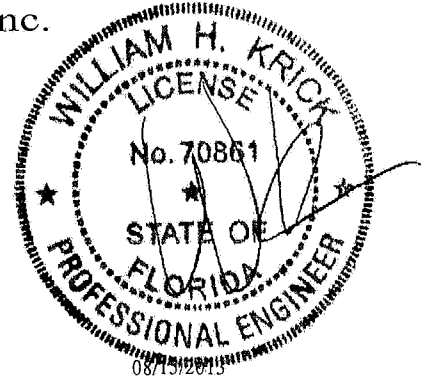


ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1UYS487-Z0115100640



Truss Fabricator **Anderson Truss Company**
Job Identification **13-215B--BRYAN ZECHER Odum Residence -- Lake City, FL**
Truss Count **23**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 12.03.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61G15-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes

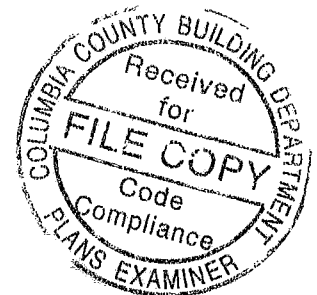
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-PB16010-12030EC1-

#	Ref	Description	Drawing#	Date
1	35503-A1	60'9"8 Specia	13226008	08/14/13
2	35504-A2	60'9"8 Specia	13226007	08/14/13
3	35505-A3	60'9"8 Specia	13227015	08/15/13
4	35506-A4	60'9"8 Specia	13227016	08/15/13
5	35507-A5	60'9"8 Specia	13227018	08/15/13
6	35508-A5A	60'9"8 Spec	13227017	08/15/13
7	35509-A6	60'9"8 Specia	13226009	08/14/13
8	35510-AGE	60'9"8 Gable	13226019	08/14/13
9	35511-AGE1	60'9"8 Gabl	13226018	08/14/13
10	35512--B1	55' Special	13226005	08/14/13
11	35513--B2	55' Special	13226001	08/14/13
12	35514--B3	55' Special	13226006	08/14/13
13	35515-B4	45'8"12 Spec	13226011	08/14/13
14	35516-BGE	45'8"12 Gabl	13226017	08/14/13
15	35517--C1	23'2" Common	13226016	08/14/13
16	35518--C2	23'2" Common	13226004	08/14/13
17	35519--C3	18' Common	13226015	08/14/13
18	35520-PBA	13'11"12 Com	13226010	08/14/13
19	35521-PBA1	13'11"12 Ga	13226002	08/14/13
20	35522-PBB	6'2"4 Common	13226003	08/14/13
21	35523-PBB1	6'2"4 Gable	13226012	08/14/13
22	35524--PBC	23'2" Gable	13226014	08/14/13
23	35525--PBC1	18' Gable	13226013	08/14/13



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

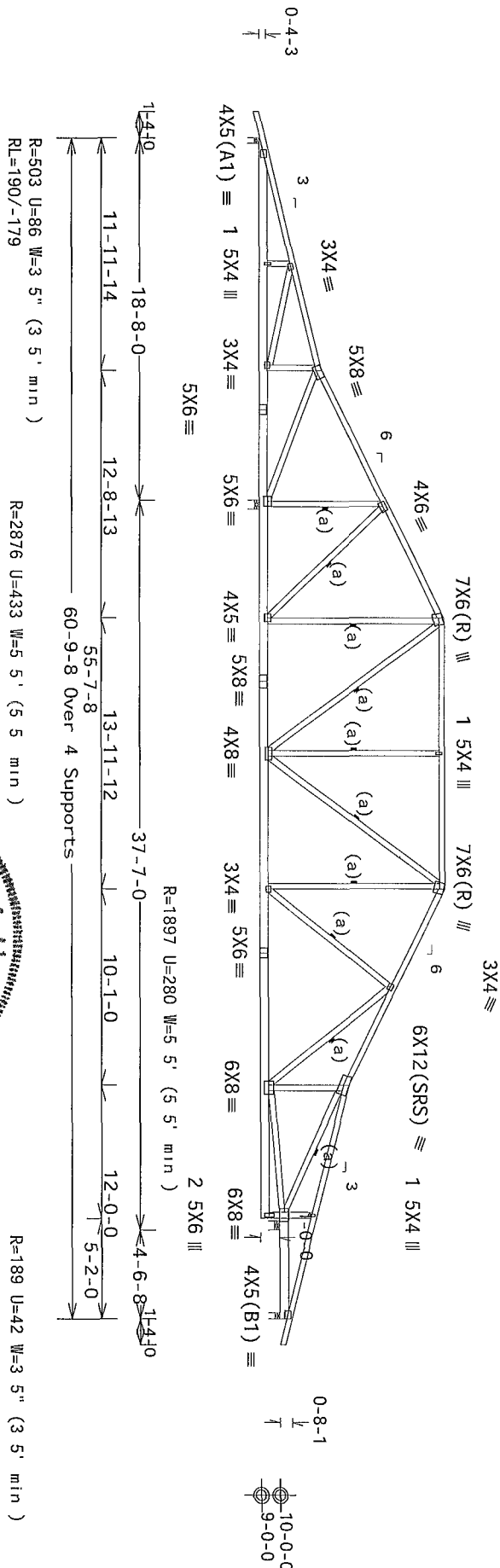
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CL05ED bldg. Located
anywhere in roof, RISK CAT II, EXP C, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCP1(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DMG to the installation contractor. Failure to follow provisions of BCS1 in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and untrained people to install trusses. See "WARNING" note below BCS1 and instruct retaining a registered professional engineer for the design of temporary bracing.



Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)
-------------	--

Scale = .125"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

A circular professional engineer seal for the State of Florida. The outer ring contains the text "STATE OF FLORIDA" at the top and "PROFESSIONAL ENGINEER" at the bottom. Inside the ring, the number "12586" is prominently displayed in the center. Below the number, the date "08/15/2011" is printed. The seal is stamped over a document that includes a table with columns for "DATE", "DESCRIPTION", and "AMOUNT". The "DATE" column contains the date "08/15/2011". The "DESCRIPTION" column contains the text "PROFESSIONAL ENGINEER". The "AMOUNT" column contains the text "12586".

08/15/2013

TC LL	20 0 PSF	REF R487-- 35503
IC DL	7 0 PSF	DATE 08/14/13
BC DL	10 0 PSF	DRW HCUSR487 13226000
BC LL	0.0 PSF	HC-ENG JB/MWP
TOT LD.	37.0 PSF	SEON- 314056
DUR FAC	1.25	
SPACING	24.0"	JREF- 1UYS487_Z01

JKER- 10Y3401_201

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

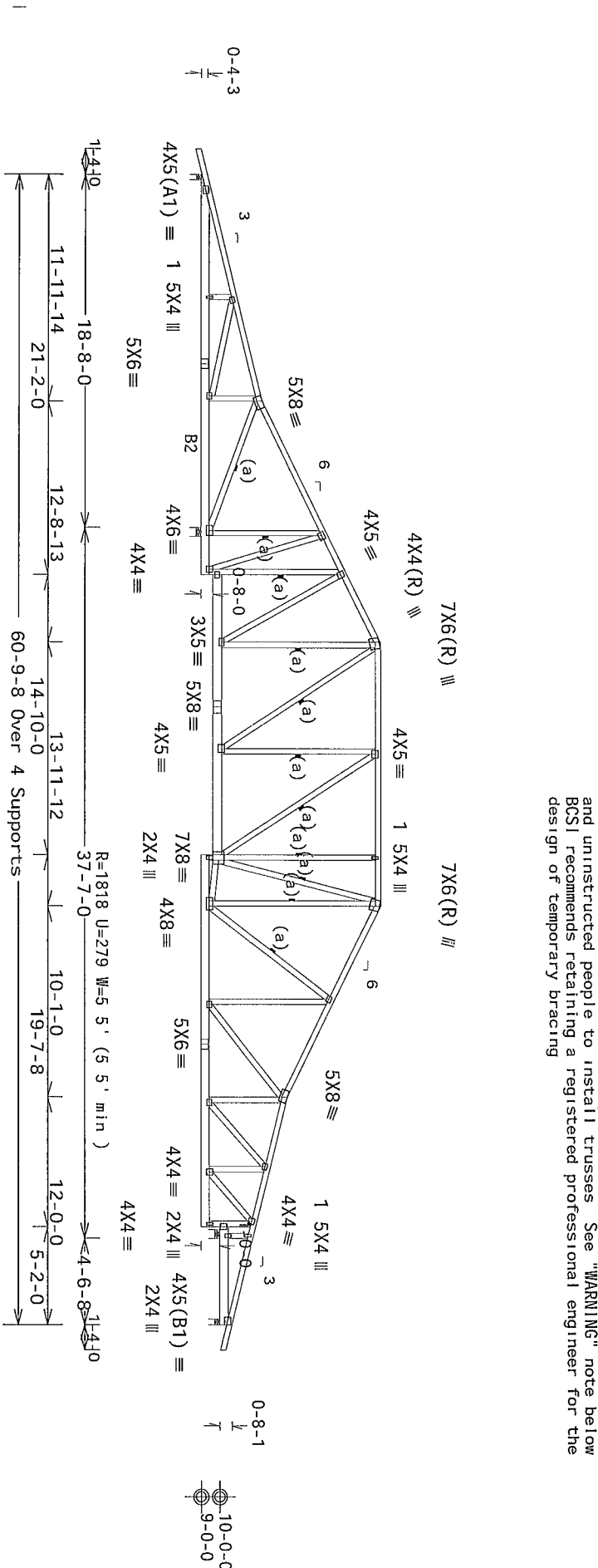
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP C, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MNFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Finish a copy of this DWG in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and untrained people to install trusses. See "WARNING" note below BCS1 recommends retaining a registered professional engineer for the design of temporary bracing.



R=164 U=61 W=35" (35' min)

Design Crit FBC2010Res/TP1-2007(STD)

$$\text{FT/RT} = 10\%(0\%) / 0(0)$$

*****WARNING***** READ AND FOLLOW ALL NOTES ON THIS SHEET!
*****IMPORTANT***** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

2021

Orlando FL, 32837
FL COA #0278

ITW Building Components Group Inc. shall not be responsible for any deviation from its published specifications or drawings if such deviation results from any failure to build the truss in conformance with the ANSI/TPI-1 or for handling or bracing of trusses. Apply plates to each face of truss and position as shown above and on drawings. The manufacturer's name and logo must appear on all printed specifications, drawings or cover pages. Letting bid drawings are not to draw new dimensions or details not shown on drawings. The user liability for the design and use of the information for any responsibility by the building design group per ANSI/TPI-1 Sec 2. For more information see www.itwbc.com

ITW BCG www.itwbc.com www.printc.org www.steelsupply.com

general notes page www.itwbc.com www.printc.org www.steelsupply.com

ICD www.itwbc.com www.printc.org www.steelsupply.com

[illegible]

FL/-/3/-/-/R/-		Scale = .125"/Ft.	
TC LL	20 0 PSF	REF	R487-- 35504
IC DL	7.0 PSF	DATE	08/14/13
BC DL	10 0 PSF	DRW	HCURS487 1322007
BC LL	0.0 PSF	HC-ENG	JB/WMPF
TOT LD.	37 0 PSF	SEQN-	314057
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UYS487_Z01

(13-215B--BRYAN ZECHER Odum Residence -- Lake City, FL - A3 60'9"8 Special)

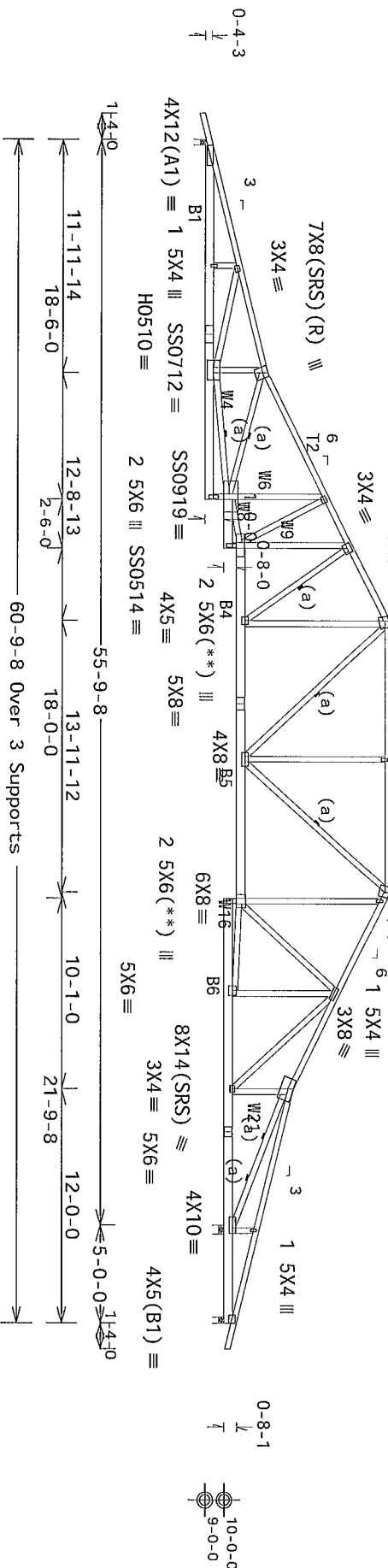
Top chord 2x4 SP #1 12A T2 2x4 SP M-30
Bot chord 2x6 SP #1 Dense 12A B1 2x6 SP SS 12A
B4 B5 B6 2x6 SP #2 12A
W4 W6 W8 W9 W16 W21 2x4 SP #1 12A
Webs 2x4 SP #3 12A
Lumber grades designated with 12A use design values approved
1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1 2013
and shall only be used on projects designed and permitted prior to
this date unless specifically approved in writing by the building
authority having jurisdiction the building designer and the project
owner

Special loads

-----Lumber Dur Fac = 1.25 / Plate Dur Fac = 1.25
TC- From 54 pif at 1.33 to 54 pif at 11.99
TC- From 56 pif at 1.33 to 56 pif at 24.73
TC- From 54 pif at 24.73 to 54 pif at 38.71
TC- From 56 pif at 38.71 to 56 pif at 48.79
TC- From 54 pif at 48.79 to 54 pif at 62.12
BC- From 4 pif at 1.33 to 4 pif at 0.00
BC- From 20 pif at 0.00 to 20 pif at 10.00
BC- From 20 pif at 10.00 to 20 pif at 18.50
BC- From 20 pif at 18.50 to 20 pif at 21.00
BC- From 20 pif at 21.00 to 20 pif at 29.00
BC- From 20 pif at 29.00 to 20 pif at 39.00
BC- From 20 pif at 39.00 to 20 pif at 51.00
BC- From 4 pif at 51.00 to 4 pif at 62.12
BC- From 200 00 lb Conc Load at 19.17

Wind loads and reactions based on MMFRS with additional C&C member design



R=2239 U=56 W=3 5" (3 5' min)
RL=190/-179

R=3218 U=82 W=5 5" (5 5' min)
R=-714 Rw=38 U=301 W=3 5" (3 5' min)

PLT TYP. 20 Gauge HS, 18 Gauge HS, Design Crit. FBC2010Res/TP1-2007(STD)
Wave
FL/RT=10%(0%)/0(0)

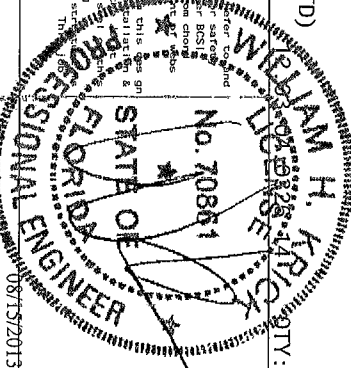
WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling shipping installing and bracing
Follow the latest edition of BCS1 (Building Component Safety Information by TPI and WDA)
Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord
shall have properly attached rigid ceiling locations shown for permanent lateral restraint
any building components Group (the (T)BSC) shall not be responsible for any device on this design
that is not shown or for handling shipping installing and bracing of trusses. Apply plates
bracing of trusses. Apply plates bracing of trusses. Apply plates bracing of trusses. Apply plates
drawing or cover page 1 string on s drawing. The submittal and use of this design for any
response b i ty solely for the design per AS/TP1 1 Sec 2. For more information see
the responsibility of the building designer per AS/TP1 1 Sec 2. WDA www.wda-industry.com
IDC www.idcstar.org

ALPINE

ITW Building Components Group Inc.

Orlando FL 32837
FL COA #0278



TC LL	20 0 PSF	REF R487-- 35505
TC DL	7.0 PSF	DATE 08/15/13
BC DL	10.0 PSF	DRW HCUS487 13227015
BC LL	0 0 PSF	HC-ENG WHK/WHK
TOT. LD.	37 0 PSF	SECN- 314333
DUR FAC.	1 25	
SPACING	24.0"	JREF- 1UYS487_Z01

Scale = .125"/Ft.

