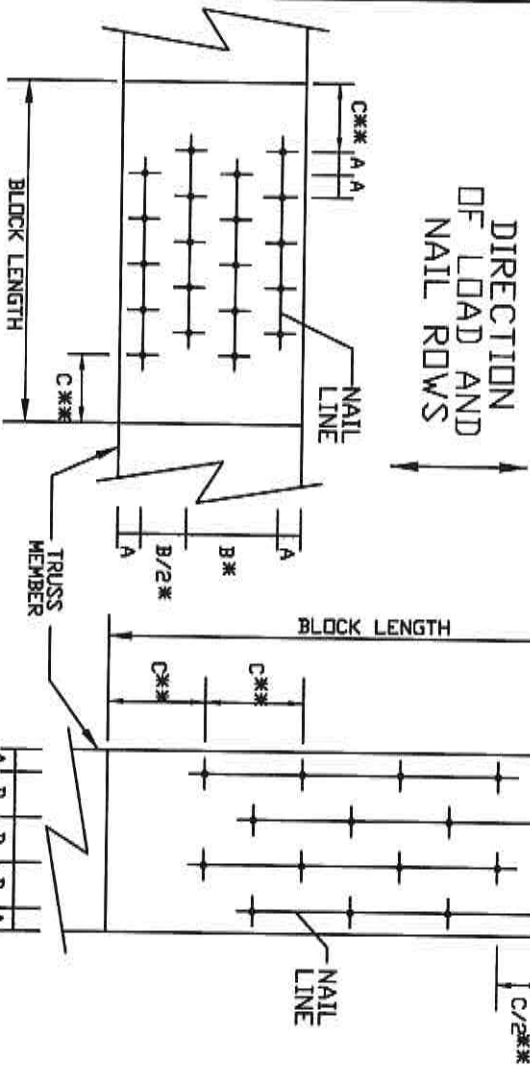
- A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
- B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
- C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

- A - EDGE DISTANCE (6 NAIL DIAMETERS)
- C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
- D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

- * SPACING MAY BE REDUCED BY 50%
- ** SPACING MAY BE REDUCED BY 33%



MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BDX (0.113"x 2.5", MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BDX (0.128"x 3", MIN)	7/8"	1 5/8"	2"	1"
12d BDX (0.128"x 3.25", MIN)	7/8"	1 5/8"	2"	1"
16d BDX (0.135"x 3.5", MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BDX (0.148"x 4", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"x 2.5", MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"x 3", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"x 3.25", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"x 3.5", MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120"x 2.5", MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"x 2.5", MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"x 3", MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"x 3", MIN)	7/8"	1 5/8"	2"	1"

LOAD APPLIED PERPENDICULAR TO GRAIN

LOAD APPLIED PARALLEL TO GRAIN



514 East Cuy Expressway
Suite 242
East Cuy, MO 63045

REF NAIL SPACE

DATE 10/01/14

DRWG CMAILSP1014

COA#0-278

STATE OF FLORIDA
PROFESSIONAL ENGINEER
FERNANDO M. VINAS
LICENSE NO. 70773
10/10/2022

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr. 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Dr. 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Dr. 100 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Gable Vertical Length		Brace		No. Braces											
Spacing	Species	Grade	No. Braces	(1) 1x4 1" L ¹ Brace	(1) 2x4 1" L ² Brace	(2) 2x4 1" L ² Brace	(2) 2x4 1" L ² Brace	(2) 2x6 1" L ² Brace	(2) 2x6 1" L ² Brace	(2) 2x6 1" L ² Brace	(2) 2x6 1" L ² Brace	(2) 2x6 1" L ² Brace	(2) 2x6 1" L ² Brace	(2) 2x6 1" L ² Brace	(2) 2x6 1" L ² Brace
12" o.c.	SPF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	SPF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
24" o.c.	SPF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Standard	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades

Group A ¹		Group B ¹	
Service-Pre-Fir	Heir-Fir	Service-Pre-Fir	Heir-Fir
#1 / #2	#2	#1 / #2	#2
Standard	Stud	Standard	Stud
Douglas Fir-Larch		Southern Pine	
#3	#3	#3	#3
Standard	Standard	Standard	Standard

