

REQUIRED ROOF VENTILATION:

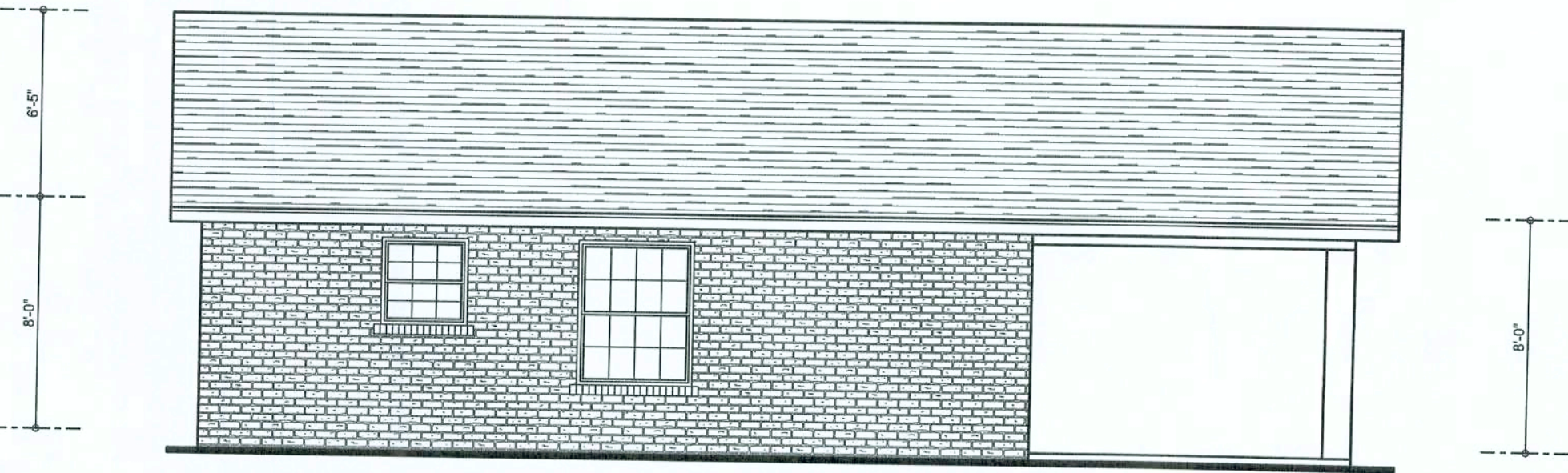
AS PER FLORIDA BUILDING CODE 2308.7

RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3" ABOVE EAVE)
800 S.F. / 300 x 50% = 1.33 S.F. RIDGE VENT AREA REQUIRED
12.12 FEET OF RIDGE VENT REQUIRED

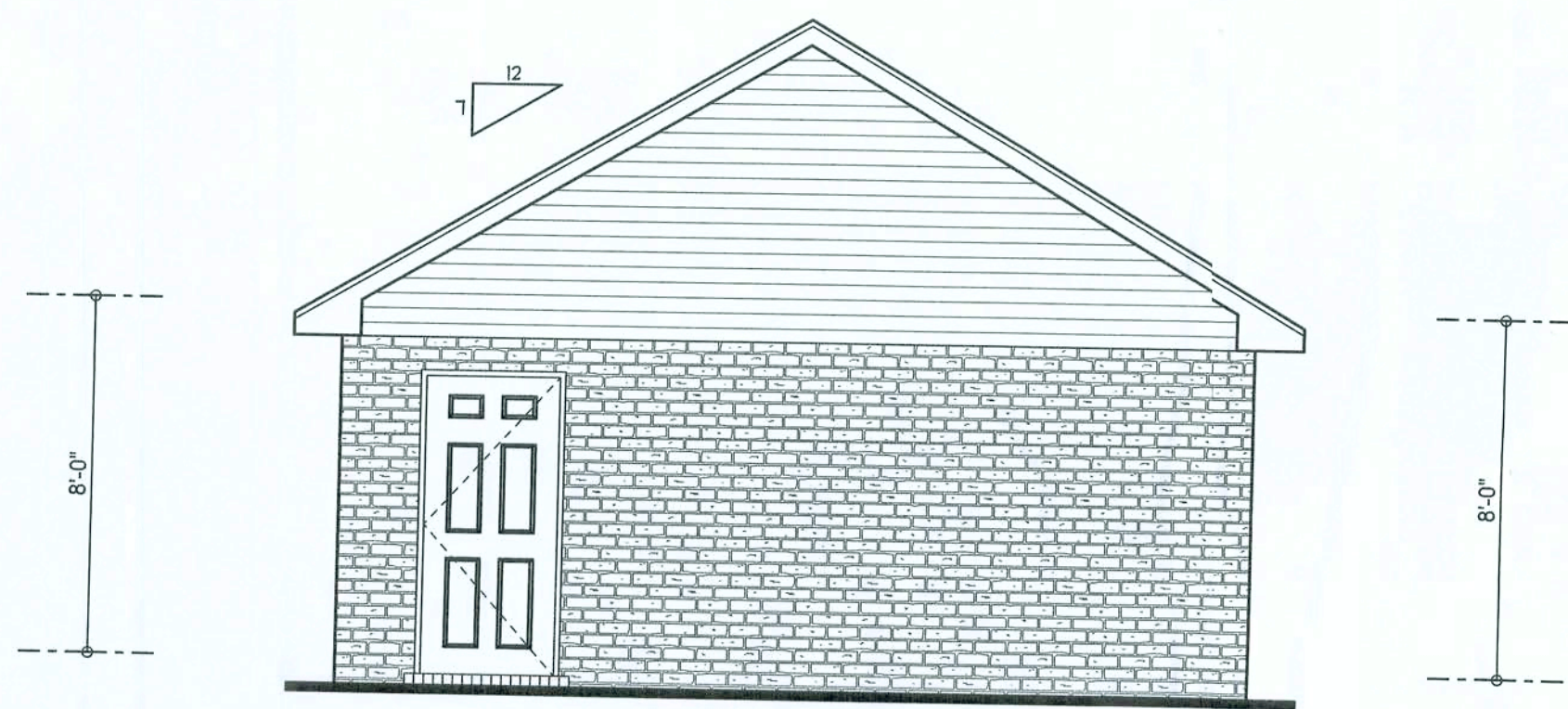
SOFFIT VENT
800 S.F. / 300 x 50% = 1.33 S.F. SOFFIT VENT AREA REQUIRED
44.33 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

