DATE 02/15/2008

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000026759

APPLICANT LAWANDA RENTZ PHONE 961-8777

ADDRESS 208 SE MARY ETHEL TERR LAKE CITY FL 32025

OWNER LAWANDA RENTZ PHONE 961-8777

ADDRESS 7148 SW CR 242 LAKE CITY FL 32025

CONTRACTOR OWNER BUILDER PHONE

LOCATION OF PROPERTY 90 W, L 247, R 242 ABOUT 2.5 MILES ON THE LEFT, SEE MAILBOX

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 137750.00

HEATED FLOOR AREA 2755.00 TOTAL AREA 3744.00 HEIGHT 21.00 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 9/12 FLOOR SLAB

LAND USE & ZONING AG-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 25-4S-15-00388-001 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 3.00

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
			Lawanda V. Rentz

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
EXISTING	08-0130	BK	JH	N

COMMENTS: FAMILY LOT AFFIDAVIT 14.9, FLOOR ONE FOOT ABOVE THE ROAD

Check # or Cash 1895

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power ____ date/app. by | Foundation ____ date/app. by | Monolithic ____ date/app. by

Under slab rough-in plumbing ____ date/app. by | Slab ____ date/app. by | Sheathing/Nailing ____ date/app. by

Framing ____ date/app. by | Rough-in plumbing above slab and below wood floor ____ date/app. by

Electrical rough-in ____ date/app. by | Heat & Air Duct ____ date/app. by | Peri. beam (Lintel) ____ date/app. by

Permanent power ____ date/app. by | C.O. Final ____ date/app. by | Culvert ____ date/app. by

M/H tie downs, blocking, electricity and plumbing ____ date/app. by | Pool ____ date/app. by

Reconnection ____ date/app. by | Pump pole ____ date/app. by | Utility Pole ____ date/app. by

M/H Pole ____ date/app. by | Travel Trailer ____ date/app. by | Re-roof ____ date/app. by

BUILDING PERMIT FEE $ 690.00 CERTIFICATION FEE $ 18.72 SURCHARGE FEE $ 18.72

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 802.44

INSPECTORS OFFICE Laurie Dodson CLERKS OFFICE MSS

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Left message 2-13-08 LH

PARENT PARCEL 25-45-15-00388-000

Columbia County Building Permit Application

For Office Use Only Application # 0801-135 Date Received 1-28-08 By LH Permit # 26759

Application Approved by - Zoning Official BLK Date 02.02.08 Plans Examiner OK JTH Date 2-5-08

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments (Special Family Lot Permit)

NOC EH Deed or PA Site Plan State Road Info Parent Parcel # Development Permi

Name Authorized Person Signing Permit LAWANDA RENTZ Phone 386.961.8777 Fax

Address 208 SE MARY ETHEL TERRACE, L. C, FL 32025

Owners Name LAWANDA Y. RENTZ Phone 386 961-8777

911 Address 7148 SW COUNTY ROAD 242, L.C, FL 32024

Contractors Name LAWANDA RENTZ Phone 386.961.8777

Address 208 SE Mary Ethel Terrace, L. C, FL 32025

Fee Simple Owner Name & Address

Bonding Co. Name & Address

Architect/Engineer Name & Address JIM DELBENE - DRAFTSMAN, MARK DISOSWAY, PE

Mortgage Lenders Name & Address CASH

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progressive Energ

Property ID Number 25-45-15-00388-001 Estimated Cost of Construction 120,000.00

Subdivision Name N/A Lot ____ Block ____ Unit ____ Phase ____

Driving Directions 90 W, (L) 247, (R) 242 about 2.5 miles on the (L) see mailbox.

Type of Construction SFD, Utility Number of Existing Dwellings on Property 0

Total Acreage 3.00 Lot Size ____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Driv

Actual Distance of Structure from Property Lines - Front 100' Side 102' Side 102' Rear 350'

Total Building Height 21' Number of Stories ONE Heated Floor Area 2755 (3744) Roof Pitch 9/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Lawanda Y. Rentz

Owner Builder or Authorized Person by Notarized Letter

Contractor Signature

Contractors License Number

Competency Card Number

STATE OF FLORIDA
COUNTY OF COLUMBIA

NOTARY STAMP/SEAL

LAURIE HODSON
MY COMMISSION # DD 38[illegible]
EXPIRES: June 28, 20[illegible]
Bonded Thru Notary Public Underwriters

Sworn to (or affirmed) and subscribed before me this 28 day of January 20 08.

Personally known ____ or Produced Identification ____

Notary Signature

(Revised Sept 2006)

Tel called LAWANDA 2-7-08

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Laurinda Y. Kentz

Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)

Contractor's License Number_____
Columbia County
Competency Card Number_____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this ____ day of _____ 20___.

Personally known_____ or Produced Identification_____

SEAL:

State of Florida Notary Signature (For the Contractor)

AFFIDAVIT OF SUBDIVIDED REAL PROPERTY
FOR USE OF IMMEDIATE FAMILY MEMBERS
FOR PRIMARY RESIDENCE

STATE OF FLORIDA
COUNTY OF COLUMBIA

BEFORE ME the undersigned Notary Public personally appeared.

WALTER RENTZ, the Owner of the parent tract which has been subdivided for immediate family primary residence use, hereinafter the Owner, and LAWANDA RENTZ, the family member of the Owner, who is the owner of the family parcel which is intended for immediate family primary residence use, hereafter the Family Member, and is related to the Owner as DAUGHTER, and both individuals being first duly sworn according to law, depose and say:

1. Both the Owner and the Family Member have personal knowledge of all matters set forth in this Affidavit.

2. The Owner holds fee simple title to certain real property situated in Columbia County, and more particularly described by reference to the Columbia county Property Appraiser Tax Parcel No. 25-4S-15-00388-000.

3. The Owner has divided his parent parcel for use of immediate family members for their primary residence and the parcel divided and the remaining parent parcel are at least ½ acre in size. Immediate family is defined as grandparent, parent, step-parent, adopted parent, sibling, child, step-child, adopted child or grandchild.

4. The Family Member is a member of the Owner's immediate family, as set forth above, and holds fee simple title to certain real property divided from the Owner's parcel situated in Columbia County and more particularly described by reference to the Columbia County Property Appraiser Tax Parcel No. 25-4S-15-00388-001.

5. No person or entity other than the Owner and Family Member claims or is presently entitled to the right of possession or is in possession of the property, and there are no tenancies, leases or other occupancies that affect the Property.

6. This Affidavit is made for the specific purpose of inducing Columbia County to recognize a family division for a family member on the parcel divided in accordance with Section 14.9 of the Columbia County Land Development Regulations.

7. This Affidavit is made and given by Affiants with full knowledge that the facts contained herein are accurate and complete, and with full knowledge that the penalties under Florida law for perjury include conviction of a felony of the third degree.

We Hereby Certify that the information contained in this Affidavit are true and correct.

Walter J. Rentz
Owner

Lawanda Y. Rentz
Family Member

Walter J. Rentz
Typed or Printed Name

Lawanda Y. Rentz
Typed or Printed Name

Subscribed and sworn to (or affirmed) before me this 29 day of January, 2008, by Lawanda Rentz (~~Owner~~) Family Member who is personally known to me or has produced _____ as identification.

Laurie Hodson
Notary Public

LAURIE HODSON
MY COMMISSION # DD 333503
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

Subscribed and sworn to (or affirmed) before me this 30th day of January, 2008, by Walter J Rentz (~~Family Member~~) owner who is personally known to me or has produced _____ as identification.

Tracy L. Duckett
Notary Public

Notary Public State of Florida
Tracy L Duckett
My Commission DD462701
Expires 08/31/2009

NOTORIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F .I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

(✓) Single Family Dwelling **( ) Two-Family Residence**
( ) Farm Outbuilding **( ) Other_____**

NEW CONSTRUCTION OR IMPROVEMENT

( ) New Construction **( ) Addition, Alteration, Modification or other Improvement**

I Lawanda Renty **, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number** _____

Lawanda Y. Renty 1-28-08
Owner Builder Signature **Date**

The above signer is personally known to me or produced identification _____

LAURIE HODSON
MY COMMISSION # DD 333503
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

Notary Signature L. H **Date** 1-28-08 **(Stamp / Seal)**

FOR BUILDING USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7).

Date 1-28-2008 **Building Official/Representative** _____

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 1/22/2008 **DATE ISSUED:** 1/23/2008

ENHANCED 9-1-1 ADDRESS:

7148 SW COUNTY ROAD 242

LAKE CITY FL 32024

PROPERTY APPRAISER PARCEL NUMBER:

25-4S-15-00388-000

Remarks:

PARENT PARCEL

Address Issued By: _____
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

1115

Approved Address

JAN 22 2008

911Addressing/GIS Dept

0801-135

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Austin Residence**	Builder:	**Owner**
Address:	**CR 242**	Permitting Office:	**Columbia Co.**
City, State:	**Lake City, FL 32055-**	Permit Number:	26759
Owner:	**Tony Austin**	Jurisdiction Number:	~~121000~~ 21000
Climate Zone:	**North**		

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		4	
5. Is this a worst case?		No	
6. Conditioned floor area (ft²)		2755 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear glass, default U-factor	0.0 ft²	327.0 ft²	
b. Default tint	0.0 ft²	0.0 ft²	
c. Labeled U or SHGC	0.0 ft²	0.0 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 281.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=19.0, 2156.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 2755.0 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage		Sup. R=6.0, 20.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 35.0 kBtu/hr	
	SEER: 14.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 35.0 kBtu/hr	
	HSPF: 7.90	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 30.0 gallons	
	EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT, CF,	

Glass/Floor Area: 0.12

Total as-built points: 31075
Total base points: 40259

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Tim Delbene

DATE: 11/12/07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **CR 242, Lake City, FL, 32055-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	2755.0	20.04	9937.8	Double, Clear	NW	2.0	5.0	16.0	25.97	0.84	347.3	
				Double, Clear	NW	2.0	8.0	25.0	25.97	0.93	600.9	
				Double, Clear	NW	13.0	9.0	24.0	25.97	0.58	362.5	
				Double, Clear	NW	4.0	8.0	40.0	25.97	0.78	814.5	
				Double, Clear	W	2.0	8.0	34.0	38.52	0.91	1196.3	
				Double, Clear	SW	2.0	8.0	15.0	40.16	0.89	533.2	
				Double, Clear	NE	2.0	9.0	35.0	29.56	0.94	971.1	
				Double, Clear	SE	2.0	8.0	60.0	42.75	0.88	2265.9	
				Double, Clear	SE	5.0	8.0	30.0	42.75	0.60	773.6	
				Double, Clear	SE	12.0	9.0	48.0	42.75	0.45	917.1	
				As-Built Total:				**327.0**			**8782.6**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			19.0	2156.0	0.90		1940.4	
Exterior	2156.0	1.70	3665.2									
Base Total:	**2156.0**		**3665.2**	**As-Built Total:**				**2156.0**			**1940.4**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	21.0	2.40	50.4	Exterior Insulated				42.0	4.10		172.2	
Exterior	62.0	6.10	378.2	Adjacent Insulated				21.0	1.60		33.6	
				Exterior Insulated				20.0	4.10		82.0	
Base Total:	**83.0**		**428.6**	**As-Built Total:**				**83.0**			**287.8**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	2755.0	1.73	4766.1	Under Attic			30.0	2755.0	1.73 X 1.00		4766.1	
Base Total:	**2755.0**		**4766.1**	**As-Built Total:**				**2755.0**			**4766.1**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	281.0(p)	-37.0	-10397.0	Slab-On-Grade Edge Insulation			0.0	281.0(p	-41.20		-11577.2	
Raised	0.0	0.00	0.0									
Base Total:			**-10397.0**	**As-Built Total:**				**281.0**			**-11577.2**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	2755.0	10.21	28128.6					2755.0	10.21		28128.6	

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **CR 242, Lake City, FL, 32055-**	PERMIT #:

BASE				AS-BUILT						
Summer Base Points:			**36529.3**	**Summer As-Built Points:**						**32328.3**
Total Summer Points	X System Multiplier	= Cooling Points		Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points	
36529.3	**0.4266**	**15583.4**		32328.3 **32328.3**	1.000 **1.00**	(1.090 x 1.147 x 1.00) **1.250**	0.244 **0.244**	0.902 **0.902**	8892.6 **8892.6**	

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **CR 242, Lake City, FL, 32055-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	2755.0	12.74	6317.8	Double, Clear	NW	2.0	5.0	16.0	24.30	1.01	392.4	
				Double, Clear	NW	2.0	8.0	25.0	24.30	1.00	609.3	
				Double, Clear	NW	13.0	9.0	24.0	24.30	1.03	600.5	
				Double, Clear	NW	4.0	8.0	40.0	24.30	1.01	984.6	
				Double, Clear	W	2.0	8.0	34.0	20.73	1.02	721.3	
				Double, Clear	SW	2.0	8.0	15.0	16.74	1.06	266.2	
				Double, Clear	NE	2.0	9.0	35.0	23.57	1.00	828.5	
				Double, Clear	SE	2.0	8.0	60.0	14.71	1.10	967.3	
				Double, Clear	SE	5.0	8.0	30.0	14.71	1.53	674.1	
				Double, Clear	SE	12.0	9.0	48.0	14.71	2.19	1547.3	
				As-Built Total:				**327.0**			**7591.6**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			19.0	2156.0	2.20		4743.2	
Exterior	2156.0	3.70	7977.2									
Base Total:	**2156.0**		**7977.2**	**As-Built Total:**				**2156.0**			**4743.2**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	21.0	11.50	241.5	Exterior Insulated				42.0	8.40		352.8	
Exterior	62.0	12.30	762.6	Adjacent Insulated				21.0	8.00		168.0	
				Exterior Insulated				20.0	8.40		168.0	
Base Total:	**83.0**		**1004.1**	**As-Built Total:**				**83.0**			**688.8**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	2755.0	2.05	5647.8	Under Attic			30.0	2755.0	2.05 X	1.00	5647.8	
Base Total:	**2755.0**		**5647.8**	**As-Built Total:**				**2755.0**			**5647.8**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	281.0(p)	8.9	2500.9	Slab-On-Grade Edge Insulation			0.0	281.0(p	18.80		5282.8	
Raised	0.0	0.00	0.0									
Base Total:			**2500.9**	**As-Built Total:**				**281.0**			**5282.8**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	2755.0	-0.59	-1625.4					2755.0	-0.59		-1625.4	

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **CR 242, Lake City, FL, 32055-**	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**21822.3**	**Winter As-Built Points:**					**22328.7**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
21822.3	**0.6274**	**13691.3**	22328.7 **22328.7**	1.000 **1.00**	(1.069 x 1.169 x 1.00) **1.250**	0.432 **0.432**	0.950 **0.950**	11442.1 **11442.1**

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **CR 242, Lake City, FL, 32055-** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
4		2746.00	10984.0	30.0	0.90	4	1.00	2684.98	1.00	10739.9	
				As-Built Total:						**10739.9**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
15583	**13691**	**10984**	**40259**	**8893**	**11442**	**10740**	**31075**

PASS

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **CR 242, Lake City, FL, 32055-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	N/A
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	N/A
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

NOTICE OF COMMENCEMENT

#26759

County Clerk's Office Stamp or Seal

Tax Parcel Identification Number 25-4S-15-00388-001

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes. the following information is provided in this **NOTICE OF COMMENCEMENT.**

1. Description of property *(legal description)*: 7148 SW CR 242
   a) Street *(job)* Address: LAKE CITY, FL 32025
2. General description of improvements: SINGLE FAMILY DWELLING
3. 0wner Information
   a) Name and address: LAWANDA RENTZ
   b) Name and address of fee simple titleholder (if other than owner) _____
   c) Interest in property 100 %
4. Contractor Information
   a) Name and address: LAWANDA RENTZ and TONY AUSTIN
   b) Telephone No.: 386.961.8777 Fax No. (Opt.) _____
5. Surety Information
   a) Name and address: _____
   b) Amount of Bond: _____
   c) Telephone No.: _____
6. Lender
   a) Name and address: N/A
   b) Phone No. _____
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
   a) Name and address: _____
   b) Telephone No.: _____ Fax No. (Opt.) _____
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(l)(b). Florida Statutes:
   a) Name and address: _____
   b) Telephone No.: _____ Fax No. (Opt.) _____
9. Expiration date of Notice of Commencement **(the expiration date is one year from the date of recording unless a different date is specified)**: 03-11-2010

Inst:200812004795 Date:3/11/2008 Time:1:42 PM
DC,P.DeWitt Cason,Columbia County Page 1 of 1

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

10. Lawanda Y. Rentz
Signature of Owner or Owner's Authorized Office/Director/Partner/Manager

Lawanda Y. Rentz
Print Name

The foregoing instrument was acknowledged before me , a Florida Notary, this 30th day of January, 2008, by:

_____ as _____ **(type of authority, e.g. officer, trustee, attorney fact) for** Lawanda Y. Rentz **(name of party on behalf of whom instrument was executed).**

Personally Known ✓ OR Produced Identification ____ Type _____

Notary Signature Tracy L. Duckett Notary Stamp or Seal:

Notary Public State of Florida
Tracy L Duckett
My Commission DD462701
Expires 08/31/2009

—AND—

11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

Lawanda Y Rentz
Signature of Natural Person Signing (in line #10 above.)

Cal-Tech Testing, Inc.

- Engineering
- Geotechnical
- Environmental

LABORATORIES

P.O. Box 1625 • Lake City, FL 32056 Tel. (386) 755-3633 • Fax (386) 752-5456

4784 Rosselle Street • Jacksonville, FL 32254 Tel. (904) 381-8901 • Fax (904) 381-8902

March 13, 2009

Dale's Excavation, Inc.
6139 SW SR 47
Lake City, Florida 32024

Attention: Mr. Dale Peeler

Subject: Construction Materials Testing
Walter Rentz Residence
Lake City, Columbia County, Florida
Cal-Tech Project No. 08-00293

Dear Mr. Peeler:

As requested by you, Cal-Tech Testing, Inc. (CTI) representatives visited the subject site to performed sampling and testing of soil backfill within the house pad area. Attached are results of tests performed during our site visits.

CTI's limited subsurface exploration report dated March 21, 2008 stated that slab-on-grades should be supported on a minimum of 3 feet of well compacted, non-expansive soils. The existing soils beneath should be removed 3 feet in depth from the existing grades and a minimum of 5 feet beyond all building perimeters.

CTI's personnel did not physically perform grade checks but did perform nuclear density testing on soil that had been placed within the building slab area. The density test's that were performed represented lift numbers 1-3 for a total of 3 feet of new fill.

We appreciate this opportunity of working with you on this project and look forward to serving you on future projects. Should you have any questions and or comments concerning this report, please contact our office at 386-755-3633.

Sincerely,
Cal-Tech Testing, Inc.

3/13/09

Nabil O. Hmeidi, P.E.
Senior Geotechnical Engineer
Licensed, Florida No. 57842

Distribution: *file*
Addressee
Mr. Johnny Kerce – Columbia County Building Department

Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
Laboratories

P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456
4784 Rosselle St., Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902
2230 Greensboro Hwy • Quincy, FL 32351 • Tel(850)442-3495 • Fax(850)442-4008

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 08-00293-01
DATE TESTED: 5/30/08
DATE REPORTED: 6/4/08

PROJECT:	Walter Rentz Residence, Lake City, FL
CLIENT:	Dale's Excavation, Inc., 6139 SW SR 47, Lake City, FL 32024
GENERAL CONTRACTOR:	Dale's Excavation, Inc.
EARTHWORK CONTRACTOR:	Dale's Excavation, Inc.
INSPECTOR:	Richard Kramer

ASTM METHOD	SOIL USE
(D-2922) Nuclear	BASE COURSE

SPECIFIED REQUIREMENTS: 95%

TEST NO.		TEST LOCATION	LIFT	TEST DEPTH	WET DENSITY (lb/ft³)	MOISTURE PERCENT	DRY DENSITY (lb/ft³)	PROCTOR TEST NO.	PROCTOR VALUE	% MAXIMUM DENSITY
1		50'N of SW Corner x 20'E of SW Corner	1	0-12"	120.2	11.5	107.8	1	113.9	95%
2		55'N of SW Corner x 20'E of SW Corner	2	0-12"	118.8	6.7	111.3	1	113.9	98%

REMARKS: The Above Tests Meet Specified Requirements.

PROCTORS

PROCTOR NO.		SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft³)	OPT. MOIST.	TYPE
1		Brown Sand Trace of Silt	113.9	10.6	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

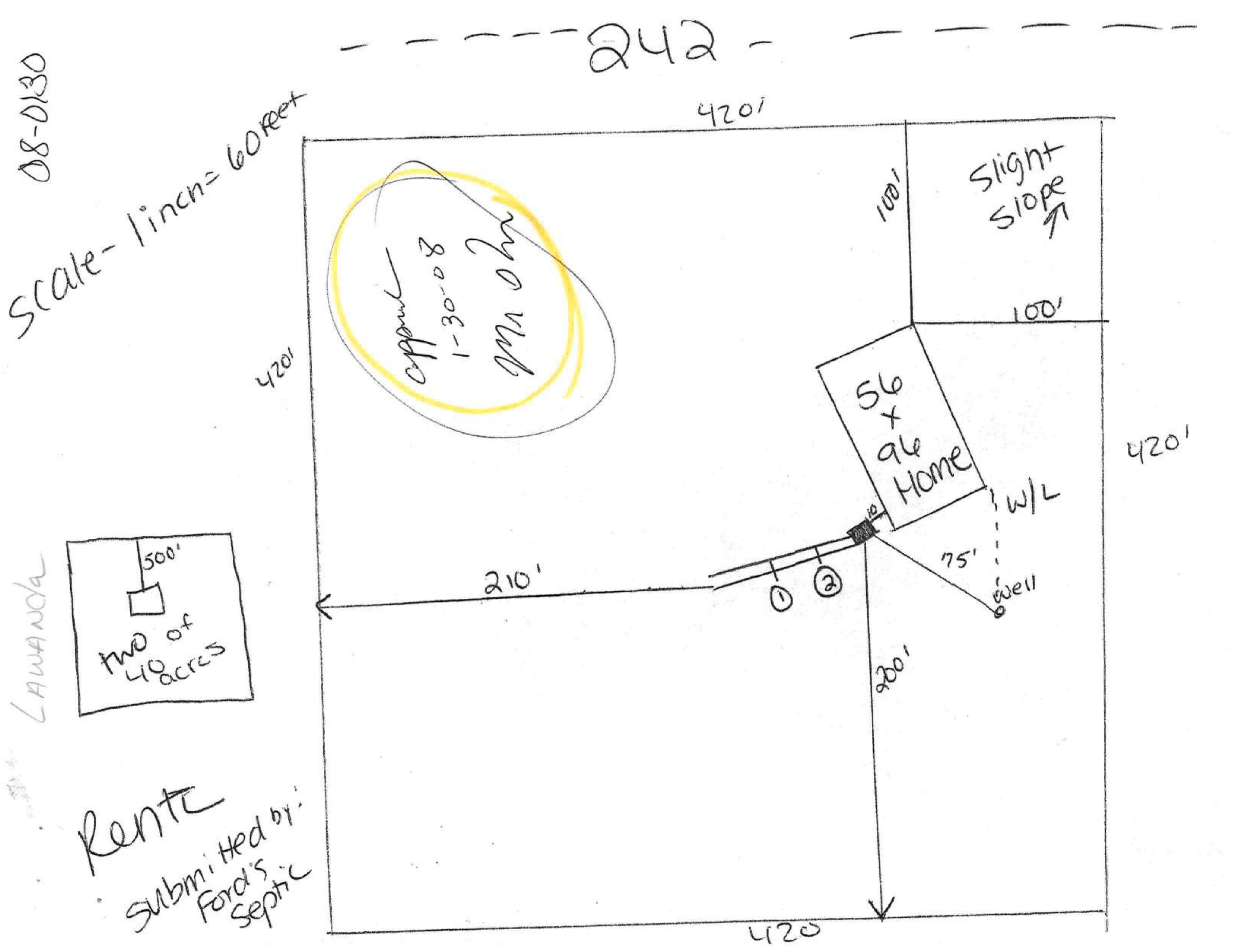
Reviewed By:

Date: 6/5/08
Licensed, Florida No: 57842

sw

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

Seal's Copy
26759


08-0130
Scale- 1inch= 60 feet
242
420'
420'
420'
420
100'
100'
Slight slope
56 x 96 Home
W/L
10
75'
Well
210'
200'
1
2
appvd 1-30-08
500'
two of 40 acres
LAWANDA
Rentz
Submitted by: Ford's Septic

Inst:200812001915 Date:1/30/2008 Time:1:40 PM
Doc Stamp-Deed:0.70
DC,P.DeWitt Cason,Columbia County Page 1 of 3

Above Space Reserved for Recording
[If required by your jurisdiction, list above the name & address of: 1) where to return this form; 2) preparer; 3) party requesting recording.]

Quitclaim Deed

Date of this Document: JANUARY 30, 2008

Reference Number of Any Related Documents: _____

Grantor:

Name WALTER J. Rentz
Street Address 7058 SW CR 242
City/State/Zip LAKE City, FL 32024

Grantee:

Name LAWANDA Y. Rentz
Street Address 208 S.E. MARY ETTA TERR
City/State/Zip LAKE City, FL 32025

Abbreviated Legal Description (i.e., lot, block, plat *or* section, township, range, quarter/quarter *or* unit, building and condo name): _____

Assessor's Property Tax Parcel/Account Number(s): 25-4S-15-00388-000 PARENT PARCEL

THIS QUITCLAIM DEED, executed this 30 day of JANUARY, 2008, by first party, Grantor, WALTER J. Rentz, whose mailing address is 7058 SW CR 242 LAKE City, FLA. 32024, to second party, Grantee, LAWANDA Y. RENTZ whose mailing address is 208 S.E. MARY ETTA TERR LAKE City, FL 32025

WITNESSETH that the said first party, for good consideration and for the sum of TEN DOLLARS Dollars ($ 10.00) paid by the said second party, the receipt whereof is hereby acknowledged, does hereby remise, release and quitclaim unto the said second party forever, all the right, title, interest and claim,

which the said first party has in and to the following described parcel of land, and improvements and appurtenances thereto in the County of Columbia, State of Florida to wit: See Schedule "A" exhibit

IN WITNESS WHEREOF, the said first party has signed and sealed these presents the day and year first written above. Signed, sealed and delivered in the presence of:

Signature of Witness
Print Name of Witness Wyndell Wallace

Signature of Witness
Print Name of Witness Tammy M Hartley

Signature of Grantor
Print Name of Grantor Walter J. Rentz

State of Florida)
County of Columbia)

On January 30, 2008, before me, Tracy L. Duckett, appeared Walter J Rentz, personally known to me (or proved to me on the basis of satisfactory evidence) to be the person(s) whose name(s) is/are subscribed to the within instrument and acknowledged to me that he/she/they executed the same in his/her/their authorized capacity(ies), and that by his/her/their signature(s) on the instrument the person(s), or the entity upon behalf of which the person(s) acted, executed the instrument.

WITNESS my hand and official seal.

Tracy L. Duckett
Signature of Notary

Affiant _____Known ✓ Produced ID
Type of ID _____

(Seal) Notary Public State of Florida
Tracy L. Duckett
My Commission DD462701
Expires 08/31/2009

Water Wells
Pumps & Service

Phone: (386) 752-6677
Fax: (386) 752-1477

Lynch Well Drilling, Inc.

173 SW Young Place
Lake City, FL 32025
www.lynchwelldrilling.com

January 24, 2008

To Whom It May Concern:

As required by building code regulations for Columbia County in order that a building permit can be issued, the following well information is provided with regard to the Tony Austin & Lawanda Rentz well on 242 Parcel # 00388-000.

Size of Pump Motor:	1 Horse Power 20 gallon GPM
Size of Pressure Tank:	81 -Gallon Bladder Tank - 25.1 Draw down
Cycle Stop Valve Used:	No
Constant Pressure System:	No

Should you require any additional information, please contact us.

Sincerely,

Linda Newcomb

Linda Newcomb
Lynch Well Drilling, Inc.

0801-135

5438.8	3540 Fin Frame	44x72 Insulated SSB Annealed
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-40 DP-47.2 Per manufacturers installation instructions.		**Certification Agency Certificate** **Installation Instructions** Verified By:
5438.9	3540 Fin Frame Triple with Continuous Head and Sill	108x72 Insulated SSB Annealed
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-35* DP-50 Per manufacturers installation instructions.		**Certification Agency Certificate** **Installation Instructions** Verified By:

MI WINDOWS and DOORS

CAPITOL BetterBilt CAPITOL ARCHITECTURAL PRODUCTS

INSTALLATION INSTRUCTIONS FOR NEW CONSTRUCTION VINYL FIN WINDOWS

READ THESE INSTRUCTIONS COMPLETELY BEFORE BEGINNING. Please inspect your MI Windows and Doors, Inc. product thoroughly before beginning installation. Inspect the opening and the product, and do not install if there is any observable damage or other irregularity. The product specification sheet and warranty include important information regarding your product and may include product-specific installation requirements (for example, types of fasteners to be used with impact resistant windows and limitations on the height at which the product may be installed); if you did not obtain copies please contact MI Windows and Doors, Inc. Local building codes may impose additional requirements, and those codes supercede these instructions.

FAILURE TO FOLLOW THESE INSTRUCTIONS, AND BUILDING CODE REQUIREMENTS, MAY AFFECT THE REMEDIES AVAILABLE UNDER YOUR WARRANTY.

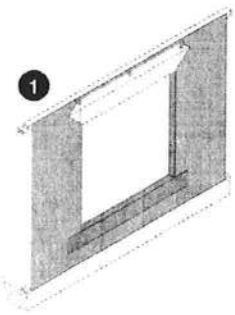
1. IF THE BUILDING HAS A WEATHER RESISTANT BARRIER (WRB) I.E. HOUSE WRAP, PREPARE THE OPENING ACCORDING TO WRB MANUFACTURER'S INSTRUCTIONS. AT EACH TOP CORNER MAKE A 45° CUT IN THE WRB. FOLD UP THE WRB SO THAT THE TOP NAIL FIN OF THE UNIT CAN BE INSTALLED UNDERNEATH IT. (See Figure 1 below) FLASHING OF THE WINDOW OPENING IS RECOMMENDED AND MAY BE REQUIRED BY SOME BUILDING CODES.
2. MAKE SURE THE ROUGH OPENING IS PLUMB, SQUARE AND THE SILL PLATE IS LEVEL. ROUGH OPENINGS SHOULD BE 1/2" LARGER THAN WINDOW FRAME IN WIDTH & HEIGHT. (See Figure 2 below)
3. CLOSE & LOCK THE SASH THROUGHOUT INSTALLATION. KEEP THE SIDE JAMBS PLUMB & SQUARE WITH HEAD AND SILL. BE CAREFUL NOT TO "CROWN UP" OR "BOW DOWN" THE SILL OR HEAD. CONSTANTLY CHECK WIDTH AT THE MEETING RAILS OF SINGLE AND DOUBLE HUNGS (CENTER POINT ON CASEMENTS) TO AVOID A "BOWED OUT" INSTALLATION. WHEN USING FLASHING APPLY THE BOTTOM PIECE BEFORE INSTALLING THE WINDOW. (See Figure 1 below) FLASHING MUST BE RATED TO MEET ASTM D-779, 24 HOUR WATER RESISTANCE TEST.
4. APPLY A CONTINUOUS 3/8" BEAD OF PREMIUM GRADE, COMPATIBLE EXTERIOR SEALANT TO THE INTERIOR (BACKSIDE) OF THE NAIL FIN NEAR THE OUTSIDE EDGE IN LINE WITH THE PRE-PUNCHED HOLES ON **ALL SIDES PRIOR** TO SETTING THE WINDOW INTO THE ROUGH OPENING. (See Figure 3 below)
5. PLACE 1/4" FLAT SHIMS ON THE ROUGH OPENING SILL PLATE UNDER THE BOTTOM CORNERS OF THE WINDOW (See Figure 4 below). THESE SHIMS SHOULD BE REMOVED WHEN INSTALLATION IS COMPLETE. DO NOT PLACE SHIMS OR BLOCKS UNDER THE SILL EXCEPT AT THE FRAME CORNERS. SET THE WINDOW ONTO THE SHIMS CENTERING THE WINDOW IN THE OPENING ALLOWING EQUAL SPACE ON EITHER SIDE. FOR WINDOWS WITH INTERMEDIATE JAMBS AND ALL SLIDER WINDOWS, CONTINUOUS SHIM OR HORIZONTAL SHIMS ARE RECOMMENDED UNDER EACH INTERMEDIATE JAMB AND MEETING RAIL TO ENSURE SILL IS LEVEL). THESE SILL SHIMS SHOULD REMAIN AFTER INSTALLATION IS COMPLETE. APPLY ADDITIONAL SHIMS AS NECESSARY TO MAINTAIN A LEVEL SILL THROUGHOUT INSTALLATION.
6. PLACE A TEMPORARY FASTENER IN THE SLOT PROVIDED IN THE NAIL FIN ON EACH TOP CORNER, CHECK LEVEL AND SQUARE OF THE WINDOW BY MEASURING THE DIAGONALS. OPEN BOTTOM SASH, CHECK THE "REVEAL" (SPACE) BETWEEN THE BOTTOM OF THE SASH AND THE WINDOW SILL. CLOSE AND RELOCK THE SASH, ADJUST IF NECESSARY. PLACE ADDITIONAL FASTENERS IN THE BOTTOM CORNERS CHECKING WINDOW AGAIN FOR LEVEL, PLUMB AND SQUARE.
7. SECURE THE WINDOW WITH FASTENERS THAT PENETRATE THE FRAMING BY A MINIMUM OF 1", CARE SHOULD BE TAKEN TO INSTALL FASTENERS STRAIGHT, NOT ANGLED. KEEP THE SASH LOCKED UNTIL ALL SIDES ARE SECURE. PRIOR TO FASTENING THE SILL AND HEAD BE SURE THEY ARE STRAIGHT AND LEVEL. FASTENERS SHOULD BE APPLIED SECURELY INTO EVERY OTHER SLOT ON ALL SIDES, DO NOT DISTORT THE NAIL FIN WITH THE FASTENERS.
8. APPLY SEALANT OVER EXPOSED FASTENER HEADS, ANY UNUSED SLOTS AND THE OUTSIDE EDGE OF THE NAIL FIN WHERE IT COMES IN CONTACT WITH THE WRB/SHEATING. **OR IF FLASHING (WINDOW TAPE) IS BEING USED** - NOTE: SILL FLASHING SHOULD HAVE BEEN APPLIED PRIOR TO INSTALLING THE WINDOW. APPLY THE SIDE FLASHING ON TOP OF THE NAIL FIN, OVERLAPPING THE SILL FLASHING AND EXTENDING UP PAST THE TOP NAIL FIN APPROXIMATELY 2". THEN APPLY THE TOP FLASHING ALSO OVER THE NAIL FIN, OVERLAPPING THE SIDE PIECES AND EXTENDING PAST THE SIDE FLASHING BY APPROXIMATELY 1". LASTLY FOLD DOWN THE WRB FLAP OVER THE FLASHING, TAPE THE DIAGONAL CUTS ABOVE EACH CORNER. (SEE FIGURE #5 BELOW)
9. PLACE SHIMS AT THE MEETING RAIL/CHECK RAIL ON THE SIDE JAMBS TO PREVENT BOWING. THESE SHIMS SHOULD REMAIN AFTER INSTALLATION. CAUTION SHOULD BE TAKEN AS TO NOT OVER SHIM, CAUSING DEFLECTION OF THE FRAME AND HINDER SASH OPERATION. CHECK THE FRAME WIDTH AT TOP, MIDDLE AND BOTTOM, IF NOT THE SAME, SHIM ACCORDINGLY. UNLOCK AND OPERATE THE SASH(S). VISUALLY INSPECT ALL SIGHT LINES. ADJUST OR SHIM AS REQUIRED TO ASSURE CONSISTENT SASH REVEAL AND EASE OF OPERATION.
10. INSULATE BETWEEN THE WINDOW FRAME & ROUGH OPENING WITH FIBERGLASS INSULATION OR EQUAL. THE SPACE MAY BE EFFECTIVELY FILLED WITH MEASURED USE OF LOW EXPANSION FOAM BUT ONLY AFTER DETERMINING THAT FOAM WILL NOT EXERT PRESSURE AGAINST THE FRAME, WHICH CAN IMPAIR OPERATION. DISTORTION OF THE FRAME WILL AFFECT THE USER'S RIGHTS UNDER THE WARRANTY.
11. ALLOW A 1/4" GAP BETWEEN THE EXTERIOR CLADDING, SIDING, BRICK, STUCCO OR STONE AND THE WINDOW FRAME ON ALL SIDES (EXCEPT VINYL J CHANNEL). THE GAP (EXPANSION JOINT) SHOULD BE FILLED WITH CORRECT SIZE BACKER ROD, THEN SEALED WITH A HIGH GRADE EXTERIOR SEALANT AND WILL NEED TO BE MAINTAINED.

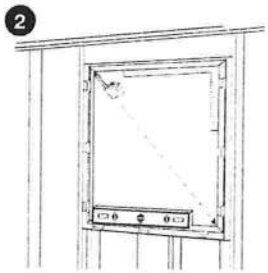
CAUTION:

- USE OF SOLVENTS OR ACIDS WILL DAMAGE COMPONENTS OF THIS PRODUCT AND WILL LIMIT RIGHTS UNDER THE WARRANTY
- VINYL WINDOWS HAVE PRE-PUNCHED SLOTS FOR INSTALLATION - FASTENING IN ANY OTHER PORTION MAY PERMANENTLY DAMAGE UNIT WHICH WILL LIMIT RIGHTS UNDER THE WARRANTY.
- IT IS THE SOLE RESPONSIBILITY OF THE OWNER, ARCHITECT, AND/OR BUILDER TO SELECT CORRECT PRODUCTS TO BE IN COMPLIANCE WITH APPLICABLE LAWS, SITE REQUIREMENTS AND BUILDING CODES AND TO ENSURE THAT INSTALLATION IS IN COMPLIANCE WITH APPLICABLE LAWS, SITE REQUIREMENTS AND BUILDING CODES.
- DO NOT STORE IN THE SUN OR LAY FLAT BEFORE OR DURING INSTALLATION.
- ANY PENETRATIONS (e.g. ALARM SENSORS) MADE THROUGH ANY PORTION OF ANY M.I., BETTERBILT OR CAPITOL PRODUCT MAY AFFECT RIGHTS UNDER THE MANUFACTURER'S WARRANTY.
- SOME LAWS AND BUILDING CODES REQUIRE SAFETY GLASS. THE ORDERING PARTY IS RESPONSIBLE TO SPECIFY SAFETY GLASS AND ENSURE COMPLIANCE WITH LOCAL LAWS AND BUILDING CODES.

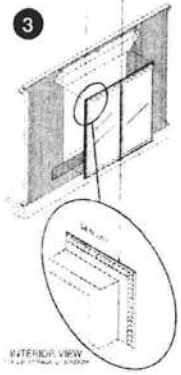
THESE INSTRUCTIONS ARE MINIMUM REQUIREMENTS ONLY, CHECK STATE AND LOCAL CODE RESTRICTIONS FOR ADDITIONAL COMPLIANCE ON INSTALLATION AND/OR FASTENING. IF UNIT HAS EXTERIOR TRIM (BRICKMOULD/J CHANNEL, ETC.) THE UNIT MUST BE SEALED BEHIND THE NAIL FIN. THE TRIM IS PROVIDED FOR AESTHETIC PURPOSES ONLY, AND NOT DESIGNED TO BE WATER TIGHT. INSTALLATION INTO MASONRY OR REPLACEMENT OPENINGS MUST BE SEALED TO THE OPENINGS USING AN APPROVED, PROPER METHOD. REFER TO AAMA 2400 AND/OR ASTM 2112 STANDARDS.

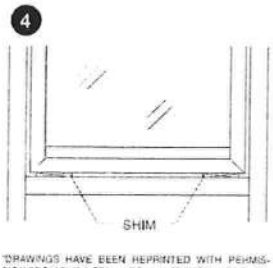
These installation instructions are provided for information only; no representation and warranty is made that these instructions set forth all of the information necessary for proper installation of the product. Given the variety of field conditions, primary responsibility for product installation rests with the installer. Do not proceed unless you have addressed the factors necessary to achieve weather-tight installation of a properly functioning product. MI Windows and Doors, Inc. assumes no liability for any personal injury or property damage incurred in installation. These instructions, together with the product specifications and warranty set forth the entire liability of MI Windows and Doors, Inc. with regard to the product.

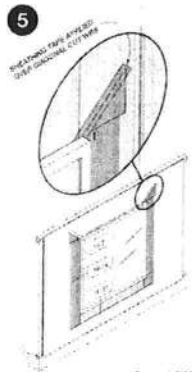




Check for level, plumb and square.







Revised 5/2006

0801-138

Builders FirstSource

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

Project Information for: L255815

Address: 7148 Southwest County Road 242
Lake City, FL
County: Columbia
Truss Count: 73
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

Truss Design Load Information:

Gravity:	**Wind:**	
Roof (psf): 42.0	Wind Standard: ASCE 7-02	Wind Exposure: B
Floor (psf): N/A	Wind Speed (mph): 110	

Note: See the individual truss drawings for special loading conditions.

Engineer of Record: Unknown at time of Seal Date
Address: Unknown at time of Seal Date

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Seal Date	No.	Drwg. #	Truss ID	Seal Date	No.	Drwg. #	Truss ID	Seal Date
1	J1931603	EJ1	2/4/08	29	J1931632	T09	2/4/08	57	J1931660	T37	2/4/08
2	J1931604	EJ1A	2/4/08	30	J1931633	T10	2/4/08	58	J1931661	T38	2/4/08
3	J1931605	EJ2	2/4/08	31	J1931634	T11	2/4/08	59	J1931662	T39	2/4/08
4	J1931606	EJ2A	2/4/08	32	J1931635	T12	2/4/08	60	J1931663	T40	2/4/08
5	J1931607	EJ2B	2/4/08	33	J1931636	T13	2/4/08	61	J1931664	T41	2/4/08
6	J1931608	EJ3	2/4/08	34	J1931637	T14	2/4/08	62	J1931665	T42	2/4/08
7	J1931609	EJ4	2/4/08	35	J1931638	T15	2/4/08	63	J1931666	T43	2/4/08
8	J1931610	EJ4A	2/4/08	36	J1931639	T16	2/4/08	64	J1931667	T44	2/4/08
9	J1931611	EJ4B	2/4/08	37	J1931640	T17	2/4/08	65	J1931668	T45	2/4/08
10	J1931612	EJ4C	2/4/08	38	J1931641	T18	2/4/08	66	J1931669	T47	2/4/08
11	J1931613	EJ4D	2/4/08	39	J1931642	T19	2/4/08	67	J1931670	T48	2/4/08
12	J1931614	EJ5	2/4/08	40	J1931643	T20	2/4/08	68	J1931671	T49	2/4/08
13	J1931615	EJ5A	2/4/08	41	J1931644	T21	2/4/08	69	J1931672	T50	2/4/08
14	J1931616	HJ2	2/4/08	42	J1931645	T22	2/4/08	70	J1931673	T51	2/4/08
15	J1931617	HJ2A	2/4/08	43	J1931646	T23	2/4/08	71	J1931674	T51G	2/4/08
16	J1931619	HJ3	2/4/08	44	J1931647	T24	2/4/08	72	J1931675	T52	2/4/08
17	J1931620	HJ4	2/4/08	45	J1931648	T25	2/4/08	73	J1931676	T53	2/4/08
18	J1931621	PB1	2/4/08	46	J1931649	T26	2/4/08				
19	J1931622	PB1A	2/4/08	47	J1931650	T27	2/4/08				
20	J1931623	PB2A	2/4/08	48	J1931651	T28	2/4/08				
21	J1931624	T01	2/4/08	49	J1931652	T29	2/4/08				
22	J1931625	T02	2/4/08	50	J1931653	T30	2/4/08				
23	J1931626	T03	2/4/08	51	J1931654	T31	2/4/08				
24	J1931627	T04	2/4/08	52	J1931655	T32	2/4/08				
25	J1931628	T05	2/4/08	53	J1931656	T33	2/4/08				
26	J1931629	T06	2/4/08	54	J1931657	T34	2/4/08				
27	J1931630	T07	2/4/08	55	J1931658	T35	2/4/08				
28	J1931631	T08	2/4/08	56	J1931659	T36	2/4/08				

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931603
L255815	EJ1	JACK	4	1	Job Reference (optional)

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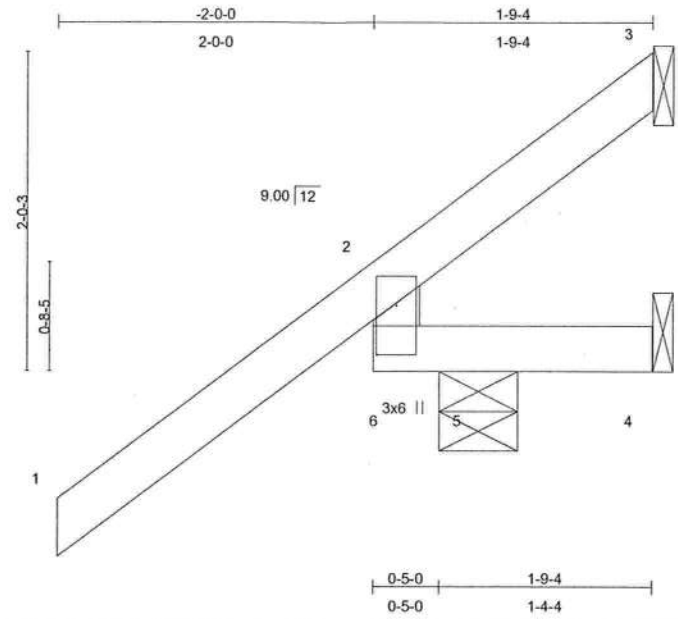

Plate Offsets (X,Y): [6:0-3-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.00	4-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	0.00	4-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.02	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 10 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-9-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=-115/Mechanical, 5=362/0-6-0, 3=-28/Mechanical
Max Horz 5=160(load case 6)
Max Uplift 4=-115(load case 1), 5=-327(load case 6), 3=-28(load case 1)
Max Grav 4=111(load case 6), 5=362(load case 1), 3=32(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-6=-231/248, 1-2=0/70, 2-3=-69/23
BOT CHORD 5-6=-0/180, 4-5=0/0

JOINT STRESS INDEX
2 = 0.00 and 6 = 0.67

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Continued on page 2

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931603
L255815	EJ1	JACK	4	1	Job Reference (optional)

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb uplift at joint 4, 327 lb uplift at joint 5 and 28 lb uplift at joint 3.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931604
L255815	EJ1A	MONO TRUSS	3	1	Job Reference (optional)

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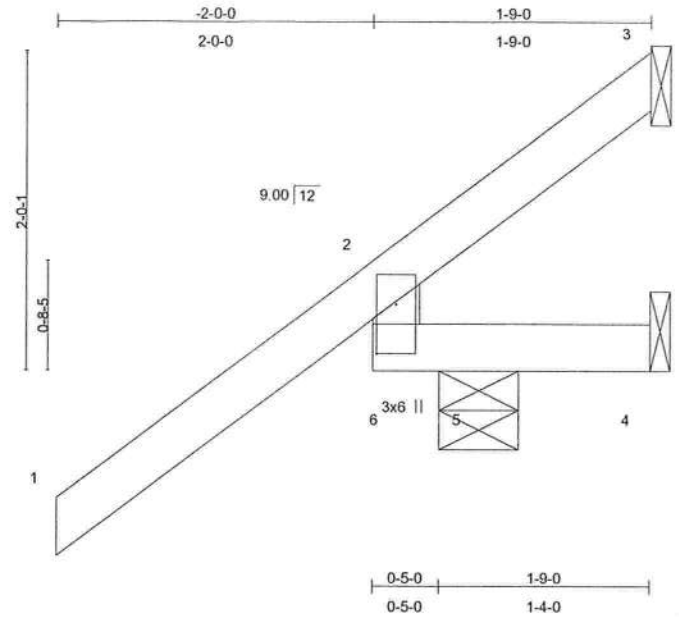

Plate Offsets (X,Y): [6:0-3-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.00	4-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	0.00	4-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.02	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 10 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-9-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=-117/Mechanical, 5=364/0-6-0, 3=-29/Mechanical
Max Horz 5=159(load case 6)
Max Uplift 4=-117(load case 1), 5=-331(load case 6), 3=-29(load case 1)
Max Grav 4=113(load case 6), 5=364(load case 1), 3=33(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/70, 2-3=-69/24, 2-6=-231/250
BOT CHORD 5-6=-0/180, 4-5=0/0

JOINT STRESS INDEX
2 = 0.00 and 6 = 0.67

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931604
L255815	EJ1A	MONO TRUSS	3	1	Job Reference (optional)

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 4, 331 lb uplift at joint 5 and 29 lb uplift at joint 3.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931605
L255815	EJ2	JACK	4	1	Job Reference (optional)

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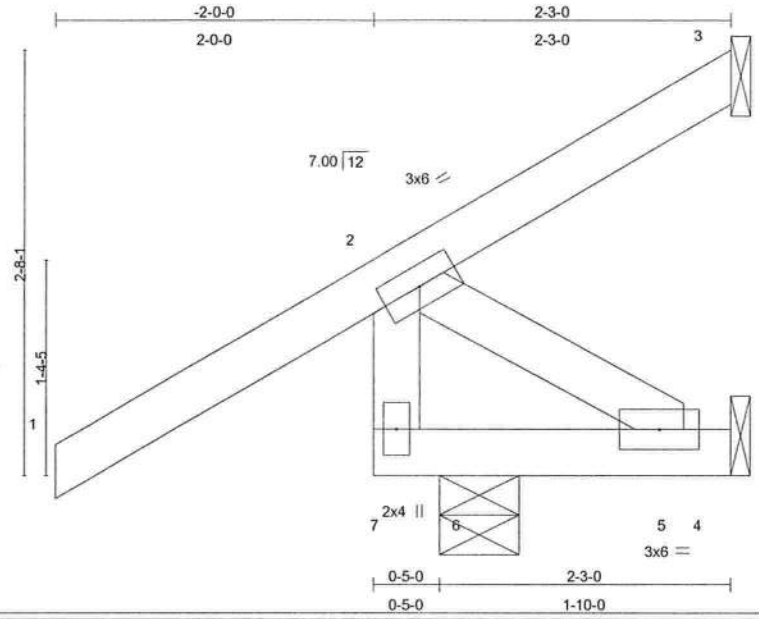

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.00	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	0.00	5-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.05	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=-6/Mechanical, 4=-73/Mechanical, 6=325/0-6-0
Max Horz 6=136(load case 6)
Max Uplift 3=-8(load case 9), 4=-73(load case 1), 6=-217(load case 6)
Max Grav 3=34(load case 6), 4=3(load case 4), 6=325(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-7=-232/173, 1-2=0/58, 2-3=-58/20
BOT CHORD 6-7=0/0, 5-6=-153/0, 4-5=0/0
WEBS 2-5=0/180

JOINT STRESS INDEX
2 = 0.10, 5 = 0.05 and 7 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931605
L255815	EJ2	JACK	4	1	Job Reference (optional)

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 3, 73 lb uplift at joint 4 and 217 lb uplift at joint 6.

