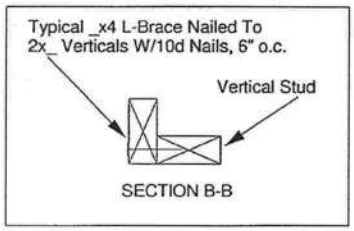
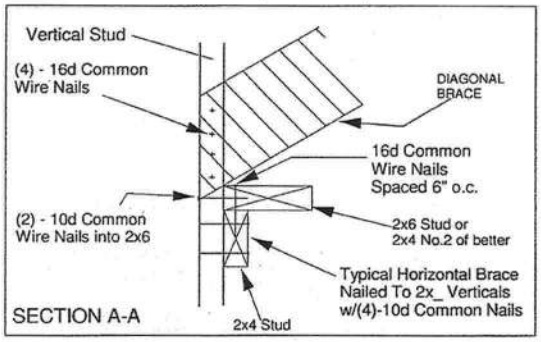
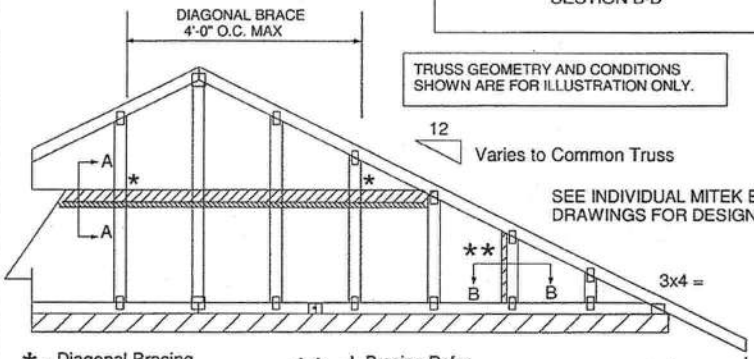
FEBRUARY 14, 2012	Standard Gable End Detail	ST-GE130-001

MiTek Industries, Inc.







TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

12 Varies to Common Truss

SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

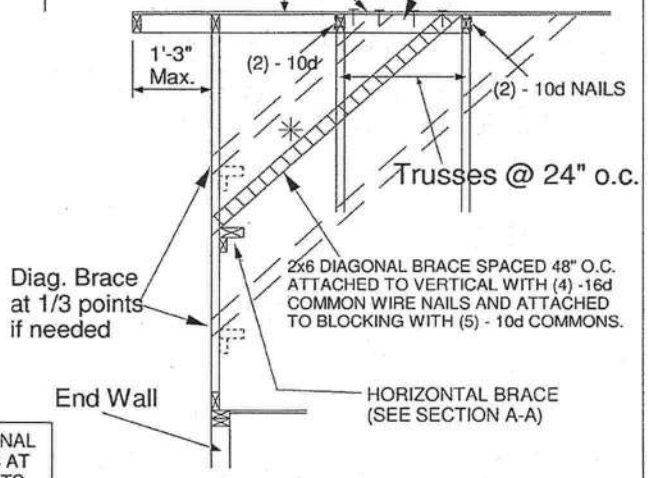
* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

24" Max

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d COMMON WIRE NAILS.

(4) - 8d NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK



NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SPF Std/Stud	12" O.C.	4-0-7	4-3-2	6-0-4	8-0-15	12-1-6
2x4 SPF Std/Stud	16" O.C.	3-7-0	3-8-4	5-2-10	7-1-15	10-8-15
2x4 SPF Std/Stud	24" O.C.	2-11-1	3-0-2	4-3-2	5-10-3	8-9-4

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d common wire nails 8in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

1109 COASTAL BAY
BOYNTON BC, FL 33435

FEBRUARY 8, 2008	LATERAL BRACING RECOMMENDATIONS	ST-STRGBCK

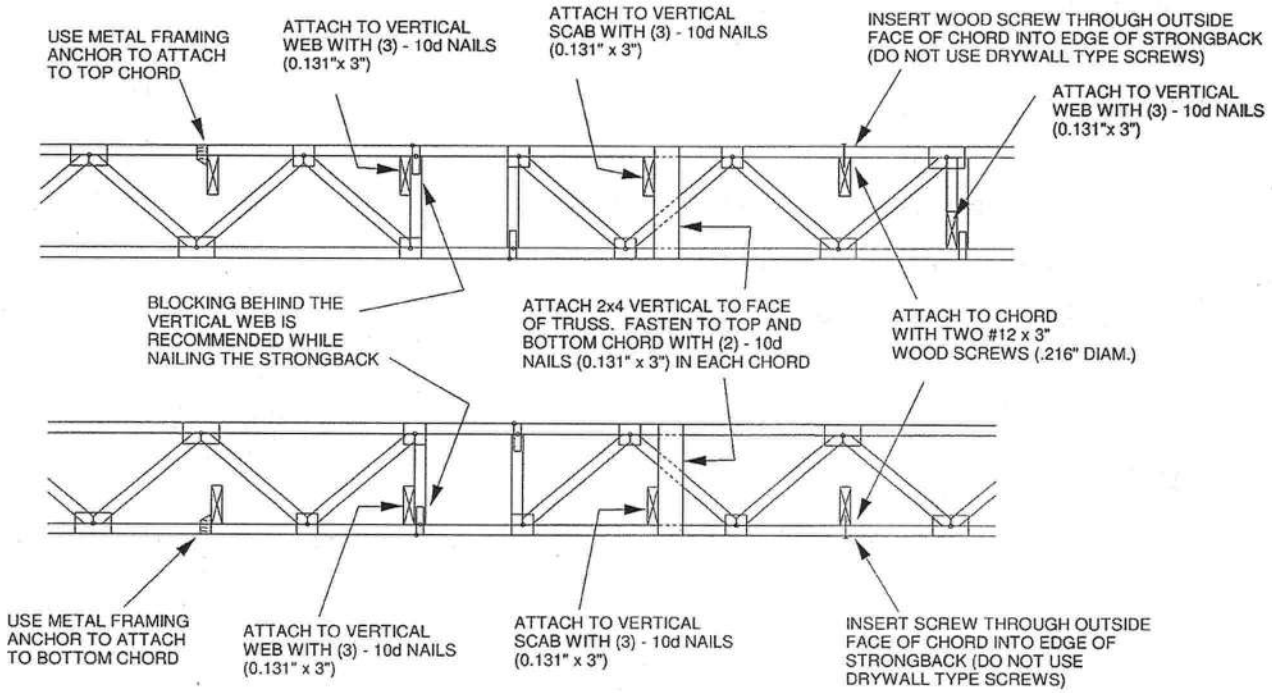
MiTek Industries, Chesterfield, MO

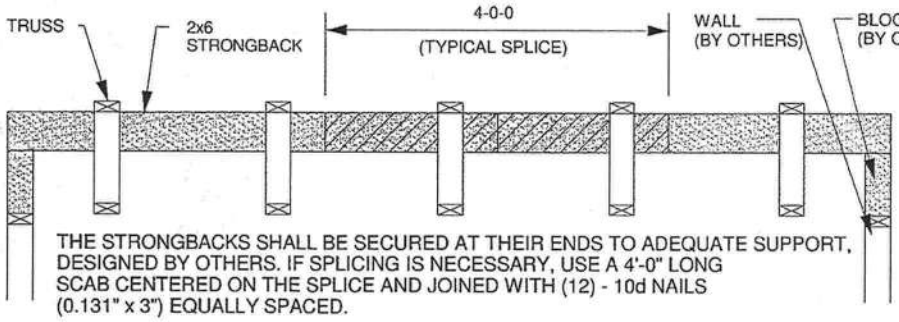
MiTek Industries, Inc.

TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





THE STRONGBACKS SHALL BE SECURED AT THEIR ENDS TO ADEQUATE SUPPORT, DESIGNED BY OTHERS. IF SPLICING IS NECESSARY, USE A 4'-0" LONG SCAB CENTERED ON THE SPLICE AND JOINED WITH (12) - 10d NAILS (0.131" x 3") EQUALLY SPACED.

ALTERNATE METHOD OF SPLICING:
OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d NAILS (0.131" x 3") STAGGERED AND EQUALLY SPACED.
(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)

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August 10, 2010	T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY	ST - T-BRACE 2

MiTek Industries, Inc.

