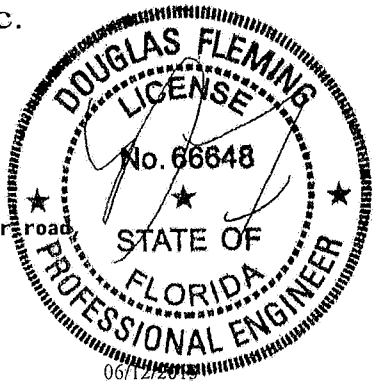
ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
 Document ID 1UX0487-Z0212090016



Truss Fabricator	Anderson Truss Company
Job Identification	13-186--Trout River Builder Tash Residence -- 287 SW Hunter road
Truss Count	8
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Res/TPI-2007(STD)
Engineering Software	Alpine Software,Version 12.03.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

Notes

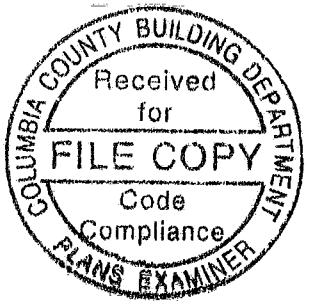
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Douglas Fleming
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: 12015EC1-GBLLETIN-GABRST10-VAL16010-

#	Ref	Description	Drawing#	Date
1	12926--A1	24' Gable	13163006	06/12/13
2	12927--A2	24' Common	13163013	06/12/13
3	12928--A3	24' Common	13163007	06/12/13
4	12929-V2	14'1"12 Valle	13163008	06/12/13
5	12930-V4	6'1"12 Valley	13163009	06/12/13
6	12931-V1	18'1"12 Valle	13163010	06/12/13
7	12932-V3	10'1"12 Valle	13163011	06/12/13
8	12933-V	22'1"12 Valley	13163012	06/12/13



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-186--Trout River Builder Tash Residence -- 287 SW Hunter road - A1 24 Gable)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A
Stack Chord SC1 2x4 SP_#1_12A Stack Chord SC2 2x4 SP_#1_12A

Lumber grades designated with '12A' use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

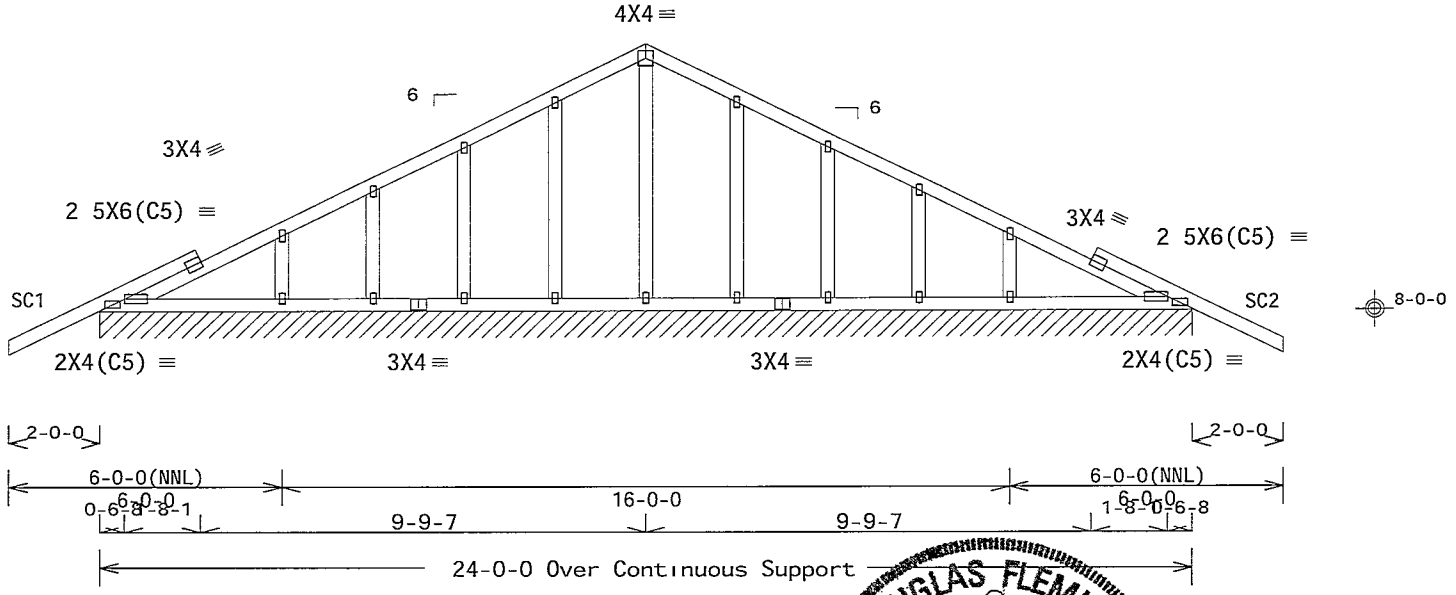
Gable end supports 8" max rake overhang

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6



R=86 PLF U=4 PLF W=24-0-0
RL=5/-5 PLF

Note All Plates Are 1 5X3 Except As Shown

PLT TYP Wave

Design Crit FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

12.03.04.0326 14 / CITY:

FL/-/4/-/-/R/-

Scale = 25"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06/12/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care n fabr cat ng handl ng shipp ng nstalling and brac ng Refer to and follow the latest ed t on of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for s pract ces prior to performing these functions Installers shall provide temporary bracing per BC Unless noted otherw se top chord shall have properly attached structural sheathing and bottom cho shall have a properly attached r g d ceiling Locations shown for permanent lateral restraint of w shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be responsible for any deviation from th s des any failure to build the truss in conformance w th ANSI/TPI 1 or for handling shipping installat on & brac ng of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Details unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos tions A seal on this drawing or cover page l st ng th s drawing nd cates acceptance of professional engineering respons b l ty solely for the des gn shown The su tabil ty and use of this design for any structure is the respons b l ty of the Bu ld ng Des gner per ANSI/TPI 1 Sec 2 For more information see Th s job s general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

TC LL	20 0 PSF	REF	R487-- 12926
TC DL	7 0 PSF	DATE	06/12/13
BC DL	10.0 PSF	DRW	HCUSR487 13163006
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD	37.0 PSF	SEQN-	15478
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UX0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-186--Trout River Builder Tash Residence -- 287 SW Hunter road, - A2 24' Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