LOAD CASE(S) Standard

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931606
L255815	EJ2A	MONO TRUSS	8	1	Job Reference (optional)

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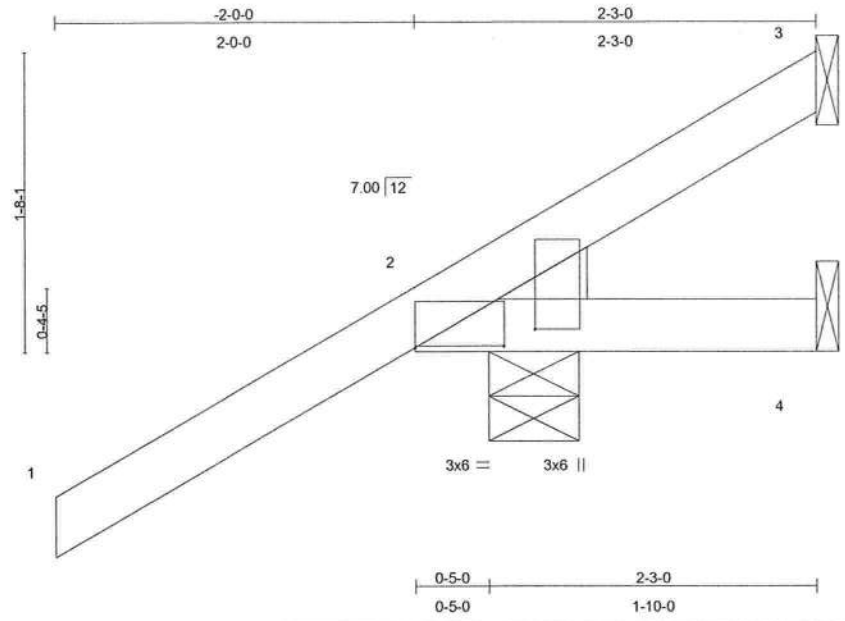

Plate Offsets (X,Y): [2:0-6-0,0-0-2], [2:0-1-4,0-8-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=-18/Mechanical, 2=254/0-6-0, 4=10/Mechanical
Max Horz 2=133(load case 6)
Max Uplift 3=-18(load case 1), 2=-231(load case 6)
Max Grav 3=37(load case 6), 2=254(load case 1), 4=29(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-69/25
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.12 and 2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931606
L255815	EJ2A	MONO TRUSS	8	1	Job Reference (optional)

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 3 and 231 lb uplift at joint 2.

LOAD CASE(S) Standard

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931607
L255815	EJ2B	MONO TRUSS	13	1	Job Reference (optional)

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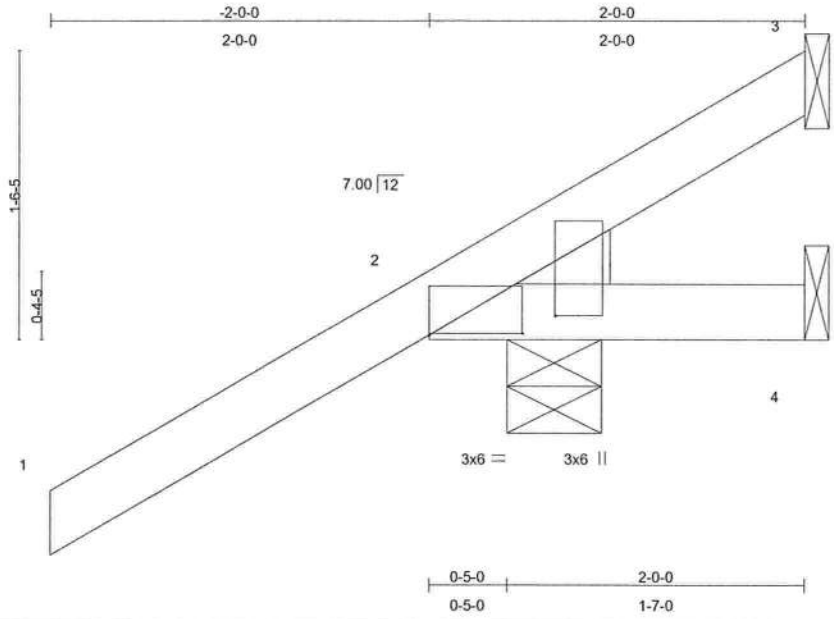

Plate Offsets (X,Y): [2:0-6-0,0-0-2], [2:0-1-4,0-8-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=231/0-6-0, 4=10/Mechanical, 3=-6/Mechanical
Max Horz 2=128(load case 6)
Max Uplift 2=-210(load case 6), 3=-9(load case 9)
Max Grav 2=231(load case 1), 4=29(load case 2), 3=29(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-59/17
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.12 and 2 = 0.09

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931607
L255815	EJ2B	MONO TRUSS	13	1	Job Reference (optional)

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 2 and 9 lb uplift at joint 3.

LOAD CASE(S) Standard

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February 4,200

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931608
L255815	EJ3	MONO TRUSS	2	1	Job Reference (optional)

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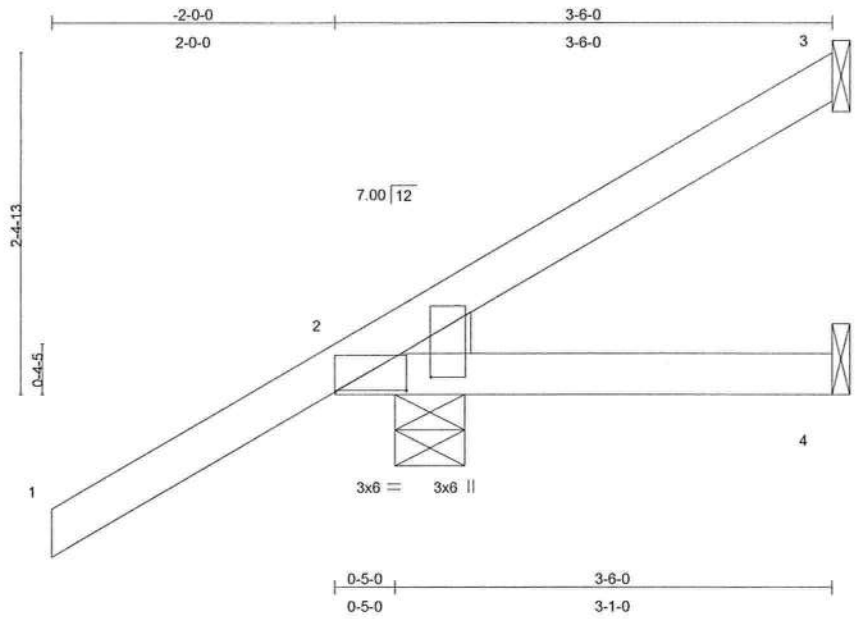

Plate Offsets (X,Y): [2:0-6-0,0-0-2], [2:0-1-4,0-8-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=43/Mechanical, 2=266/0-6-0, 4=16/Mechanical
Max Horz 2=167(load case 6)
Max Uplift 3=-40(load case 7), 2=-204(load case 6)
Max Grav 3=43(load case 1), 2=266(load case 1), 4=48(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-70/16
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13 and 2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931608
L255815	EJ3	MONO TRUSS	2	1	Job Reference (optional)

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 3 and 204 lb uplift at joint 2.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931609
L255815	EJ4	MONO TRUSS	8	1	Job Reference (optional)

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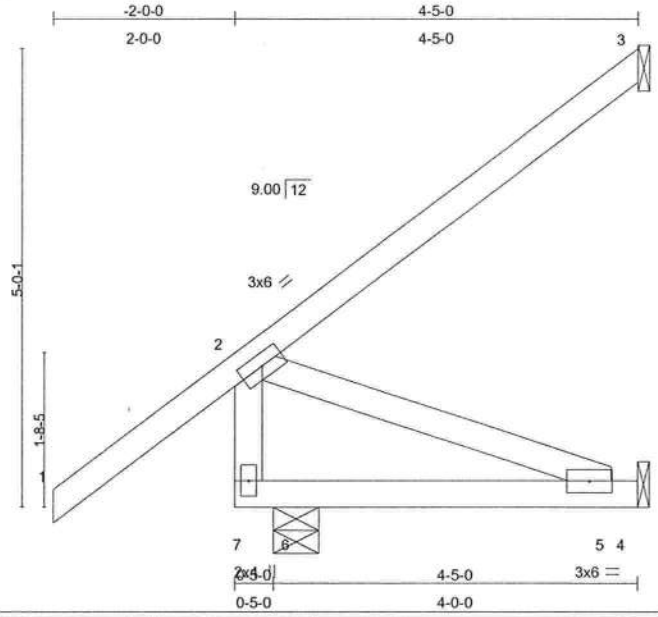

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.02	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	0.02	5-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.08	Horz(TL)	-0.02	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 27 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-5-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=84/Mechanical, 4=-19/Mechanical, 6=320/0-6-0
Max Horz 6=284(load case 6)
Max Uplift 3=-83(load case 6), 4=-67(load case 6), 6=-117(load case 6)
Max Grav 3=84(load case 1), 4=45(load case 2), 6=320(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/70, 2-3=-86/37, 2-7=-259/112
BOT CHORD 6-7=-16/35, 5-6=-314/0, 4-5=0/0
WEBS 2-5=0/335

JOINT STRESS INDEX
2 = 0.15, 5 = 0.09 and 7 = 0.09

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931609
L255815	EJ4	MONO TRUSS	8	1	Job Reference (optional)

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 3, 67 lb uplift at joint 4 and 117 lb uplift at joint 6.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931610
L255815	EJ4A	MONO HIP	2	1	Job Reference (optional)

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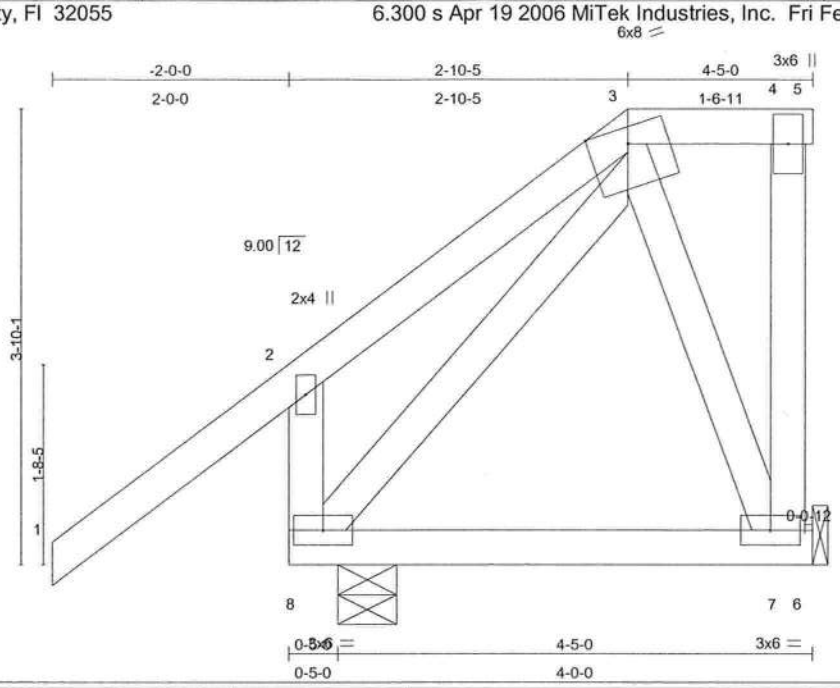

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.01	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.02	7-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.11	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 36 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 7=105/Mechanical, 8=276/0-6-0
Max Horz 8=234(load case 6)
Max Uplift 7=-102(load case 6), 8=-131(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/70, 2-3=-72/320, 3-4=0/0, 4-5=0/0, 4-7=-40/42, 2-8=-235/446
BOT CHORD 7-8=-49/16, 6-7=0/0
WEBS 3-7=-46/137, 3-8=-346/0

JOINT STRESS INDEX
2 = 0.24, 3 = 0.08, 4 = 0.01, 7 = 0.10 and 8 = 0.23

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces , and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 7 and 131 lb uplift at joint 8.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931610
L255815	EJ4A	MONO HIP	2	1	Job Reference (optional)

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931611
L255815	EJ4B	MONO HIP	2	1	Job Reference (optional)

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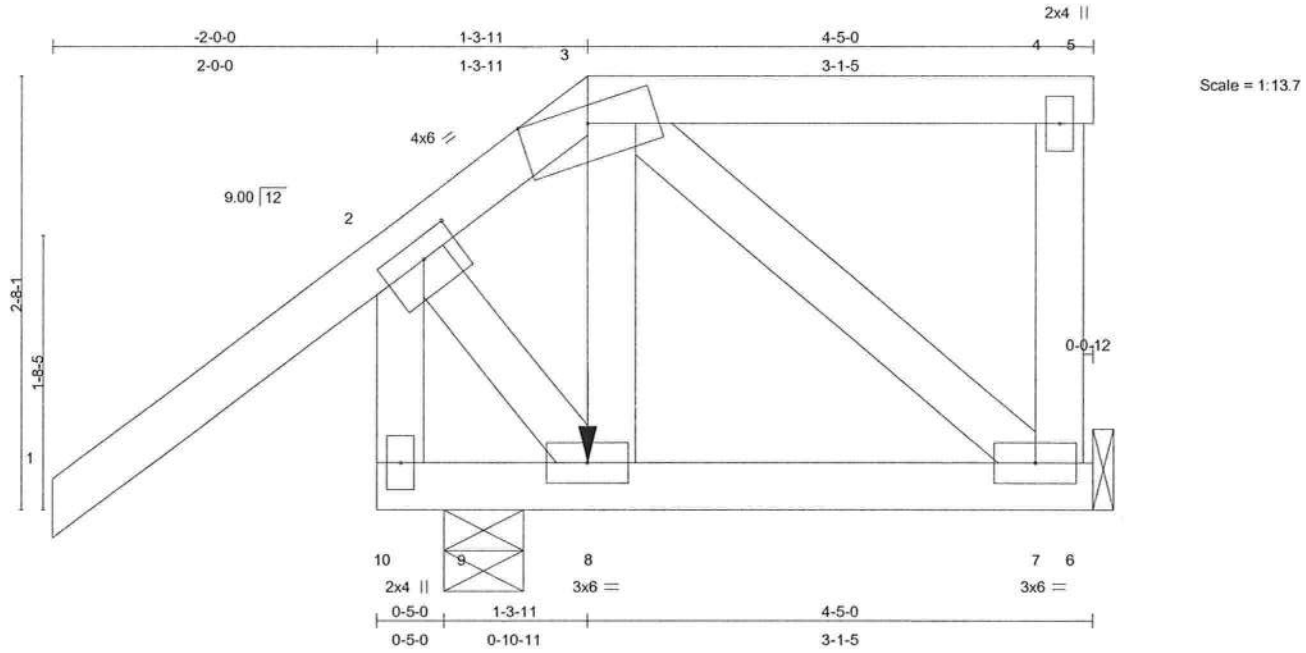

Plate Offsets (X,Y): [2:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.00	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.00	7-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.03	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
10-0-0 oc bracing: 6-7.

REACTIONS (lb/size) 7=65/Mechanical, 9=322/0-6-0
Max Horz 9=180(load case 5)
Max Uplift 7=-77(load case 4), 9=-203(load case 5)
Max Grav 7=73(load case 2), 9=322(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/70, 2-3=-20/37, 3-4=-17/10, 4-5=0/0, 4-7=-82/69, 2-10=-214/147
BOT CHORD 9-10=-78/125, 8-9=-78/19, 7-8=-35/7, 6-7=0/0
WEBS 3-7=-15/66, 3-8=-141/67, 2-8=-40/91

JOINT STRESS INDEX
2 = 0.77, 3 = 0.13, 4 = 0.26, 7 = 0.08, 8 = 0.05 and 10 = 0.55

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left exposed ; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931611
L255815	EJ4B	MONO HIP	2	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 77 lb uplift at joint 7 and 203 lb uplift at joint 9.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 2-3=-54, 3-4=-54, 4-5=-14, 6-10=-10
   Concentrated Loads (lb)
   Vert: 8=-7(F)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931612
L255815	EJ4C	MONO TRUSS	2	1	Job Reference (optional)

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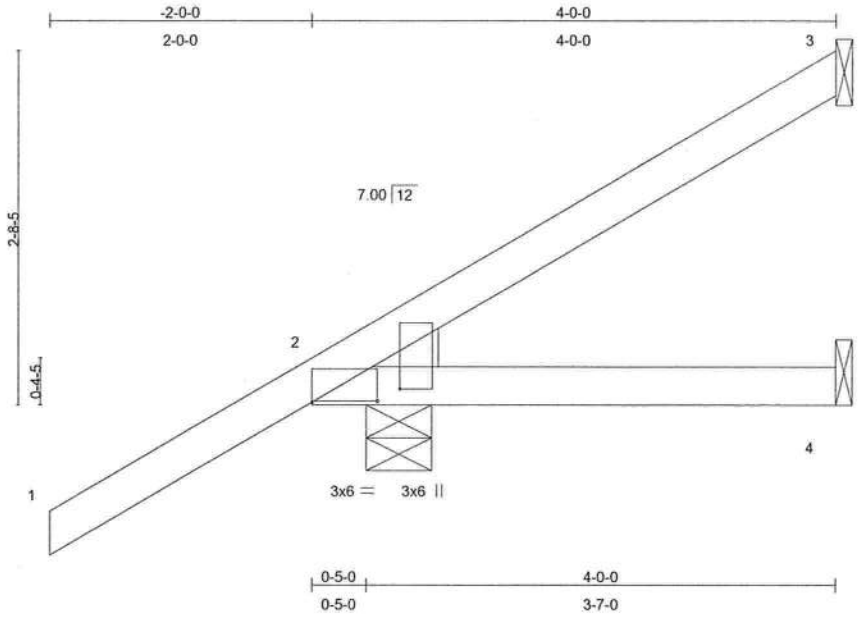

Plate Offsets (X,Y): [2:0-6-0,0-0-2], [2:0-1-4,0-8-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.02	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 17 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=62/Mechanical, 2=277/0-6-0, 4=18/Mechanical
Max Horz 2=180(load case 6)
Max Uplift 3=-54(load case 6), 2=-200(load case 6)
Max Grav 3=62(load case 1), 2=277(load case 1), 4=55(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-73/23
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13 and 2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931612
L255815	EJ4C	MONO TRUSS	2	1	Job Reference (optional)

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 3 and 200 lb uplift at joint 2.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931613
L255815	EJ4D	JACK	1	1	Job Reference (optional)

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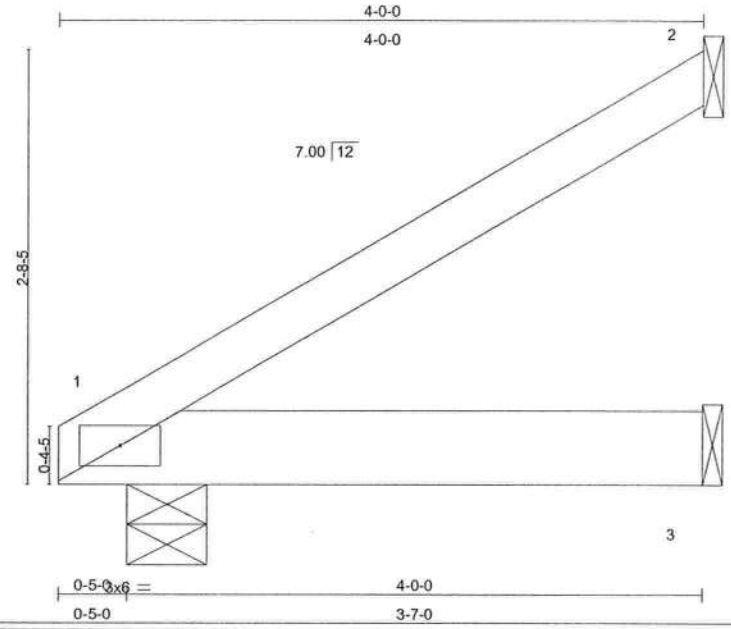

Scale = 1:13.8

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.01	1-3	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.01	1-3	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=281/0-6-0, 2=100/Mechanical, 3=182/Mechanical
Max Horz 1=104(load case 5)
Max Uplift 1=-111(load case 5), 2=-101(load case 5), 3=-73(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-67/42
BOT CHORD 1-3=0/0

JOINT STRESS INDEX
1 = 0.05

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 1, 101 lb uplift at joint 2 and 73 lb uplift at joint 3.
5) Girder carries tie-in span(s): 7-10-0 from 0-0-0 to 4-0-0
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931613
L255815	EJ4D	JACK	1	1	Job Reference (optional)

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 1-3=-99(F=-89)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931614
L255815	EJ5	MONO TRUSS	7	1	Job Reference (optional)

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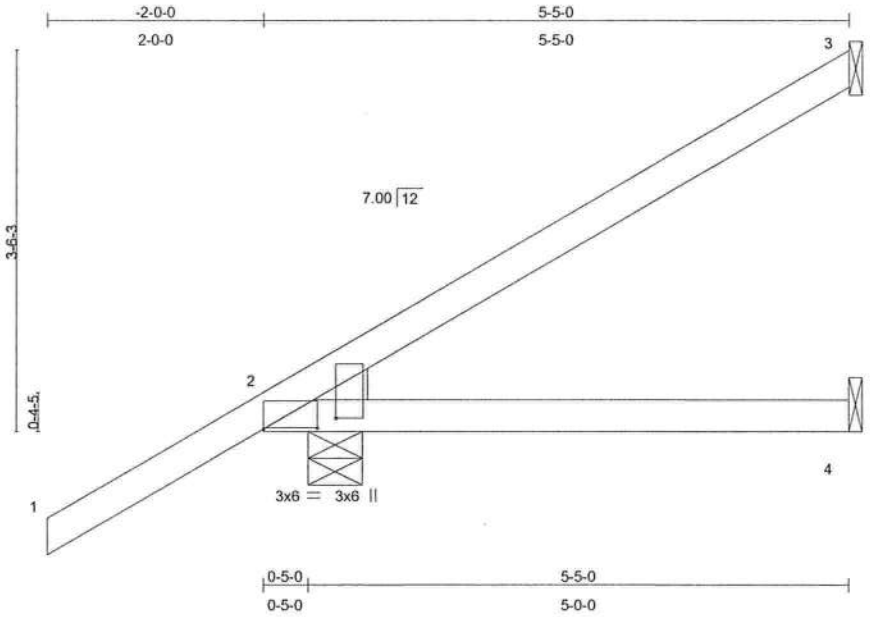

Plate Offsets (X,Y): [2:0-6-0,0-0-2], [2:0-1-4,0-8-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.04	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.06	2-4	>983	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=111/Mechanical, 2=312/0-6-0, 4=26/Mechanical
Max Horz 2=219(load case 6)
Max Uplift 3=-106(load case 6), 2=-198(load case 6)
Max Grav 3=111(load case 1), 2=312(load case 1), 4=77(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-87/44
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14 and 2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931614
L255815	EJ5	MONO TRUSS	7	1	Job Reference (optional)

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 106 lb uplift at joint 3 and 198 lb uplift at joint 2.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931615
L255815	EJ5A	MONO HIP	2	1	Job Reference (optional)

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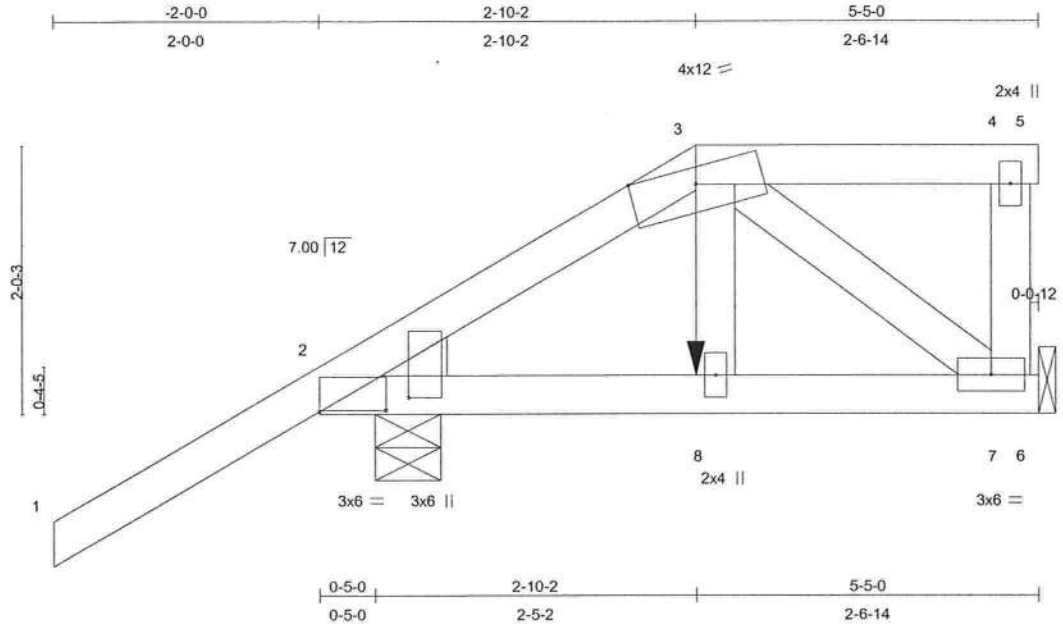

Plate Offsets (X,Y): [2:0-6-0,0-0-2], [2:0-1-4,0-8-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.00	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.00	2-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.03	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 7=180/Mechanical, 2=332/0-6-0
Max Horz 2=152(load case 5)
Max Uplift 7=-110(load case 4), 2=-243(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-205/68, 3-4=-0/0, 4-5=0/0, 4-7=-78/71
BOT CHORD 2-8=-68/114, 7-8=-70/120, 6-7=0/0
WEBS 3-7=-148/86, 3-8=-16/108

JOINT STRESS INDEX
2 = 0.16, 2 = 0.08, 3 = 0.08, 4 = 0.04, 7 = 0.06 and 8 = 0.08

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 7 and 243 lb uplift at joint 2.

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Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931615
L255815	EJ5A	MONO HIP	2	1	Job Reference (optional)

NOTES

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-4=-64(F=-10), 4-5=-14, 2-8=-10, 6-8=-12(F=-2)
   Concentrated Loads (lb)
   Vert: 8=-41(F)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931616
L255815	HJ2	JACK	2	1	Job Reference (optional)

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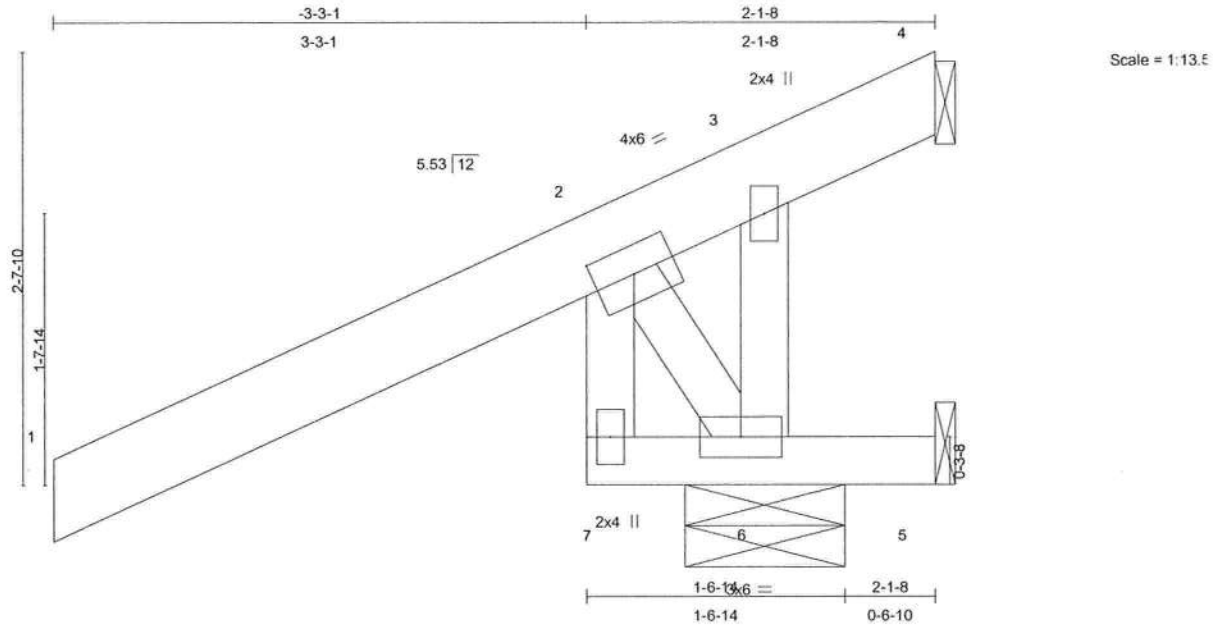

Plate Offsets (X,Y): [2:0-2-15,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	0.00	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	0.00	5-6	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.07	Horz(TL)	-0.03	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 23 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-1-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=-354/Mechanical, 5=-129/Mechanical, 6=712/0-11-11
Max Horz 6=111(load case 5)
Max Uplift 4=-354(load case 1), 5=-129(load case 1), 6=-634(load case 5)
Max Grav 4=313(load case 5), 5=118(load case 5), 6=712(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-7=-136/121, 1-2=0/77, 2-3=0/28, 3-4=-160/131
BOT CHORD 6-7=0/0, 5-6=0/0
WEBS 3-6=-440/242, 2-6=0/192

JOINT STRESS INDEX
2 = 0.09, 3 = 0.18, 6 = 0.14 and 7 = 0.07

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left exposed ; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 354 lb uplift at joint 4, 129 lb uplift at joint 5 and 634 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54
   Trapezoidal Loads (plf)
   Vert: 2=-3(F=26, B=26)-to-4=-38(F=8, B=8), 7=0(F=5, B=5)-to-5=-7(F=1, B=1)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931617
L255815	HJ2A	JACK	2	1	Job Reference (optional)

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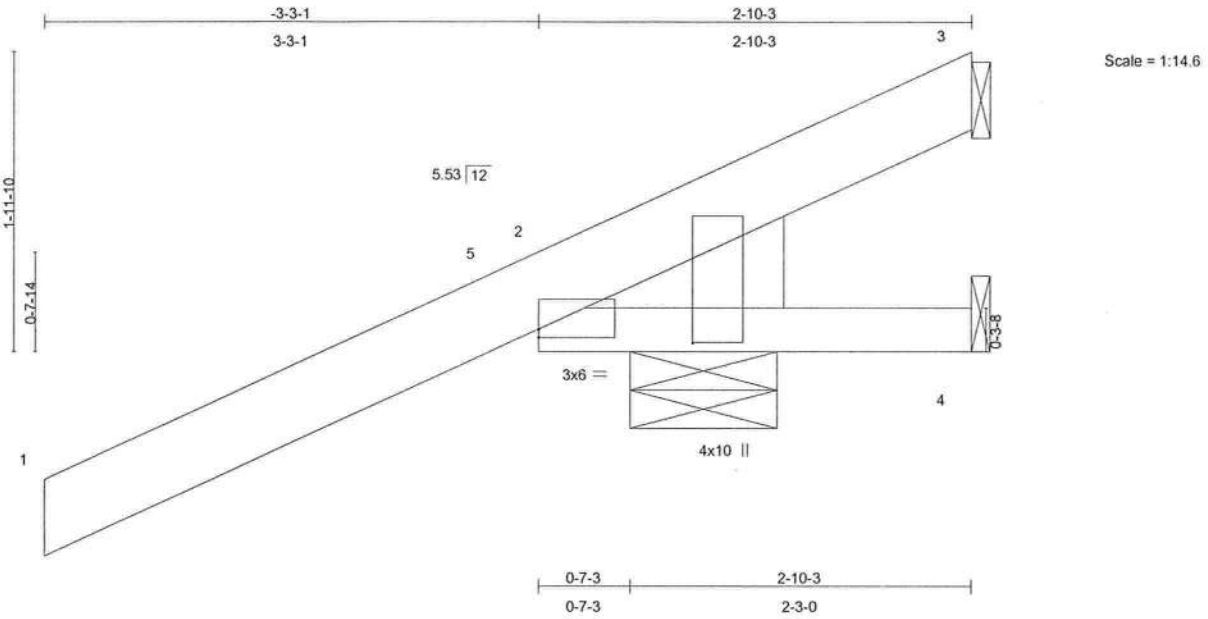

Plate Offsets (X,Y): [2:0-0-0,0-0-11], [2:0-1-1,1-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.00	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 24 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 8 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=-107/Mechanical, 2=340/0-11-11, 4=6/Mechanical
Max Horz 2=93(load case 5)
Max Uplift 3=-107(load case 1), 2=-347(load case 5)
Max Grav 3=140(load case 5), 2=340(load case 1), 4=31(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-5=0/57, 2-5=0/58, 2-3=-85/74
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.23 and 2 = 0.08

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 107 lb uplift at joint 3 and 347 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54
   Trapezoidal Loads (plf)
   Vert: 5=0(F=27, B=27)-to-3=-38(F=8, B=8), 2=-1(F=5, B=5)-to-4=-7(F=1, B=1)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931619
L255815	HJ3	JACK	2	1	Job Reference (optional)

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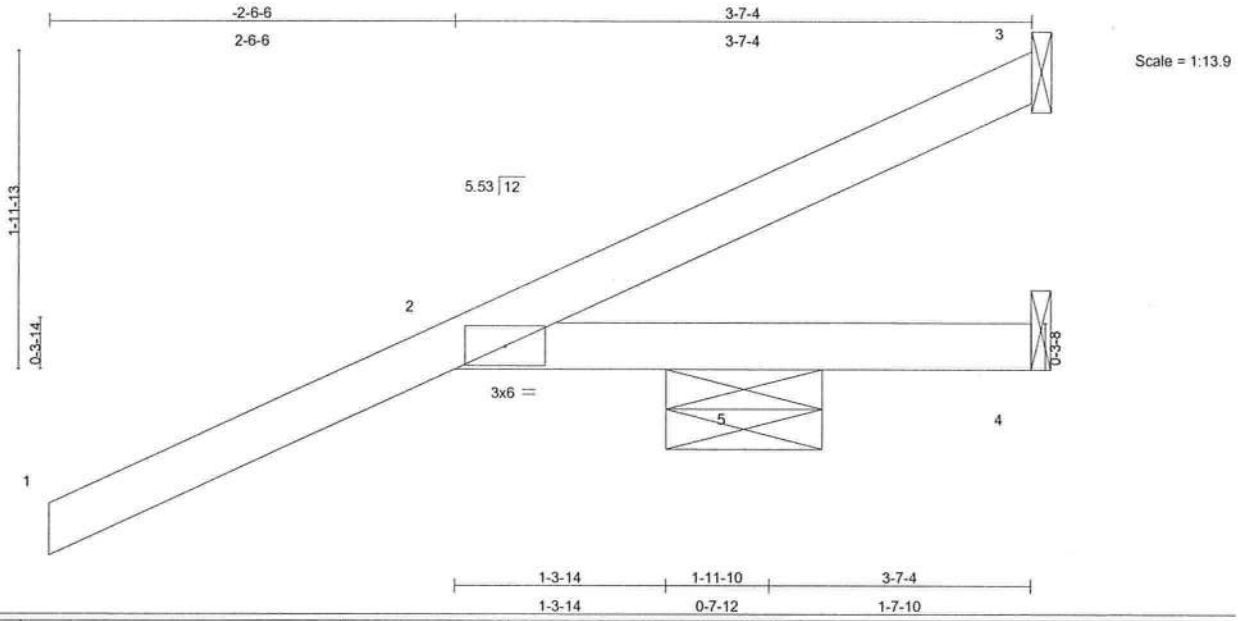

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.02	4-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	0.02	4-5	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.09	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=9/Mechanical, 4=-219/Mechanical, 5=449/0-11-11
Max Horz 5=100(load case 5)
Max Uplift 4=-219(load case 1), 5=-479(load case 5)
Max Grav 3=45(load case 3), 4=240(load case 5), 5=449(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/56, 2-3=-33/18
BOT CHORD 2-5=-2/100, 4-5=0/0

JOINT STRESS INDEX
2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 lb uplift at joint 4 and 479 lb uplift at joint 5.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931619
L255815	HJ3	JACK	2	1	Job Reference (optional)

NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=0(F=27, B=27)-to-3=-49(F=3, B=3), 2=0(F=5, B=5)-to-4=-9(F=0, B=0)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931620
L255815	HJ4	JACK	1	1	Job Reference (optional)

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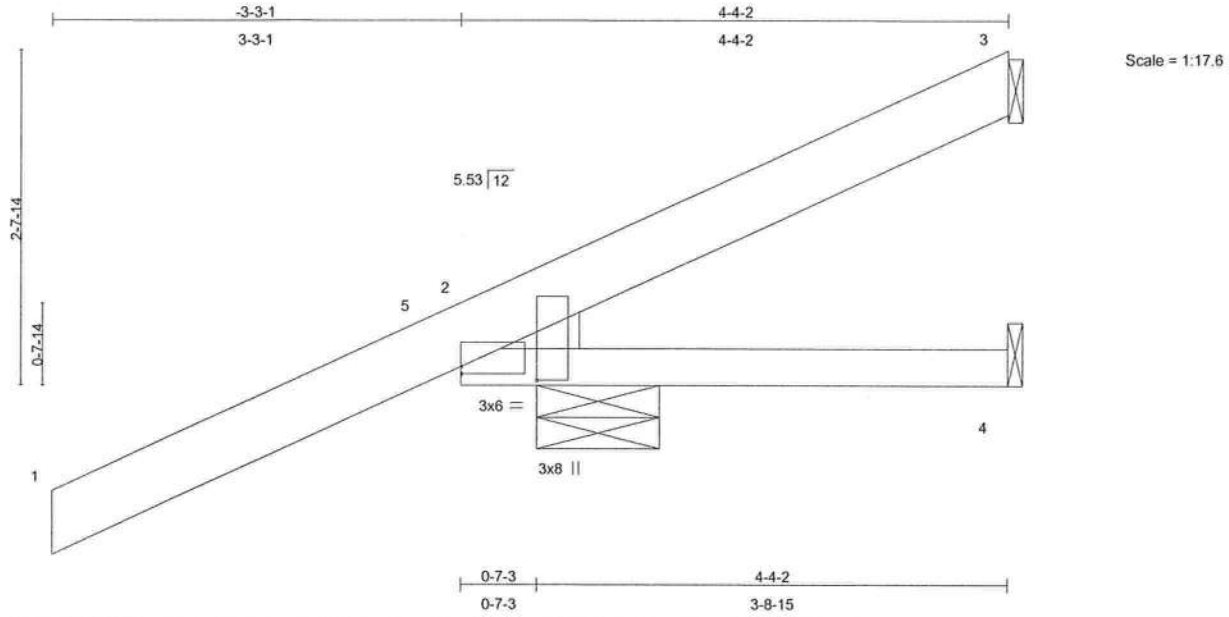

Plate Offsets (X,Y): [2:0-0-0,0-0-11], [2:0-1-5,0-7-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 27 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=-7/Mechanical, 2=317/0-11-11, 4=15/Mechanical
Max Horz 2=125(load case 5)
Max Uplift 3=-7(load case 1), 2=-284(load case 5)
Max Grav 3=50(load case 3), 2=317(load case 1), 4=54(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-5=0/57, 2-5=0/58, 2-3=-65/23
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.21 and 2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931620
L255815	HJ4	JACK	1	1	Job Reference (optional)

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 3 and 284 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54
   Trapezoidal Loads (plf)
   Vert: 5=0(F=27, B=27)-to-3=-59(F=-2, B=-2), 2=-1(F=5, B=5)-to-4=-11(F=-0, B=-0)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931621
L255815	PB1	PIGGYBACK	19	1	Job Reference (optional)

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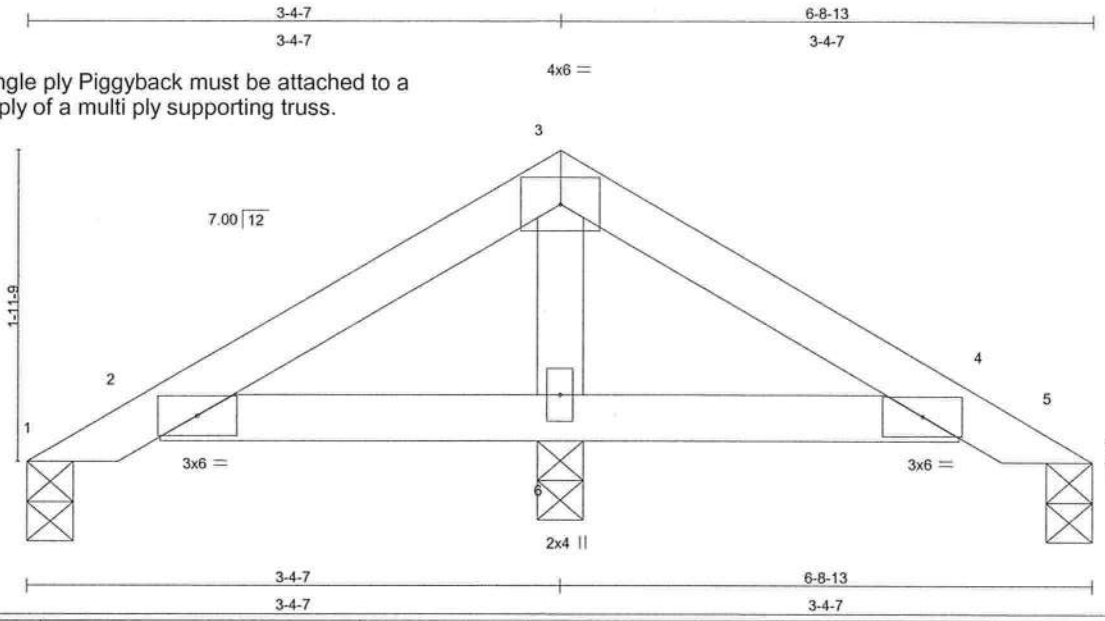

Note: A single ply Piggyback must be attached to a single ply of a multi ply supporting truss.

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.08	Vert(LL)	-0.00	4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 21 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=41/0-3-8, 5=41/0-3-8, 6=332/0-3-8
Max Horz 1=-52(load case 4)
Max Uplift 1=-10(load case 6), 5=-24(load case 4), 6=-68(load case 6)
Max Grav 1=60(load case 10), 5=60(load case 11), 6=332(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-44/49, 2-3=-57/148, 3-4=-57/148, 4-5=-26/16
BOT CHORD 2-6=-91/98, 4-6=-91/98
WEBS 3-6=-271/174

JOINT STRESS INDEX
2 = 0.21, 3 = 0.19, 4 = 0.21 and 6 = 0.10

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931621
L255815	PB1	PIGGYBACK	19	1	Job Reference (optional)

NOTES

5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 1, 24 lb uplift at joint 5 and 68 lb uplift at joint 6.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931622
L255815	PB1A	HIP PIGGYBACK	1	1	Job Reference (optional)

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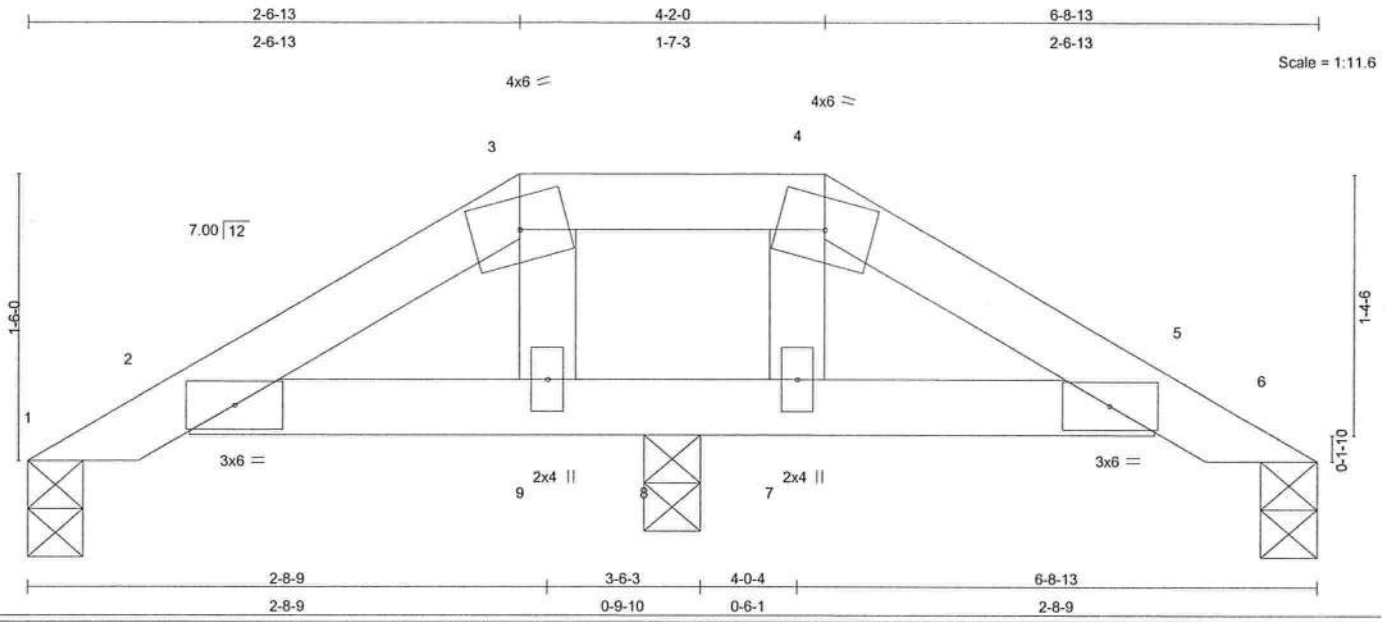

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.06	Vert(LL)	-0.00	2-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.01	2-9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 21 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=59/0-3-8, 6=59/0-3-8, 8=297/0-3-8
Max Horz 1=40(load case 5)
Max Uplift 1=-15(load case 6), 6=-25(load case 4), 8=-49(load case 6)
Max Grav 1=73(load case 10), 6=72(load case 11), 8=297(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-31/36, 2-3=-21/85, 3-4=0/62, 4-5=-21/85, 5-6=-31/22
BOT CHORD 2-9=-43/55, 8-9=-62/70, 7-8=-62/70, 5-7=-43/55
WEBS 3-9=-136/107, 4-7=-136/107

JOINT STRESS INDEX
2 = 0.12, 3 = 0.13, 4 = 0.13, 5 = 0.12, 7 = 0.06 and 9 = 0.06

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931622
L255815	PB1A	HIP PIGGYBACK	1	1	Job Reference (optional)

NOTES

6) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1, 25 lb uplift at joint 6 and 49 lb uplift at joint 8.
8) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931623
L255815	PB2A	MONO HIP PIGGYBACK	1	1	Job Reference (optional)

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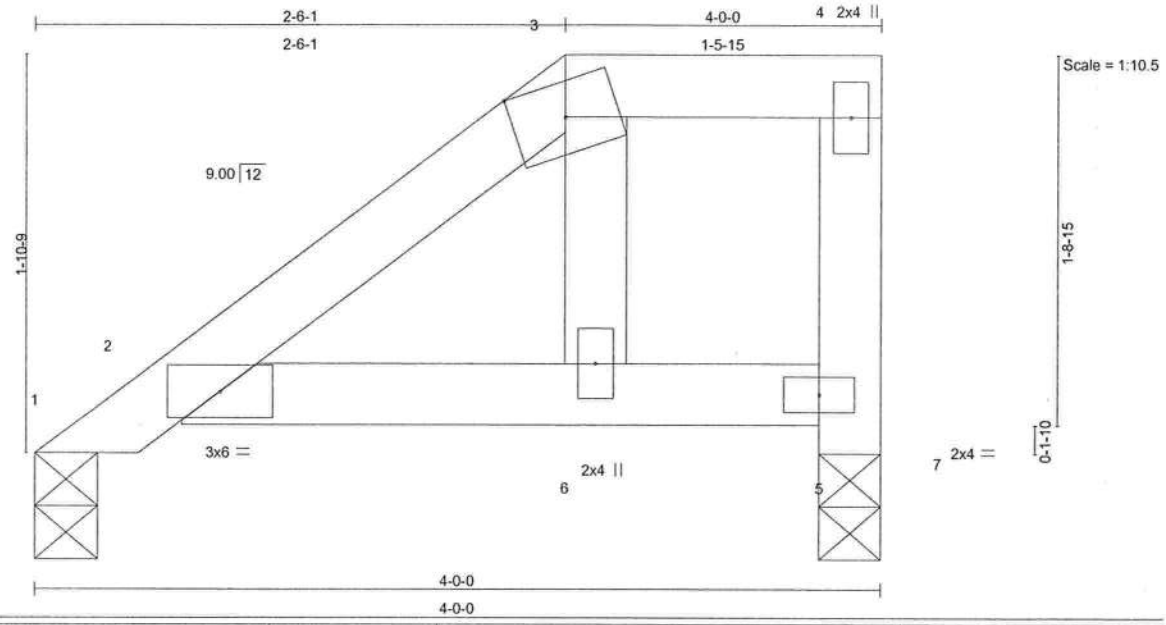

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	-0.01	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.01	Horz(TL)	-0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=120/0-3-8, 7=119/0-3-8
Max Horz 1=58(load case 6)
Max Uplift 1=-15(load case 6), 7=-36(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-63/0, 2-3=-90/24, 3-4=-54/52, 5-7=-119/115, 4-5=-64/68
BOT CHORD 2-6=-58/56, 5-6=-53/54
WEBS 3-6=-23/50

JOINT STRESS INDEX
2 = 0.24, 3 = 0.02, 4 = 0.37, 5 = 0.49 and 6 = 0.03

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931623
L255815	PB2A	MONO HIP PIGGYBACK	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1 and 36 lb uplift at joint 7.