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

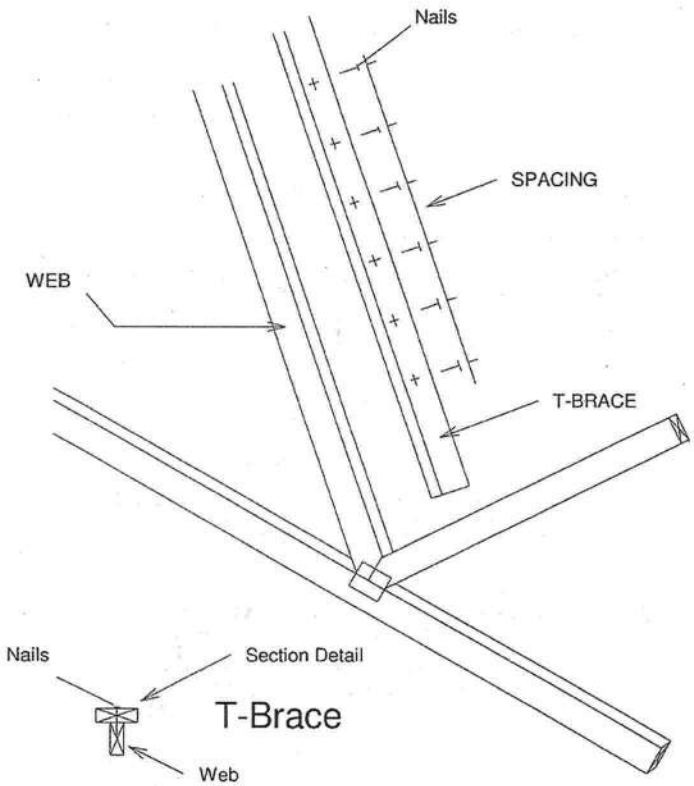
Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

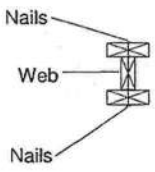
Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

	Brace Size for One-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

	Brace Size for Two-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.





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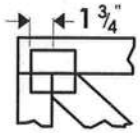
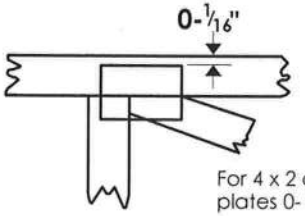
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

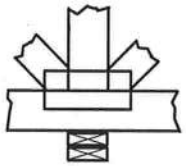
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

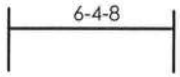


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

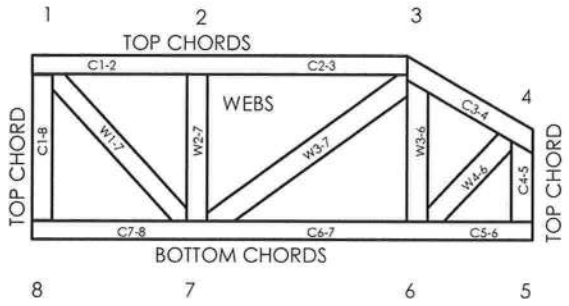
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B,
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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Boynton, FL 33435

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	I6332624
463946	T14	Hip Truss	1	2	Job Reference (optional)	

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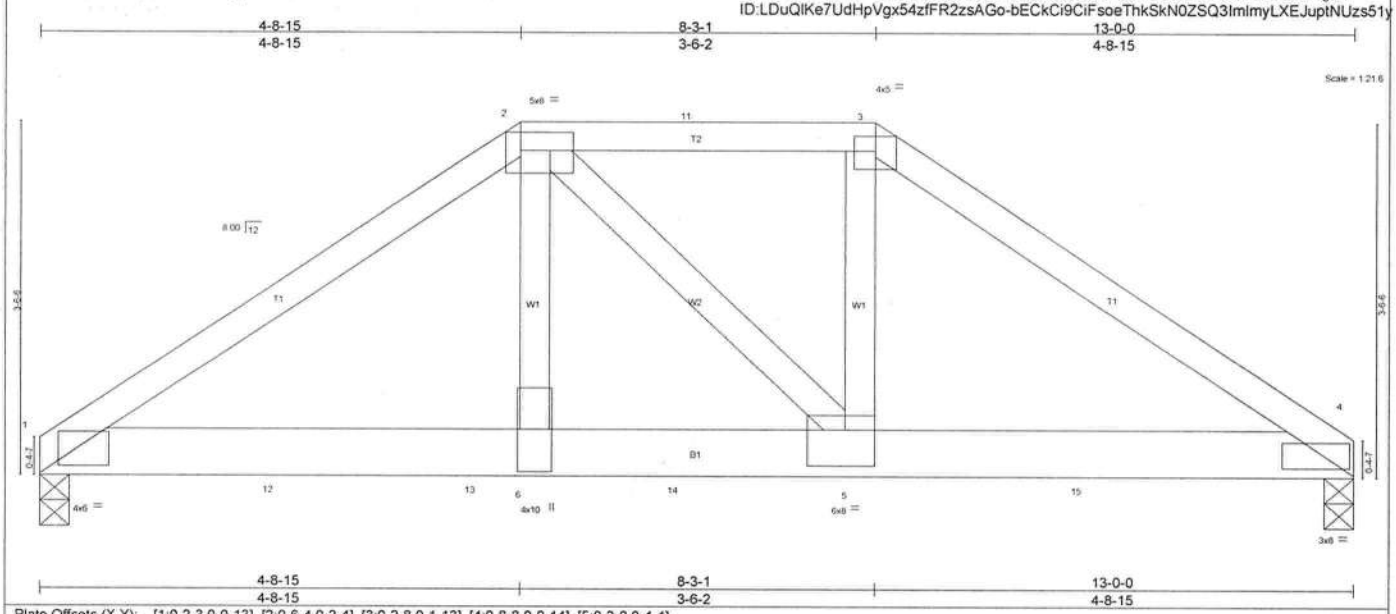


Plate Offsets (X,Y): [1:0-2-3,0-0-13], [2:0-6-4,0-2-4], [3:0-2-8,0-1-13], [4:0-8-8,0-0-14], [5:0-3-8,0-4-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.07	6-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.14	6-8	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.51	Horz(TL)	0.03	4	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 135 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SYP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

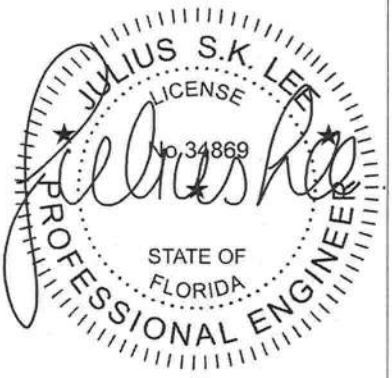
REACTIONS (lb/size) 1=3563/0-3-8 (min. 0-2-8), 4=3453/0-3-8 (min. 0-2-7)
Max Horz 1=-80(LC 6)
Max Uplift 1=-1097(LC 8), 4=-1228(LC 9)
Max Grav 1=4227(LC 2), 4=4097(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-5097/1351, 2-11=-4186/1198, 3-11=-4186/1198, 3-4=-5009/1359
BOT CHORD 1-12=-1098/4122, 12-13=-1098/4122, 6-13=-1098/4122, 6-14=-1123/4237, 5-14=-1123/4237, 5-15=-1081/4074, 4-15=-1081/4074
WEBS 2-6=-537/2500, 3-5=-547/2429

NOTES (12-14)
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=1097, 4=1228.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 40 lb down and 113 lb up at 4-8-15, and 40 lb down and 113 lb up at 6-6-0, and 40 lb down and 113 lb up at 8-3-1 on top chord, and 1041 lb down and 250 lb up at 0-4-12, 1041 lb down and 250 lb up at 2-4-12, 1041 lb down and 250 lb up at 4-4-12, 71 lb down at 4-8-15, 1041 lb down and 250 lb up at 6-4-12, 24 lb down at 6-6-0, 71 lb down at 8-2-4, 1040 lb down and 250 lb up at 8-4-12, and 1042 lb down and 251 lb up at 10-4-12, and 1039 lb down and 429 lb up at 12-4-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
13) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Continued on page 2



January 24,2013

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	
463946	T13	COMMON TRUSS	2	1	Job Reference (optional)	I6332622

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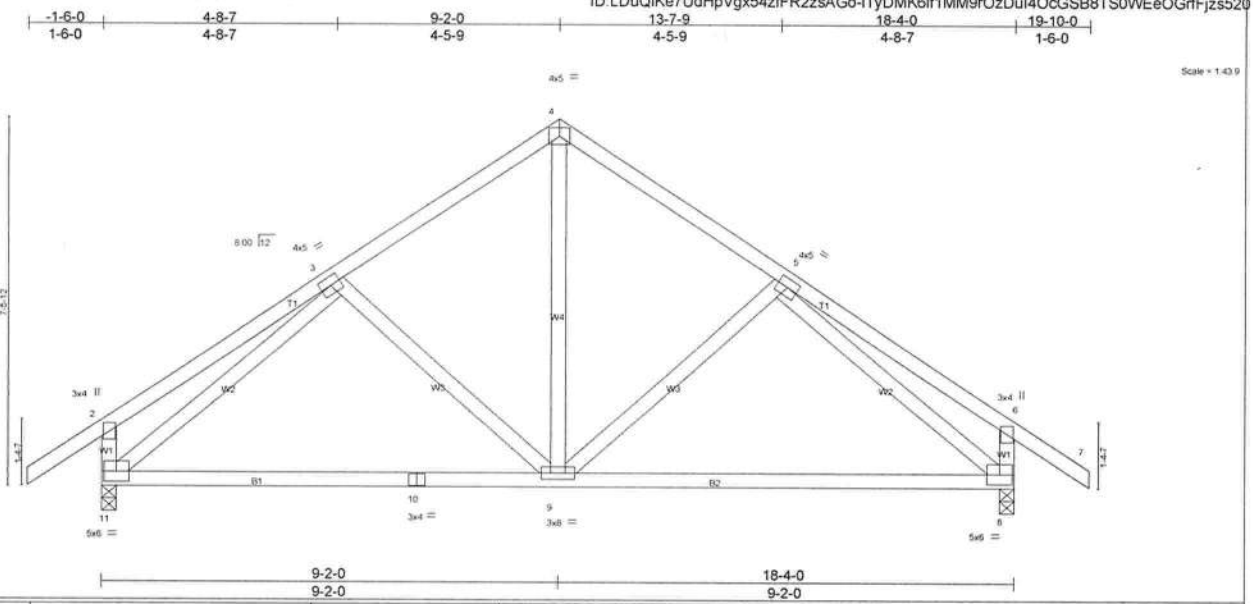