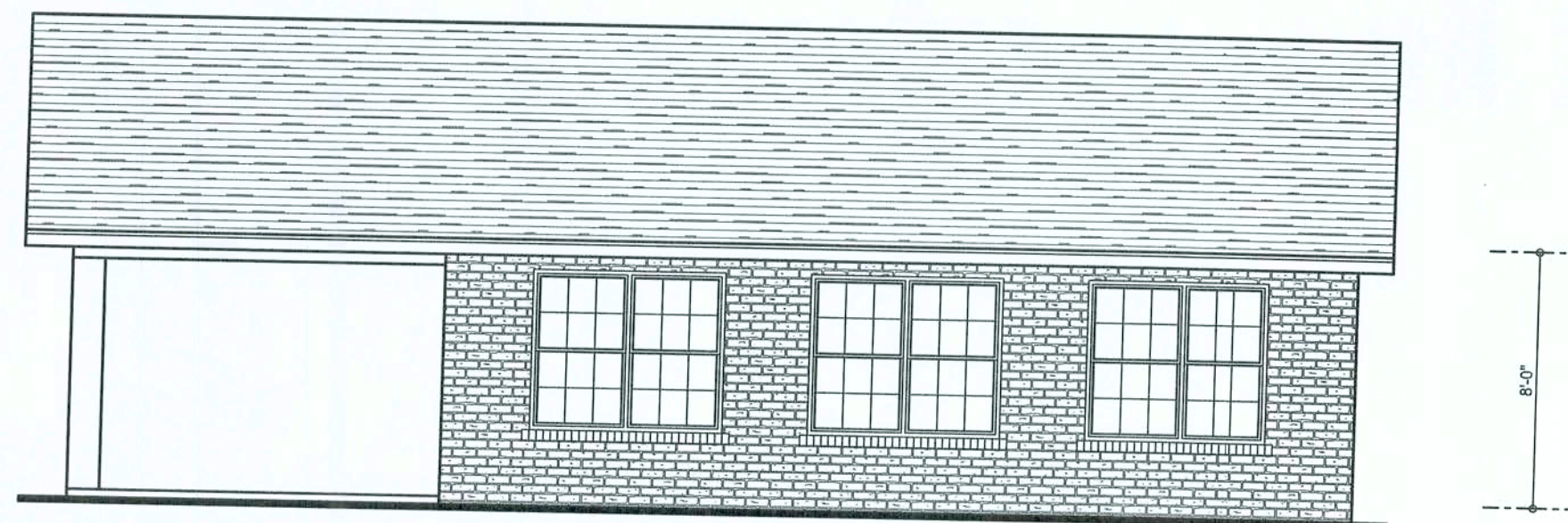
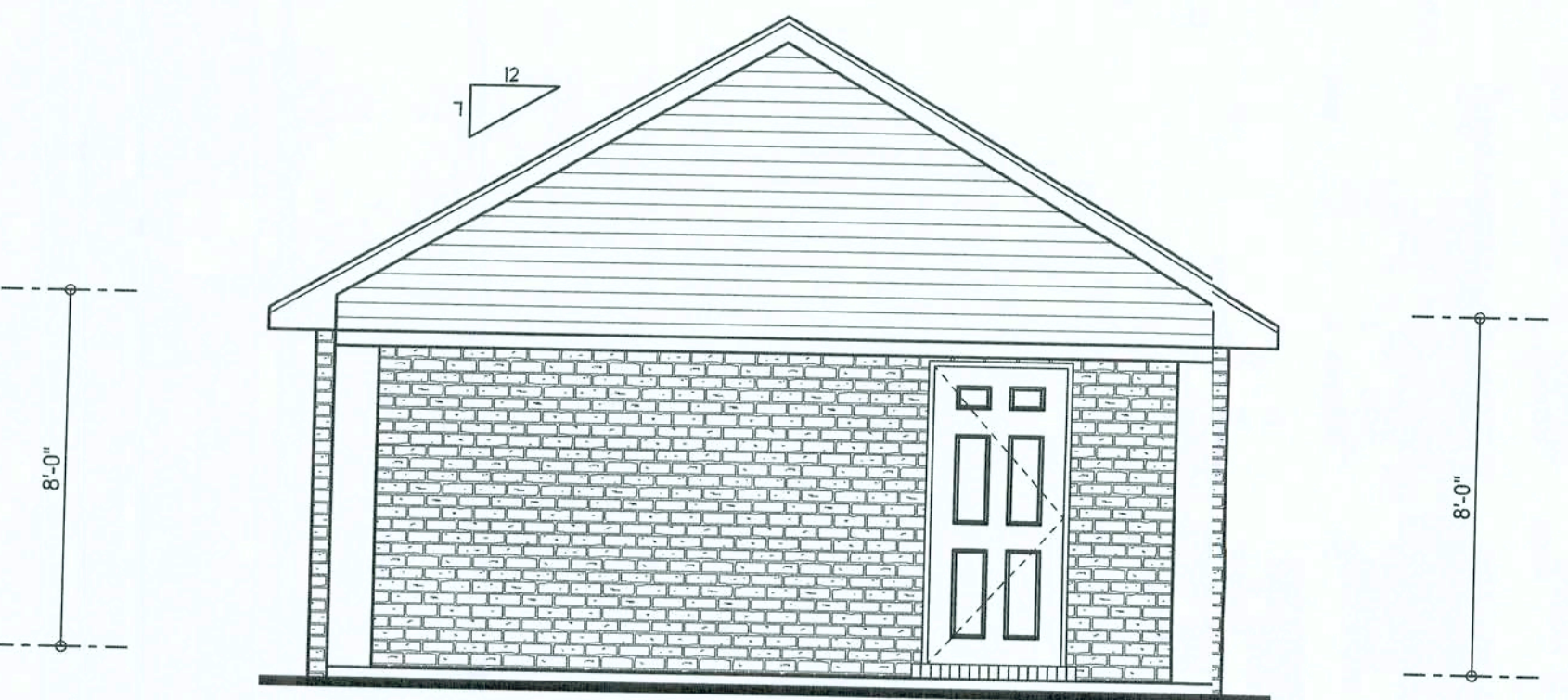
1. RIDGE VENTS = 16 IN²/FT (1.1 FT²/FT)
2. OFF-RIDGE VENTS = .70 FT² PER 4" UNIT
3. SOFFIT VENTS = 4.3 IN²/FT (.33 FT²/FT)



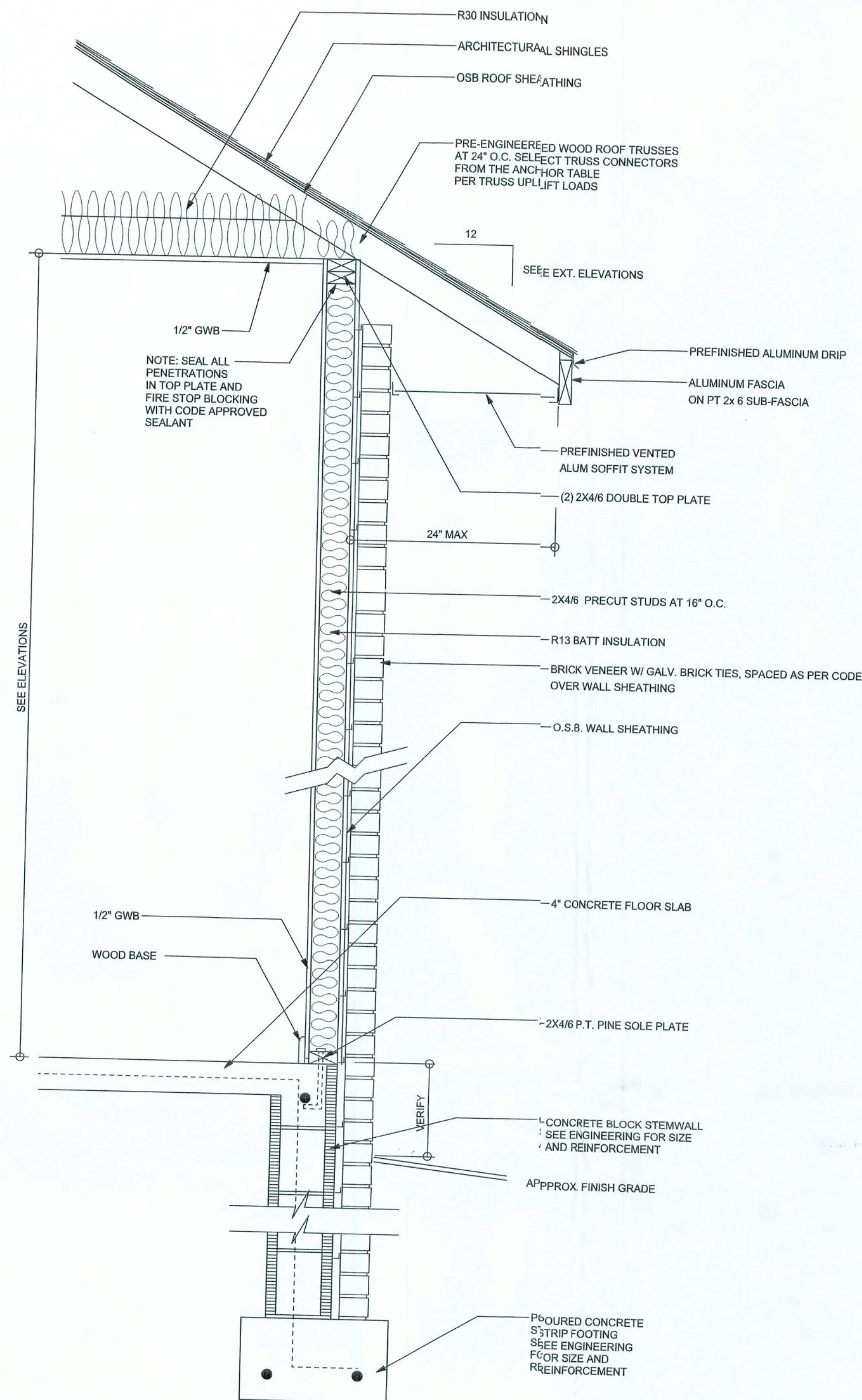
FRONT ELEVATION
SCALE: 3/16" = 1'-0"