7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931624
L255815	T01	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Feb 04 08:57:13 2008 Page 1

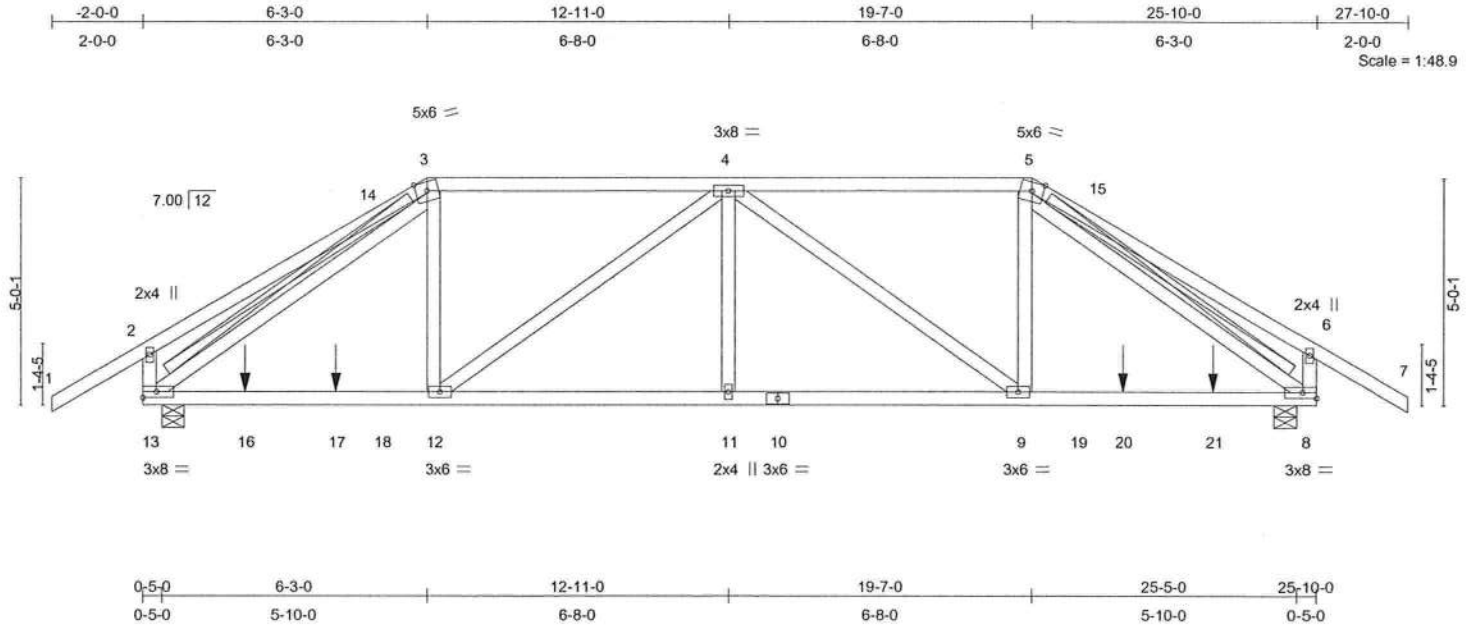

Plate Offsets (X,Y): [6:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	-0.08	11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.15	9-11	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.70	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 154 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-6-14 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-13, 5-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 13=1372/0-6-0, 8=1372/0-6-0
Max Horz 13=143(load case 4)
Max Uplift 13=-431(load case 4), 8=-431(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-14=-343/183, 3-14=-208/190, 3-4=-1318/545, 4-5=-1317/545, 5-15=-208/190, 6-15=-342/183, 6-7=0/58, 2-13=-410/248, 6-8=-409/248
BOT CHORD 13-16=-524/1302, 16-17=-524/1302, 17-18=-524/1302, 12-18=-524/1302, 11-12=-690/1775, 10-11=-690/1775, 9-10=-690/1775, 9-19=-419/1302, 19-20=-419/1302, 20-21=-419/1302, 8-21=-419/1302
WEBS 3-13=-1360/563, 3-12=-173/547, 4-12=-627/328, 4-11=0/213, 4-9=-627/328, 5-9=-173/547, 5-8=-1361/563

JOINT STRESS INDEX
2 = 0.74, 3 = 0.57, 4 = 0.57, 5 = 0.57, 6 = 0.75, 8 = 0.67, 9 = 0.36, 10 = 0.61, 11 = 0.34, 12 = 0.36 and 13 = 0.67

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931624
L255815	T01	HIP	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 431 lb uplift at joint 13 and 431 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 2-14=-54, 3-14=-83(F=-29), 3-5=-83(F=-29), 5-15=-83(F=-29), 6-15=-54, 6-7=-54, 13-18=-10, 18-19=-15(F=-5), 8-19=-10
   Concentrated Loads (lb)
   Vert: 16=-73(F) 17=-105(F) 20=-105(F) 21=-73(F)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931625
L255815	T02	HIP	1	1	Job Reference (optional)

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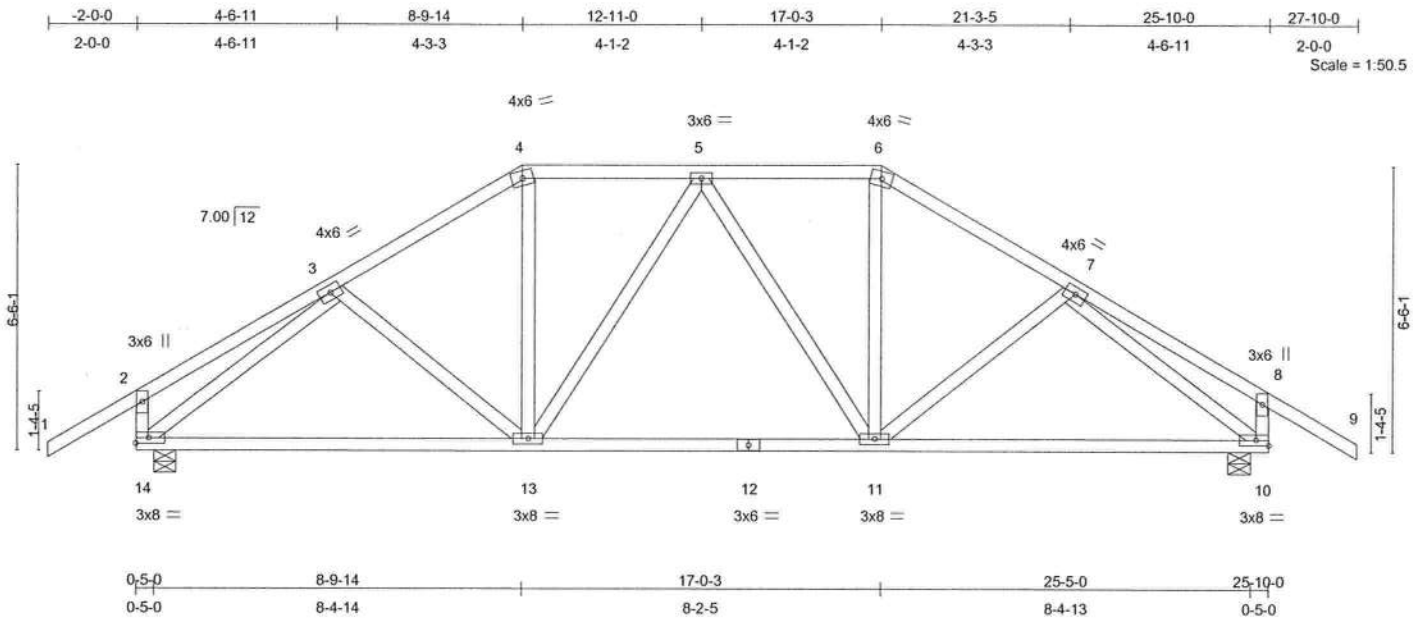

Plate Offsets (X,Y): [7:0-0-0,0-0-0], [8:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.09	13-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.17	13-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.03	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 162 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 14=933/0-6-0, 10=933/0-6-0
Max Horz 14=184(load case 5)
Max Uplift 14=-245(load case 6), 10=-244(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-231/83, 3-4=-934/511, 4-5=-762/492, 5-6=-762/492, 6-7=-934/511, 7-8=-231/83, 8-9=0/58, 2-14=-286/245, 8-10=-285/245
BOT CHORD 13-14=-262/777, 12-13=-199/826, 11-12=-199/826, 10-11=-225/776
WEBS 3-13=-82/106, 4-13=-86/249, 5-13=-199/155, 5-11=-200/154, 6-11=-86/249, 7-11=-82/106, 3-14=-931/454, 7-10=-931/454

JOINT STRESS INDEX
2 = 0.27, 3 = 0.28, 4 = 0.39, 5 = 0.42, 6 = 0.39, 7 = 0.28, 8 = 0.27, 10 = 0.59, 11 = 0.57, 12 = 0.31, 13 = 0.57 and 14 = 0.59

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931625
L255815	T02	HIP	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 245 lb uplift at joint 14 and 244 lb uplift at joint 10.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931626
L255815	T03	HIP	1	1	Job Reference (optional)

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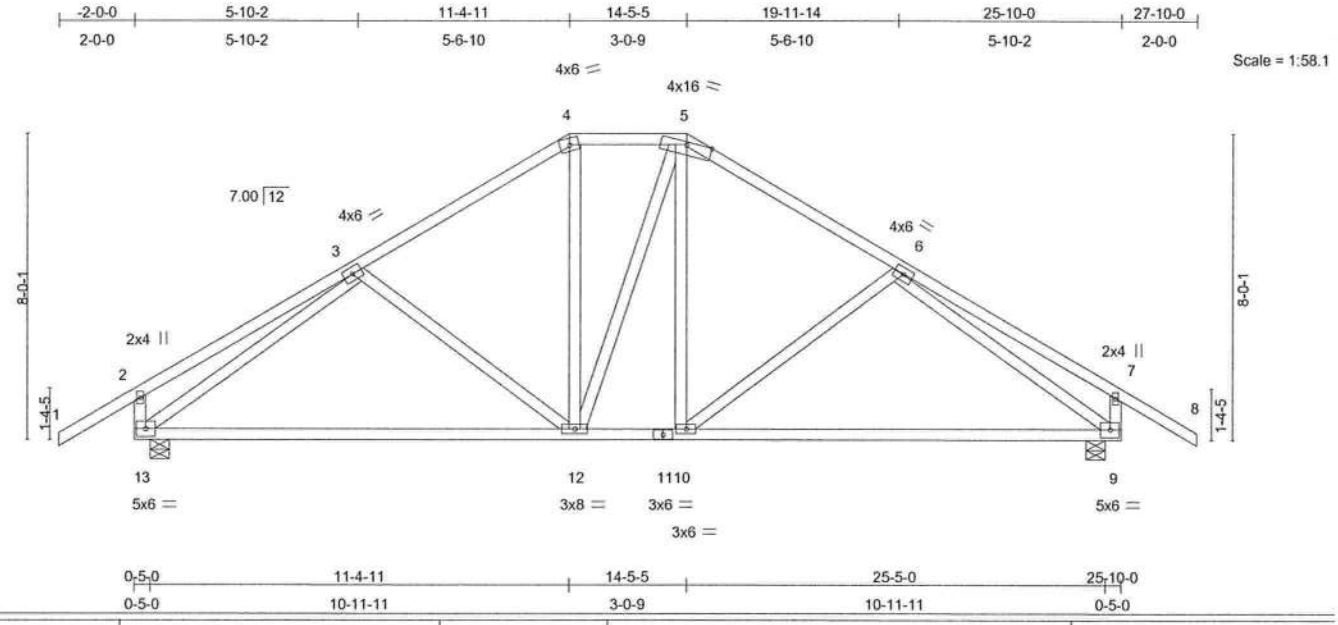

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.27	9-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.48	9-10	>637	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.03	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3 *Except* 2-13 2 X 4 SYP No.2, 7-9 2 X 4 SYP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 9=933/0-6-0, 13=933/0-6-0
Max Horz 13=226(load case 5)
Max Uplift 9=-258(load case 7), 13=-258(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/58, 2-3=-395/102, 3-4=-859/478, 4-5=-675/473, 5-6=-859/478, 6-7=-395/102, 7-8=0/58, 2-13=-355/268, 7-9=-355/268
BOT CHORD	12-13=-223/802, 11-12=-60/675, 10-11=-60/675, 9-10=-223/802
WEBS	3-12=-183/206, 4-12=-82/222, 5-12=-133/134, 5-10=-82/223, 6-10=-183/206, 3-13=-836/436, 6-9=-835/436

JOINT STRESS INDEX

2 = 0.60, 3 = 0.28, 4 = 0.45, 5 = 0.94, 6 = 0.28, 7 = 0.60, 9 = 0.74, 10 = 0.34, 11 = 0.79, 12 = 0.64 and 13 = 0.74

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931626
L255815	T03	HIP	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 258 lb uplift at joint 9 and 258 lb uplift at joint 13.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931627
L255815	T04	COMMON	8	1	Job Reference (optional)

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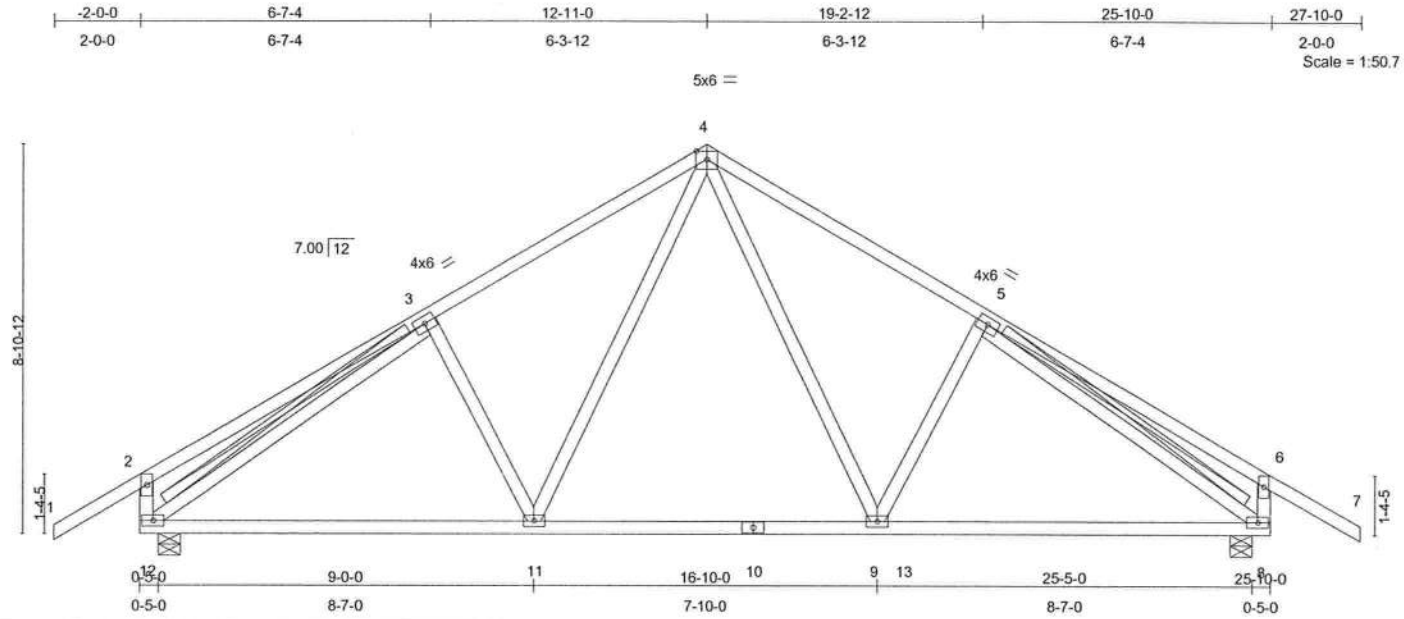

Plate Offsets (X,Y): [5:0-0-0,0-0-0], [6:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	0.23	9-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.42	9-11	>733	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 158 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-12, 5-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 12=1139/0-6-0, 8=1148/0-6-0
Max Horz 12=-250(load case 4)
Max Uplift 12=-321(load case 6), 8=-324(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-224/165, 3-4=-1288/715, 4-5=-1303/722, 5-6=-226/167, 6-7=0/58, 2-12=-355/316, 6-8=-357/317
BOT CHORD 11-12=-332/1100, 10-11=-130/849, 9-10=-130/849, 9-13=-337/1112, 8-13=-337/1112
WEBS 3-11=-173/208, 4-11=-258/498, 4-9=-273/529, 5-9=-171/207, 3-12=-1256/515, 5-8=-1268/520

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JOINT STRESS INDEX
2 = 0.29, 3 = 0.36, 4 = 0.49, 5 = 0.36, 6 = 0.29, 8 = 0.71, 9 = 0.46, 10 = 0.78, 11 = 0.46 and 12 = 0.70

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931627
L255815	T04	COMMON	8	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 321 lb uplift at joint 12 and 324 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 2-4=-54, 4-6=-54, 6-7=-54, 11-12=-10, 11-13=-60(F=-50), 8-13=-10

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931628
L255815	T05	COMMON	6	1	Job Reference (optional)

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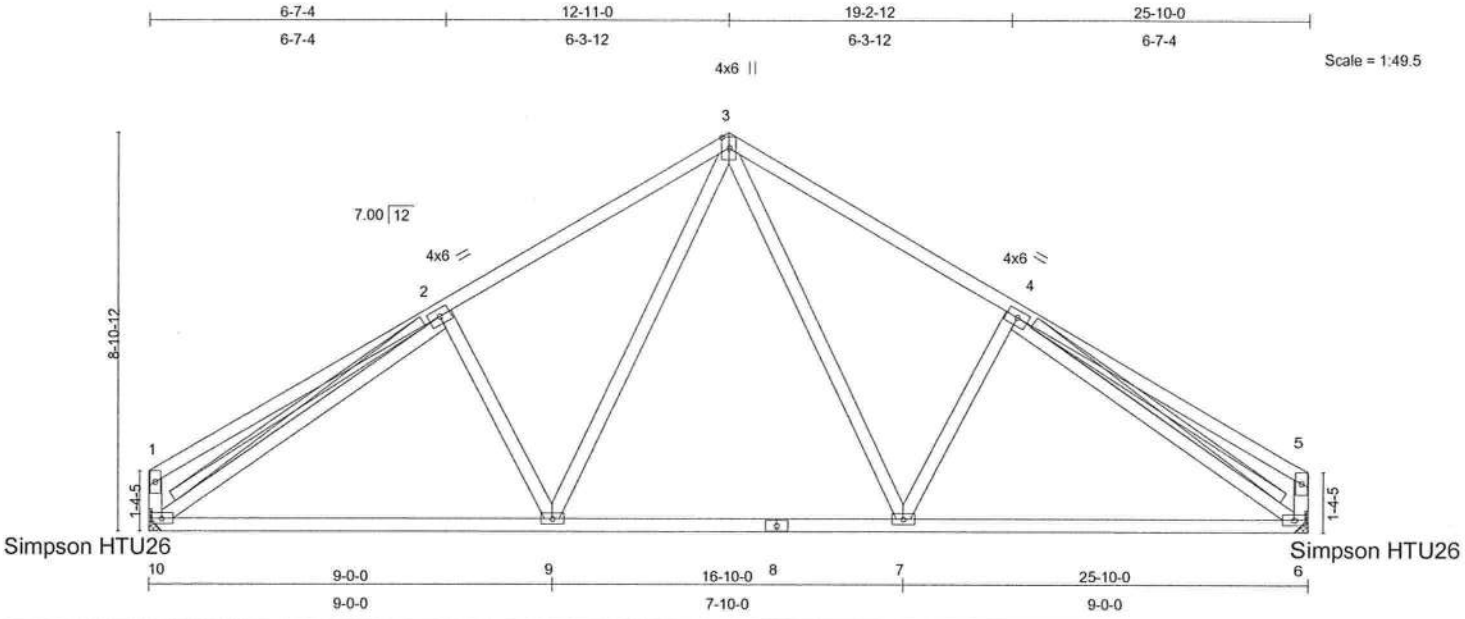

Plate Offsets (X,Y): [4:0-0-0,0-0-0], [5:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	0.23	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.42	7-9	>731	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.44	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 151 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-0-2 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 2-10, 4-6
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 10=1013/Mechanical, 6=1013/Mechanical
Max Horz 10=-207(load case 4)
Max Uplift 10=-226(load case 6), 6=-226(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-308/207, 2-3=-1296/722, 3-4=-1296/722, 4-5=-308/207, 1-10=-286/207, 5-6=-286/208
BOT CHORD 9-10=-468/1114, 8-9=-257/848, 7-8=-257/848, 6-7=-468/1115
WEBS 2-9=-191/220, 3-9=-269/516, 3-7=-269/516, 4-7=-191/220, 2-10=-1161/478, 4-6=-1161/478

JOINT STRESS INDEX
1 = 0.56, 2 = 0.33, 3 = 0.70, 4 = 0.33, 5 = 0.56, 6 = 0.69, 7 = 0.46, 8 = 0.78, 9 = 0.46 and 10 = 0.69

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931628
L255815	T05	COMMON	6	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 226 lb uplift at joint 10 and 226 lb uplift at joint 6.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-5=-54, 9-10=-10, 7-9=-60(F=-50), 6-7=-10

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Florida PE No. 34869
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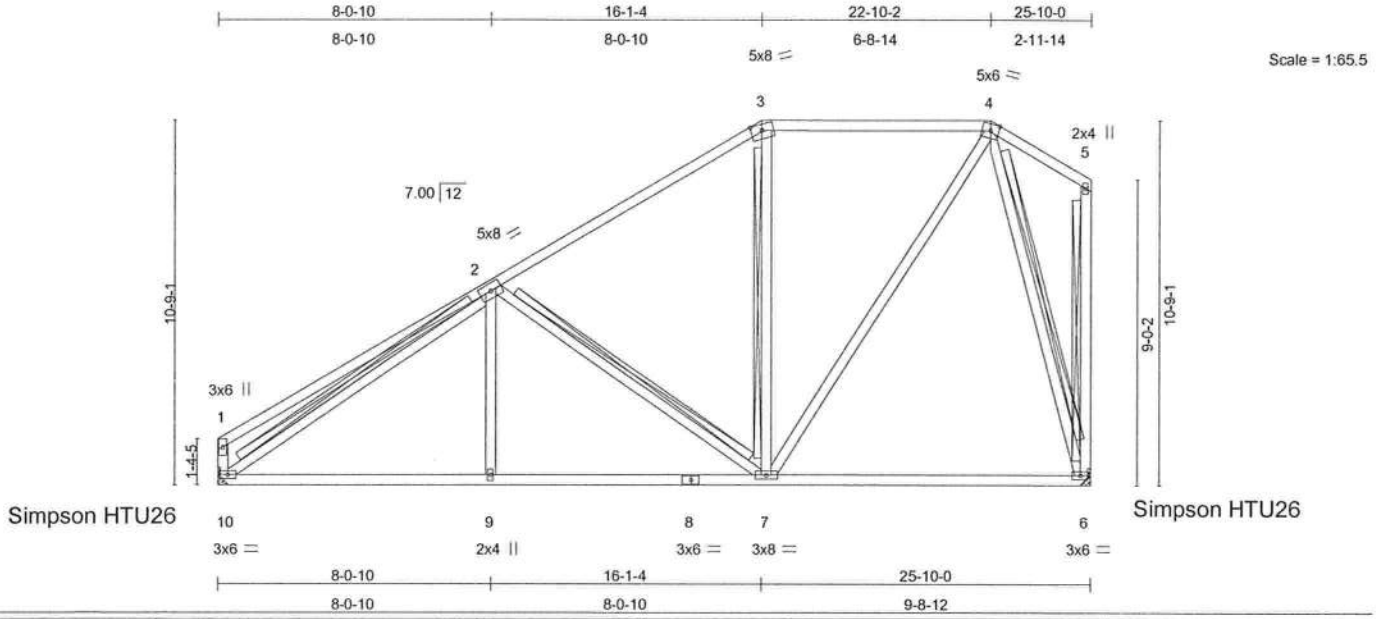
February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931629
L255815	T06	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:53:56 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.16	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.29	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 181 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
4-7 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-3-15 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 2-7, 3-7, 5-6, 4-6, 2-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 6=817/Mechanical, 10=817/Mechanical
Max Horz 10=265(load case 6)
Max Uplift 6=-194(load case 5), 10=-150(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-449/329, 2-3=-687/323, 3-4=-496/361, 4-5=-37/60, 1-10=-382/289, 5-6=-45/38
BOT CHORD 9-10=-569/834, 8-9=-569/834, 7-8=-569/834, 6-7=-118/206
WEBS 2-9=0/230, 2-7=-412/340, 3-7=-135/133, 4-7=-320/554, 4-6=-756/459, 2-10=-664/77

JOINT STRESS INDEX

1 = 0.76, 2 = 0.51, 3 = 0.76, 4 = 0.58, 5 = 0.47, 6 = 0.56, 7 = 0.57, 8 = 0.27, 9 = 0.33 and 10 = 0.72

NOTES

1) Unbalanced roof live loads have been considered for this design.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931629
L255815	T06	HIP	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 194 lb uplift at joint 6 and 150 lb uplift at joint 10.

LOAD CASE(S) Standard

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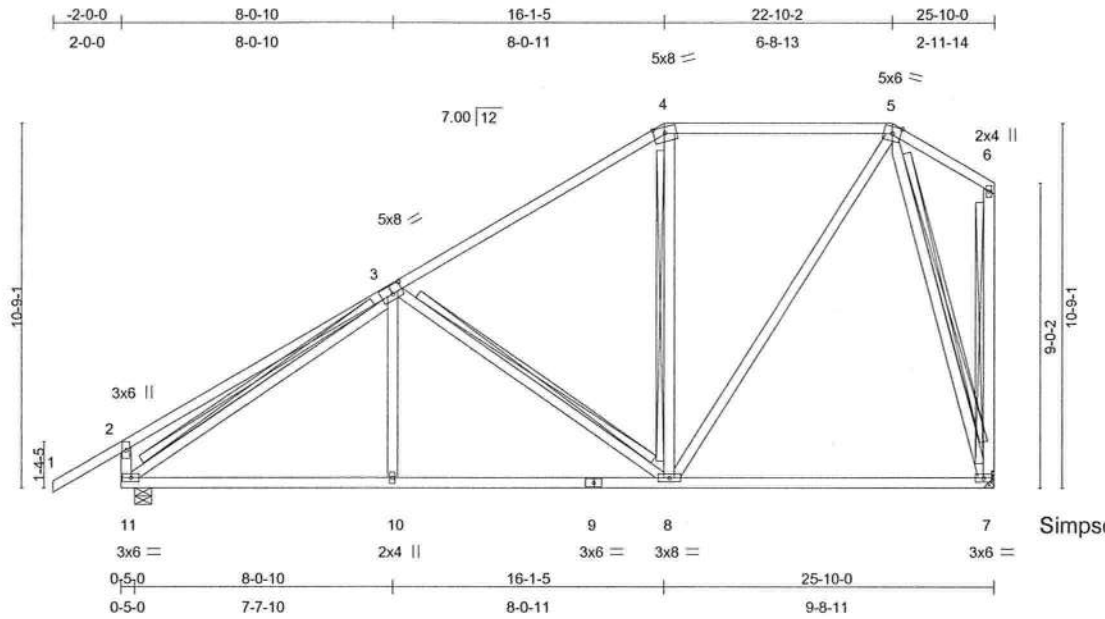
Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931630
L255815	T07	HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:53:58 2008 Page 1



Simpson HTU26

Plate Offsets (X,Y): [3:0-3-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.16	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.29	7-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 184 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
5-8 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 8-4-8 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-8, 4-8, 6-7, 5-7, 3-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 7=812/Mechanical, 11=938/0-6-0
Max Horz 11=355(load case 6)
Max Uplift 7=-194(load case 5), 11=-242(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-335/272, 3-4=-683/319, 4-5=-492/360, 5-6=-37/60, 2-11=-441/386, 6-7=-45/40
BOT CHORD 10-11=-563/820, 9-10=-562/821, 8-9=-562/821, 7-8=-117/204
WEBS 3-10=0/230, 3-8=-398/332, 4-8=-142/139, 5-8=-319/550, 5-7=-750/455, 3-11=-769/131

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JOINT STRESS INDEX
2 = 0.47, 3 = 0.60, 4 = 0.78, 5 = 0.57, 6 = 0.47, 7 = 0.56, 8 = 0.57, 9 = 0.26, 10 = 0.33 and 11 = 0.72

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931630
L255815	T07	HIP	2	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 194 lb uplift at joint 7 and 242 lb uplift at joint 11.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931631
L255815	T08	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Feb 04 09:03:26 2008 Page 1

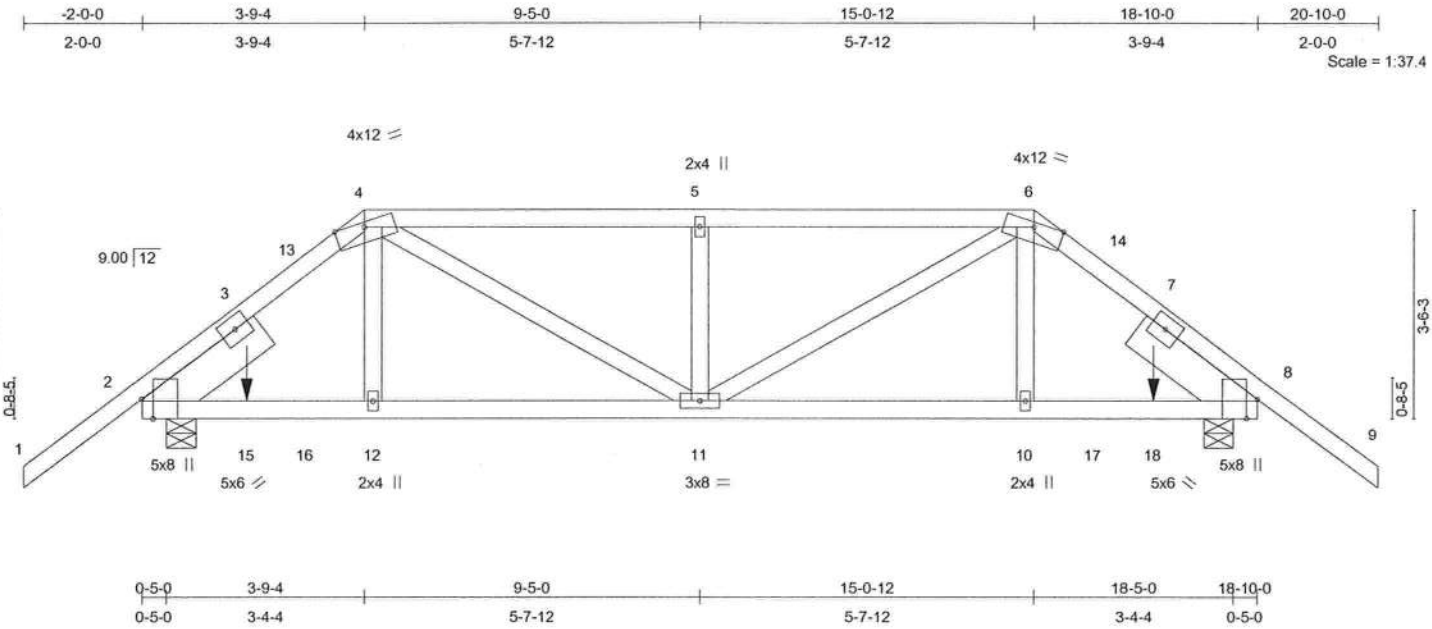

Plate Offsets (X,Y): [2:0-3-15,Edge], [8:0-3-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	0.06	11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.11	10-11	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.28	Horz(TL)	0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 112 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP 2400F 2.0E 2-4-8, Right 2 X 8 SYP No.1D 2-4-8

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-6-3 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 8-9-6 oc bracing.

REACTIONS (lb/size) 2=1223/0-6-0, 8=1223/0-6-0
Max Horz 2=87(load case 4)
Max Uplift 2=-477(load case 4), 8=-477(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-1435/610, 3-13=-1371/617, 4-13=-1305/626, 4-5=-1674/834, 5-6=-1674/834, 6-14=-1304/627, 7-14=-1371/618, 7-8=-1435/610, 8-9=0/53
BOT CHORD	2-15=-501/1066, 15-16=-501/1066, 12-16=-501/1066, 11-12=-500/1072, 10-11=-426/1072, 10-17=-428/1066, 17-18=-428/1066, 8-18=-428/1066
WEBS	4-12=0/216, 4-11=-406/720, 5-11=-596/427, 6-11=-407/720, 6-10=0/216

JOINT STRESS INDEX

2 = 0.71, 2 = 0.54, 3 = 0.00, 4 = 0.74, 5 = 0.34, 6 = 0.74, 7 = 0.00, 8 = 0.71, 8 = 0.54, 10 = 0.34, 11 = 0.68 and 12 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931631
L255815	T08	HIP	1	1	Job Reference (optional)

NOTES

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 477 lb uplift at joint 2 and 477 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-13=-54, 4-13=-96(F=-42), 4-6=-96(F=-42), 6-14=-96(F=-42), 9-14=-54, 2-16=-10, 16-17=-18(F=-8), 8-17=-10
   Concentrated Loads (lb)
   Vert: 15=-180(F) 18=-180(F)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931632
L255815	T09	HIP	1	1	Job Reference (optional)

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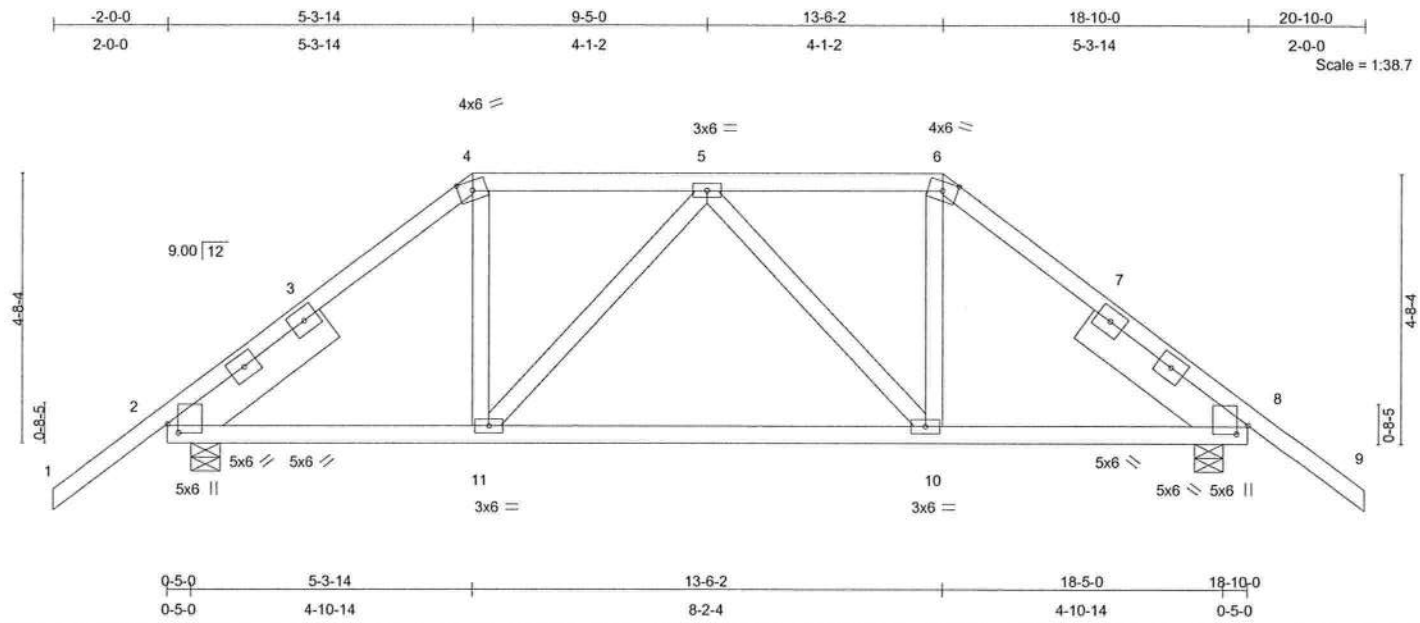

Plate Offsets (X,Y): [2:0-1-15,0-2-4], [8:0-1-15,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.07	10-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.14	10-11	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.12	Horz(TL)	0.02	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 117 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 3-4-3, Right 2 X 8 SYP No.1D 3-4-3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=711/0-6-0, 8=711/0-6-0
Max Horz 2=-120(load case 4)
Max Uplift 2=-193(load case 6), 8=-193(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-780/312, 3-4=-660/336, 4-5=-533/333, 5-6=-533/333, 6-7=-660/336, 7-8=-780/312, 8-9=0/53
BOT CHORD	2-11=-154/526, 10-11=-172/630, 8-10=-69/526
WEBS	4-11=-60/245, 5-11=-201/150, 5-10=-201/150, 6-10=-60/245

JOINT STRESS INDEX

2 = 0.59, 2 = 0.15, 2 = 0.15, 3 = 0.00, 4 = 0.36, 5 = 0.37, 6 = 0.36, 7 = 0.00, 8 = 0.59, 8 = 0.15, 8 = 0.15, 10 = 0.37 and 11 = 0.37

NOTES

1) Unbalanced roof live loads have been considered for this design.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931632
L255815	T09	HIP	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 2 and 193 lb uplift at joint 8.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931633
L255815	T10	HIP	1	1	Job Reference (optional)

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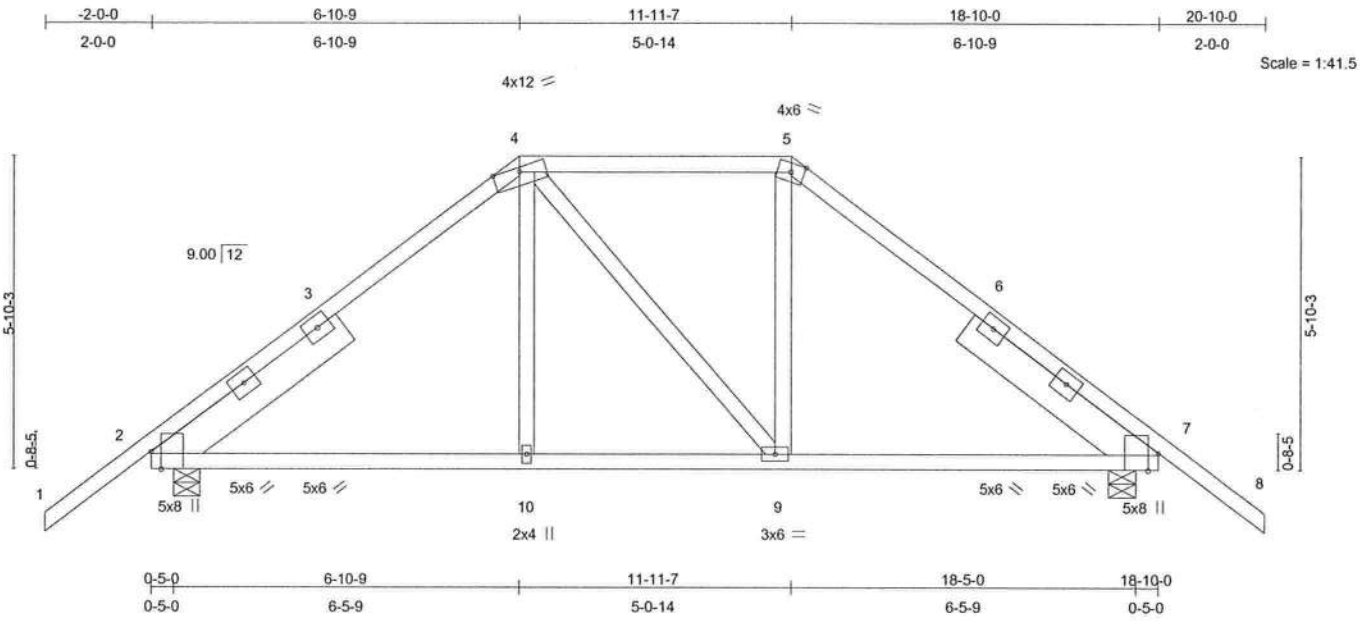

Plate Offsets (X,Y): [2:0-3-15,Edge], [7:0-3-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.04	2-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.08	2-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 122 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 4-3-14, Right 2 X 8 SYP No.1D 4-3-14

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=711/0-6-0, 7=711/0-6-0
Max Horz 2=-152(load case 4)
Max Uplift 2=-202(load case 6), 7=-202(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-743/304, 3-4=-611/321, 4-5=-489/345, 5-6=-611/321, 6-7=-743/304, 7-8=0/53
BOT CHORD	2-10=-126/487, 9-10=-126/489, 7-9=-42/487
WEBS	4-10=0/173, 4-9=-124/124, 5-9=-46/173

JOINT STRESS INDEX

2 = 0.55, 2 = 0.14, 2 = 0.14, 3 = 0.00, 4 = 0.96, 5 = 0.59, 6 = 0.00, 7 = 0.55, 7 = 0.14, 7 = 0.14, 9 = 0.38 and 10 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931633
L255815	T10	HIP	1	1	Job Reference (optional)

NOTES

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 202 lb uplift at joint 2 and 202 lb uplift at joint 7.

LOAD CASE(S) Standard

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931634
L255815	T11	HIP	1	1	Job Reference (optional)

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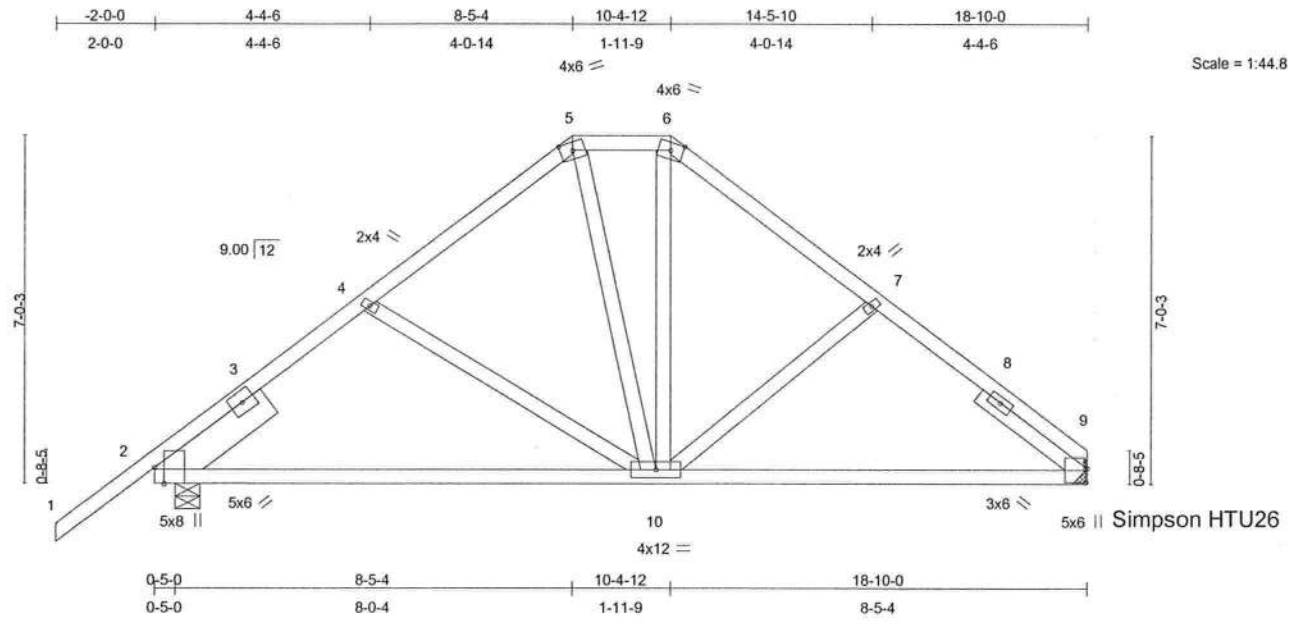

Plate Offsets (X,Y): [2:0-3-15,Edge], [9:0-3-7,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.13	2-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.23	2-10	>962	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.02	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 116 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 2-8-1, Right 2 X 4 SYP No.3 2-8-1

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 9=597/Mechanical, 2=716/0-6-0
Max Horz 2=208(load case 5)
Max Uplift 9=-118(load case 7), 2=-209(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-774/343, 3-4=-643/361, 4-5=-567/318, 5-6=-431/332, 6-7=-600/338, 7-8=-659/371, 8-9=-774/356
BOT CHORD	2-10=-171/539, 9-10=-199/560
WEBS	4-10=-170/172, 6-10=-96/195, 7-10=-180/209, 5-10=-67/182

JOINT STRESS INDEX

2 = 0.65, 2 = 0.29, 3 = 0.00, 4 = 0.33, 5 = 0.33, 6 = 0.28, 7 = 0.33, 8 = 0.00, 9 = 0.77, 9 = 0.28 and 10 = 0.47

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931634
L255815	T11	HIP	1	1	Job Reference (optional)

NOTES

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 9 and 209 lb uplift at joint 2.

LOAD CASE(S) Standard

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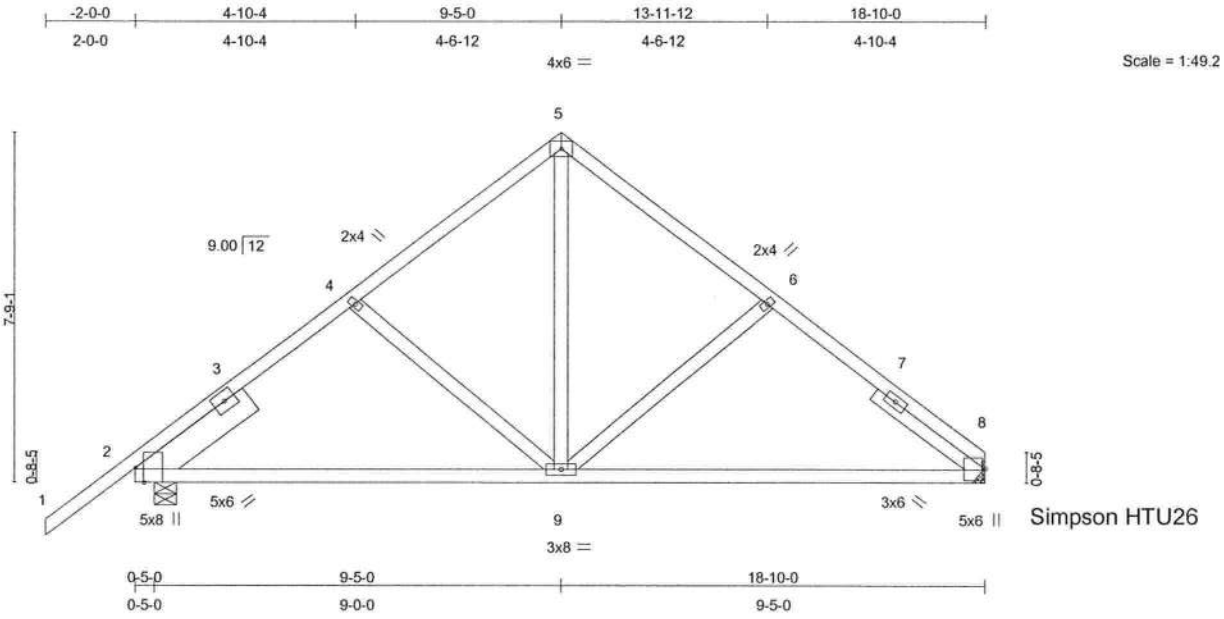
Boynton Beach, FL 33435

February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931635
L255815	T12	COMMON	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:54:02 2008 Page 1



Simpson HTU26

Plate Offsets (X,Y): [2:0-3-15,Edge], [8:0-3-3,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.10	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.19	8-9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.20	Horz(TL)	0.02	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 109 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 2-11-12, Right 2 X 4 SYP No.3 2-11-12

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=597/Mechanical, 2=716/0-6-0
Max Horz 2=227(load case 5)
Max Uplift 8=-121(load case 7), 2=-212(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-772/329, 3-4=-626/350, 4-5=-570/330, 5-6=-572/333, 6-7=-685/360, 7-8=-760/344
BOT CHORD	2-9=-158/532, 8-9=-182/548
WEBS	4-9=-193/191, 5-9=-210/383, 6-9=-213/222

JOINT STRESS INDEX

2 = 0.55, 2 = 0.29, 3 = 0.00, 4 = 0.33, 5 = 0.39, 6 = 0.33, 7 = 0.00, 8 = 0.83, 8 = 0.27 and 9 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931635
L255815	T12	COMMON	6	1	Job Reference (optional)

NOTES

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 121 lb uplift at joint 8 and 212 lb uplift at joint 2.