(13-215B--BRYAN ZECHER Odum Residence -- Lake City FL - A4 60'9"8 Special)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TROSS MFR

Top chord 2x4 SP #1_12A T2 2x4 SP M-30
Bot chord 2x6 SP #2_12A B1 B2 B3 2x6 SP #1_Dense_12A
W4 W6 W8 W9 W21 2x4 SP #1_12A
Lumber grades designated with 12A use design values approved
1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1 2013
and shall only be used on projects designed and permitted prior to
this date unless specifically approved in writing by the building
authority having jurisdiction the building designer and the project
owner

Negative reaction(s) of -678# MAX (See below) from a non-wind load
case requires uplift connection

(**) 2 plate(s) require special positioning Refer to scaled plate
plot details for special positioning requirements

120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg not located
within 17 00 ft from roof edge RISK CAT I EXP C wind TC DL=3 5
psf wind BC DL=5 0 psf GCPI (+/-)=0 18

Wind loads and reactions based on MMFRS with additional CXC member
design

In lieu of structural panels use purlins to brace all flat TC @ 24
OC

Bottom chord checked for 10 00 psf non-concurrent live load

Calculated vertical deflection is 0 64 due to live load and 0 85 due
to dead load at X = 18-4-7

MMFRS loads based on trusses located at least 15 00 ft from roof
edge

Calculated horizontal deflection is 0 14 due to live load and 0 19
due to dead load

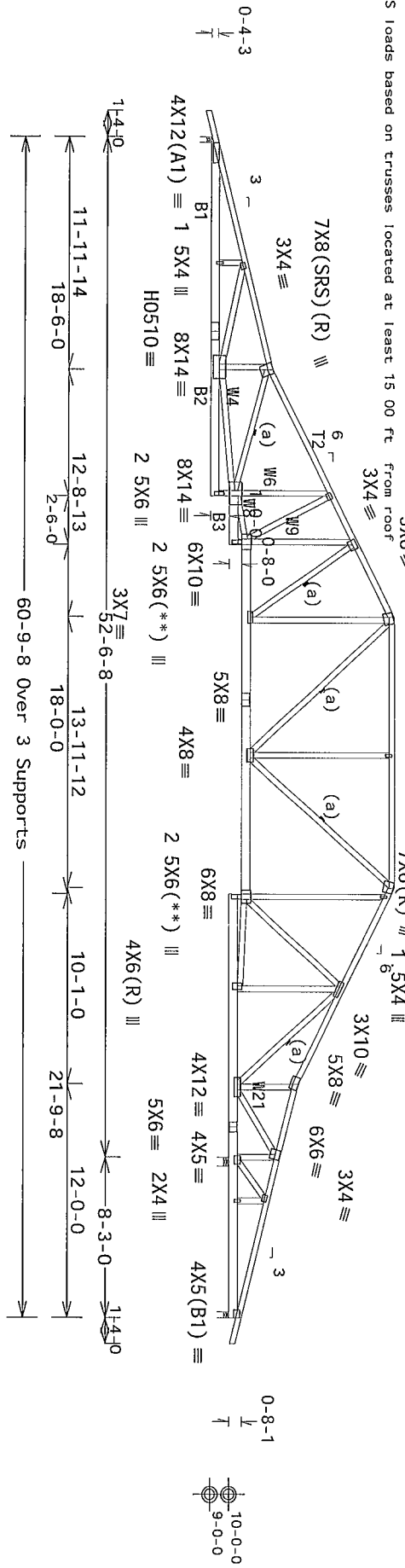
(a) Continuous lateral bracing equally spaced on member
Truss passed check for 20 psf additional bottom chord live load in
areas with 42'-high x 24'-wide clearance

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50

WARNING Furnish a copy of this DWG to the installation contractor
Failure to follow provisions of BCSI in handling and installation of
trusses can result in serious injuries Do not permit inexperienced
and untrained people to install trusses See WARNING note below
BCSI recommends retaining a registered professional engineer for the
design of temporary bracing

SPECIAL LOADS

-----LUMBER DUR FAC = 1 25 / PLATE DUR FAC = 1 25	
TC - From	59 PLF at 1 33 to 59 PLF at 0 00
TC - From	54 PLF at 0 00 to 54 PLF at 11 99
TC - From	56 PLF at 11 99 to 56 PLF at 24 73
TC - From	54 PLF at 24 73 to 54 PLF at 38 71
TC - From	56 PLF at 38 71 to 56 PLF at 48 79
TC - From	54 PLF at 48 79 to 54 PLF at 60 79
TC - From	59 PLF at 60 79 to 59 PLF at 62 13
TC - From	20 PLF at 0 00 to 20 PLF at 25 17
BC - From	20 PLF at 25 17 to 20 PLF at 28 33
BC - From	20 PLF at 28 33 to 20 PLF at 35 33
BC - From	60 PLF at 35 33 to 60 PLF at 39 00
BC - From	20 PLF at 39 00 to 20 PLF at 60 79
BC - From	200 LB Conc Load at 19 17



R=2163 U=53 W=3 5" (3 5" min)
R=190/-179

R=3485 U=78 W=5 5" (5 5" min)
R=679 Rw=28 U=258 W=3 5" (3 5" min)

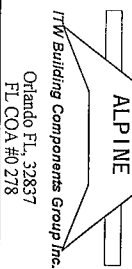
PLT TYP. 20 Gauge HS, Wave

Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%/0/0/0)

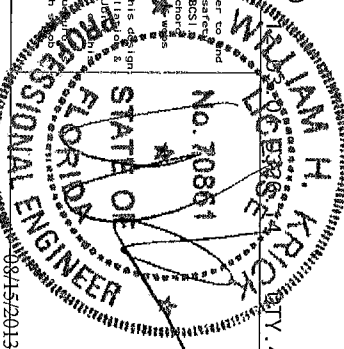
FL/-/3/-/-/R/- Scale = .125"/Ft.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, shipping, handling, and bracing. Follow the latest edition of BCSI (Building Components Safety Information) by TPI and WTC. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r and ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3 B7 or B10 as applicable.



ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or for any damage to the truss or its components. The user of this design is responsible for the design of the building and the use of this design for any structure. The user of this design is responsible for the design of the building and the use of this design for any structure. The user of this design is responsible for the design of the building and the use of this design for any structure.



TC LL	20 0 PSF	REF	R487-- 35506
TC DL	7 0 PSF	DATE	08/15/13
BC DL	10 0 PSF	DRW	H0US487 13227016
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT LD.	37 0 PSF	SEQN-	314354
DUR. FAC	1 25	JREF-	1UYS487_Z01
SPACING	24 0"		

Lumber grades designated with 12A use design values approved 1/5/2012 by ALSC

120 mph wind within 17 00 ft from roof edge	15 00 ft mean hgt	ASCE 7-10	CLOSED bldg	not located
psf	BC DL=5 0 psf	Gcpi(+/-)=0 18	RISK CAT 11	EXP C wind TC DL=3 5

wind loads and reactions based on MMFRS with additional C&C member design

SPECIAL LOADS

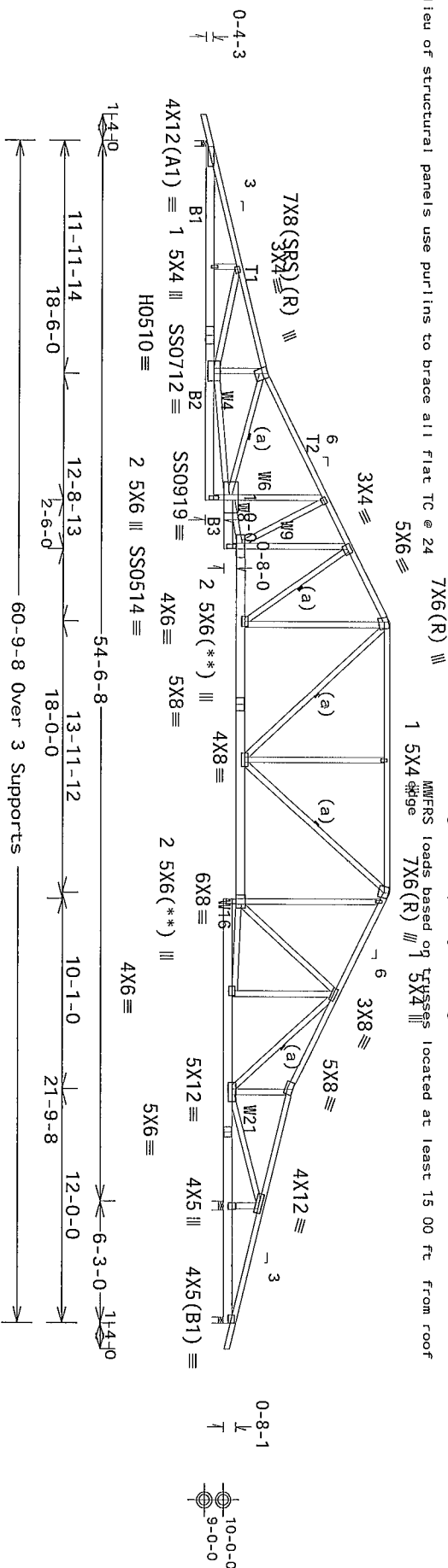
TC	From	DUR	FAC	=	1.33	PLATE	DUR	FAC	=	1.25
TC	From	59	PLF	at	-1	25	59	PLF	at	0
TC	From	54	PLF	at	1	00	54	PLF	at	11
TC	From	56	PLF	at	1	99	56	PLF	at	24
TC	From	54	PLF	at	24	73	54	PLF	at	38
TC	From	56	PLF	at	38	71	56	PLF	at	48
TC	From	54	PLF	at	48	79	54	PLF	at	62
TC	From	59	PLF	at	60	79	59	PLF	at	62
TC	From	20	PLF	at	0	00	20	PLF	at	25
TC	From	60	PLF	at	25	17	60	PLF	at	28
TC	From	60	PLF	at	28	33	60	PLF	at	33
TC	From	60	PLF	at	33	33	60	PLF	at	39
TC	From	20	PLF	at	39	00	20	PLF	at	60
TC	From	200	LB Conc	Load	at	19	17	20	PLF	at

In lieu of structural panels use purlins to brace all flat TC @ 24
OC

$\begin{matrix} \diagdown & \diagup \\ \text{7X6(R)} & \text{7X6(R)} \end{matrix}$

MIN/FIS loads based on trusses located at least 15' 00" from roof edge

$\begin{matrix} \diagdown & \diagup \\ \text{5X6} & \text{5X6} \end{matrix}$



R=2279 U=55 W=3 5" (3 5' min)
RL=190/-179

R=3521 U=79 W=5 5" (5 5" min)
R=-818 R_w=43 U=314 W=3 5" (3 5" min)

Wave	Design Crit	FBC2010Res/TP1-2007 (STD)	FT/RT=10%(0%)/0(0)
PLT TYP. 20 Gauge HS, 18 Gauge HS,			

$$\text{FT/RT} = 10\% (0\%) / 0 (0)$$

QTY:4 FL/-/3/-/-/R/-

Scale = .125"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing, and bracing. Follow the latest edition of BCSI (Building Component Safety) Information on by B71 and W74C. Practices noted otherwise to performing these functions. Installers shall provide temporary bracing. Unbraced members shall have a property attached structural shoring and bracing shall have bracing installed per BCSI section 83.87 or 810 as applicable.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

general notes page ITW BCG www.itwbgc.com TPI www.tpinet.org WTCA www.sbcindustry.com
ICC www.iccsafe.org

08/15/2017

TC LL	20 0 PSF	REF	R487 -- 35507
TC DL	7 0 PSF	DATE	08/15/13
BC DL	10 0 PSF	DRW	HOURS487 13227018
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	314368
DUR FAC	1 25		
SPACING	24.0"	JREF-	1UYS487_Z01

Top chord 2x4 SP 2850f-2 3E T3 T4 2x4 SP #1_12A
Bot chord 2x6 SP #2_12A W4 2x4 SP 2850f-2 3E
W5 W20 2x4 SP #1_12A

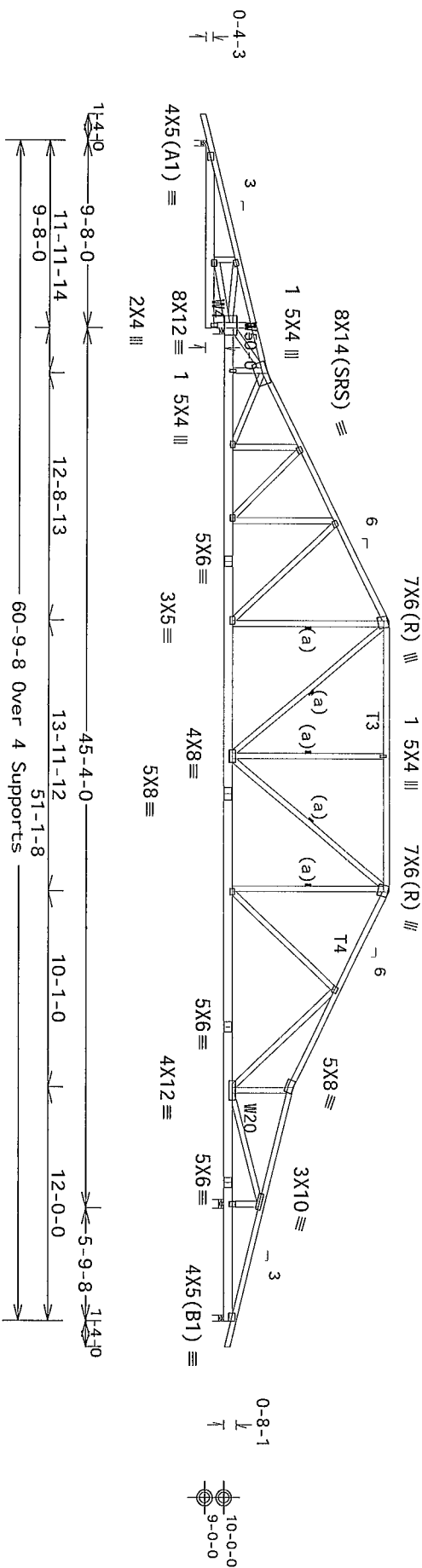
Lumber grades designated with 12A use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction the building designer and the project owner

Special loads

TC- From	54 pif at -1 33 to	54 pif at 11 99
TC- From	56 pif at 11 99 to	56 pif at 24 73
TC- From	54 pif at 24 73 to	56 pif at 38 71
TC- From	56 pif at 38 71 to	54 pif at 48 79
TC- From	54 pif at 48 79 to	4 pif at 62 12
BC- From	20 pif at -1 33 to	20 pif at 0 00
BC- From	20 pif at 0 00 to	120 pif at 9 67
BC- From	120 pif at 9 67 to	20 pif at 19 67
BC- From	20 pif at 19 67 to	20 pif at 21 67
BC- From	20 pif at 21 67 to	20 pif at 33 67
BC- From	20 pif at 33 67 to	20 pif at 45 67
BC- From	20 pif at 45 67 to	4 pif at 62 12

Wind loads and reactions based on MMFRS with additional C&C member design

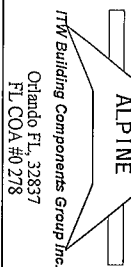


Note All Plates Are 3X4 Except As Shown
R=277 U=69 W=3 5" (3 5' min)
R=190/-179 R=3185 U=196 W=3 5 (3 5 min)

PLT TYP Wave
Design Crit FBC2010Res/TP1-2007(STD)
FTR/RT=10(0%)/0(0)

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!

Trusses require extreme care in fabricating handling shipping and bracing. Follow the latest edition of BCS (Building Component Safety Information) by TPI and WTC. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCS section 89.07 or 81.0 as applicable.



ITW Building Components Group, Inc.
Orlando FL 32837
FL COA #0 278

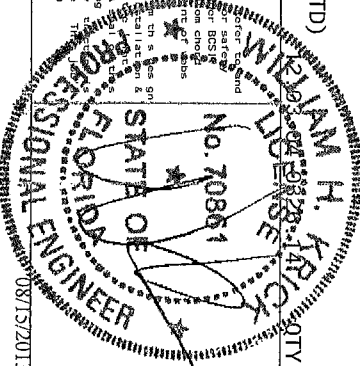
Negative reaction(s) of -223# MAX (See below) from a non-wind load case requires uplift connection
120 mph wind 15 00 ft mean ht ASCE 7-10 CLOSED bldg not located within 17 00 ft from roof edge RISK CAT II EXP C wind TC DL=3 5 psf wind BC DL=5 0 psf GCP(+/-)=0 18

(a) Continuous lateral bracing equally spaced on member
In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Bottom chord checked for 10 00 psf non-concurrent live load
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

WARNING Furnish a copy of this DWG to the installation contractor Failure to follow provisions of BCS in handling and installation of trusses can result in serious injuries Do not permit inexperienced and untrained people to install trusses See WARNING note below BCS recommends retaining a registered professional engineer for the design of temporary bracing

MMFRS loads based on trusses located at least 15 00 ft from roof edge



QTY. 1	FL/-/3/-/-/R/-	Scale = .125"/Ft.
TC LL	20 0 PSF	REF R487-- 35508
TC DL	7 0 PSF	DATE 08/15/13
BC DL	10 0 PSF	DRW HCUS487 1322/017
BC LL	0 0 PSF	HC-ENG WHK/WHK
TOT LD	37 0 PSF	SEON- 314439
DUR FAC	1 25	
SPACING	24.0"	JREF- 1UVS487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Webbs 2x4 SP_#3_12A W5, W18 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 11/5/2012 by ALSC

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 in

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DMG to the installation contractor. Failure to follow provisions of BSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and untrained people to install trusses. See **WARNING** note below BSI) recommends retaining a registered professional engineer for the design of temporary bracing.