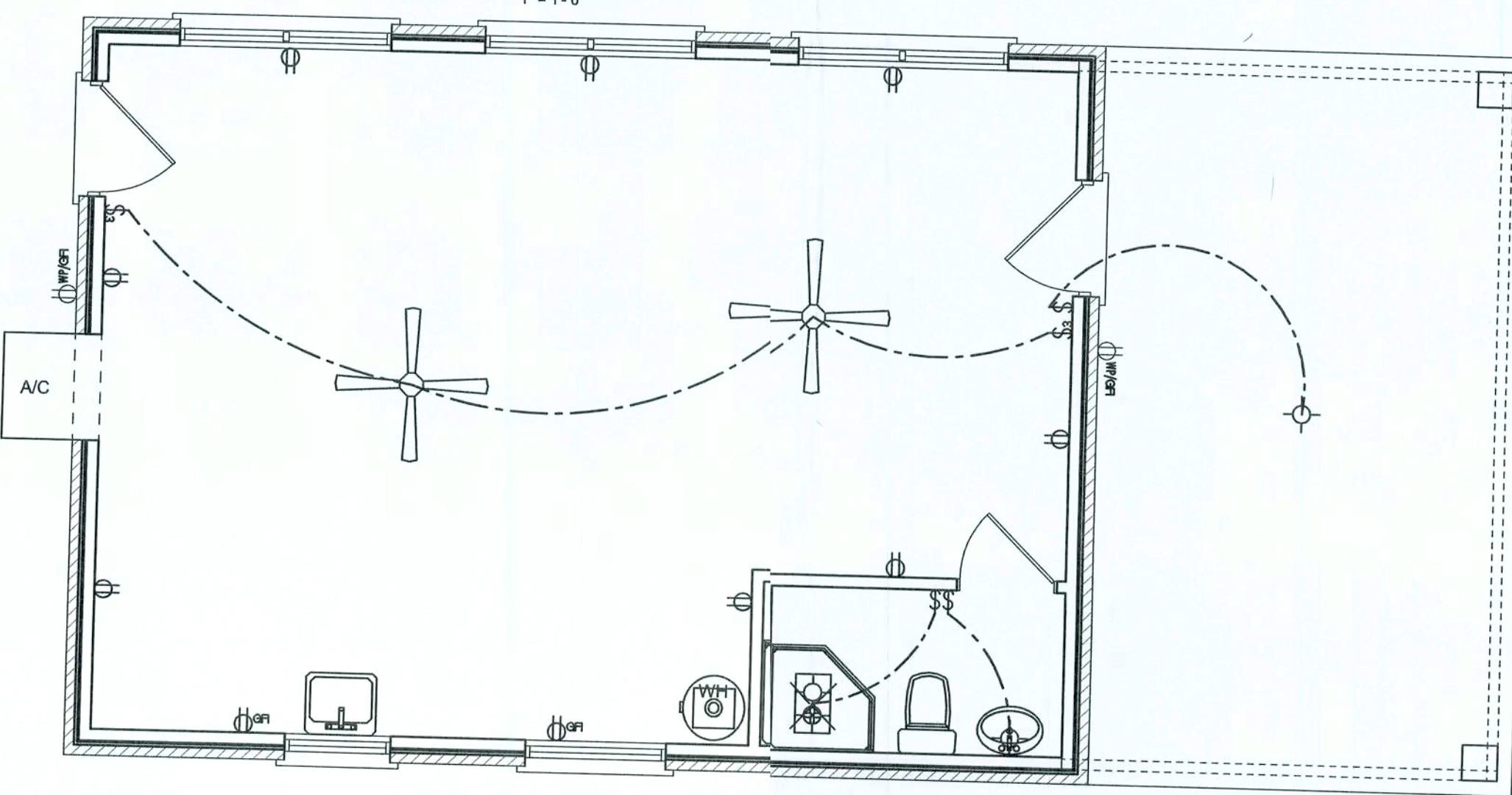
LEFT ELEVATION
SCALE: 1/4" = 1'-0"



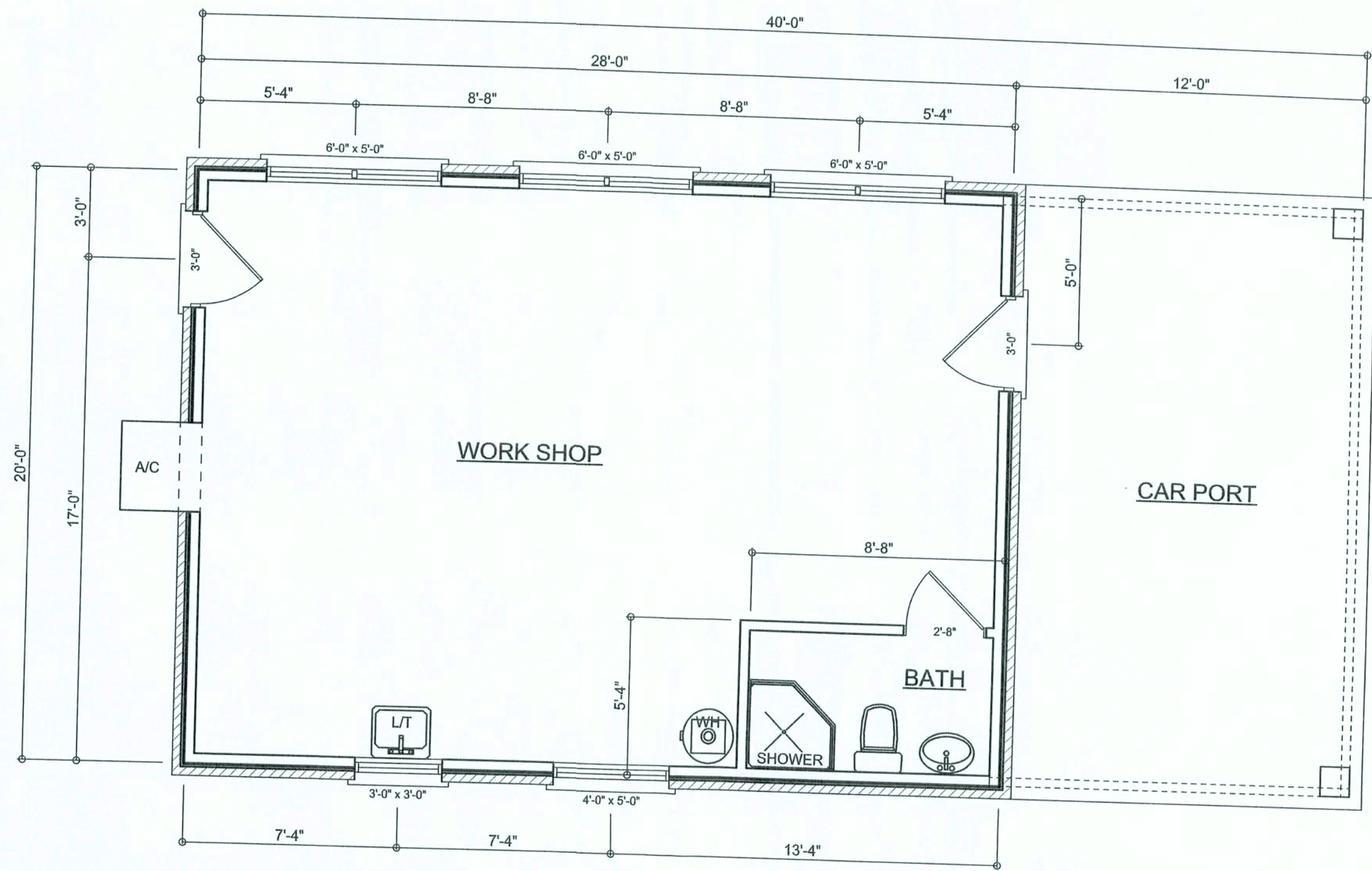
REAR ELEVATION
SCALE: 3/16" = 1'-0"



TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA
SCALE: 1" = 1'-0"



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"



FLOOR PLAN
SCALE: 1/4" = 1'-0"

AREA SUMMARY

| | | |
|----------------|-----|-------|
| WORK SHOP AREA | 560 | S. F. |
| CAR PORT AREA | 240 | S. F. |
| TOTAL AREA | 800 | S. F. |

ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NATL. ELECTRIC CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL

| ELECTRICAL LEGEND | |
|-------------------|--------------------------------------|
| | CEILING FAN (PRE-WIRE FOR LIGHT KIT) |
| | DOUBLE SECURITY LIGHT |
| | 2X4 FLUORESCENT LIGHT FIXTURE |
| | RECESSED CAN LIGHT |
| | BATH EXHAUST FAN WITH LIGHT |
| | BATH EXHAUST FAN |
| | LIGHT FIXTURE |
| | DUPLEX OUTLET |
| | 220v OUTLET |
| | GFI DUPLEX OUTLET |
| | SMOKE DETECTOR |
| | WALL SWITCH |
| | 3 WAY WALL SWITCH |
| | 4 WAY WALL SWITCH |
| | WATER PROOF GFI OUTLET |
| | PHONE JACK |
| | TELEVISION JACK |
| | GARAGE DOOR OPENER |
| | WALL HEATER |

REVISIONS

| | |
|--|--|
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| | |
| | |

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway,
PE No. 53915, POB 868, Lake City, FL
32066, 386-754-6419

DIMENSIONS:
Stated dimensions supersede noted
dimensions. Refer all questions to
Mark Disosway, P.E. for resolution.
Do not proceed without clarification.

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permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section R301.2.1, Florida building
code residential 2004, to the best of my
knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISOSWAY
P.E. 53915

Mark Disosway
3/3/06
SEAL

Ralph & Carolyn
Norris Garage

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PRINTED DATE:
March 03, 2006

DRAWN BY:
David Disosway

CHECKED BY:

FINAL DATE:
03 / Mar / 06

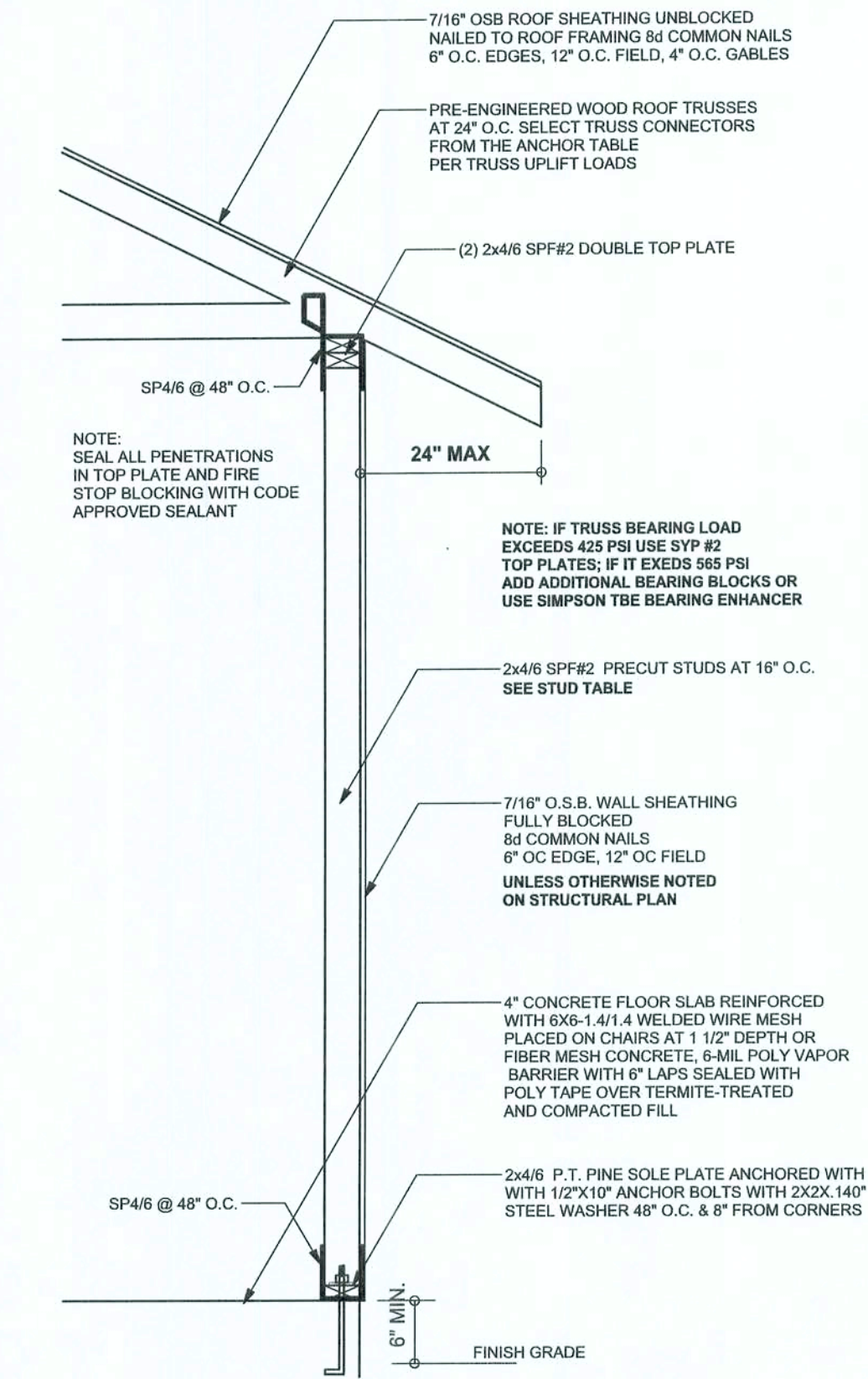
JOB NUMBER:
602154

DRAWING NUMBER

A-1

OF 3 SHEETS

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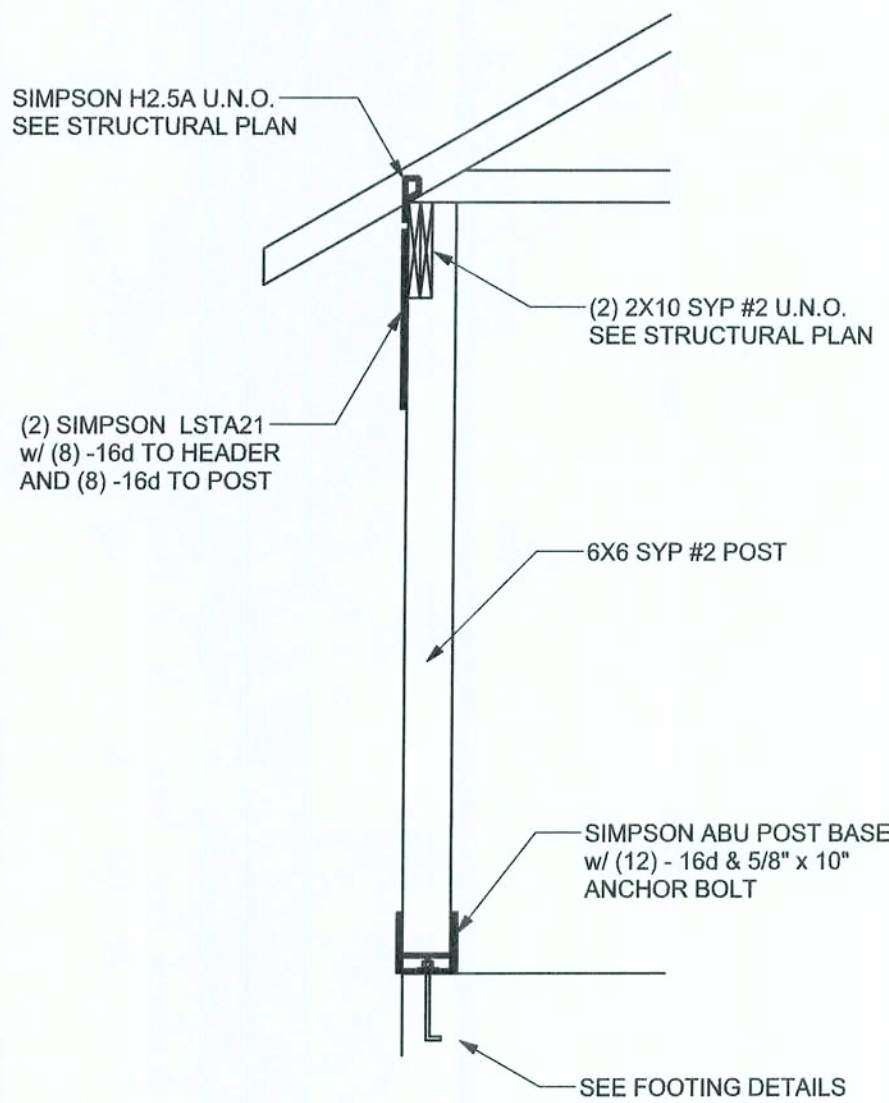
ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