LOAD CASE(S) Standard

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931636
L255815	T13	MONO HIP	1	2	Job Reference (optional)

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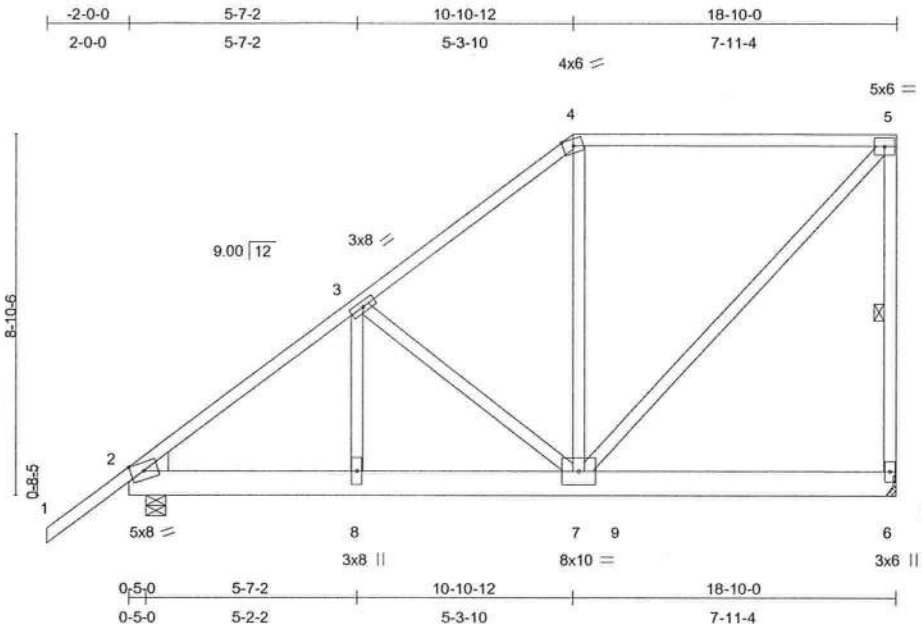

Simpson HGUS26-2

Plate Offsets (X,Y): [2:0-3-14,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.08	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.16	6-7	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.71	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 302 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-6

REACTIONS (lb/size) 6=4548/Mechanical, 2=5196/0-6-0
Max Horz 2=335(load case 5)
Max Uplift 6=-1623(load case 4), 2=-1819(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/61, 2-3=-5884/1944, 3-4=-3589/1213, 4-5=-2840/1030, 5-6=-3269/1185
BOT CHORD 2-8=-1689/4491, 7-8=-1689/4491, 7-9=-26/56, 6-9=-26/56
WEBS 3-8=-938/2666, 3-7=-2162/861, 4-7=-619/1733, 5-7=-1510/4141

JOINT STRESS INDEX
2 = 0.74, 2 = 0.00, 3 = 0.80, 4 = 0.52, 5 = 0.71, 6 = 0.43, 7 = 0.44 and 8 = 0.43

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

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February 4,2008

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931636
L255815	T13	MONO HIP	1	2	Job Reference (optional)

NOTES

3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1623 lb uplift at joint 6 and 1819 lb uplift at joint 2.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-5=-54, 2-9=-511(F=-501), 6-9=-391(F=-381)

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931637
L255815	T14	HIP	1	2	Job Reference (optional)

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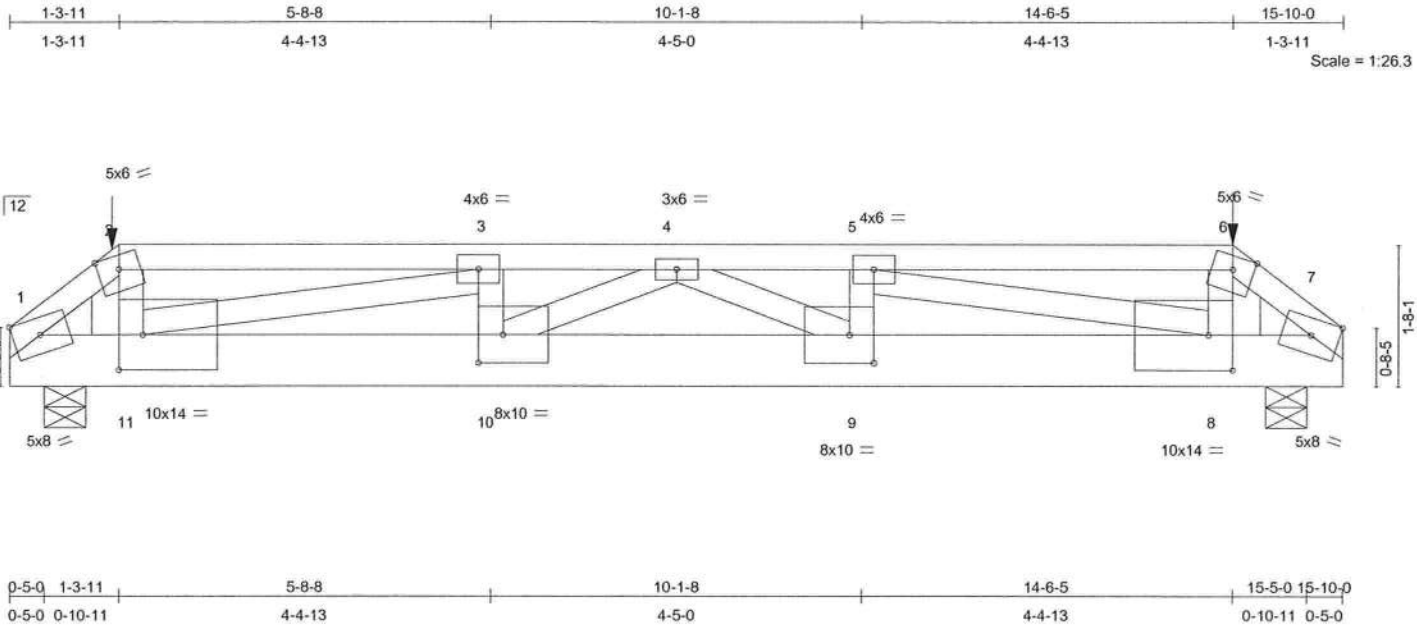

Plate Offsets (X,Y): [1:Edge,0-2-6], [7:0-3-14,Edge], [8:0-3-8,0-5-0], [9:0-3-8,0-4-0], [10:0-3-8,0-4-0], [11:0-3-8,0-5-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.21	9-10	>861	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.41	9-10	>453	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.72	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 200 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 6 SYP No.1D, Right: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4371/0-6-0, 7=4371/0-6-0
Max Horz 1=-33(load case 3)
Max Uplift 1=-1595(load case 4), 7=-1595(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6507/2372, 2-3=-5009/1850, 3-4=-11684/4295, 4-5=-11685/4296, 5-6=-5010/1851, 6-7=-6508/2373
BOT CHORD 1-11=-1660/4472, 10-11=-4295/11684, 9-10=-4405/11971, 8-9=-4287/11685, 7-8=-1639/4472
WEBS 2-11=-1595/4462, 6-8=-1595/4463, 3-10=-671/1937, 5-9=-670/1936, 3-11=-6949/2556, 4-9=-330/136, 5-8=-6949/2555, 4-10=-331/136

JOINT STRESS INDEX
1 = 0.83, 1 = 0.00, 2 = 0.98, 3 = 0.97, 4 = 0.05, 5 = 0.97, 6 = 0.98, 7 = 0.83, 7 = 0.00, 8 = 0.83, 9 = 0.20, 10 = 0.20 and 11 = 0.83

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

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February 4,2008

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES.
					J1931637
L255815	T14	HIP	1	2	Job Reference (optional)

NOTES

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1595 lb uplift at joint 1 and 1595 lb uplift at joint 7.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 2-6=-57(F=-3), 6-7=-54, 1-7=-512(F=-502)
   Concentrated Loads (lb)
   Vert: 2=-9(F) 6=-9(F)

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931638
L255815	T15	HIP	1	1	Job Reference (optional)

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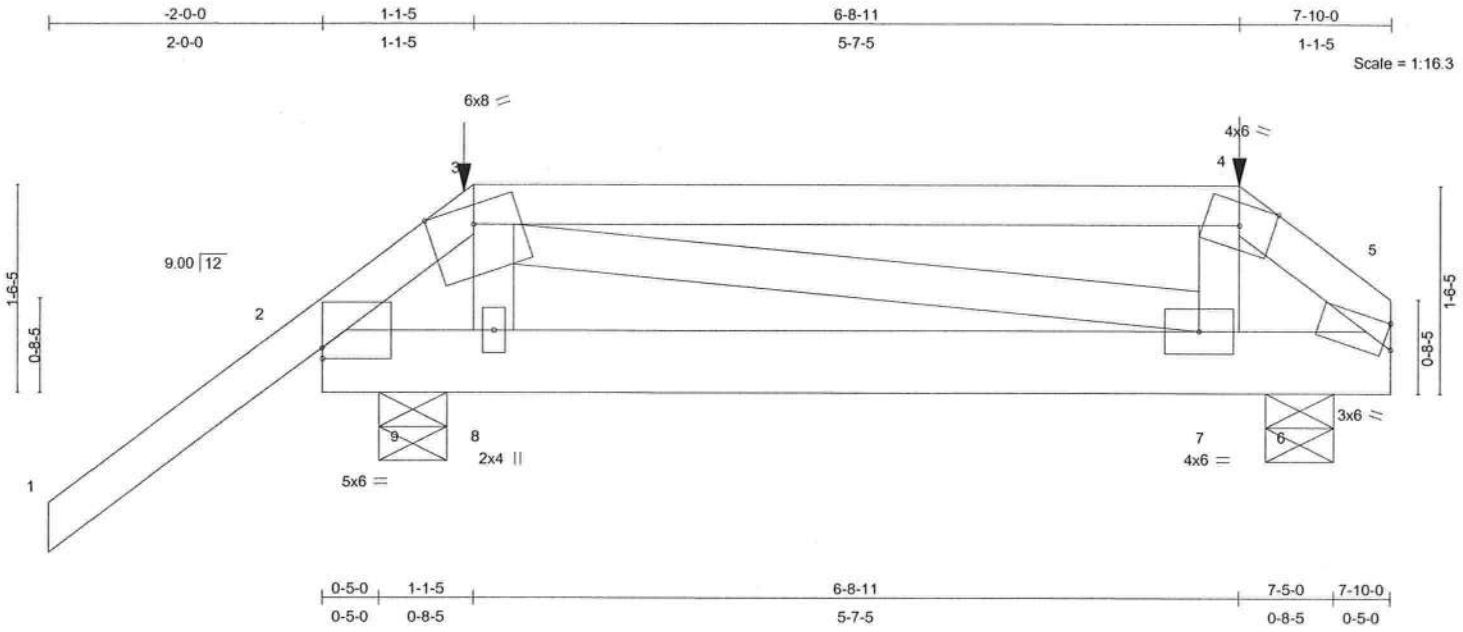

Plate Offsets (X,Y): [2:0-0-1,0-0-15], [5:Edge,0-2-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.00	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.01	7-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.05	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 9=391/0-6-0, 6=228/0-6-0
Max Horz 9=75(load case 5)
Max Uplift 9=-169(load case 5), 6=-86(load case 6)
Max Grav 9=391(load case 1), 6=249(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/56, 2-3=-148/99, 3-4=-167/57, 4-5=-185/43
BOT CHORD 2-9=-49/178, 8-9=-60/165, 7-8=-69/147, 6-7=-70/184, 5-6=-70/184
WEBS 4-7=-156/95, 3-8=-253/183, 3-7=-107/113

JOINT STRESS INDEX
2 = 0.61, 3 = 0.38, 4 = 0.33, 5 = 0.44, 7 = 0.05 and 8 = 0.10

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 169 lb uplift at joint 9 and 86 lb uplift at joint 6.

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February 4,2008

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NOTES

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-4=-54, 4-5=-54, 2-5=-10
   Concentrated Loads (lb)
   Vert: 3=-5(F) 4=-5(F)

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931639
L255815	T16	HIP	1	1	Job Reference (optional)

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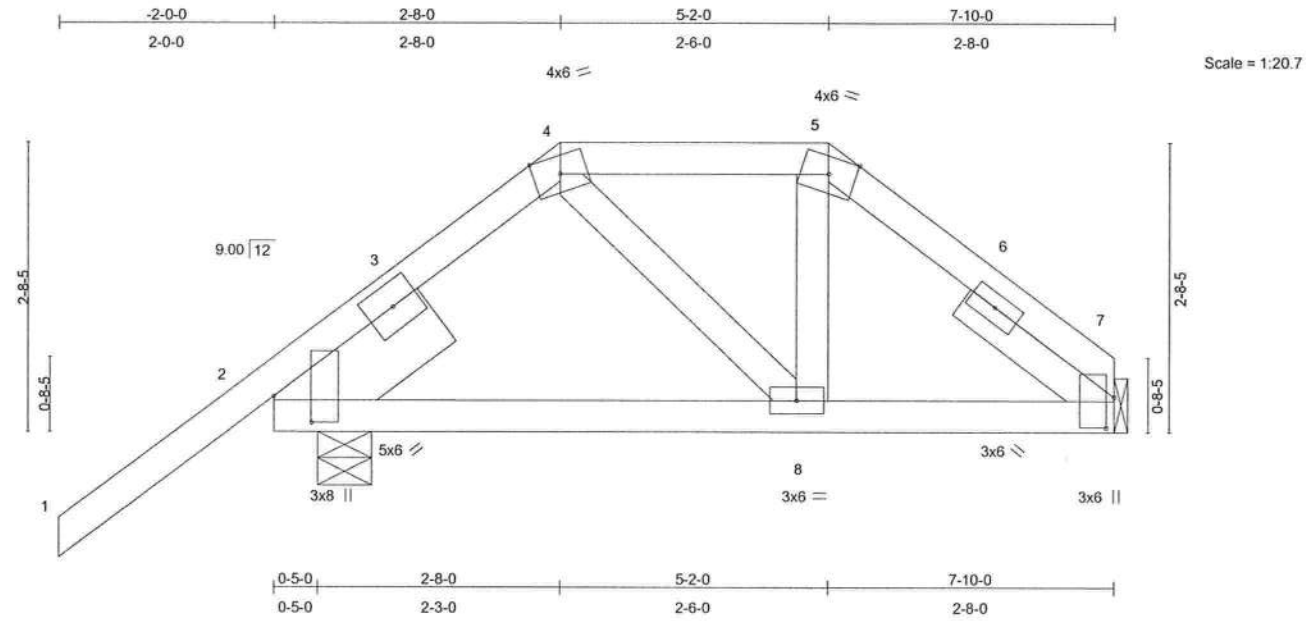

Plate Offsets (X,Y): [2:0-2-15,0-4-4], [7:0-3-7,0-0-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.01	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 1-8-4, Right 2 X 4 SYP No.3 1-8-4

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 7=237/Mechanical, 2=372/0-6-0
Max Horz 2=87(load case 5)
Max Uplift 7=-42(load case 7), 2=-144(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-277/154, 3-4=-225/160, 4-5=-184/164, 5-6=-205/154, 6-7=-280/143
BOT CHORD	2-8=-60/155, 7-8=-52/184
WEBS	4-8=-44/55, 5-8=-2/89

JOINT STRESS INDEX

2 = 0.67, 2 = 0.12, 3 = 0.00, 4 = 0.06, 5 = 0.18, 6 = 0.00, 7 = 0.22, 7 = 0.10 and 8 = 0.06

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931639
L255815	T16	HIP	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 7 and 144 lb uplift at joint 2.

LOAD CASE(S) Standard

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931640
L255815	T17	SPECIAL	1	1	Job Reference (optional)

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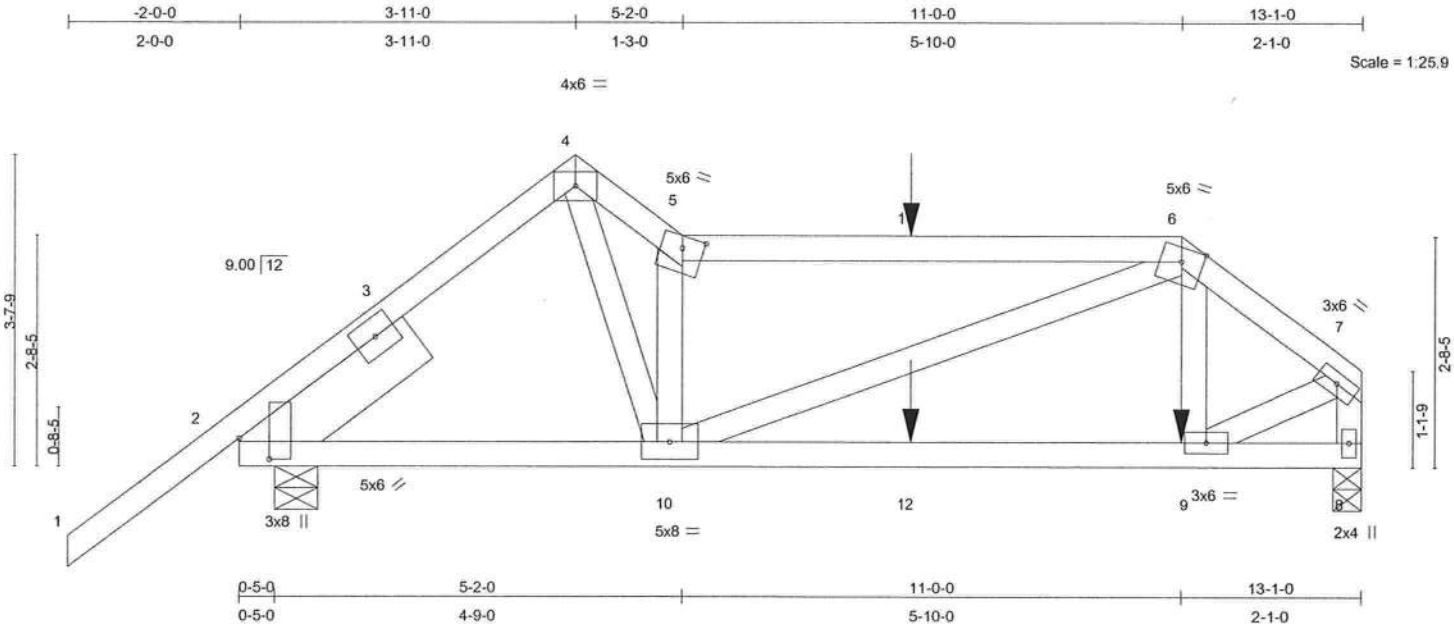

Plate Offsets (X,Y): [2:0-2-15,0-4-4], [5:0-3-0,0-1-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.07	9-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.15	9-10	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.38	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 77 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3 *Except* 7-8 2 X 4 SYP No.2
SLIDER	Left 2 X 8 SYP No.1D 2-4-11

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-3-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=673/0-6-0, 8=688/0-4-0
Max Horz 2=115(load case 5)
Max Uplift 2=-218(load case 5), 8=-226(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-752/187, 3-4=-681/205, 4-5=-1197/411, 5-11=-940/319, 6-11=-940/319, 6-7=-766/245, 7-8=-788/240
BOT CHORD	2-10=-170/504, 10-12=-239/660, 9-12=-239/660, 8-9=-102/56
WEBS	4-10=-397/1173, 5-10=-1055/405, 6-10=-89/302, 6-9=-78/88, 7-9=-325/849

JOINT STRESS INDEX

2 = 0.58, 2 = 0.29, 3 = 0.00, 4 = 0.68, 5 = 0.61, 6 = 0.63, 7 = 0.66, 8 = 0.47, 9 = 0.49 and 10 = 0.69

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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February 4,2008

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NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 218 lb uplift at joint 2 and 226 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-5=-54, 5-11=-54, 6-11=-77(F=-23), 6-7=-54, 2-12=-10, 9-12=-14(F=-4), 8-9=-10
   Concentrated Loads (lb)
   Vert: 9=-56(F) 11=-100(F) 12=-182(F)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931641
L255815	T18	SPECIAL	1	1	Job Reference (optional)

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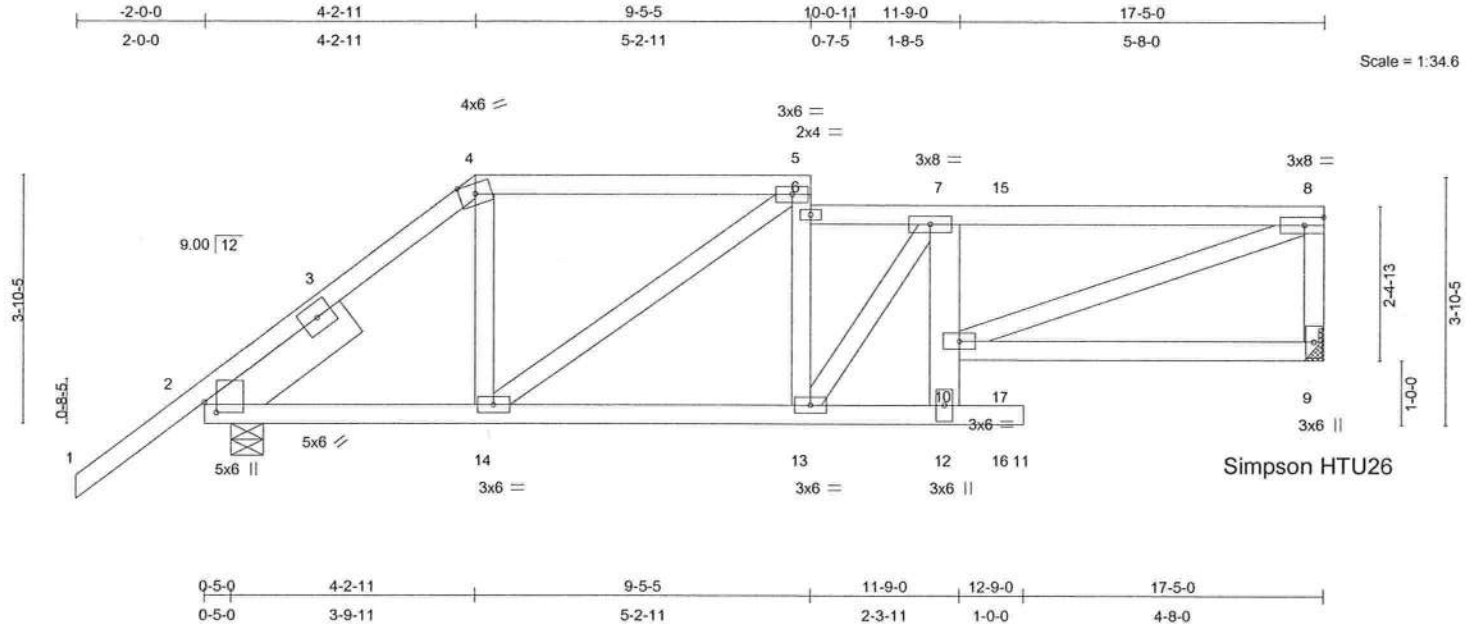

Plate Offsets (X,Y): [2:0-1-15,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.08	11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.16	11	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.67	Horz(TL)	0.03	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 108 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 7-12 2 X 6 SYP No.1D
WEBS	2 X 4 SYP No.3 *Except* 4-14 2 X 4 SYP No.2, 5-13 2 X 4 SYP No.2
SLIDER	Left 2 X 8 SYP No.1D 2-7-15

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-7-13 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 9=636/Mechanical, 2=684/0-6-0
Max Horz 2=172(load case 5)
Max Uplift 9=-205(load case 4), 2=-181(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-760/152, 3-4=-628/173, 4-5=-534/165, 6-7=-788/229, 7-15=-1146/344, 8-15=-1146/344, 8-9=-573/217
BOT CHORD	2-14=-165/527, 13-14=-199/686, 12-13=-232/769, 12-16=0/0, 11-16=0/0, 10-12=-13/119, 7-10=-263/130, 10-17=-42/120, 9-17=-42/120
WEBS	4-14=-3/235, 5-14=-225/72, 6-13=-111/47, 5-6=-75/52, 7-13=-1/105, 8-10=-322/1093

JOINT STRESS INDEX

2 = 0.69, 2 = 0.30, 3 = 0.00, 4 = 0.32, 5 = 0.49, 6 = 0.58, 7 = 0.70, 8 = 0.60, 9 = 0.31, 10 = 0.64, 12 = 0.62, 13 = 0.43 and 14 = 0.35

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.

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February 4,2008

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931641
L255815	T18	SPECIAL	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 205 lb uplift at joint 9 and 181 lb uplift at joint 2.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-5=-54, 6-15=-54, 8-15=-70(F=-16), 2-12=-10, 12-16=-10, 11-16=-13(F=-3), 10-17=-10, 9-17=-13(F=-3)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931642
L255815	T19	SPECIAL	1	1	Job Reference (optional)

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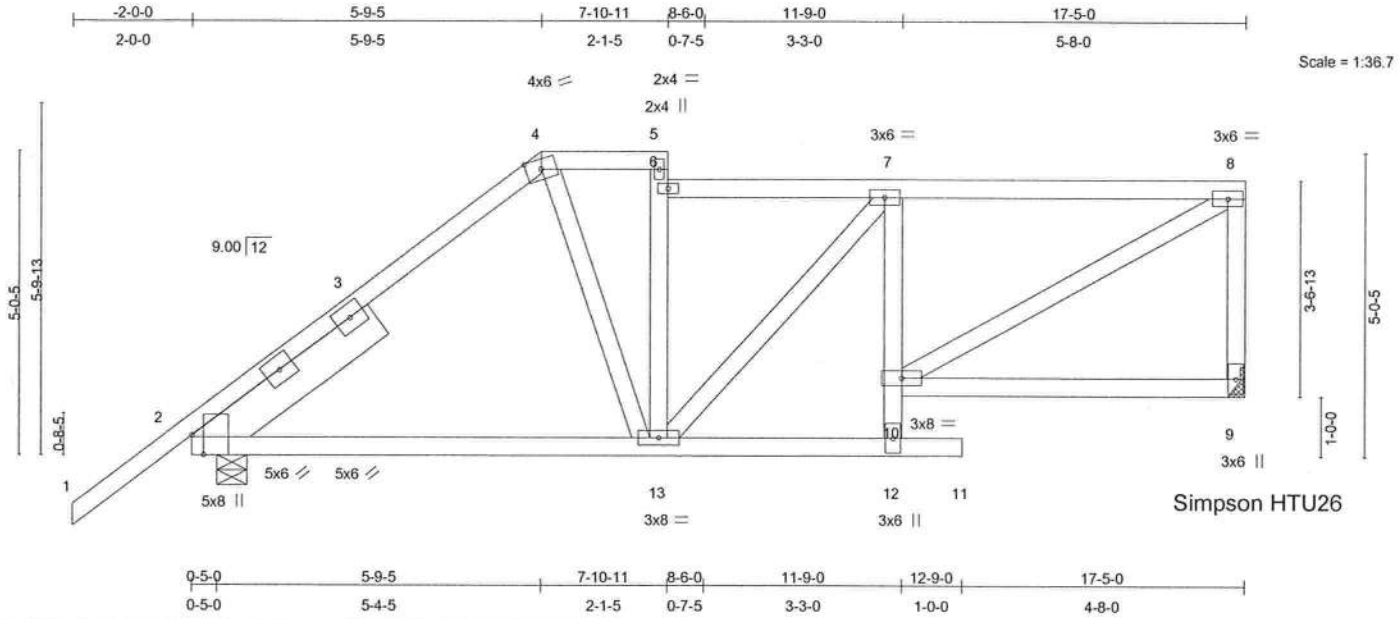

Plate Offsets (X,Y): [2:0-3-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.11	11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.18	11	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.84	Horz(TL)	0.07	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 112 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 3-7-9

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 9=554/Mechanical, 2=670/0-6-0
Max Horz 2=209(load case 6)
Max Uplift 9=-152(load case 5), 2=-176(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-678/241, 3-4=-564/268, 4-5=-491/275, 6-7=-543/304, 7-8=-656/354, 8-9=-490/290
BOT CHORD	2-13=-287/450, 12-13=-301/548, 11-12=0/0, 10-12=-16/115, 7-10=-271/209, 9-10=-30/58
WEBS	4-13=-48/178, 6-13=-128/69, 5-6=-36/48, 8-10=-374/690, 7-13=-51/86

JOINT STRESS INDEX

2 = 0.55, 2 = 0.13, 2 = 0.13, 3 = 0.00, 4 = 0.51, 5 = 0.39, 6 = 0.39, 7 = 0.57, 8 = 0.59, 9 = 0.28, 10 = 0.86, 12 = 0.74 and 13 = 0.62

NOTES

1) Unbalanced roof live loads have been considered for this design.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931642
L255815	T19	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 152 lb uplift at joint 9 and 176 lb uplift at joint 2.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931643
L255815	T20	SPECIAL	1	1	Job Reference (optional)

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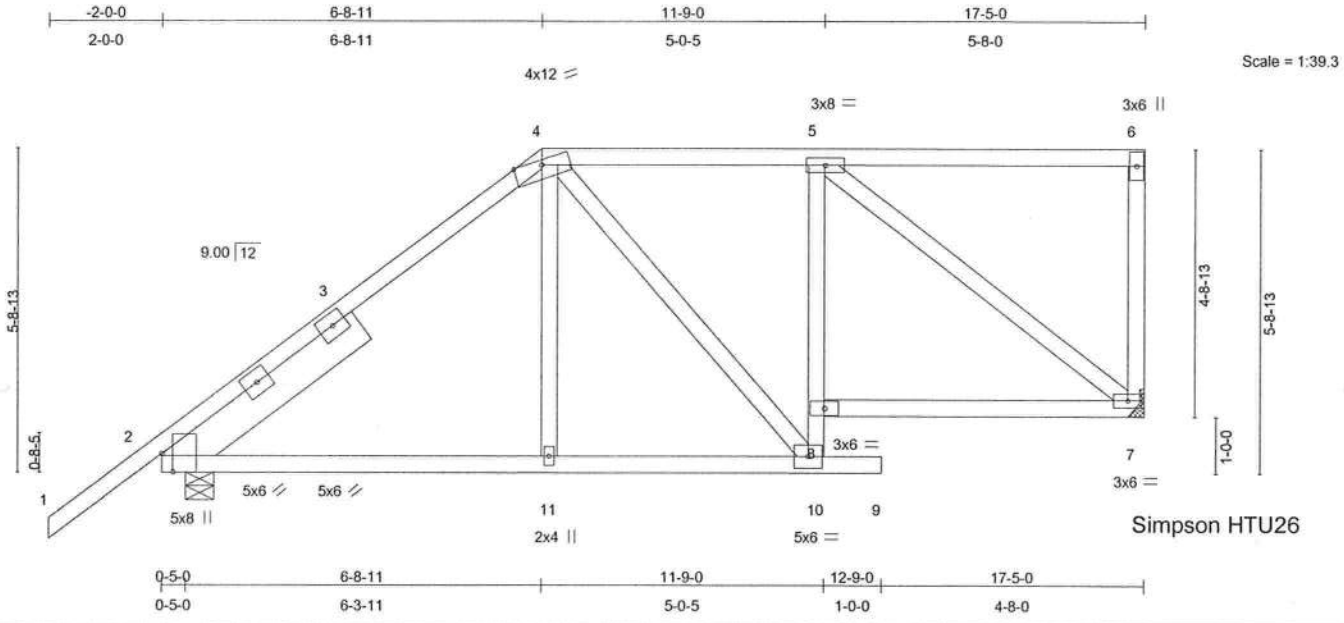

Simpson HTU26

Plate Offsets (X,Y): [2:0-3-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.08	9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.13	9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 115 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 5-10 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 4-2-11

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 7=554/Mechanical, 2=670/0-6-0
Max Horz 2=232(load case 6)
Max Uplift 7=-153(load case 5), 2=-176(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-676/223, 3-4=-551/240, 4-5=-429/238, 5-6=-38/20, 6-7=-142/103
BOT CHORD	2-11=-283/439, 10-11=-282/440, 9-10=0/0, 8-10=-46/136, 5-8=-18/202, 7-8=-270/486
WEBS	4-11=0/147, 4-10=-102/78, 5-7=-569/318

JOINT STRESS INDEX

2 = 0.55, 2 = 0.13, 2 = 0.13, 3 = 0.00, 4 = 0.90, 5 = 0.74, 6 = 0.27, 7 = 0.39, 8 = 0.44, 10 = 0.72 and 11 = 0.33

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931643
L255815	T20	SPECIAL	1	1	Job Reference (optional)

NOTES

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 153 lb uplift at joint 7 and 176 lb uplift at joint 2.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931644
L255815	T21	SPECIAL	1	1	Job Reference (optional)

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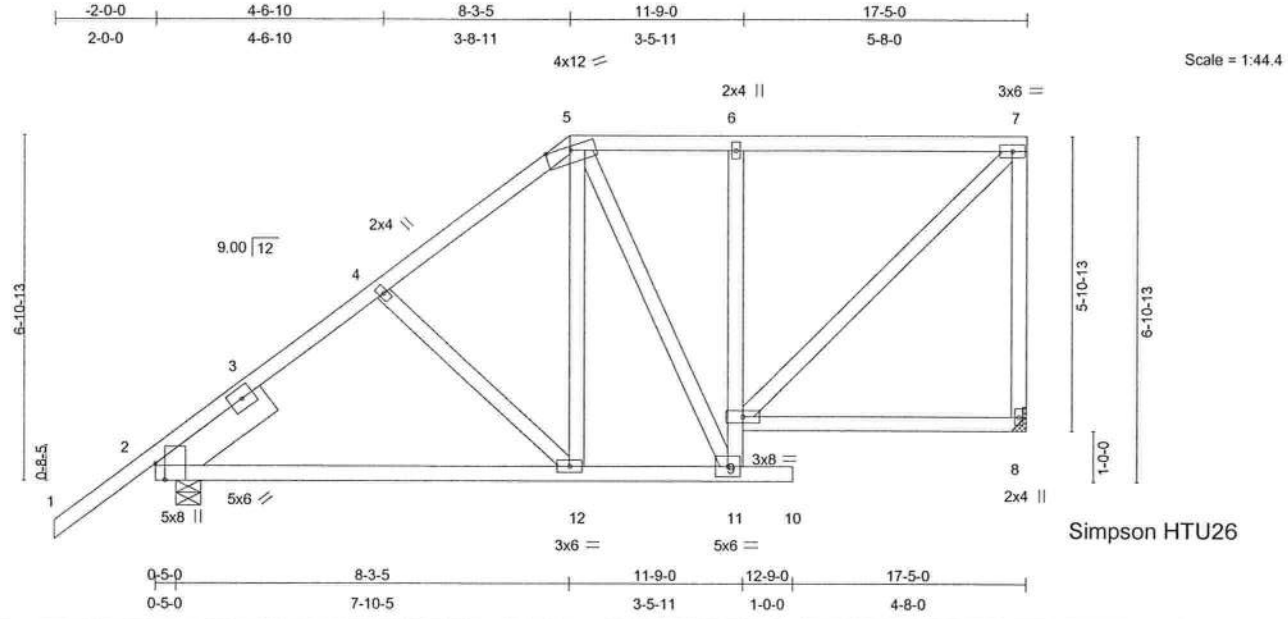

Simpson HTU26

Plate Offsets (X,Y): [2:0-3-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.09	2-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.17	2-12	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 124 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 6-11 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 8 SYP No.1D 2-7-8

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=554/Mechanical, 2=670/0-6-0
Max Horz 2=269(load case 6)
Max Uplift 8=-156(load case 5), 2=-174(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-699/219, 3-4=-561/237, 4-5=-519/226, 5-6=-352/213, 6-7=-386/231, 7-8=-500/317
BOT CHORD 2-12=-363/478, 11-12=-237/368, 10-11=0/0, 9-11=-70/144, 6-9=-248/178, 8-9=-14/28
WEBS 4-12=-153/173, 5-12=-43/199, 5-11=-90/62, 7-9=-305/505

JOINT STRESS INDEX

2 = 0.55, 2 = 0.27, 3 = 0.00, 4 = 0.33, 5 = 0.60, 6 = 0.81, 7 = 0.53, 8 = 0.74, 9 = 0.61, 11 = 0.69 and 12 = 0.35

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931644
L255815	T21	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 156 lb uplift at joint 8 and 174 lb uplift at joint 2.

LOAD CASE(S) Standard

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931645
L255815	T22	SPECIAL	1	1	Job Reference (optional)

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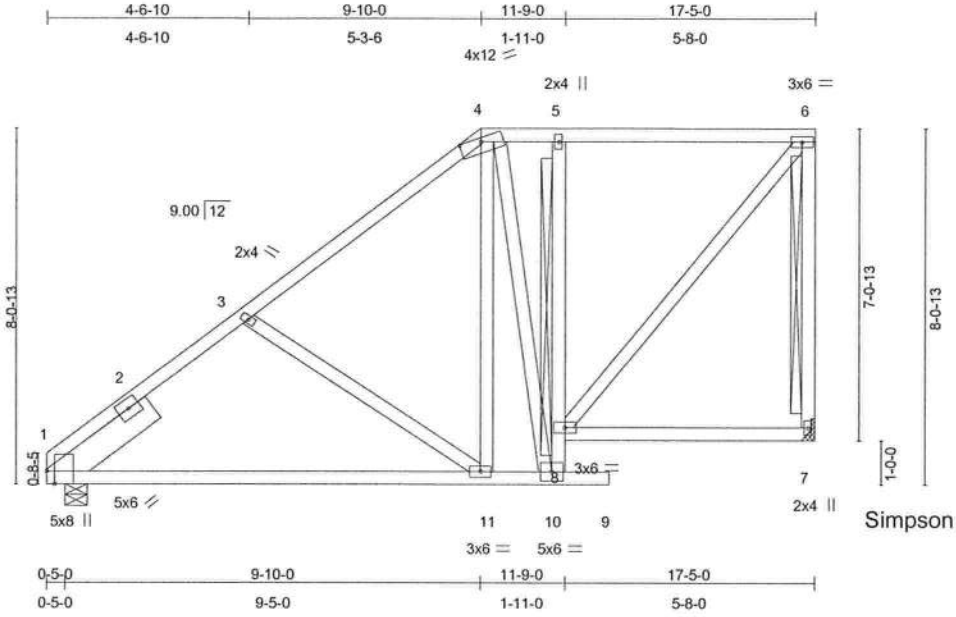

Simpson HTU26

Plate Offsets (X,Y): [1:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.15	1-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.26	1-11	>788	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 131 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 5-10 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 2-9-7

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 9-6-8 oc bracing. Except: T-Brace: 2 X 4 SYP No.3 - 5-8
WEBS	T-Brace: 2 X 4 SYP No.3 - 6-7 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 1=556/0-6-0, 7=561/Mechanical
Max Horz 1=250(load case 6)
Max Uplift 1=-76(load case 6), 7=-161(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-703/223, 2-3=-585/239, 3-4=-494/181, 4-5=-306/204, 5-6=-328/215, 6-7=-513/348
BOT CHORD	1-11=-436/515, 10-11=-223/326, 9-10=0/0, 8-10=-195/227, 5-8=-234/164, 7-8=-11/20
WEBS	3-11=-229/257, 4-11=-17/256, 6-8=-319/480, 4-10=-148/95

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JOINT STRESS INDEX

1 = 0.65, 1 = 0.25, 2 = 0.00, 3 = 0.33, 4 = 0.67, 5 = 0.70, 6 = 0.51, 7 = 0.63, 8 = 0.57, 10 = 0.69 and 11 = 0.34

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931645
L255815	T22	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 1 and 161 lb uplift at joint 7.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931646
L255815	T23	SPECIAL	1	1	Job Reference (optional)

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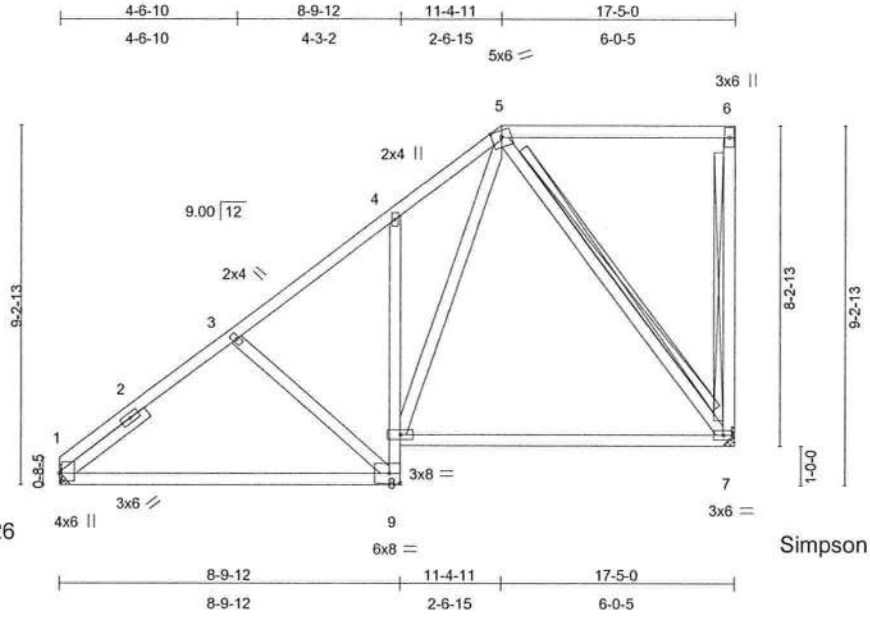

Simpson HTU26

Simpson HTU26

Plate Offsets (X,Y): [1:0-2-8,0-0-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.14	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.27	7-8	>755	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 116 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 4-9 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 4 SYP No.3 2-9-8

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 9-0-8 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 6-7, 5-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 1=553/Mechanical, 7=553/Mechanical
Max Horz 1=287(load case 6)
Max Uplift 1=-67(load case 6), 7=-169(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-722/187, 2-3=-601/208, 3-4=-547/183, 4-5=-530/321, 5-6=-23/9, 6-7=-152/105
BOT CHORD	1-9=-456/515, 8-9=-138/192, 4-8=-160/183, 7-8=-212/276
WEBS	3-9=-187/221, 5-8=-294/400, 5-7=-426/340

JOINT STRESS INDEX

1 = 0.89, 1 = 0.26, 2 = 0.00, 3 = 0.33, 4 = 0.51, 5 = 0.44, 6 = 0.26, 7 = 0.56, 8 = 0.82 and 9 = 0.67

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931646
L255815	T23	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint 1 and 169 lb uplift at joint 7.

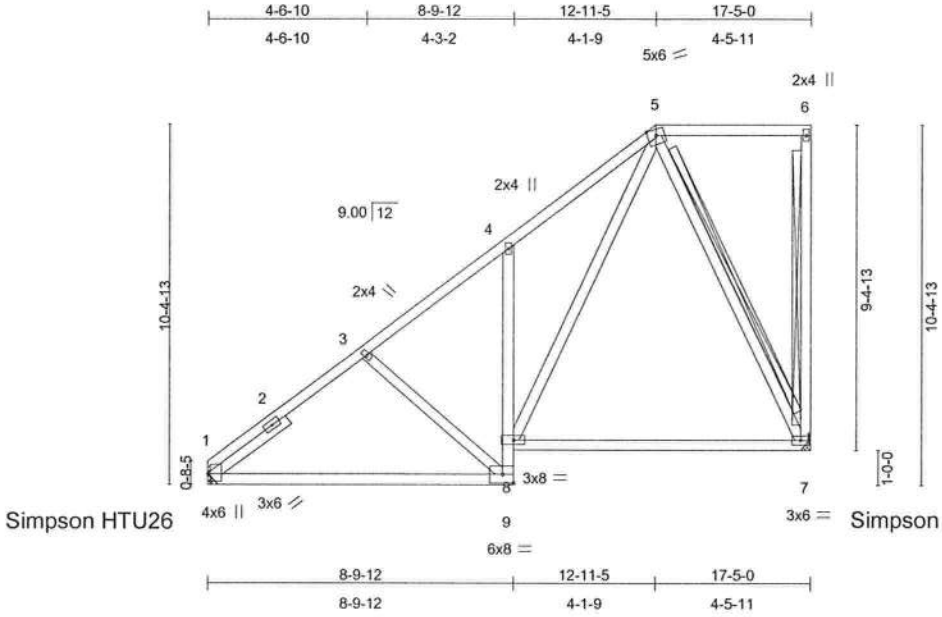
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931647
L255815	T24	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:54:11 2008 Page 1



Scale: 3/16"=1'

Simpson HTU26 Simpson HTU26

Plate Offsets (X,Y): [1:0-2-8,0-0-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.14	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.28	7-8	>731	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 121 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 4-9 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 4 SYP No.3 2-9-8

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 8-10-10 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 6-7, 5-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 1=553/Mechanical, 7=553/Mechanical
Max Horz 1=324(load case 6)
Max Uplift 1=-52(load case 6), 7=-203(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-721/140, 2-3=-600/161, 3-4=-547/138, 4-5=-587/338, 5-6=-18/3, 6-7=-107/68
BOT CHORD	1-9=-470/513, 8-9=-136/191, 4-8=-246/264, 7-8=-163/198
WEBS	3-9=-180/216, 5-8=-396/506, 5-7=-432/372

JOINT STRESS INDEX

1 = 0.88, 1 = 0.26, 2 = 0.00, 3 = 0.33, 4 = 0.57, 5 = 0.37, 6 = 0.48, 7 = 0.52, 8 = 0.84 and 9 = 0.67

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February 4,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931647
L255815	T24	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 1 and 203 lb uplift at joint 7.

LOAD CASE(S) Standard

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931648
L255815	T25	SPECIAL	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:54:12 2008 Page 1

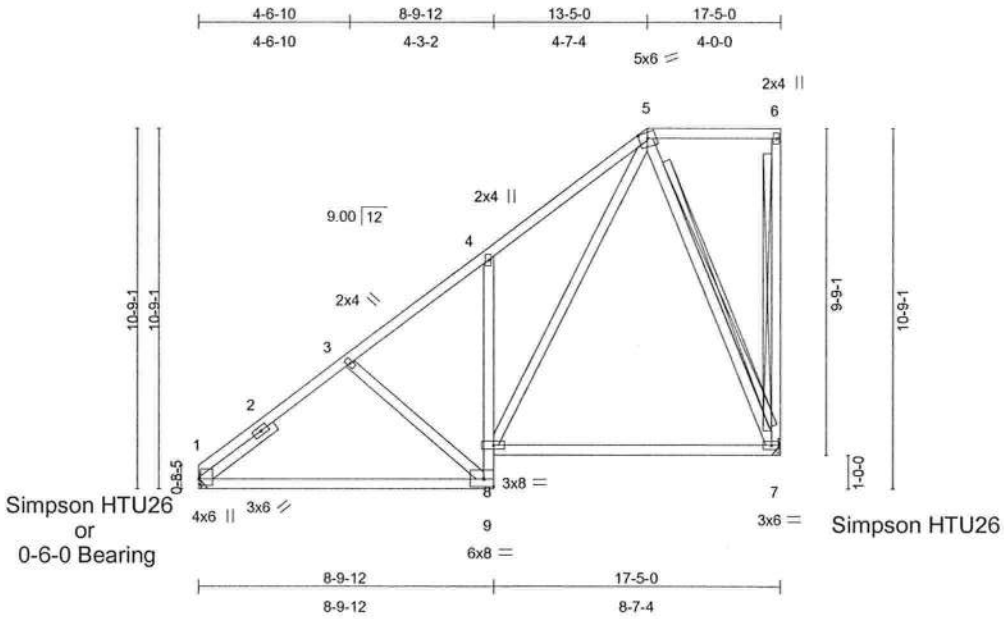

Plate Offsets (X,Y): [1:0-2-8,0-0-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.14	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.29	7-8	>725	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 123 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 4-9 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 4 SYP No.3 2-9-8

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD	Rigid ceiling directly applied or 8-10-3 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 6-7, 5-7 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance. Brace must cover 90% of web length.
JOINTS	1 Brace at Jt(s): 6

REACTIONS (lb/size) 1=553/Mechanical, 7=553/Mechanical
Max Horz 1=336(load case 6)
Max Uplift 1=-46(load case 6), 7=-214(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-720/125, 2-3=-599/146, 3-4=-547/123, 4-5=-601/340, 5-6=-16/2, 6-7=-90/55
BOT CHORD	1-9=-473/512, 8-9=-135/190, 4-8=-267/285, 7-8=-149/177
WEBS	3-9=-177/214, 5-8=-424/535, 5-7=-440/388

JOINT STRESS INDEX
1 = 0.88, 1 = 0.26, 2 = 0.00, 3 = 0.33, 4 = 0.65, 5 = 0.41, 6 = 0.45, 7 = 0.51, 8 = 0.85 and 9 = 0.67

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February 4,2008

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931648
L255815	T25	SPECIAL	3	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1 and 214 lb uplift at joint 7.

LOAD CASE(S) Standard

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931649
L255815	T26	SPECIAL	1	1	Job Reference (optional)

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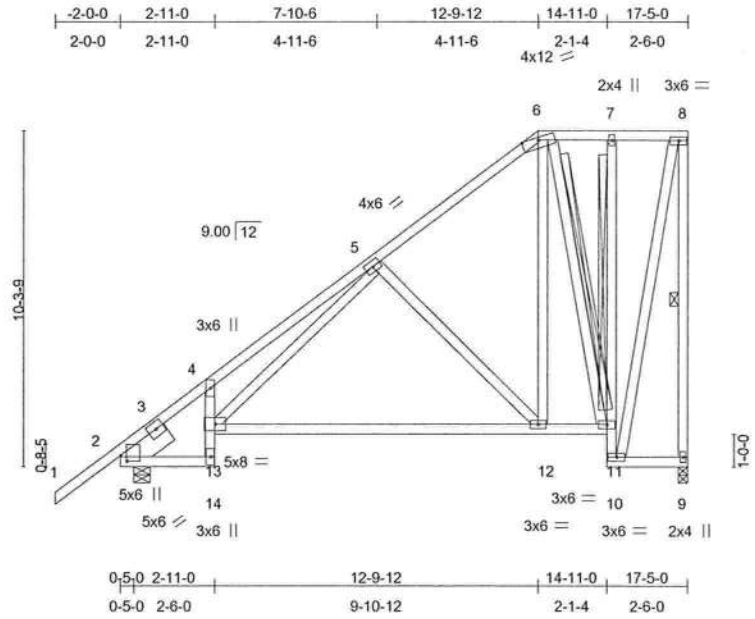

Plate Offsets (X,Y): [2:0-1-15,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.20	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.39	12-13	>527	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.10	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 159 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 7-10 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 1-7-15

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-8-5 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
	T-Brace: 2 X 4 SYP No.3 - 7-11
WEBS	1 Row at midpt 8-9
	T-Brace: 2 X 4 SYP No.3 - 6-11

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 9=546/0-3-8, 2=667/0-6-0
Max Horz 2=379(load case 6)
Max Uplift 9=-193(load case 6), 2=-144(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-796/168, 3-4=-693/168, 4-5=-1172/578, 5-6=-358/112, 6-7=-110/81, 7-8=-107/78, 8-9=-543/398
BOT CHORD	2-14=-451/496, 13-14=-63/120, 4-13=-302/300, 12-13=-395/457, 11-12=-161/221, 10-11=-454/341, 7-11=-115/81, 9-10=-2/3
WEBS	5-13=-451/647, 5-12=-341/335, 6-12=-210/452, 6-11=-457/330, 8-10=-352/483

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February 4,2008

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931649
L255815	T26	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.85, 2 = 0.30, 3 = 0.00, 4 = 0.64, 5 = 0.25, 6 = 0.58, 7 = 0.33, 8 = 0.36, 9 = 0.33, 10 = 0.36, 11 = 0.62, 12 = 0.36, 13 = 0.66 and 14 = 0.63

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 9 and 144 lb uplift at joint 2.

LOAD CASE(S) Standard

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931650
L255815	T27	SPECIAL	1	1	Job Reference (optional)

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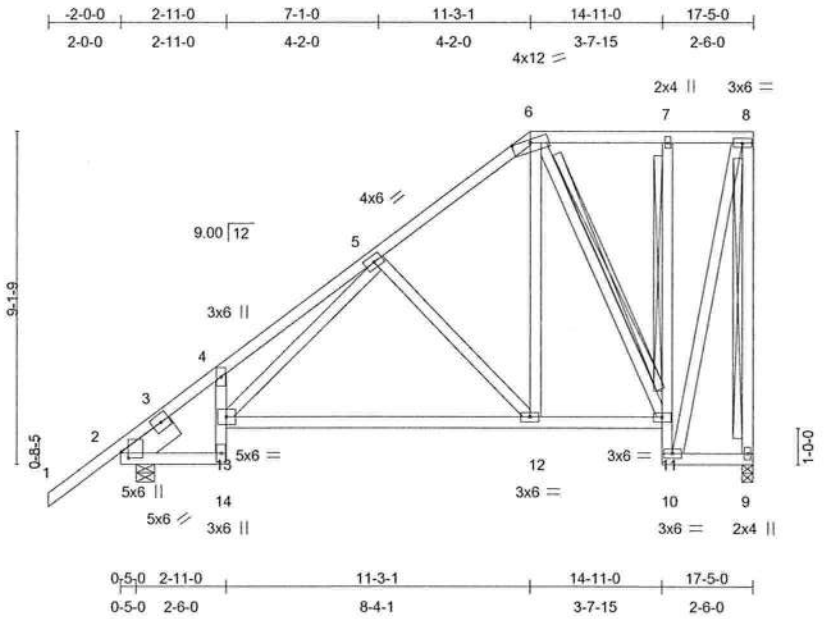

Plate Offsets (X,Y): [2:0-1-15,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.13	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.25	12-13	>819	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.09	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 148 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 7-10 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 1-7-15

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-10-11 oc purlins, except end verticals.	
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing. Except:	
	T-Brace:	2 X 4 SYP No.3 - 7-11
WEBS	T-Brace:	2 X 4 SYP No.3 - 8-9, 6-11

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 9=546/0-3-8, 2=667/0-6-0
Max Horz 2=341(load case 6)
Max Uplift 9=-167(load case 5), 2=-159(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-794/207, 3-4=-691/207, 4-5=-1115/572, 5-6=-431/183, 6-7=-133/90, 7-8=-126/85, 8-9=-549/376
BOT CHORD	2-14=-428/494, 13-14=-61/113, 4-13=-256/257, 12-13=-407/493, 11-12=-209/293, 10-11=-466/329, 7-11=-154/107, 9-10=-2/3
WEBS	5-13=-382/538, 5-12=-298/291, 6-12=-190/360, 6-11=-366/272, 8-10=-343/506

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February 4,2008

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JOINT STRESS INDEX

2 = 0.84, 2 = 0.30, 3 = 0.00, 4 = 0.73, 5 = 0.26, 6 = 0.61, 7 = 0.33, 8 = 0.36, 9 = 0.33, 10 = 0.36, 11 = 0.58, 12 = 0.36, 13 = 0.66 and 14 = 0.62

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 167 lb uplift at joint 9 and 159 lb uplift at joint 2.