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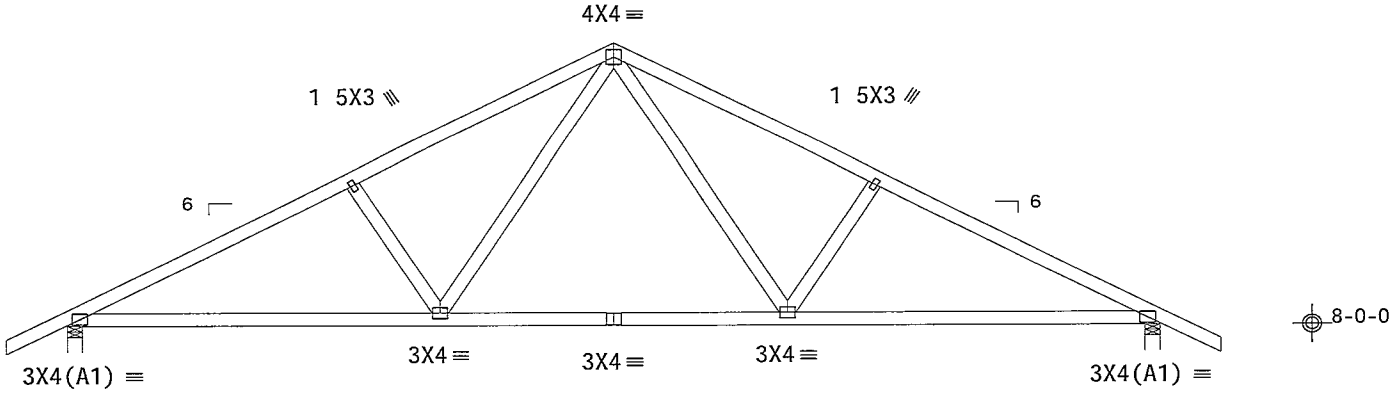
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

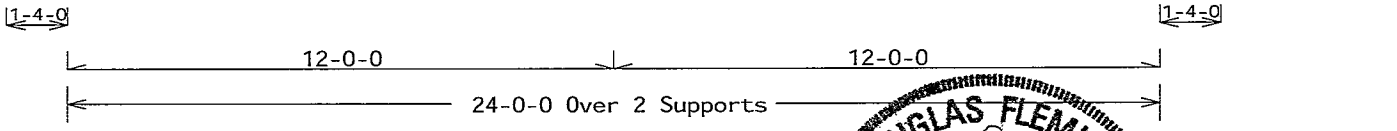
Wind loads and reactions based on MWFRS with additional C&C member design

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=1039 U=41 W=4" (4' min)
RL=108/-108

R=1039 U=41 W=4" (4" min)

Design Crit· FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.14 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 06/12/2013

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ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from th s des any failure to build the truss in conformance with ANSI/TPI 1 or for handl ng shipp ng nstallation & bracing of trusses Apply plates to each face of truss and pos tion as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate pos t ons A seal on th s drawing or cover page l sting this draw ng ndicates acceptance of profess onal eng neer ng respons b l ity solely for the design shown The suitabil ty and use of th s des gn for any structure s the respons b l ity of the Build ng Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW-BCG www twbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www ccsafe org

TC LL	20.0 PSF	REF	R487-- 12927
TC DL	7.0 PSF	DATE	06/12/13
BC DL	10 0 PSF	DRW	HCUSR487 13163013
BC LL	0.0 PSF		HC-ENG JB/DF
TOT LD.	37 0 PSF	SEQN-	15479
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UX0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-186--Trout River Builder Tash Residence -- 287 SW Hunter road, - A3 24' Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with 12A use design values approved 1/5/2012 by ALSC

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MWFRS loads based on trusses located at least 7 50 ft from roof edge

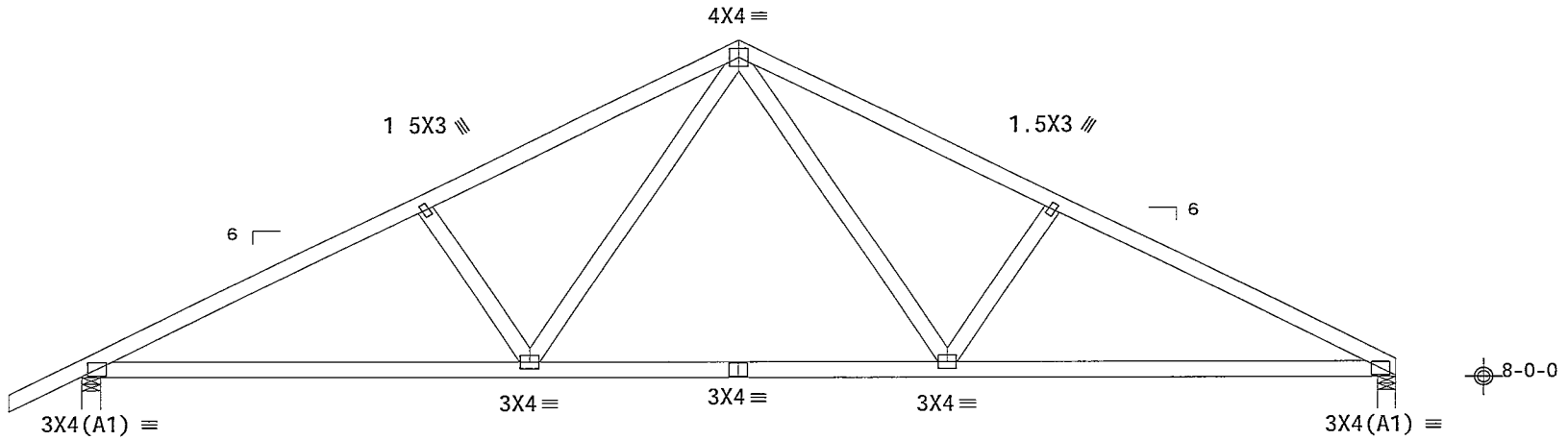
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

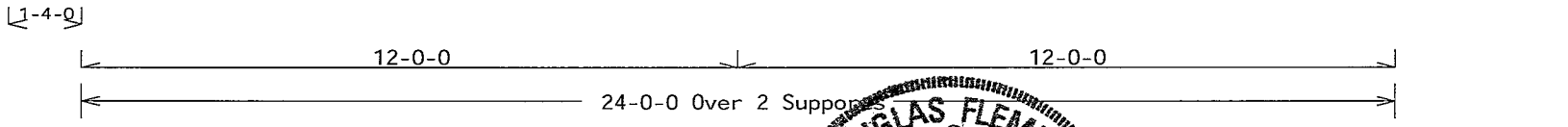
Wind loads and reactions based on MWFRS with additional C&C member design

Truss passed check for 20 psf additional bottom chord live load in areas with 42 -high x 24 -wide clearance

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=1042 U=42 W=4 (4 min)
RL=95/-100

R=957 U=29 W=4 (4' min)

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0) 12 03 04 0326 14 QTY. 3 FL/-/4/-/-/R/- Scale =.3125"/Ft.