Negative reaction(s) of -272# MAX (See below) from a non-wind load case requires uplift connection

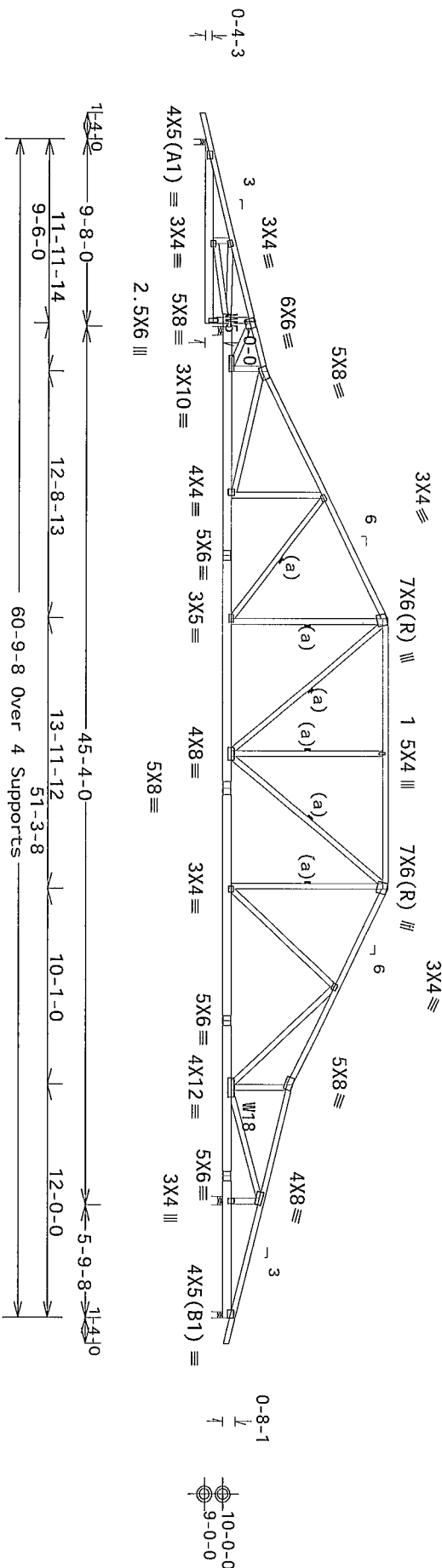
120 mph wind, 15 00 ft mean ht, ASCE 7-10, CLOSED bldg, not located
within 17 00 ft from roof edge, RISK CAT 11, EXP C, wind TC DL=3 5
psf, wind BC DL=5 0 psf GcP(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss passed check for 20 psf additional bottom chord live load in areas with 42'-high x 24'-wide clearance

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

MMFRS loads based on trusses located at least 15 00 ft from roof edge



R=278 U=69 W=35 (35" min)
RL=190/-179

R=3286 U=55 W=5 5 (5 5' min)

R=2608 U=57 W=5 5' (5 5" min)
R=121/-272 U=80 W=3 5' (3 5" min)

PLT TYP. Wave

Design Crit	FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)	

QTY 2 FL/-/3/-/-/R/-

Scale = .125"/Ft

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

WILLIAM H. KRICK
DECEMBER 1963

08/15/2013

TC LL	20 0 PSF	REF R487-- 35509
TC DL	7 0 PSF	DATE 08/14/13
BC DL	10 0 PSF	DRW HCURS487 13226009
BC LL	0.0 PSF	HC-ENG JB/WMPF
TOT LD.	37 0 PSF	SEQN- 314061
DUR FAC	1 25	
SPACING	24 0"	JREF- 1UYS487_Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x6 SP_#2_12A
Webs 2x4 SP_#3_12A
Stack chord SC1 2x4 SP_#1_12A
Stack chord SC2 2x4 SP_#1_12A

See DWGS A12015ENC100212 GBLLETIN0212, & GABRST100212 for more requirements

requirements

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

(**) 3 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements

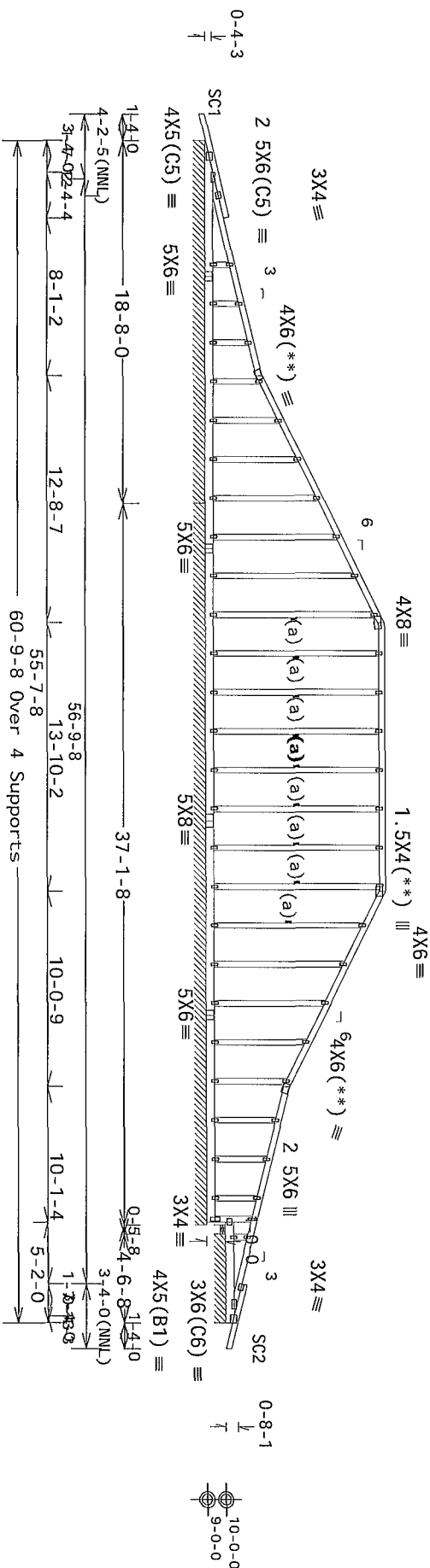
120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg Located anywhere in roof RISK CAT II EXP C wind TC DL=3 5 psf wind BC DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Stacked top chord must NOT be notched or cut in area (NNL).
Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24 o c Center plate on stacked/dropped chord interface plate length perpendicular to chord length Splice top chord in notchable area using 3x6

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this **DMG** to the installation contractor. Failure to follow provisions of **BCS1** in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and untrained people to install trusses. See **WARNING** note below **BCS1** recommends retaining a registered professional engineer for the design of temporary bracing.



R=81 PLF U=15 PLF W=18-8-0
RL=10/-10 PLF

R=72 PLF U=11 PLF W=37-1-8

R=-19 R_w=10 U=16 W=5 5' (5 5 min)
R=114 PLF U=19 PLF W=4-6-8

Note All Plates Are 1 5X4 Except As Shown

Design Crit FBC2010Res/TP1-2007(STD)

PLT TYP. Wave

$$FT/RT=10\%(0\%)/0(0)$$

QTY 1 FL/-/3/-/-/R/-

Scale = .125"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trespasser report rectification ensure no fabric cutting, hanging up, spilling, ironing and bracing. Follow the latest version of BS581 (Building Component Safety Information by TPI and WTA) practices prior to performing any these final ones. Installers shall provide temporary bracing for BS581. Unless noted otherwise, top chord shall have properly attached structural sheath and boom chord shall have a properly attached rigid end cap. Load zones shown for permanent lateral restraint shall have bracing installed per BS581 section B3, B7 or B10 as appropriate.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

the responsibility of the building designer per ANSI/AP1 Sec 2
general notes page 17W-B06 www twbco.com TPI www tpinst.org WTCA www sbcindustry.com
ICC www iccsafe.org

TC LL	20 0 PSF	REF R487-- 35510
TC DL	7 0 PSF	DATE 08/14/13
BC DL	10 0 PSF	DRW HCUR487 13226019
BC LL	0 0 PSF	HC-ENG JB/MPF
TOT. LD.	37 0 PSF	SEON- 314062
DUR. FAC.	1.25	
SPACING	24 0"	JREF- 1UYS487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with 12A use design values approved 1/5/2012 by ALSC

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Deflection meets L/240 live and L/180 total load Creep increases factor for dead load is 1.50

120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg Located anywhere in roof RISK CAT II Exp C wind TC DL=3.5 psf wind BC DL=5.0 psf GCP(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

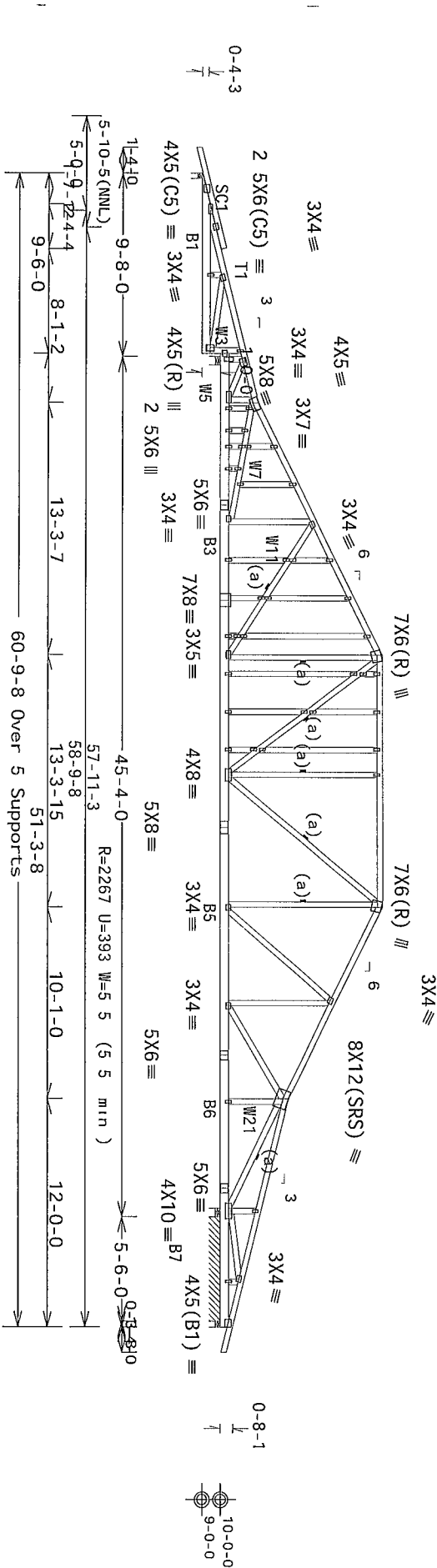
See DWGS A12015ENC100212 6BLLETIN0212 & GABRST100212 for more requirements

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 0C

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries Do not permit inexperienced and untrained people to install trusses See **WARNING** note below BCSI recommends retaining a registered professional engineer for the design of temporary bracing



R=400 U=58 W=3 5" (3 5' min)
RL=191/-181 R=2104 U=341 W=5 5 (5 5' min)

R=152 U=50 W=3 5" (3 5" min)
R=12/-48 PLF U=12 PLF W=5-6-0

Note All Plates Are 1.5X4 Except As Shown

Design Crit FBC2010Res/TP1-2007(STD)

PLT TYP Wave

$$FT/RT=10\%(0\%)/0(0)$$

QTY.1 FL/-/3/-/-/R/-

Scale = .125"/Ft

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

W
D
M

TC LL	20 0 F
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REF	R487--	3551
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Trusses require extreme care in fabricating, handling, shipping and bracing. For more information, contact the American Institute of Steel Construction, Inc. (AISC), 900 North Dearborn Street, Chicago, IL 60610. For the latest edition of BSI (Building Component Safety Information) by TPI and WTA (The Safety of Trusses), contact the Building Component Safety Institute, 1000 North Dearborn Street, Chicago, IL 60610.

for to end
safety

TC DL 7 0 F

DATE 08/14/13

shall have a properly attached end chord. The top chord shall provide temporary bracing and bottom chords shall have a properly attached end chord. The top chord shall provide temporary bracing and bottom chords shall have a properly attached end chord.

Country	1950	1960	1970	1980	1990	2000	2010	2020	2030	2040	2050
Japan	7.0	7.5	8.0	8.5	9.0	9.5	10.0	10.5	11.0	11.5	12.0
Germany	10.0	10.5	11.0	11.5	12.0	12.5	13.0	13.5	14.0	14.5	15.0
France	11.0	11.5	12.0	12.5	13.0	13.5	14.0	14.5	15.0	15.5	16.0
Italy	12.0	12.5	13.0	13.5	14.0	14.5	15.0	15.5	16.0	16.5	17.0
Spain	13.0	13.5	14.0	14.5	15.0	15.5	16.0	16.5	17.0	17.5	18.0
Sweden	14.0	14.5	15.0	15.5	16.0	16.5	17.0	17.5	18.0	18.5	19.0
Belgium	15.0	15.5	16.0	16.5	17.0	17.5	18.0	18.5	19.0	19.5	20.0
United Kingdom	16.0	16.5	17.0	17.5	18.0	18.5	19.0	19.5	20.0	20.5	21.0
Canada	17.0	17.5	18.0	18.5	19.0	19.5	20.0	20.5	21.0	21.5	22.0
USA	18.0	18.5	19.0	19.5	20.0	20.5	21.0	21.5	22.0	22.5	23.0
China	19.0	19.5	20.0	20.5	21.0	21.5	22.0	22.5	23.0	23.5	24.0
India	20.0	20.5	21.0	21.5	22.0	22.5	23.0	23.5	24.0	24.5	25.0
South Africa	21.0	21.5	22.0	22.5	23.0	23.5	24.0	24.5	25.0	25.5	26.0
South America	22.0	22.5	23.0	23.5	24.0	24.5	25.0	25.5	26.0	26.5	27.0
Sub-Saharan Africa	23.0	23.5	24.0	24.5	25.0	25.5	26.0	26.5	27.0	27.5	28.0
North Africa	24.0	24.5	25.0	25.5	26.0	26.5	27.0	27.5	28.0	28.5	29.0
Middle East	25.0	25.5	26.0	26.5	27.0	27.5	28.0	28.5	29.0	29.5	30.0
Asia	26.0	26.5	27.0	27.5	28.0	28.5	29.0	29.5	30.0	30.5	31.0
Europe	27.0	27.5	28.0	28.5	29.0	29.5	30.0	30.5	31.0	31.5	32.0
World	28.0	28.5	29.0	29.5	30.0	30.5	31.0	31.5	32.0	32.5	33.0

BC DL 10 0 F

DRW HCUSR487 132260

ITW Bu. had no Componente Ground Inc. (THURCO) shall not be removed by the first day after the date of the

BC 11 00 E

HC-FENG IR/M/PF

Apply plates to each face of truss and post it on as shown above and on the joint.

STATISTICS

TOT ID 37 0 E

311063 SEEN

Details unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A section drawing on cover page stating that a drawing indicates acceptance of professional engineering. The date and use of this drawing is indicated.

3000

101 ED	21.01
DIP EAC	1.25

014003

general notes page 17W BCG www.17wbog.com 7P1 www.7p1inst.org WTCA www.sbcindustry.com

ESCAPE

DOI: 10.1002/anie.201100011

1100 4150 407 70

ICC www.iccnate.org

08/1

SPACING 24.0"

JREF- 10YS481_20

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

(a) Continuous lateral bracing equally spaced on member

in lieu of structural panels use purlins to brace all flat TC @ 24 in OC

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Negative reaction(s) of -267 # MAXX (See below) from a non-wind load case requires uplift connection

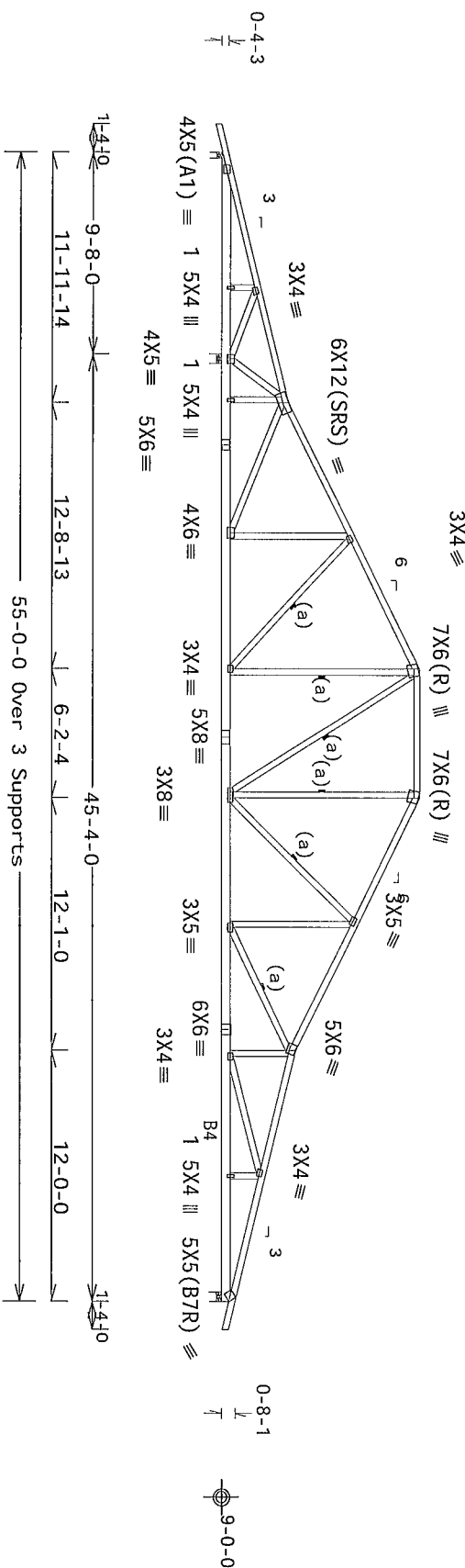
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6 50 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCP(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Truss passed check for 20 psf additional bottom chord live load in areas with 42 -high x 24 -wide clearance

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MMFRS loads based on trusses located at least 7 50 ft from roof edge



R=1791 U=291 W=5 5" (5 5" min)

PLT TYP. Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)
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QTY.3 FL/-/3/-/-/R/-

Scale = .125"/Ft.