LOAD CASE(S) Standard

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931651
L255815	T28	SPECIAL	1	1	Job Reference (optional)

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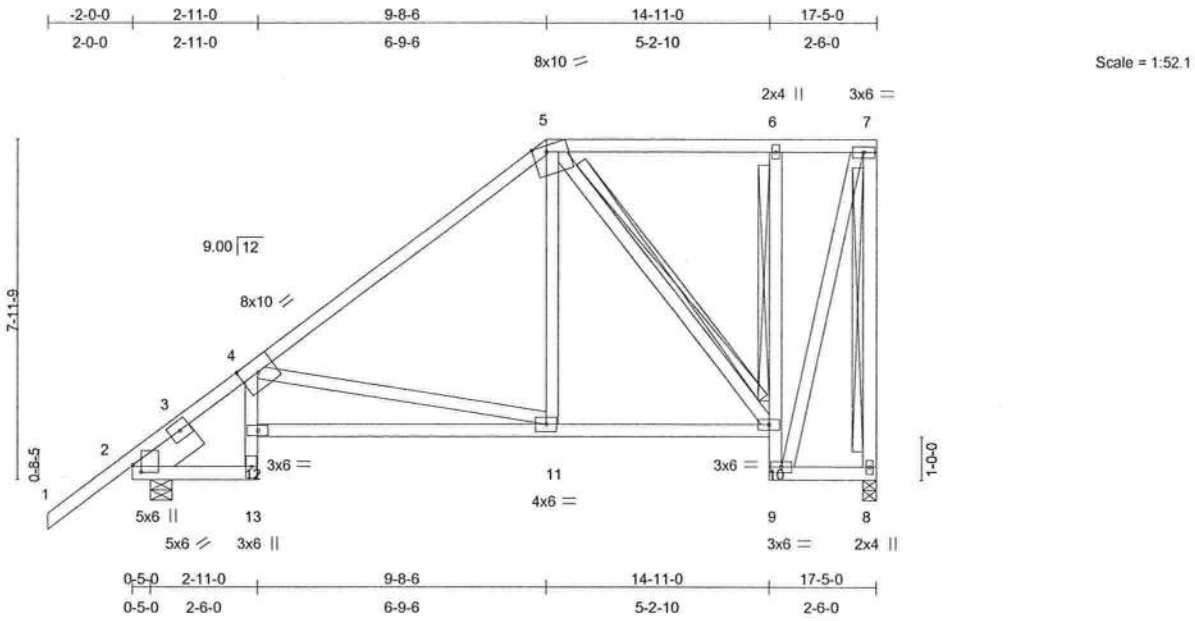

Plate Offsets (X,Y): [2:0-1-15,0-2-4], [5:0-3-14,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.10	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.15	11-12	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 133 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
6-9 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 8 SYP No.1D 1-7-15

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 6-10
WEBS T-Brace: 2 X 4 SYP No.3 - 7-8, 5-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 8=546/0-3-8, 2=667/0-6-0
Max Horz 2=304(load case 6)
Max Uplift 8=-164(load case 5), 2=-170(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-796/246, 3-4=-736/250, 4-5=-590/216, 5-6=-163/99, 6-7=-149/93, 7-8=-559/357
BOT CHORD 2-13=-422/510, 12-13=-45/102, 4-12=-13/198, 11-12=-756/933, 10-11=-274/387, 9-10=-473/310, 6-10=-188/152, 8-9=-2/3
WEBS 4-11=-562/494, 5-11=-80/302, 5-10=-358/278, 7-9=-328/529

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JOINT STRESS INDEX

2 = 0.67, 2 = 0.30, 3 = 0.00, 4 = 0.59, 5 = 0.60, 6 = 0.41, 7 = 0.37, 8 = 0.33, 9 = 0.43, 10 = 0.45, 11 = 0.28, 12 = 0.37 and 13 =

February 4,2008

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931651
L255815	T28	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 lb uplift at joint 8 and 170 lb uplift at joint 2.

LOAD CASE(S) Standard

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931652
L255815	T29	SPECIAL	1	1	Job Reference (optional)

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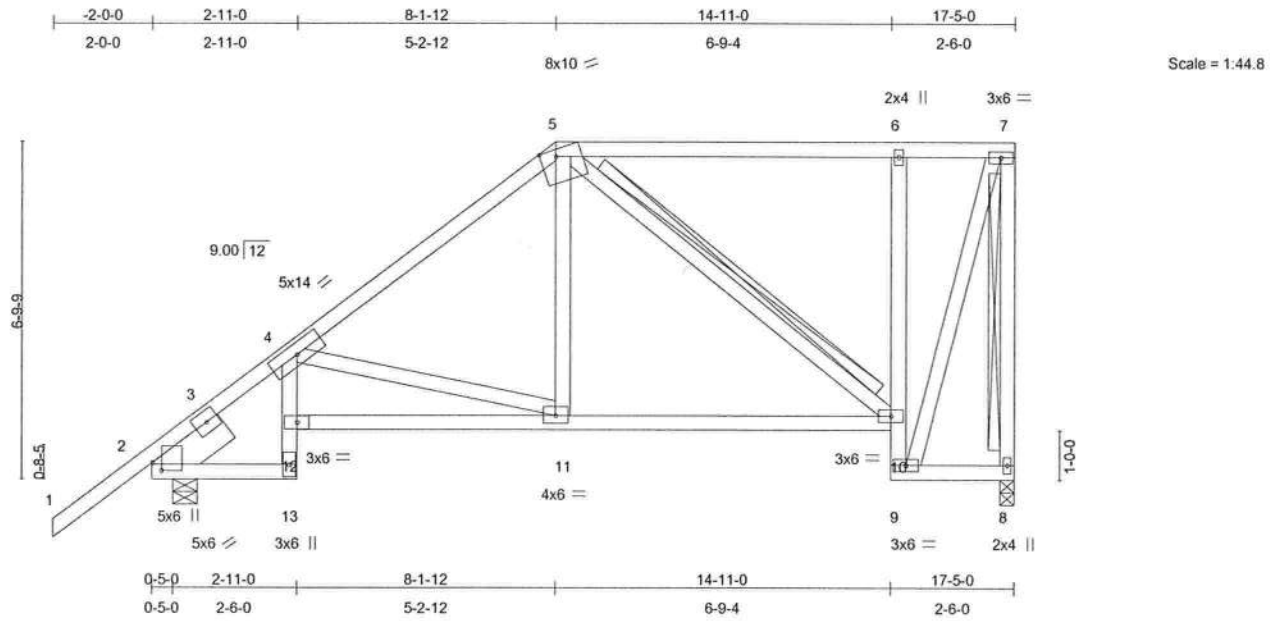

Plate Offsets (X,Y): [2:0-1-15,0-2-4], [5:0-3-14,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	0.08	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.11	10-11	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.28	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 124 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 6-9 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 1-7-15

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 7-8, 5-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 8=546/0-3-8, 2=667/0-6-0
Max Horz 2=266(load case 6)
Max Uplift 8=-161(load case 5), 2=-176(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-791/273, 3-4=-690/275, 4-5=-657/285, 5-6=-223/128, 6-7=-190/113, 7-8=-573/342
BOT CHORD	2-13=-384/499, 12-13=-47/97, 4-12=-19/154, 11-12=-643/834, 10-11=-313/471, 9-10=-507/324, 6-10=-287/204, 8-9=-1/2
WEBS	4-11=-380/343, 5-11=-68/293, 5-10=-318/236, 7-9=-351/587

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JOINT STRESS INDEX

2 = 0.74, 2 = 0.30, 3 = 0.00, 4 = 0.80, 5 = 0.58, 6 = 0.88, 7 = 0.41, 8 = 0.33, 9 = 0.58, 10 = 0.63, 11 = 0.29, 12 = 0.41 and 13 = 0.58

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES.
L255815	T29	SPECIAL	1	1	J1931652 Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 161 lb uplift at joint 8 and 176 lb uplift at joint 2.

LOAD CASE(S) Standard

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931653
L255815	T30	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:54:16 2008 Page 1

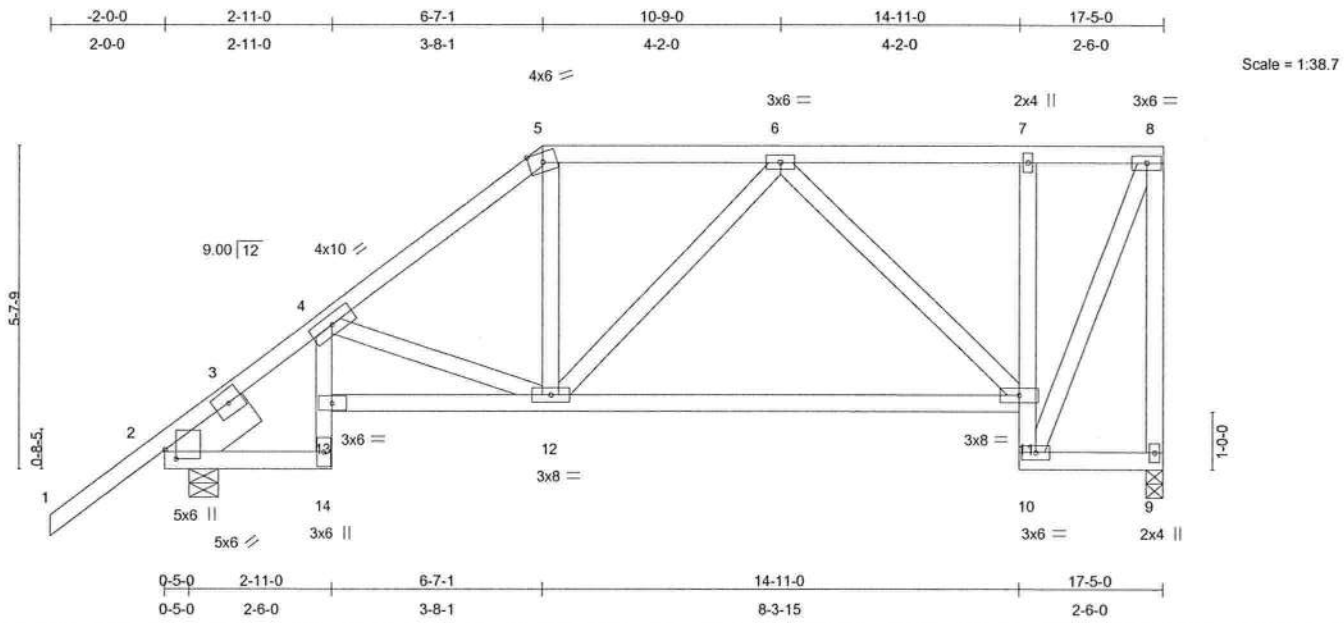

Plate Offsets (X,Y): [2:0-1-15,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.10	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.20	11-12	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.24	Horz(TL)	0.10	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 118 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 7-10 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 1-7-15

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 9=546/0-3-8, 2=667/0-6-0
Max Horz 2=229(load case 6)
Max Uplift 9=-158(load case 5), 2=-178(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-788/297, 3-4=-687/298, 4-5=-727/344, 5-6=-543/334, 6-7=-259/136, 7-8=-216/122, 8-9=-583/328
BOT CHORD	2-14=-349/496, 13-14=-44/91, 4-13=-19/88, 12-13=-564/783, 11-12=-315/515, 10-11=-457/282, 7-11=-166/131, 9-10=-2/3
WEBS	4-12=-263/246, 5-12=-45/231, 6-12=-41/104, 6-11=-364/255, 8-10=-315/558

JOINT STRESS INDEX

2 = 0.74, 2 = 0.30, 3 = 0.00, 4 = 0.92, 5 = 0.30, 6 = 0.36, 7 = 0.70, 8 = 0.49, 9 = 0.33, 10 = 0.59, 11 = 0.79, 12 = 0.56, 13 = 0.48 and 14 = 0.57

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February 4,2008

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931653
L255815	T30	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 9 and 178 lb uplift at joint 2.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931654
L255815	T31	SPECIAL	1	1	Job Reference (optional)

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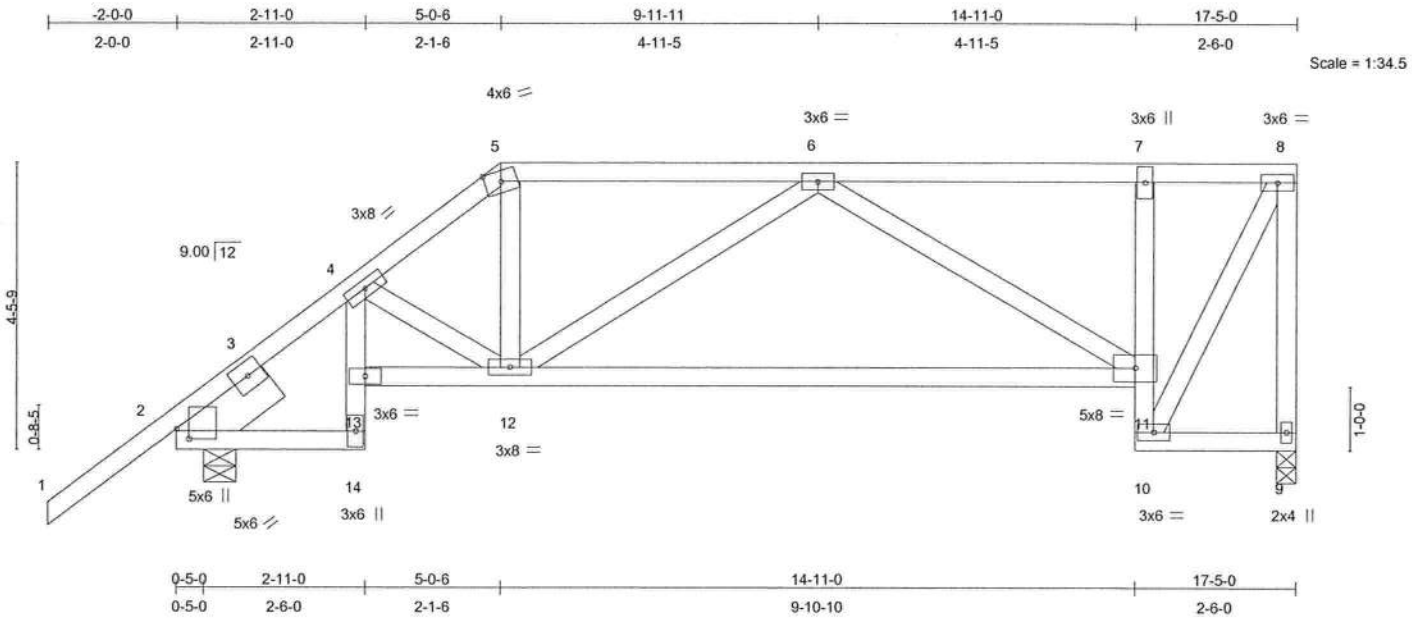

Plate Offsets (X,Y): [2:0-1-15,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.17	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.33	11-12	>632	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.28	Horz(TL)	0.11	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 109 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 7-10 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 1-7-15

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 9=546/0-3-8, 2=667/0-6-0
Max Horz 2=191(load case 6)
Max Uplift 9=-170(load case 4), 2=-177(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-784/315, 3-4=-682/316, 4-5=-816/400, 5-6=-663/370, 6-7=-371/180, 7-8=-280/153, 8-9=-596/321
BOT CHORD	2-14=-310/492, 13-14=-40/86, 4-13=-37/0, 12-13=-470/710, 11-12=-434/737, 10-11=-450/272, 7-11=-180/147, 9-10=-1/3
WEBS	4-12=-69/120, 5-12=-87/305, 6-12=-90/140, 6-11=-431/300, 8-10=-324/591

JOINT STRESS INDEX

2 = 0.75, 2 = 0.30, 3 = 0.00, 4 = 0.55, 5 = 0.29, 6 = 0.34, 7 = 0.34, 8 = 0.49, 9 = 0.33, 10 = 0.69, 11 = 0.62, 12 = 0.56, 13 = 0.56 and 14 = 0.56

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February 4,2008

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931654
L255815	T31	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 9 and 177 lb uplift at joint 2.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931655
L255815	T32	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:54:18 2008 Page 1

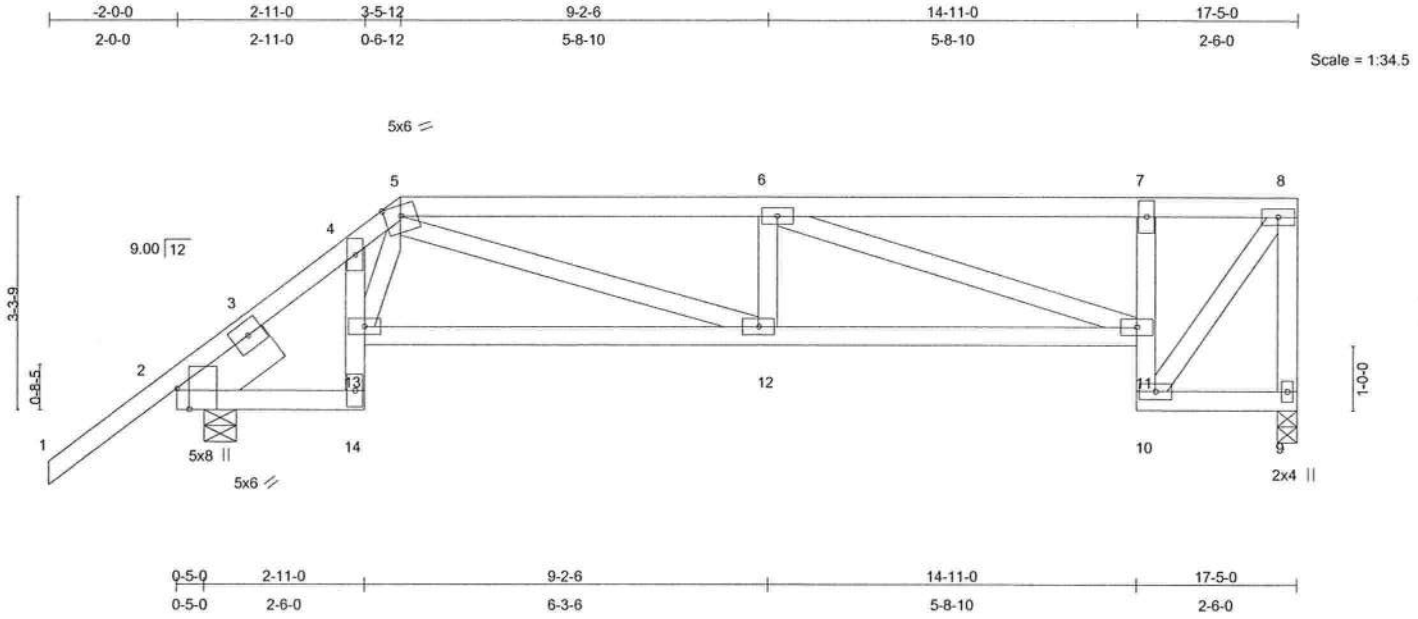

Plate Offsets (X,Y): [2:0-3-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	0.09	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.16	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.13	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 101 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 7-10 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 1-7-15

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-6-1 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 5-11-7 oc bracing.

REACTIONS (lb/size) 9=546/0-3-8, 2=667/0-6-0
Max Horz 2=154(load case 6)
Max Uplift 9=-186(load case 4), 2=-170(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-789/331, 3-4=-685/332, 4-5=-847/404, 5-6=-1246/654, 6-7=-582/304, 7-8=-388/207, 8-9=-601/323
BOT CHORD	2-14=-271/486, 13-14=-35/105, 4-13=-142/138, 12-13=-428/770, 11-12=-654/1246, 10-11=-445/255, 7-11=-201/156, 9-10=-1/6
WEBS	5-13=-29/228, 5-12=-239/529, 6-12=-94/107, 6-11=-702/369, 8-10=-348/648

JOINT STRESS INDEX

2 = 0.55, 2 = 0.30, 3 = 0.00, 4 = 0.58, 5 = 0.68, 6 = 0.34, 7 = 0.51, 8 = 0.47, 9 = 0.33, 10 = 0.68, 11 = 0.65, 12 = 0.34, 13 = 0.61 and 14 = 0.62

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NOTES

1) Unbalanced roof live loads have been considered for this design.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931655
L255815	T32	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 186 lb uplift at joint 9 and 170 lb uplift at joint 2.

LOAD CASE(S) Standard

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931656
L255815	T33	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:54:19 2008 Page 1

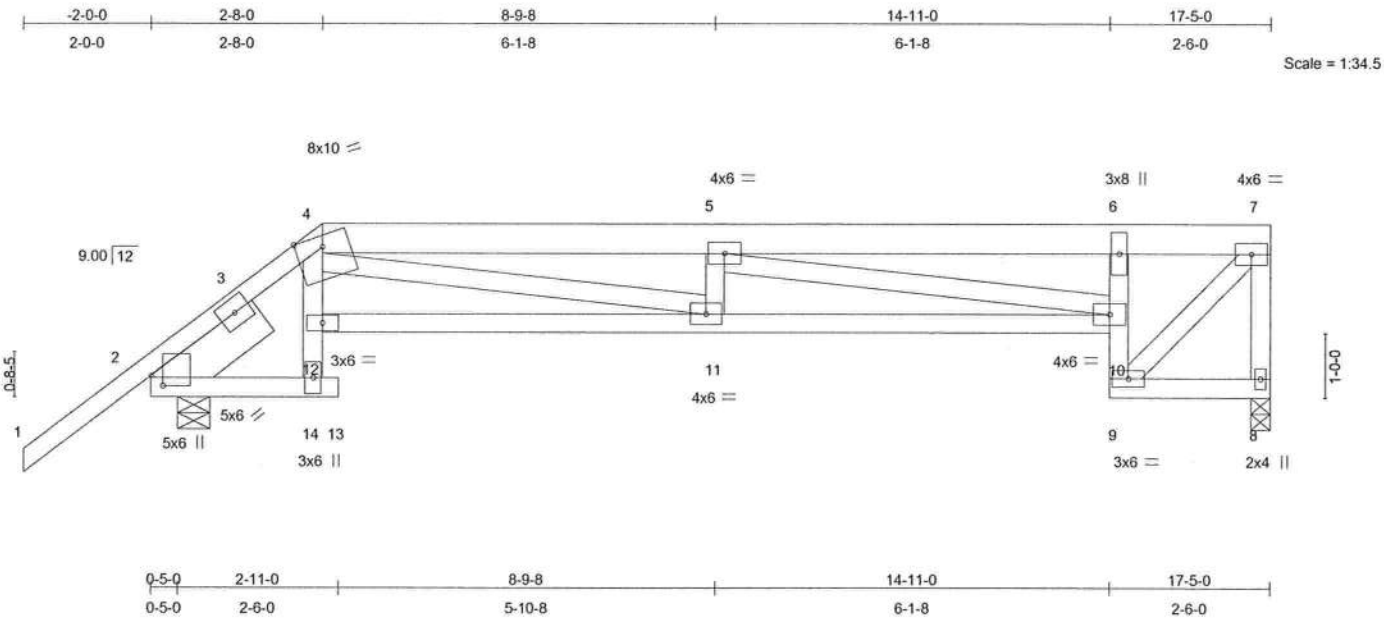

Plate Offsets (X,Y): [2:0-1-15,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	0.13	11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.22	11-12	>947	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.15	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 108 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2 *Except* 4-7 2 X 6 SYP No.1D
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 1-11-11

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 8=547/0-3-8, 2=670/0-6-0
Max Horz 2=131(load case 6)
Max Uplift 8=-192(load case 4), 2=-163(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-810/341, 3-4=-717/345, 4-5=-1792/915, 5-6=-939/477, 6-7=-452/242, 7-8=-600/319
BOT CHORD	2-14=-260/510, 13-14=0/0, 12-14=-29/120, 4-12=-12/209, 11-12=-507/1016, 10-11=-917/1790, 9-10=-393/227, 6-10=-182/152, 8-9=-1/8
WEBS	5-11=-93/123, 5-10=-870/449, 7-9=-350/647, 4-11=-418/790

JOINT STRESS INDEX

2 = 0.84, 2 = 0.31, 3 = 0.00, 4 = 0.42, 5 = 0.26, 6 = 0.69, 7 = 0.28, 8 = 0.33, 9 = 0.65, 10 = 0.73, 11 = 0.32, 12 = 0.37 and 14 = 0.63

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February 4,2008

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931656
L255815	T33	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 192 lb uplift at joint 8 and 163 lb uplift at joint 2.

LOAD CASE(S) Standard

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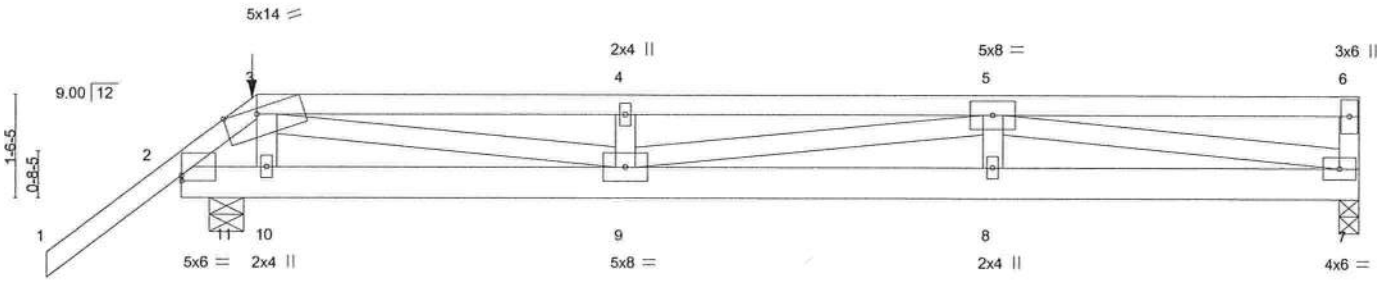
February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931657
L255815	T34	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Feb 01 12:05:42 2008 Page 1

Scale = 1:32.8



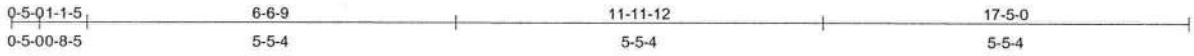


Plate Offsets (X,Y): [2:0-0-1,0-0-15], [3:0-5-14,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.12	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.22	8-9	>898	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.73	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 98 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 7=520/0-3-8, 11=699/0-6-0
Max Horz 11=99(load case 5)
Max Uplift 7=-190(load case 3), 11=-218(load case 3)
Max Grav 7=528(load case 10), 11=699(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/56, 2-3=-515/136, 3-4=-1655/583, 4-5=-1655/583, 5-6=-245/97, 6-7=-151/79
BOT CHORD 2-11=-106/427, 10-11=-129/427, 9-10=-114/409, 8-9=-591/1654, 7-8=-591/1654
WEBS 3-10=-251/183, 3-9=-480/1318, 4-9=-285/158, 5-9=-34/40, 5-8=0/150, 5-7=-1442/506

JOINT STRESS INDEX
2 = 0.58, 3 = 0.76, 4 = 0.34, 5 = 0.34, 6 = 0.46, 7 = 0.48, 8 = 0.34, 9 = 0.60 and 10 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 7 and 218 lb uplift at joint 11.

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February 4,2008

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931657
L255815	T34	MONO HIP	1	1	Job Reference (optional)

NOTES

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-6=-54, 2-7=-10
   Concentrated Loads (lb)
   Vert: 3=-5(F)

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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931658
L255815	T35	HIP	1	2	Job Reference (optional)

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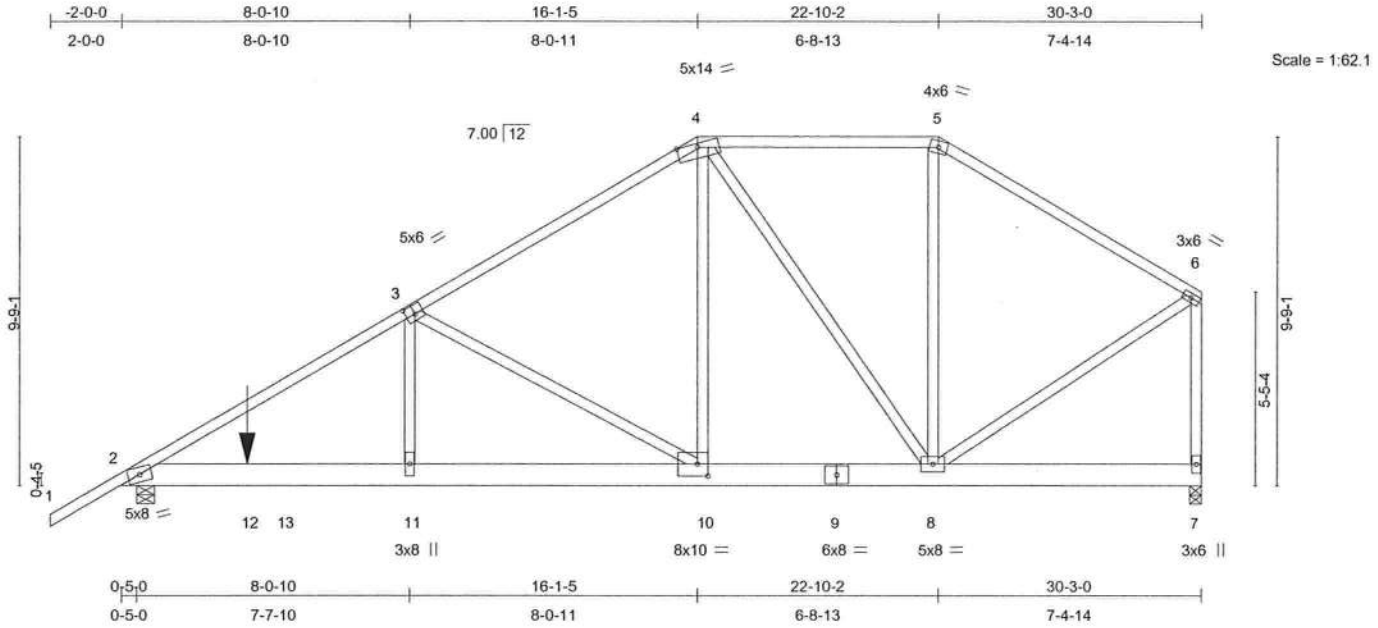

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [10:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.13	10-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.25	10-11	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.99	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 458 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-14 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 7=3027/0-4-0, 2=4130/0-6-0
Max Horz 2=277(load case 5)
Max Uplift 7=-911(load case 6), 2=-1362(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/60, 2-3=-6741/2133, 3-4=-4074/1333, 4-5=-2284/798, 5-6=-2723/874, 6-7=-3026/940
BOT CHORD 2-12=-2006/5725, 12-13=-2006/5725, 11-13=-2006/5725, 10-11=-1994/5691, 9-10=-1199/3472, 8-9=-1199/3472, 7-8=-33/51
WEBS 3-11=-732/2212, 3-10=-2612/959, 4-10=-1142/3284, 4-8=-2117/802, 5-8=-372/1048, 6-8=-899/2684

JOINT STRESS INDEX
2 = 0.68, 3 = 0.59, 4 = 0.79, 5 = 0.53, 6 = 0.90, 7 = 0.25, 8 = 0.62, 9 = 0.43, 10 = 0.34 and 11 = 0.36

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.

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NOTES

4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 911 lb uplift at joint 7 and 1362 lb uplift at joint 2.
9) Girder carries tie-in span(s): 17-5-0 from 4-6-0 to 22-6-0

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-5=-54, 5-6=-54, 2-13=-10, 8-13=-257(F=-247), 7-8=-10
   Concentrated Loads (lb)
   Vert: 12=-636(F)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931659
L255815	T36	SPECIAL	3	1	Job Reference (optional)

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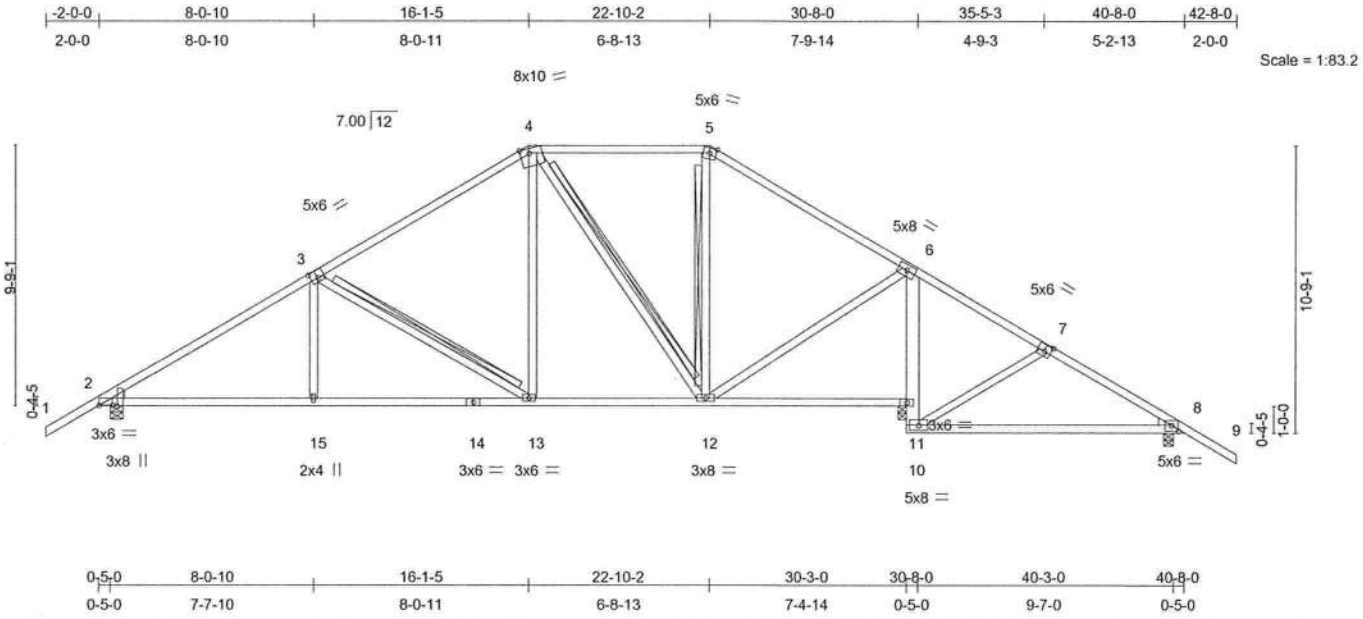

Plate Offsets (X,Y): [2:0-0-4,Edge], [2:0-6-0,0-0-2], [3:0-3-0,0-3-4], [4:0-4-1,Edge], [7:0-3-0,0-3-0], [8:0-3-0,0-2-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	0.31	8-10	>385	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.27	8-10	>446	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.31	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 237 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
6-10 2 X 6 SYP No.1D, 8-10 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-8 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-13, 4-12, 5-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1066/0-6-0, 8=368/0-4-0, 11=1381/0-4-0
Max Horz 2=291(load case 5)
Max Uplift 2=-325(load case 6), 8=-399(load case 7), 11=-246(load case 7)
Max Grav 2=1066(load case 1), 8=400(load case 11), 11=1381(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-1496/740, 3-4=-975/630, 4-5=-547/564, 5-6=-741/561, 6-7=-18/346, 7-8=-274/448, 8-9=0/54
BOT CHORD 2-15=-464/1191, 14-15=-465/1190, 13-14=-465/1190, 12-13=-259/749, 11-12=-113/0, 10-11=-318/219, 6-11=-1146/361, 8-10=-246/178
WEBS 3-15=0/265, 3-13=-518/350, 4-13=-137/402, 4-12=-416/158, 5-12=-150/131, 6-12=-102/765, 7-10=-264/264

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931659
L255815	T36	SPECIAL	3	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.65, 2 = 0.26, 3 = 0.73, 4 = 0.75, 5 = 0.66, 6 = 0.36, 7 = 0.41, 8 = 0.85, 8 = 0.00, 10 = 0.78, 11 = 0.75, 12 = 0.71, 13 = 0.34, 14 = 0.42 and 15 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 325 lb uplift at joint 2, 399 lb uplift at joint 8 and 246 lb uplift at joint 11.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931660
L255815	T37	SPECIAL	1	1	Job Reference (optional)

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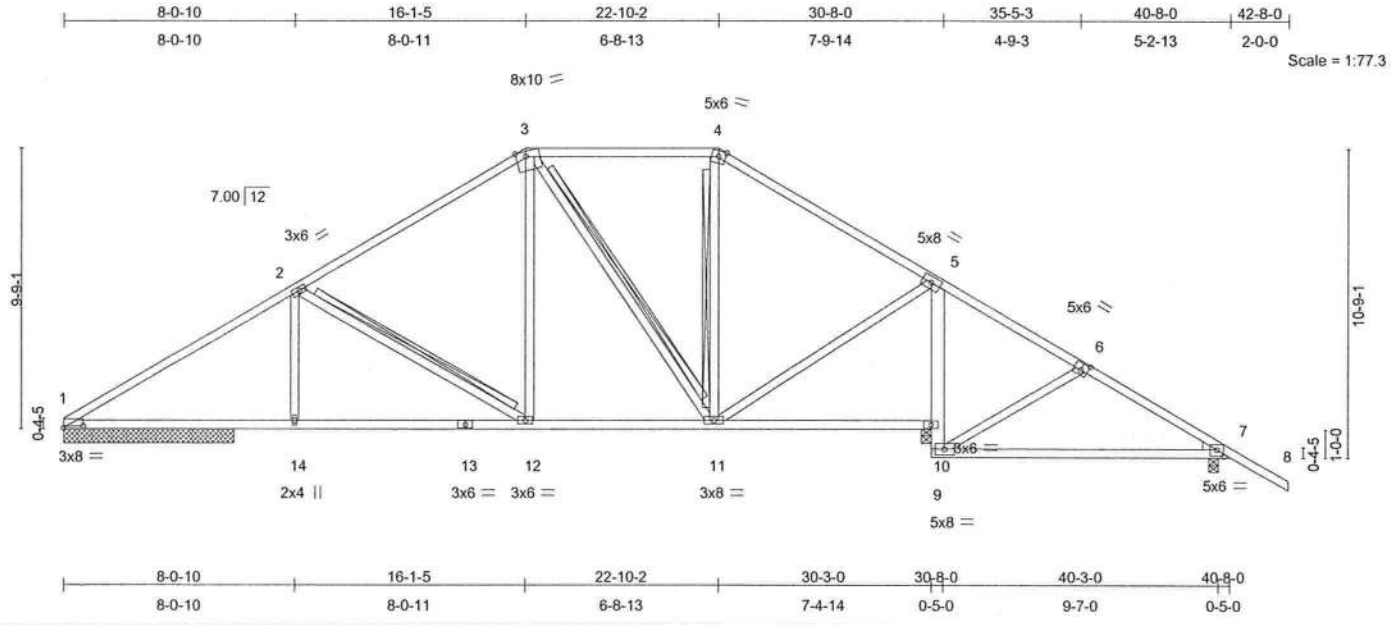

Plate Offsets (X,Y): [1:0-8-0,0-0-10], [3:0-4-1,Edge], [6:0-3-0,0-3-0], [7:0-3-0,0-2-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	0.31	7-9	>384	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.27	7-9	>444	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 232 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
5-9 2 X 6 SYP No.1D, 7-9 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-12 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 3-4.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 2-12, 3-11, 4-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 1=946/5-11-0, 7=361/0-4-0, 10=1402/0-4-0
Max Horz 1=-310(load case 4)
Max Uplift 1=-229(load case 6), 7=-405(load case 7), 10=-241(load case 7)
Max Grav 1=946(load case 1), 7=396(load case 11), 10=1402(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1559/804, 2-3=-993/653, 3-4=-552/579, 4-5=-747/578, 5-6=-10/368, 6-7=-268/470, 7-8=0/54
BOT CHORD 1-14=-465/1259, 13-14=-465/1259, 12-13=-465/1259, 11-12=-236/762, 10-11=-126/0, 9-10=-317/219, 5-10=-1168/357, 7-9=-264/173
WEBS 2-14=0/273, 2-12=-580/404, 3-12=-156/414, 3-11=-431/159, 4-11=-149/132, 5-11=-99/787, 6-9=-265/263

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931660
L255815	T37	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

1 = 0.71, 2 = 0.40, 3 = 0.74, 4 = 0.66, 5 = 0.35, 6 = 0.41, 7 = 0.85, 7 = 0.00, 9 = 0.78, 10 = 0.75, 11 = 0.73, 12 = 0.34, 13 = 0.44 and 14 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 1, 405 lb uplift at joint 7 and 241 lb uplift at joint 10.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931661
L255815	T38	SPECIAL	2	1	Job Reference (optional)

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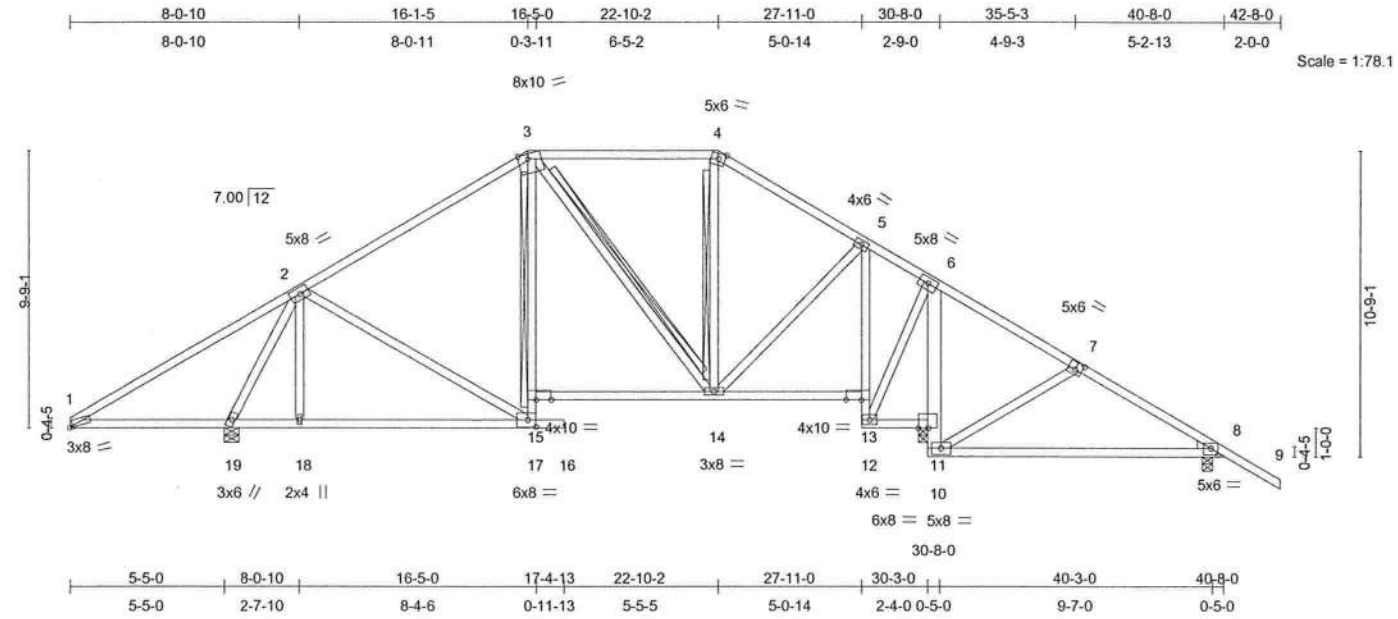

Plate Offsets (X,Y): [1:0-1-6,0-0-3], [3:0-4-1,Edge], [7:0-3-0,0-3-0], [8:0-3-0,0-2-11], [13:0-6-8,Edge], [15:0-6-8,Edge], [17:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	0.30	8-10	>398	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.28	8-10	>427	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.11	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 253 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
3-17 2 X 4 SYP No.3, 5-12 2 X 4 SYP No.3
6-10 2 X 6 SYP No.1D, 8-10 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 3-4.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 3-15
WEBS T-Brace: 2 X 4 SYP No.3 - 3-14, 4-14
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 11=1285/0-4-0, 8=296/0-4-0, 19=1139/0-6-0
Max Horz 19=-310(load case 4)
Max Uplift 11=-188(load case 7), 8=-417(load case 7), 19=-430(load case 6)
Max Grav 11=1285(load case 1), 8=326(load case 11), 19=1139(load case 1)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931661
L255815	T38	SPECIAL	2	1	Job Reference (optional)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-350/464, 2-3=-621/415, 3-4=-372/450, 4-5=-496/452, 5-6=-111/403, 6-7=0/362, 7-8=-134/470, 8-9=0/54

BOT CHORD 1-19=-296/380, 18-19=-209/335, 17-18=-209/335, 16-17=0/0, 15-17=-45/125, 3-15=-45/153, 14-15=-171/451, 13-14=-22/94, 12-13=-704/84, 5-13=-695/96, 11-12=-278/28, 10-11=-326/218, 6-11=-1032/163, 8-10=-265/59

WEBS 2-18=-33/139, 2-17=-163/251, 3-14=-180/117, 4-14=-146/89, 5-14=-119/444, 6-12=-39/744, 7-10=-278/277, 2-19=-1114/695

JOINT STRESS INDEX

1 = 0.85, 2 = 0.49, 3 = 0.68, 4 = 0.46, 5 = 0.30, 6 = 0.38, 7 = 0.45, 8 = 0.85, 8 = 0.00, 10 = 0.74, 11 = 0.44, 12 = 0.44, 13 = 0.10, 14 = 0.56, 15 = 0.38, 17 = 0.72, 18 = 0.33 and 19 = 0.29

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 188 lb uplift at joint 11, 417 lb uplift at joint 8 and 430 lb uplift at joint 19.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931662
L255815	T39	SPECIAL	1	1	Job Reference (optional)

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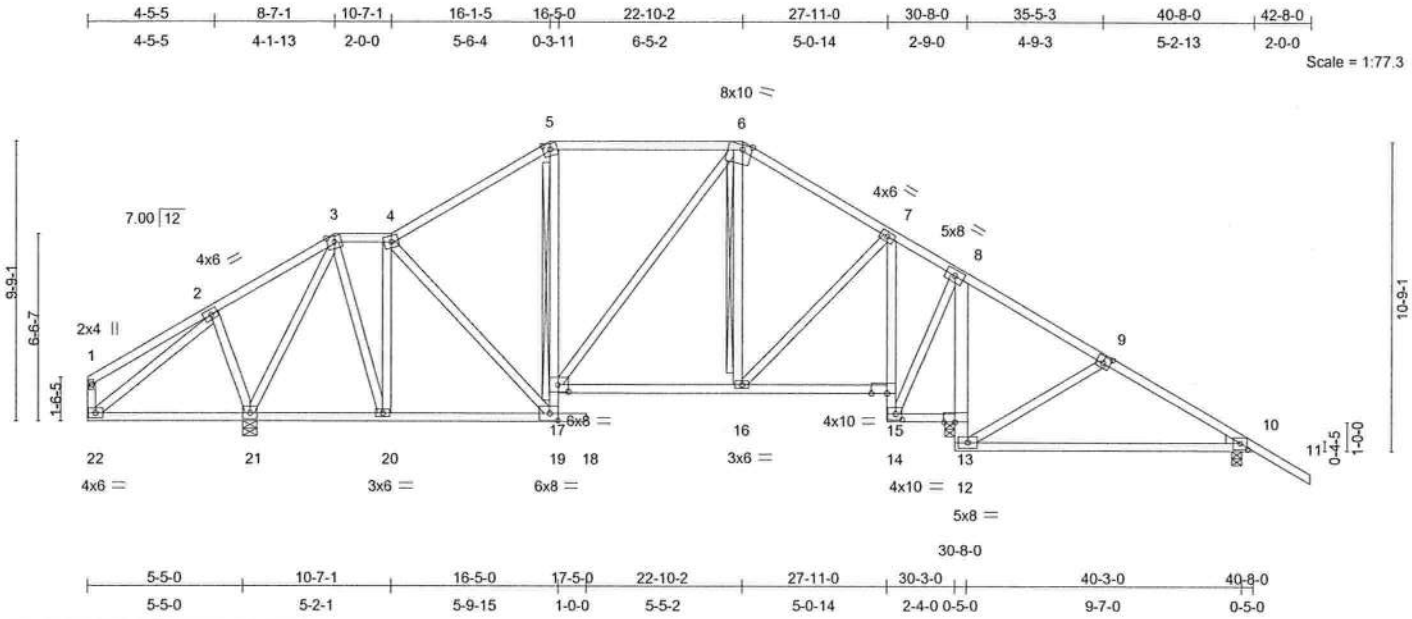

Plate Offsets (X,Y): [6:0-4-1,Edge], [9:0-3-0,0-3-0], [10:0-3-0,0-2-11], [13:0-4-8,0-0-0], [14:0-2-12,0-2-8], [15:0-6-8,Edge], [17:0-4-8,0-3-0], [19:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	0.30	10-12	>398	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.28	10-12	>425	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.10	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 280 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
5-19 2 X 4 SYP No.3, 7-14 2 X 4 SYP No.3
8-12 2 X 6 SYP No.1D, 10-12 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 3-4, 5-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 5-17
WEBS T-Brace: 2 X 4 SYP No.3 - 6-16
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 13=1282/0-4-0, 10=298/0-4-0, 21=1132/0-6-0
Max Horz 21=-308(load case 4)
Max Uplift 13=-186(load case 7), 10=-418(load case 7), 21=-428(load case 6)
Max Grav 13=1282(load case 1), 10=324(load case 11), 21=1132(load case 1)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931662
L255815	T39	SPECIAL	1	1	Job Reference (optional)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-106/178, 2-3=-137/293, 3-4=-330/263, 4-5=-560/425, 5-6=-444/428, 6-7=-497/454, 7-8=-108/408, 8-9=0/368, 9-10=-131/476, 10-11=0/54, 1-22=-130/152

BOT CHORD 21-22=-123/186, 20-21=-170/273, 19-20=-190/343, 18-19=0/0, 17-19=-33/117, 5-17=-131/95, 16-17=-102/372, 15-16=-27/77, 14-15=-699/78, 7-15=-689/89, 13-14=-275/22, 12-13=-326/218, 8-13=-1029/160, 10-12=-270/57

WEBS 2-21=-268/281, 3-21=-887/533, 3-20=-344/569, 4-20=-548/372, 4-19=-105/139, 6-17=-108/169, 6-16=-208/98, 7-16=-110/444, 8-14=-34/739, 9-12=-278/277, 2-22=-379/227

JOINT STRESS INDEX

1 = 0.54, 2 = 0.30, 3 = 0.30, 4 = 0.37, 5 = 0.51, 6 = 0.52, 7 = 0.30, 8 = 0.37, 9 = 0.45, 10 = 0.85, 10 = 0.00, 12 = 0.74, 13 = 0.49, 14 = 0.33, 15 = 0.10, 16 = 0.36, 17 = 0.23, 19 = 0.59, 20 = 0.53, 21 = 0.26 and 22 = 0.39

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 5x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 186 lb uplift at joint 13, 418 lb uplift at joint 10 and 428 lb uplift at joint 21.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931663
L255815	T40	SPECIAL	1	1	Job Reference (optional)

