

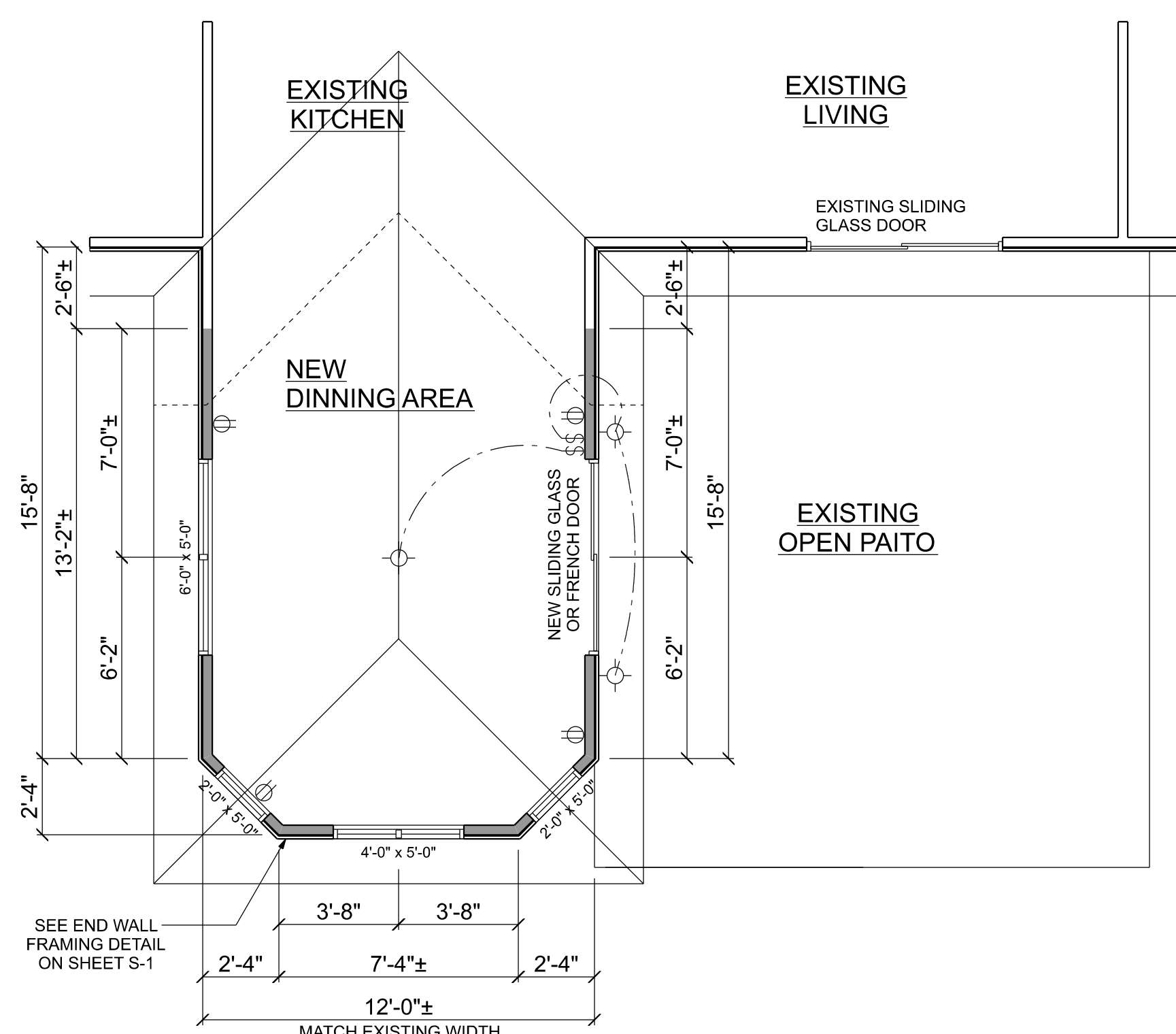
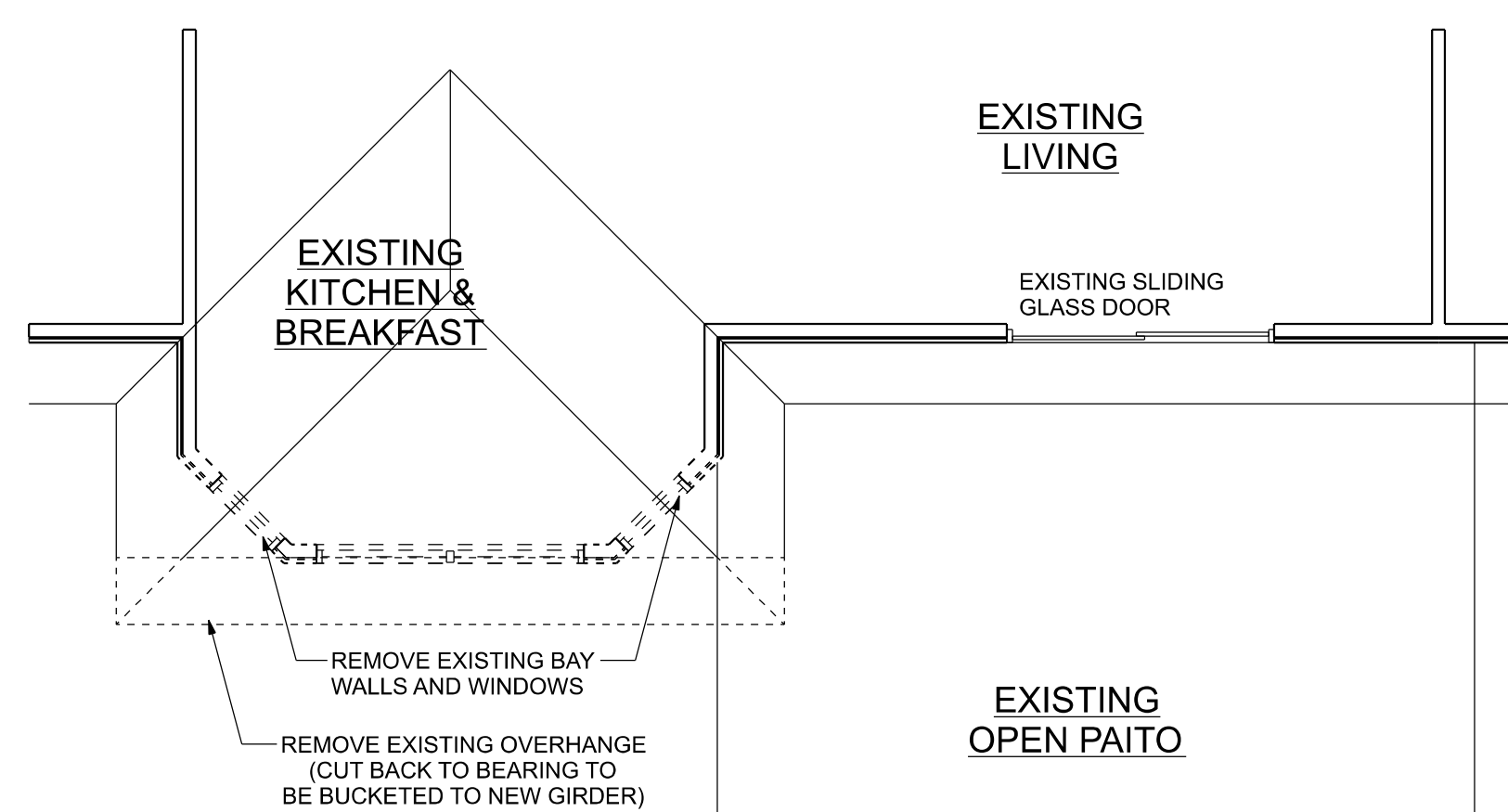