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TC LL	20 0 PSF	REF	R487-- 12928
TC DL	7 0 PSF	DATE	06/12/13
BC DL	10 0 PSF	DRW	HCUSR487 13163007
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	15480
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UX0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-186--Trout River Builder Tash Residence -- 287 SW Hunter road, - V2 14'1 12 Valley)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

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MWFRS loads based on trusses located at least 7 50 ft from roof edge

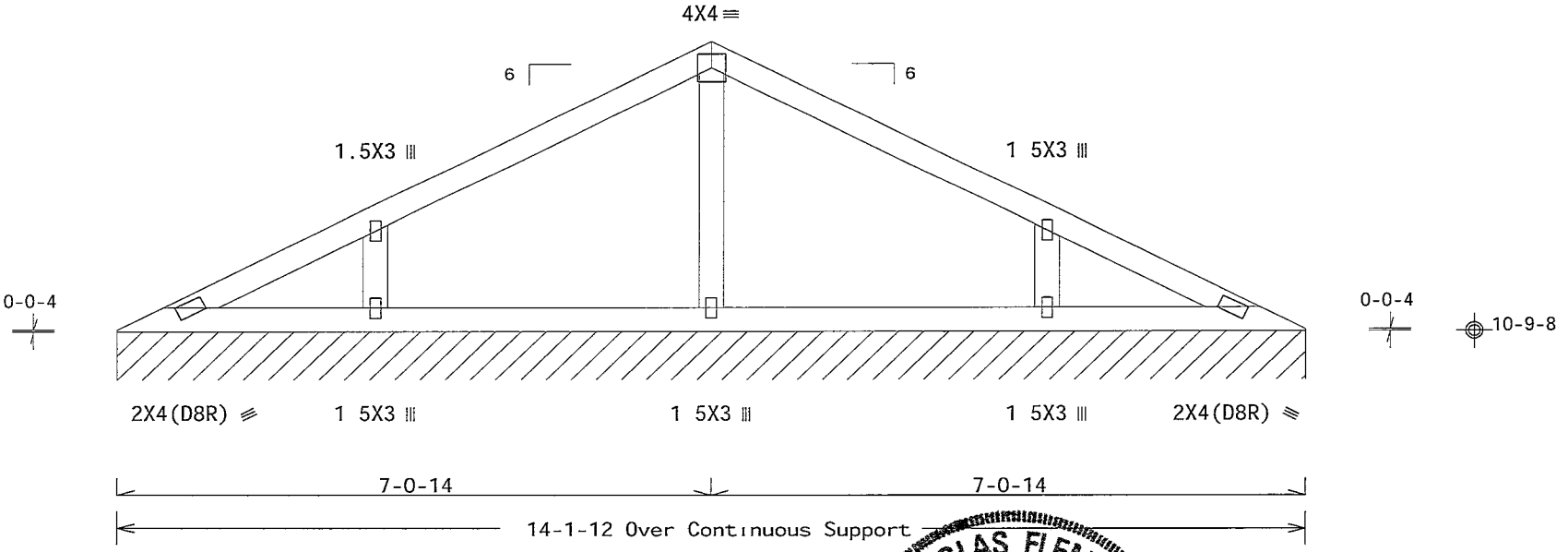
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

See DWG VAL160100212 for valley details



R=76 PLF U=2 PLF W=14-1-12
RL=3/-3 PLF

Design Crit FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 12 03 04 0326 14 QTY:

PLT TYP. Wave

FL/-/4/-/-/R/- Scale =.5"/Ft.

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TC LL	20.0 PSF	REF	R487-- 12929
TC DL	7 0 PSF	DATE	06/12/13
BC DL	10.0 PSF	DRW	HCUSR487 13163008
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT LD	37.0 PSF	SEQN-	15481
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UX0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-186--Trout River Builder Tash Residence -- 287 SW Hunter road, - V4 6'1'12 Valley)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with '12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

See DWG VAL160100212 for valley details

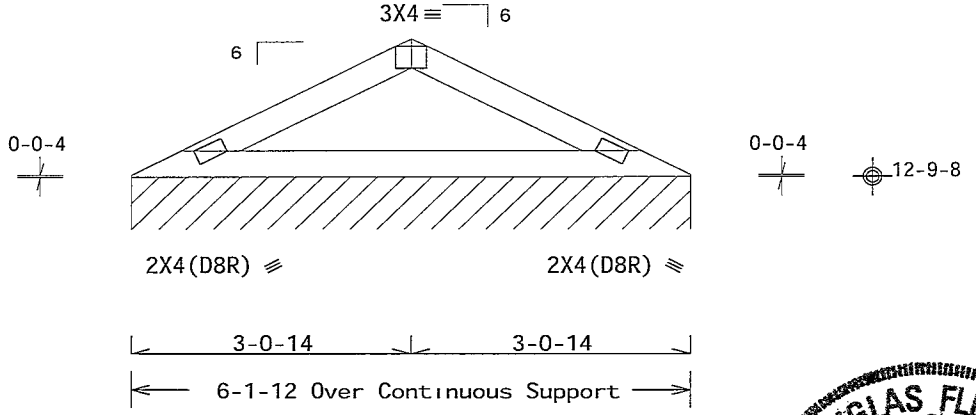
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge



R=75 PLF U=0 PLF W=6-1-12

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

12 03 04.0326.14 / CTY:

FL/-/4/-/-/R/-

Scale =.5"/Ft.

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ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any deviation from th s des any failure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pp ng installation brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos tions A seal on th s draw ng or cover page l st ng th s draw ng nd cates acceptance of professional eng neering respons b l ty solely for the des gn shown The su tabil ty and use of this des gn for any structure is the respons b l ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more informat on see Th s job s general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

TC LL	20.0 PSF	REF	R487-- 12930
TC DL	7.0 PSF	DATE	06/12/13
BC DL	10.0 PSF	DRW	HCUSR487 13163009
BC LL	0.0 PSF		HC-ENG JB/DF
TOT LD.	37 0 PSF	SEQN-	15482
DUR.FAC.	1 25		
SPACING	24.0"		JREF- 1UX0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-186--Trout River Builder Tash Residence -- 287 SW Hunter road, - V1 18'1'12 Valley)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

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MWFRS loads based on trusses located at least 7 50 ft from roof edge

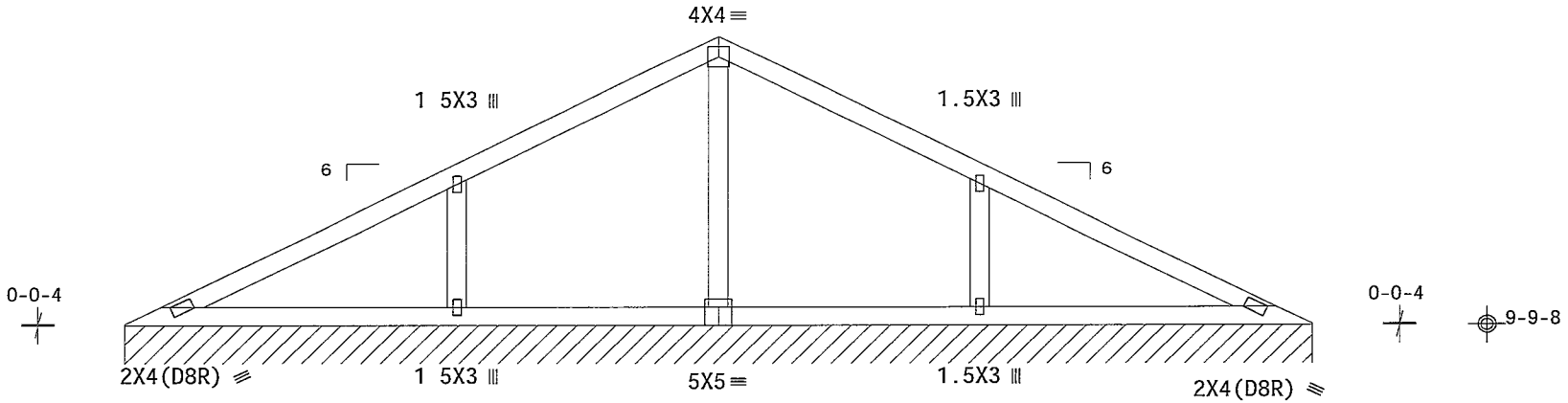
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