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.13	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.23	9-11	>922	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.02	8	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 113 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

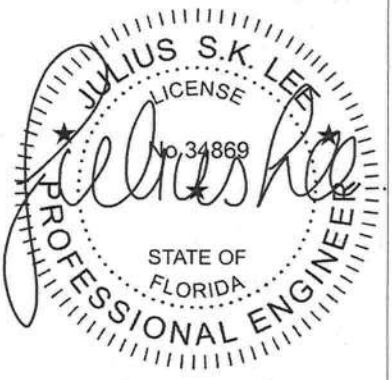
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 11=560/0-3-8 (min. 0-1-8), 8=560/0-3-8 (min. 0-1-8)
Max Horz 11=248(LC 9)
Max Uplift 11=-290(LC 12), 8=-290(LC 13)
Max Grav 11=666(LC 2), 8=666(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=-714/337, 4-5=-714/337, 2-11=-366/247, 6-8=-366/247
BOT CHORD 10-11=-226/639, 9-10=-226/639, 8-9=-157/642
WEBS 4-9=-205/418, 3-11=-697/243, 5-8=-697/243

NOTES (8-10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=290, 8=290.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 24,2013

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1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	I6332620
463946	T11G	GABLE	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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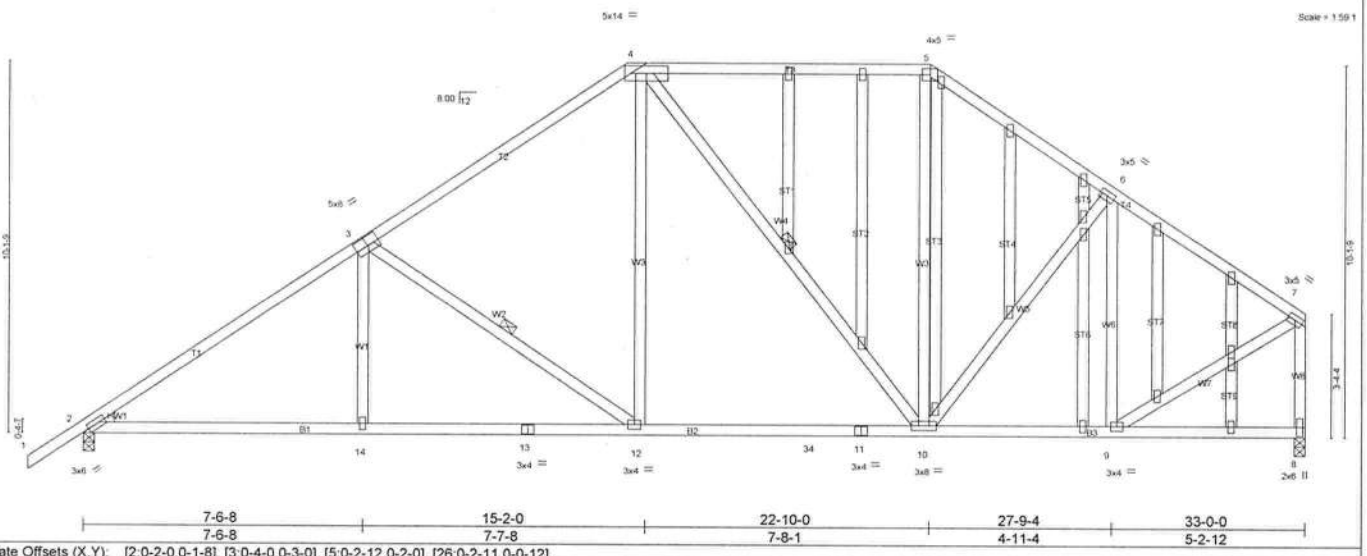

Plate Offsets (X,Y): [2:0-2-0,0-1-8], [3:0-4-0,0-3-0], [5:0-2-12,0-2-0], [26:0-2-11,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.15	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.24	10-12	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 273 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except*
W4: 2x4 SP No.2
OTHERS 2x4 SP No.3
WEDGE
Left: 2x4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-3-12 oc bracing.
WEBS 1 Row at midpt 3-12, 4-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1026/0-3-8 (min. 0-1-12), 8=953/0-3-8 (min. 0-1-9)
Max Horz 2=335(LC 12)
Max Uplift2=-480(LC 12), 8=-369(LC 13)
Max Grav 2=1141(LC 2), 8=1043(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1982/764, 3-4=-1533/685, 4-5=-1105/601, 5-6=-1333/642, 6-7=-1174/496, 7-8=-1312/547
BOT CHORD 2-14=-1112/2213, 13-14=-646/1597, 12-13=-646/1597, 12-34=-348/1075, 11-34=-348/1075, 10-11=-348/1075, 9-10=-334/889
WEBS 3-12=-645/404, 4-12=-180/553, 4-10=-298/170, 5-10=-110/311, 6-9=-505/230, 7-9=-375/1010

NOTES (12-14)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=480, 8=369.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
13) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

January 24,2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	
463946	T10G	GABLE	1	1		I6332618
					Job Reference (optional)	

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ID:LDuQlKe7UdHpVgx54zfFR2zsAGo-MW9KJd2ZqUj424V?RLivhYYXV7k1LD1vE_8uZVzs525

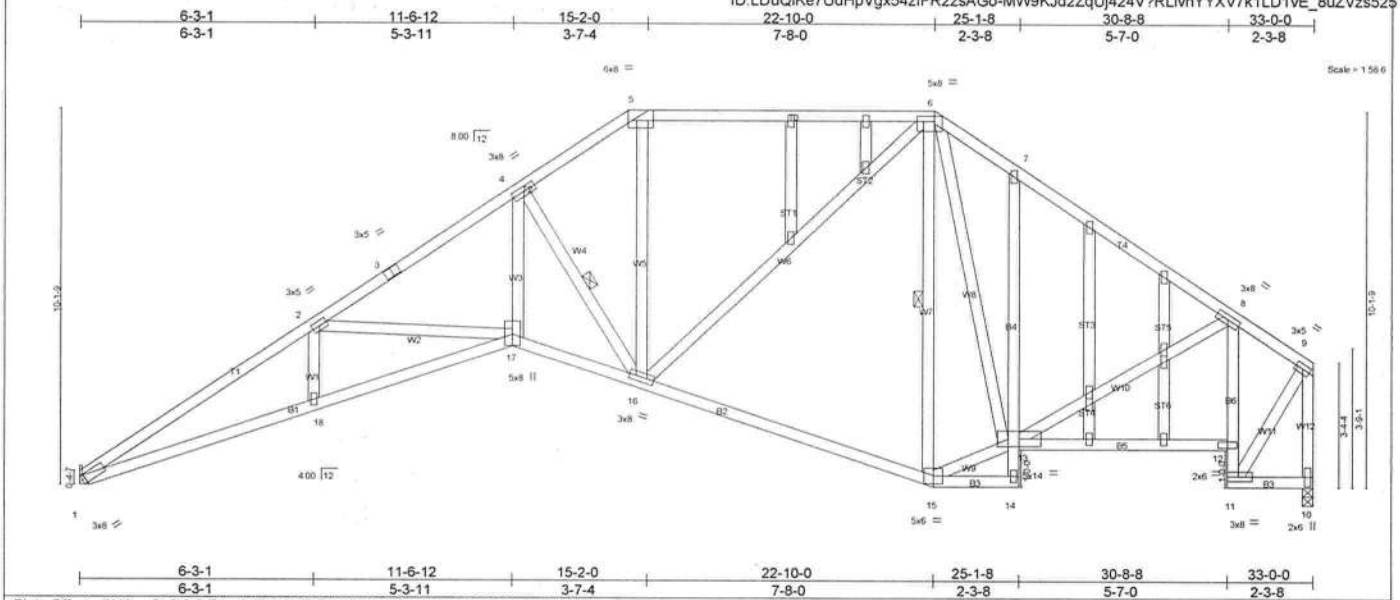

Plate Offsets (X,Y): [1:0-0-0,Edge], [5:0-6-15,Edge], [6:0-5-8,0-1-12], [17:0-3-11,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	0.28	17-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.47	17-18	>838	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.41	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 249 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2 *Except* T3: 2x4 SYP No.1
BOT CHORD	2x4 SP No.2 *Except* B4: 2x4 SP No.3
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-7-10 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 4-11-10 oc bracing.
WEBS	1 Row at midpt 4-16, 6-15