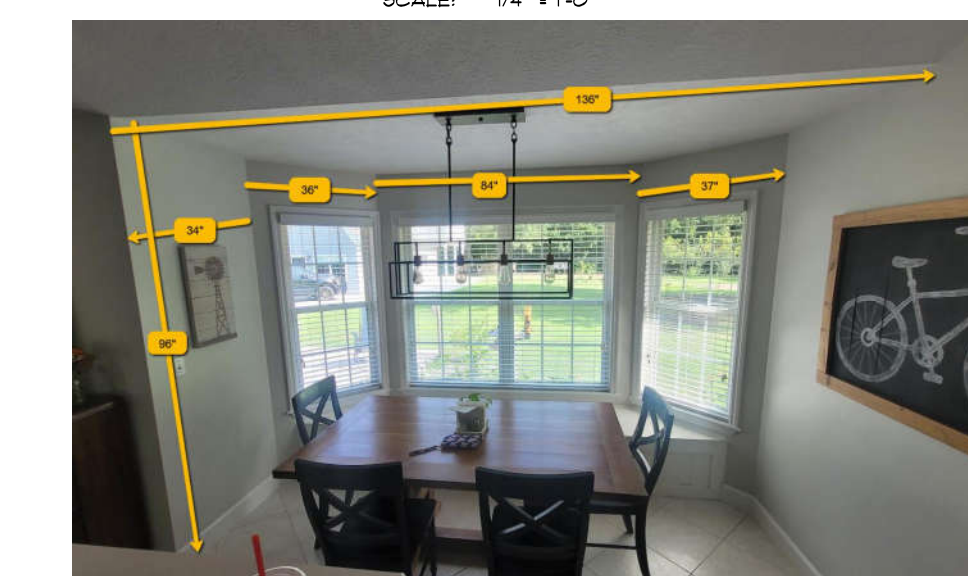
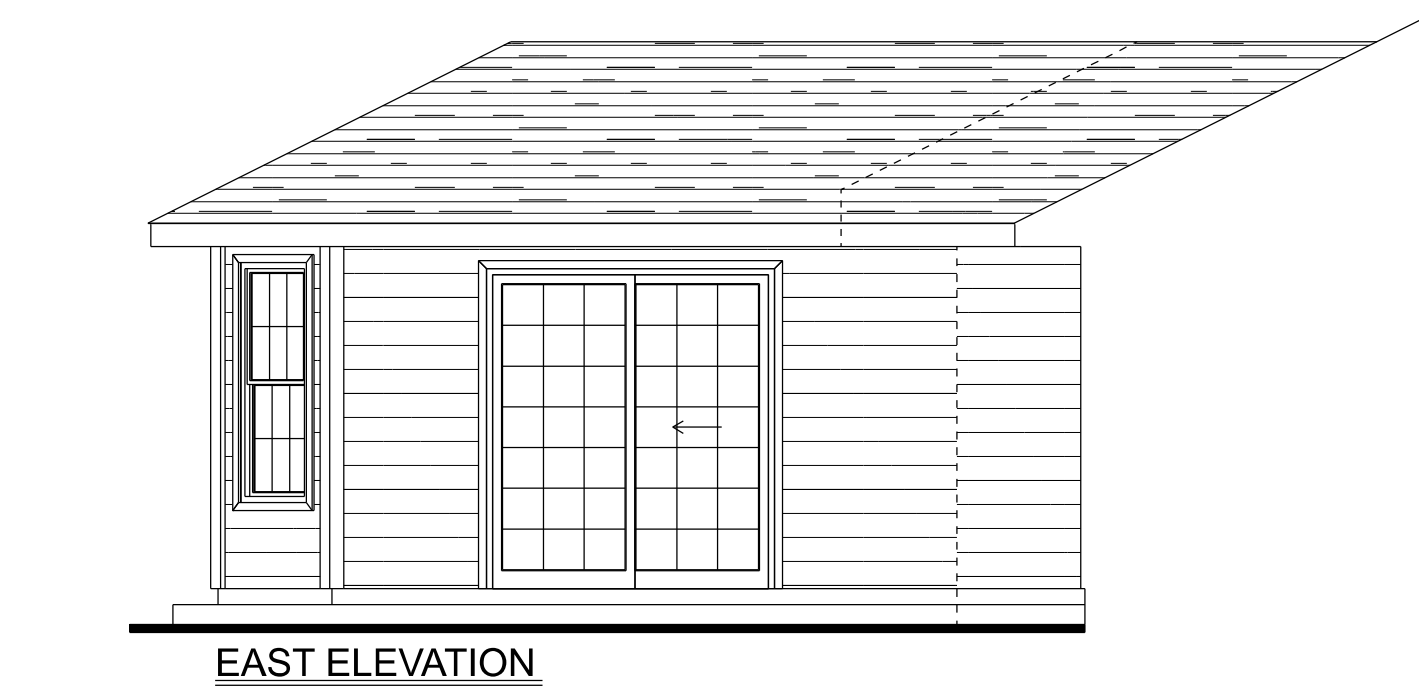