See DWG VAL160100212 for valley details



R=76 PLF U=2 PLF W=18-1-12
RL=3/-3 PLF

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 06/12/2013

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 0326.14 QTY: FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
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Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 12931
TC DL	7.0 PSF	DATE	06/12/13
BC DL	10 0 PSF	DRW	HCUSR487 13163010
BC LL	0.0 PSF		HC-ENG JB/DF
TOT LD	37 0 PSF	SEQN-	15483
DUR.FAC.	1.25		
SPACING	24 0"	JREF-	1UX0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-186--Trout River Builder Tash Residence -- 287 SW Hunter road, - V3 10'1 12 Valley)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with 12A use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

MWFRS loads based on trusses located at least 7 50 ft from roof edge

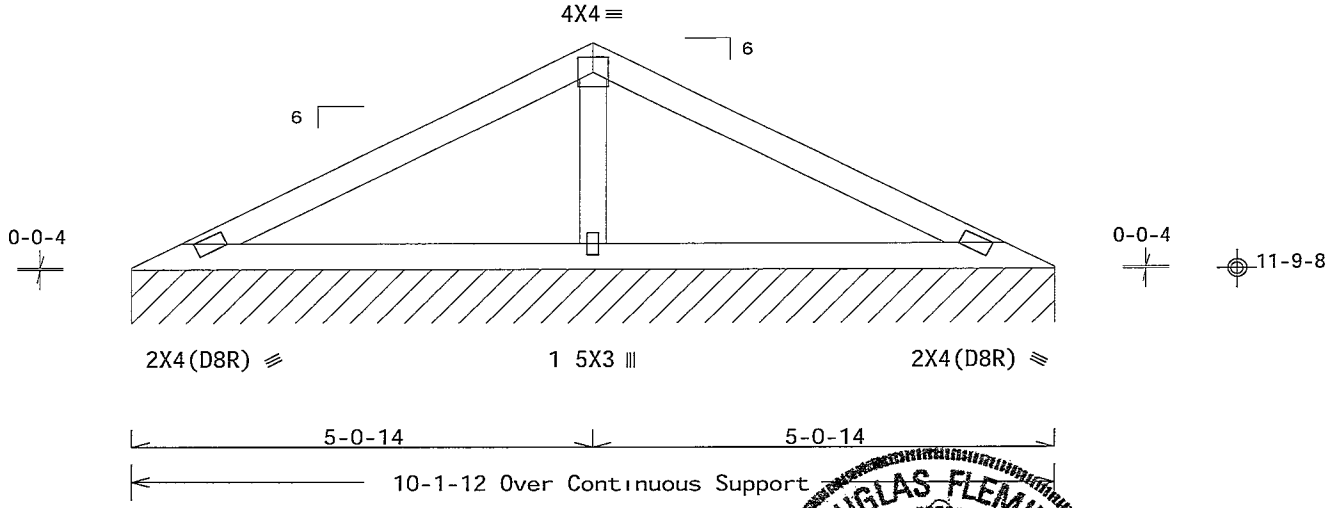
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

See DWG VAL160100212 for valley details



R=75 PLF U=1 PLF W=10-1-12
RL=3/-3 PLF

Design Crit FBC2010Res/TPI-2007(STD) 12 03.04 0326.14 /0 Y+1 FL/-/4/-/-/R/- Scale =.5"/Ft.

PLT TYP. Wave FT/RT=10%(0%)/0(0)

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 06/12/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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ITW Bu lding Components Group Inc (ITWBCG) shall not be responsible for any dev ation from this design any fa lure to build the truss n conformance with ANSI/TPI 1 or for handling sh pping installation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherw se Refer to drawings 160A-Z for standard plate pos t ons A seal on th s drawing or cover page listing th s drawing indicates acceptance of professional eng neer ng respons bility solely for the design shown The suitabil ty and use of th s des gn for any structure s the respons bility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW-BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www ccsafe org

TC LL	20.0 PSF	REF	R487-- 12932
TC DL	7.0 PSF	DATE	06/12/13
BC DL	10 0 PSF	DRW	HCUSR487 13163011
BC LL	0.0 PSF		HC-ENG JB/DF
TOT LD.	37 0 PSF	SEQN-	15484
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UX0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-186--Trout River Builder Tash Residence -- 287 SW Hunter road, - V 22 1'12 Valley)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A' use design values approved 1/5/2012 by ALSC

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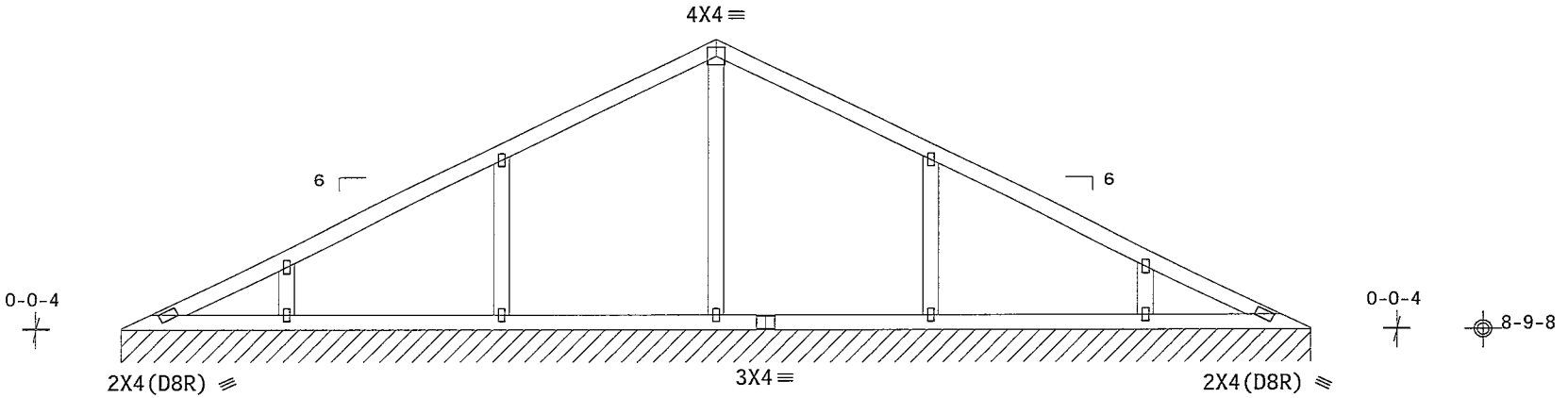
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