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=885/Mechanical, 10=884/0-3-8 (min. 0-1-8)
Max Horz 1=290(LC 9)
Max Uplift 1=-419(LC 12), 10=-371(LC 13)
Max Grav 1=1049(LC 2), 10=1047(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	1-2=-3615/1544, 2-3=-2956/1279, 3-4=-2887/1295, 4-5=-1797/888, 5-6=-1513/788, 6-7=-1402/770, 7-8=-1378/647, 8-9=-738/325, 9-10=-1366/587
BOT CHORD	1-1=-696/1352, 1-18=-1384/3128, 17-18=-1388/3137, 16-17=-955/2376, 15-16=-318/890, 7-13=-324/267, 12-13=-313/726, 11-12=-805/362, 8-12=-790/385
WEBS	2-17=-651/427, 4-17=-756/1833, 4-16=-1782/822, 5-16=-270/659, 6-16=-289/662, 6-15=-591/254, 13-15=-282/879, 6-13=-317/600, 8-13=-141/318, 9-11=-470/1086

NOTES (12-15)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=419, 10=371.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
13) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
15) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

January 24,2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	
463946	T09	Piggyback Base Truss	1	1	Job Reference (optional)	I6332616

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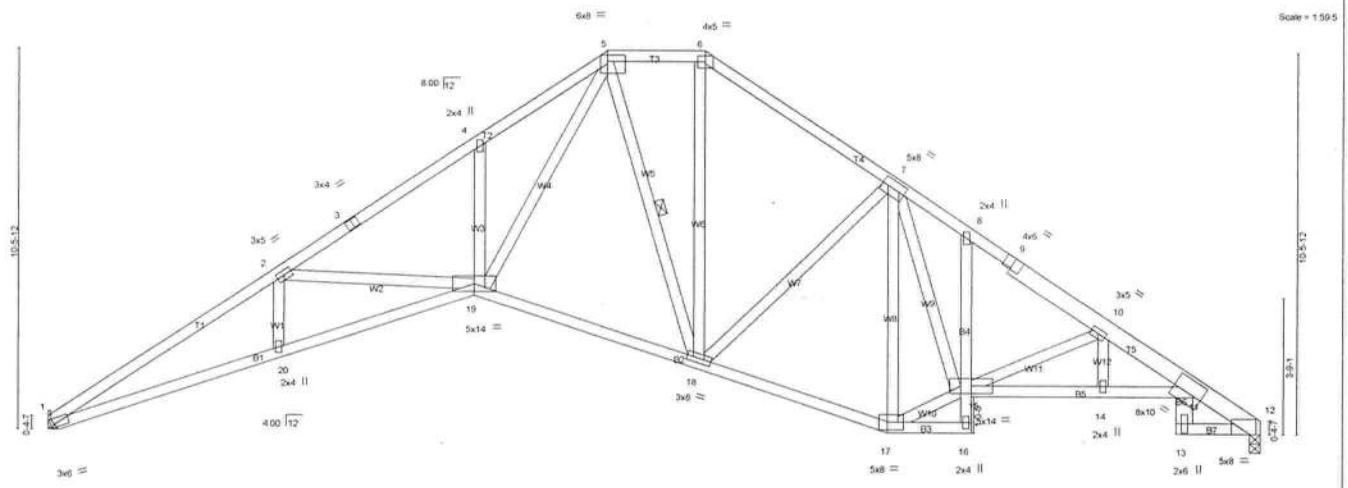
ID:LDuQlKe7UdHpVgx54zfFR2zsAGo-xxTBhb0hXZLVBdmRID9C4wx0Hwha8o2TY1vEzBzs528

6-3-1	11-6-12	15-2-0	17-10-0	22-10-0	25-1-8	28-8-10	30-8-8	33-0-0	34-6-0
6-3-1	5-3-11	3-7-4	2-8-0	5-0-0	2-3-8	3-7-2	1-11-14	2-3-8	1-6-0



6-3-1	11-6-12	17-10-0	22-10-0	25-1-8	28-8-10	30-8-8	33-0-0
6-3-1	5-3-11	6-3-4	5-0-0	2-3-8	3-7-2	1-11-14	2-3-8

Plate Offsets (X,Y): [5:0-5-12,0-2-0], [6:0-2-8,0-1-13], [9:0-3-0,Edge], [11:0-5-0,0-4-9], [12:0-6-2,0-0-0], [17:0-5-4,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	0.32	19	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.56	18-19	>706	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.52	12	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 217 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except*
T5: 2x6 SYP SS
BOT CHORD 2x4 SP No.2 *Except*
B4: 2x4 SP No.3, B5: 2x4 SYP No.1, B6: 2x6 SYP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-6-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-6-3 oc bracing.
WEBS 1 Row at midpt 5-18

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=888/Mechanical, 12=888/0-3-8 (min. 0-1-9)
Max Horz 1=-253(LC 8)
Max Uplift 1=-241(LC 12), 12=-238(LC 13)
Max Grav 1=1052(LC 2), 12=1052(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-3721/1414, 2-3=-3029/1117, 3-4=-2959/1140, 4-5=-3151/1325, 5-6=-1394/698, 6-7=-1682/760, 7-8=-2175/955, 8-9=-2209/934, 9-10=-2292/920, 10-11=-2982/1171, 11-12=-1088/429
BOT CHORD 1-20=-1127/3134, 19-20=-1127/3140, 18-19=-277/1283, 17-18=-426/1364, 14-15=-1028/2773, 11-14=-1025/2764, 11-13=-116/317, 12-13=-272/700
WEBS 2-19=-684/407, 4-19=-437/289, 5-19=-840/2233, 5-18=-443/194, 6-18=-260/635, 7-18=-269/239, 7-17=-953/325, 15-17=-396/1304, 7-15=-428/1123, 10-15=-1198/521

NOTES (10-13)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=241, 12=238.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
13) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

January 24,2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve
463946	T07	Piggyback Base Truss	1	1	I6332614

Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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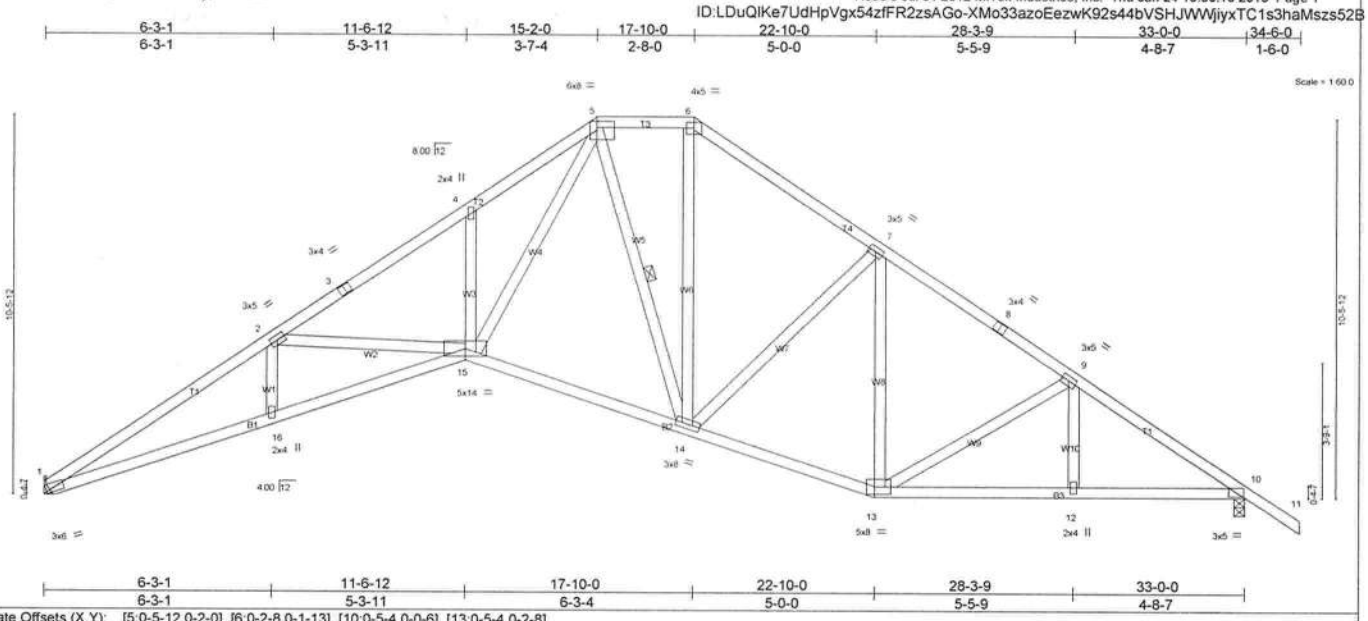

Plate Offsets (X,Y): [5:0-5-12,0-2-0], [6:0-2-8,0-1-13], [10:0-5-4,0-0-6], [13:0-5-4,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.25	15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.47	15-16	>846	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.32	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 197 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-10 oc bracing.
WEBS 1 Row at midpt 5-14