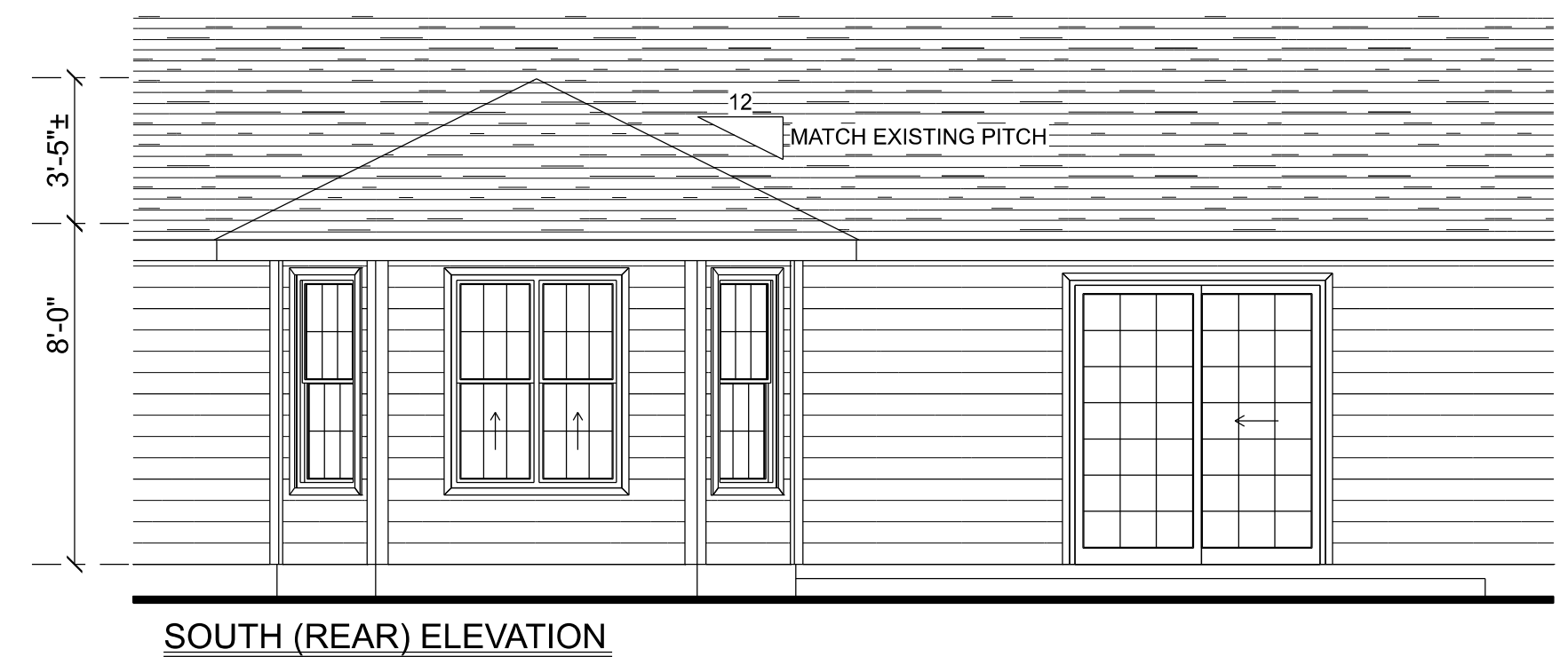
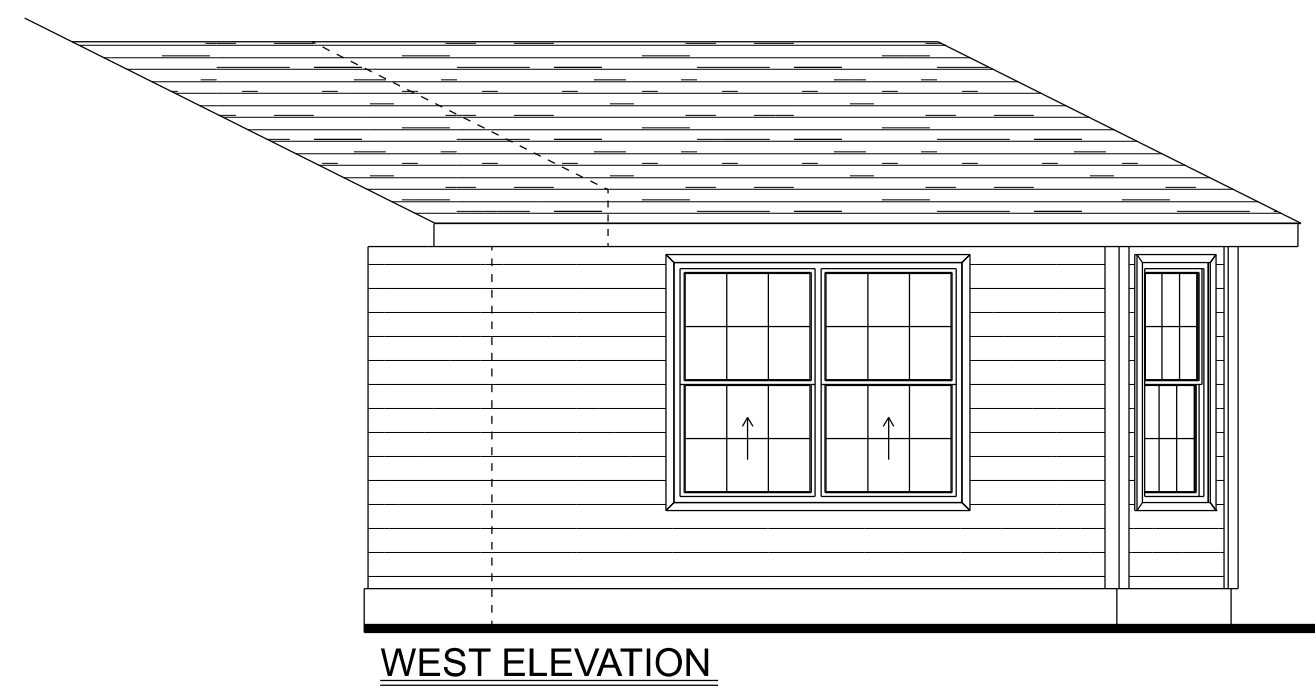


TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durawall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below:

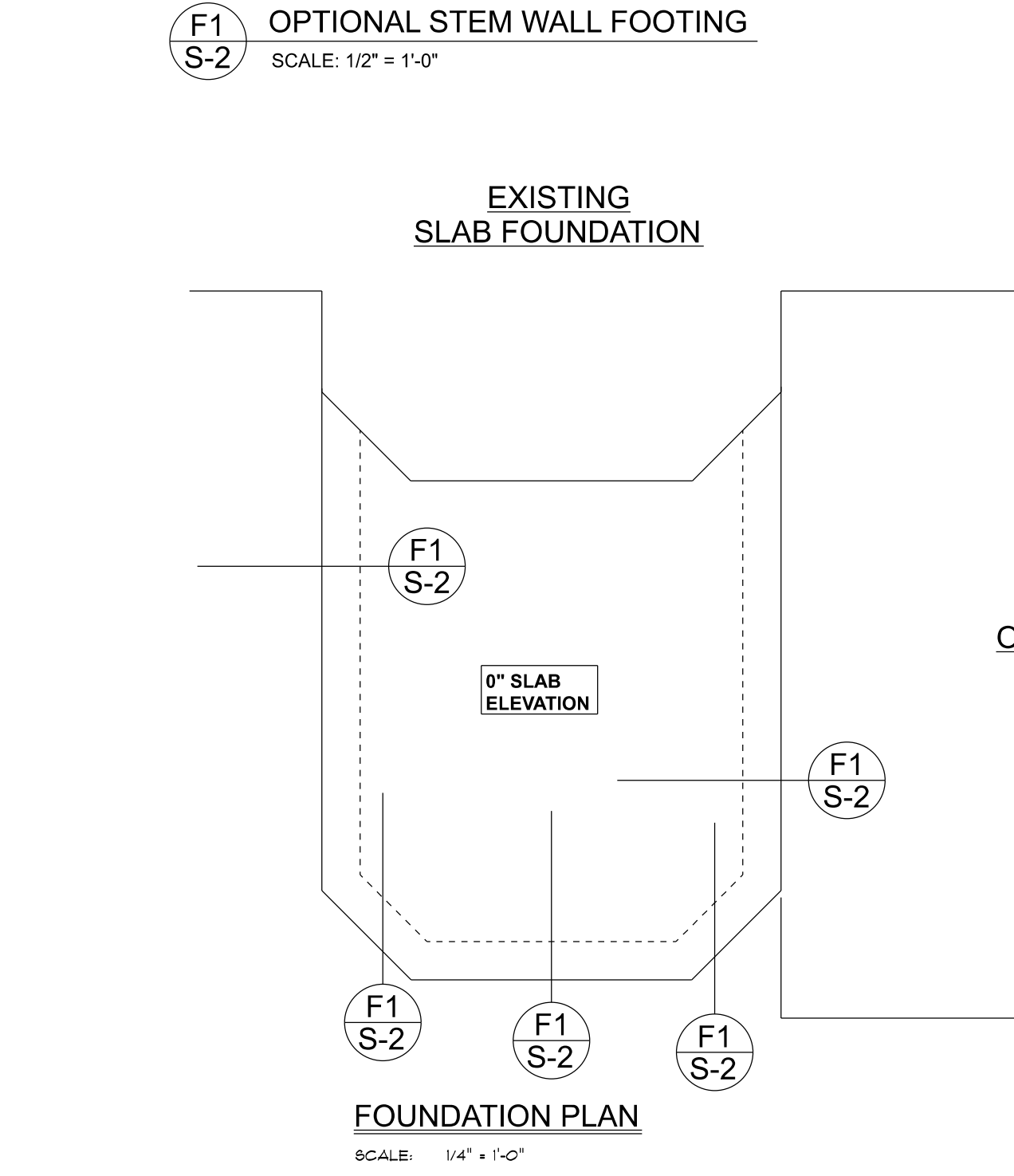
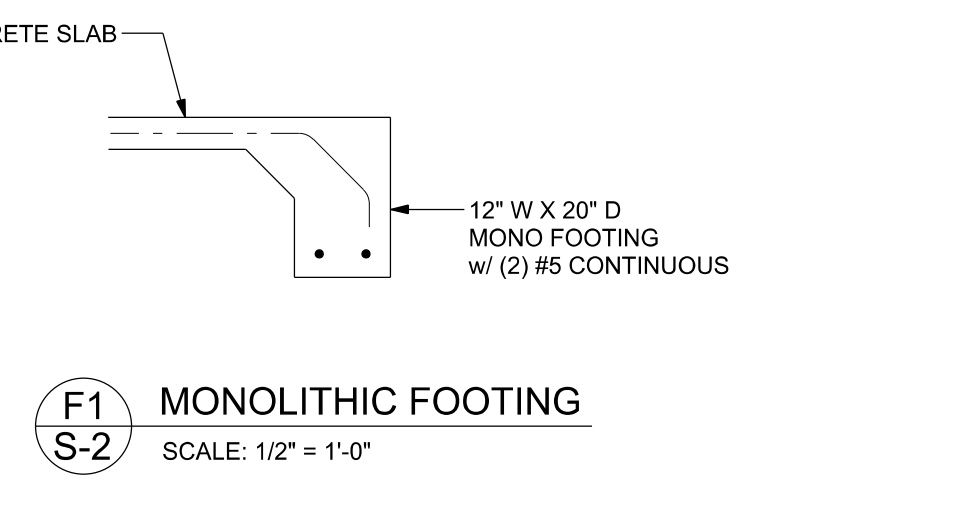
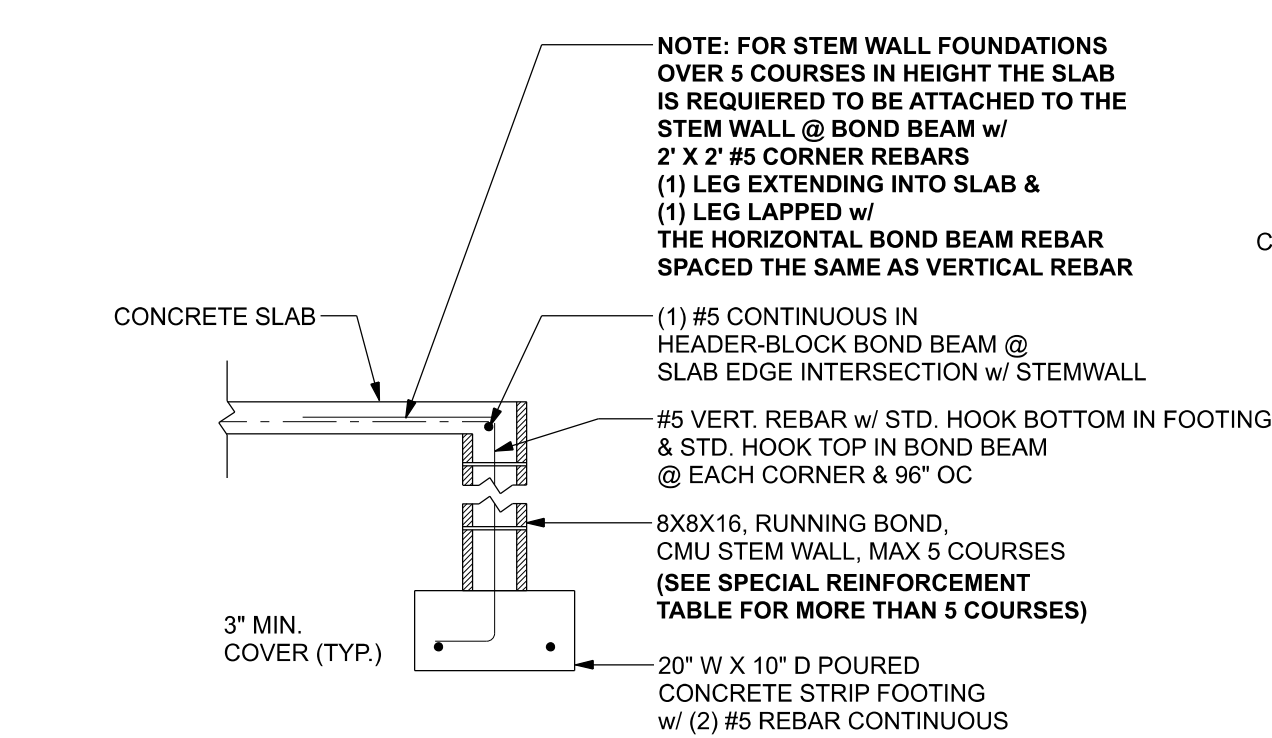
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength
2.1	Mortar
2.2	Grout
2.3	CMU standard
2.3	Clay brick standard
2.4	Reinforcing bars, #3 - #11
2.4F	Coating for corrosion protection
2.4F	Coating for corrosion protection
3.3.E.2	Pipes, conduits, and accessories
3.3.E.7	Movement joints

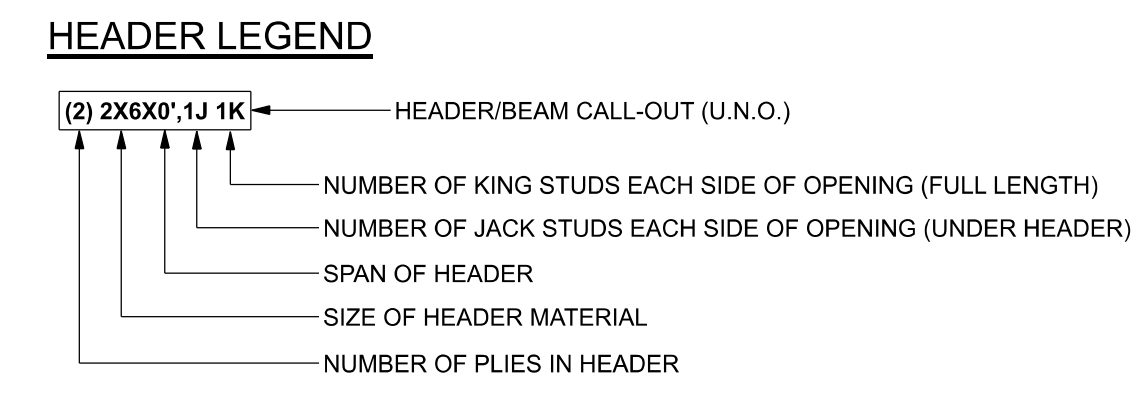


BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL.