See DWG VAL160100212 for valley details



R=76 PLF U=2 PLF W=22-1-12
RL=3/-3 PLF

Note All Plates Are 1.5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12 03 04 0326 14 QTY 1 FL/-/4/-/-/R/- Scale =.3125"/Ft

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ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 12933
TC DL	7.0 PSF	DATE	06/12/13
BC DL	10.0 PSF	DRW	HCUSR487 13163012
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	15485
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UX0487_Z02

Gable Stud Reinforcement Detail

ASCE 7-10 140 mph Wind Speed 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

☐ 130 mph Wind Speed 15' Mean Height Partially Enclosed Exposure C Kzt = 1.00
☐ or 100 mph Wind Speed 15' Mean Height Enclosed Exposure D Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No. Brace	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
24" o.c.	SPF/HF	#1 / #2	4'-10"	8'-2"	8'-6"	9'-8"	10'-1"	11'-6"	12'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		#3	4'-7"	7'-9"	8'-3"	9'-7"	9'-11"	11'-5"	11'-10"	14'-0"	14'-0"	14'-0"	14'-0"
		Stud	4'-7"	8'-1"	8'-4"	9'-7"	9'-11"	11'-5"	11'-10"	14'-0"	14'-0"	14'-0"	14'-0"
		Standard	4'-7"	8'-1"	8'-4"	9'-7"	9'-11"	11'-5"	11'-10"	14'-0"	14'-0"	14'-0"	14'-0"
	SP/DFL	#1	4'-11"	8'-3"	8'-7"	9'-9"	10'-1"	11'-7"	12'-1"	14'-0"	14'-0"	14'-0"	14'-0"
		#2	4'-10"	8'-2"	8'-6"	9'-8"	10'-1"	11'-6"	12'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		#3	4'-7"	6'-11"	7'-4"	9'-3"	9'-10"	11'-5"	11'-10"	14'-0"	14'-0"	14'-0"	14'-0"
		Stud	4'-7"	6'-11"	7'-4"	9'-3"	9'-10"	11'-5"	11'-10"	14'-0"	14'-0"	14'-0"	14'-0"
		Standard	4'-7"	6'-0"	6'-4"	8'-0"	8'-6"	10'-10"	11'-7"	12'-6"	13'-5"	14'-0"	14'-0"
16" o.c.	SPF/HF	#1 / #2	5'-6"	9'-5"	9'-9"	11'-1"	11'-6"	13'-2"	13'-9"	14'-0"	14'-0"	14'-0"	14'-0"
		#3	5'-3"	9'-3"	9'-9"	10'-11"	11'-4"	13'-0"	13'-7"	14'-0"	14'-0"	14'-0"	14'-0"
		Stud	5'-3"	9'-3"	9'-7"	10'-11"	11'-4"	13'-0"	13'-7"	14'-0"	14'-0"	14'-0"	14'-0"
		Standard	5'-3"	9'-3"	9'-7"	10'-11"	11'-4"	13'-0"	13'-7"	14'-0"	14'-0"	14'-0"	14'-0"
	SP/DFL	#1	5'-8"	9'-5"	9'-10"	11'-2"	11'-7"	13'-3"	13'-10"	14'-0"	14'-0"	14'-0"	14'-0"
		#2	5'-6"	9'-5"	9'-9"	11'-1"	11'-6"	13'-2"	13'-9"	14'-0"	14'-0"	14'-0"	14'-0"
		#3	5'-3"	8'-6"	9'-0"	10'-11"	11'-4"	13'-0"	13'-7"	14'-0"	14'-0"	14'-0"	14'-0"
		Stud	5'-3"	8'-6"	9'-0"	10'-11"	11'-4"	13'-0"	13'-7"	14'-0"	14'-0"	14'-0"	14'-0"
		Standard	5'-3"	7'-4"	7'-10"	9'-9"	10'-5"	13'-0"	13'-7"	14'-0"	14'-0"	14'-0"	14'-0"
12" o.c.	SPF/HF	#1 / #2	6'-1"	10'-4"	10'-8"	12'-2"	12'-8"	13'-2"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		#3	5'-9"	10'-2"	10'-7"	12'-0"	12'-6"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		Stud	5'-9"	10'-2"	10'-7"	12'-0"	12'-6"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		Standard	5'-9"	10'-2"	10'-7"	12'-0"	12'-6"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
	SP/DFL	#1	6'-2"	10'-5"	10'-9"	12'-3"	12'-9"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		#2	6'-1"	10'-4"	10'-8"	12'-2"	12'-8"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		#3	5'-9"	9'-9"	10'-5"	12'-0"	12'-6"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		Stud	5'-9"	9'-9"	10'-5"	12'-0"	12'-6"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		Standard	5'-9"	8'-6"	9'-0"	11'-3"	12'-1"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"

Bracing Group Species and Grades

Group A

Spruce Pine Fir	
#1 / #2	Standard
#3	Stud

Hem Fir	
#2	Stud
#3	Standard

Douglas Fir Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B

Hem Fir
#1 & Btr
#1

Douglas Fir Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress Rated Board)

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load)

Gable end supports load from 4'-0" outlookers with 2'-0" overhang or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January 2012 ruling

Attach "L" braces with 10d (0.128"x3.0" min) nails

* For (1) "L" brace space nails at 2" o.c. in 18" end zones and 4" o.c. between zones

** For (2) "L" braces space nails at 3" o.c. in 18" end zones and 6" o.c. between zones

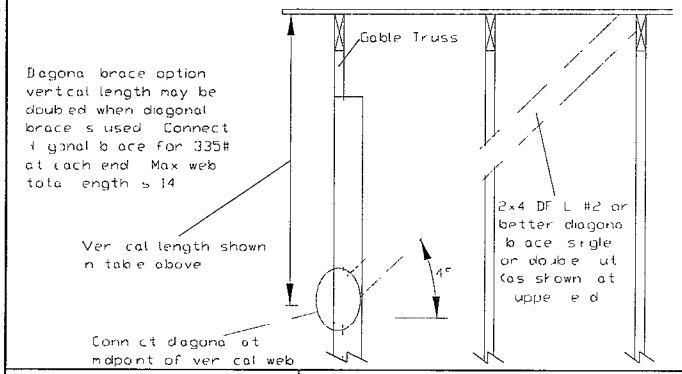
"L" bracing must be a minimum of 80% of web member length