****IMPORTANT****
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

These requirements are in *fact* not handling shipping, installing and bracing of structural steel members. The contractor is to follow the latest edition of BECS (Building Construction Survey Information on by TPI and WTC) practices prior to performing these functions. Installers shall provide temporary bracing for GSI's unless noted otherwise as top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached "r" of ceiling. Locations shown for permanent lateral bracing shall have installed per GSI sections 33, B7 or B10 as apply cable

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

ICC www.iccscate.org

TC LL	20.0 PSF	REF	R487-- 35513
TC DL	7.0 PSF	DATE	08/14/13
BC DL	10.0 PSF	DRW	H05R487 13225001
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT. LD.	37.0 PSF	SEQN-	314083
DUR FAC.	1 25	FROM	CVB
SPACING	24.0"	JREF-	1UYS487 Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 in.

Calculated vertical deflection is 0.67" due to live load and 0.87 due to dead load at X = 18'-2.9"

MMFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6 50 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3 5 psf, wind BC DL=5 0 psf 6Cp1(+/-)=0 18

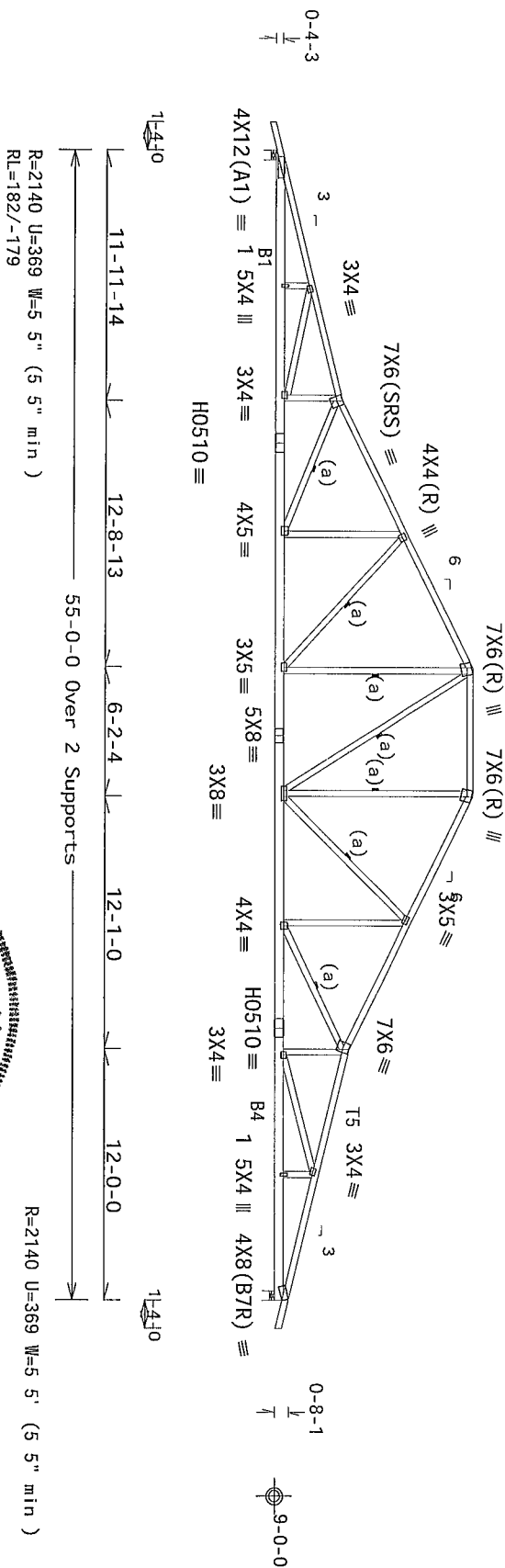
Wind loads and reactions based on MNFRS with additional C&C member design

Calculated horizontal deflection is 0 14 due to live load and 0 18" due to dead load

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses See "WARNING" note below



PLT TYP. 20 Gauge HS, Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)

1 FL/-/3/-/-/R/- Scale = .125"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require a minimum grade of fabric cladding, sheathing and bracing. Follow the latest edition of BCS (but if no Compartment Safety Information by TP1 and WTA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCS. Unless noted otherwise, top chord shall have properly attached structural sheathing and bracing. Trusses shall have a properly attached rafter or king location system for permanent lateral restraint. Trusses shall have bracing installed per BCS section 83.07 or 810 as applicable.

ALPINE

ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0 278

general notices page 11-W-DCU www
icc www iccsafe.org twocj.com ipi www cpinst.org WUA www socindustry.com

08/15/2014

TC LL	20 0 PSF	REF	R487-- 35514
IC DL	7 0 PSF	DATE	08/14/13
BC DL	10 0 PSF	DRW	HGUSR487 1322606
BC LL	0 0 PSF	HC-ENG	JB/WMP
TOT LD	37 0 PSF	SEQN-	314066
DUR FAC.	1 25	FROM	CVB
SPACING	24 0"	JREF-	1UYS487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI(+/-)=0 18

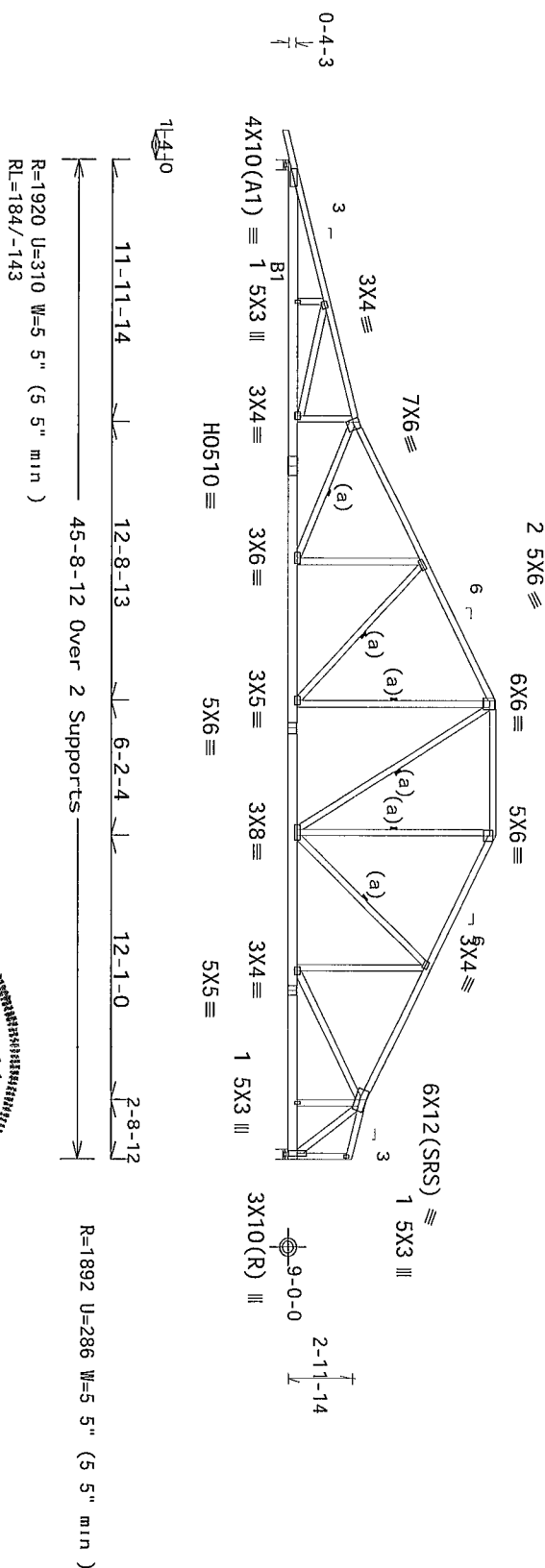
Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Truss passed check for 20 psf additional bottom chord live load in areas with 42'-high x 24'-wide clearance

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit FBC2010Res/TP1-2007(STD)

$$\text{FT/RT} = 10\%(0\%) / 0(0)$$

2 FL/-/3/-/-/R/-

Scale = .125"/Ft.

ALPINE

ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0278

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!**
****IMPORTANT** FURNISH THIS DECISION TO ALL CONTRACTORS INCLUDING INSTALLERS**

Trussess require extreme care in their catting, handling, lifting, moving and bracing. Refer to the following the latest edition of the BCSI (Building Component Safety) Information on (by TPI and WITCA) practices and/or to performing these functions. Installers shall provide temporary bracing per BCSI Unitss and must otherwise as top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI Section B3, B7 or B10 as appli cable.

17W Building Components Group Inc. (17WBCG) shall not be responsible for any damage to or from the design, detailing, manufacturing or installation of the trussess. Apply the following to the BCSI (Building Component Safety) Information on (by TPI and WITCA) details and cover pages. Refer to draw ngs 160A-2 for standard detailss and to the AISC (American Institute of Steel Construction) Manual for details and cover pages. Refer to draw ngs 160A-2 for standard detailss and to the AISC (American Institute of Steel Construction) Manual for details and cover pages. Refer to draw ngs 160A-2 for standard detailss and to the AISC (American Institute of Steel Construction) Manual for details and cover pages.

responsibility for solely for the design and construction of the trussess. The trussess shall be designed and constructed in accordance with the AISC (American Institute of Steel Construction) Manual for details and cover pages. Refer to draw ngs 160A-2 for standard detailss and to the AISC (American Institute of Steel Construction) Manual for details and cover pages.

general notice page 17W-BG5 www.17wbcg.com TPI www.tpi.com WITCA www.stcindustry.com

08/15/2013

TC LL	20 0 PSF	REF R487-- 35515
TC DL	7 0 PSF	DATE 08/14/13
BC DL	10.0 PSF	DRW HOURS487 13226011
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT LD	37 0 PSF	SEQN- 314067
DUR FAC	1 25	FROM CVB
SPACING	24.0"	JREF- 1UYS487 Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP C, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Right end vertical not exposed to wind pressure

(a) Continuous lateral bracing equally spaced on member

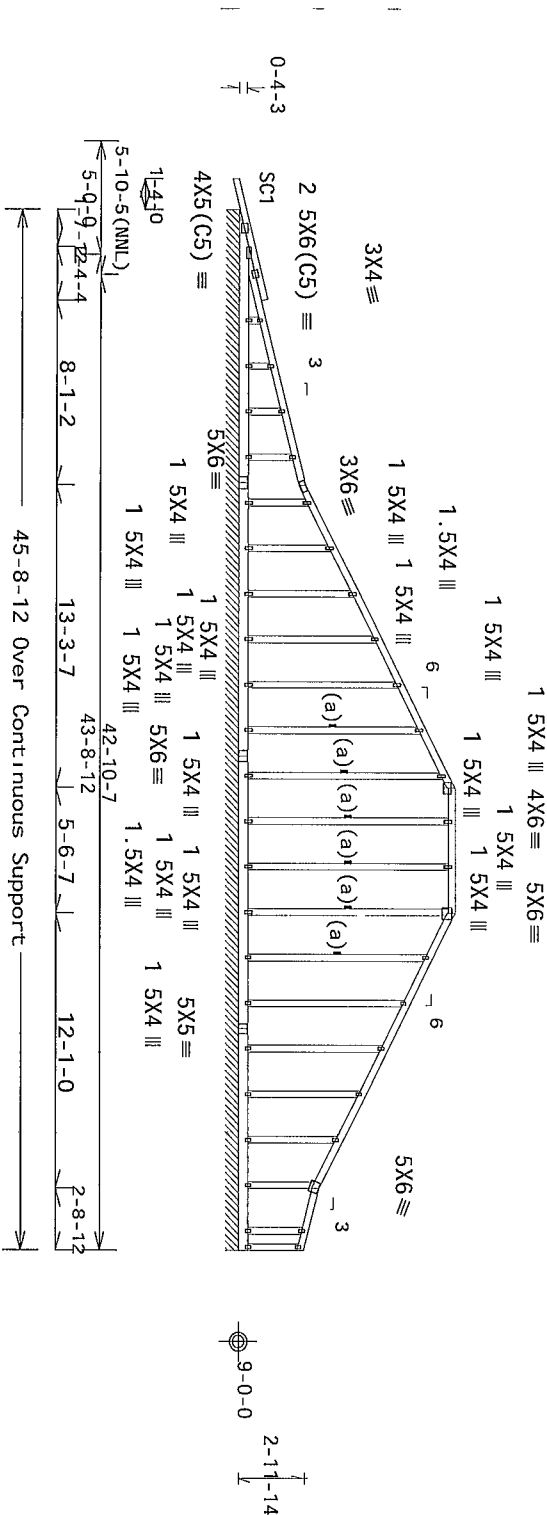
In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets $L/240$ live and $L/180$ total load Creep increases

Factor for dead load is 1 50

Gable end supports 8" max rake overhang



R=77 PLF U=12 PLF W=45-8-12
RL=4/-3 PLF

Note All Plates Are 1.5X3 Except As Shown

PLT TYP. Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)

FL/-/3/-/-/R/-

Scale = .125"/Ft.

ALPINE

ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0278

[illegible]

2009/1 = 2001

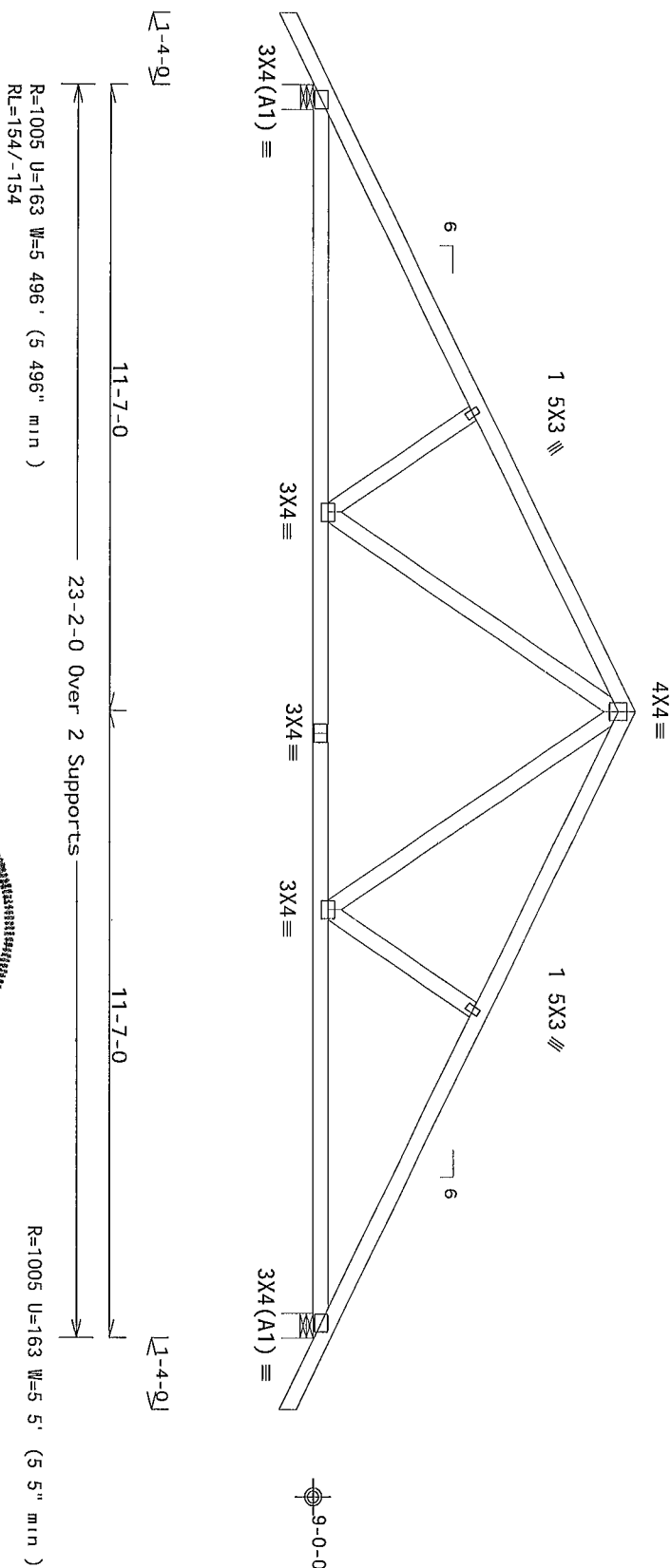
TC LL	20 0 PSF	REF	R487-- 35516
TC DL	7 0 PSF	DATE	08/14/13
BC DL	10 0 PSF	DRW	H05R487 13226017
BC LL	0 0 PSF	HC-ENG	JB/MPP
TOT LD	37 0 PSF	SEQN-	314068
DUR FAC	1 25		
SPACING	24.0"	JREF-	1UYS487 Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP C, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MNFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load
Deflection meets $L/240$ live and $L/180$ total load Creep increase
factor for dead load is 1 50



Design Crit	FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)	

$$FT/RT=10\%(0\%)/0(0)$$

5 FL/-/3/-/-/R/-/

Scale = .3125"/Ft

ALPINE

ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0278

[illegible]

Professional Engineer Seal for the State of Florida, No. 70861. The seal features a star emblem and the text "STATE OF FLORIDA" and "PROFESSIONAL ENGINEER".