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=886/Mechanical, 10=962/0-3-8 (min. 0-1-11)
Max Horz 1=-269(LC 8)
Max Uplift 1=-240(LC 12), 10=-271(LC 13)
Max Grav 1=1050(LC 2), 10=1143(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-3741/1367, 2-3=-3048/1067, 3-4=-2979/1089, 4-5=-3171/1274, 5-6=-1394/686, 6-7=-1686/749, 7-8=-1710/759, 8-9=-1770/743, 9-10=-2053/801
BOT CHORD 1-16=-1046/3177, 15-16=-1045/3182, 14-15=-219/1312, 13-14=-384/1387, 12-13=-542/1636, 10-12=-542/1636
WEBS 2-15=-687/412, 4-15=-436/288, 5-15=-788/2258, 5-14=-450/187, 6-14=-261/647, 7-14=-281/262, 9-13=-378/209

NOTES (9-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=240, 10=271.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
12) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

January 24,2013

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	
463946	T06G	Piggyback Base Truss	1	1	Job Reference (optional)	I6332613

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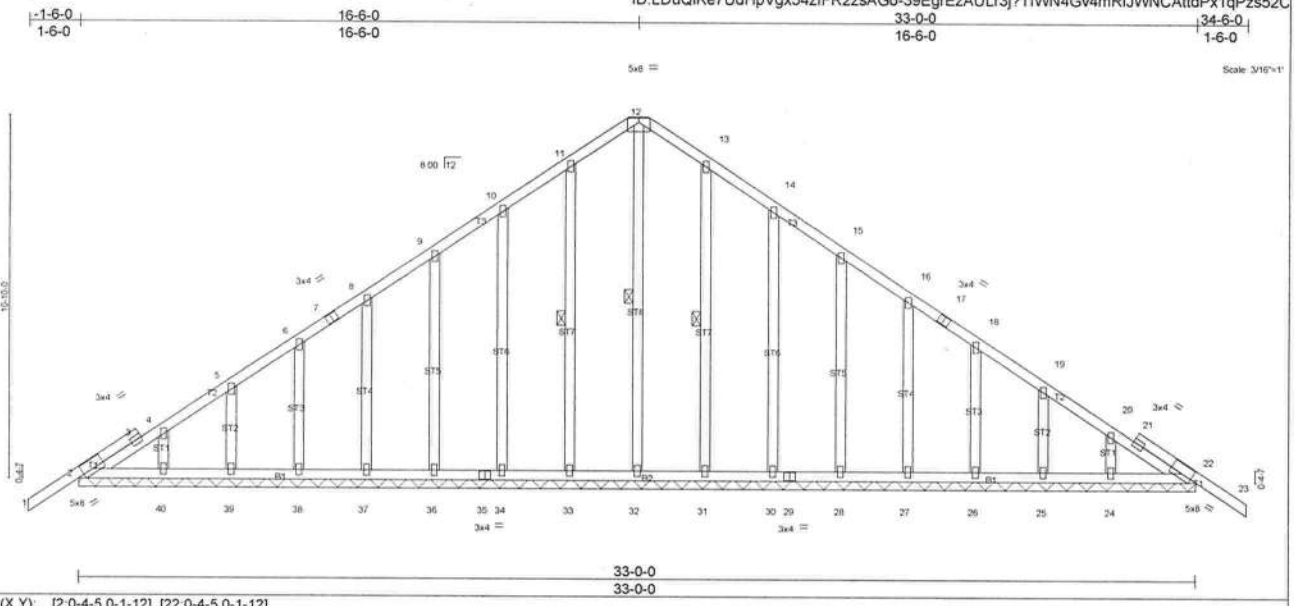

Plate Offsets (X,Y): [2:0-4-5,0-1-12], [22:0-4-5,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.01	23	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	23	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.23	Horz(TL)	0.01	22	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix)							Weight: 241 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 12-32, 11-33, 13-31

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 33-0-0.
(lb) - Max Horz 2=-341(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 2, 32, 40, 31, 24, 22 except 33=-104(LC 12), 34=-117(LC 12), 36=-111(LC 12), 37=-112(LC 12), 38=-110(LC 12), 39=-120(LC 12), 30=-120(LC 13), 28=-110(LC 13), 27=-112(LC 13), 26=-110(LC 13), 25=-119(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 33, 34, 36, 37, 38, 39, 40, 31, 30, 28, 27, 26, 25, 24, 22 except 32=277(LC 13)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-317/249, 3-4=-310/262, 4-5=-264/241, 10-11=-222/283, 11-12=-276/331, 12-13=-276/331, 13-14=-222/267
BOT CHORD 2-40=-186/276, 39-40=-186/276, 38-39=-186/276, 37-38=-186/276, 36-37=-186/276, 35-36=-186/276, 34-35=-186/276, 33-34=-186/276, 32-33=-186/276, 31-32=-186/276, 30-31=-186/276, 29-30=-186/276, 28-29=-186/276, 27-28=-186/276, 26-27=-186/276, 25-26=-186/276, 24-25=-186/276, 22-24=-186/276
WEBS 12-32=-273/185

NOTES (12-14)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 32, 40, 31, 24, 22 except (jt=lb) 33=104, 34=117, 36=111, 37=112, 38=110, 39=120, 30=120, 28=110, 27=112, 26=110, 25=119.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
13) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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JULIUS S.K. LEE LICENSE No.34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

January 24,2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	
463946	T05G	GABLE	1	1	Job Reference (optional)	I6332611

Builders FirstSource, Lake City, FL 32055

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ID:LDuQlKe7UdHpVgx54zfFR2zsAGo-AO_A?swgQ6LdEO9uHX0KlEbari?dGBxHinzpgezs52G

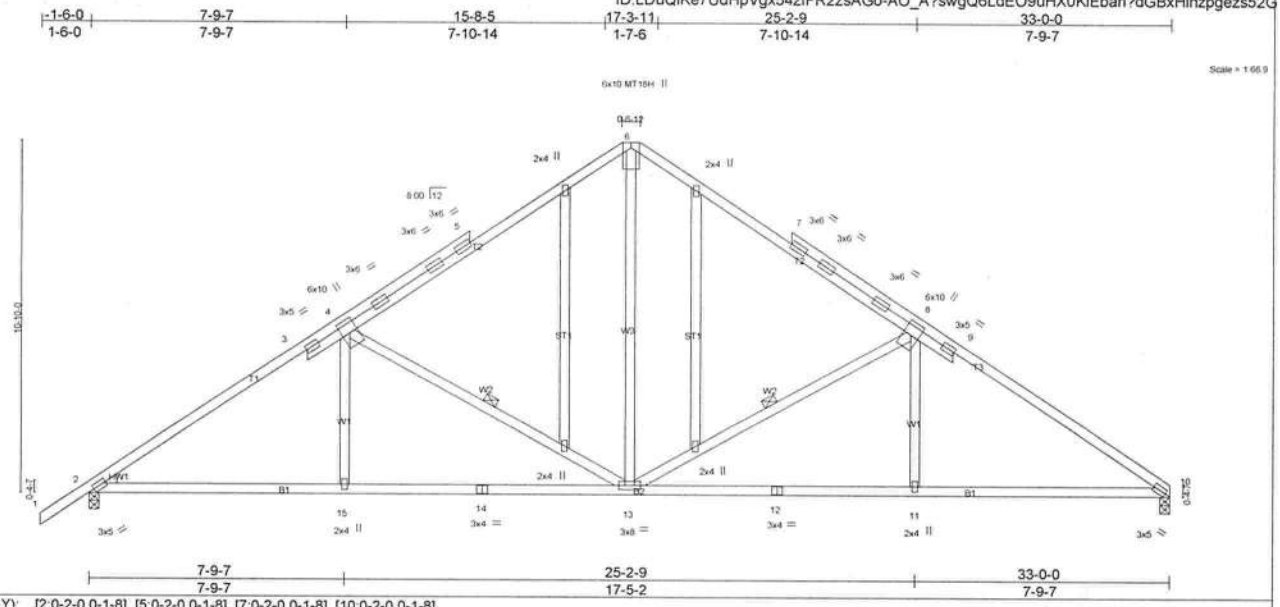

Plate Offsets (X,Y): [2:0-2-0,0-1-8], [5:0-2-0,0-1-8], [7:0-2-0,0-1-8], [10:0-2-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.12	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.26	13-15	>999	180	MT18H	244/190
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.08	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 212 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SYP No.1 *Except* T1,T3: 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3
WEDGE
Left: 2x4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 7-4-3 oc bracing.
WEBS 1 Row at midpt 4-13, 8-13

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=957/0-3-8 (min. 0-1-12), 10=891/0-3-8 (min. 0-1-10)
Max Horz 2=349(LC 11)
Max Uplift 2=-495(LC 12), 10=-445(LC 13)
Max Grav 2=1137(LC 2), 10=1056(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2055/786, 3-4=-1910/803, 4-5=-1473/630, 5-6=-1366/652, 6-7=-1366/653, 7-8=-1473/631, 8-9=-1911/805, 9-10=-2055/788
BOT CHORD 2-15=-717/1602, 14-15=-641/1602, 13-14=-641/1602, 12-13=-550/1602, 11-12=-550/1602, 10-11=-738/1602
WEBS 4-13=-787/492, 8-13=-788/496, 6-13=-395/888

NOTES (11-13)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are MT20 plates unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=495, 10=445.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

January 24,2013

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	
463946	T03	Half Hip Truss	1	1		I6332609
					Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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ID:LDuQlKe7UdHpVgx54zfFR2zsAGo-mpJ1Nrtn7Bz3NxRJcPSd7bzBxUzY3vYr0pl94Jzs52J