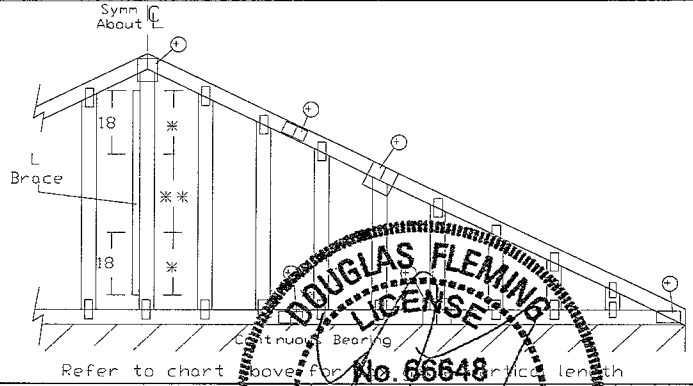
Gable Vertical Plate Sizes

Vertical Length	No. Splice
Less than 4'-0"	1X4 or 2X3
Greater than 4'-0" but less than 11'-6"	2X4
Greather than 11'-6"	2.5X4

+ Refer to common truss design for peak, splice and heel plates

Refer to the Building Designer for conditions not addressed by this detail





DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 06/12/2013

Jun 12 '13

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ITW Building Components Group Inc.

Earth City, MO 63045

MAX TOT LD 60 PSF

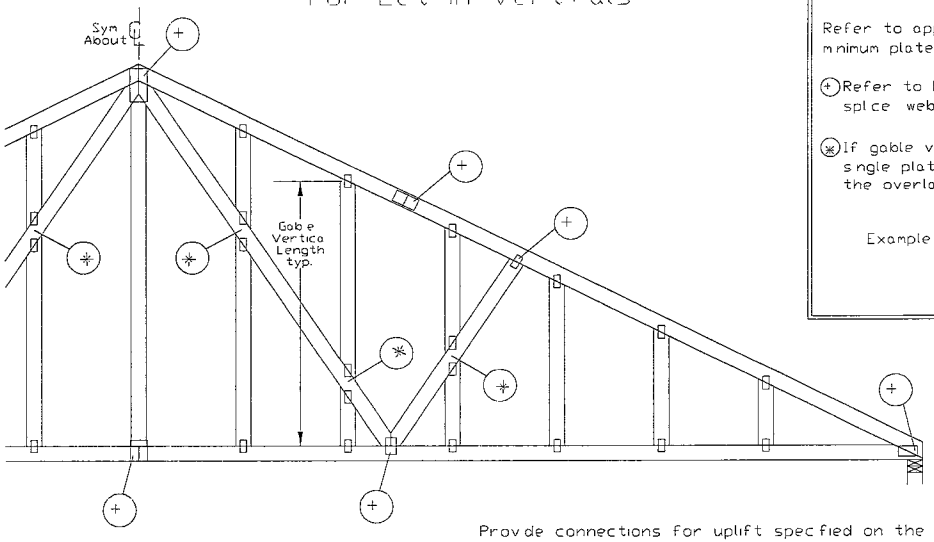
MAX SPACING 24.0

REF ASCE7-10-GAB12015

DATE 2/14/12

DRWG A12015ENC100212

Gable Detail
For Let-in Verticals

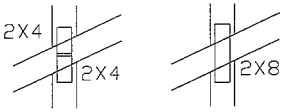


Gable Truss Plate Sizes

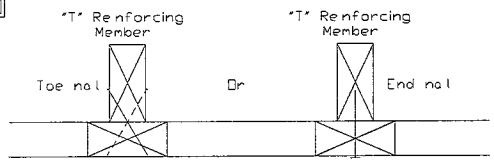
Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs

(+) Refer to Engineered truss design for peak splice web and heel plates

(*) If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web



"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members multiply "T" increase by length (based on appropriate ITW gable detail)

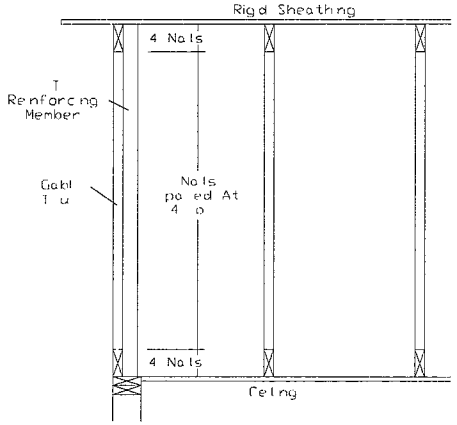
Maximum allowable T reinforced gable vertical length is 14' from top to bottom chord

"T" reinforcing member material must match size species and grade of the "L" reinforcing member

Web Length Increase w/ T Brace

"T" Reinf Mbr Size	"T" Increase
2x4	30"
2x6	20"

Example
- ASCE 7-10 Wind Speed 120 mph
- Mean Roof Height 30 ft Kzt = 1.00
- Gable Vertical 24" oc SP #3
- "T" Reinforcing Member Size = 2x4
- "T" Brace Increase (From Above) = 30" = 1'-6"
- (1) 2x4 "L" Brace Length = 8'-7"
- Maximum T Reinforced Gable Vertical Length = 1'-6" x 8'-7" = 11'-2"



Provide connections for uplift specified on the engineered truss design

Attach each "T" reinforcing member with

End Driven Nails
- 10d Common (0.148" x 3") nails at 4" oc plus
- (4) nails in the top and bottom chords

Toenailed Nails
- 10d Common (0.148"x3") nails at 4" oc plus
- (4) toenails in the top and bottom chords

This detail to be used with the appropriate ITW gable detail for ASCE wind load

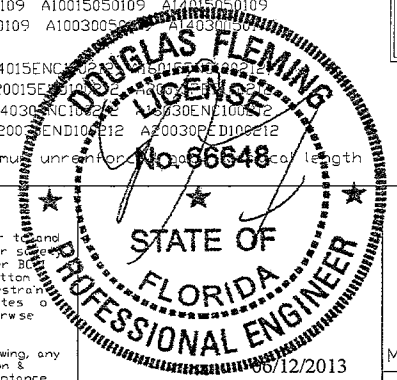
ASCE 7-98 Gable Detail Drawings
A13015980109 A12015980109 A11015980109 A10015980109
A13030980109 A12030980109 A11030980109 A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109 A12015020109 A11015020109 A10015020109 A14015020109
A13030020109 A12030020109 A11030020109 A10030020109 A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109 A12015050109 A11015050109 A10015050109 A14015050109
A13030050109 A12030050109 A11030050109 A10030050109 A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212 A12015ENC100212 A14015ENC100212 A15015ENC100212
A18015ENC100212 A20015ENC100212 A20015END100212 A20015END100212
A11530ENC100212 A12030ENC100212 A14030ENC100212 A15030ENC100212
A18030ENC100212 A20030ENC100212 A20030END100212 A20030END100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length



Jun 12 '13

Building Components Group Inc.

