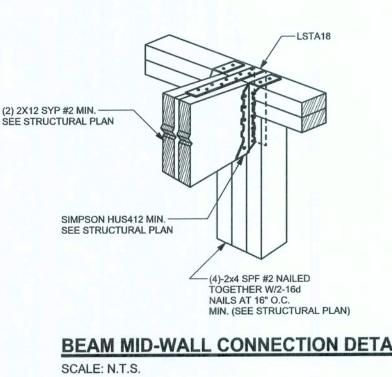


EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

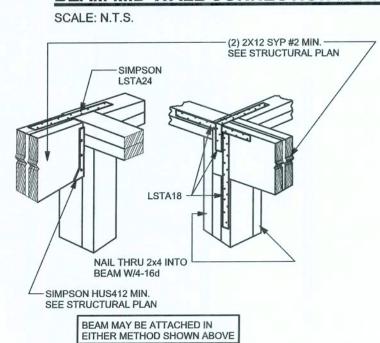
SCALE: 3/4" = 1'-0"

| (1) 2x4 @ 16" OC | TO 11'-9" STUD HEIGHT |
|------------------|------------------------|
| (1) 2x4 @ 12" OC | TO 13'-0" STUD HEIGHT |
| (1) 2x6 @ 16" OC | TO 18'-10' STUD HEIGHT |
| (1) 2x6 @ 12" OC | TO 20.0' STUD HEIGHT |

THIS STUD HEIGHT TABLE IS PER WECM 2001, TABLE 3,20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

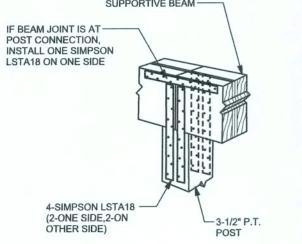






BEAM CORNER CONNECTION. DETAIL

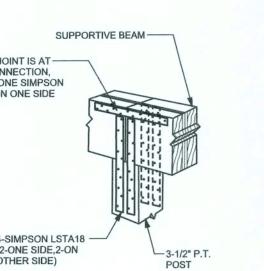


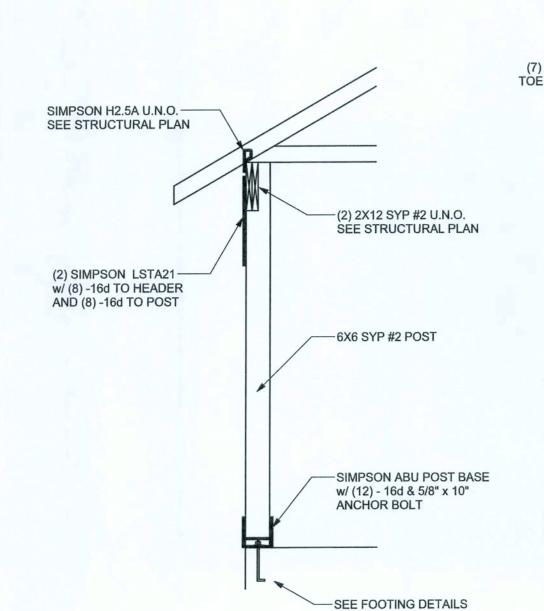


SUPPORTIVE CENTER POST TO EAM DETAIL



- NON-SUPPORTIV





TYPICAL PORCH POST DETAIL

IF TRUSS TO WALL STRAPS ARE NAILILED FOR LESS THAN 3750 Ib UPLIFT USE TO THE HEADER THE SPH4/6 @ 48" OO.C. 3 X 3 X 1/8" WASHER ARE NOT REQUIRED -NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 3" O.C. FOR UPLIFT (7) .131 x 3 1/4" GUN NAILS--- SP4/6 @ 48" O.C. (U.N.O.) /---(7) .131 x 3 1/4" GUN NAILS TOE NAILED THRU HEADER TOE NAILED THRU HEADER INTO KING STUD INTO KING STUD CRIPPLES IF REQUIRED (5) .131 x 3 1/4" GUN NAILS TOE NAILED THRU SILL INTO JACK STUD U.N.O. TYPICAL STRAPPING (U.N.O.) (SEE STRUCTURAL PLAN) (1) 2X6 SPF #2 SILL UP TO 7'-6" U.N.O. (2) 2X4 SPF #2 SILL UP TO 7'-8" U.N.O. (1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

FOR LESS THAN 1500 Ib UPLIFT USE

2 X 2 X 1/8" WASHER

TYPICAL 1 STORY HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"