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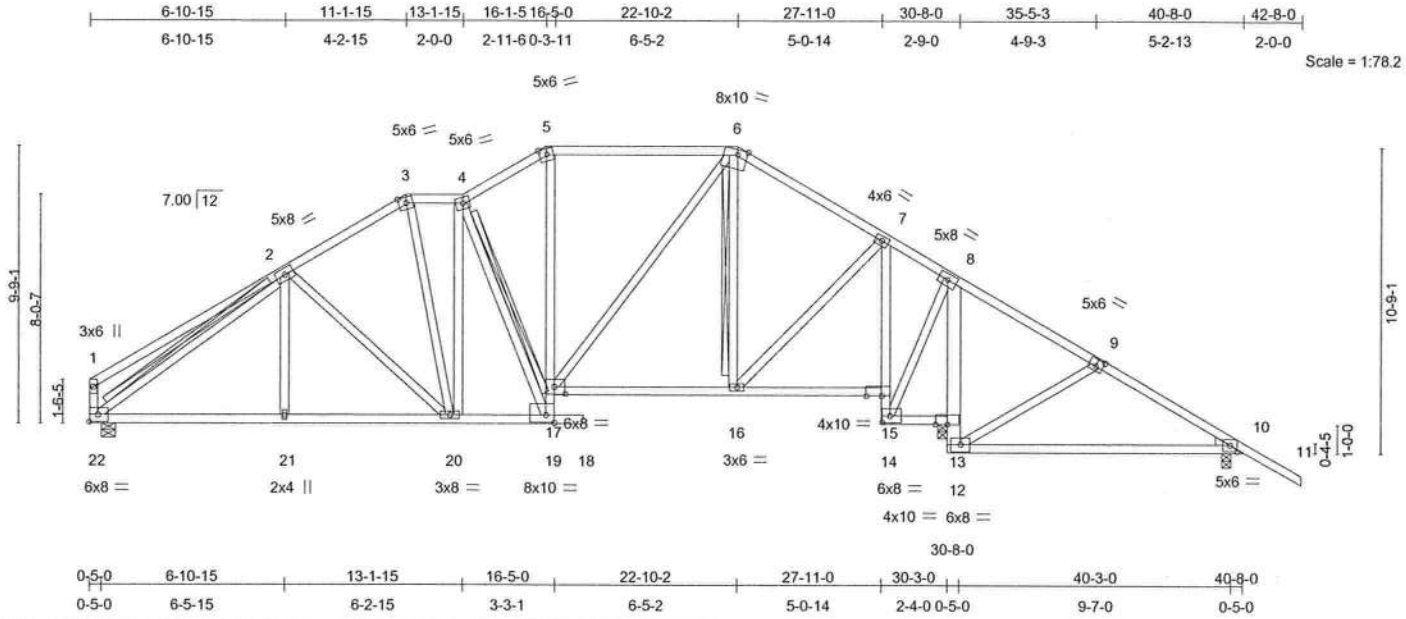

Plate Offsets (X,Y): [6:0-4-1,Edge], [9:0-3-0,0-3-0], [10:0-3-0,0-2-11], [14:0-3-4,0-2-12], [15:0-6-8,Edge], [17:0-4-9,0-3-0], [19:0-3-8,0-3-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.73	Vert(LL)	0.33	10-12	>366	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.29	10-12	>414	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.76	Horz(TL)	-0.15	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 292 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
7-14 2 X 4 SYP No.3, 8-12 2 X 6 SYP No.1D
10-12 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 3-4, 5-6.
BOT CHORD Rigid ceiling directly applied or 5-8-15 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 2-22, 4-19, 6-16
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 13=1594/0-4-0, 10=218/0-4-0, 22=899/0-6-0
Max Horz 22=325(load case 5)
Max Uplift 13=-840(load case 4), 10=-288(load case 7), 22=-556(load case 6)
Max Grav 13=1594(load case 1), 10=279(load case 11), 22=899(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-351/433, 2-3=-935/1127, 3-4=-801/1100, 4-5=-821/1115, 5-6=-730/1044, 6-7=-635/864, 7-8=-74/295, 8-9=-242/483, 9-10=-275/279, 10-11=0/54, 1-22=-320/338
BOT CHORD 21-22=-1029/916, 20-21=-1029/916, 19-20=-867/811, 18-19=0/0, 17-19=-600/434, 5-17=-364/226, 16-17=-383/489, 15-16=-30/46, 14-15=-955/863, 7-15=-947/796, 13-14=-424/486, 12-13=-326/219, 8-13=-1305/1107, 10-12=-216/238
WEBS 2-22=-881/908, 2-21=-236/207, 2-20=-233/350, 3-20=-447/273, 4-20=-189/159,

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1165 Coastal Bay Blvd
Boynton Beach, FL 33435

February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931663
L255815	T40	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

1 = 0.67, 2 = 0.56, 3 = 0.43, 4 = 0.30, 5 = 0.55, 6 = 0.52, 7 = 0.45, 8 = 0.55, 9 = 0.45, 10 = 0.85, 10 = 0.00, 12 = 0.54, 13 = 0.60, 14 = 0.29, 15 = 0.10, 16 = 0.42, 17 = 0.37, 19 = 0.52, 20 = 0.60, 21 = 0.33 and 22 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 840 lb uplift at joint 13, 288 lb uplift at joint 10 and 556 lb uplift at joint 22.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931664
L255815	T41	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:54:28 2008 Page 1

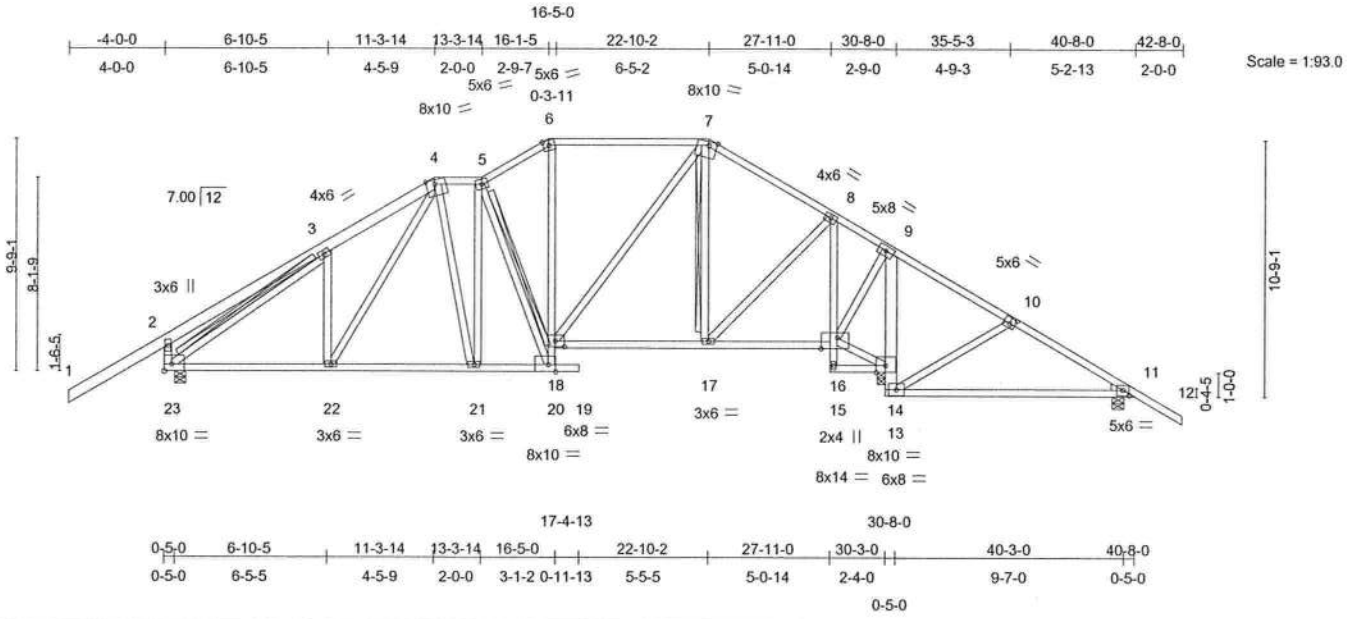

Plate Offsets (X,Y): [4:0-4-1,Edge], [7:0-4-1,Edge], [10:0-3-0,0-3-0], [11:0-3-0,0-2-11], [14:0-4-12,Edge], [16:0-8-5,Edge], [18:0-4-10,0-3-0], [20:0-3-10,Edge], [23:Edge,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	0.29	11-13	>407	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.28	11-13	>431	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.13	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 317 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
1-4 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
8-15 2 X 4 SYP No.3, 9-13 2 X 6 SYP No.1D
11-13 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-5, 6-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-20, 7-17, 3-23
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 14=1567/0-4-0, 11=227/0-6-0, 23=1141/0-6-0
Max Horz 23=284(load case 5)
Max Uplift 14=-256(load case 6), 11=-402(load case 7), 23=-404(load case 6)
Max Grav 14=1567(load case 1), 11=290(load case 11), 23=1141(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/113, 2-3=-187/98, 3-4=-1053/680, 4-5=-765/580, 5-6=-795/613, 6-7=-710/586, 7-8=-626/529, 8-9=-100/368, 9-10=0/470, 10-11=-57/349, 11-12=0/54, 2-23=-476/444
BOT CHORD 22-23=-325/848, 21-22=-263/741, 20-21=-258/772, 19-20=0/0, 18-20=-210/405, 6-18=-98/231, 17-18=-153/481, 16-17=-64/72, 15-16=-70/61, 8-16=-928/249, 14-15=-60/16, 13-14=-320/217, 9-14=-1090/320, 11-13=-215/0
WEBS 3-22=-120/158, 5-20=-282/221, 7-18=-145/417, 7-17=-350/117, 8-17=-131/650,

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
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Boynton Beach, FL 33435

February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931664
L255815	T41	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.76, 3 = 0.40, 4 = 0.18, 5 = 0.30, 6 = 0.51, 7 = 0.52, 8 = 0.32, 9 = 0.41, 10 = 0.44, 11 = 0.85, 11 = 0.00, 13 = 0.53, 14 = 0.31, 15 = 0.47, 16 = 0.31, 17 = 0.41, 18 = 0.35, 20 = 0.54, 21 = 0.36, 22 = 0.43 and 23 = 0.18

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 256 lb uplift at joint 14, 402 lb uplift at joint 11 and 404 lb uplift at joint 23.

LOAD CASE(S) Standard

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Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931665
L255815	T42	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:54:29 2008 Page 1

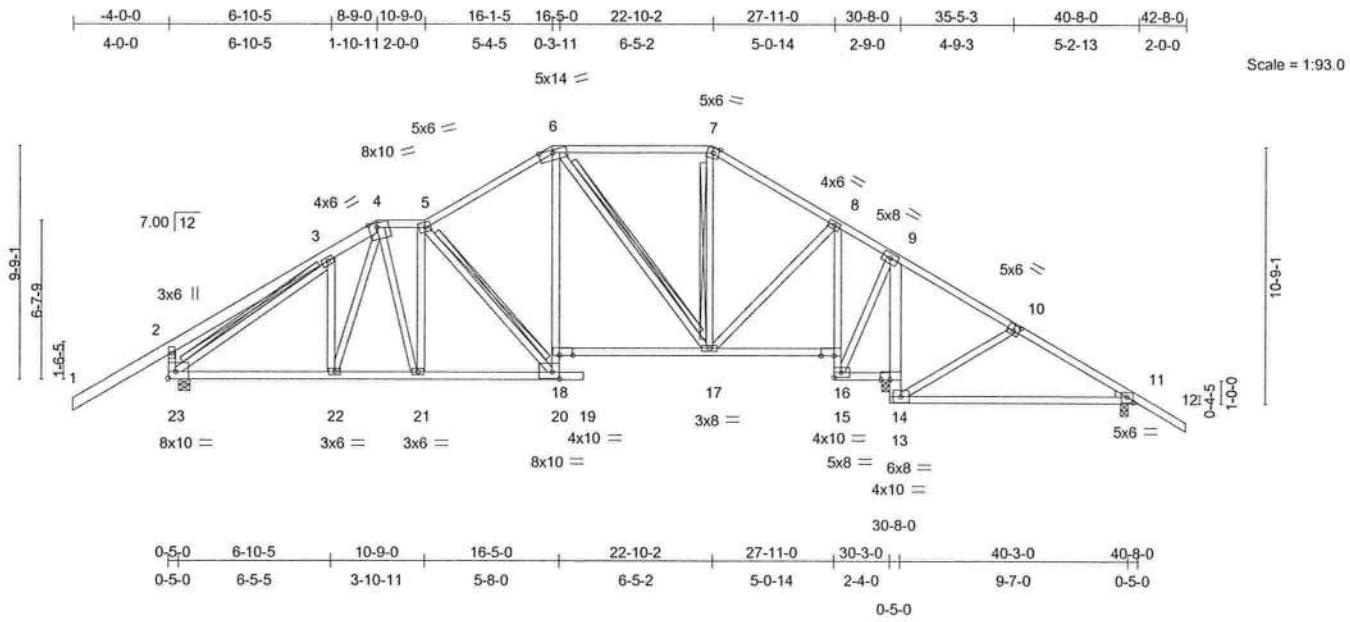

Plate Offsets (X,Y): [4:0-4-1,Edge], [10:0-3-0,0-3-0], [11:0-3-0,0-2-11], [14:0-4-8,0-0-0], [16:0-6-8,Edge], [18:0-6-11,Edge], [20:0-3-9,Edge], [23:Edge,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	0.31	11-13	>388	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.30	19	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.16	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 305 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except* 1-4 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except* 8-15 2 X 4 SYP No.3, 9-13 2 X 6 SYP No.1D 11-13 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-5, 6-7.
BOT CHORD Rigid ceiling directly applied or 5-8-13 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-20, 6-17, 7-17, 3-23
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 14=1600/0-4-0, 11=202/0-4-0, 23=1133/0-6-0
Max Horz 23=284(load case 5)
Max Uplift 14=-273(load case 6), 11=-399(load case 7), 23=-400(load case 6)
Max Grav 14=1600(load case 1), 11=270(load case 11), 23=1133(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/113, 2-3=-204/97, 3-4=-1004/628, 4-5=-872/588, 5-6=-835/587, 6-7=-466/503, 7-8=-603/511, 8-9=-56/331, 9-10=-7/515, 10-11=-41/332, 11-12=0/54, 2-23=-486/451
BOT CHORD 22-23=-313/830, 21-22=-280/791, 20-21=-290/887, 19-20=0/0, 18-20=-176/361, 6-18=-151/421, 17-18=-229/701, 16-17=-48/31, 15-16=-966/258, 8-16=-958/274, 14-15=-454/101, 13-14=-326/219, 9-14=-1298/362, 11-13=-242/39
WEBS 3-22=-126/132, 4-22=-114/169, 4-21=-212/309, 5-21=-337/245, 5-20=-346/255,

Continued on page 2

Julius Lee
Truss Design Engineer
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Boynton Beach, FL 33435

February 4,2008

JOINT STRESS INDEX

2 = 0.74, 3 = 0.39, 4 = 0.21, 5 = 0.39, 6 = 0.80, 7 = 0.49, 8 = 0.34, 9 = 0.42, 10 = 0.46, 11 = 0.85, 11 = 0.00, 13 = 0.54, 14 = 0.60, 15 = 0.39, 16 = 0.10, 17 = 0.66, 18 = 0.58, 20 = 0.47, 21 = 0.37, 22 = 0.49 and 23 = 0.20

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 273 lb uplift at joint 14, 399 lb uplift at joint 11 and 400 lb uplift at joint 23.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
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February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931666
L255815	T43	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:54:31 2008 Page 1

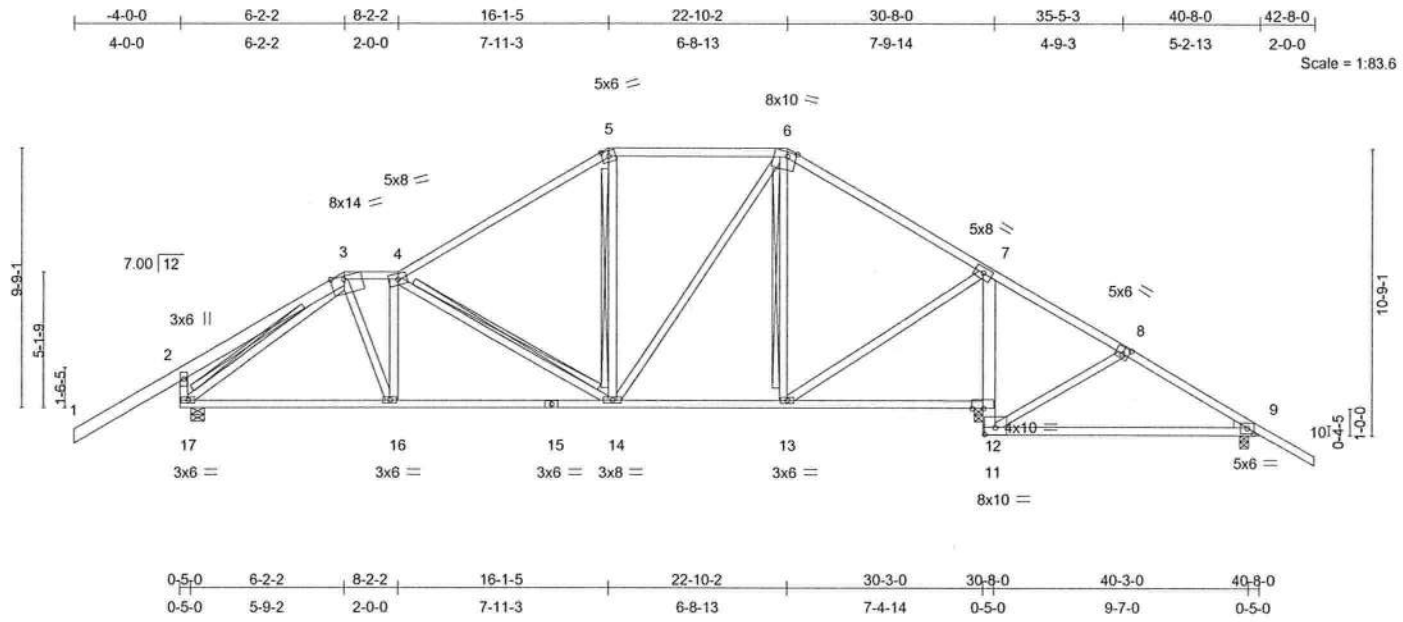

Plate Offsets (X,Y): [3:0-6-1,Edge], [6:0-4-1,Edge], [8:0-3-0,0-3-0], [9:0-3-0,0-2-11], [11:0-4-12,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	0.31	9-11	>387	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.27	9-11	>448	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.03	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 269 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except* 1-3 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except* 7-11 2 X 6 SYP No.1D, 9-11 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-10-8 oc purlins, except end verticals, and 2-0-0 oc purlins (5-2-13 max.): 3-4, 5-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-14, 5-14, 6-13, 3-17
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 9=384/0-4-0, 17=1188/0-6-0, 12=1352/0-4-0
Max Horz 17=284(load case 5)
Max Uplift 9=-400(load case 7), 17=-416(load case 6), 12=-243(load case 7)
Max Grav 9=408(load case 11), 17=1188(load case 1), 12=1352(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/113, 2-3=-214/96, 3-4=-1118/662, 4-5=-968/620, 5-6=-744/621, 6-7=-748/560, 7-8=-35/362, 8-9=-290/463, 9-10=0/54, 2-17=-485/455
BOT CHORD 16-17=-341/866, 15-16=-419/1137, 14-15=-419/1137, 13-14=-170/548, 12-13=-97/5, 11-12=-318/219, 7-12=-1118/343, 9-11=-259/191
WEBS 3-16=-433/700, 4-16=-575/455, 4-14=-458/305, 5-14=-47/204, 6-14=-154/406, 6-13=-334/91, 7-13=-88/733, 8-11=-264/263, 3-17=-1064/504

Julius Lee
Truss Design Engineer
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1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 4,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931666
L255815	T43	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.74, 3 = 0.37, 4 = 0.77, 5 = 0.73, 6 = 0.72, 7 = 0.36, 8 = 0.41, 9 = 0.85, 9 = 0.00, 11 = 0.23, 12 = 0.49, 13 = 0.42, 14 = 0.57, 15 = 0.41, 16 = 0.62 and 17 = 0.67

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 400 lb uplift at joint 9, 416 lb uplift at joint 17 and 243 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931667
L255815	T44	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:54:32 2008 Page 1

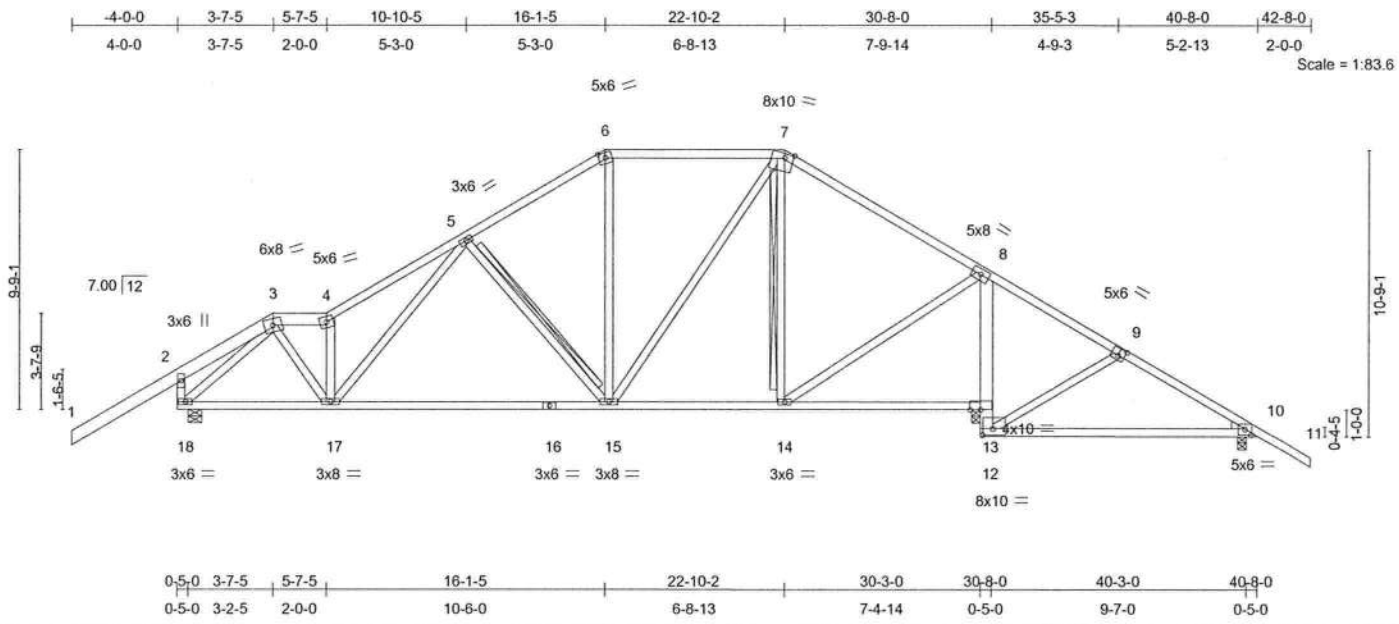

Plate Offsets (X,Y): [7:0-4-1,Edge], [9:0-3-0,0-3-0], [10:0-3-0,0-2-11], [12:0-4-8,0-3-0], [13:0-4-12,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	0.31	10-12	>387	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.46	15-17	>799	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.03	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 270 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*
1-3 2 X 6 SYP No.1D, 3-4 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
8-12 2 X 6 SYP No.1D, 10-12 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-0-15 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 3-4, 6-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-15, 7-14
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 10=379/0-4-0, 18=1186/0-6-0, 13=1358/0-4-0
Max Horz 18=284(load case 5)
Max Uplift 10=-400(load case 7), 18=-416(load case 6), 13=-244(load case 7)
Max Grav 10=405(load case 11), 18=1186(load case 1), 13=1358(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/113, 2-3=-66/107, 3-4=-1244/644, 4-5=-1463/806, 5-6=-918/636, 6-7=-734/607, 7-8=-744/560, 8-9=-21/360, 9-10=-285/462, 10-11=0/54, 2-18=-421/474
BOT CHORD 17-18=-335/748, 16-17=-347/977, 15-16=-347/977, 14-15=-171/547, 13-14=-102/0, 12-13=-318/219, 8-13=-1122/344, 10-12=-257/187
WEBS 3-17=-516/899, 4-17=-945/606, 5-17=-152/404, 5-15=-378/290, 6-15=-108/232, 7-15=-141/393, 7-14=-348/91, 8-14=-95/744, 9-12=-264/264, 3-18=-1091/412

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 4,2008

Continued on page 2

JOINT STRESS INDEX

2 = 0.74, 3 = 0.30, 4 = 0.60, 5 = 0.40, 6 = 0.50, 7 = 0.73, 8 = 0.33, 9 = 0.41, 10 = 0.85, 10 = 0.00, 12 = 0.25, 13 = 0.49, 14 = 0.42, 15 = 0.57, 16 = 0.35, 17 = 0.90 and 18 = 0.41

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) -4-0-0 to 42-8-0 zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 400 lb uplift at joint 10, 416 lb uplift at joint 18 and 244 lb uplift at joint 13.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34169
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 4,2008

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931668
L255815	T45	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Feb 01 12:09:18 2008 Page 1

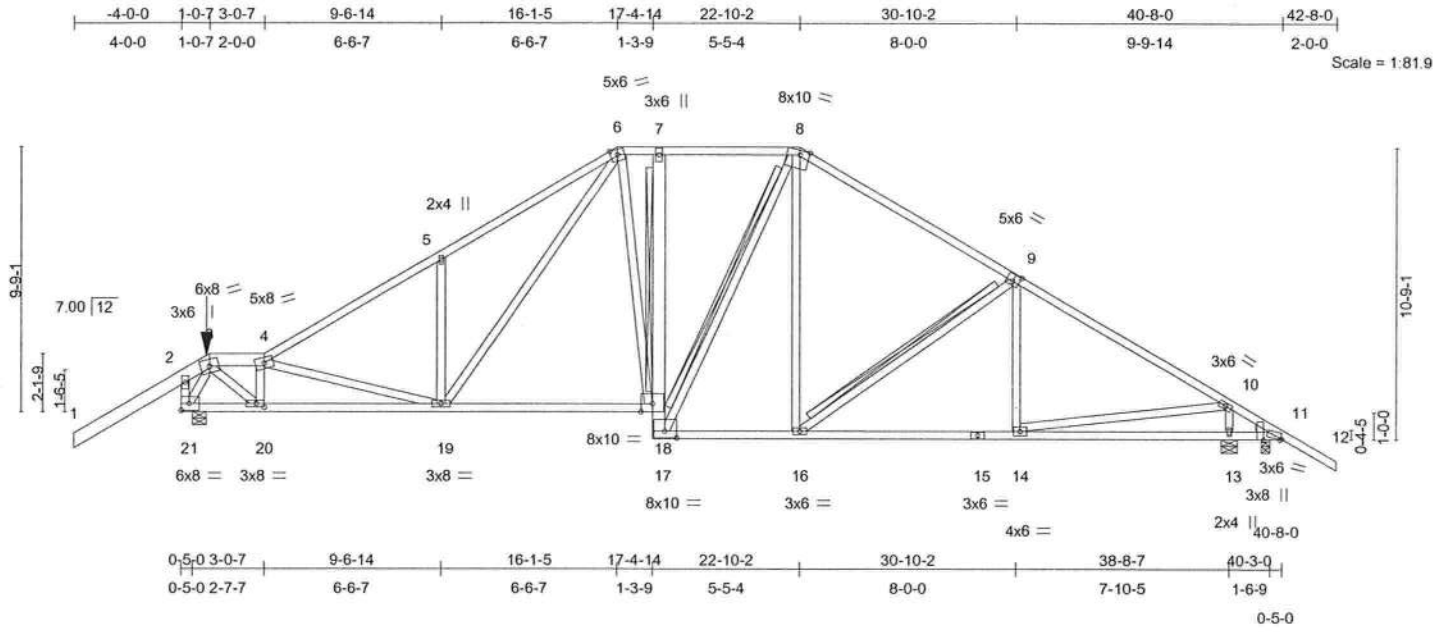

Plate Offsets (X,Y): [8:0-4-1,Edge], [9:0-3-0,0-3-4], [11:0-0-4,Edge], [11:0-0-11,Edge], [17:0-5-0,0-3-0], [20:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.20	18-19	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.43	18-19	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.89	Horz(TL)	0.17	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 295 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*
1-3 2 X 6 SYP No.1D, 3-4 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
7-17 2 X 6 SYP No.1D, 15-17 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-4-8 oc purlins, except end verticals, and 2-0-0 oc purlins (5-3-6 max.): 3-4, 6-8.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
Except:
T-Brace: 2 X 4 SYP No.3 - 7-18
WEBS T-Brace: 2 X 4 SYP No.3 - 8-17, 9-16
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 11=41/0-4-0, 21=1465/0-6-0, 13=1423/0-7-2
Max Horz 21=-258(load case 4)
Max Uplift 11=-181(load case 7), 21=-438(load case 6), 13=-237(load case 7)
Max Grav 11=78(load case 11), 21=1465(load case 1), 13=1423(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/113, 2-3=-249/363, 3-4=-1570/648, 4-5=-1867/846, 5-6=-1874/1045, 6-7=-1203/749, 7-8=-1115/738, 8-9=-1364/760, 9-10=-1642/776, 10-11=-26/158, 11-12=0/54, 2-21=-727/893
BOT CHORD 20-21=-219/355, 19-20=-505/1663, 18-19=-224/1177, 17-18=-134/168, 7-18=-183/100, 16-17=-211/1085, 15-16=-434/1329, 14-15=-434/1329, 13-14=-101/52, 11-13=-101/52
WEBS 3-20=-856/1672, 4-20=-1133/616, 4-19=-145/91, 5-19=-367/337, 8-17=-187/239, 8-16=-105/268, 9-16=-334/278, 9-14=-119/123, 3-21=-796/77, 6-18=-180/359, 6-19=-381/631, 10-13=-1402/691, 10-14=-490/1395

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Builders FirstSource

JOINT STRESS INDEX

2 = 0.55, 3 = 0.48, 4 = 0.76, 5 = 0.34, 6 = 0.57, 7 = 0.51, 8 = 0.71, 9 = 0.76, 10 = 0.70, 11 = 0.76, 11 = 0.35, 13 = 0.51, 14 = 0.59, 15 = 0.44, 16 = 0.35, 17 = 0.30, 18 = 0.37, 19 = 0.65, 20 = 0.88 and 21 = 0.25

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) -4-0-0 to 42-8-0 zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 11, 438 lb uplift at joint 21 and 237 lb uplift at joint 13.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 2-3=-54, 3-4=-54, 4-6=-54, 6-8=-54, 8-12=-54, 18-21=-10, 11-17=-10
   Concentrated Loads (lb)
   Vert: 3=-5(F)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931669
L255815	T47	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:54:35 2008 Page 1

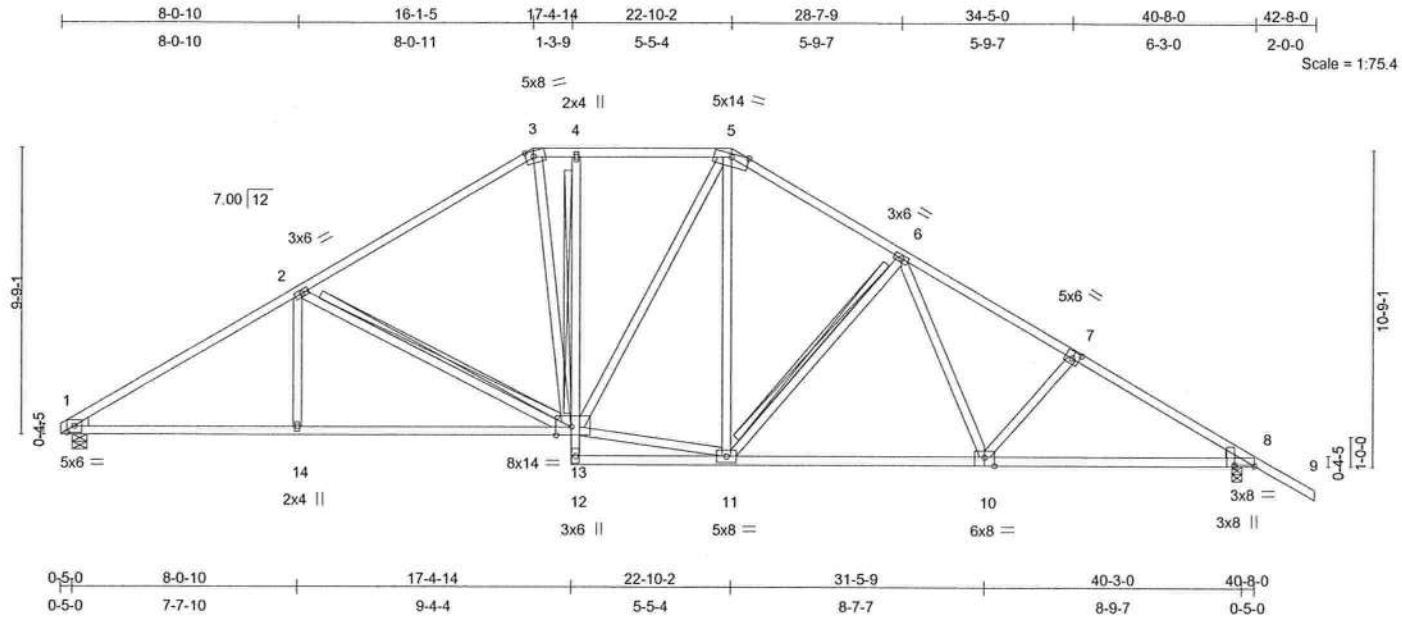

Plate Offsets (X,Y): [1:0-3-0,0-2-11], [3:0-3-4,Edge], [7:0-3-0,0-3-0], [8:0-8-0,0-0-10], [10:0-4-0,Edge], [13:0-6-4 ,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	-0.17	13-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.37	13-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 254 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 4-12 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-12 oc purlins, except 2-0-0 oc purlins (4-8-2 max.): 3-5.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 4-13
WEBS T-Brace: 2 X 4 SYP No.3 - 2-13, 6-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 1=1285/0-6-0, 8=1408/0-4-0
Max Horz 1=-310(load case 4)
Max Uplift 1=-255(load case 6), 8=-365(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2192/999, 2-3=-1610/838, 3-4=-1346/824, 4-5=-1345/827, 5-6=-1501/860, 6-7=-1988/993, 7-8=-2195/1005, 8-9=0/54
BOT CHORD 1-14=-626/1791, 13-14=-626/1791, 12-13=0/57, 4-13=-142/148, 11-12=-122/8, 10-11=-493/1530, 8-10=-689/1808
WEBS 2-14=0/288, 2-13=-566/389, 3-13=-154/460, 11-13=-233/1246, 5-13=-193/356, 5-11=-171/257, 6-11=-472/340, 6-10=-127/373, 7-10=-257/233

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931669
L255815	T47	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

1 = 0.85, 1 = 0.00, 2 = 0.40, 3 = 0.91, 4 = 0.33, 5 = 0.94, 6 = 0.41, 7 = 0.46, 8 = 0.72, 8 = 0.26, 10 = 0.47, 11 = 0.57, 12 = 0.15 , 13 = 0.25 and 14 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 255 lb uplift at joint 1 and 365 lb uplift at joint 8.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931670
L255815	T48	SPECIAL	1	1	Job Reference (optional)

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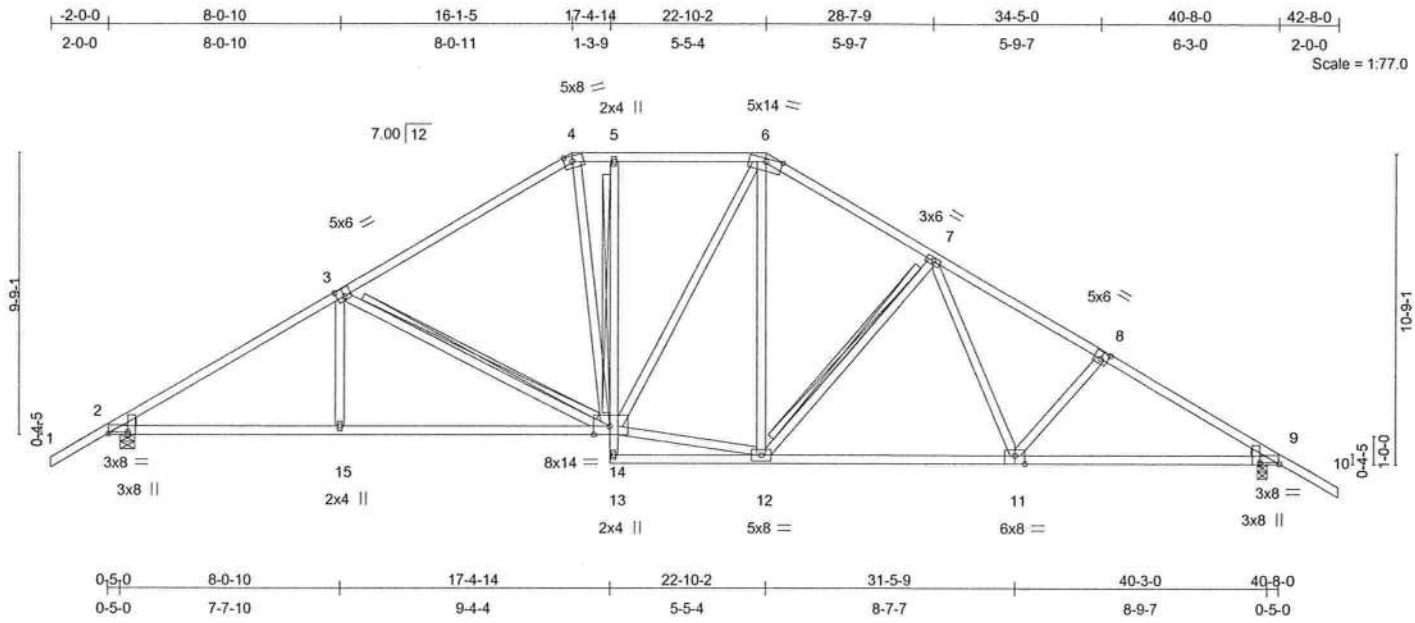

Plate Offsets (X,Y): [2:0-0-4,Edge], [2:0-8-0,0-0-10], [3:0-3-0,0-3-4], [4:0-3-4,Edge], [8:0-3-0,0-3-0], [9:0-8-0,0-0-10], [9:0-0-4 ,Edge], [11:0-4-0,Edge], [14:0-6-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.17	14-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.37	14-15	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 257 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
5-13 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-0 oc purlins, except
2-0-0 oc purlins (4-8-4 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 5-14
WEBS T-Brace: 2 X 4 SYP No.3 - 3-14, 7-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1410/0-6-0, 9=1405/0-4-0
Max Horz 2=-287(load case 4)
Max Uplift 2=-353(load case 6), 9=-364(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-2160/955, 3-4=-1600/827, 4-5=-1338/814, 5-6=-1338/816, 6-7=-1494/851, 7-8=-1981/984, 8-9=-2189/996, 9-10=0/54
BOT CHORD 2-15=-581/1758, 14-15=-582/1756, 13-14=0/57, 5-14=-142/146, 12-13=-124/0, 11-12=-485/1524, 9-11=-681/1803
WEBS 3-15=0/285, 3-14=-536/351, 4-14=-149/460, 12-14=-235/1248, 6-14=-192/353, 6-12=-171/257, 7-12=-472/340, 7-11=-127/373, 8-11=-257/233

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931670
L255815	T48	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.71, 2 = 0.29, 3 = 0.73, 4 = 0.91, 5 = 0.33, 6 = 0.94, 7 = 0.41, 8 = 0.46, 9 = 0.71, 9 = 0.26, 11 = 0.47, 12 = 0.57, 13 = 0.43 , 14 = 0.23 and 15 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 353 lb uplift at joint 2 and 364 lb uplift at joint 9.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931671
L255815	T49	HIP	1	1	Job Reference (optional)

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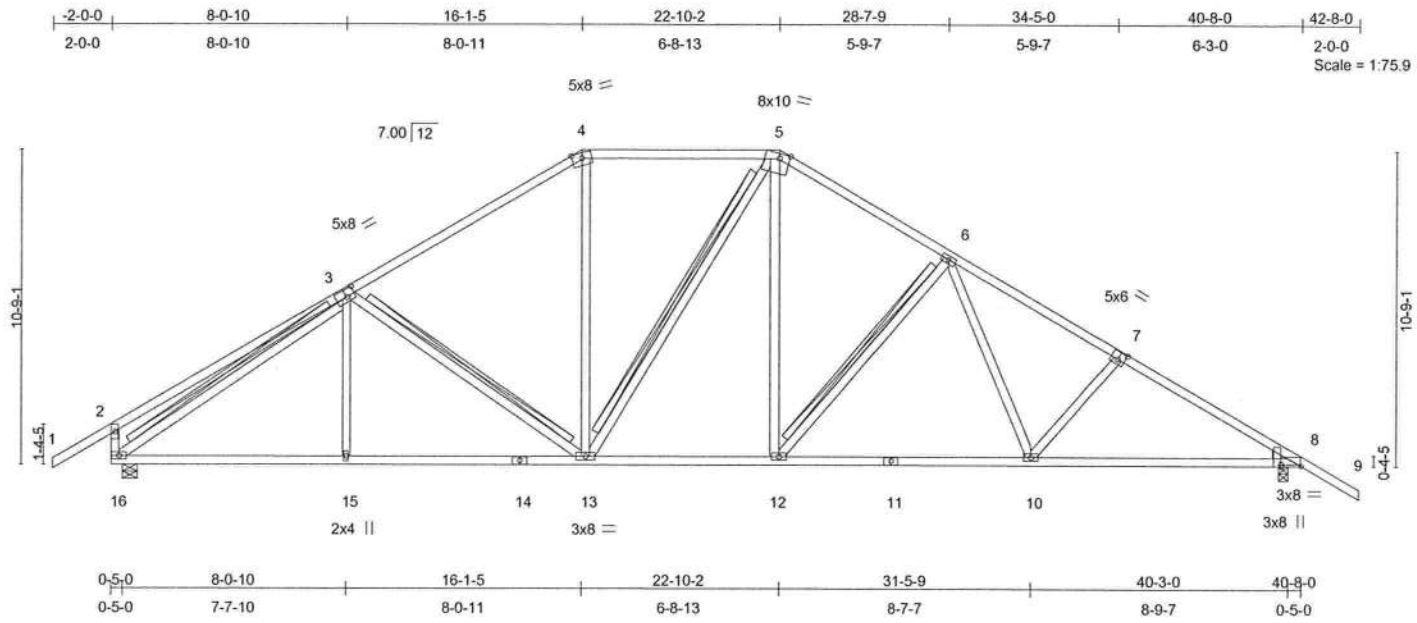

Plate Offsets (X,Y): [3:0-3-12,0-3-0], [5:0-4-1,Edge], [7:0-3-0,0-3-0], [8:0-8-0,0-0-10], [8:0-0-4,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	-0.14	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.29	10-12	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.80	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 253 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-11 oc purlins, except end verticals, and 2-0-0 oc purlins (5-4-8 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 7-7-10 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-16, 3-13, 5-13, 6-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 16=1407/0-6-0, 8=1408/0-4-0
Max Horz 16=-303(load case 4)
Max Uplift 16=-346(load case 6), 8=-360(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-385/303, 3-4=-1510/821, 4-5=-1212/797, 5-6=-1501/854, 6-7=-1988/987, 7-8=-2196/998, 8-9=0/54, 2-16=-469/403
BOT CHORD 15-16=-492/1463, 14-15=-491/1463, 13-14=-491/1463, 12-13=-272/1235, 11-12=-488/1530, 10-11=-488/1530, 8-10=-684/1809
WEBS 3-16=-1513/576, 3-15=0/245, 3-13=-339/274, 4-13=-121/376, 5-13=-219/179, 5-12=-219/446, 6-12=-469/341, 6-10=-127/373, 7-10=-260/232

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931671
L255815	T49	HIP	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.54, 3 = 0.82, 4 = 0.79, 5 = 0.56, 6 = 0.41, 7 = 0.46, 8 = 0.72, 8 = 0.26, 10 = 0.47, 11 = 0.57, 12 = 0.38, 13 = 0.58, 14 = 0.51, 15 = 0.33 and 16 = 0.75

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 346 lb uplift at joint 16 and 360 lb uplift at joint 8.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931672
L255815	T50	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Feb 01 12:12:55 2008 Page 1

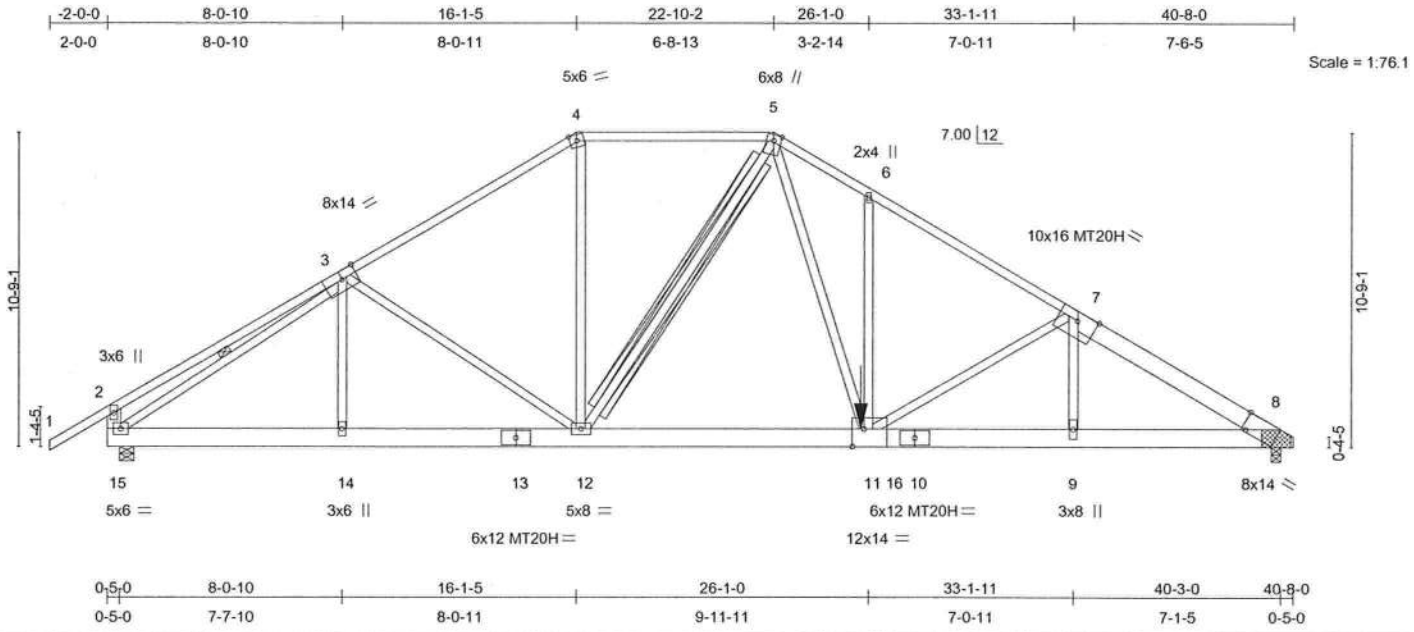

Plate Offsets (X,Y): [3:0-6-4,Edge], [8:0-1-13,Edge], [11:0-4-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.22	9-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.42	9-11	>999	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.96	Horz(TL)	-0.09	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 659 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except* 7-8 2 X 8 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except* 5-11 2 X 4 SYP No.2, 2-15 2 X 6 SYP No.1D

BRACING

TOP CHORD Structural wood sheathing directly applied, except 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 3-15
I-Brace: 2 X 4 SYP No.3 - 5-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 8=7139/0-4-3 (0-4-0 + bearing block), 15=3578/0-6-0
Max Horz 8=283(load case 4)
Max Uplift 8=-2294(load case 3), 15=-1002(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 5-6=-9431/3127, 6-7=-9548/3070, 7-8=-11976/3836, 4-5=-4609/1448, 1-2=0/61, 2-3=-95/173, 3-4=-5414/1613
BOT CHORD 14-15=-1176/4477, 13-14=-1175/4477, 12-13=-1175/4477, 11-12=-1964/6008, 11-16=-3518/10310, 10-16=-3518/10310, 9-10=-3518/10310, 8-9=-3493/10236
WEBS 7-9=-707/2096, 7-11=-2542/986, 6-11=-161/167, 5-11=-2489/6990, 5-12=-2772/1130, 4-12=-733/2259, 3-12=-315/385, 3-14=0/202, 2-15=-309/260, 3-15=-5504/1446

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JOINT STRESS INDEX

2 = 0.15, 3 = 0.38, 4 = 0.59, 5 = 0.91, 6 = 0.34, 7 = 0.85, 8 = 0.60, 8 = 0.00, 8 = 0.00, 9 = 0.34, 10 = 0.69, 11 = 0.71, 12 = 0.35, 13 = 0.23, 14 = 0.16 and 15 = 0.65

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES.
L255815	T50	HIP	1	2	J1931672 Job Reference (optional)

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
   Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.
   Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
   Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, Except member 6-11 2 X 4 - 1 row at 0-6-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) 2 X 8 SYP 2400F 2.0E bearing block 12" long at jt. 8 attached to each face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners per block. Bearing is assumed to be SYP.
4) Unbalanced roof live loads have been considered for this design.
5) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
6) Provide adequate drainage to prevent water ponding.
7) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) All plates are MT20 plates unless otherwise indicated.
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2294 lb uplift at joint 8 and 1002 lb uplift at joint 15.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
      Vert: 5-8=-54, 4-5=-54, 1-4=-54, 15-16=-10, 8-16=-274(F=-264)
   Concentrated Loads (lb)
      Vert: 11=-4548(F)

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931673
L255815	T51	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 01 09:54:39 2008 Page 1