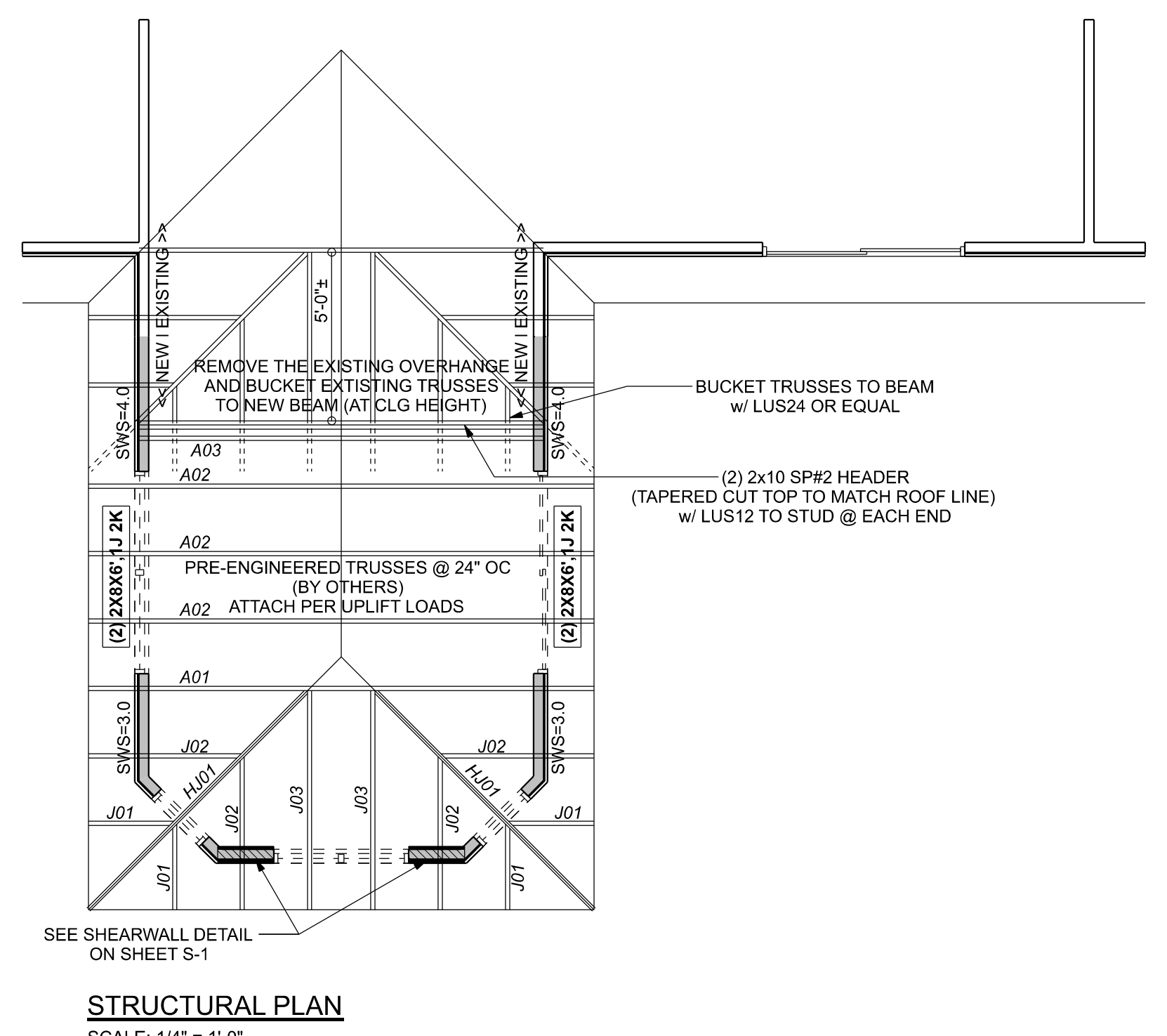
UNLESS NOTED OTHERWISE (MINIMUM REQUIREMENTS)
SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS

BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCREWED DOWN w/ MIN. OPTION #1 OR OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD



	TRANSVERSE	LONGITUDINAL
ACTUAL	1950 LBF	5544 LBF
REQUIRED	1855 LBF	2231 LBF

- STRUCTURAL PLAN NOTES**
- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
 - PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE



FOUNDATION NOTES

FN-1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING

FN-2 IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN

FN-3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X6-1/4" WELDED WIRE MESH PLACED ON CHAIRS @ 8" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD)

JONNY HANDY
REAR ADDITION
PROJECT ADDRESS:
320 SW Airline Glen
Lake City, FL 32024

FL PE 53915
This item has been digitally signed and sealed by Mark Disoway PE on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Mark Disoway
Digital Signature
C=US, O=Florida, dnQualifier=A01410C000017E97DE07CA000746F0, CN=Mark d Disoway
2024-09-01 03:54:01

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

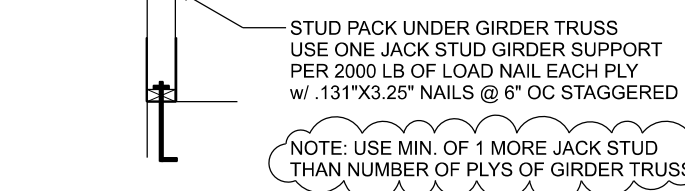
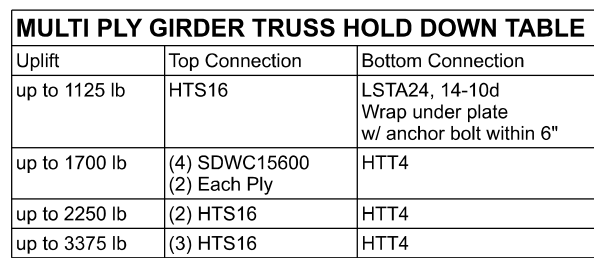
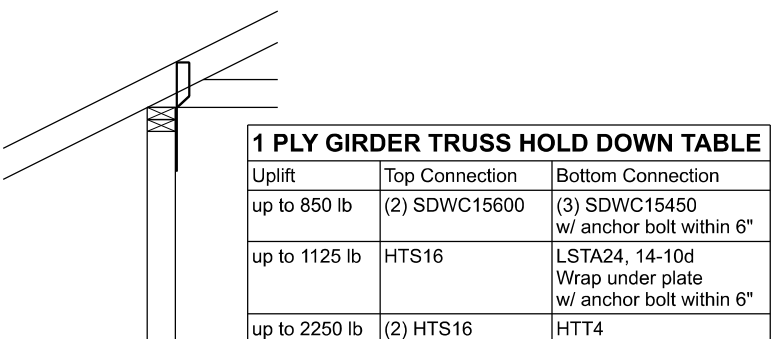
LIMITATION: This design is valid for one building, at specified location.