Earth City MO 63045

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ITWBCG: www.twbcg.com, TPI: www.tpinst.org WTCA: www.sbcindustry.org, ICC: www.iccsafe.org

MAX TOT LD	60 PSF
DUR FAC	ANY
MAX SPACING	24.0

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212

ASCE 7-10 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph 30ft Mean Hgt ASCE 7-10 Enclosed Exp C or
100 mph 30ft Mean Hgt ASCE 7-10, Enclosed Exp D or
100 mph, 30ft Mean Hgt ASCE 7-10, Part Enclosed Exp C
Kzt - 1.00 Wind TC DL=5.0 psf Wnd BC DL=5.0 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber plates and other information not shown on this detail

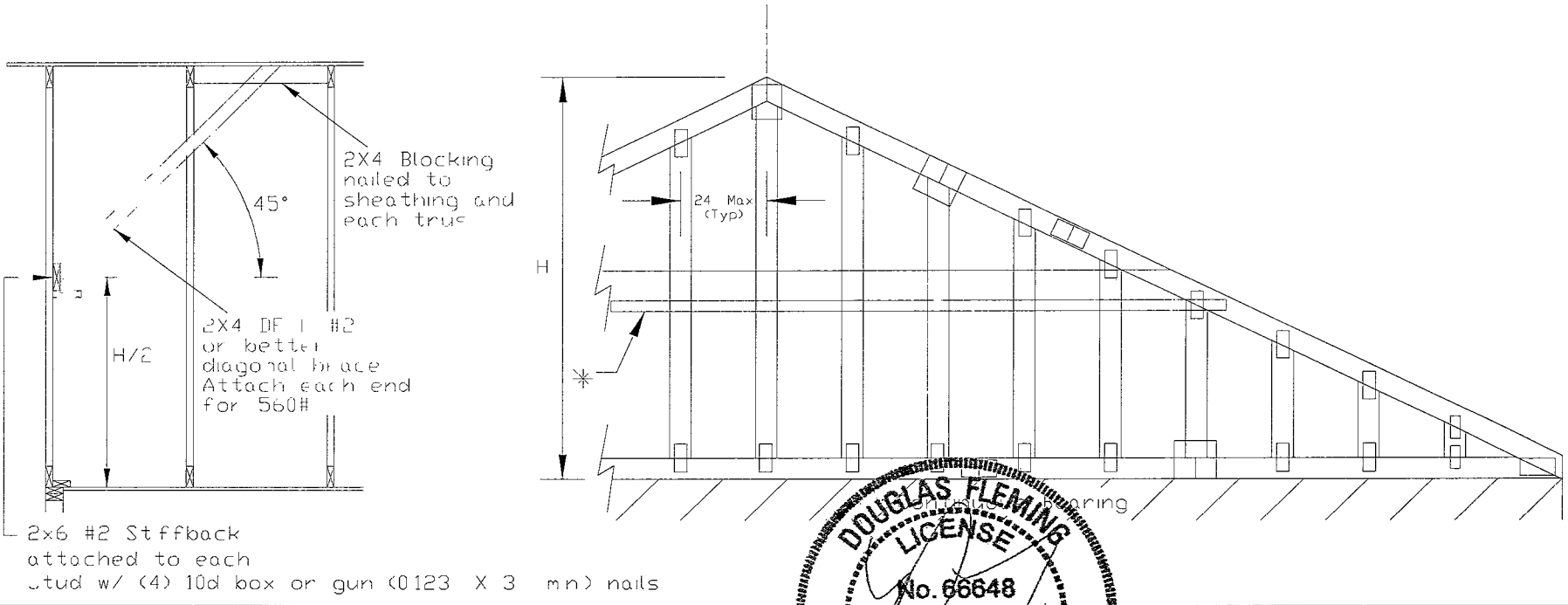
Nails 10d box or gun (0.128 x 3" min) nails

H Less than 4.6' - no stud bracing required

H Greater than 4.6' to 7.6' in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6.0' (see detail below or refer to DRWG A12030ENC10)

H Greater than 7.6' to 12.0' max
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4.0' (see detail below or refer to DRWG A12030ENC10)

✳ Optional 2x L reinforcement attached to stiffback with 10d box or gun (0.128" x 3" min) nails @ 6" o.c.



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06/12/2013

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MAX TOT LD 60 PSF

MAX SPACING

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212

Valley Detail - ASCE 7-10 160 mph 30' Mean Height, Enclosed, Exp C, Kzt=100

Top Chord 2x4 SP #2N SPF #1/#2 DF-L #2 or better
Bot Chord 2x4 SP #2N or SPF #1/#2 or better
Webs 2x4 SP #3 SPF #1/#2 DF-L #2 or better

** Attach each valley to every supporting truss with
(2) 16d box (0.135 x 3.5 nails toe-nailed for
ASCE 7-10 160 mph 30' Mean Height Enclosed
Building Exp C Wind TC DL=5 psf Kzt = 1.00
Or
ASCE 7-10 140 mph 30' Mean Height Enclosed
Building Exp D Wind TC DL 5 psf Kzt = 1.00

Unless specified on engineer's sealed design apply 1x4 T brace 80% length of web same species and SRB grade or better attached with 8d box (0.113 x 2.5") nails at 6" o.c. or continuous lateral bracing equally spaced for vertical valley webs greater than 7'-9"

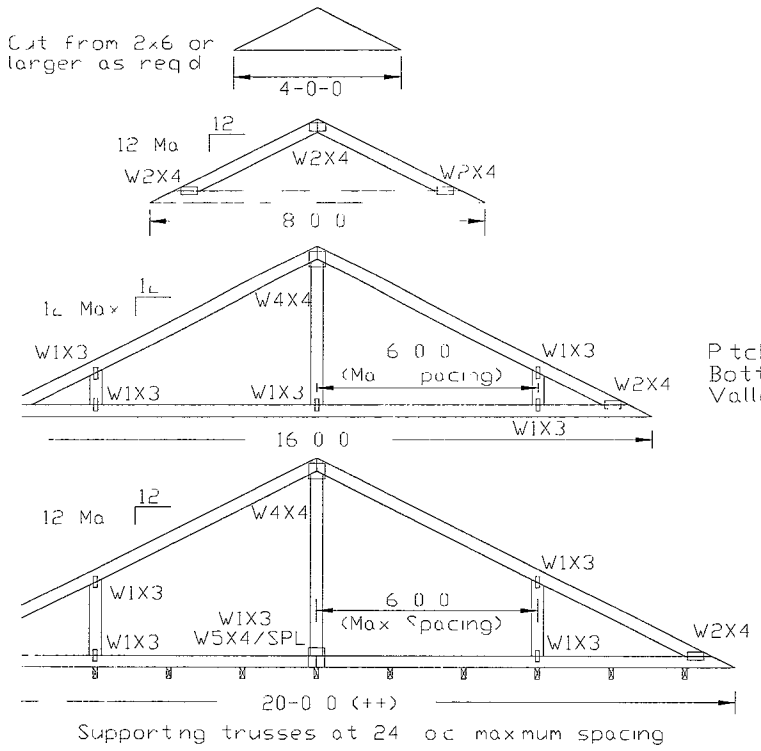
For verticals over 10'-0" tall, apply (2) 1x4 T braces, 80% length of web same species and SRB grade or better attached with 8d box (0.113 x 2.5") nails @ 6" o.c.