TC LL	20 0 PSF	REF R487-- 35517
TC DL	7 0 PSF	DATE 08/14/13
BC DL	10 0 PSF	DRW HCURSR487 1322601
BC LL	0.0 PSF	HC-ENG JB/M/PF
TOT LD	37 0 PSF	SEON- 314069
DUR. FAC	1 25	
SPACING	24 0"	JREF- 1UYS487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

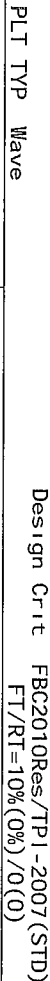
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT 11, EXP C wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpl(+/-)=0 18

Wind loads and reactions based on MNFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets $L/240$ live and $L/180$ total load Creep increases

Factor for dead load is 1 50


$$\text{FT/RT} = 10\%(0\%) / 0(0)$$

6 FL/-/3/-/-/R/-

Scale = .3125"/Ft.



ALPINE

ITTW Building Components Group Inc.

Orlando FL, 32837
FL COA #0 278

Orlando FL, 32837
FL COA #0278

[illegible]

WILLIAM H. KRICHEDOLF
 No. 70861
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 08/15/2011

08/15/2018

TC LL	20 0 PSF	REF R487-- 35518
IC DL	7 0 PSF	DATE 08/14/13
BC DL	10 0 PSF	DRW HCUSR487 13226004
BC LL	0 0 PSF	HC-ENG JB/WPF
TOT LD	37 0 PSF	SEQN- 314070
DUR FAC	1 25	
SPACING	24.0"	JREF- 1UYS487_Z01

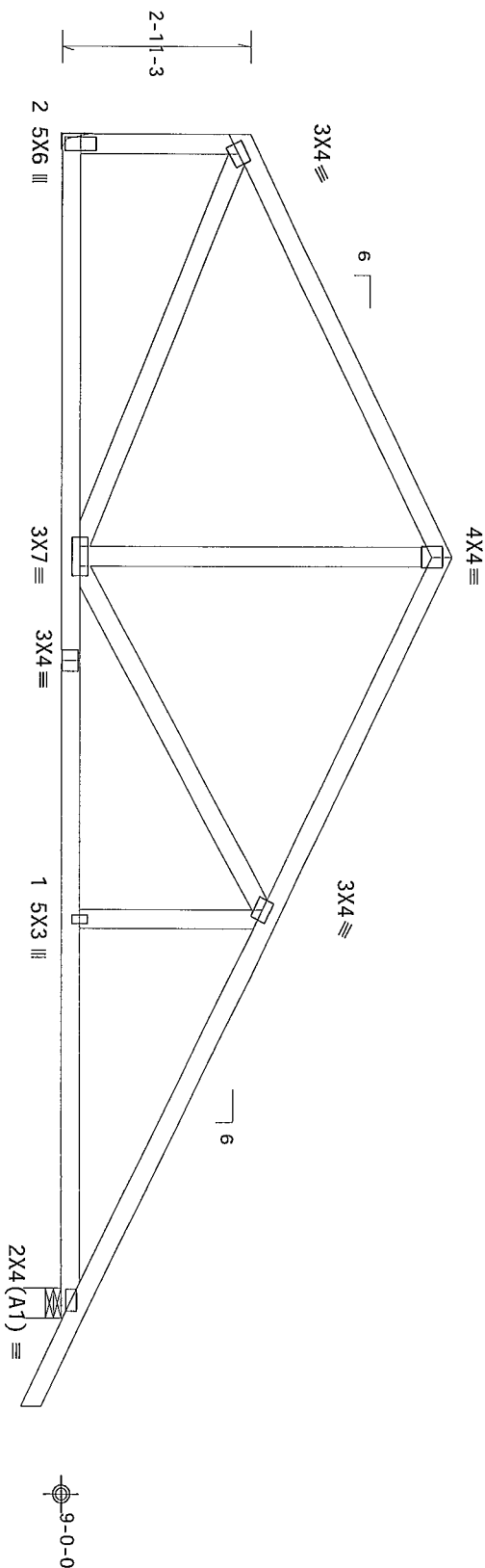
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP C, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50



R=770 U=128 W=5 5

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)

$$FT/RT=10\%(0\%)/0(0)$$

1 FL/-/3/-/-/R/-

Scale = 375"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0 278

****IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS**

Trusses require extreme care in fabricating, handling, shipping, storing, installing, and bracing. Follow the latest edition of BCSP (Building Components Safety Institute) info. de TPI and WTCA practices prior to performing these functions. Installers shall provide temporary bracing and bracing noted otherwise. Do not exceed shall have properly attached structural sheath ng and bracing shall have bracing installed per BCSP sections 83, 87 or 810 as applicable.

1TW Building Components Group Inc. (TWBCG) shall not be responsible for any deviation from the design of the trusses in accordance with the ANSI/TPI 1-2010 for the installation of the trusses. Apply plates to each face of truss and posit on as shown above and on. Do not use unnoted otherwise. Refer to draw ng 160A-Z for standard plate positions. A drawing of cover plate 1 at the end of a drawing and notes describing for proper install ng near the end of the truss. The drawings of the trusses shall be used in accordance with the drawings of the trusses b y the end of the Bu id ng Doc per ANSI/TPI 1-2010 Sec 2. For more information see the general notes page 1TW BCG www twbcg com TPI www tpinet org WTCA www stic industry com general notes page 1TW BCG www twbcg com TPI www tpinet org WTCA www stic industry com

Professional Engineer Seal for the State of Florida, No. 70861, dated 08/15/01.

08/15/2011

TC LL	20 0 PSF	REF	R487-- 35519
IC DL	7.0 PSF	DATE	08/14/13
BC DL	10.0 PSF	DRW	HCUR487 1322015
BC LL	0.0 PSF	HC-ENG	JB/MPP
TOT LD.	37 0 PSF	SEQN-	314071
DUR FAC	1 25		
SPACING	24.0"	JREF-	1UYS487_Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

Wind loads and reactions based on MMFRS with additional C&C member design

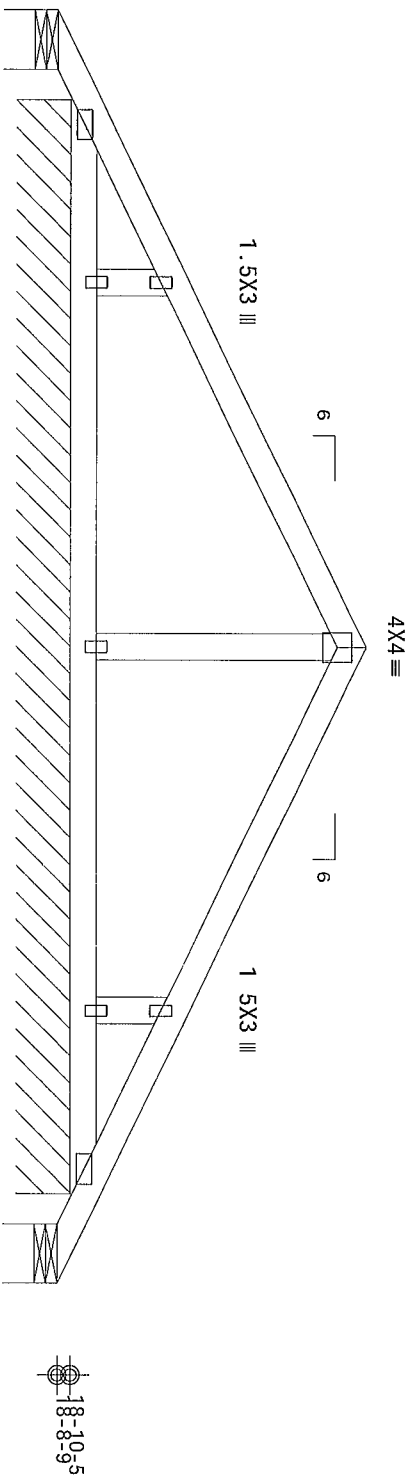
In lieu of rigid ceiling use purlins to brace BC @ 24 OC

Refer to DWG PB160100212 for piggback details

Special loads
-----Lumber
TC- From Dur Fac =1 25 / Plate Dur Fac =1 25
56 pif at 0 00 to 56 pif at 6 99
TC- From 56 pif at 6 99 to 56 pif at 13 98
BC- From 4 pif at 0 00 to 4 pif at 13 98

120 mph wind, 20 46 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT 11, EXP C, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpl (+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=26 Rw=41 U=28 W=7 826" (7 826" min)
RL=72/-72
R=62 PLF U=25 PLF W=12-0-2
R=26 U=11 W=7 826" (7 826 min)
RL=72/-72

PLT TYP. Wave

Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

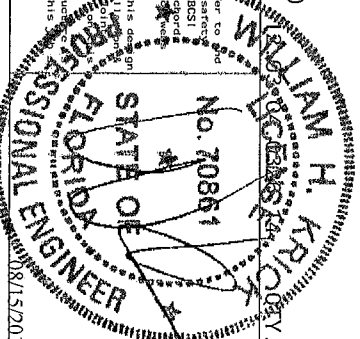
WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

IMPORTANT Trusses require extreme care in fabricating, handling, installing and bracing. Follow the latest edition of BCSI (Building Component Safety Information by TP1 and WTCA) for practices prior to performing these functions. Installers shall provide temporary bracing and bolting unless noted otherwise. Top chord shall have properly attached structural sheathing and bolting shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI section 83 B7 or B10 as applicable.

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design. Any failure to build in conformance with ANSI/TP1 1 or for handling, shipping and on site installation shall be the responsibility of the contractor. ITWBCG shall not be responsible for any deviation from this design. Any failure to build in conformance with ANSI/TP1 1 or for handling, shipping and on site installation shall be the responsibility of the contractor. ITWBCG shall not be responsible for any deviation from this design. Any failure to build in conformance with ANSI/TP1 1 or for handling, shipping and on site installation shall be the responsibility of the contractor.

ALPINE

ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0278



TC LL	20.0 PSF	REF R487-- 35520
TC DL	7.0 PSF	DATE 08/14/13
BC DL	10.0 PSF	DRW HCUR487 13226010
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT LD.	37.0 PSF	SEQN- 314072
DUR FAC	1.25	
SPACING	24.0"	JREF- 1UYS487_Z01

Scale = .5"/Ft.

Top chord 2x4 SP #1 12A
Bot chord 2x4 SP #1 12A
Webs 2x4 SP #3 12A

Lumber grades designated with 12A" use design values approved 1/5/2012 by ALSC

Wind loads and reactions based on MMFERS with additional C&C member design

Gable end supports 8" max rake overhang

In lieu of rigid ceiling use purlins to brace BC @ 24 OC

Refer to DWG PB160100212 for piggyback details

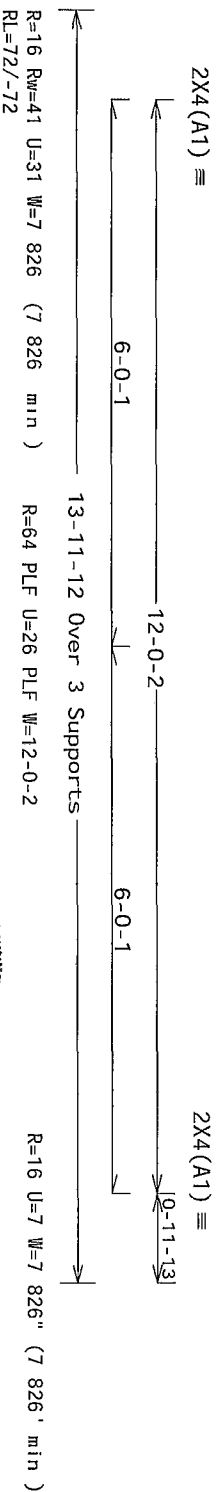
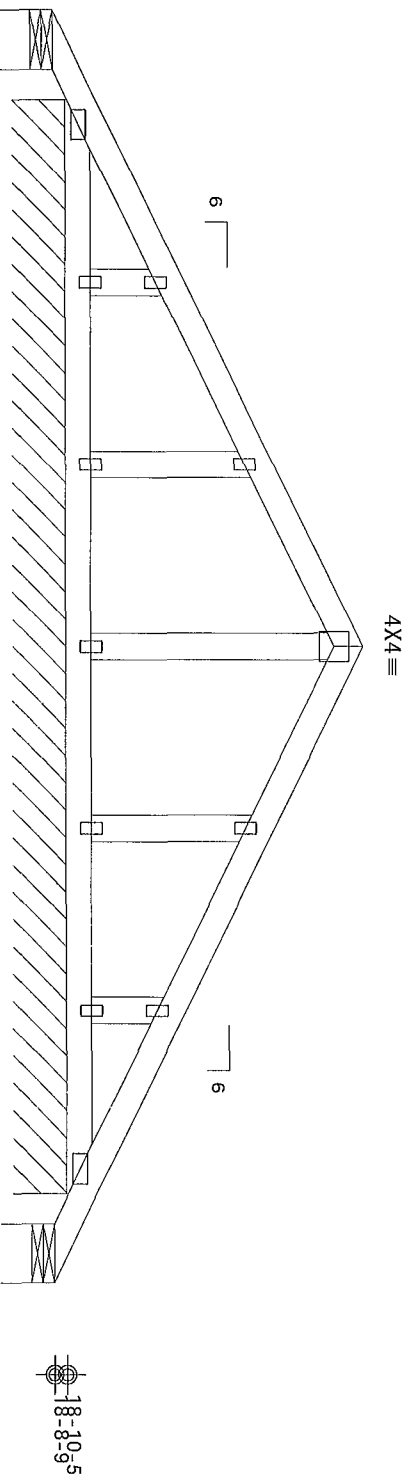
Special loads

Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 pif at 0 00 to 56 pif at 6 99
TC- From 56 pif at 6 99 to 56 pif at 13 98
BC- From 4 pif at 0 00 to 4 pif at 13 98

120 mph wind, 20 46 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3 5 psf, wind BC DL=2 0 psf 6Cpl(+/-)=0 18

See DWGS A12030ENC100212, GBLLET11N0212, & GABRST100212 for more requirements

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Note All Plates Are 1 5X3 Except As Shown
Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

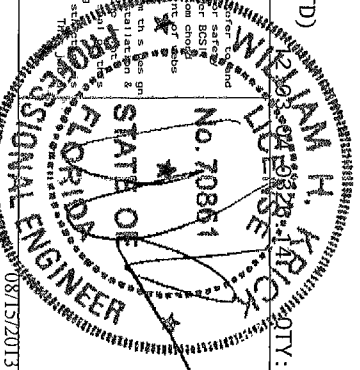
PLT TYP. Wave
Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0 278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating handling shipping and installing. Follow the latest edition of BCS (Building Component Safety Information by TPI and WFO) practices prior to performing these functions. Installers shall provide temporary bracing for BCS. Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCS sections B3 B7 or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design or any modification to the design. The user of this design shall be responsible for obtaining all necessary permits and for ensuring that the design is used in accordance with the design. Refer to drawings 180A-2 for standard plate positions. A drawing or cover page stating this design and the suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TP1 1 Sec 2. For more information see the general notes page ITW-BCG www.itwbcg.com www.tpi.net.org WFOA www.wfoa.org industry.com IBC www.icba.org



TC LL	20 0 PSF	REF R487-- 35521
TC DL	7.0 PSF	DATE 08/14/13
BC DL	10.0 PSF	DRW HCUSR487 13226002
BC LL	0 0 PSF	HC-ENG JB/MFP
TOT LD.	37 0 PSF	SEON- 314073
DUR FAC	1.25	
SPACING	24.0"	JREF- 1UVS487_Z01

08/15/2013

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

Wind loads and reactions based on MMFRS with additional C&C member design

In lieu of rigid ceiling use purlins to brace BC @ 24' 00"

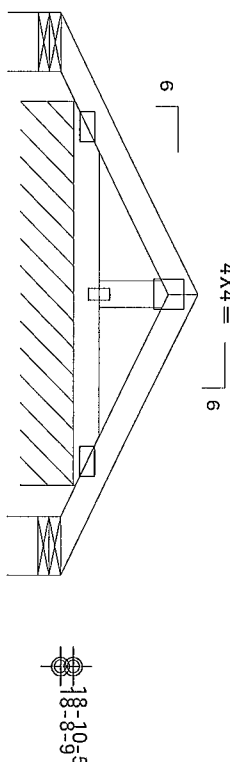
Refer to DWG PB160100212 for piggyback details

Special loads

Lumber Dur Fac = 1 25 / Plate Dur Fac = 1 25
TC-From 56 pif at 0 00 to 56 pif at 3 09
TC-From 56 pif at 3 09 to 56 pif at 6 19
BC-From 4 pif at 0 00 to 4 pif at 6 19

120 mph wind, 19 49 ft mean ht, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP C, wind TC DL=3 5 psf, wind BC
DL=2 0 psf Gcpl(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase
Factor for dead load is 1.50


$$2X_4(A_1) \equiv 1 \quad 5X_3 \equiv 2X_4(A_1) \equiv$$

4-2-10 2-1-5 2-1-5 0-1-13

6-2-4 Over 3 Supports $\xrightarrow{\quad R=13 \text{ U}=6 \text{ W}=7 \text{ 826"} \quad} (7 \text{ 826"} \text{ min})$

R=13 U=13 W=7 826" (7 826" min)
RL=-30 R=72 PLF U=28 PLF W=4-2-10

PLT TYP. Wave	Design of the POLYMER, 11-200 (CIB)	FT/RT=10%(0%)/0(0)
RANDOMLINE PFA AND COLLOR ALL NORMED ON WAVE AVERAGE		

ALPINE

ITW Building Components Group Inc
Orlando FL, 32837
FL COA #0278

**** IMPORTANT ****

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Tenors required to ensure care in building Component Safety Info on mg and bracing and other safety information follow the latest edition of BS6187 (Building Component Safety Information) by TPI and WDA.