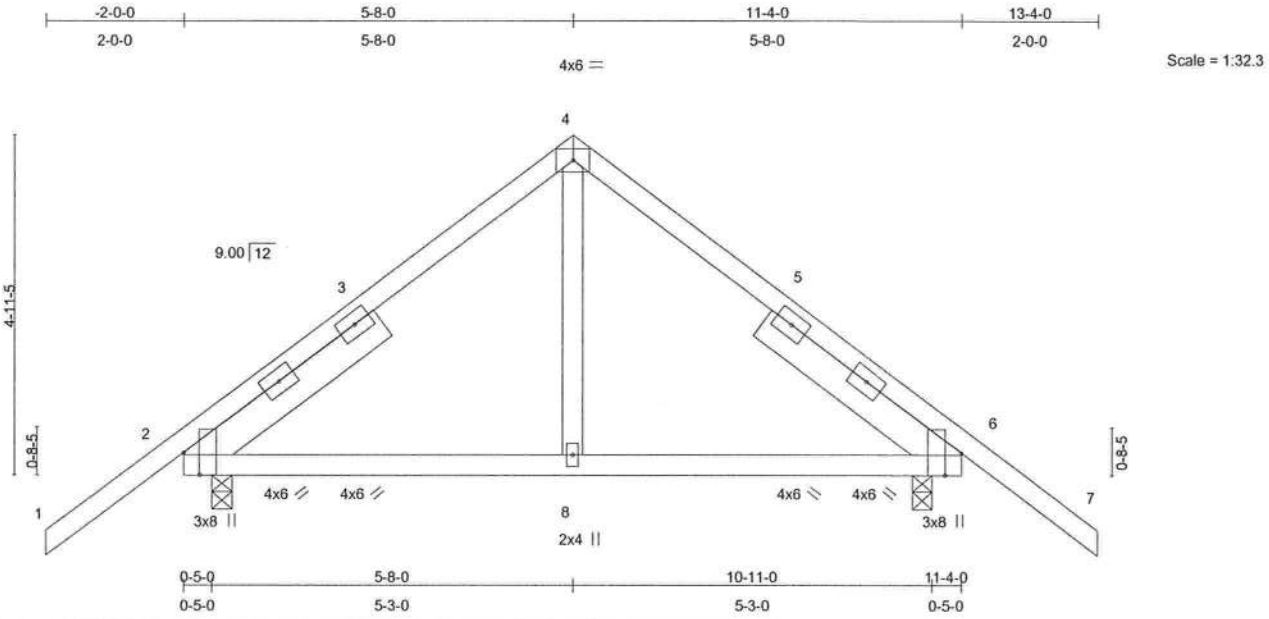

Plate Offsets (X,Y): [2:0-3-15,Edge], [6:0-3-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.05	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.03	2-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.12	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 69 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 6 SYP No.1D 3-5-11, Right 2 X 6 SYP No.1D 3-5-11

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=471/0-3-8, 6=471/0-3-8
Max Horz 2=-126(load case 4)
Max Uplift 2=-308(load case 6), 6=-308(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-401/451, 3-4=-292/470, 4-5=-292/470, 5-6=-401/451, 6-7=0/53
BOT CHORD	2-8=-171/234, 6-8=-171/234
WEBS	4-8=-331/177

JOINT STRESS INDEX

2 = 0.51, 2 = 0.08, 2 = 0.08, 3 = 0.00, 4 = 0.52, 5 = 0.00, 6 = 0.51, 6 = 0.08, 6 = 0.08 and 8 = 0.13

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931673
L255815	T51	COMMON	2	1	Job Reference (optional)

NOTES

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 2 and 308 lb uplift at joint 6.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931674
L255815	T51G	GABLE	1	1	Job Reference (optional)

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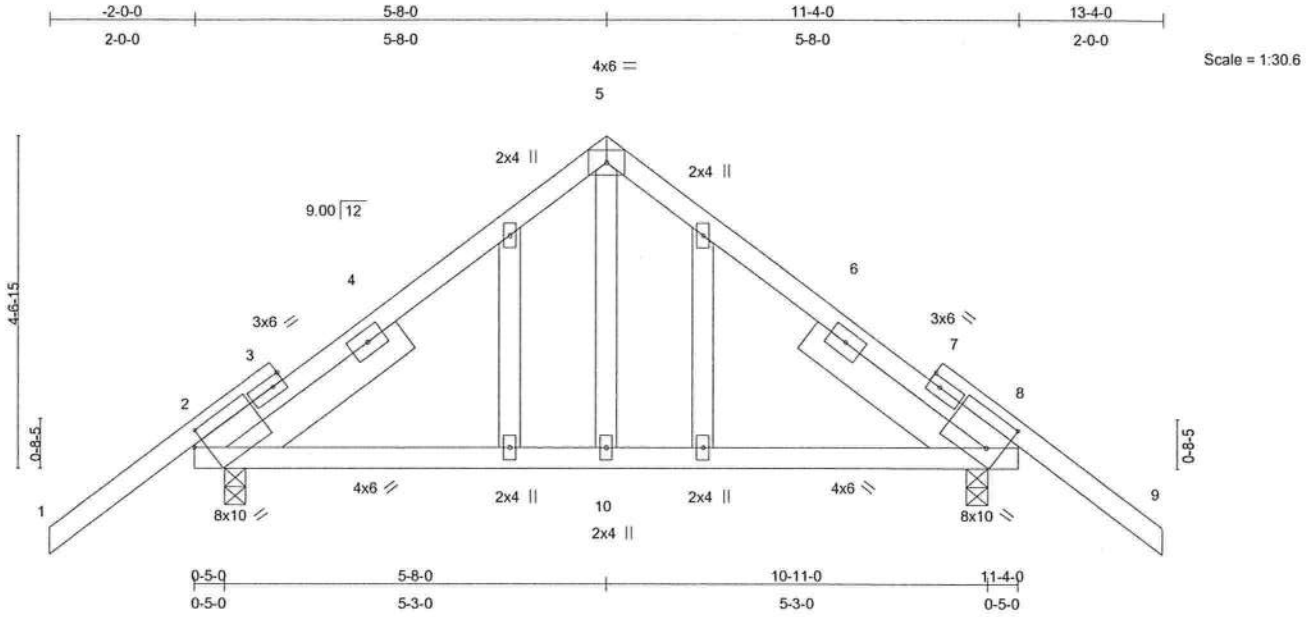

Plate Offsets (X,Y): [2:0-1-12,0-2-5], [8:0-2-7,0-5-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.04	2-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.02	2-10	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.09	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 78 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3
SLIDER	Left 2 X 6 SYP No.1D 2-10-14, Right 2 X 6 SYP No.1D 2-10-14

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=543/0-3-8, 8=543/0-3-8
Max Horz 2=-147(load case 4)
Max Uplift 2=-487(load case 6), 8=-487(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-7/76, 2-3=-396/458, 3-4=-366/455, 4-5=-284/452, 5-6=-284/452, 6-7=-366/455, 7-8=-396/458, 8-9=-7/76
BOT CHORD	2-10=-173/228, 8-10=-173/228
WEBS	5-10=-288/150

JOINT STRESS INDEX

2 = 0.47, 2 = 0.02, 2 = 0.00, 3 = 0.00, 3 = 0.04, 4 = 0.00, 5 = 0.49, 6 = 0.00, 7 = 0.00, 7 = 0.04, 8 = 0.47, 8 = 0.02, 8 = 0.00, 10 = 0.12, 11 = 0.00, 12 = 0.00, 13 = 0.00 and 14 = 0.00

NOTES

1) Unbalanced roof live loads have been considered for this design.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931674
L255815	T51G	GABLE	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable studs spaced at 1-4-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 487 lb uplift at joint 2 and 487 lb uplift at joint 8.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Gable truss supports 1' 0" max. rake gable overhang.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-64(F=-10), 5-9=-64(F=-10), 2-8=-10

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931675
L255815	T52	COMMON	1	1	Job Reference (optional)

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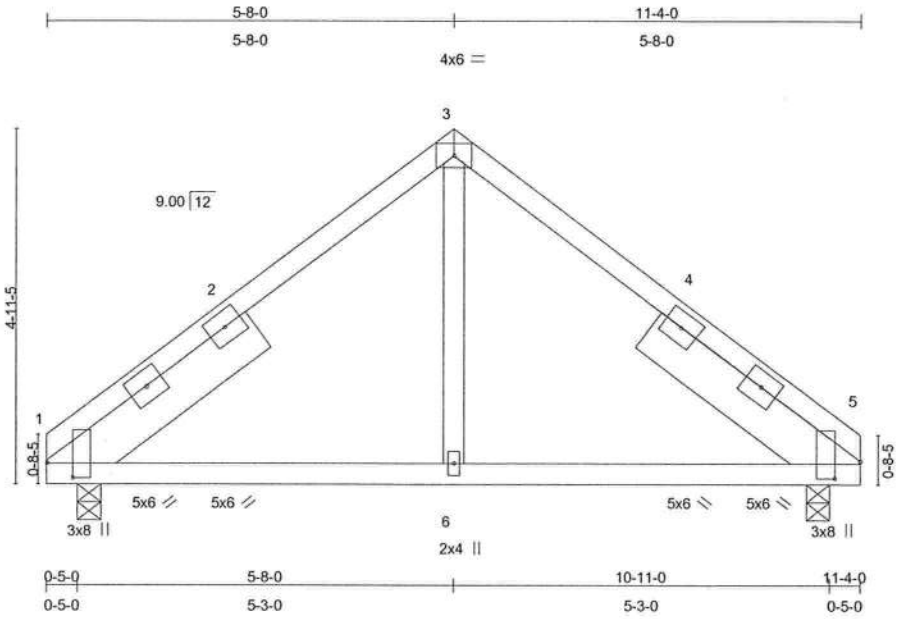

Plate Offsets (X,Y): [1:0-2-8,0-4-4], [5:0-2-15,0-4-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	0.05	1-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.03	1-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.13	Horz(TL)	-0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 66 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 3-5-11, Right 2 X 8 SYP No.1D 3-5-11

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=363/0-4-0, 5=363/0-4-0
Max Horz 1=-128(load case 4)
Max Uplift 1=-215(load case 6), 5=-215(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-407/487, 2-3=-330/511, 3-4=-330/511, 4-5=-407/487
BOT CHORD	1-6=-297/264, 5-6=-297/264
WEBS	3-6=-342/178

JOINT STRESS INDEX

1 = 0.71, 1 = 0.08, 1 = 0.08, 2 = 0.00, 3 = 0.65, 4 = 0.00, 5 = 0.71, 5 = 0.08, 5 = 0.08 and 6 = 0.13

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931675
L255815	T52	COMMON	1	1	Job Reference (optional)

NOTES

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 215 lb uplift at joint 1 and 215 lb uplift at joint 5.

LOAD CASE(S) Standard

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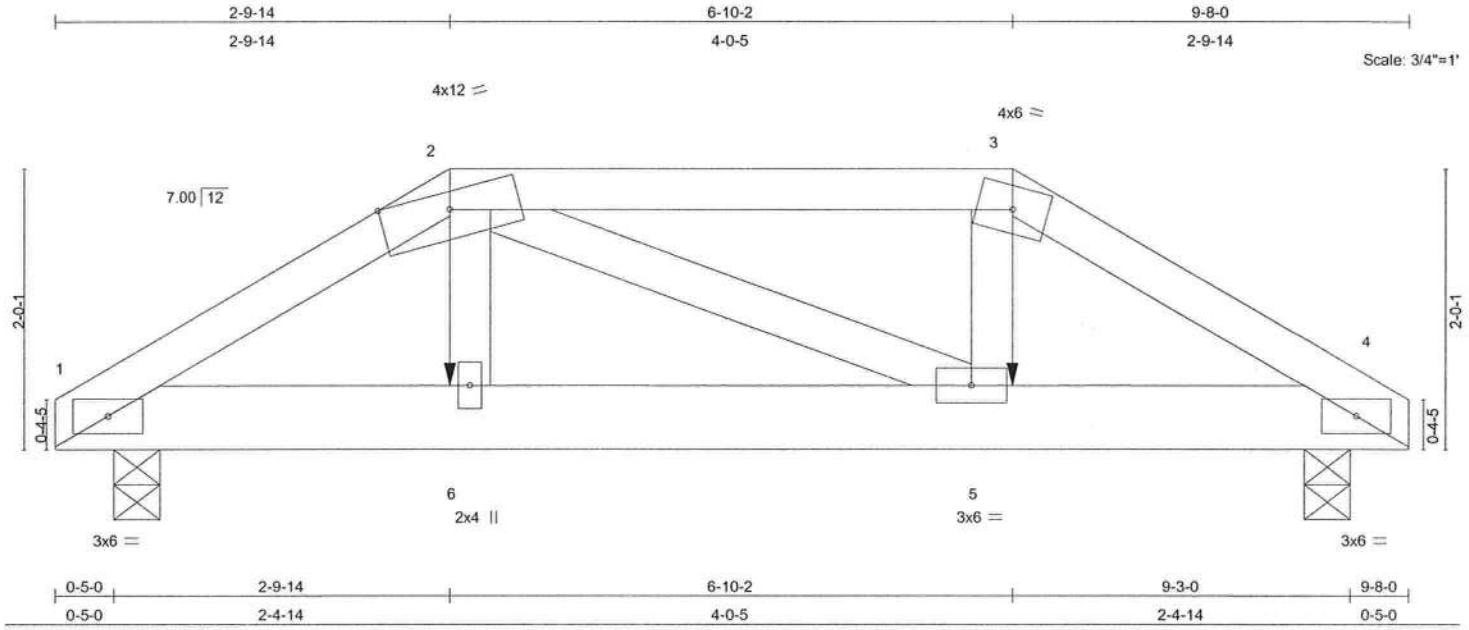
Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931676
L255815	T53	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Feb 04 09:06:29 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.10	Vert(LL)	-0.02	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.04	5-6	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.15	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 6 SYP No.1D
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=1505/0-4-0, 4=1505/0-4-0
Max Horz 1=45(load case 4)
Max Uplift 1=-516(load case 4), 4=-516(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-2236/778, 2-3=-1966/709, 3-4=-2221/772
BOT CHORD	1-6=-690/1897, 5-6=-718/1981, 4-5=-644/1883
WEBS	2-6=-321/943, 2-5=-60/49, 3-5=-321/942

JOINT STRESS INDEX

1 = 0.47, 2 = 0.39, 3 = 0.27, 4 = 0.47, 5 = 0.31 and 6 = 0.34

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
   Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
   Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
   Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.

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February 4,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	LAWANDA RENTZ & TONY AUSTIN RES. J1931676
L255815	T53	HIP	1	2	Job Reference (optional)

NOTES

6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 516 lb uplift at joint 1 and 516 lb uplift at joint 4.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 2-3=-61(F=-7), 3-4=-54, 1-6=-257(B=-247), 5-6=-258(F=-1, B=-247), 4-5=-257(B=-247)
   Concentrated Loads (lb)
   Vert: 6=-40(F) 5=-40(F)

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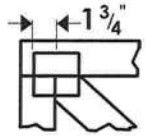
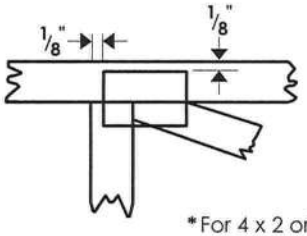
Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

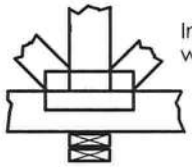
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

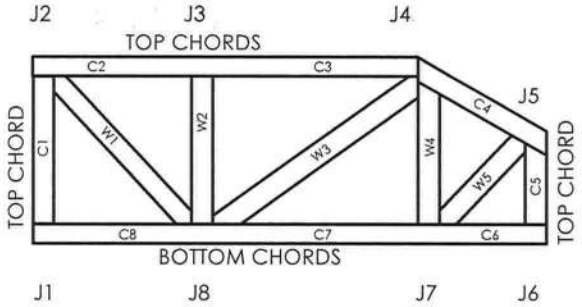
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	5' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 6"	11' 6"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	5' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	8' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	6' 11"	6' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 6"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 5"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 6"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#3
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

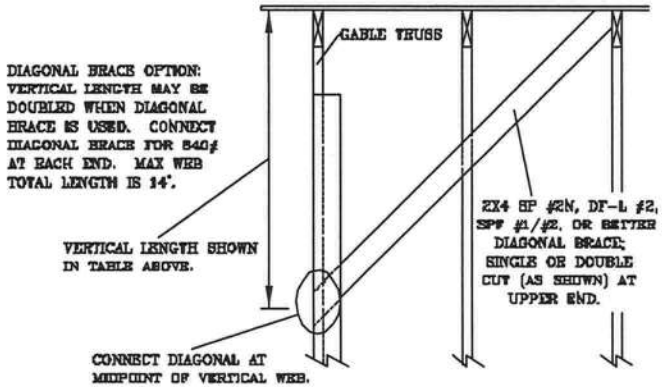
ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

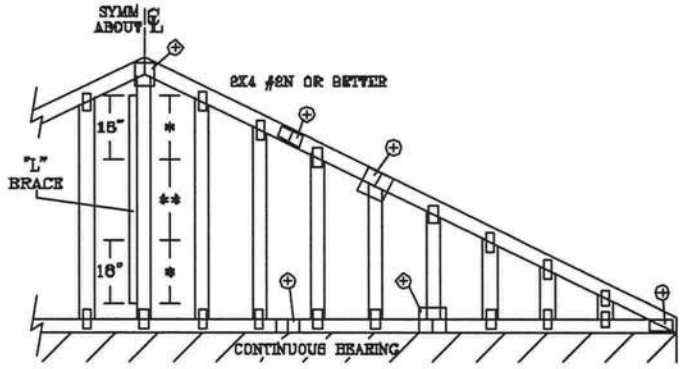
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR, SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161
No: 34869
STATE OF FLORIDA

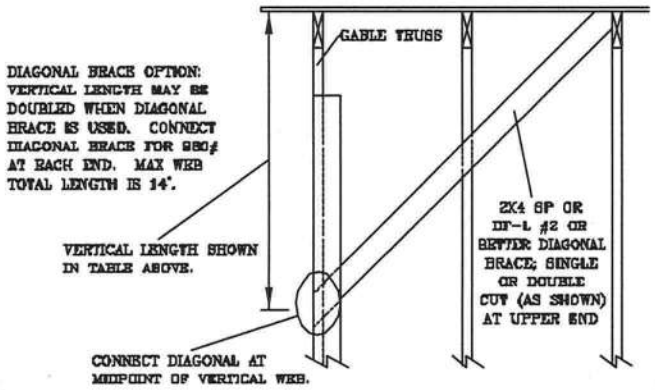
MAX. TOT. LD. 60 PSF

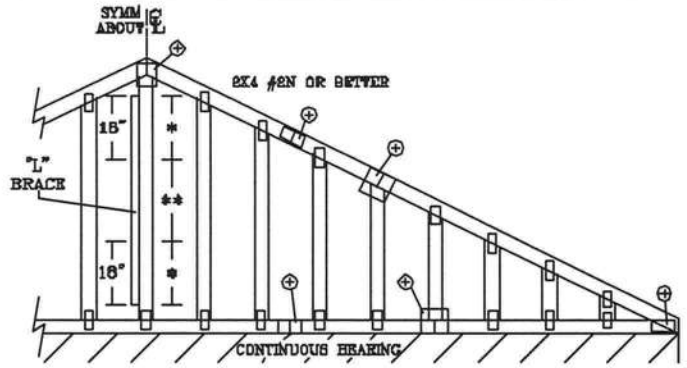
MAX. SPACING 24.0"

REF ASCE7-02-GAB13015
DATE 11/26/03
DRWG MITEK STD GABLE 15 E HT
-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH	2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
					GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
	24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 6"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
			#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
			STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
			STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
		SP DFL	#1	3' 6"	5' 8"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
			#2	3' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
			#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 6"
			STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"
			STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
	16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
			#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
			STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
			STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
		SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
			#2	3' 11"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
			#3	3' 9"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 5"	14' 0"	14' 0"
			STUD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
			STANDARD	3' 6"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
	12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 5"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
			#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
			STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
			STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
		SP DFL	#1	4' 5"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
			#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
			#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
			STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
			STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#3
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	MD SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

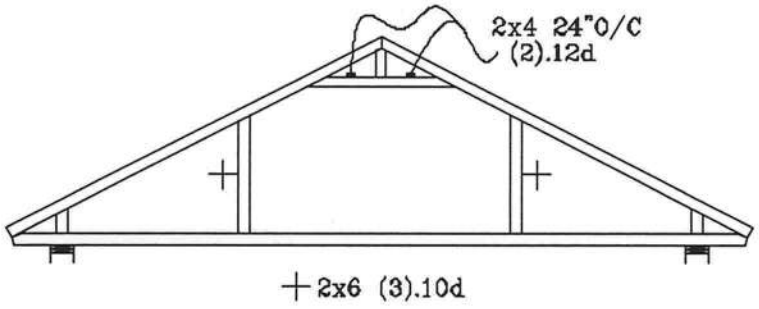
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

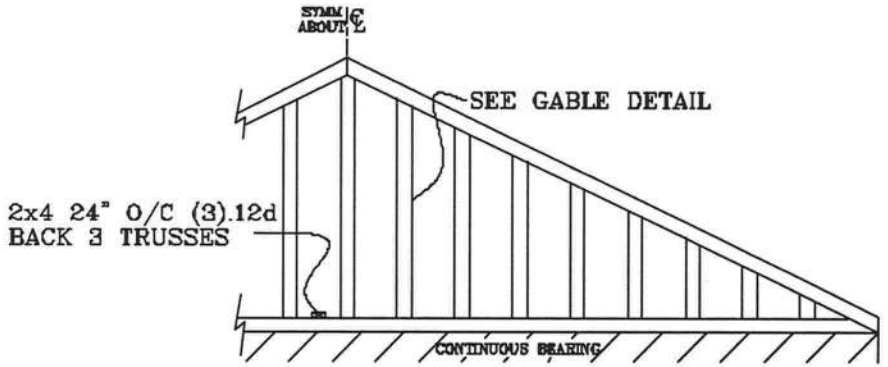
MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
DATE 11/26/03
DWG MTEK STD GABLE 30' E W7
-ENG

TYPICAL ATTIC TRUSS BRACING

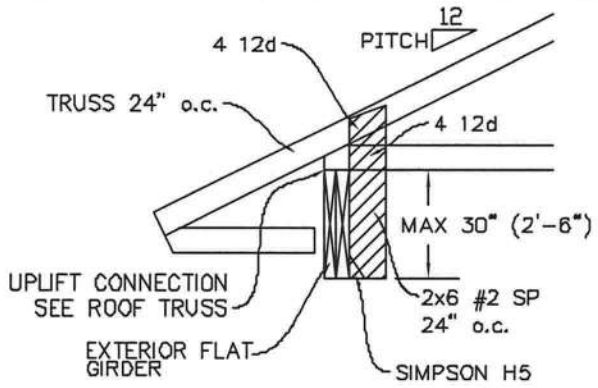


GABLE END TRUSS DETAIL

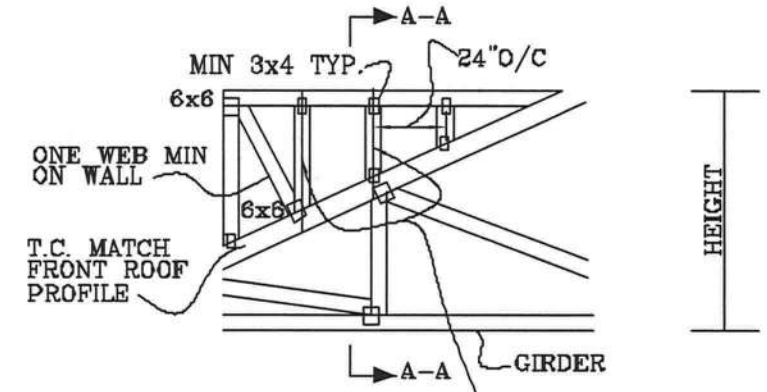


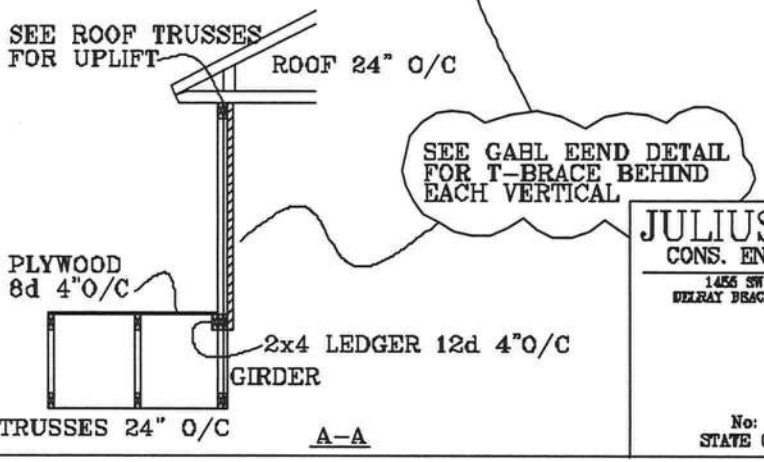
MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

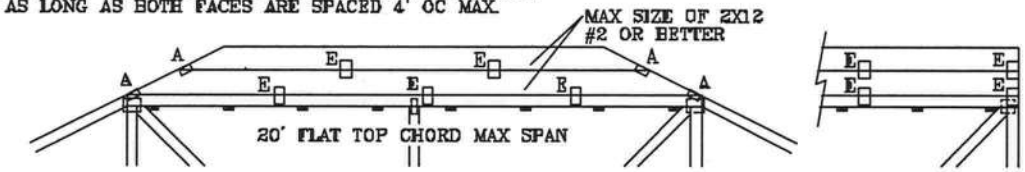
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

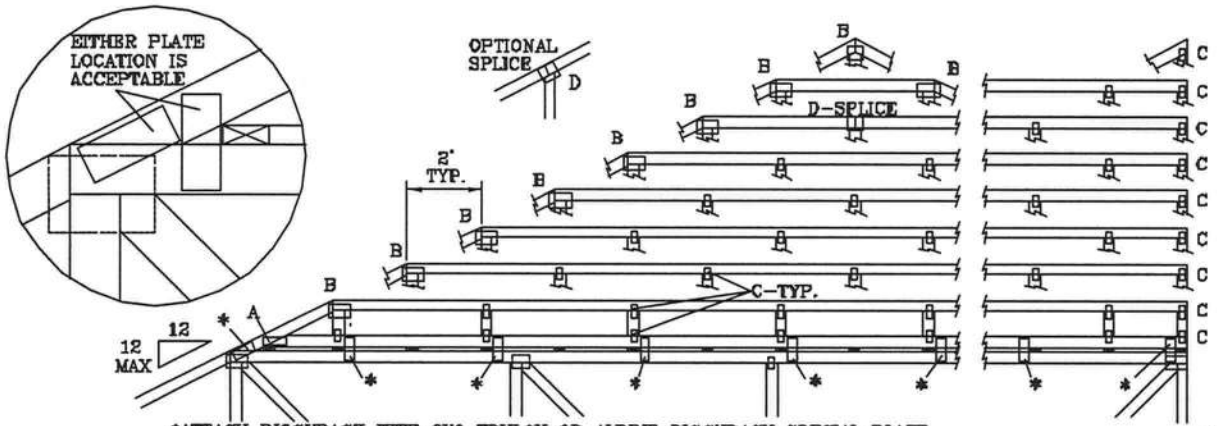
110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.





*ATTACH PIGGYBACK WITH 3X6 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

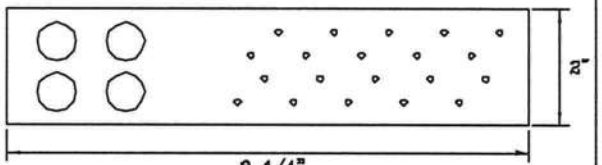
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 647,045

JULIUS LEE'S
CONS. ENGINEERS P.A.
1468 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

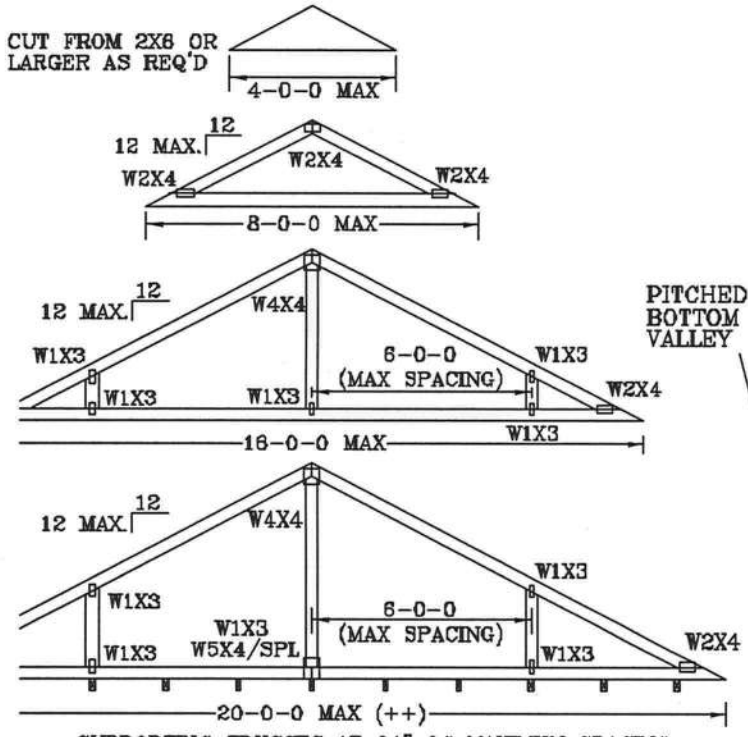
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

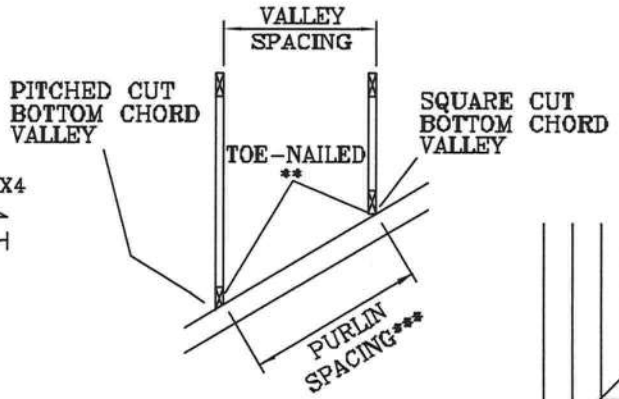
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

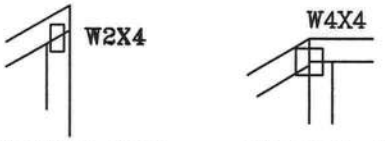
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

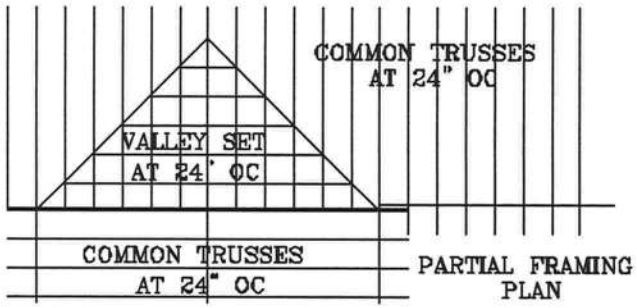
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DURFAC.	1.25	1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

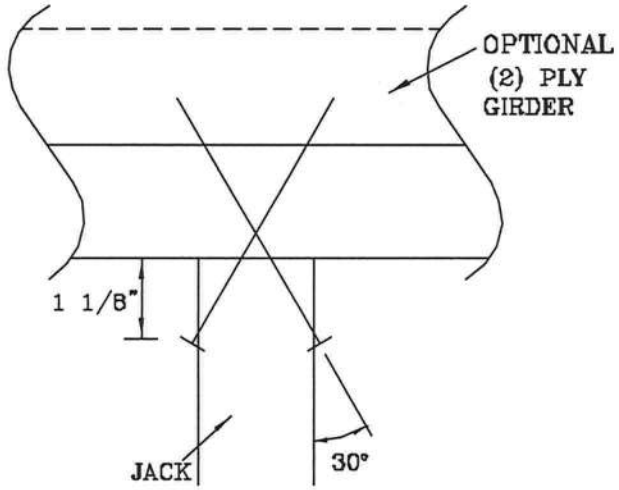
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

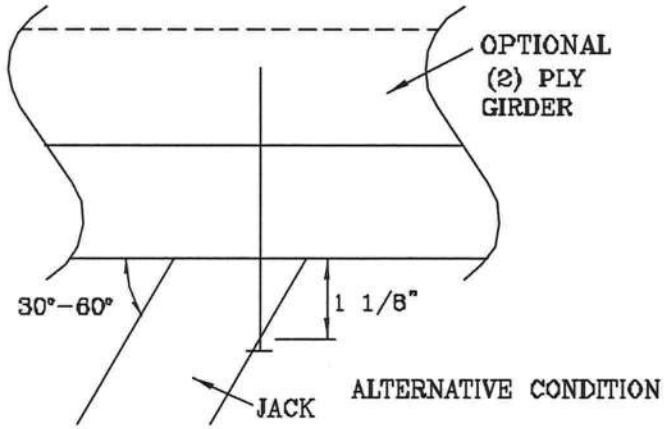
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

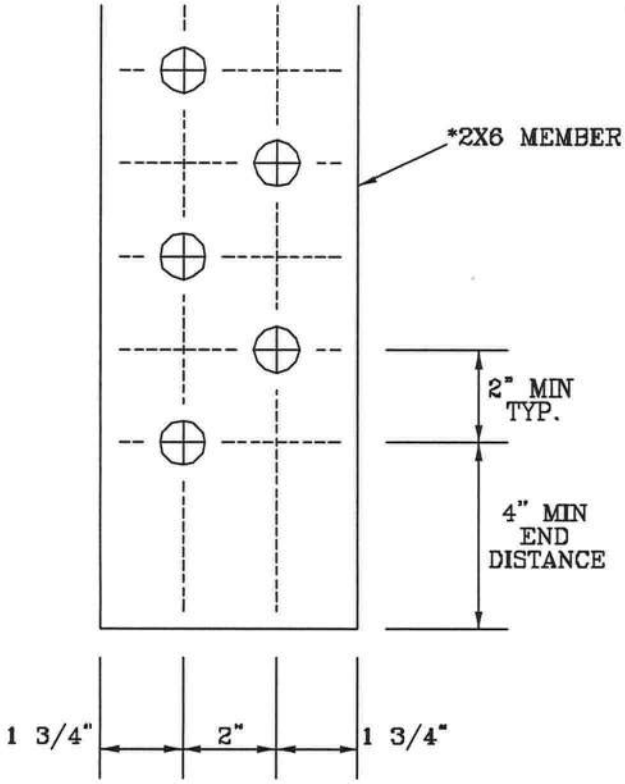
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

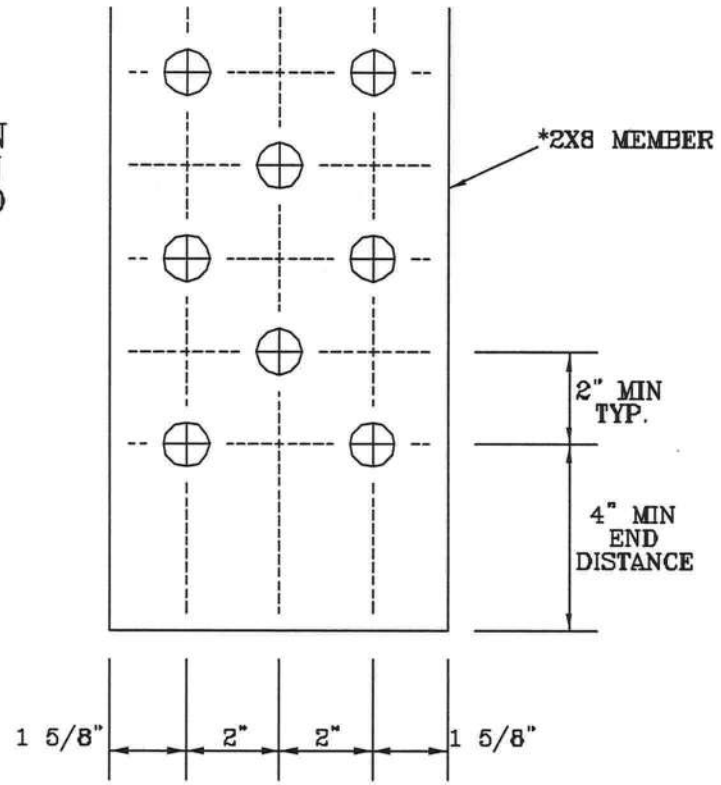
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A626.016

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1450 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF BOLT SPACING
TC DL	PSF	DATE 11/26/03
BC DL	PSF	DRWG CNBOLTSP1103
BC LL	PSF	-ENG JL
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

TRULOX CONNECTION DETAIL

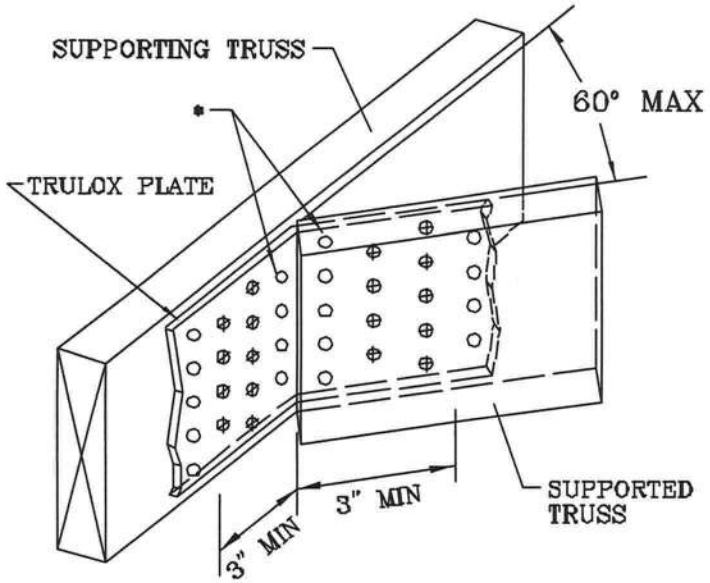
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

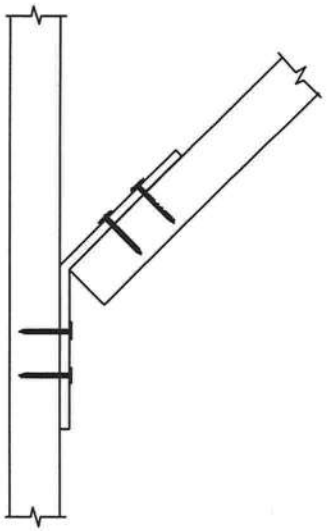
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

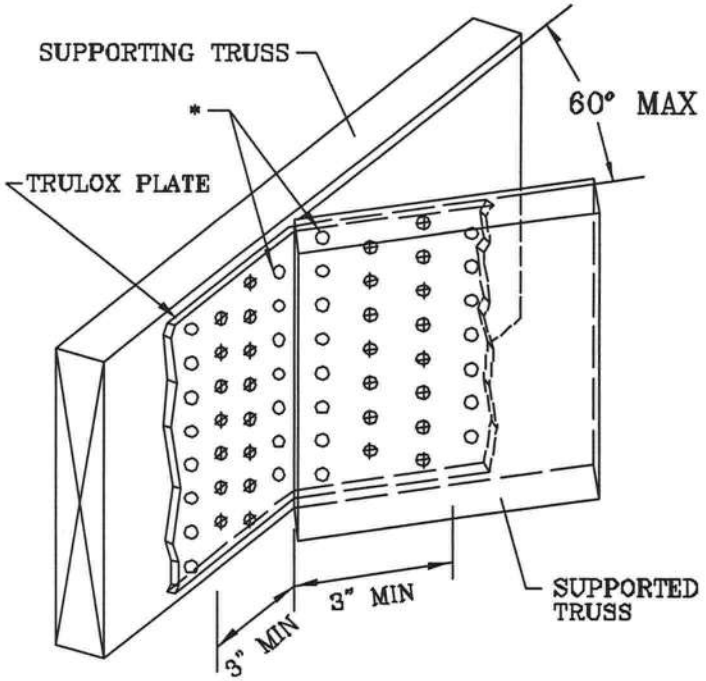
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

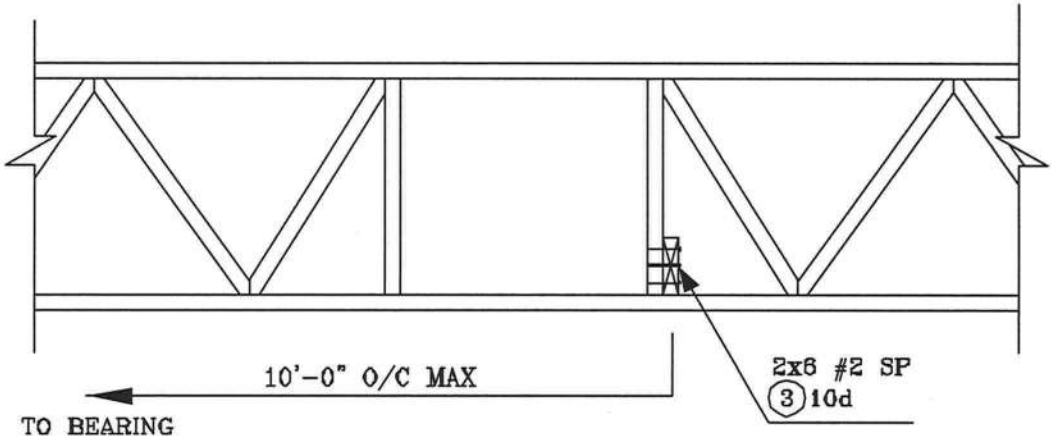
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2151

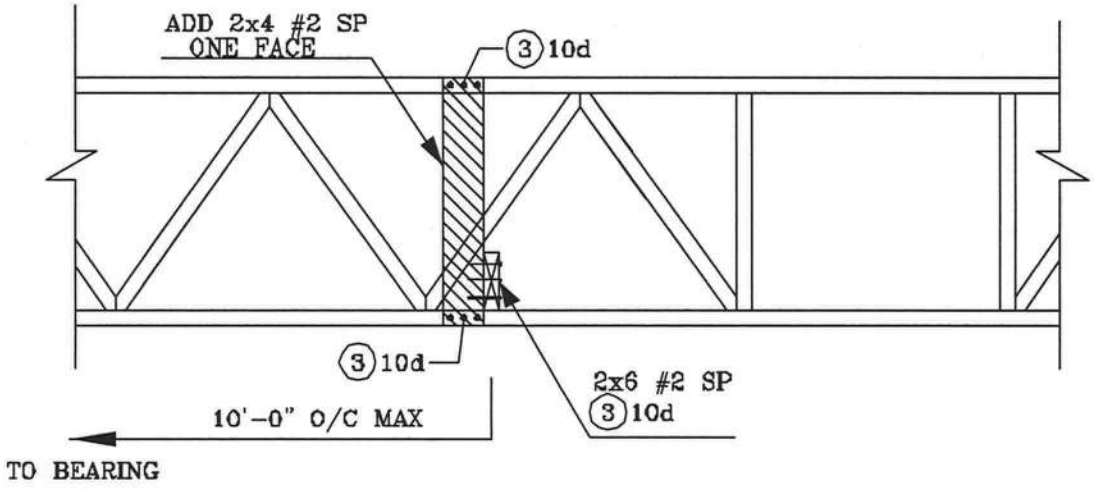
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM–42 OR FLAT TRUSS

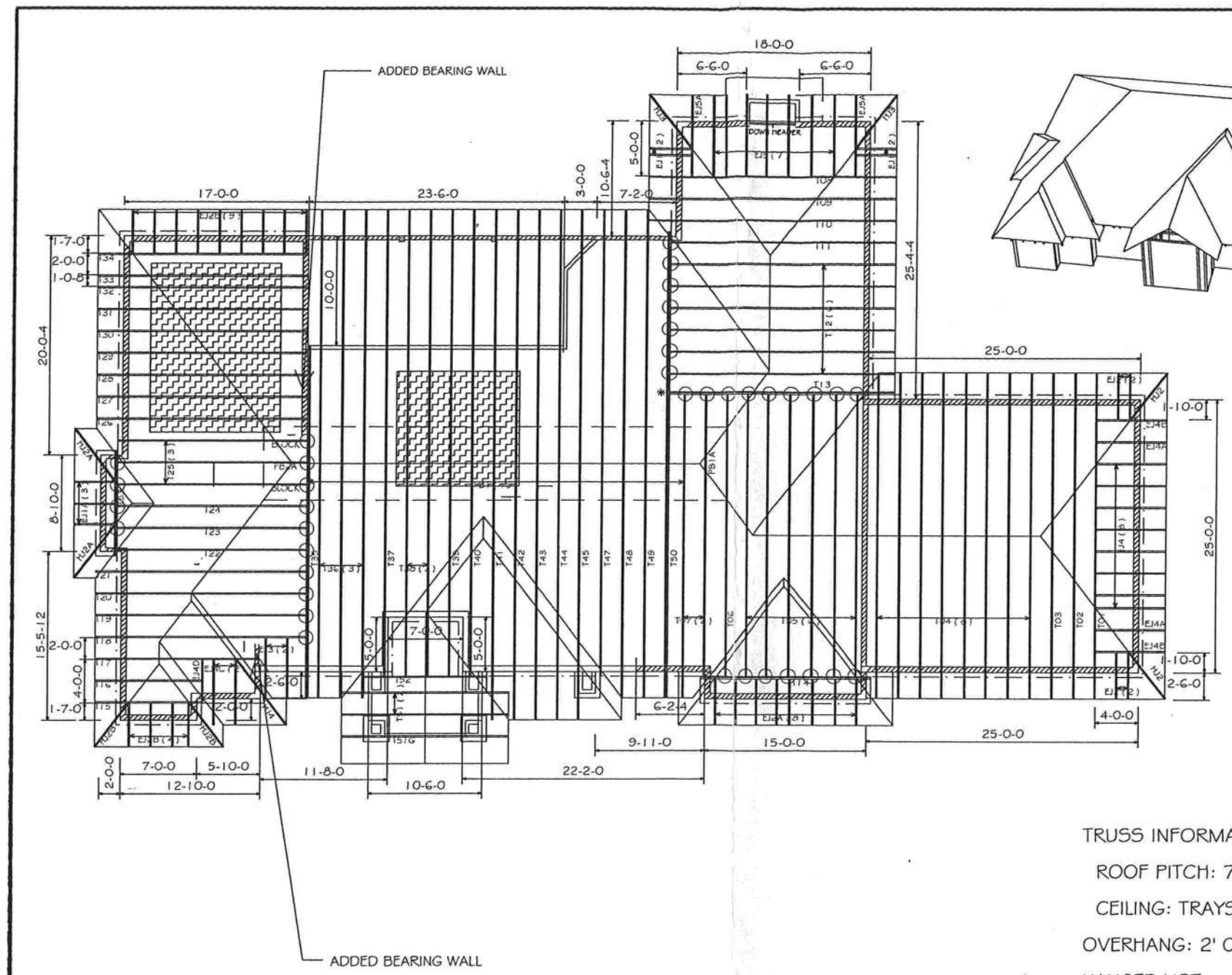


ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA



TRUSS INFORMATION:

ROOF PITCH: 7-9/12

CEILING: TRAYS @ MSTR., & LIVING

OVERHANG: 2' 0"

HANGER LIST:
37 - HTU26
1 - HGUS26-2 (CONNECTS T13 TO T50)

VALLEY:
FRAMED BY OTHERS

NOTE: ADDED ADDITIONAL BRG WALLS SEE NOTES
ALL HEADERS AND BEAMS BY OTHERS

BEARING HEIGHT SCHEDULE

10' 0"

9' 0"

INDICATES TRAY CEILING

O = HTU26

* = HGUS26-2

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL YIO5 FOR ALTERNATE BRACING REQUIREMENTS.

3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4) ALL TRUSSES ARE DESIGNED FOR 2 o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8) BEAM/HEADER/LINTEL (MBR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Expected Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource
Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973
Lake City
PHONE: 386-755-6894 FAX: 386-755-7473
Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: N/A

LEGAL ADDRESS: GAINESVILLE, FL

MODEL: TONY AUSTIN RES. REVISION:

SCALE: NTS

DATE: 10/8/07 DRAWN BY: JP JOB #: L255815

1956.2	Glass-Seal AR	A 3 tab asphalt shingle.
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Asphalt shingles shall be used only on roof slopes of 2:12 or greater. Not approved for use in HVHZ.		**Certification Agency Certificate** **Installation Instructions** Verified By:

Architectural Testing

AAMA/WDMA/CSA 101/I.S.2/A440-05 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No.:	67853.02-109-47
Test Date:	11/20/06
Report Date:	03/05/07
Expiration Date:	11/20/10

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on a Series/Model 3540/3240 (fin), PVC single hung window at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 1168 x 1524 (46 x 60) rating. Test specimen description and results are reported herein. The sample was provided by the client.

Test Specification: The test specimen was evaluated in accordance with AAMA/WDMA/CSA 101/I.S.2/A440-05, *Standard/Specification for Windows, Doors, and Unit Skylights.*

Test Specimen Description:

Series/Model: 3540/3240 (Fin)

Product Type: PVC Single Hung Window

Overall Size: 1168 mm (46") wide by 1524 mm (60") high

Sash Size: 1118 mm (44") wide by 746 mm (29-3/8") high

Overall Area: 0.83 m^2 (19.16 ft^2)

Finish: All vinyl was white.

Frame Construction: The frame was constructed from extruded PVC. The corners were mitered and welded. The fixed meeting rail was secured to each jamb with a plastic clip. The clip was secured to each jamb with three #6 x 5/8" flat head screws and was secured to the fixed meeting rail with three #6 x 1-1/4" flat head screws.

130 Derry Court
York, PA 17406-8405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

- Architectural Testing

AAMA/WDMA/CSA 101/I.S.2/A440-05
TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.

SERIES/MODEL: 3540/3240 (Fin)
PRODUCT TYPE: PVC Single Hung Window

Title	Summary of Results
Primary Product Designator	H-R35 1168 x 1524 (46 x 60)
Design Pressure	1689 Pa (35.3 psf)
Negative Design Pressure	2400 Pa (50.16 psf)
Uniform Load Structural Test Pressure	+2536 Pa (53.0 psf) -3600 Pa (75.24 psf)

Test Completion Date: 11/20/06

Reference must be made to Report No. 67853.02-109-47, dated 03/05/07 for complete test specimen description and data.

130 Derry Court
York, PA 17406-8405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Test Specimen Description: (Continued)

Sash Construction: The sash was constructed from extruded PVC. The corners were mitered and welded.

Weatherstripping:

Description	Quantity	Location
0.187" backed by 0.230" polypile with center fin	2 Rows	Sash stiles
0.187" backed by 0.230" polypile with center fin	1 Row	Operable meeting rail
0.187" backed by 0.230" polypile with center fin	1 Row	Sill leg
3/8" diameter single leaf foam-filled vinyl bulb	1 Row	Bottom rail
1/8" diameter foam-filled vinyl bulb	1 Row	Fixed meeting rail

Glazing Details: The window utilized 7/8" thick sealed insulating glass constructed from two sheets of 3/32" clear annealed glass with an aluminum reinforced butyl spacer system. The glass was interior glazed onto sash glazing tape and secured with snap-in PVC glazing beads.

Drainage:

Description	Quantity	Location
3/4" by 1/8" weepslot	2	Interior hollow
3/4" by 1/8" weepslot	2	Middle hollow
1/2" by 1/8" weepslot	2	3" from edge of frame draining screen track
1" by 1/8" weepslot	2	Sill face
3/4" by 1/16" weepslot	2	Bottom rail of sash

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Metal sweep lock	2	6" from ends of top rail
Constant force balance	2	One in each jamb
Metal pivot bars	2	Ends of bottom rail
Plastic tilt latches	2	Ends of top rail

Reinforcement: All sash members were reinforced with "I" shaped, roll-formed aluminum, (Drawing #GVL-451-020). The fixed meeting rail utilized custom shaped roll-formed aluminum reinforcement, (Drawing #RF-1045-020).

Screen Construction: The screen was constructed from roll-formed aluminum square-cut and keyed with plastic keys. The fiberglass mesh was secured with a flexible vinyl spline.