(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

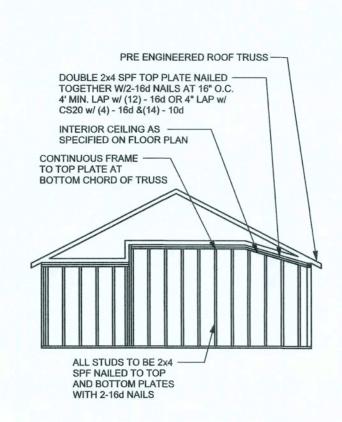
ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

| JPLIFT LBS. SYP | UPLIFT LBS. SPF | TRUSS CONNECTOR* | TO PLATES | TO RAFTER/TRUSS | TO STUDS |
|------------------|------------------|------------------------|------------------|-----------------|--------------------------------------|
| < 420 | < 245 | H5A | 3-8d | 3-8d | |
| < 455 | < 265 | H5 | 4-8d | 4-8d | |
| < 360 | < 235 | H4 | 4-8d | 4-8d | |
| < 455 | < 320 | H3 | 4-8d | 4-8d | |
| < 415 | < 365 | H2.5 | 5-8d | 5-8d | |
| < 600 | < 535 | H2.5A | 5-8d | 5-8d | |
| < 950 | < 820 | H6 | 8-8d | 8-8d | |
| < 745 | < 565 | H8 | 5-10d, 1 1/2" | 5-10d, 1 1/2" | |
| < 1465 | < 1050 | H14-1 | 13-8d | 12-8d, 1 1/2" | |
| < 1465 | < 1050 | H14-2 | 15-8d | 12-8d, 1 1/2" | |
| < 990 | < 850 | H10-1 | 8-8d, 1 1/2" | 8-8d, 1 1/2" | |
| < 760 | < 655 | H10-2 | 6-10d | 6-10d | |
| < 1470 | < 1265 | H16-1 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1470 | < 1265 | H16-2 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1000 | < 860 | MTS24C | 7-10d 1 1/2" | 7-10d 1 1/2" | |
| < 1450 | < 1245 | HTS24 | 12-10d 1 1/2" | 12-10d 1 1/2" | |
| < 2900 | < 2490 | 2 - HTS24 | | | |
| < 2050 | < 1785 | LGT2 | 14 -16d | 14 -16d | |
| | | HEAVY GIRDER TIEDOWNS* | | | TO FOUNDATION |
| < 3965 | < 3330 | MGT | | 22 -10d | 1-5/8" THREADED ROI 12" EMBEDMENT |
| < 10980 | < 6485 | HGT-2 | | 16 -10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 10530 | < 9035 | HGT-3 | | 16 -10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 9250 | < 9250 | HGT-4 | | 16 -10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| | | STUD STRAP CONNECTOR* | | | TO STUDS |
| < 435 | < 435 | SSP DOUBLE TOP PLATE | 3 -10d | | 4 -10d |
| < 455 | < 420 | SSP SINGLE SILL PLATE | 1 -10d | | 4 -10d |
| < 825 | < 825 | DSP DOUBLE TOP PLATE | 6 -10d | | 8 -10d |
| < 825 | < 600 | DSP SINGLE SILL PLATE | 2 -10d | | 8 -10d |
| < 885 | < 760 | SP4 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH4 | | | 10-10d, 1 1/2" |
| < 885 | < 760 | SP6 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH6 | | | 10-10d, 1 1/2" |
| < 1235 | < 1165 | LSTA18 | 14-10d | | |
| < 1235 | < 1235 | LSTA21 | 16-10d | | B4-11 |
| < 1030 | < 1030 | CS20 | 18-8d | | |
| < 1705 | < 1705 | CS16 | 28-8d | | |
| | | STUD ANCHORS* | TO STUDS | | TO FOUNDATION |
| < 1350 | < 1305 | LTT19 | 8-16d | | 1/2" AB |
| < 2310 | < 2310 | LTTI31 | 18-10d, 1 1/2" | | 1/2" AB |
| < 2775 | < 2570 | HD2A | 2-5/8" BOLTS | | 5/8" AB |
| < 4175 | < 3695 | HTT16 | 18 - 16d | | 5/8" AB |
| < 1400 | < 1400 | PAHD42 | 16-16d | | 44013-11-11 |
| | | 7.020000 | | | |
| < 3335 | < 3335 | HPAHD22 | 16-16d | | |
| < 3335 < 2200 | < 3335 < 2200 | HPAHD22 ABU44 | 16-16d 12-16d | | 1/2" AB |