Mark Disoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
240962
#1
OF 2 SHEETS

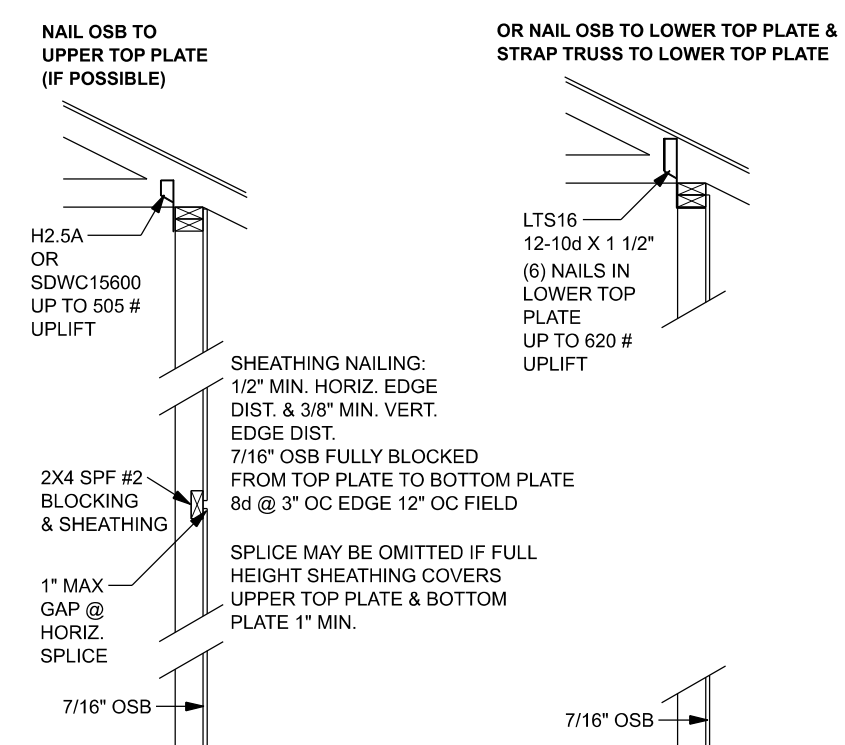
ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

Table with columns: Wind Speed, Sheathing Thickness, Required Nail, Nail spacing along panel edges, Nail spacing along intermediate supports in the panel field. Rows include 120 mph Exp. B, 130 mph Exp. C, 140 mph Exp. D, 150 mph Exp. C, 150 mph Exp. D.