-1-6-0	5-9-9	9-0-0	14-6-13	21-0-0	27-5-2	33-0-0
1-6-0	5-9-9	3-2-7	5-6-13	6-5-2	6-5-2	5-6-14

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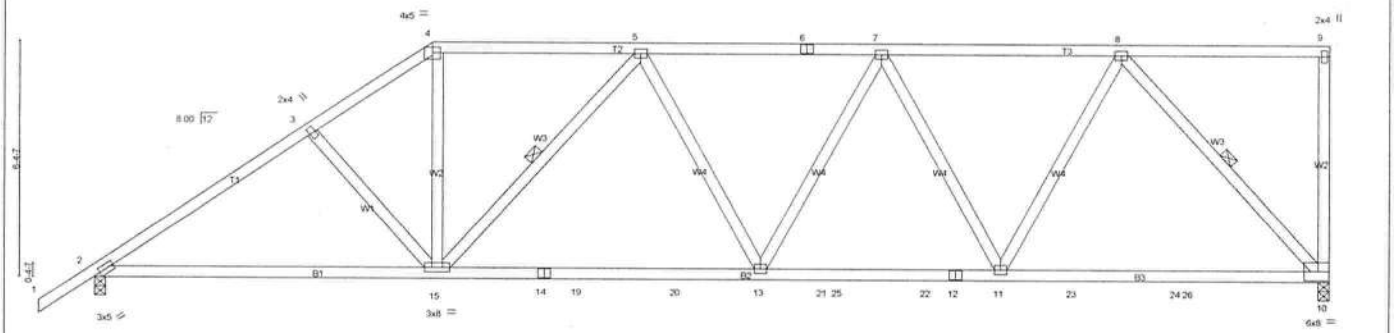

9-0-0	17-9-6	24-2-9	33-0-0
9-0-0	8-9-6	6-5-3	8-9-7

Plate Offsets (X,Y): [2:0-2-0,0-1-8], [4:0-2-8,0-1-13]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.19	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.34	10-11	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.59	Horz(TL)	0.08	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 191 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-4-9 oc bracing.
WEBS 1 Row at midpt 5-15, 8-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=1041/0-3-8 (min. 0-1-8), 2=1044/0-3-8 (min. 0-1-10)
Max Horz 2=231(LC 12)
Max Uplift 10=-319(LC 9), 2=-242(LC 9)
Max Grav 10=1044(LC 2), 2=1139(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1889/798, 3-4=-1715/784, 4-5=-1425/694, 5-6=-1711/805, 6-7=-1711/805, 7-8=-1341/615
BOT CHORD 2-15=-832/1813, 14-15=-852/1721, 14-19=-852/1721, 19-20=-852/1721, 13-20=-852/1721, 13-21=-781/1620, 21-25=-781/1620, 22-25=-781/1620, 12-22=-781/1620, 11-12=-781/1620, 11-23=-476/965, 23-24=-476/965, 24-26=-476/965, 10-26=-476/965
WEBS 3-15=-326/219, 4-15=-286/702, 5-15=-475/238, 7-11=-596/355, 8-11=-297/805, 8-10=-1426/711

NOTES (10-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x4 MT20 unless otherwise indicated.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=319, 2=242.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

January 24,2013

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	
463946	T02	Half Hip Truss	1	1		I6332608
					Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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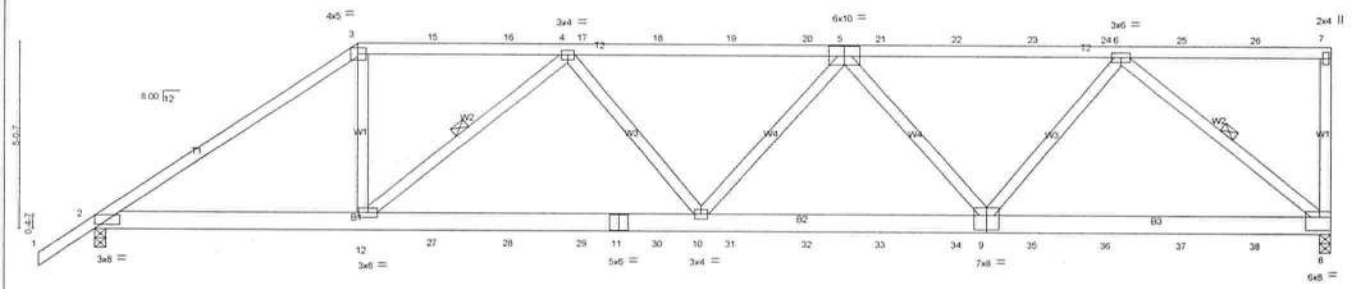
ID:LDuQlKe7UdHpVgx54zfFR2zsAGo-IdlfAVs9MtrCIns72hxObORxI4ewKN1io9?cXtzs52K

-1-6-0	7-0-0	12-7-6	20-0-1	27-4-11	33-0-0
1-6-0	7-0-0	5-7-6	7-4-11	7-4-10	5-7-6

Scale = 1:58.4



7-0-0	16-2-4	23-9-12	33-0-0
7-0-0	9-2-4	7-7-8	9-2-4

Plate Offsets (X,Y): [2:0-4-1,0-1-8], [3:0-2-12,0-2-0], [5:0-5-0,0-3-4], [8:Edge,0-4-4], [9:0-4-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	0.20	10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.36	10-12	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.93	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 198 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SYP No.1
BOT CHORD 2x6 SYP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-5 oc bracing.
WEBS 1 Row at midpt 4-12, 6-8

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 8=1726/0-3-8 (min. 0-2-7), 2=1641/0-3-8 (min. 0-2-5)
Max Horz 2=186(LC 8)
Max Uplift 8=-934(LC 5), 2=-773(LC 8)
Max Grav 8=2044(LC 2), 2=1942(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3102/1306, 3-15=-2533/1137, 15-16=-2533/1137, 4-16=-2533/1137, 4-17=-3489/1545, 17-18=-3489/1545, 18-19=-3489/1545, 19-20=-3489/1545, 5-20=-3489/1545, 5-21=-2763/1206, 21-22=-2763/1206, 22-23=-2763/1206, 23-24=-2763/1206, 6-24=-2763/1206, 7-8=-261/185
BOT CHORD 2-12=-1122/2494, 12-27=-1589/3418, 27-28=-1589/3418, 28-29=-1589/3418, 11-29=-1589/3418, 11-30=-1589/3418, 10-30=-1589/3418, 10-31=-1577/3430, 31-32=-1577/3430, 32-33=-1577/3430, 33-34=-1577/3430, 9-34=-1577/3430, 9-35=-940/1990, 35-36=-940/1990, 36-37=-940/1990, 37-38=-940/1990, 8-38=-940/1990
WEBS 3-12=-486/1244, 4-12=-1232/661, 4-10=0/287, 5-10=0/281, 5-9=-1054/587, 6-9=-437/1270, 6-8=-2586/1226

NOTES (11-13)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=934, 2=773.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 86 lb down and 111 lb up at 7-0-0, 86 lb down and 111 lb up at 9-0-12, 86 lb down and 111 lb up at 11-0-12, 86 lb down and 111 lb up at 13-0-12, 86 lb down and 111 lb up at 15-0-12, 86 lb down and 111 lb up at 17-0-12, 86 lb down and 111 lb up at 19-0-12, 86 lb down and 111 lb up at 21-0-12, 86 lb down and 111 lb up at 23-0-12, 86 lb down and 111 lb up at 25-0-12, 86 lb down and 111 lb up at 27-0-12, 86 lb down and 111 lb up at 29-0-12, and 86 lb down and 111 lb up at 31-0-12, and 86 lb down and 111 lb up at 32-10-4 on top chord, and 329 lb down and 155 lb up at 7-0-0, 50 lb down at 9-0-12, 50 lb down at 11-0-12, 50 lb down at 13-0-12, 50 lb down at 15-0-12, 50 lb down at 17-0-12, 50 lb down at 19-0-12, 50 lb down at 21-0-12, 50 lb down at 23-0-12, 50 lb down at 25-0-12, 50 lb down at 27-0-12, 50 lb down at 29-0-12, and 50 lb down at 31-0-12, and 50 lb down at 32-10-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

JULIUS S.K. LEE LICENSE No.34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