GRADE & SPECIES TABLE

| | | Fb (psi) | E (10 ⁶ psi) |
|------|--------------|----------|-------------------------|
| 2x8 | SYP #2 | 1200 | 1.6 |
| 2x10 | SYP #2 | 1050 | 1.6 |
| 2x12 | SYP #2 | 975 | 1.6 |
| GLB | 24F-V3 SP | 2400 | 1.8 |
| LSL | TIMBERSTRAND | 1700 | 1.7 |
| LVL | MICROLAM | 2900 | 2.0 |
| PSL | PARALAM | 2900 | 2.0 |



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS, BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 48 * DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

| OI LOII IOAL | Y NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. |
|-------------------------------|---|
| CONFIRM SITE BACKFILL HEIG | CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND HT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. |
| PROVIDE MATE REQUIREMENT | RIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 S FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. |
| BELIEVE THE P | ITINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU AN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL DENGINEER IMMEDIATELY. |
| DESIGN, PLACE | USS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS MENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, SS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL |

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION 1609 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS DE REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;

MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10%

DESIGN DATA

2-5/8" AB

| BUILDIN | IG IS NOT IN THE WIND-BORNE DEBRIS | REGION | 1 | | | |
|------------|--|--------------|------------------------------------|--------------|----------------|------|
| 1.) BA | SIC WIND SPEED = 110 MPH | | | | | |
| 2.) WII | ND EXPOSURE = B | | | | | |
| 3.) WII | ND IMPORTANCE FACTOR = 1.0 | | | | | |
| 4.) BU | LDING CATEGORY = II | | | | | |
| 5.) RO | OF ANGLE = 10-45 DEGREES | | | | | |
| | AN ROOF HEIGHT = <30 FT | | | | | |
| 7.) INT | ERNAL PRESSURE COEFFICIENT = N/A | (ENCLC | SED BUILD | ING) | | |
| 8.) CO | MPONENTS AND CLADDING DESIGN WI | IND PRE | SSURES [T | ABLE | R301.2 | (2)] |
| K | 2 2 | Zone | Effective Wi | | ea (ft2) | |
| 7 | 2 2 2 1 | 1 | 19.9 -21.8 | | -18.1 | |
| <i>y</i> \ | 4 3 4 | 3 | 19.9 -42.1 19.9 -42.1 | | -29.1 -29.1 | |
| 2 | 5,5 | 4 | 21.8 -23.6 | | -20.4 | |
| * | The state of the s | 5 | 21.8 -29.1 | 18.5 | -22.6 | |
| 3 | 2 2 3 | Wor (Zone | & Windows st Case 5, 10 ft2) | 21.8 | -29.1 | |
| 2 | 4 3 4 5 5 5 2 2 2 | | rage Door arage Door | 19.5 18.5 | -21.3 -20.4 | |
| DESIGN | LOADS | | | | | |
| FLOOR | 40 PSF (ALL OTHER DWELLING ROOM | 1S) | | | | |
| | 30 PSF (SLEEPING ROOMS) | | | | | |
| | 20 PSF (ATTICS WITH STORAGE) | | | | | |
| | 10 PSF (ATTICS WITHOUT STORAGE, < | <3:12) | | | | |
| | 40 PSF (DECKS) | | | | | |
| | 60 PSF (EXTERIOR BALCONIES) | | | | | |
| ROOF | 20 PSF (FLAT OR <4:12) | | | | | |
| | 16 PSF (4:12 TO <12:12) | | | | | |
| | 12 PSF (12:12 AND GREATER) | | | | | |
| | | | | | | |

PE No;3915, POB 868, Lake City, FL

REVISIONS

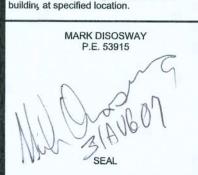
SOFTPLAN

32056,386-754-5419 dimensons. Refer all questions to Mark Csosway, P.E. for resolution. Do not roceed without clarification.

COPYIIGHTS AND PROPERTY RIGHTS: Mark Dsosway, P.E. hereby expressly resen its comnon law copyrights and property right i these istruments of service. This document is not to be reproduced, altered or copied in any form ormanner without first the express written permision and consent of Mark Disosway.