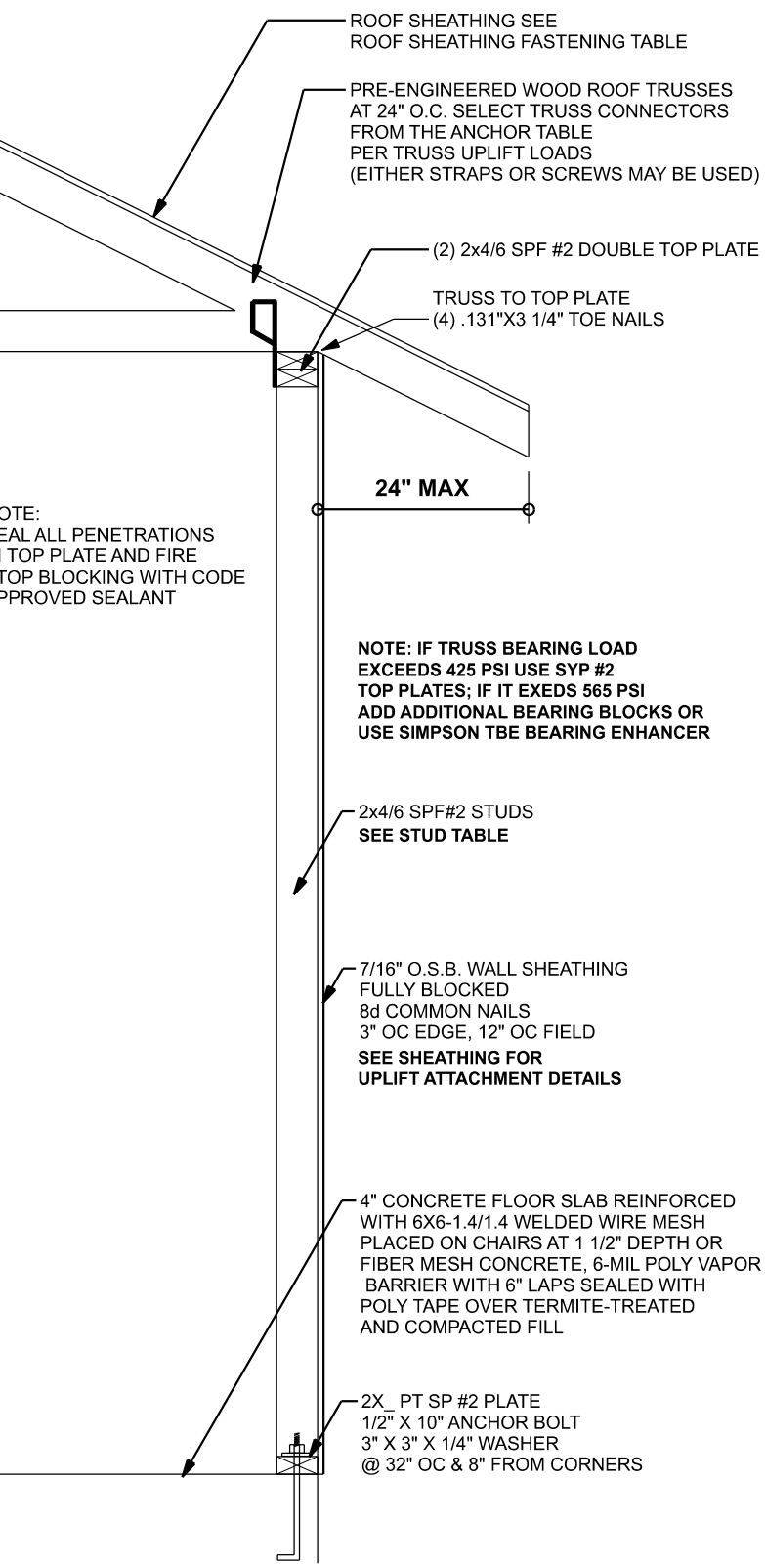


(TYP.) GIRDER TRUSS HOLD DOWN DETAIL WOOD FRAME w/ STRAPS & ANCHORS

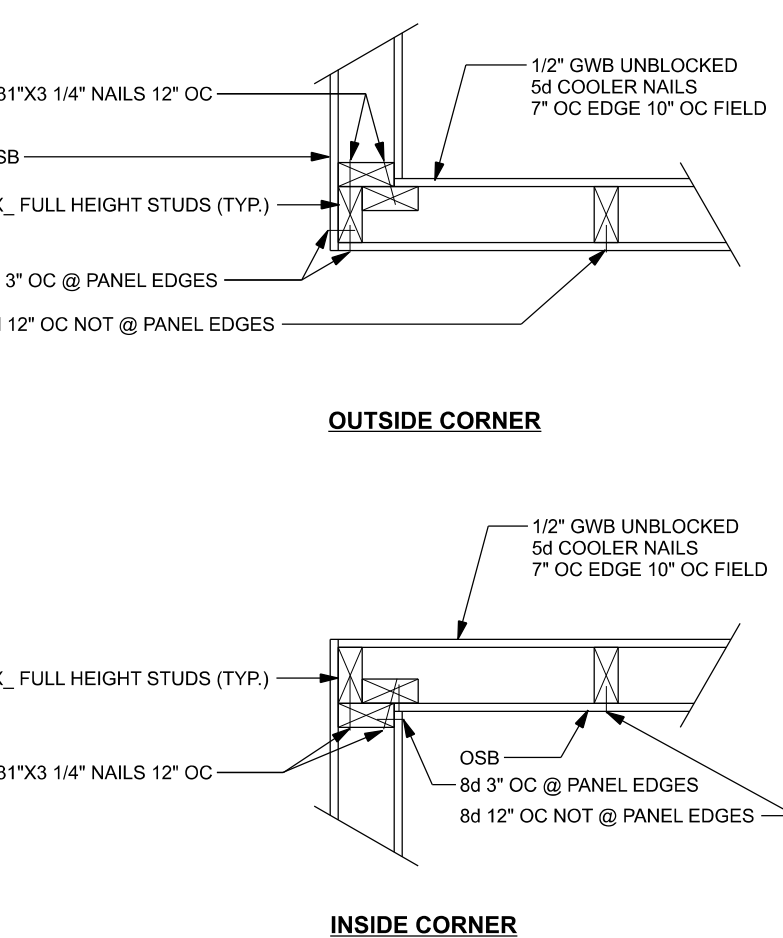
Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



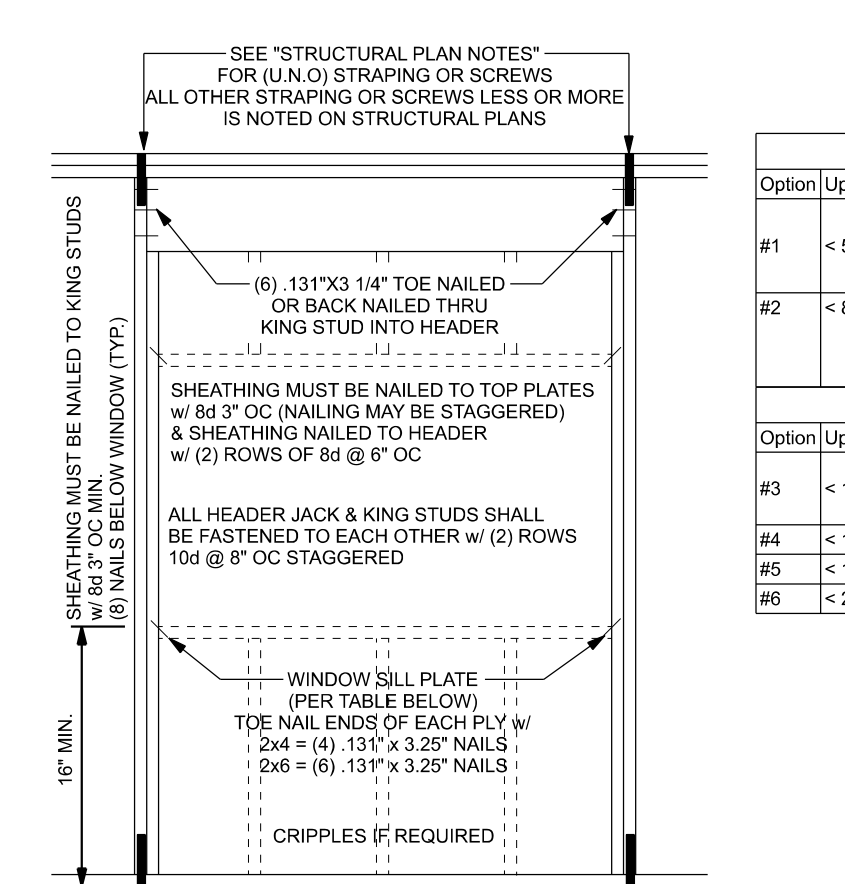
SHEATHING FOR UPLIFT ATTACHMENT DETAILS ONE STORY WOOD FRAME



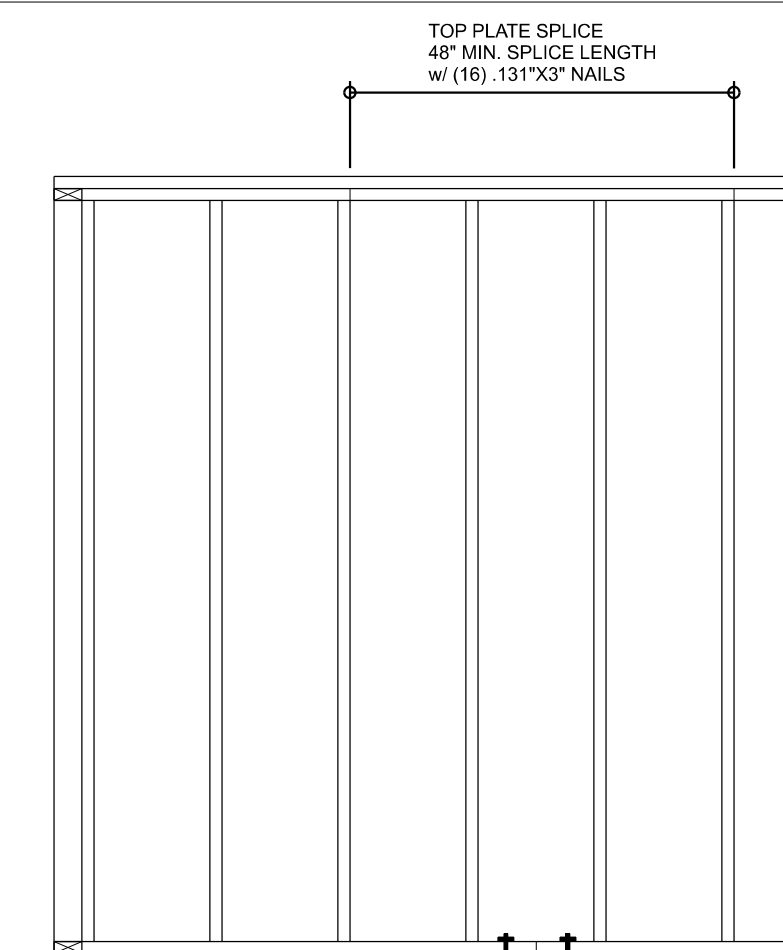
ONE STORY WALL SECTION SCALE: 3/4" = 1'-0"



(TYP.) CORNER FRAMING WOOD FRAME



TYPICAL HEADER STRAPING OR SCREWS DETAIL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