January 24,2013

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	
463946	T01	Common Truss	10	1		I6332606
					Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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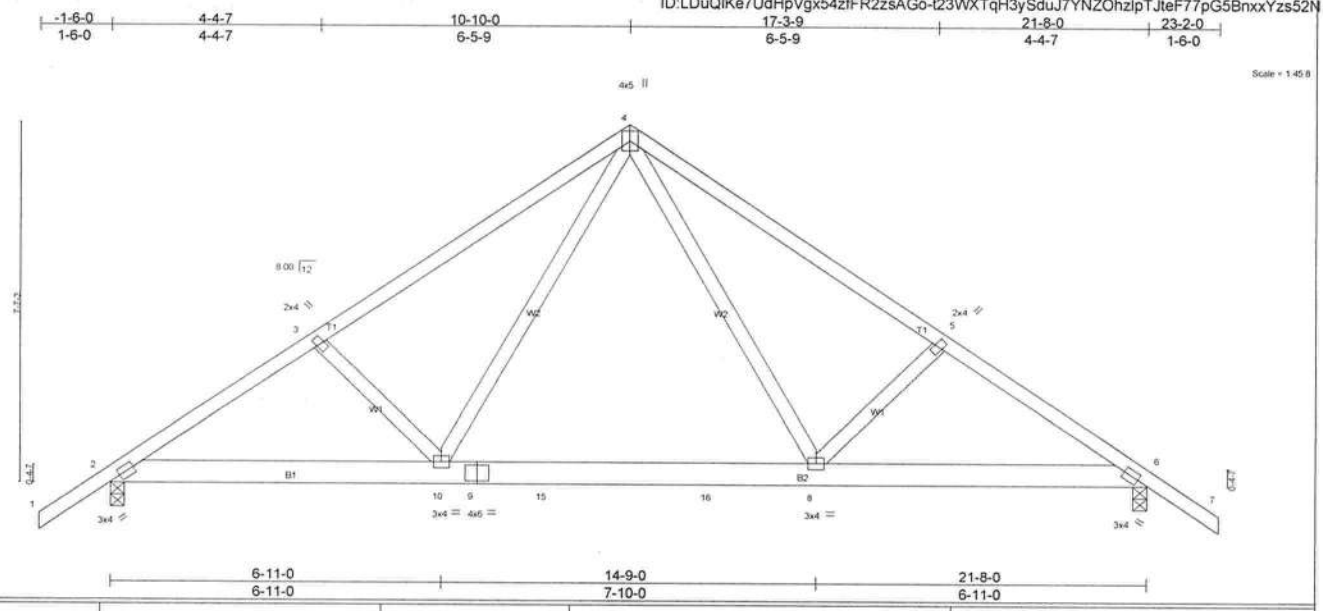

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	0.11	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.20	8-10	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.51	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 129 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SYP No.1
BOT CHORD 2x6 SYP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-7-12 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=901/0-3-8 (min. 0-1-10), 6=901/0-3-8 (min. 0-1-10)
Max Horz 2=-233(LC 10)
Max Uplift 2=-466(LC 12), 6=-466(LC 13)
Max Grav 2=1009(LC 2), 6=1009(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2117/779, 3-4=-1957/761, 4-5=-1958/761, 5-6=-2118/779
BOT CHORD 2-10=-655/1717, 9-10=-256/993, 9-15=-256/993, 15-16=-256/993, 8-16=-256/993, 6-8=-547/1731
WEBS 4-8=-372/929, 5-8=-429/321, 4-10=-373/928, 3-10=-429/321

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 466 lb uplift at joint 2 and 466 lb uplift at joint 6.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-44, 4-7=-44, 2-10=-10, 10-15=-61(F=-51), 15-16=-91(F=-51), 8-16=-61(F=-51), 6-8=-10

January 24,2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	I6332604
463946	PB01G	GABLE	2	1	Job Reference (optional)	

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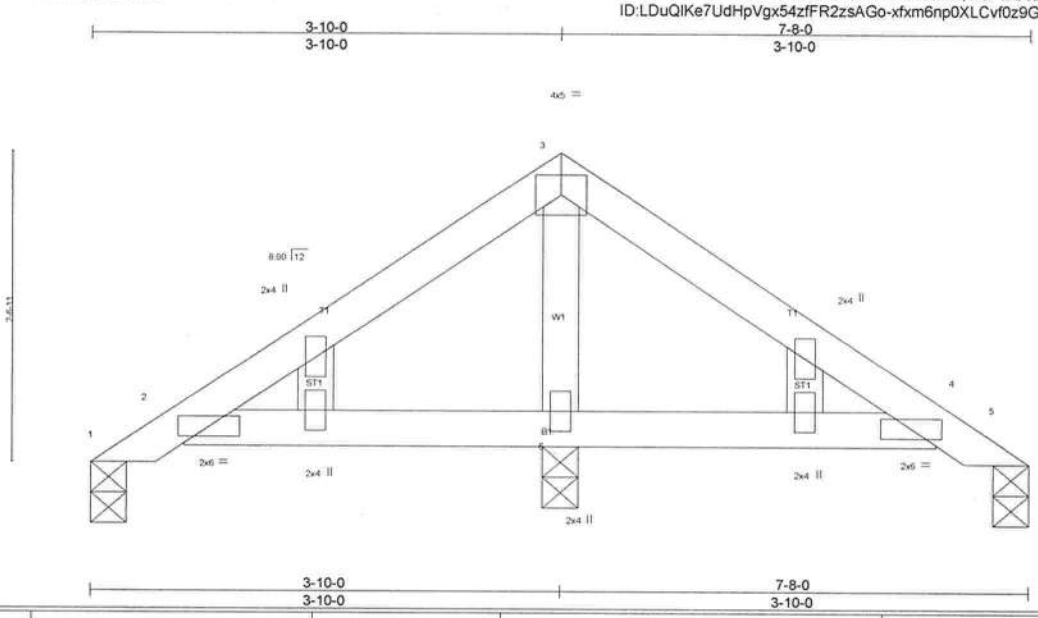

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	-0.01	16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.01	6-13	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 27 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=56/0-3-8 (min. 0-2-15), 6=297/0-3-8 (min. 0-1-8), 5=52/0-3-8 (min. 0-2-14)
Max Horz 1=77(LC 9)
Max Uplift 1=-28(LC 12), 6=-145(LC 12), 5=-51(LC 13)
Max Grav 1=90(LC 27), 6=351(LC 2), 5=86(LC 28)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
BOT CHORD 2-6=-234/341, 4-6=-218/331
WEBS 3-6=-354/169

NOTES (12-14)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Gable studs spaced at 2-0-0 oc.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 1, 145 lb uplift at joint 6 and 51 lb uplift at joint 5.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
13) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

January 24,2013

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	
463946	HJ02	Diagonal Hip Girder	2	1	Job Reference (optional)	I6332602

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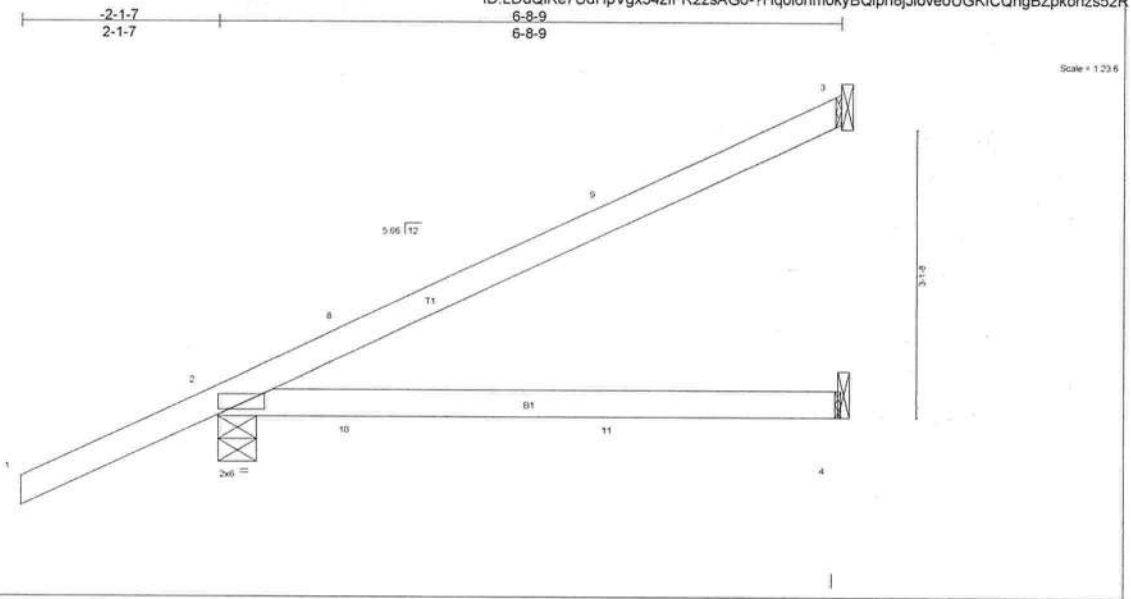

Plate Offsets (X,Y): [2:0-2-8,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.05	4-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.08	4-7	>948	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 25 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=108/Mechanical, 2=277/0-4-15 (min. 0-1-8), 4=32/Mechanical
Max Horz 2=194(LC 8)
Max Uplift 3=-142(LC 8), 2=-147(LC 8)
Max Grav 3=132(LC 2), 2=340(LC 2), 4=76(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-8=-367/252
BOT CHORD 2-10=-465/550

NOTES (9-11)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 142 lb uplift at joint 3 and 147 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 32 lb down and 47 lb up at 1-5-12, 32 lb down and 47 lb up at 1-5-12, and 4 lb down and 71 lb up at 4-3-11, and 4 lb down and 71 lb up at 4-3-11 on top chord, and 24 lb up at 1-5-12, 24 lb up at 1-5-12, and 4 lb down and 3 lb up at 4-3-11, and 4 lb down and 3 lb up at 4-3-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-44, 4-5=-10
Concentrated Loads (lb)
Vert: 8=25(F=12, B=12) 9=-4(F=-2, B=-2) 10=22(F=11, B=11) 11=-7(F=-4, B=-4)