Practitioners prior to performing these functions installers shall provide temporary bracing for BS6187 components and shall ensure that all components are adequately braced and secured during installation.

Installers shall have a properly attached 0.6 d or 1 mg lanyard shown for each fall arrest point. Lanyards shall have bracing installed per BSCL sections B3, B7 or B10 as applicable.

TW Build mg Components Group Inc (TWBCG) shall not be responsible for any delay related to this assignment or any failure to build the trusses in conformance with ANSI/TPI-1 or for handling, shipping, installation & bracing of trusses. Apply plates to each face of trusses and post it on as shown above and on back of trusses.

[illegible]

QTY. 11	FL/-/3/-/-/R/-	Scale = .5"/Ft
---------	----------------	----------------

TC LL	20.0 PSF	REF	R487-- 35522
TC DL	7 0 PSF	DATE	08/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13226003
BC LL	0.0 PSF	HC-ENG	JB/M/PF
TOT LD.	37 0 PSF	SEQN-	314074
DUR FAC	1.25		
SPACING	24.0"	JREF-	1UWS487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Webs 2x4 SP_#3_12A

Special loads	Dur	Fac = 1	25 /	Plate	Dur	Fac = 1	25)
-----Lumber							
TC- From	56	pif at	0 00 to	56	pif at	3 09	
TC- From	56	pif at	3 09 to	56	pif at	6 19	
BC- From	4	pif at	0 00 to	4	pif at	6 19	

120 mph wind 19 49 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP C, wind TC DL=3 5 psf, wind BC
DL=2 0 psf GCp1 (+/-)=0 18

See DWGS A12030ENC100212, GBLLET1N0212, & GABRST100212 for more requirements

Deflection meets $L/240$ live and $L/180$ total load Creep increases factor for dead load is 1.50

Deflection meets $L/240$ live factor for dead load is 1.50

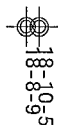


Diagram illustrating the decomposition of the interval $[0, 1]$ into subintervals. The interval is divided into four parts with lengths $2-1-5$, $4-2-10$, $2-1-5$, and $0-1-13$. The first three parts are marked with single arrows, and the last part is marked with a double arrow.

R=13 U=6 W=7 826 (7 826 min)

R=13 U=6 W=7 826 (7 826 min)

Design Crit FBC2010Res/TP1-2007(STD)

****WARNING****
READ AND FOLLOW ALL NOTES ON THIS SHEET

****IMPORTANT****
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Orlando FL, 32837
FL COA #0278

[illegible]

08-15-2013

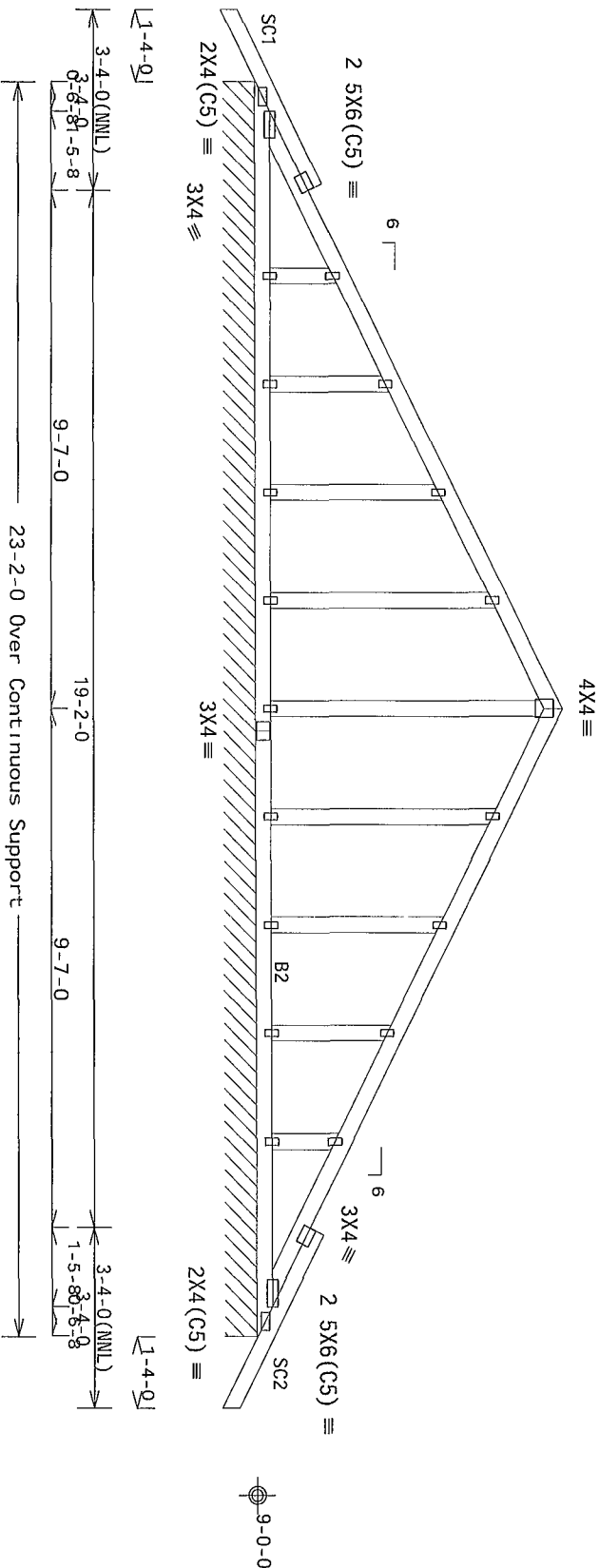
FL/-3/-/-/R/-		Scale = 5"/Ft.	
TC LL	20 0 PSF	REF	R487-- 35523
TC DL	7 0 PSF	DATE	08/14/13
BC DL	10 0 PSF	DRW	HCSR487 13226012
BC LL	0 0 PSF	HC-ENG	JB/WMPF
TOT LD	37 0 PSF	SEQN-	314075
DUR FAC	1 25		
SPACING	24.0"	JREF-	1UYS487_Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_2850F-2 3E B2 2x4 SP M-30
Webs 2x4 SP_#3_12A
Stack Chord SC1 2x4 SP_#1_12A Stack Chord SC2 2x4 SP_#1_12A
Lumber grades designated with 12A' use design values approved
1/5/2012 by ALSC

Stacked top chord must NOT be notched or cut in area (NML) Dropped
top chord braced at 24" o c intervals Attach stacked top chord (SC)
to dropped top chord in notched area using 3x4 tie-plates 24" o c
Center plate on stacked/dropped chord interface, plate length
perpendicular to chord length Splice top chord in notched area
using 3x6

Gable end supports 8" max rake overhang

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC
DL=5.0 psf GCP(+/-)=0.18
Wind loads and reactions based on MMFRS with additional C&C member
design
See DWGS A12015ENC100212, 6BLLET1N0212, & GABRST100212 for more
requirements
Bottom chord checked for 10.00 psf non-concurrent live load
Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1.50



R=83 PLF U=14 PLF W=23-2-0
RL=7/-7 PLF

Note All Plates Are 1 5X3 Except As Shown

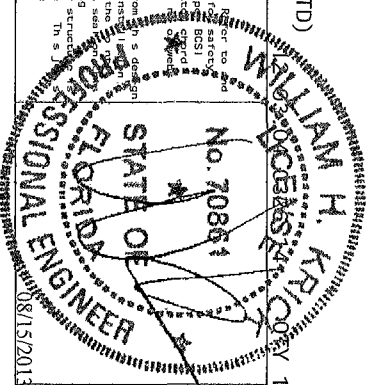
PLT TYP Wave Design Cr-t FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

ALPINE

ITW Building Components Group Inc.

Orlando FL 32837
FL COA #0278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping and bracing. Follow the latest edition of BCSI (Building Components Safety Information) by TPI and WFOA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r g d c e l i n g. Locate one shown for permanent lateral restraint. Shall have bracing installed per BCSI section 83.87 or 810 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design of any future building. Apply plates to each face of truss and position as shown above and on the drawings. Trusses shall be braced in accordance with the design of the truss and on the drawings. The design of the truss shall be the responsibility of the building designer per ANSI/TPI 1 Sec 2. For more information on the general notes page ITW-BCG www.itwbcg.com TPI www.tpi.net WFOA www.wfoa.com
ITC www.icra.org



TC LL	20.0 PSF	REF R487-- 35524
IC DL	7.0 PSF	DATE 08/14/13
BC DL	10.0 PSF	DRW HCUSR487 13226014
BC LL	0.0 PSF	HC-ENG JB/MPF
TOT LD	37.0 PSF	SEON- 314076
DUR FAC	1.25	
SPACING	24.0"	JREF- 1UYS487_Z01

(13-215B--BRYAN ZECHER Odum Residence -- Lake City, FL - PBC1 18 Gable)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

Left end vertical not exposed to wind pressure

See DWGS A12015ENC100212, GBLLET1N0212, & GABRST100212 for more requirements

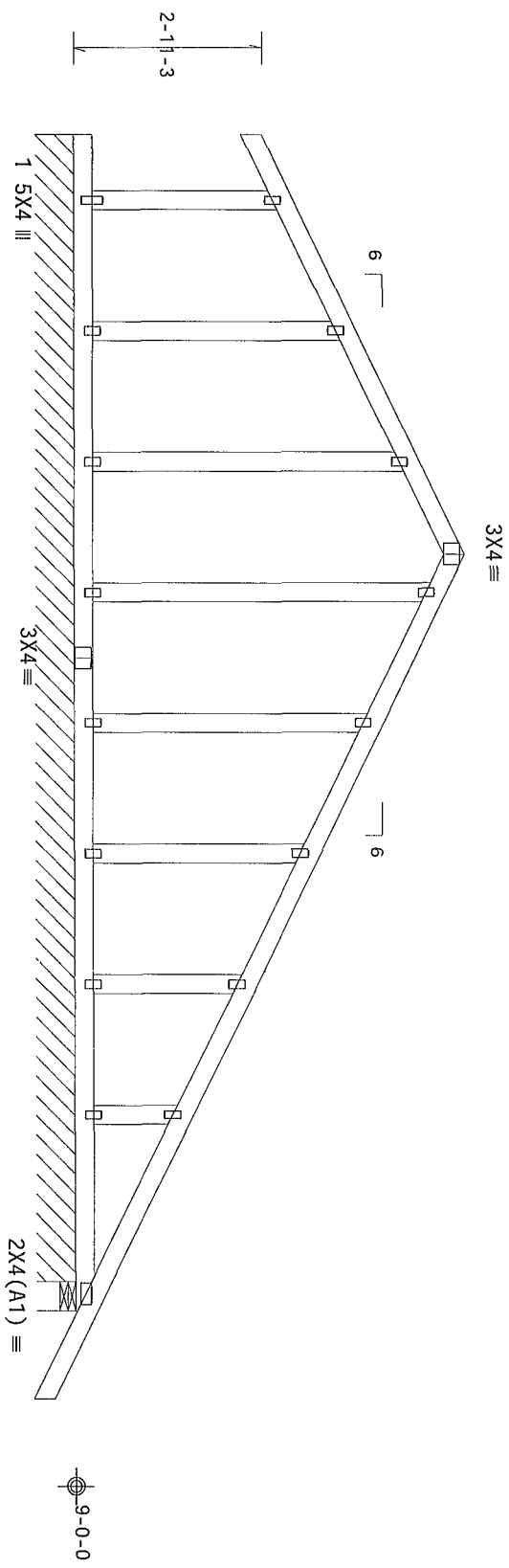
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf 6Cpl(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design

Gable end supports 8" max rake overhang

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50



Note All Plates Are 1.5X3 Except As Shown.
Design Crit. FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

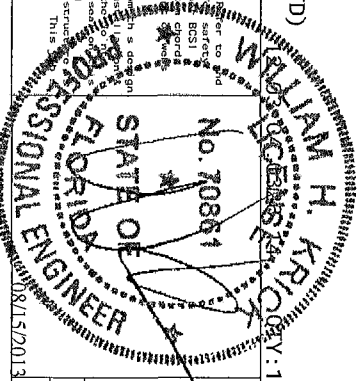
PLT TYP. Wave
R=70 PLF U=13 PLF W=17-6-8
RL=6/-7 PLF

R=213 U=40 W=5.473" (5.473" min)

ALPINE

ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, shipping, handling, and bracing. Follow the latest edition of BCSI (Building Component Safety) Information on by TPI and WDA for BCSI practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have properly attached structural sheathing. Lateral bracing shall be provided per BCSI sections B5, B7, or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design or construction of the truss in accordance with the design and construction of the truss. A drawing or cover page listing this design shall indicate acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TP1 1 Sec 2. For more information on see BCSI information page 17th-800 www.bcsi.org WDA www.structure.com IBC www.bcsi.org



FL/-/3/-/-/R/-		Scale = .375"/Ft	
TC LL	20.0 PSF	REF	R487-- 35525
TC DL	7.0 PSF	DATE	08/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13226013
BC LL	0.0 PSF	HC-ENG	JB/WMP
TOT LD	37.0 PSF	SEON-	314077
DUR. FAC	1.25		
SPACING	24.0"	JREF-	10VS487_Z01

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED

THIS DIAL IS ONLY APPLICABLE FOR CHANGING THE RECEIVED
FIB SWTWN ON SINGLE PLY SEALED DF IGN, TO T BRAKING OF CAB
36 INU

ALTERNATIVE BRACING SPECIFIED IN HARTILLUW 4 FOR CONJUGATE
TOP MINIMUM ALTERNATIVE BRACING PL PLAN DE IN WITH F DDP1 TE
BRACING

W/B MEMBER	SPECIFIED CLB BRACING	T OP L	ALTERNATIVE BRACING	BRA L	SCAB	BPACI
X3 DR 2X4	1 ROW	2X4	1 2X4			
X3 DR 2X4	2 ROWS	2X6	2 2X4			
2X6	1 ROW	2X4	1 2X6			
2X6	2 ROWS	2X6	2 2X4 ()			
2Y8	1 ROW	2X6	1 2X8			
2Y8	2 ROWS	2X6	2 2X6*			

I BRACE L-BRACE AND SCAB BRACE TO BE SAME FELLIES AND GRADE
 NOT BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON
 ENGINEER'S SEALED DESIGN

4) CENTER SCAB ON WIDE FACE OF WEB APPLY (1) CAB TJ EACH
FACE OF WEB



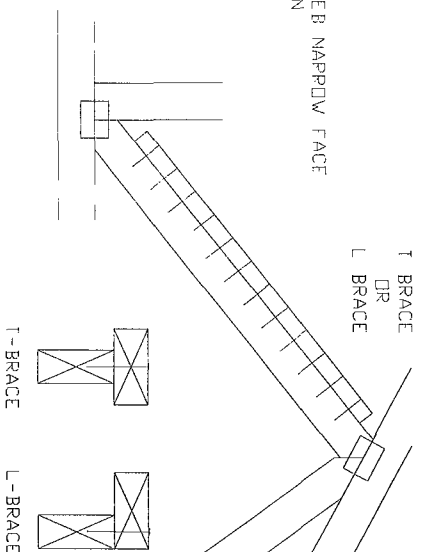
Building Components Group Inc.

ng Components Group Inc.