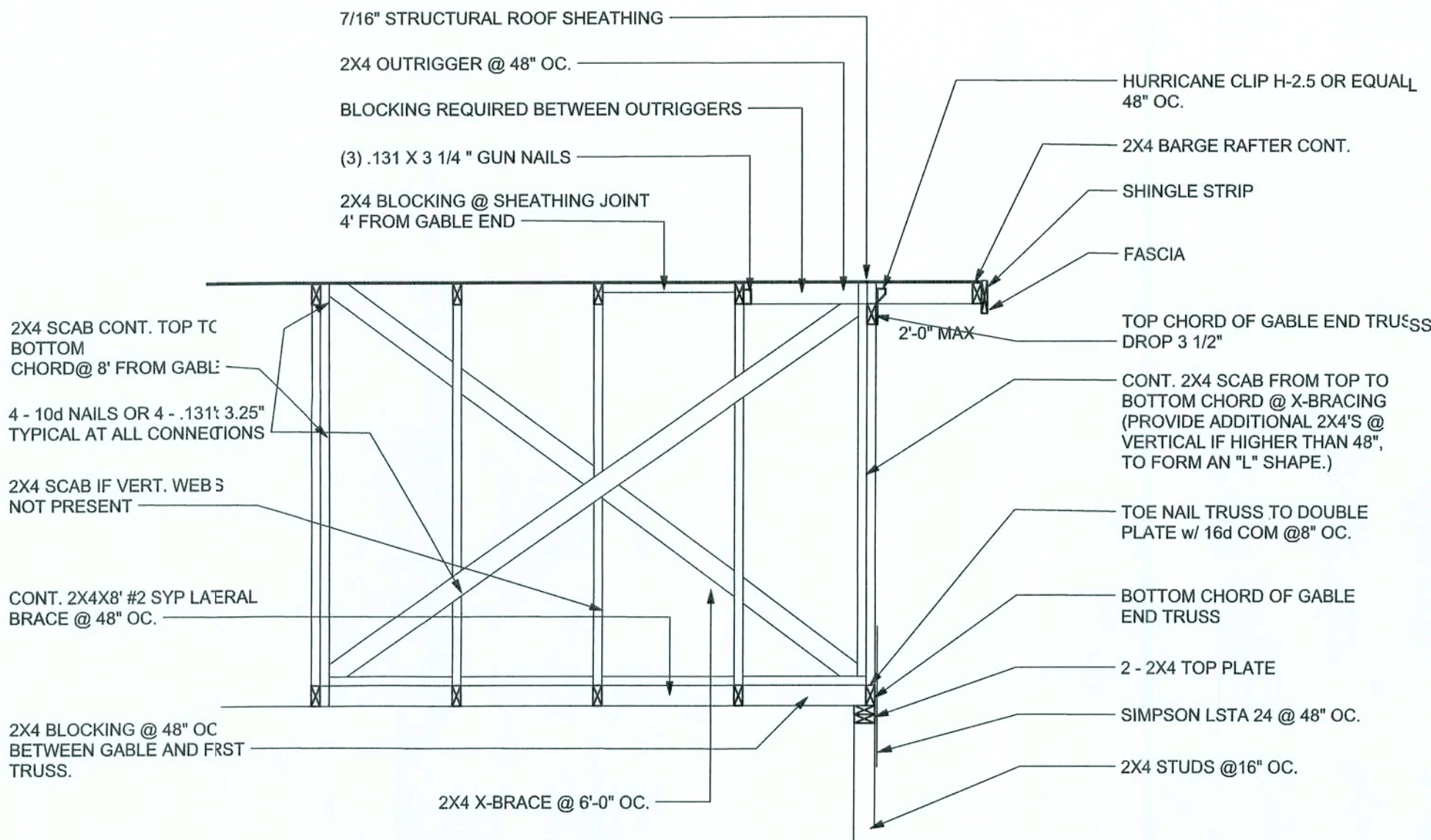
| | |
|------------------|------------------------|
| (1) 2x4 @ 16" OC | TO 11'-9" STUD HEIGHT |
| (1) 2x4 @ 12" OC | TO 13'-0" STUD HEIGHT |
| (1) 2x6 @ 16" OC | TO 18'-10" STUD HEIGHT |
| (1) 2x6 @ 12" OC | TO 20'-0" STUD HEIGHT |

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTPLIED BY USE FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13'-6" O.C.



TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

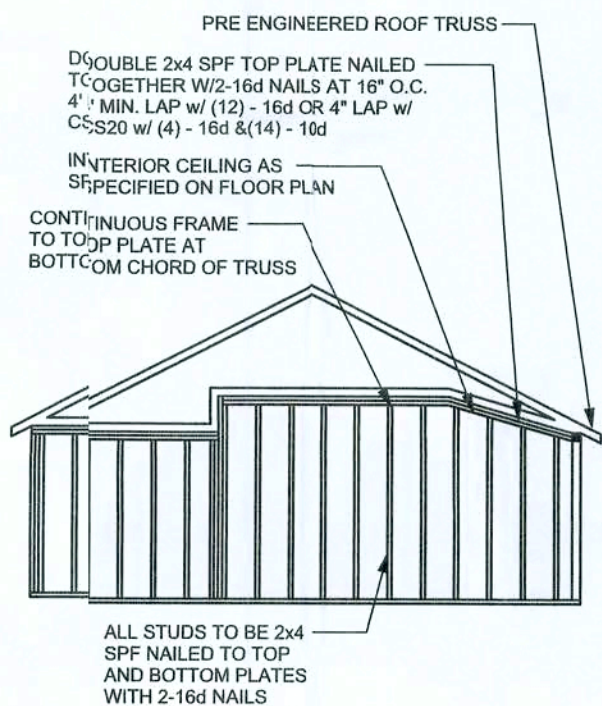


TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP

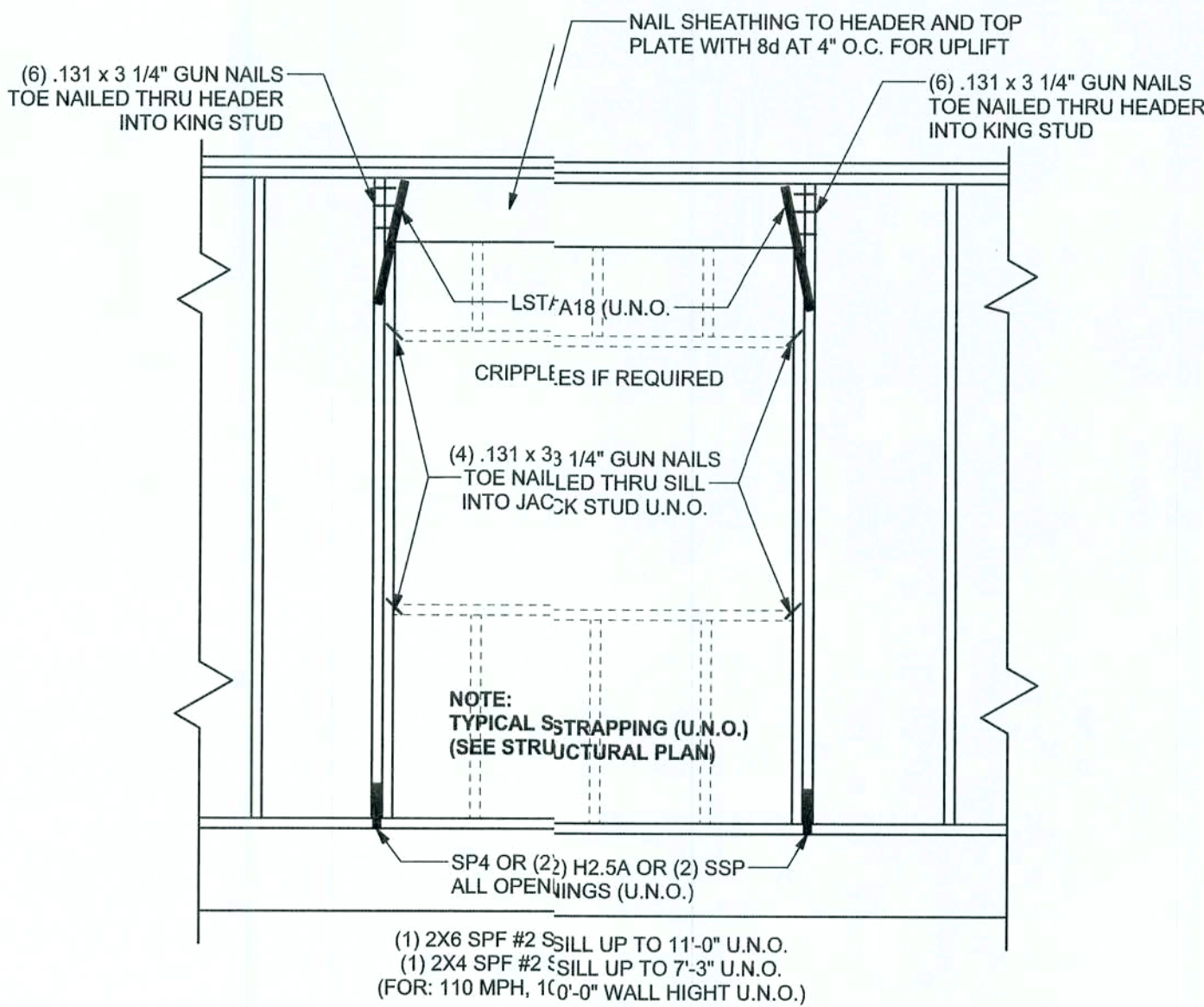
GRADE & SPECIES TABLE

| | | Fb (psi) | E (10 ⁶ psi) |
|------|--------------|----------|-------------------------|
| 2x8 | SYP #2 | 1200 | 1.6 |
| 2x10 | SYP #2 | 1050 | 1.6 |
| 2x12 | SYP #2 | 975 | 1.6 |
| GLB | 24F-V3 SP | 2400 | 1.8 |
| LSL | TIMBERSTRAND | 1700 | 1.7 |
| LVL | MICROLAM | 1600 | 1.9 |
| PSL | PARALAM | 2900 | 2.0 |



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.



TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSSES TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDERS RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI

WELDED WIRE REINFORCED SLAB: 8" x 8" W1 x 4 W1 x 4, FB = 60KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH/ WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWW OR REINFORCED CONCRETE OR REINFORCED BOND BEAM OR 12\"/>

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI. ALL LAP SPLICES 40\"/>

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2,400, E = 1,800,000. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16\"/>

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7\"/>

WASHERS: WASHERS USED WITH 1/2\"/>

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

| | Specific Requirements |
|--|---|
| 1.4A Compressive strength | 8" block bearing walls F _m = 1500 psi |
| 2.1 Mortar | ASTM C 270, Type N, UNO |
| 2.2 Grout | ASTM C 476, admixtures require approval |
| 2.3 CMU standard | ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block |
| 2.3 Clay brick standard | ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.25"x11.25" |
| 2.4 Reinforcing bars, #3 - #11 | ASTM 615, Grade 60, F _y = 60 ksi, Lap splices min 48 bar dia. (30" for #5) |
| 2.4F Coating for corrosion protection | Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS |
| 2.4F Coating for corrosion protection | Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS |
| 3.3.E.2 Pipes, conduits, and accessories | Any not shown on the project drawings require engineering approval. |
| 3.3.E.7 Movement joints | Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings. |

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

| UPLIFT LBS. SYP | UPLIFT LBS. SPF | TRUSS CONNECTOR* | TO PLATES | TO RAFTER/TRUSS | TO STUDS |
|-----------------|-----------------|------------------------|----------------|-----------------|-----------------------------------|
| < 420 | < 245 | H5A | 3-8d | 3-8d | |
| < 455 | < 265 | H5 | | 4-8d | 4-8d |
| < 360 | < 235 | H4 | 4-8d | 4-8d | |
| < 455 | < 320 | H3 | | 4-8d | 4-8d |
| < 415 | < 365 | H2.5 | 5-8d | 5-8d | |
| < 600 | < 535 | H2.5A | 5-8d | 5-8d | |
| < 950 | < 820 | H8 | 8-8d | 8-8d | |
| < 745 | < 565 | H8 | 5-10d, 1 1/2" | 5-10d, 1 1/2" | |
| < 1465 | < 1050 | H14-1 | 13-8d | 12-8d, 1 1/2" | |
| < 1465 | < 1050 | H14-2 | 15-8d | 12-8d, 1 1/2" | |
| < 590 | < 850 | H10-1 | 8-8d, 1 1/2" | 8-8d, 1 1/2" | |
| < 760 | < 655 | H10-2 | 6-10d | 6-10d | |
| < 1470 | < 1265 | H16-1 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1470 | < 1265 | H16-2 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1000 | < 860 | MTS24C | 7-10d 1 1/2" | 7-10d 1 1/2" | |
| < 1450 | < 1245 | HTS24 | 12-10d 1 1/2" | 12-10d 1 1/2" | |
| < 2900 | < 2490 | 2-HTS24 | | | |
| < 2050 | < 1765 | LG72 | 14-16d | 14-16d | |
| | | HEAVY GIRDER TIEDOWNS* | | | TO FOUNDATION |
| < 3965 | < 3330 | MGT | | 22-10d | 1-5/8" THREADED ROD 12" EMBEDMENT |
| < 10980 | < 6485 | HGT-2 | | 16-10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 10330 | < 9035 | HGT-3 | | 16-10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 9250 | < 9250 | HGT-4 | | 16-10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| | | STUD STRAP CONNECTOR* | | | TO STUDS |
| < 435 | < 435 | SSP DOUBLE TOP PLATE | 3-10d | | 4-16d |
| < 455 | < 420 | SSP SINGLE SILL PLATE | 1-10d | | 4-10d |
| < 825 | < 825 | DSP DOUBLE TOP PLATE | 6-10d | | 8-10d |
| < 825 | < 600 | DSP SINGLE SILL PLATE | 2-10d | | 8-10d |
| < 885 | < 760 | SP4 | | | 6-10d, 1 1/2" |
| < 1240 | < 1085 | SPH | | | 10-10d, 1 1/2" |
| < 885 | < 760 | SP6 | | | 6-10d, 1 1/2" |
| < 1240 | < 1085 | SPH8 | | | 10-10d, 1 1/2" |
| < 1235 | < 1165 | LSTA18 | 14-10d | | |
| < 1235 | < 1235 | LSTA21 | 16-10d | | |
| < 1030 | < 1030 | CS20 | 18-8d | | |
| < 1705 | < 1705 | CS16 | 28-8d | | |
| | | STUD ANCHORS* | | TO STUDS | TO FOUNDATION |
| < 1350 | < 1305 | LTT19 | 8-16d | 1/2" AB | |
| < 2310 | < 2310 | LTT31 | 18-10d, 1 1/2" | 1/2" AB | |
| < 2775 | < 2570 | HD2A | 2-5/8" BOLTS | 5/8" AB | |
| < 4175 | < 3695 | HTT16 | 18-16d | 5/8" AB | |
| < 1400 | < 1400 | PAHD42 | 16-16d | | |
| < 3335 | < 3335 | HPAH22 | 16-16d | | |
| < 2200 | < 2200 | ABU44 | 12-16d | 1/2" AB | |
| < 2300 | < 2300 | ABU66 | 12-16d | 1/2" AB | |
| < 2320 | < 2320 | ABU88 | 18-16d | 2-5/8" AB | |