January 24,2013

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	
463946	HJ01	Diagonal Hip Girder	1	1	Job Reference (optional)	I6332601

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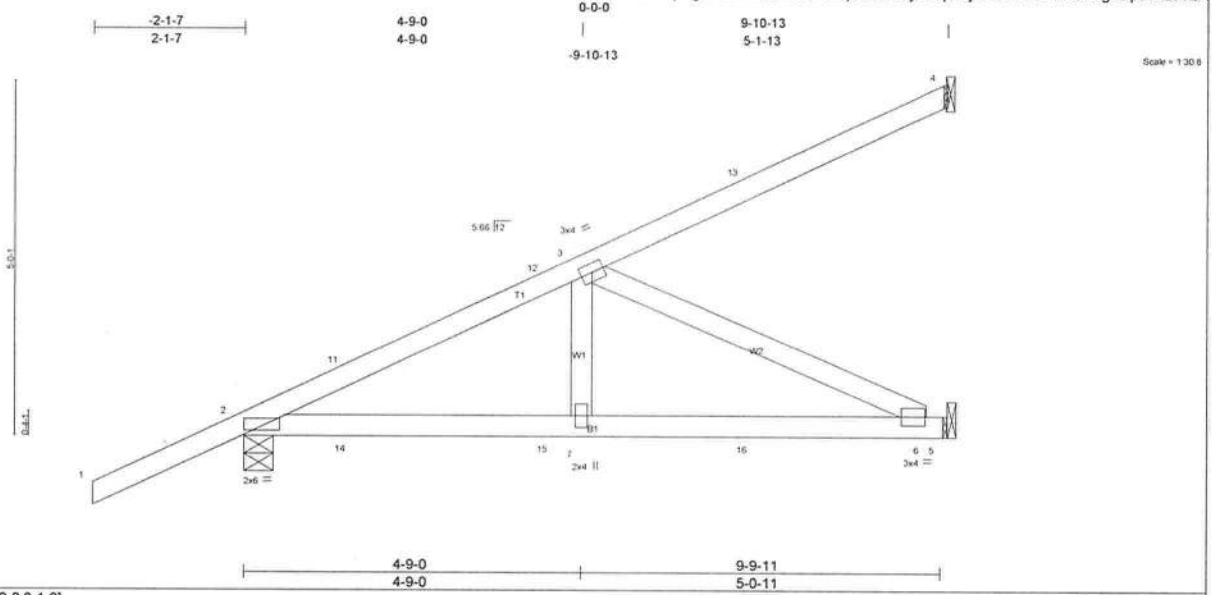

Plate Offsets (X,Y): [2:0-2-8,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.05	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.10	6-7	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.34	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 45 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-2-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=234/Mechanical, 2=360/0-4-15 (min. 0-1-8), 5=242/Mechanical
Max Horz 2=267(LC 8)
Max Uplift 4=-261(LC 8), 2=-195(LC 8), 5=-152(LC 8)
Max Grav 4=286(LC 2), 2=438(LC 2), 5=308(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-11=-557/252, 11-12=-482/272, 3-12=-480/246
BOT CHORD 2-14=-410/480, 14-15=-410/480, 7-15=-410/480, 7-16=-410/480, 6-16=-410/480
WEBS 3-6=-530/453

NOTES (9-11)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 4, 195 lb uplift at joint 2 and 152 lb uplift at joint 5.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 32 lb down and 47 lb up at 1-5-12, 32 lb down and 47 lb up at 1-5-12, 4 lb down and 71 lb up at 4-3-11, 4 lb down and 71 lb up at 4-3-11, 45 lb down and 119 lb up at 7-1-10, and 45 lb down and 119 lb up at 7-1-10, and 140 lb down and 104 lb up at 9-10-1 on top chord, and 24 lb up at 1-5-12, 24 lb up at 1-5-12, 4 lb down and 3 lb up at 4-3-11, 4 lb down and 3 lb up at 4-3-11, 27 lb down at 7-1-10, and 27 lb down at 7-1-10, and 80 lb down at 9-10-1 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-44, 5-8=-10

Continued on page 2

January 24,2013

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1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	
463946	EJ01	Jack-Partial Truss	14	1	Job Reference (optional)	I6332599

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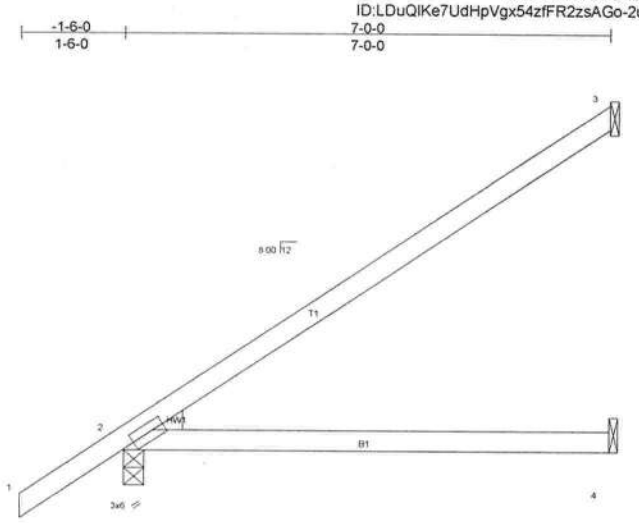

Plate Offsets (X,Y): [2:0-2-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.06	4-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.12	4-7	>718	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.01	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 27 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEDGE
Left: 2x4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=115/Mechanical, 2=294/0-3-8 (min. 0-1-8), 4=32/Mechanical
Max Horz 2=183(LC 12)
Max Uplift 3=-104(LC 12), 2=-68(LC 12)
Max Grav 3=158(LC 21), 2=351(LC 2), 4=80(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1131/494
BOT CHORD 2-4=-1306/2274

NOTES (7-9)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at joint 3 and 68 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

January 24,2013

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Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SIMQUE - LOT 125 The Preserve	
463946	CJ02	Jack-Open Truss	6	1	Job Reference (optional)	I6332597

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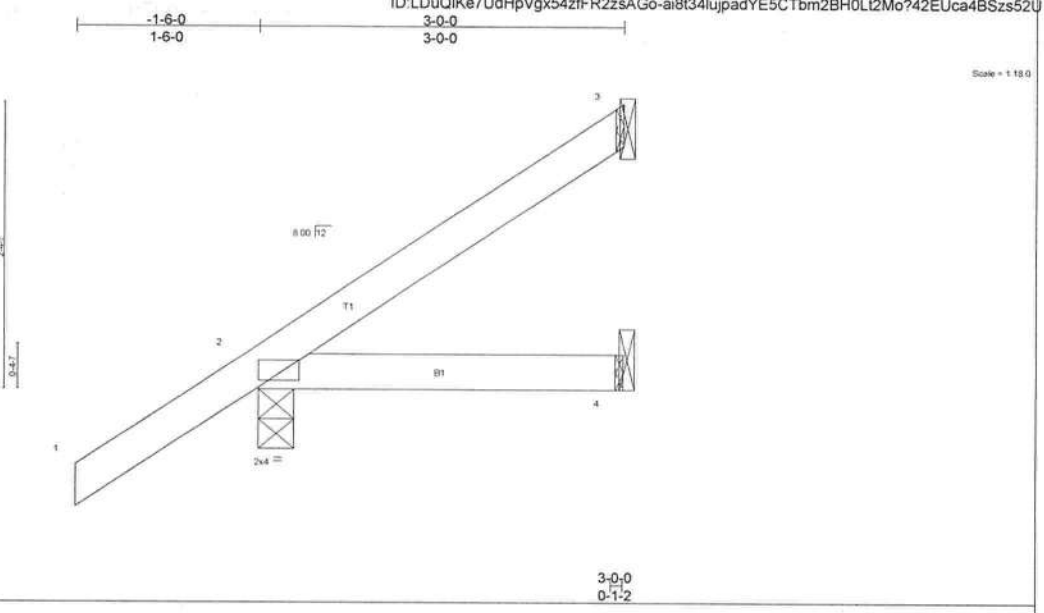


Plate Offsets (X,Y): [2:0-1-9,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	4-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	4-7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 13 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

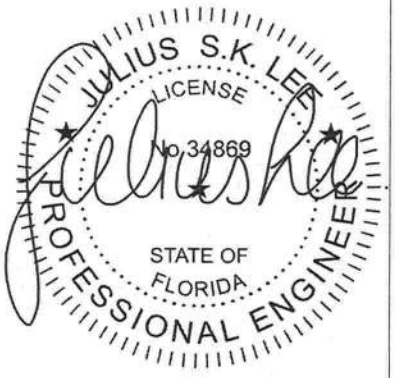
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=46/Mechanical, 2=165/0-3-8 (min. 0-1-8), 4=14/Mechanical
Max Horz 2=137(LC 12)
Max Uplift 3=-64(LC 12), 2=-88(LC 12)
Max Grav 3=68(LC 21), 2=199(LC 2), 4=34(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
BOT CHORD 2-4=-87/436

NOTES (7-9)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 3 and 88 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 24,2013

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