Earth City MO 63045

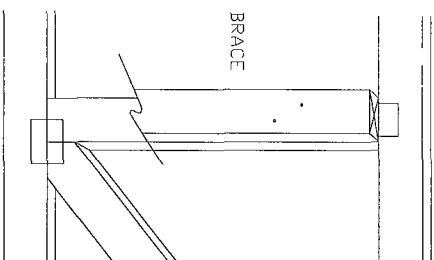
T-BRACING
OR
L BRACING

APPLY TO EITHER OF OF WEB NARROW FACE
ATTACH WITH 100 LUX DR GUN
60126 x 3 MIN N/L
AT 9 BC
BRA E 15 A
MINIMUM 800 OF W I
MEMBER LENGTH



CLAB FRACTION

APPLY SCAB, 1/4" WIDE FACE OF WEB
NO MORE THAN (1) TAB PER FACE
ATTACH WITH 10d BOX DP GUN
2 1/2" x 3" MIN. NAILS
AT 6" OC
BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



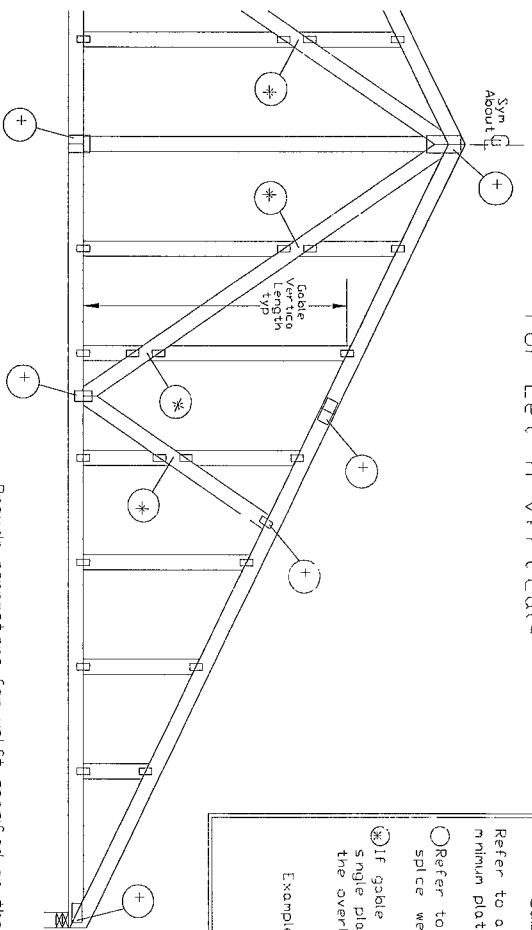
SCAB BRACE

A circular seal for William H. Krick, No. 70861. The seal features a star in the center and a signature across the bottom. The text "WILLIAM H. KRICK" is arched across the top, and "No. 70861" is printed in the center. The seal is partially obscured by a vertical line and some text from the adjacent page.

TC LL	PSF	REF	CLB SUBST
TC DL	PSF	DATE	1/1/09
BC DL	PSF	DRWG	BRCLBSUB0109
BC LL	PSF		
TOT LD	PSF		
DUP FAC			

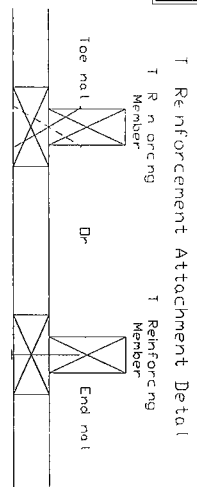
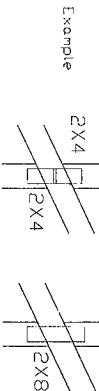
08/15/2013

Gable Detail For Let-in Vertical



Gable Truss Plate Sizes

- Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs
- Refer to Engineered truss design for peak splice web and heel plates
- If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web



To convert from L to T reinforcing members multiply "L" increase by length based on appropriate ITW gable detail

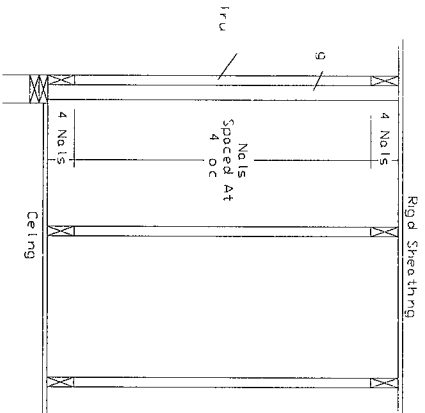
Maximum allowable T reinforced gable vertical length is 14' from top to bottom chord

"T" reinforcing member material must match specification of the L reinforcing member

T Reinf	"T"
Member Size	30 7"
Length	20 7'

Example
 A 14' 7 10' Wind Speed 120 mph
 Mean Roof Height 30 ft Kzt = 1.00
 Gable Vertical 24 OC SP #3
 T Reinforcing Member Size - 2x4
 T Brace Increase (from Above) - 30% = 1.30
 (1) 2x4 L Brace Length 8 7'
 Maximum T Reinforced Gable Vertical Length 130 x 8 7' = 11 2'

See appropriate ITW gable detail for maximum vertical length

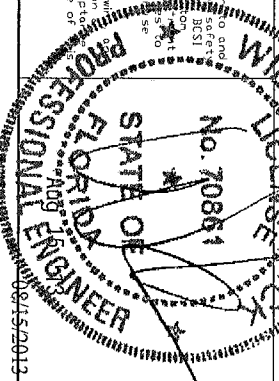


****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING. **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**



Building Components Group Inc.

Earth City MO 63045



REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLETTIN0212
MAX TOT LD	60 PSF
DUR FAC	ANY
MAX SPACING	24 0

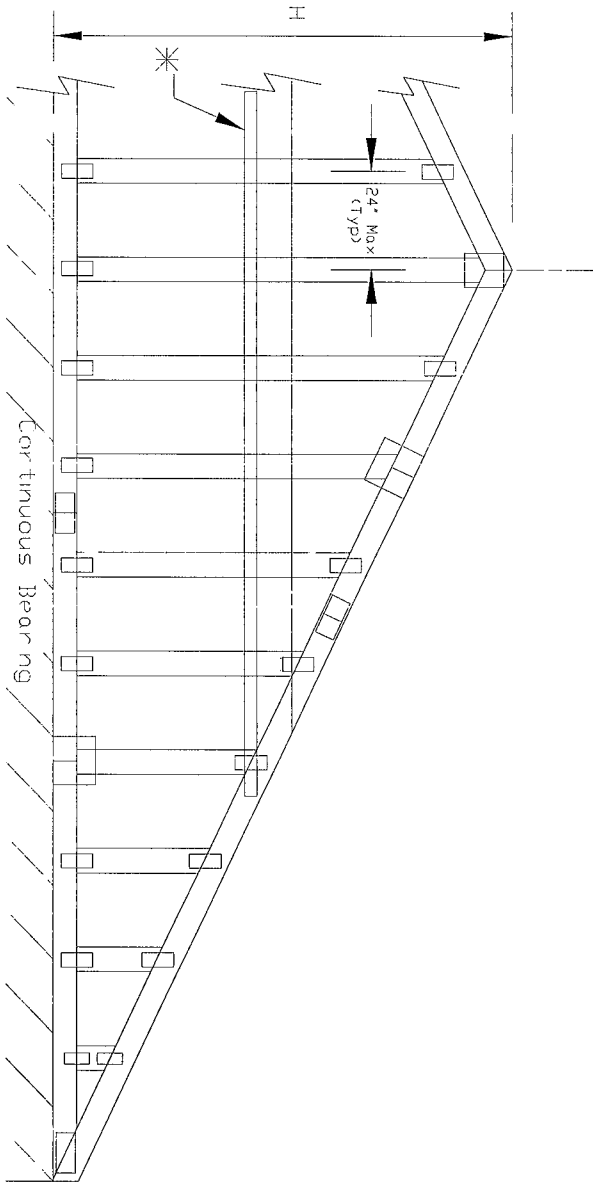
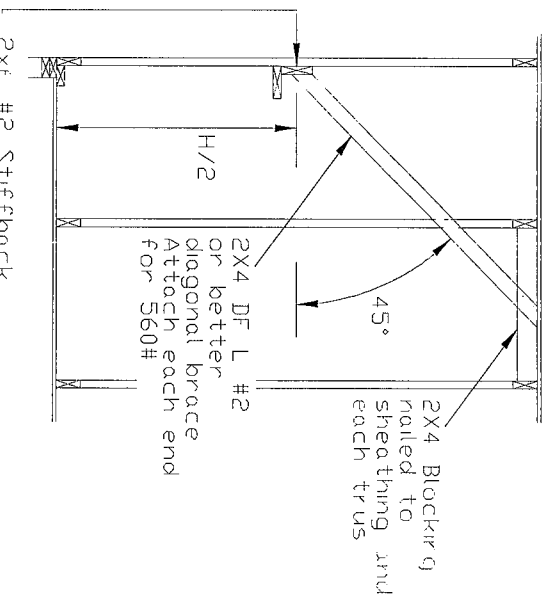
ASCE 7-10 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

- 120 mph 30ft Mean Hgt ASCE 7-10 Enclosed Exp C or
- 100 mph 30ft Mean Hgt ASCE 7-10 Enclosed Exp D or
- 100 mph 30ft Mean Hgt ASCE 7-10 Partial Enclosed Exp C
- 100 mph Wind TC DL 50 psf Wind BC D 30 psf
- 1 Central chord bracing requirement
- 1 Continuous roof sheathing
- 1 Continuous ceiling diaphragm
- 1 Engineer's sealed design reference (this detail)
- 1 Lumber plates and other information not shown
- 1 This detail
- 1 No 10d box or gun (0128 x3 min) nails

- H Less than 46 no stud bracing required
- H Greater than 46 to 76 in length provide 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 60' (see detail below or refer to IPWG A12030ENC10)
- H Greater than 76 to 120 max provide 2x6 stiffback at mid-height and brace to roof diaphragm every 40' see detail below or refer to IPWG A12030ENC10
- * Optional 2 L-reinforcement attached to stiffback with 10d box or gun (0128 x3 min) nails @ 6' or

2x6 #2 Stiffback
attached to each
stud w/ (4) 10d box or gun (0128

min) nails



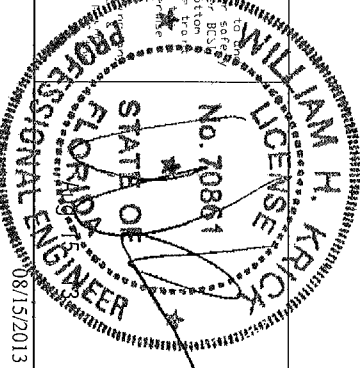
Building Components Group Inc.

Earth City MO 63045

***WARNING: READ AND FOLLOW ALL INSTRUCTIONS ON THIS DRAWING. INSTALLERS

1. Users require the use of the correct fasteners and practices prior to performing the work. The fasteners and practices noted otherwise are for informational purposes only. The user is responsible for ensuring that the fasteners and practices are appropriate for the specific application. Refer to drawings 160A 7 or 160A 8 for details.

11/11 Building Components Group Inc. has been authorized by the State of Florida to provide engineering services. The user is responsible for ensuring that the fasteners and practices are appropriate for the specific application. Refer to drawings 160A 7 or 160A 8 for details.



08/15/2013

REF GE WALKER
DATE 2/14/13
DRWG GABRST100212

MAX 101 ID 60 PSF
MAX PALING

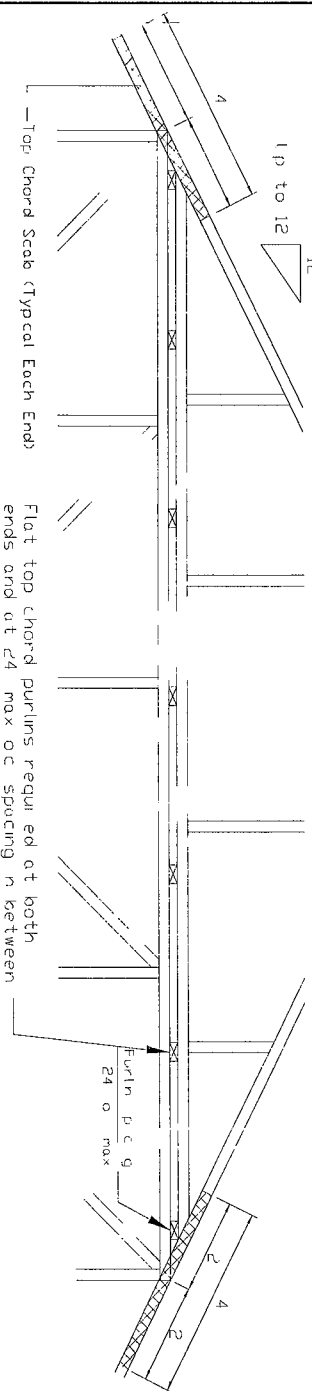
160 mph Wind 3000 ft Mean Hgt A-JCE 7 10 En 0.927 B-Bq located anywhere n roof Exp C Wind Dir - 50 ps (mm) Kzt-10
Pr 140 mph wnd 3000 ft Mean Hgt A-JCE 7 10 En 0.9 B-Bq located anywhere n roof Exp D Wind Dir 50 psf (mm) Kzt-0

Not Top chords of trusses supporting psgy ck pl ses ut br ad quae ly, croced by st thng u purlins. The ouliding Engine of R od f al

Maximum truss spacing is 24' o.c. detail is not applicable if support additional loads such as upola steeply chimney or drag strut loads

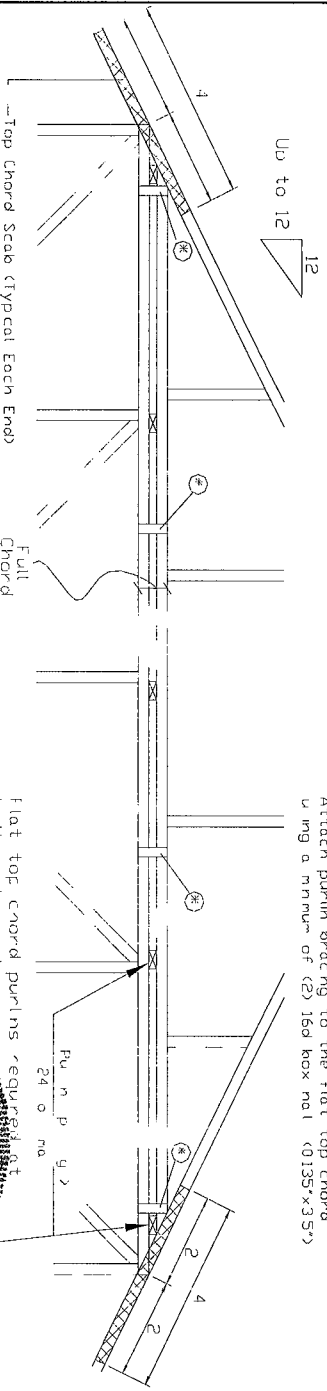
** Refer to Engineers sealed true design drawing & pg 3b & and k of test specimens

12
up to 12



Flat top chord purlins required at both ends and at 24 max oc spacing between

Def + 0.1 B Purlin Spacing / 24" OC



Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5")

	Full			flat top chord f
typical Each End)	Chord			bath ends surin
	Depth			
Note If pupils or cheating r	t	f	e fed on ti	fat too f t A
+ uses our n mis b	t	t	d no mo	and i h + A



TMC

Building Components Group Inc.

Building Components Group Inc.

Earth City MO 63045

[illegible]

SPACING 240

240

* In addition provide connection with one of the following methods

TRIMOX

Use 3x8 Trulox plates for 2x4 chord member and 3x10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8" o.c. with (4) 0120 x1375 nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4" o.c. front to back faces.

APA Rated Gusset

8'x8'x7/16" (mm) APA rated sheathing gussets
(rect, 9'x6') Attach @ 8" on with (8) 6d common
(0.113x2.2) nails per gusset (4) in cap bottom
chord and (4) in base front to chord. Gussets
may be staggered 4" OC front to back. 1 cbs

2x4 SPF #2 full chord depth scabs (each ace)
Attach @ 8" o.c. with (6) 10d box nails (0.128" x 3")
per scab (3) n cap bottom chord and (3) n
bo" trus top chord. Scabs may be staggered
4" c front to back faces

28PB Wave Piggyback Plate

One 28PB wave piggyback plate to each face
e 8 or 8 attach teetin to piggyback at time of
fabrication attach to supporting truss with
(4) 0120 1375 nuts per face per ply
Piggyback plate may be stogered 4' oc front
to back
accs.

REF PIGGYBACK

DATE 2/14/16

DPWG PB16010 v.12

	0.0	0.5	1.0	1.5	2.0
0.0	0.0000	0.0000	0.0000	0.0000	0.0000
0.5	0.0000	0.0000	0.0000	0.0000	0.0000
1.0	0.0000	0.0000	0.0000	0.0000	0.0000
1.5	0.0000	0.0000	0.0000	0.0000	0.0000
2.0	0.0000	0.0000	0.0000	0.0000	0.0000

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466	467	468	469	470	471	472	473	474	475	476	477	478	479	480	481	482	483	484	485	486	487	488	489	490	491	492	493	494	495	496	497	498	499	500	501	502	503	504	505	506	507	508	509	510	511	512	513	514	515	516	517	518	519	520	521	522	523	52
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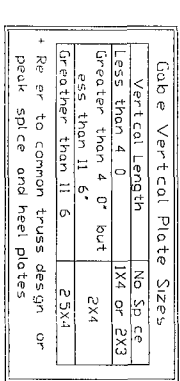
Professional Engineer Seal for the State of Florida, No. 70861, dated 08/15/2013.