Group B ¹		Group C ¹	
Heir-Fir	Heir-Fir	Heir-Fir	Heir-Fir
#1 & 3/4"	#1	#1 & 3/4"	#1
Douglas Fir-Larch		Southern Pine	
#1	#1	#1	#1
Standard	Standard	Standard	Standard

1x4 Braces shall be SDB (Stress-Rated Board) or 1x4 Ss. Pine use only Industrial SS or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes

Wind load deflection criterion is L/240.
Provide uplift connections for 100 plf over continuous bearing 45 psf TC Dead Load.
Gable end supports load from 4' 0" outcroppers with 2' 0" overhang or 12" plywood overhang.

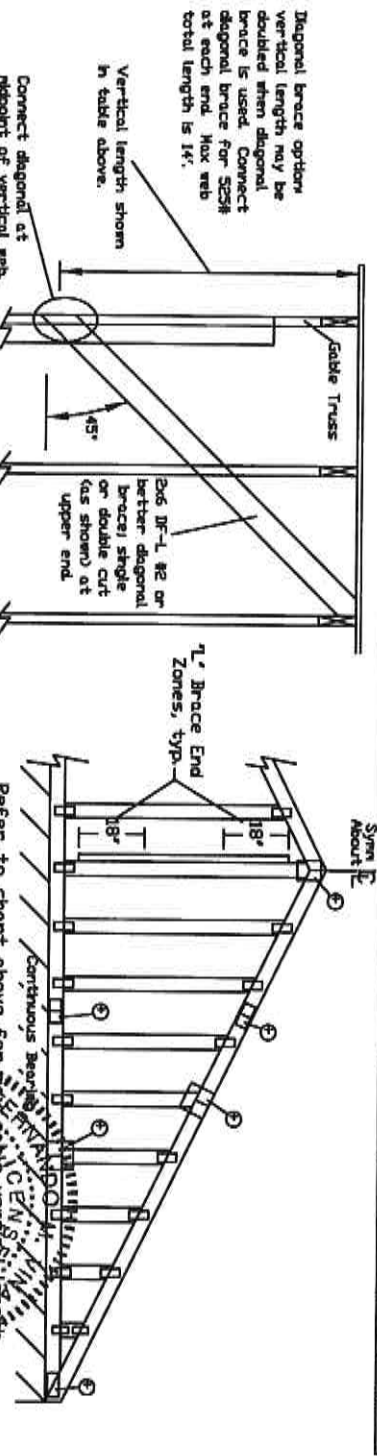
Attach 1" L¹ braces with 10d (125x3.0" min) nails.

* For CD 1" L¹ brace: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.
* For CD 1" L¹ brace: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.
1" L¹ bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.



Diagonal brace options: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for SDB# at each end. Max web total length is 14'.

Vertical length shown in table above.

Correct diagonal at midpoint of vertical web.

1" L¹ Brace End Zones, typ.

Continuous bearing 45 psf TC Dead Load.

Refer to chart above for gable vertical length.



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Suite 242
Earth City, MO 63045

IMPORTANT READ AND FOLLOW ALL NOTES ON THIS DRAWING

Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and follow the latest edition of ASCE 7-16 (Minimum Design Loads and Associated Criteria for Buildings and Other Structures) for all applicable provisions. Trusses shall be braced in accordance with the provisions of ASCE 7-16, Section 6.2.2. Trusses shall be braced in accordance with the provisions of ASCE 7-16, Section 6.2.2. Trusses shall be braced in accordance with the provisions of ASCE 7-16, Section 6.2.2.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the design shown on this drawing. The liability and use of this drawing for any structure is the responsibility of the Building Designer per ASCE 7-16, Section 6.2.2. For more information see the job's general notes page and these web sites: www.alpinecomponents.com www.asce.org



COA#0-278

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0'

REF ASCE 7-16-GAB14030
DATE 01/26/2018
DWG A14030ENC160118