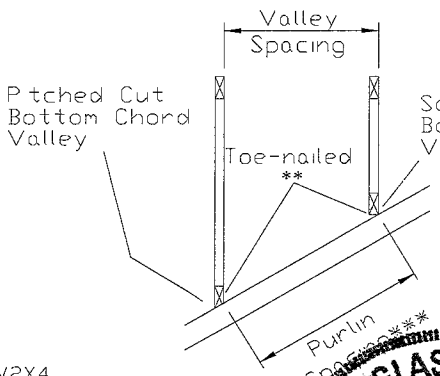
Top chord of truss beneath valley set must be braced with properly attached rated sheathing applied prior to valley truss installation
Or
Purlins at 24" o.c. or as otherwise specified on Engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design

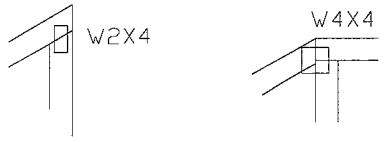
*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord

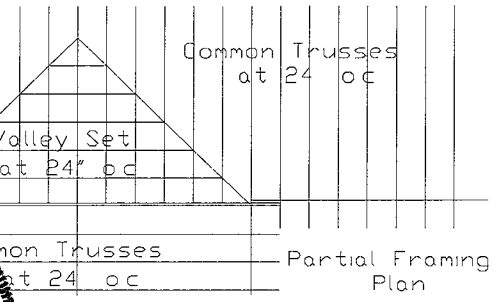
++ Larger spans may be built as long as the vertical height does not exceed 14'-0"

Bottom chord may be square or pitched cut as shown









DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06/12/2013

Jun 12 '13

Building Components Group Inc.
Earth City MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

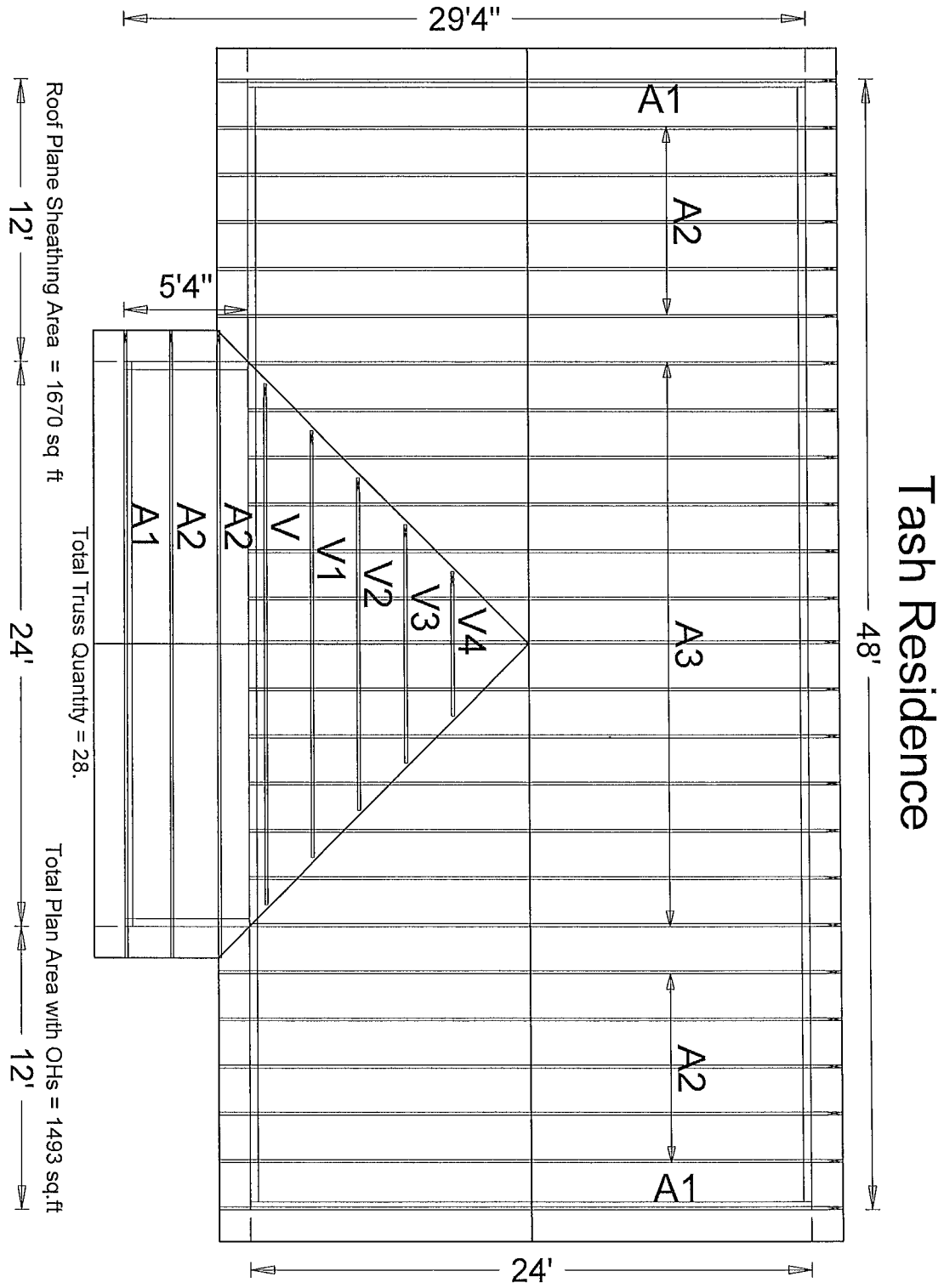
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10 as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see this job's general notes page and these web sites:
ITWBCG: www.twbcg.com TPI: www.tpinst.org WTCA: www.sbcindustry.org, ICC: www.iccsafe.org

TC LL	30	30	40PSF
TC DL	20	15	7PSF
BC DL	10	10	10 PSF
BC LL	0	0	0PSF
TOT LD	60	55	57PSF
DURFAC	1.25/1.33	1.15	1.15
SPACING		24.0	

REF	VALLEY DETAIL
DATE	2/14/12
DRWG	VAL160100212

COLUMBIA COUNTY BUILDING DEPARTMENT
PLANS EXAMINER
FILE COPY
Received for
Code Compliance



PAGE NO 1 OF 1	JOB NO 13-186	JOB DESCRIPTION Trout River Builder / Tash Residence ADDRESS 287 SW Hunter road Columbia County JOB #: 13-186 DESIGNER Jon Williams SALESMAN Curt V Burlingame	PlanName Created 06-11-2013	ANDERSON TRUSS COMPANY