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B

3.) WIND IMPORTANCE FACTOR = 1.0

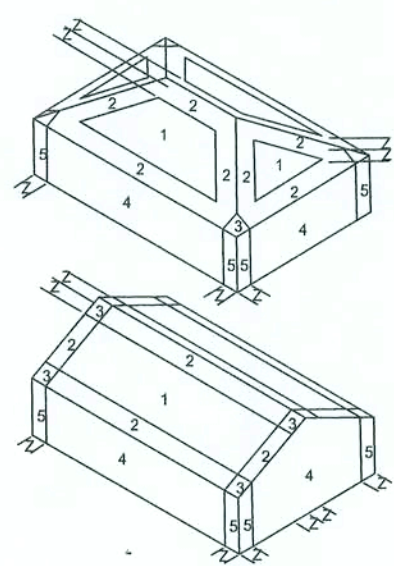
4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



| Zone | Effective Wind Area (ft ²) | 10 | 100 |
|--|--|-------|-------|
| 1 | 19.9 - 21.8 | 18.1 | -18.1 |
| 2 | 19.9 - 25.5 | 18.1 | -21.8 |
| 2.0H | | -40.6 | -40.6 |
| 3 | 19.9 - 25.5 | 18.1 | -21.8 |
| 3.0H | | 68.3 | -42.4 |
| 4 | 21.8 - 23.6 | 18.5 | -20.4 |
| 5 | 21.8 - 29.1 | 18.5 | -22.6 |
| Doors & Windows Worst Case (Zone 5, 10 ft ²) | | 21.8 | -29.1 |
| 8x7 Garage Door | | 19.5 | -22.9 |
| 16x7 Garage Door | | 18.5 | -21.0 |

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (SLEEPING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

ROOF 20 PSF (FLAT OR <4:12)

16 PSF (4:12 TO <12:12)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

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SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway,
PE No. 53915 POB 868, Lake City, FL
32056, 386-74-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915

3/16

SEAL

Raph & Carolyn
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PRINTED DATE:
March 03, 2006

DRAWN BY: David Disoway CHECKED BY:

FINALS DATE:
03 / Mar 06

JOB NUMBER:
602154

DRAWING NUMBER

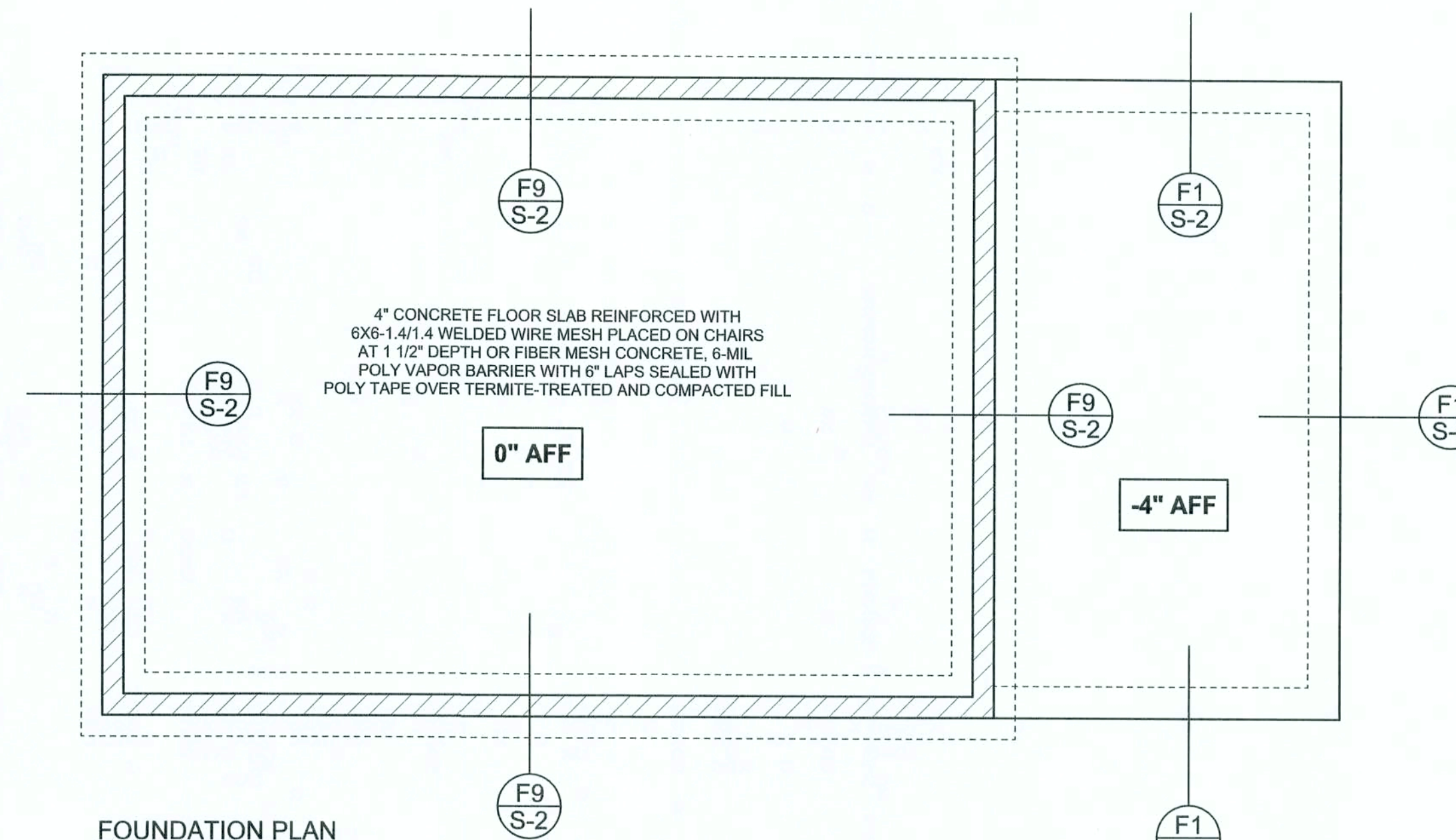
S-1

OF 3 SHEETS

REVISIONS

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| | |
| | |

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

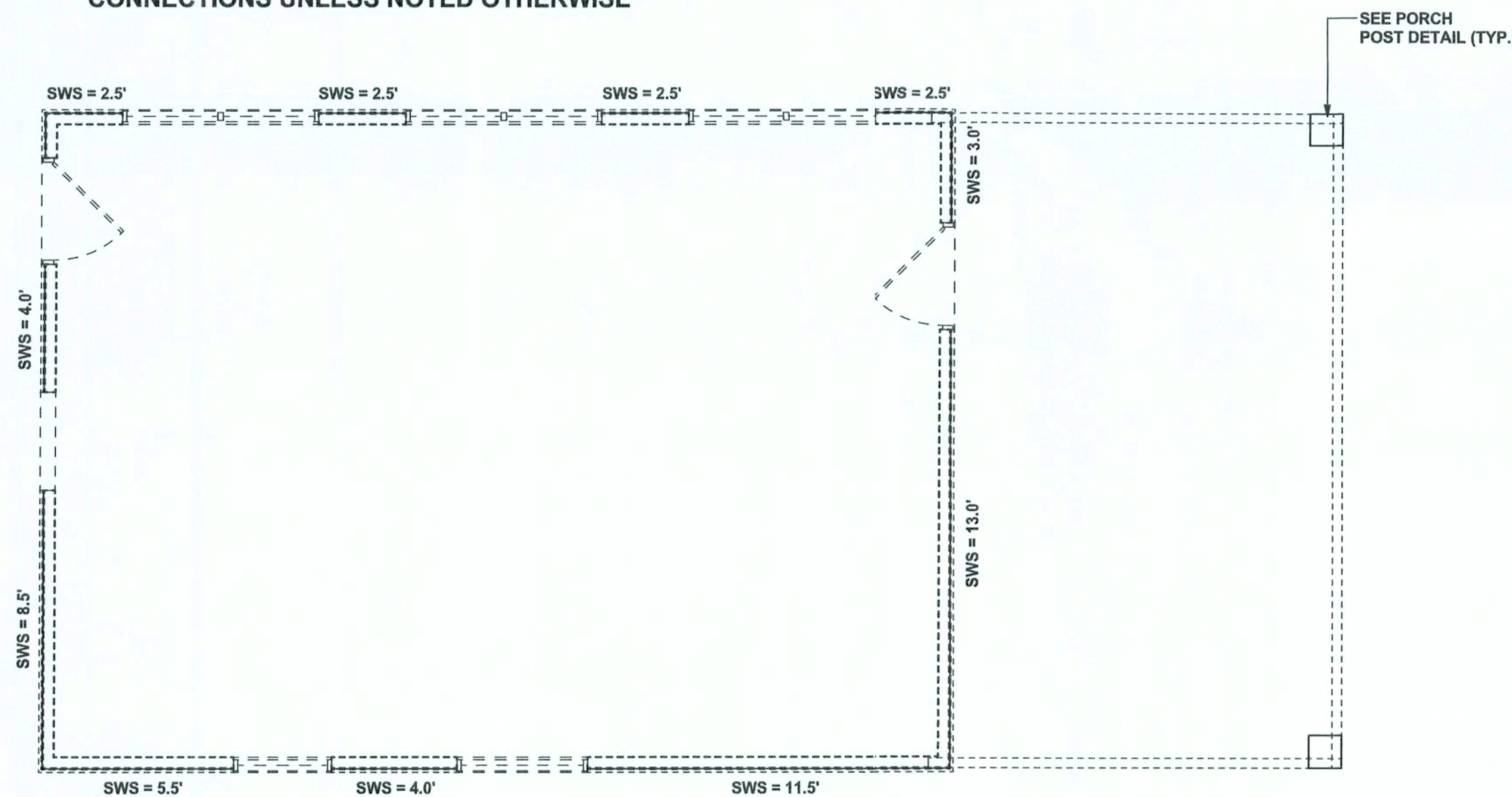


FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

USE H2.5A (5351b) FOR ALL TRUSS TO WALL FRAME AND PORCHBEAM
CONNECTIONS UNLESS NOTED OTHERWISE



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03. BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

| | |
|------------|--|
| SWS = 0.0' | 1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.) |
| SWS = 0.0' | 2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.) |
| IBW | 1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1 |
| IBW | 2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1 |

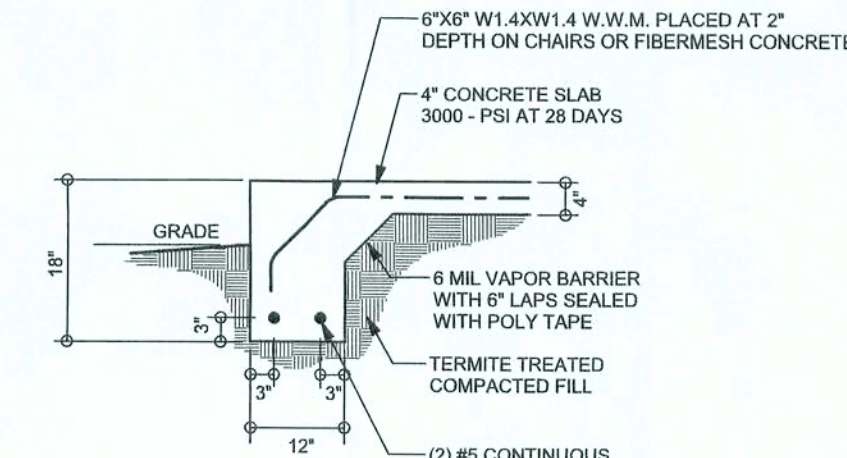
HEADER LEGEND

| | |
|-------------------|-------------------------------------|
| (2) 2X10X7, 1J 1K | HEADER/BEAM CALL-OUT (U.N.O.) |
| ↑ | NUMBER OF KING STUDS (FULL LENGTH) |
| ↑ | NUMBER OF JACK STUDS (UNDER HEADER) |
| ↑ | SPAN OF HEADER |
| ↑ | SIZE OF HEADER MATERIAL |
| ↑ | NUMBER OF PLIES IN HEADER |

TOTAL SHEAR WALL SEGMENTS

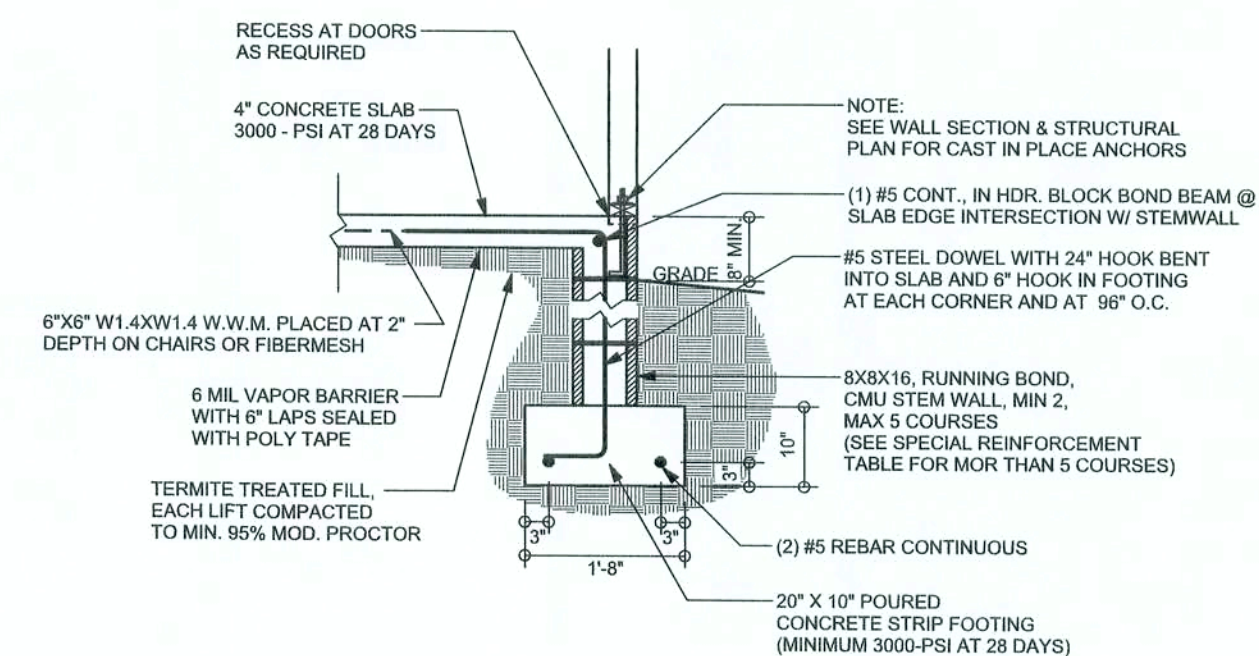
SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

| | REQUIRED | ACTUAL |
|--------------|----------|--------|
| TRANSVERSE | 21.0' | 28.5' |
| LONGITUDINAL | 15.0' | 31.0' |



F1 MONOLITHIC FOOTING

SCALE: 1/2" = 1'-0"



F9 STEM WALL FOOTING

SCALE: 1/2" = 1'-0"

WINDLOAD ENGINEER: Mark Disosway,
PE No. 53915, P.O. Box 868, Lake City, FL
32055, 386-75-5419

DIMENSIONS
Stated dimensions supersede scaled
dimensions. Refer all questions to
Mark Disosway, P.E. for resolution.
Do not proceed without clarification.

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permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section R301.2.1, Florida building
code, effective 2004, to the best of my
knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISOSWAY
P.E. 53915

3/3/6
SEAL

Ralph & Carolyn
Norris Garage

ADDRESS:
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Columbia County, Florida

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Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
March 03, 2006

DRAWN BY: David Disosway

CHECKED BY:

FINAL DATE:
03 / Mar /06

JO3 NUMBER:
602154

DRAWING NUMBER

S-2

OF 3 SHEETS