CERTITICATION: I hereby certify that I have examined this plan, and that the applicable portion of the plan, relating to wind engine complywith section R301.2.1, florida building code reidential 2004, to the best of my

LIMITATION: This design is valid for one



RICHARD KEEN

Dunken Residence

ADDRESS: Gum Swamp Road Columbia County, Florida

Vark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Prone: (386) 754 - 5419 Fax: (386) 269 - 4871

August 30, 2007 STRUCTURAL BY BerSparks Ben Sparks

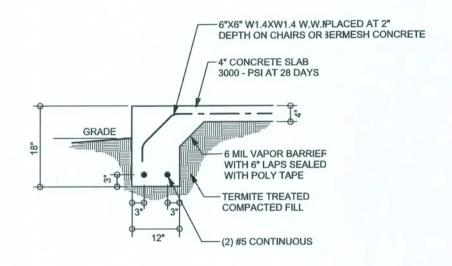
30 Aug / 07 JOB NUMBER: 708299

> **S-1** OF 3 SHEETS

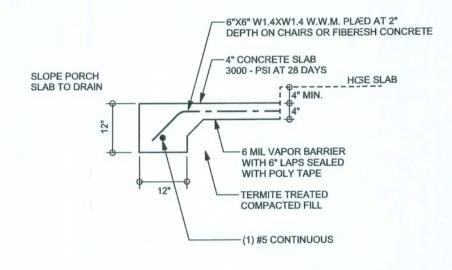
DRAWING NUMBER

REVISIONS

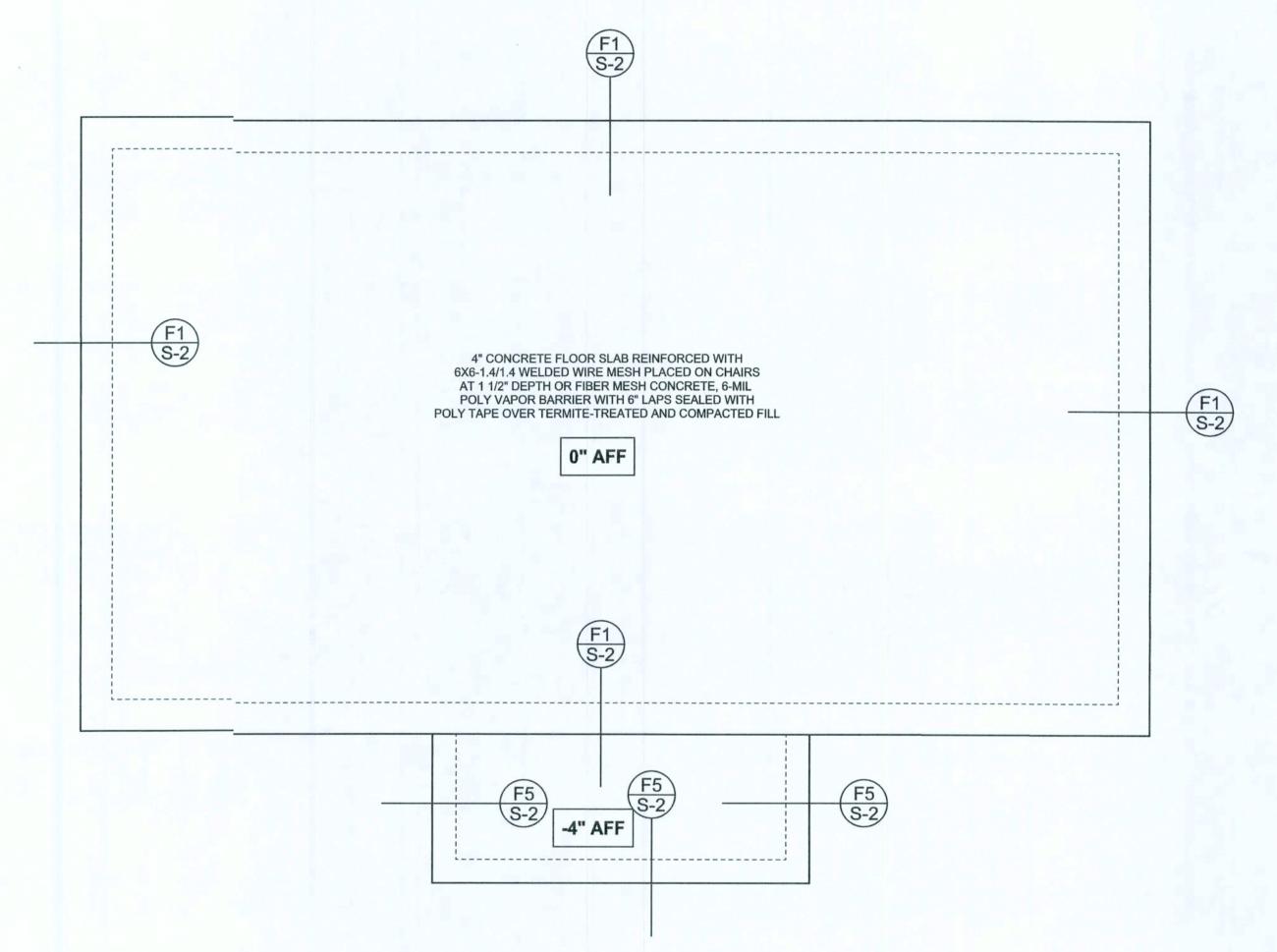
SOFTPIAN ARCHITECTURAL DESIGN SOFTWAR



F1 MONOLITHIC FOOTING
S-2 SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"



FOUNDATION PLAAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAAL SHEETS
ARE NOT EXACT. REFER TO 5 ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

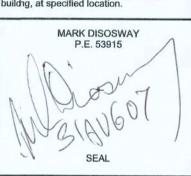
WIN)LOAD ENGINEER: Mark Disosway, PE to.53915, POB 868, Lake City, FL 3205, 386-754-5419

DIMINSIONS:
Statd dimensions supercede scaled dimesions. Refer all questions to Marl Disosway, P.E. for resolution. Do not proceed without clarification.

COP'RIGHTS AND PROPERTY RIGHTS:
MarlDisosway, P.E. hereby expressly reserves
its cmmon law copyrights and property right in
thes instruments of service. This document is
not t be reproduced, altered or copied in any
formor manner without first the express written
permssion and consent of Mark Disosway.

CERTFICATION: I hereby certify that I have exarined this plan, and that the applicable portins of the plan, relating to wind engineering com/y with section R301.2.1, florida building coderesidential 2004, to the best of my

LIMI'ATION: This design is valid for one building, at specified location.



RICHARD KEEN

Dunken Residence

ADDRESS: Gum Swamp Road Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Fhone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: August 30, 2007