Installation: The window was installed into a Spruce-Pine-Fir wood buck. The fin was set onto a bead of silicone. The window was secured with #8 x 1-5/8" screws 3" from ends and 12" on center around the perimeter of the frame through the fin into the wood buck.

Test Results: The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
5.3.4.2	Uniform Load Deflection per ASTM E 330		See Note #1
5.3.4.3	Uniform Load Structural per ASTM E 330		See Note #1

***Note #1**: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance".*

Test Results: (Continued)

Optional Performance

Paragraph	Title of Test - Test Method	Results	Allowed
4.4.2.6	Uniform Load Deflection per ASTM E 330 (Deflections were taken on the meeting rail) (Loads were held for 52 seconds)		
	1689 Pa (35.3 psf) (positive)	10.4 mm (0.41")	See Note #2
	2400 Pa (50.16 psf) (negative)	13.0 mm (0.51")	See Note #2

***Note #2**: The deflections reported are not limited by AAMA/WDMA/CSA 101/I.S.2/A440-05 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

4.4.2.6	Uniform Load Structural per ASTM E 330 (Permanent sets were taken on the meeting rail) (Loads were held for 10 seconds)		
	2536 Pa (53.0 psf) (positive)	1.0 mm (0.04")	4.32 mm (0.17") max.
	3600 Pa (75.24 psf) (negative)	1.3 mm (0.05")	4.32 mm (0.17") max.

Drawing Reference: The test specimen drawings have been reviewed by Architectural Testing, Inc. and are representative of the test specimen reported herein.

Detailed drawings, data sheets, representative samples of test specimens, a copy of this report, or other pertinent project documentation will be retained by Architectural Testing, Inc. for a period of four years from the original test date. At the end of this retention period, such materials shall be discarded without notice and the service life of this report will expire. Results obtained are tested values and were secured by using the designated test methods. This report does not constitute certification of this product nor an opinion or endorsement by this laboratory. It is the exclusive property of the client so named herein and relates only to the specimens tested. This report may not be reproduced, except in full, without the written approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Digitally Signed by: Jeremy R. Bender

Jeremy R. Bender
Technician

Digitally Signed by: Michael D. Stremmel

Michael D. Stremmel, P.E.
Senior Project Engineer

JRB:clo

Attachments (pages): This report is complete only when all attachments listed are included.
Appendix-A: Alteration Addendum (1)

Revision Log

Rev. #	Date	Page(s)	Revision(s)
0	03/05/07	N/A	Original report issue

Appendix A

Alteration Addendum

***Note**: No alterations were required.*

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.

SERIES/MODEL: 3540
PRODUCT TYPE: PVC Triple Single Hung

Title	Summary of Results
Rating	H-R30* 108 x 74
Operating Force	17 lbf max.
Air Infiltration	0.11 cfm/ft^2
Water Resistance Test Pressure	4.50 psf
Uniform Load Deflection Test Pressure	±47.2 psf
Uniform Load Structural Test Pressure	+52.5 psf, -70.8 psf
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 50172.01-122-47 for complete test specimen description and data.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No.:	50172.01-122-47
Revision 1:	08/30/04
Test Dates:	06/11/04
Through:	07/07/04
Report Date:	07/27/04
Expiration Date:	07/07/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on a Series/Model 3540, triple single hung window at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R30* 108 x 74 rating. Reference should be made to Report No. 01-45617.02 for Gateway Performance results. Test specimen description and results are reported herein.

***General Note**: An asterisk (*) next to the performance grade indicates that the size tested for optional performance was smaller than the Gateway test size for the product type and class.*

Test Specification: The test specimen was evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 3540

Product Type: PVC Triple Single Hung

Overall Size: 8' 11-5/8" wide by 6' 1-3/4" high

Interior Sash Size (3): 2' 9-3/4" wide by 3' 0-1/8" high

Fixed Daylight Opening Size (3): 2' 7-3/4" wide by 2' 9-3/16" high

Screen Size: 2' 9" wide by 2' 11-1/4" high

Overall Area: 55.1 ft^2

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No.:	50172.01-122-47
Revision 1:	08/30/04
Test Dates:	06/11/04
Through:	07/07/04
Report Date:	07/27/04
Expiration Date:	07/07/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on a Series/Model 3540, triple single hung window at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R30* 108 x 74 rating. Reference should be made to Report No. 01-45617.02 for Gateway Performance results. Test specimen description and results are reported herein.

***General Note**: An asterisk (*) next to the performance grade indicates that the size tested for optional performance was smaller than the Gateway test size for the product type and class.*

Test Specification: The test specimen was evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 3540

Product Type: PVC Triple Single Hung

Overall Size: 8' 11-5/8" wide by 6' 1-3/4" high

Interior Sash Size (3): 2' 9-3/4" wide by 3' 0-1/8" high

Fixed Daylight Opening Size (3): 2' 7-3/4" wide by 2' 9-3/16" high

Screen Size: 2' 9" wide by 2' 11-1/4" high

Overall Area: 55.1 ft^2

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Constant force balances	6	One per jamb
Metal cam locks with adjacent keepers	6	Meeting rail, 7" from each end
Plastic tilt latches	6	Each end of the interior meeting rail
Metal pivot pins	6	Each end of the bottom rail

Drainage:

Description	Quantity	Location
3/32" by 1/2" weepslot	12	Bottom rail, 2 at each end
1/8" by 1" weepslot	2	Sill, 3" from each end
3/16" by 1/2" weepslot	2	Screen track, 2-1/2" from each end

Reinforcement: The interior meeting rail and bottom rail utilized a roll-formed "I beam" steel reinforcement (Drawing #GVL-451-020). The fixed meeting rail utilized a steel reinforcement (Drawing #RF-104S-020). The intermediate frame rails utilized a steel reinforcement (Drawing #2.75x.125 steel plate).

Installation: The unit was installed into a wood test buck. The nail fin was set against a silicone bedding and fastened to the buck with #6 by 1-5/8" screws, 2" from corners and 8" on center. 3/4" washers were utilized along the entire length of the sill, at midspan of the head and jambs, and at all corners.

Test Results: The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.6.1.1	Operating Force	17 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.11 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets (or exceeds) the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.3	Water Resistance per ASTM E 547 (with and without screen)		See Note #2

Note #2: *The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance".*

2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the mullion) (Loads were held for 52 seconds)		
	35.0 psf (positive)	0.39"	See Note #3
	35.0 psf (negative)	0.54"	See Note #3

Note #3: *The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the mullion) (Loads were held for 10 seconds)		
	52.5 psf (positive)	<0.01"	0.27" max.
	52.5 psf (negative)	0.07"	0.27" max.
2.2.6.1.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Interior meeting rail	0.13"/26%	0.50"/100%
	Bottom rail	0.11"/22%	0.50"/100%
	In remaining direction - 50 lbs		
	Left stile	0.09"/18%	0.50"/100%
	Right stile	0.10"/20%	0.50"/100%
2.1.7	Welded Corner Test	Meets as stated	Meets as stated

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1	No entry	No entry
	Test A2	No entry	No entry
	Test A3	No entry	No entry
	Test A4	No entry	No entry
	Test A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) 4.50 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the mullion) (Loads were held for 52 seconds)		
	47.2 psf (positive)	0.73"	See Note #3
	47.2 psf (negative)	0.92"	See Note #3
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the mullion) (Loads were held for 10 seconds)		
	52.5 psf (positive)	<0.01"	0.27" max.
	70.8 psf (negative)	0.21"	0.27" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Digitally Signed by: Jeramie D. Grabosch

Jeramie D. Grabosch
Technician

Digitally Signed by: Steven M. Urich

Steven. M. Urich, P.E.
Senior Project Engineer

JDG:vlm

Underwriters Laboratories Inc.®

Northbrook Division
333 Pfingsten Road
Northbrook, IL 60062-2096 USA
www.ul.com
tel: 1 847 272 8800

June 17, 2005

Tamko Roofing Products
Ms. Kerri Eden
P.O. Box 1404
220 W. 4th Street
Joplin, MO 64802-1404

Our Reference: R2919

This is to confirm that "Elite Glass-Seal AR", "Heritage 30 AR", "Heritage 50 AR", "Glass-Seal AR" manufactured at Tuscaloosa, AL and "Elite Glass-Seal AR", "Heritage 30 AR", "Heritage XL AR", "Heritage 50 AR" manufactured at Frederick, MD and "Heritage 30 AR", "Heritage XL AR", and "Heritage 50 AR" manufactured in Dallas, TX are UL Listed asphalt glass mat shingles and have been evaluated in accordance with ANSI/UL 790, Class A (ASTM E108), ASTM D3462, ASTM D3161 or UL 997 modified to 110 mph when secured with four nails.

Let me know if you have any further questions.

Very truly yours,

Alpesh Patel (Ext. 42522)
Engineer Project
Fire Protection Division

Reviewed by,

Randall K. Laymon (Ext. 42687)
Engineer Sr Staff
Fire Protection Division

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC

SERIES/MODEL: 420/430/440
PRODUCT TYPE: Aluminum Sliding Glass Door

	Summary of Results		
Title	**Test Specimen #1**	**Test Specimen #2**	**Test Specimen #3**
Rating	SGD-R25 182 x 96	SGD-R35 182 x 80	SGD-R40 144 x 96
Operating Force	17 lbf max.	17 lbf max.	N/A
Air Infiltration	0.23 cfm/ft^2	0.27 cfm/ft^2	N/A
Water Resistance Test Pressure	3.75/6.0/9.0 psf	6.0 psf	N/A
Uniform Load Deflection Test Pressure	±35.0 psf	±35.0 psf	+40.0 psf/-40.1 psf
Uniform Load Structural Test Pressure	±37.5 psf	±52.5 psf	+60.0 psf/-60.2 psf
Forced Entry Resistance	Grade 10	Grade 10	N/A

Reference should be made to ATI Report No. 52112.01-122-47 for complete test specimen description and data.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No.:	52112.01-122-47
Revision 2:	09/14/05
Test Dates:	06/30/04
Through:	08/12/04
Report Date:	08/30/04
Expiration Date:	07/02/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on three Series/Model 420/430/440, aluminum sliding glass doors at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: SGD-R25 182 x 96; Test Specimen #2: SGD-R35 182 x 80; Test Specimen #3: SGD-R40 144 x 96. Test specimen description and results are reported herein.

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 420/430/440

Product Type: Aluminum Sliding Glass Door

Test Specimen #1: SGD-R25 182 x 96 (XXO)

Overall Size: 15' 1-3/4" wide by 8' 0" high

Active Door Panel Size (2): 5' 0-1/2" wide by 7' 11" high

Fixed Door Panel Size: 5' 1" wide by 7' 11" high

Screen Size: 5' 0-3/8" wide by 7' 11" high

Overall Area: 121.2 ft^2

Reinforcement: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520).

130 Derry Court
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phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Test Specimen Description: (Continued)

Test Specimen #2: SGD-R35 182 x 80 (OXX)

Overall Size: 15' 1-3/4" wide by 6' 8" high

Active Door Panel Size (2): 5' 0-1/2" wide by 6' 7" high

Fixed Door Panel Size: 4' 8-7/8" wide by 6' 2-5/8" high

Screen Size: 5' 0-3/8" wide by 6' 7" high

Overall Area: 101 ft^2

Reinforcement: No reinforcement was utilized.

Test Specimen #3: SGD-R40 144 x 96 (OXO)

Overall Size: 12' 0" wide by 8' 0" high

Active Door Panel Size: 3' 8-1/4" wide by 7' 10-1/2" high

Fixed Door Panel Size (2): 3' 8-3/4" wide by 7' 6-1/2" high

Screen Size: 3' 11-1/2" wide by 7' 11-3/8" high

Overall Area: 96 ft^2

Reinforcement: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520). The interlock utilized an aluminum reinforcement (Drawing #SECT4237).

The following descriptions apply to all specimens.

Finish: All aluminum was painted.

Glazing Details: All glazing consisted of a single sheet of 3/16" thick clear tempered glass that was channel glazed with a wrap around rubber gasket.

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.187" backed by 0.270" high polypile with center fin	2 Rows	Stiles
1/2" wide by 1" long polypile dust plug	2 Pieces	Corner of head, jamb, and top and bottom of panel retainer
0.187" backed by 0.250" high polypile with center fin	2 Rows	Top rail
0.187" backed by 0.350" high polypile with center fin	2 Rows	Bottom rail
0.187" backed by 0.230" high polypile with center fin	1 Row	Panel interlock, screen stiles

Frame Construction: The frame was constructed of extruded aluminum. Corners were coped, butted, sealed, and fastened with two #8 x 5/8" screws. An aluminum panel adaptor was added to the screen adaptor and secured with #6 x 3/8" pan head screws located 3-1/2" from the ends and 14" on center through the screen adaptor into the panel adaptor. The jambs utilized a panel jamb retainer on the fixed panels secured to the jambs with two #6 x 1/2" screws through the retainer into the jambs. The panels were placed in the retainer and secured to the frame with two #8 x 1/2" screws located through the retainers into the panels. Three panel jamb retainers were utilized to secure the fixed panels, located at panel top and bottom and one midspan. The fixed panels also utilized an aluminum sill retainer clip located at the sill. The sill utilized an optional aluminum sill extender.

Door Panel Construction: The door panels were constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" x 3/4" screw at the bottom and two #8 x 3/4" screws at the top.

Screen Construction: The screen was constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" x 3/4" screw and one #8 x 1" screw at the bottom and one #8 x 1" screw at the top.

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Locking handle	1	44" from active panel bottom
Roller assembly	2	3" from bottom rail ends
Screen locking handle	1	46" from screen bottom rail
Screen rollers	2	Corners of bottom rail

Drainage:

Description	Quantity	Location
Sloped sill	1	Sill
1/2" long drain off notches	6	Ends of vertical sill legs

Installation: The units were installed into a #2 Spruce-Pine-Fir wood test buck. The units were fastened to the test buck with two rows of #8 x 1-1/4" screws, 8" from each end and 23" on center. The exterior perimeter was sealed with silicone.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO)			
2.2.1.6.1	Operating Force	17 lbf	20 lbf max.
	Breakaway force	24 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283		
	1.57 psf (25 mph)	0.23 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: *The tested specimen meets (or exceeds) the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547		
	(with and without screen)		
	2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330		
	(Deflections reported were taken on the meeting rail)		
	(Loads were held for 52 seconds)		
	15.0 psf (positive)	0.56"	See Note #2
	15.0 psf (negative)	0.57"	See Note #2

Note #2: *The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

2.1.4.2	Uniform Load Structural per ASTM E 330		
	(Permanent sets reported were taken on the meeting stile)		
	(Loads were held for 10 seconds)		
	22.5 psf (positive)	0.02"	0.30" max.
	22.5 psf (negative)	0.03"	0.30" max.
2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction - 70 lbs		
	Locking stile	0.12"/24%	0.50"/100%
	Interlock stile	0.12"/24%	0.50"/100%

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO) (Continued)			
2.2.1.6.2	Deglazing Test per ASTM E 987 In remaining direction - 50 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 842		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A6	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) 3.75 psf	No leakage	No leakage
4.3	Water Resistance per ASTM E 547 (with and without screen) (with sill riser) 6.0 psf	No leakage	No leakage
4.3	Water Resistance per ASTM E 547 (with and without screen) (with 2-5/8" Dade County sill extension) 9.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	35.0 psf (positive)	2.98"	See Note #2
	35.0 psf (negative)	2.52"	See Note #2

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO) (Continued)			
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	37.5 psf (positive)	0.20"	0.36" max.
	37.5 psf (negative)	0.19"	0.36" max.
Test Specimen #2: SGD-R35 182 x 80 (OXX)			
2.2.1.6.1	Operating Force	17 lbf	20 lbf max.
	Breakaway force	21 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283		
	1.57 psf (25 mph)	0.27 cfm/ft^2	0.3 cfm/ft^2 max.
	Note #1: *The tested specimen meets (or exceed) the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*		
2.1.3	Water Resistance per ASTM E 547 (with and without screen)		
	2.86 psf	No leakage	No leakage
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Locking stile	0.12"/24%	0.50"/100%
	Interlock stile	0.12"/24%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 842		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A6	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #2: SGD-R35 182 x 80 (OXX) (Continued)			
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) (with sill riser) 6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	35.0 psf (positive)	1.28"	See Note #2
	35.0 psf (negative)	1.33"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	52.5 psf (positive)	0.13"	0.30" max.
	52.5 psf (negative)	0.15"	0.30" max.
Test Specimen #3: SGD-R40 144 x 96 (OXO)			
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	40.0 psf (positive)	1.42"	See Note #2
	40.1 psf (negative)	1.28"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	60.0 psf (positive)	0.27"	0.37" max.
	60.2 psf (negative)	0.30"	0.37" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Digitally Signed by: Mark A. Hess

Mark A. Hess
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

MH:vlm

Revision Log

Rev. #	Date	Page(s)	Revision(s)
0	08/30/04	N/A	Original report issue
1	09/13/04	Cover page	Switch Specimens 1 and 2 / Added 430/440 to Series/Model
1	09/13/04	Page 1 and 2	Switch Specimen 1 and 2 sizes Added 430/440 to Series/Model on Page 1
1	09/13/04	Pages 4 through 7	Switch Specimen 1 and 2 test results / Specimen 2 optional performance water resistance from 3.75 psf to 6.00 psf with sill riser.
2	09/14/05	Page 2	Corrected configuration of Test Specimen #3
2	09/14/05	Page 3	Added additional Weatherstripping

Cal-Tech Testing, Inc.

- Engineering
- Geotechnical
- Environmental

LABORATORIES

P.O. Box 1625 • Lake City, FL 32056 Tel. (386) 755-3633 • Fax (386) 752-5456

4784 Rosselle Street • Jacksonville, FL 32254 Tel. (904) 381-8901 • Fax (904) 381-8902

February 27, 2009

Dale's Excavation, Inc.
6139 SW SR 47
Lake City, Florida 32024

Attention: Mr. Dale Peeler

Subject: Construction Materials Testing
Walter Rentz Residence
Lake City, Columbia County, Florida
Cal-Tech Project No. 08-00293

Dear Mr. Peeler:

As requested by you, Cal-Tech Testing, Inc. (CTI) representatives visited the subject site to performed sampling and testing of soil backfill within the house pad area. Attached are results of tests performed during our site visits.

We appreciate this opportunity of working with you on this project and look forward to serving you on future projects. Should you have any questions and or comments concerning this report, please contact our office at 386-755-3633.

Sincerely,
Cal-Tech Testing, Inc.

Nabil O. Hmeidi, P.E. 2/27/09
Senior Geotechnical Engineer
Licensed, Florida No. 57842

Distribution: *file*
Addressee
Mr. Johnny Kerce – Columbia County Building Department

Cal-Tech Testing, Inc.

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- Environmental

Laboratories

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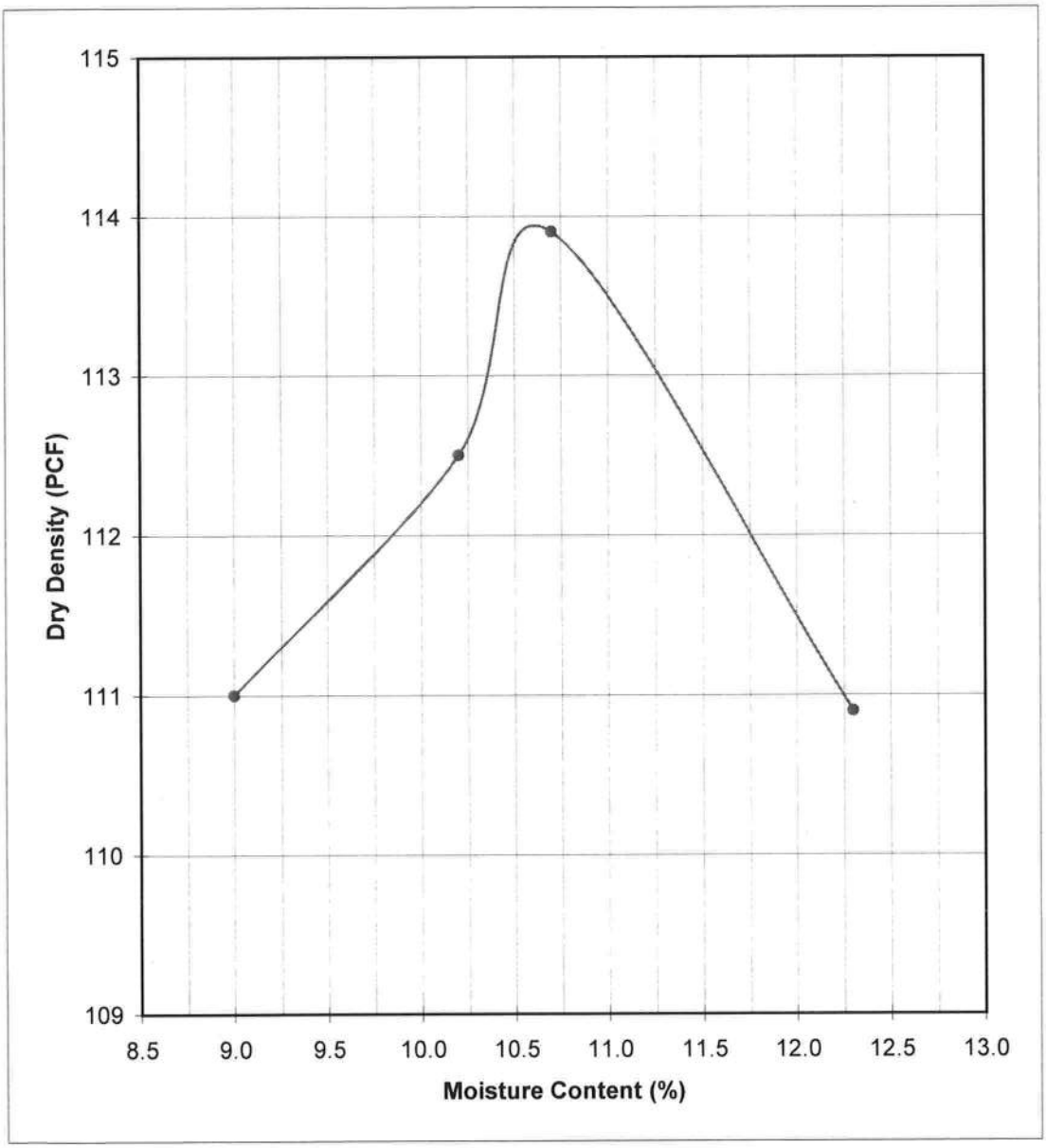
2230 Greensboro Hwy • Quincy, FL 32351 • Tel(850)442-3495 • Fax(850)442-4008

REPORT OF LABORATORY COMPACTION TEST

Client: Dale's Excavation, Inc., 6139 SW SR 47, Lake City 32024
Project Name: Walter Rentz Residence
Project Location: Lake City, FL
Contractor: Dale's Excavation, Inc.

File No: 08-00293
Date: 6/4/2008
Lab No: 11122



PROCTOR DATA

Proctor No.: 1

Modified Proctor (ASTM D-1557) ☑

Standard Proctor (ASTM D-698) ☐

Maximum Dry Dens. Pcf: 113.9

Optimum Moisture Percent: 10.6

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

Sample Description: Brown Sand Trace of Silt
Sample Location: House Pad
Proposed Use: Building Fill
Sampled By: Richard Kramer **Date:** 5/30/2008
Tested By: Sandra Yates **Date:** 6/3/2008
Remarks: 1cc: Client
1cc: File

Linda Creamer, CEO, DBE
Linda M. Creamer
President - CEO
Reviewed By:
Date: 6/25/09
Licensed, Florida No: 57842

SW

Cal-Tech Testing, Inc.

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2230 Greensboro Hwy • Quincy, FL 32351 • Tel(850)442-3495 • Fax(850)442-4008

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 08-00293-01
DATE TESTED: 5/30/08
DATE REPORTED: 2/25/09

PROJECT:	Walter Rentz Residence, Lake City, FL
CLIENT:	Dale's Excavation, Inc., 6139 SW SR 47, Lake City, FL 32024
GENERAL CONTRACTOR:	Dale's Excavation, Inc.
EARTHWORK CONTRACTOR:	Dale's Excavation, Inc.
INSPECTOR:	Richard Kramer

ASTM METHOD	**SOIL USE**
(D-2922) Nuclear	BASE COURSE

SPECIFIED REQUIREMENTS: 95%

TEST NO.		TEST LOCATION	LIFT	TEST DEPTH	WET DENSITY (lb/ft^3)	MOISTURE PERCENT	DRY DENSITY (lb/ft^3)	PROCTOR TEST NO.	PROCTOR VALUE	% MAXIMUM DENSITY
1		50'N of SW Corner x 20'E of SW Corner	1	0-12"	120.2	11.5	107.8	1	113.9	95%
2		55'N of SW Corner x 20'E of SW Corner	2	0-12"	118.8	6.7	111.3	1	113.9	98%

REMARKS: The Above Tests Meet Specified Requirements.

PROCTORS

PROCTOR NO.		SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft^3)	OPT. MOIST.	TYPE
1		Brown Sand Trace of Silt	113.9	10.6	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

Reviewed By:

Date: 2/25/09
Licensed, Florida No: 57842

sw

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

Cal-Tech Testing, Inc.

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2230 Greensboro Hwy • Quincy, FL 32351 • Tel(850)442-3495 • Fax(850)442-4008

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 08-00293-01
DATE TESTED: 6/2/08
DATE REPORTED: 2/25/09

PROJECT:	Walter Rentz Residence, Lake City, FL
CLIENT:	Dale's Excavation, Inc., 6139 SW SR 47, Lake City, FL 32024
GENERAL CONTRACTOR:	Dale's Excavation, Inc.
EARTHWORK CONTRACTOR:	Dale's Excavation, Inc.
INSPECTOR:	Chad Day

ASTM METHOD	SOIL USE
(D-2922) Nuclear	BUILDING FILL

SPECIFIED REQUIREMENTS: 95%

TEST NO.		TEST LOCATION	LIFT	TEST DEPTH	WET DENSITY (lb/ft^3)	MOISTURE PERCENT	DRY DENSITY (lb/ft^3)	PROCTOR TEST NO.	PROCTOR VALUE	% MAXIMUM DENSITY
3		SW Corner, 30'E x 25'N	3	0-12"	118.5	6.3	111.5	1	113.9	98%
4		NECorner, 30'W x 30'S	3	0-12"	119.2	8.9	109.5	1	113.9	96%

REMARKS: The Above Tests Meet Specified Requirements.

PROCTORS

PROCTOR NO.		SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft^3)	OPT. MOIST.	TYPE
1		Brown Sand Trace of Silt	113.9	10.6	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

Reviewed By:

Date: 2/25/09
Licensed, Florida No: 57842

sw

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

Cal-Tech Testing, Inc.

- Engineering
- Geotechnical
- Environmental

LABORATORIES

P.O. Box 1625 • Lake City, FL 32056 Tel. (386) 755-3633 • Fax (386) 752-5456
4784 Rosselle Street • Jacksonville, FL 32254 Tel. (904) 381-8901 • Fax (904) 381-8902
2230 Greensboro Highway • Quincy, FL 32351 Tel. (850) 442-3495 • Fax (850) 442-4008

March 21, 2008

Mr. Tony Austin
506 SW Putter Gln
Lake City, Florida 32025

Reference: Limited Subsurface Exploration
Austin Residence
Lake City, Columbia County, Florida
Cal-Tech Project No. 08-00167-01

Dear Mr. Austin:

Cal-Tech Testing, Inc. (**CTI**) has completed the subsurface exploration and engineering evaluation for the proposed new residence in Lake City, Columbia County, Florida. See attached Vicinity Map for approximate location of residence. Our work was performed in conjunction with and authorized by you.

Introduction

We understand the proposed residential structure will be a single-story, wood-farmed, brick-veneered building. The building will be supported on a conventional shallow foundation system. Detailed structural loads have not been provided to us; however, we assume column and wall loads will not exceed 20 kips and 2.0 kips per foot, respectively.

Field Program

In order to determine the shallow soil conditions, we performed two (2) Dynamic Cone Penetration (DCP) tests at the approximate locations shown on the attached Field Exploration Plan. The tests extended to a depth of 10 feet below the existing ground surface by using a manually operated bucket-type auger. The tests were performed in general accordance with ASTM D 1452-80 ("*Soil Investigation and Sampling by Auger Borings*"). Representative samples of the soils were brought to the ground surface by the auger process and transported to our laboratory for visual evaluation and classification.

The DCP is a hand-operated device developed in 1959 and used in augered borings to provide data that can be correlated to the standard penetration tests. The DCP uses a 15-pound steel mass falling 20 inches to strike an anvil to cause the penetration of a 1½" O.D. cone (45° vertex

26759

angle) that has been seated on the surface to be tested. The blows required to advance the embedded cone is then recorded for each 1¾" increment and averaged for two or three intervals. The average blows is correlated to penetration resistance "*N*" values derived from the Standard Penetration Test (SPT). The penetration resistance is used as an index to derive soil parameters from various empirical correlations.

Subsurface Conditions

Typically, the soil profile as disclosed by DC-1 and DC-2 consisted of about 1 to 1½ feet of reddish brown, sandy clay (CL) underlain by about 3 to 8½ feet of light gray and reddish tan, mottled, clay (CH). At the location of DC-1, this stratum is underlain by about 6 feet of yellowish tan and light gray, clay (CL-CH).

The measurement from the existing ground surface to the groundwater table was attempted following completion of each auger boring excavation. The groundwater table was not encountered within any of the borings. Typically, the soil profile at the subject site consists of clayey material which is relatively restrictive to groundwater movement. Due to the relatively short time frame of the field exploration, the groundwater may not have had sufficient time to stabilize. For a true "stabilized" groundwater level reading, piezometers would be required.

For a more detailed description of the subsurface conditions encountered, please refer to the attached boring logs. Note that transition between soil types may be gradual and not abrupt as indicated by the boring logs; therefore, the thickness of soil layers should be considered approximate.

Discussion and Recommendations

The results of the test borings and our visual observation of the site soils indicate the presence of clayey soils within the upper 10 feet of the existing ground surface. These clayey soils have a moderate to high potential for volume change (shrink/swell), mainly as a result of fluctuation in their water content. Typically, clayey soils shrink with the decrease in water content and swell with increase in the water content. This change in volume of the supporting soils beneath the foundation and slab-on-grade may result in structural deformation. To alleviate adverse effects of volume changes of the supporting soils, it is our recommendations that all footings and slabs-on-grade be supported on a minimum of 3 feet of non-expansive soils. This will necessitate the overexcavation of the existing clayey soils from within the building and driveway areas and replacement with well-compacted structural fill. The overexcavation and replacement should extend a minimum horizontal margin of 5 feet beyond all building perimeters.

The exposed surface at the time of the overexcavation will consist of clayey soils. Exposed clayey soils should be compacted to a minimum of 98% of the standard Proctor maximum dry density (ASTM D-698). Subsequent structural fill placed above the overexcavated level should consist of an inorganic, non-plastic, granular soil containing less than 10 percent material passing the No. 200 mesh sieve (relatively clean sand with a Unified Soil

Classification of SP or SP-SM). Structural fill may be placed in thin loose lifts not exceeding 12 inches in thickness and compacted using a heavy steel drum roller (vibratory mode disengaged) with a total operating static weight (including fuel and water) of at least 10 tons and a drum diameter of 5 feet. The granular structural fill should be compacted to densities equivalent to at least 95 percent of the modified Proctor maximum dry density (ASTM D-1557).

Compaction of exposed soils in deeper excavations may cause pumping and/or yielding of the soils being compacted. This instability is caused by accumulation of pore water pressure in the subgrade that is being compacted. To allow this excess pressure to dissipate, the contactor may temporarily halt the compaction operation. In any event, it is recommended to maintain a distance of at least 2 feet between the groundwater level and the surfaces being compacted.

Provided the supporting soils are prepared as indicated above, the our recommendations the proposed building may be designed using a net allowable bearing pressure of 2,000 pounds per square foot (psf) bearing on newly placed, well-compacted structural fill.

The evaluation and recommendations are based upon subsurface conditions encountered at a specific locations and time as presented within this report. However, subsurface conditions may exist that differ from our findings. We request that we be notified if dissimilar subsurface conditions are encountered.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Should you have questions concerning this report or if we may be further service, please contact this office.

Respectfully submitted,
Cal-Tech Testing, Inc.

David B. Brown
Executive Vice President

3/21/08
Nabil O. Hmeidi, P.E.
Senior Geotechnical Engineer
Licensed, Florida No. 57842

Distribution: *file (1 copy)*
Addressee (2 copies)

Attachments: *Vicinity Map (1 page)*
Field Exploration Plan (1 page)
Unified Soil Classification System chart (1 page)
Key To Test Data (1 page)

ATTACHMENTS

CAL-TECH TESTING, INC.
3309 SW SR 247
Lake City, Florida 32024
Telephone: (386) 755-3633
Fax: (386) 752-5456

BORING NUMBER DC-1

CLIENT Mr. Tony Austin
PROJECT NAME Austin Residence
PROJECT NUMBER 08-00167-01
PROJECT LOCATION Lake City, Columbia County, Florida
DATE STARTED 03/19/08 **COMPLETED** 03/19/08
GROUND ELEVATION _____ **HOLE SIZE** _____
DRILLING CONTRACTOR Cal-Tech Testing, Inc.
GROUND WATER LEVELS:
DRILLING METHOD Hand Auger
AT TIME OF DRILLING ---
LOGGED BY N.H. **CHECKED BY** N.H.
AT END OF DRILLING --- Not Encountered
NOTES Hand Auger & Dynamic Cone Penetrometer
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	▲ SPT N VALUE ▲ 20 40 60 80 / PL MC LL 20 40 60 80 / ☐ FINES CONTENT (%) ☐ 20 40 60 80
0.0		Reddish brown, sandy clay (CL) *N VALUES ARE EXTRAPOLATED	AU 1		5			
		Light gray and reddish tan, mottled, clay (CH)	AU 2		4			
2.5			AU 3		5			
			AU 4		7			
		Yellowish tan and light gray, clay (CL-CH)	AU 5		7			
5.0			AU 6		8			
			AU 7		9			
7.5			AU 8		9			
			AU 9		9			
10.0			AU 10		10			

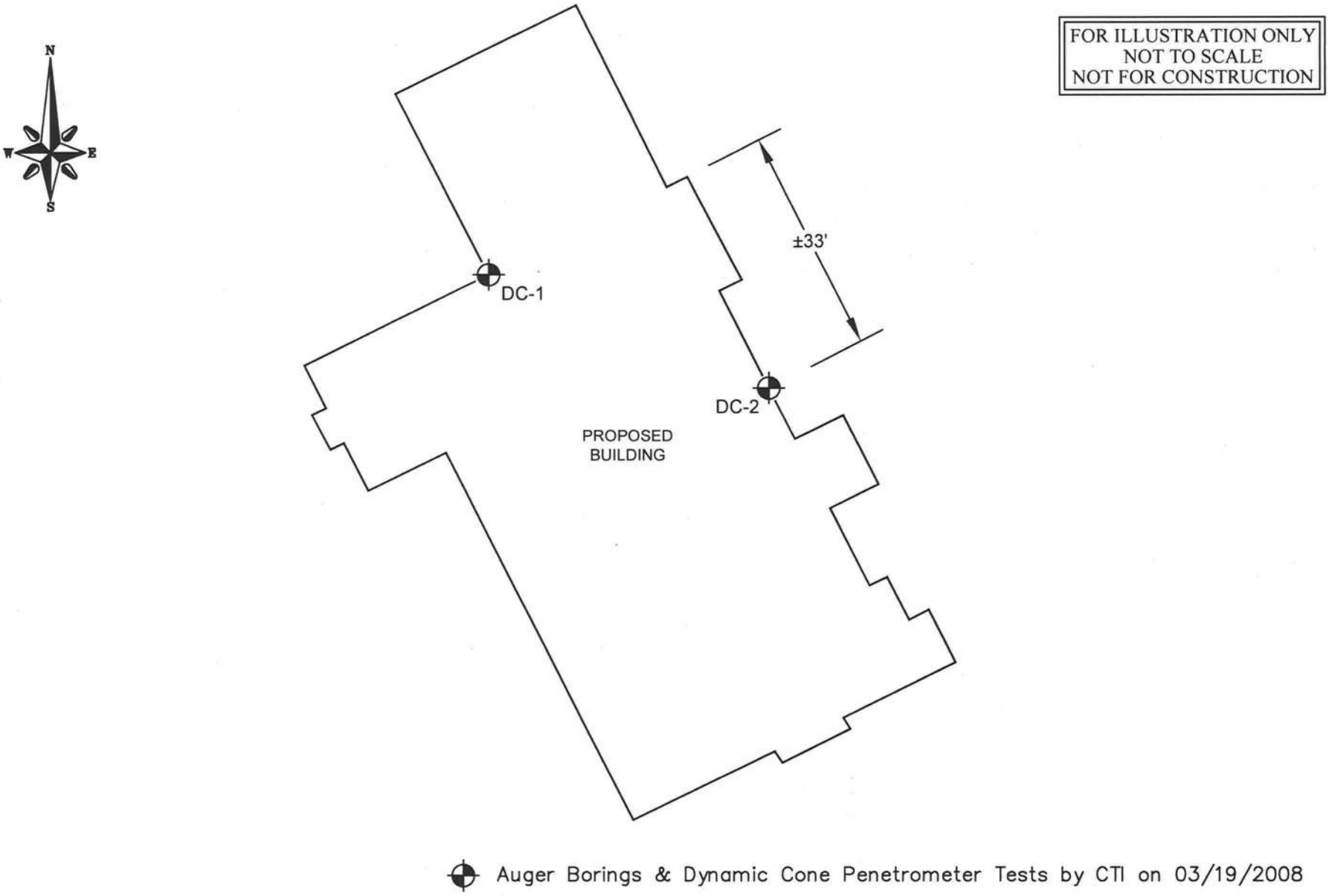
Bottom of borehole at 10.0 feet.

CAL-TECH TESTING, INC.
3309 SW SR 247
Lake City, Florida 32024
Telephone: (386) 755-3633
Fax: (386) 752-5456

BORING NUMBER DC-2

CLIENT Mr. Tony Austin
PROJECT NAME Austin Residence
PROJECT NUMBER 08-00167-01
PROJECT LOCATION Lake City, Columbia County, Florida
DATE STARTED 03/19/08 COMPLETED 03/19/08
GROUND ELEVATION _____ HOLE SIZE _____
DRILLING CONTRACTOR Cal-Tech Testing, Inc.
GROUND WATER LEVELS:
DRILLING METHOD Hand Auger
AT TIME OF DRILLING ---
LOGGED BY N.H. CHECKED BY N.H.
AT END OF DRILLING --- Not Encountered
NOTES Hand Auger & Dynamic Cone Penetrometer
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	▲ SPT N VALUE ▲ 20 40 60 80 / PL MC LL 20 40 60 80 / ☐ FINES CONTENT (%) ☐ 20 40 60 80
0.0		Reddish brown, sandy clay (CL) *N VALUES ARE EXTRAPOLATED	AU 1		4			
			AU 2		4			
		Light gray and reddish tan, mottled, clay (CH)	AU 3		5			
2.5			AU 4		6			
			AU 5		6			
5.0			AU 6		7			
			AU 7		7			
7.5			AU 8		7			
			AU 9		8			
			AU 10		9			
10.0		Bottom of borehole at 10.0 feet.						



Auger Borings & Dynamic Cone Penetrometer Tests by CTI on 03/19/2008

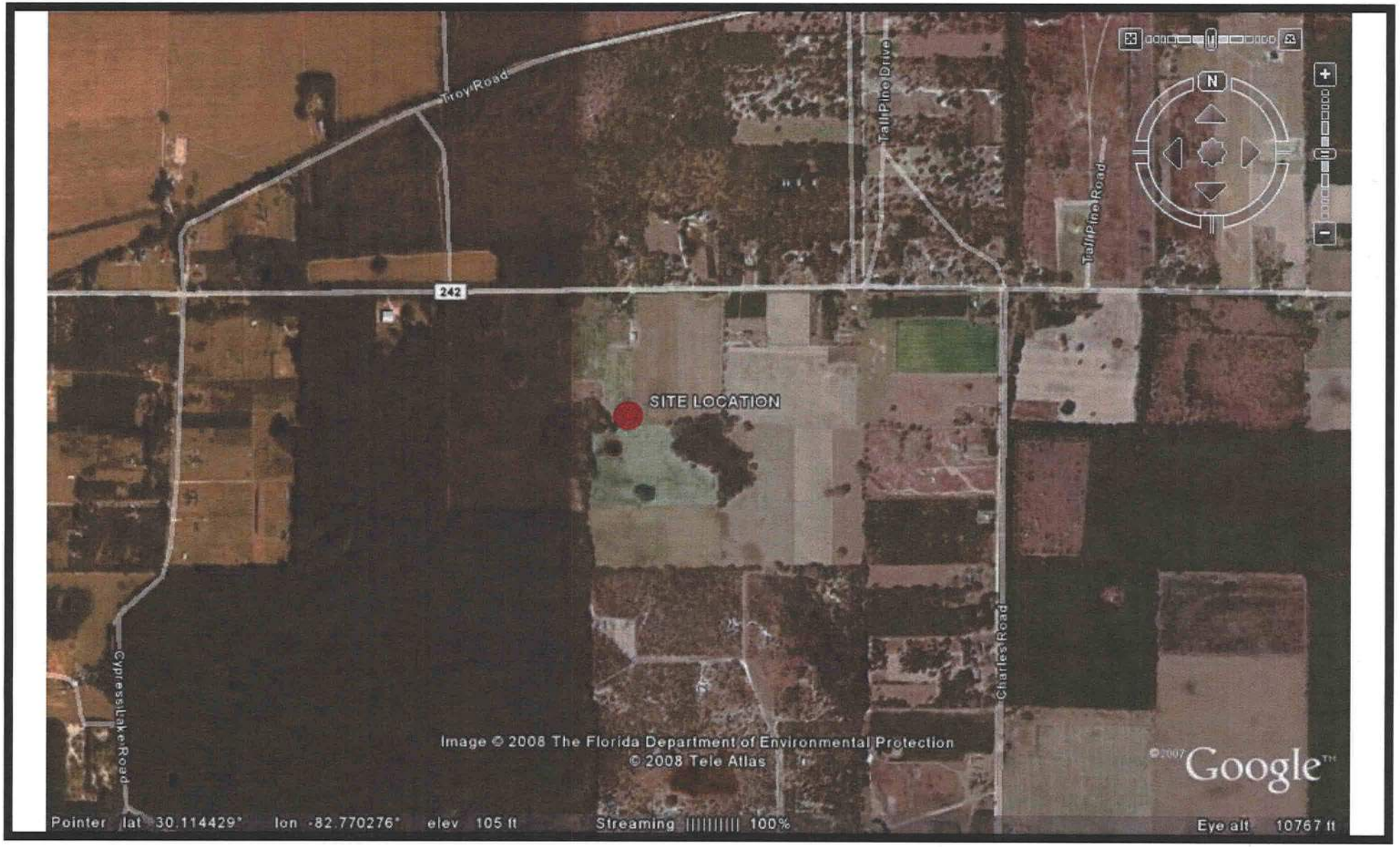
REVISIONS		NAMES	DATE
	DRAWN BY:	N.H.	03/20/2008
	CHECKED BY:		
	DRILLER(S):	B.W. & M.T.	03/19/2008
	EQUIPMENT:	Hand Auger	
	CAL-TECH PROJECT No.: 08-00167-01		

LIMITED SUBSURFACE EXPLORATION
AUSTIN RESIDENCE
LAKE CITY, COLUMBIA COUNTY, FLORIDA

CAL-TECH TESTING, INC.
P.O. Box 1625
Lake City, Florida 32056-1625
Phone: (386) 755-3633
Fax: (386) 752-5456

FIELD EXPLORATION PLAN

Project No. 08-00167-01		DATE: 03/20/2008	FIGURE: 2
DRAWN:	APPROVED:	SCALE: N.T.S.	SHEET: 1/1



CAL-TECH TESTING, INC. **P.O. Box 1625** **Lake City, Florida 32056-1625** **Phone: (386) 755-3633** **Fax: (386) 752-5456**	**VICINITY MAP** **LIMITED GEOTECHNICAL EXPLORATION** **AUSTIN RESIDENCE** **LAKE CITY, COLUMBIA COUNTY, FLORIDA** **Cal-Tech Testing Project No. 08-00167-01**

Figure 1

UNIFIED SOIL CLASSIFICATION SYSTEM
ASTM DESIGNATION D-2487

MAJOR DIVISIONS			GROUP SYMBOL	TYPICAL NAMES
COARSE GRAINED SOILS (More than half of the material is larger than No. 200 sieve)	Gravels (more than half of the coarse fraction is larger than No. 4 sieve)	Clean gravels	GW	Well-graded gravels, gravel-sand mixtures, little or no fines.
			GP	Poorly graded gravels, gravel-sand mixture, little or no fines.
		Gravel with fines	GM	Silty gravels, gravel-sand-silt mixtures.
			GC	Clayey gravels, gravel-sand-clay mixtures.
	Sands (more than half of the coarse fraction is smaller than No. 4 sieve)	Clean sands	SW	Well-graded sands, gravelly sands, little or no fines.
			SP	Poorly graded sands, gravelly sands, little or no fines.
		Sands with fine	SM	Silty sands, sand-silt mixtures.
			SC	Clayey sands, sand-clay mixtures.
FINE GRAINED SOILS (More than half of the material is finer than No. 200 sieve)	Silts and Clays (LL less than 50)		ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity.
			CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clay.
			OL	Organic silts and organic silty clays of low plasticity.
	Silts and Clays (LL greater than 50)		MH	Inorganic silts, micaceous or diato-maceous fine sandy or silty soils, elastic silts.
			CH	Inorganic clays of high plasticity, fat clay.
			OH	Organic clays of medium to high plasticity, organic silts.
	Highly Organic Soils		Pt	Peat and other highly organic soils.

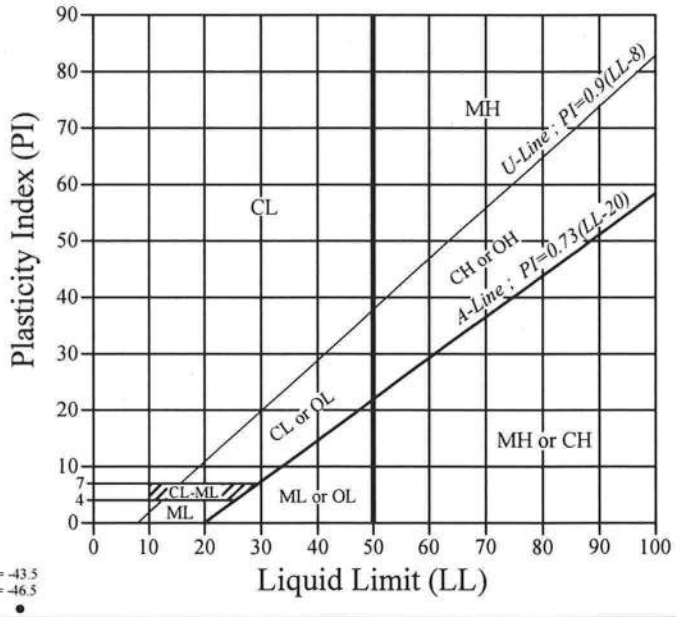
LABORATORY CLASSIFICATION CRITERIA

Determine percentage of sand and gravel from grain size curve
Depending on percentage of fines (fraction smaller than No. 200 Sieve size), coarse grained soils are classified as follows:
Less than 5% ..... GW, GP, SW, SP
More than 12% ... GM, GC, SM, SC
5 to 12% ..... Borderline cases requiring dual symbols

$C_u = \frac{D_{60}}{D_{10}} > 4 \;;\; 1 < C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}} < 3$	
Not meeting all gradation requirments of GW	
Atterberg Limits below A-Line or PI less than 4	Above A-Line with PI between 4 and 7 are borderline cases requiring the use of dual symbols.
Atterberg Limits above A-Line or PI greater than 7	
$C_u = \frac{D_{60}}{D_{10}} > 6 \;;\; 1 < C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}} < 3$	
Not meeting all gradation requirments of SW	
Atterberg Limits below A-Line or PI less than 4	Limits plotting in hatched zone with PI between 4 and 7 are borderline cases requiring the use of dual symbols.
Atterberg Limits above A-Line or PI greater than 7	

PLASTICITY CHART

1. Plot intersection of PI as determined by the Atterberg Limits tests.
2. Points plotted above the A-Line indicate clay soils.
3. Points plotted below the A-Line indicate silt.



LL= 43.5
PI = 46.5

CAL-TECH TESTING, INC.
P.O. Box 1625
Lake City, Florida 32056-1625
Phone: 386-755-3633 Fax: 386-752-5456

5% Max. Passing the U.S. No. 200 Sieve ..... SP
5% - 12% Passing the U.S. No. 200 Sieve ..... SM-SP
12% - 50% Passing the U.S. No. 200 Sieve ..... SM/SC

KEY TO TEST DATA

STANDARD PENETRATION TEST:-

Soil sampling and penetration testing is performed in accordance with ASTM D-1586. The standard penetration resistance ("*N*") is the number of blows of a 140-pound hammer falling 30 inches to drive a 2-inch O.D., 1.4-inch I.D. split spoon sampler one foot.

ROCK CORE DRILLING:-

Rock sampling and core drilling is performed in accordance with ASTM D-2113. The rock quality designation percentage (RQD) is determined by summing only pieces of core that are at least 4 inches long, and dividing by the "run" length.

Relation of RQD and In-situ Rock Quality	
RQD (%)	Rock Quality
90 –100	Excellent
75 – 90	Good
50 –75	Fair
25 – 50	Poor
0 – 25	Very Poor

RELATIVE DENSITY:-

SANDS:

Very loose - less than 4 blows/ft.
Loose - 5 to 10 blows/ft.
Medium - 11 to 30 blows/ft.
Dense - 31 to 50 blows/ft.
Very dense - over 50 blows/ft.

SILTS AND CLAYS:

Very soft - less than 2 blows/ft.
Soft - 3 to 4 blows/ft.
Medium stiff - 5 to 8 blows/ft.
Stiff - 9 to 15 blows/ft.
Very stiff - 16 to 30 blows/ft.
Hard - 31 to 50 blows/ft.
Very hard - over 50 blows/ft.

ROCKS:

Soft - Rock core crumbles when handled.
Medium - Can break core with hands.
Moderately hard - Thin edges of rock core can be broken with fingers.
Hard - Thin edges of core can not be broken with fingers.
Very hard - Can not be scratched with knife.

GROUNDWATER:- Water levels shown on boring logs are taken immediately upon completion of boring, and are intended for general information. The apparent level may have been altered by the drilling process. Groundwater levels, if desired, can be monitored over a long time interval.

Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: 536 SE BAYA
City LAKE CITY Phone 7521703

Site Location: Subdivision
Lot # Block# Permit # 26759
Address 7148 SW CR 242

Product used	Active Ingredient	% Concentration
☑ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☑ Soil ☐ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
DWELLING	985	134	90.

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

2-16-10	10:00	DAVID FULLER
Date	Time	Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink
10/05 ©