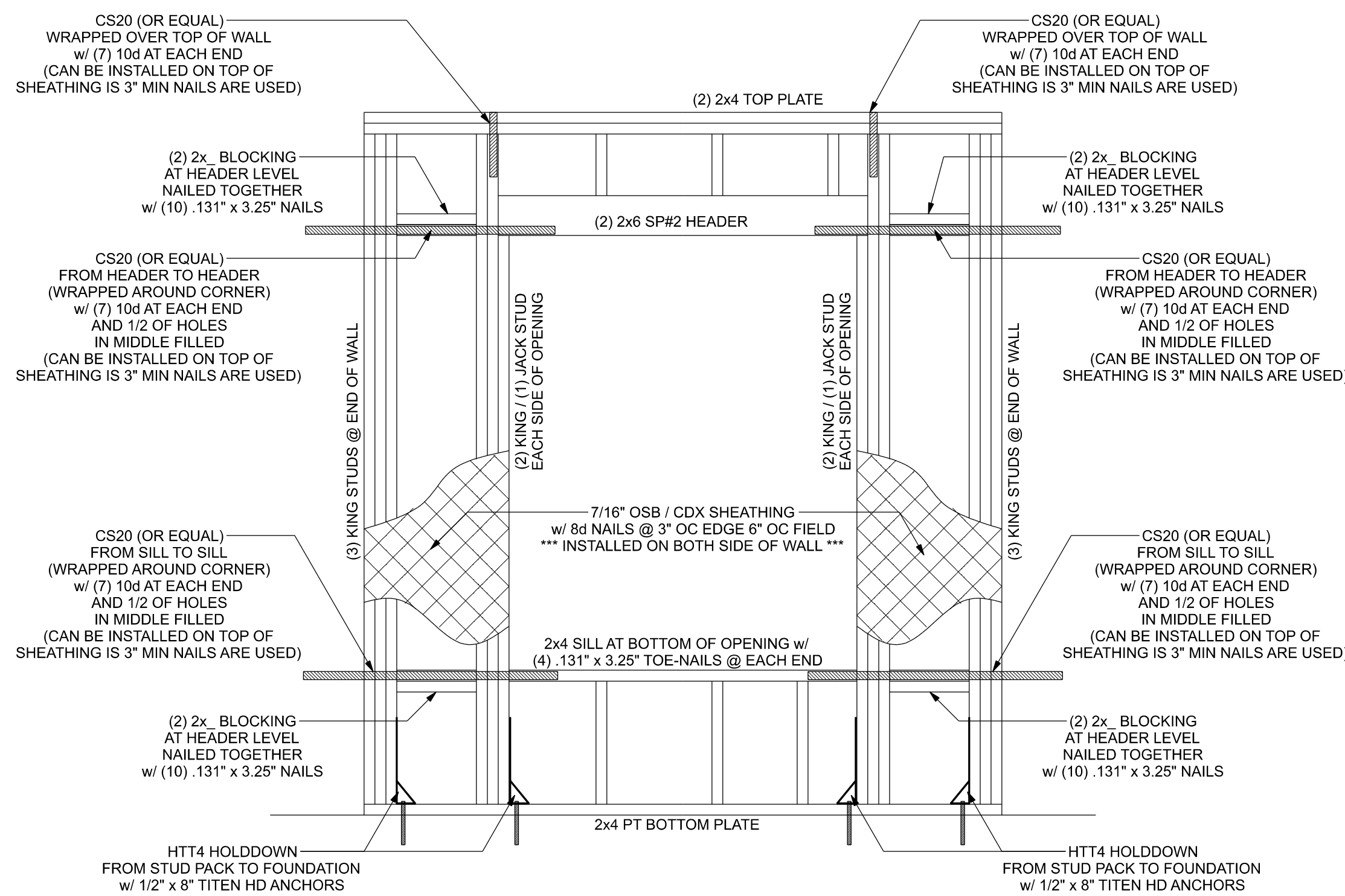


(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME

HEADER SCREWS TABLE with columns: Option, Uplift, Top Connection, Bottom Connection. Rows include #1, #2, #3, #4, #5, #6.

HEADER STRAP TABLE with columns: Option, Uplift, Top Connection, Bottom Connection. Rows include #3, #4, #5, #6.

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT table with columns: DESIGN, WIND SPEED, MAX. SPANS FOR SPF #2, BASED ON WFCM TABLE A.3.2(8). Rows include 130 MPH EXP. C.



END WALL FRAMING (SHEATHING ON BOTH SIDES)

CONNECTOR TABLE with columns: Uplift SP, Uplift SPF, Truss Connector, To Plate, To Truss/Rafter. Rows include 805, 615, 1340, 720, 1450, 1235, 1640, 1030, 585, 1085, 771, 1235, 1825, 4235, 1825, 4235, 2475, 1900, 2475.

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS: THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR. Rows include (1) 2x4 @ 16" OC, (1) 2x4 @ 12" OC, (1) 2x6 @ 16" OC, (1) 2x6 @ 12" OC.

GRADE & SPECIES TABLE with columns: GRADE, SP #2, Fb, E. Rows include 2x8, 2x10, 2x12, GLB, LSL, LVL, PSL.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCL. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL RAFTERS AND TRUSSES. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S BASIS ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 4x16 EACH END, 2X6 RAFTERS 700 LB EACH END. SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOils TEST PROVES OTHERWISE). CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 2500 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, F8 + 8KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.F.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'. FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 12 TO 18 INCHES, DOSAGE AMOUNTS FROM 0.175 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. ANCHORS: ANCHORS AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL. CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. JOINT RATIOS OF SLAB AREAS SHALL NOT EXCEED 1:5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND ARCHITECT'S REQUIREMENTS FOR CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE). REBAR: ASTM A615, GRADE 40, DEFORMED BARS, Fy = 60 KSI. ALL LAP SPLICES 40" DB (25" FOR 45 BARS); UNLO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, U.S.C. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS: SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO DRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED. STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS' INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS. ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUVED CMU. BUILDER'S RESPONSIBILITY: THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCL REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. ROOF SYSTEM DESIGN: THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCL IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCL REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

JONNY HANDY REAR ADDITION PROJECT ADDRESS: 320 SW Airfield Glen Lake City, FL 33024

FL PE 53915 This item has been digitally signed and sealed by Mark Disoway PE on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

C=US, O=Florida, dnQualifier=A014 10C000017E97 DE07CA000746F0, CN=Mark d Disoway 2024-09-01 03: 54:23

DESIGN CRITERIA & LOADS: BUILDING CODE 8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023), WINDLOADS BASIC WIND SPEED 130 MPH, WIND EXPOSURE 1, RISK CATEGORY II, ENCLOSURE CLASSIFICATION ENCLOSED, COEFFICIENT 0.18, ROOF ANGLE 7-45 DEGREES, MEAN ROOF HEIGHT 30 FT, C&C DESIGN PRESSURES SEE TABLE, FLOOR LOADING ROOMS OTHER THAN SLEEPING ROOMS 40 PSF LIVE LOAD, SLEEPING ROOMS 30 PSF LIVE LOAD, ROOF LOADING FLAT OR < 4:12 20 PSF LIVE LOAD, 4:12 TO < 12:12 16 PSF LIVE LOAD, 12:12 & GREATER 12 PSF LIVE LOAD, SOIL BEARING CAPACITY 1500 PSF, THIS BUILDING IS NOT IN THE FLOOD ZONE.

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C) EFFECTIVE WIND AREA (FT2) ZONE 4 INTERIOR ZONE 5 END 4 FROM ALL OUTSIDE CORNER, 0 - 20 +25.6(Vasd) -27.8(Vasd) +25.6(Vasd) -34.2(Vasd), 0 - 20 +42.6(Vult) -46.2(Vult) +42.6(Vult) -57.0(Vult)

JOB NUMBER: 240962 S-1 OF 2 SHEETS