DRAWN BY: STRUCTURAL BY: Ben Sparks Ben Sparks

FNALS DATE: 31 / Aug / 07

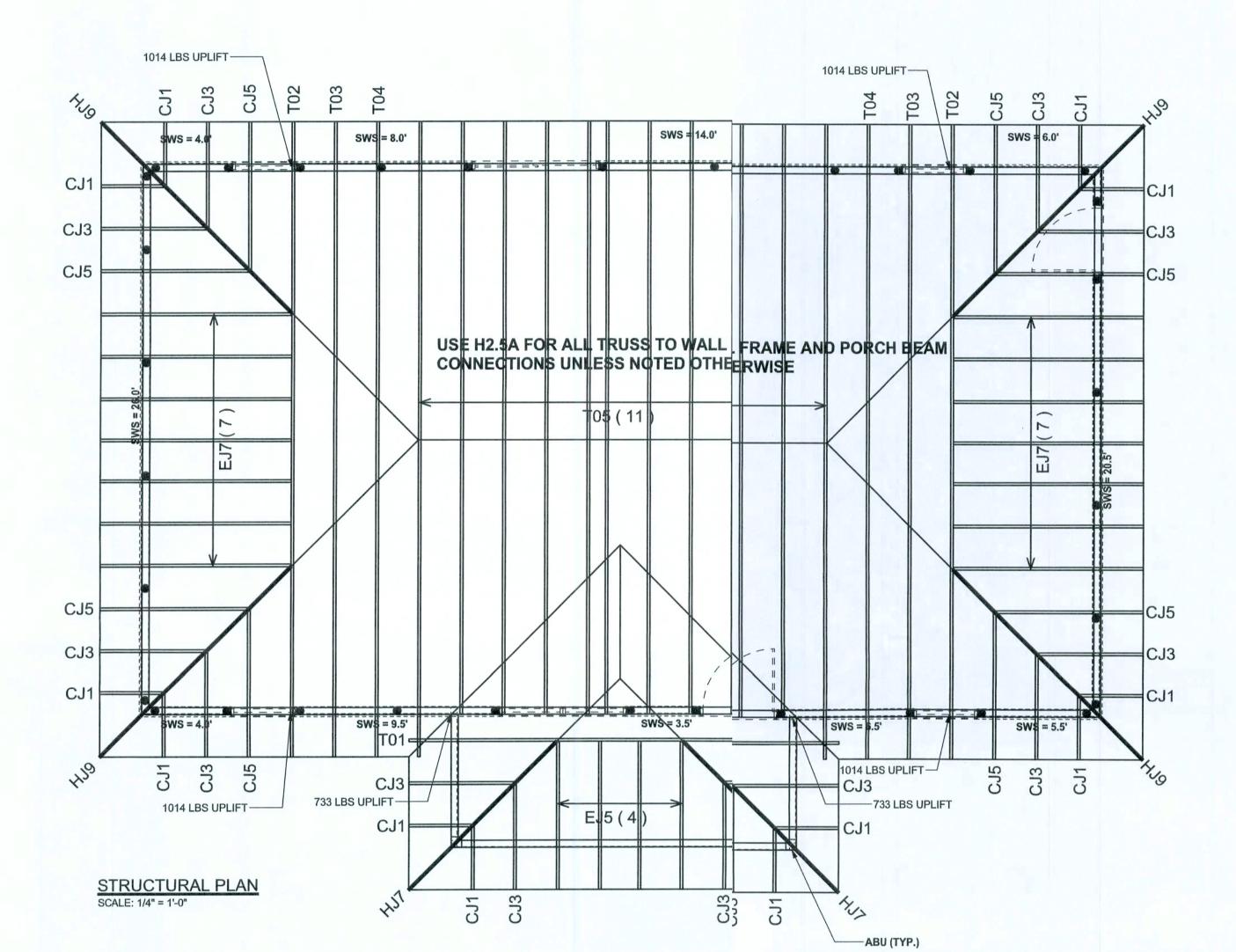
> JOB NUMBER: 708299

> > S-2 OF 3 SHEETS

DRAWING NUMBER

REVISIONS

SOFTPIXN



STRUCTURAL PLAN NOTES

EACH SIDE (U.N.O.)

TRUSS PACKAGE

ALL LOAD BEARING FRAME WALL & PORCH HEADERS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3

LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED

SN-1 ALL LOAD BEARING FRANCE WALL & FORGITIES STALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

DIMENSIONS ON STRUCTURAL SHEETS

FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD

SN-3 ARE NOT EXACT. REFER TO ARCHITECTURAL

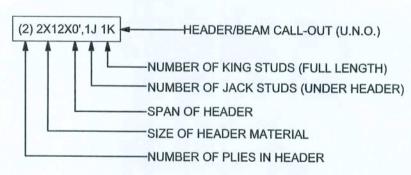


| SWS = 0.0' | 1ST FLOOR EXTERIOR WALL |
|------------|--|
| SWS = 0.0' | 2ND FLOOR EXTERIOR |
| IBW | 1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1 |
| IBW | 2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1 |

THREADED ROD LEGEND - INDICATES LOCATION OF:

1ST FLOOR 1/2" A307 ALL THREADED ROD - INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

| | REQUIRED | ACTUAL |
|--------------|----------|--------|
| TRANSVERSE | 35.0' | 46.5' |
| LONGITUDINAL | 15.0' | 61.0' |

MSTA30, 10-10d (1700lb) (5) NAILS EACH SIDE OF STUD (OR STRAP STUD TO HEADER 20-10d) LTT20B, 10-16d (1750lb)----1/2" ANCHOR w/ 7" EMBEDMENT, SIMPSON AT—— (MAY BE RECESSED BELOW FINISHED FLOOR)

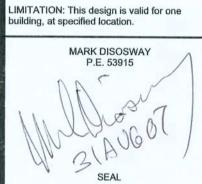
ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED IN WALL SCALE: 1/2" = 1'-0"

> WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:
Mark Disosway, P.E. hereby expressly reserves
its common law copyrights and property right in
these instruments of service. This document is
not to be reproduced, altered or copied in any
form or manner without first the express written
permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my



RICHARD KEEN

Dunken Residence

ADDRESS: Gum Swamp Road Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: August 30, 2007 DRAWN BY: STRUCTURAL BY:

Ben Sparks Ben Sparks

FINALS DATE:

30 / Aug / 07 JOB NUMBER: 708299

> **S-3** OF 3 SHEETS

DRAWING NUMBER

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE (JOB #L161233)