08/15/2013

ASCE 7-10 120 mph Wind Speed, 30 Mean Height Enclosed Exposure C, $K_z t = 1.00$

Dr	100	Mph	Wind Speed	30	Mean Height	Partially Enclosed	Exposure C	Kzt = 1.00
Dr	100	mph	Wind Speed	30	Mean Height	Enclosed	Exposure D	Kzt = 1.00

1x4 Braces shall be S&B (Stress Rated Board).
 **For 1x4 So. Pine use only Industrial 55 or
 Industrial 45 Stress Rated Board
 value may be used with these if uses

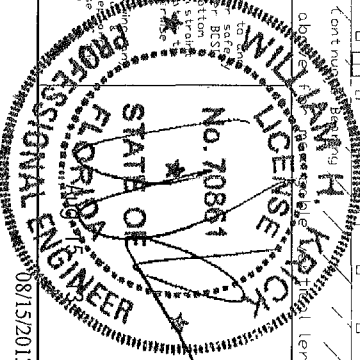


Refer to the Building Designer for conditions not addressed by this detail

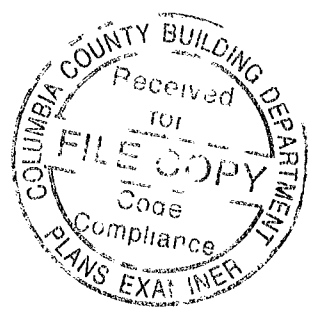
*****WARNING: ** READ AND FOLLOW ALL INSTRUCTIONS ON THIS DRAWING. **
*****IMPORTANT: ** FURNISH THE DRAWING IN ALL IN FACTORS IN THE INITIAL RS

MAX TOT LD 60 PSF

Earth City MO 63045



08/15/2013

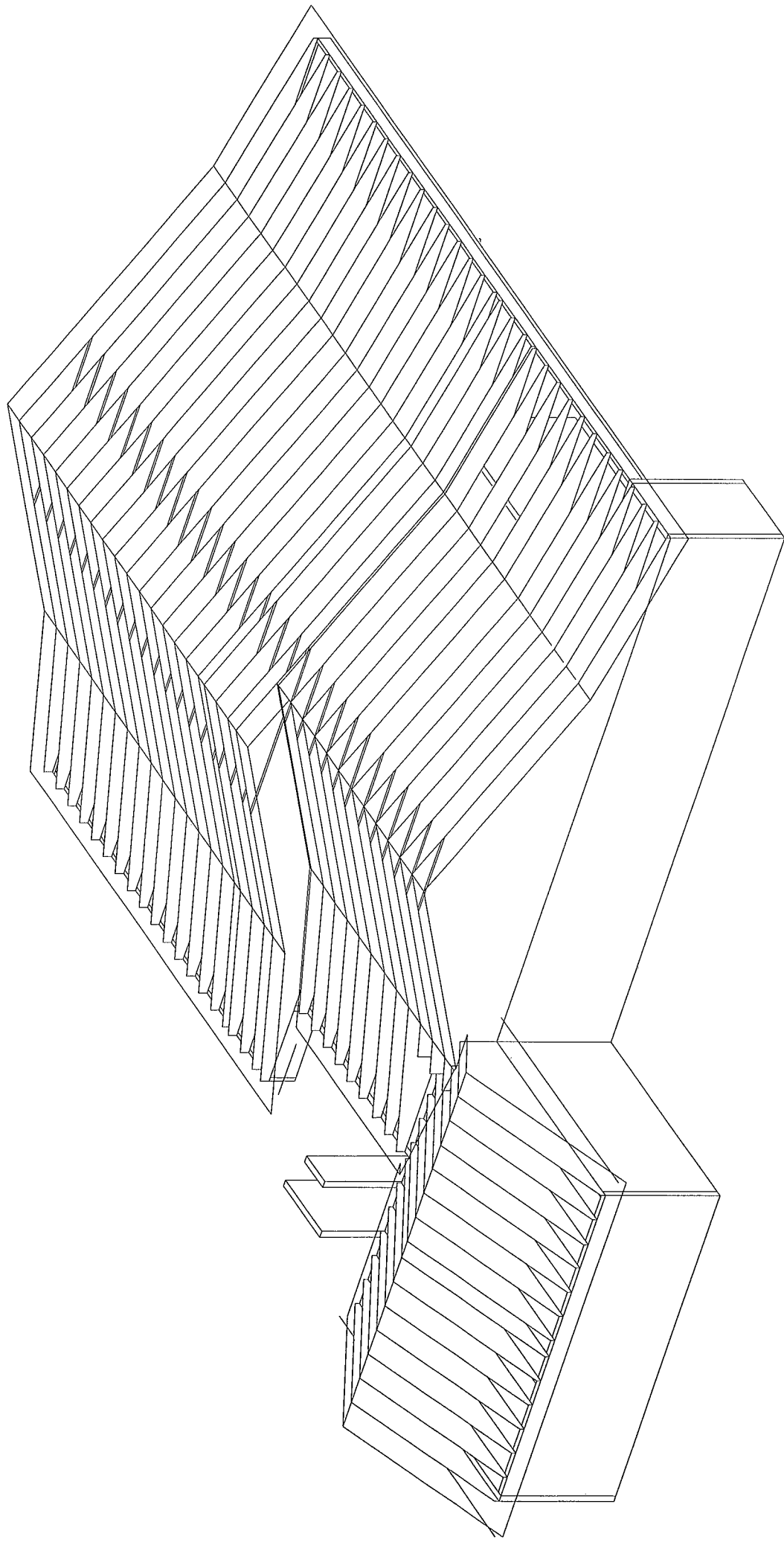


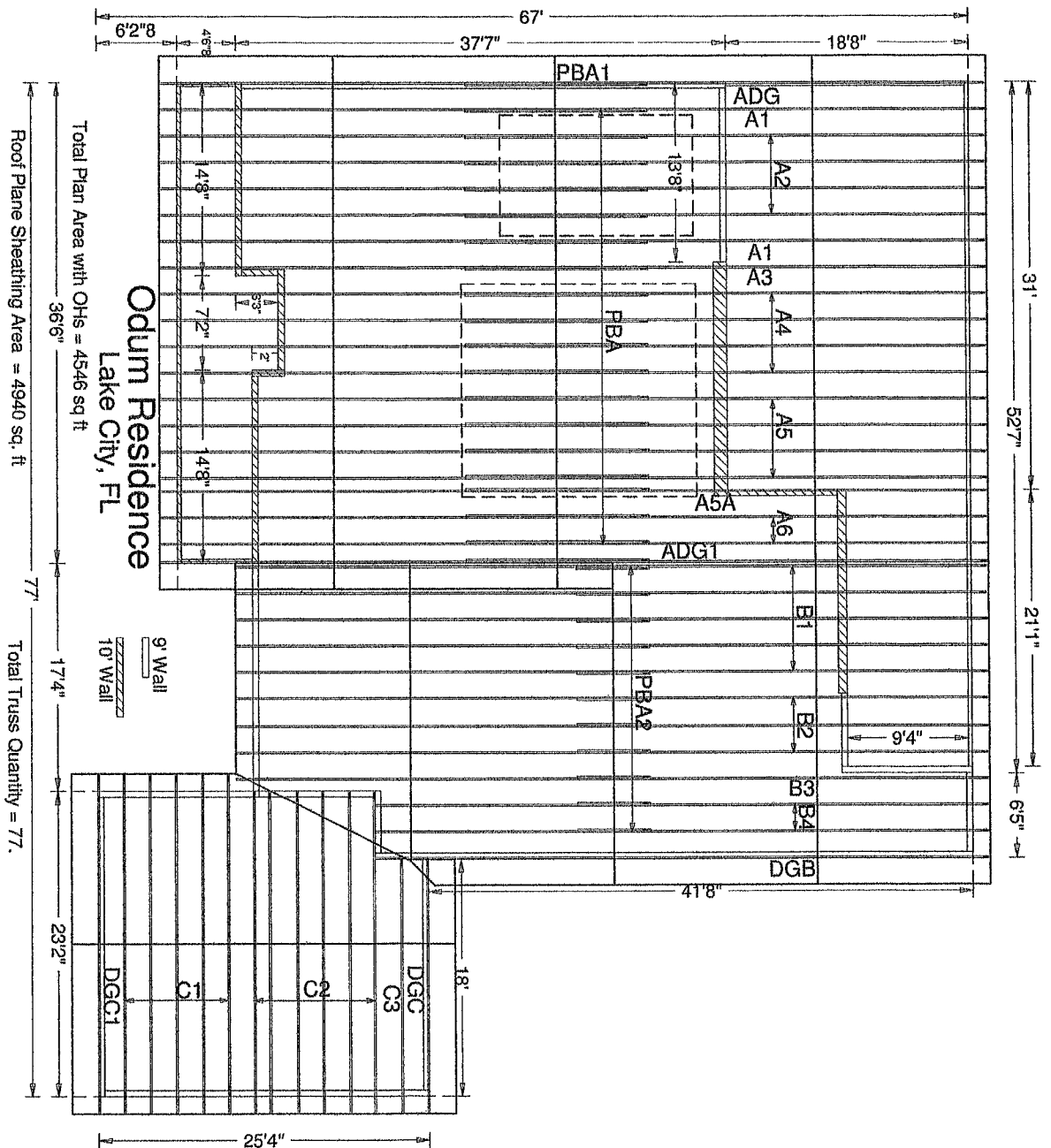
Odum Residence
Lake City, FL

Total Plan Area with OHs = 4332 sq ft

Total Truss Quantity = 65

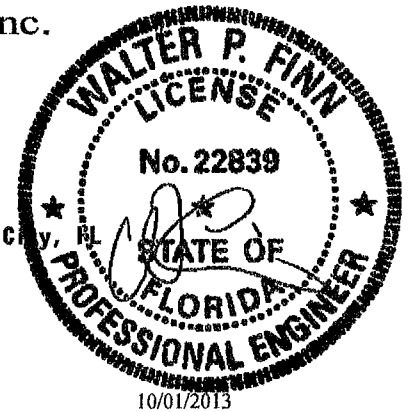
Roof Plane Sheathing Area = 4690 sq ft





ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V04487-Z0101114133



Truss Fabricator **Anderson Truss Company**
Job Identification. **13-215D--BRYAN ZECHER /Odum Residence -- Brown Road Lake City, FL**
Truss Count **3**
Model Code **Florida Building Code 2010**
Truss Criteria. **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 12.03.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61015-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes.

- 1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1**
- 2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.**
- 3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114**

Walter P. Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-160TL-

#	Ref	Description	Drawing#	Date
1	33162-B1	55' Special R	13274002	10/01/13
2	33163-A6	60'9"8 Specia	13274003	10/01/13
3	33164-A5A	60'9"8 Speci	13274004	10/01/13

Top chord 2x4 SP #1-13B
Bot chord 2x6 SP #2-13B
Webs 2x4 SP #3-13B W6, W19 2x4 SP #2-13B.

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member.

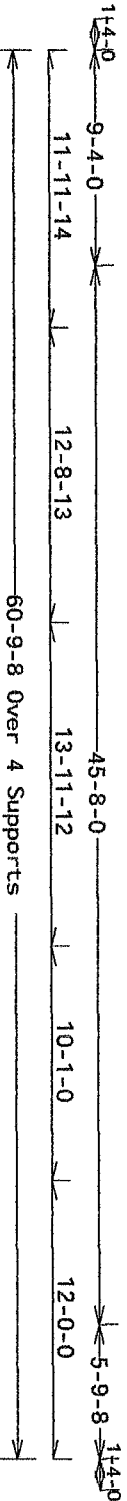
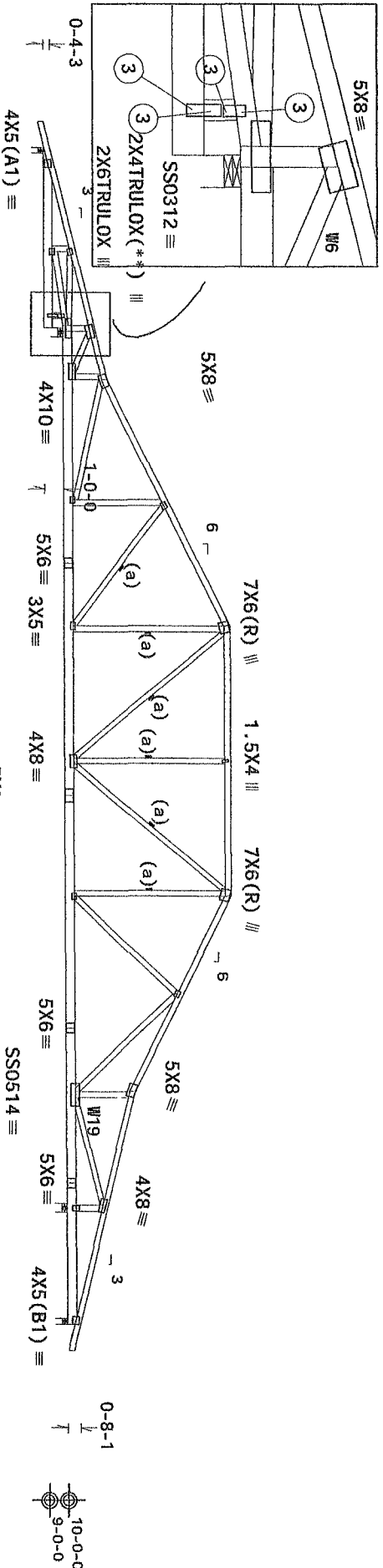
Truss passed check for 20 psf additional bottom chord live load in areas with 42'-high x 24'-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MMFRS loads based on trusses located at least 15'00 ft. from roof edge.

11 GAUGE (0.120"x1.375" nails required for trulox plate attachment. Nails specified in circles must be applied to each face of each truss ply. See DWG 1601TL for nailing and trulox plate requirements.

(0.120"x1.25" nail maybe used in lieu of nail called out above)



R=272 U=70 W=3.5" (3.5" min.)
R=190/-179 R=3320 U=57 W=5.5" (5.5" min.)

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TP1-2007(Std)

FT/RT=10%(0%)/0(0)

PLT TYP. 18 Gauge HS Wave, Trulox

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety Information) by TPI and WFOA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3 B7 or B10 as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from any failure to build the truss in conformance with ANSI/TP1 1 or for handling, shipping, installing, bracing or otherwise. Apply plates to each face of truss and position as shown above and on the details. Unless noted otherwise, bracing shall be installed per BCSI sections B3 B7 or B10 as applicable. Details are provided for standard plate bracing. The installer shall be responsible for the responsibility of the Building Designer per ANSI/TP1 1 Sec 2. For more information see the general notes page ITW-BCG www.itwbcg.com TPI www.tpiinc.org WFOA www.structure.com This is a general notes page ITW-BCG www.itwbcg.com TPI www.tpiinc.org WFOA www.structure.com

No. 22839

STATE OF FLORIDA

PROFESSIONAL ENGINEER

FL/-/4/-/R/-

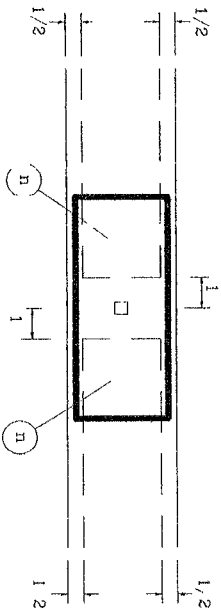
Scale = .125"/Ft.

R=2616 U=57 W=5.5" (5.5" min.)
R=130/-255 U=73 W=3.5" (3.5" min.)

TC LL	20.0 PSF	REF	R9114- 33163
TC DL	7.0 PSF	DATE	10/01/13
BC DL	10.0 PSF	DRW	HUIS9114 13274003
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT. LD.	37.0 PSF	SECM-	322574
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1V04487_Z01

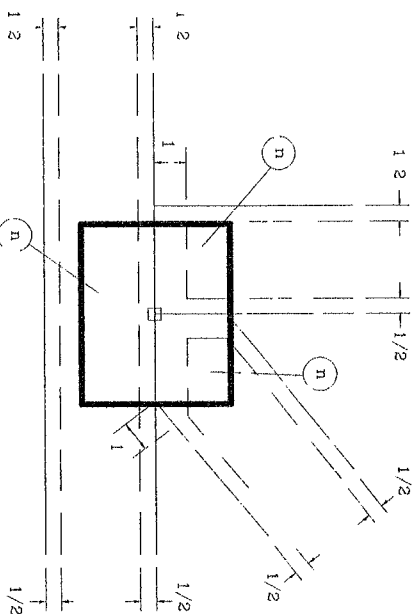
TRULOX INFORMATION DETAIL

TYPICAL OFF PANEL SPLICE

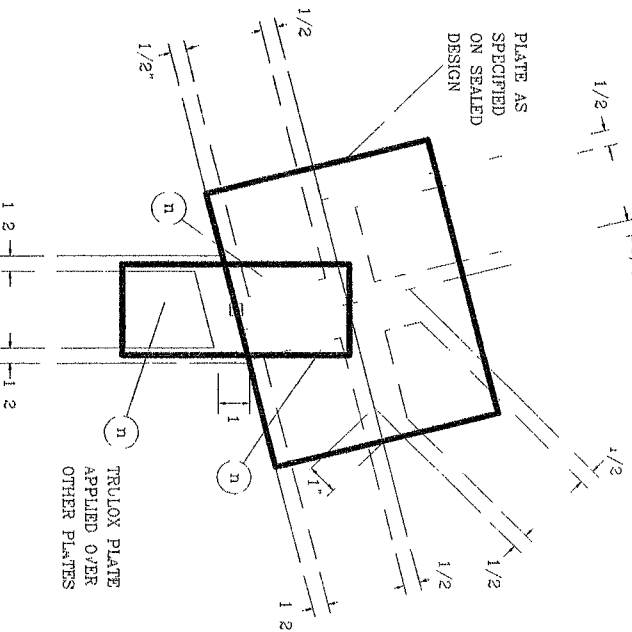


DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE AS SHOWN BY DASHED LINES
NAILS MUST NOT SPLIT LUMBER

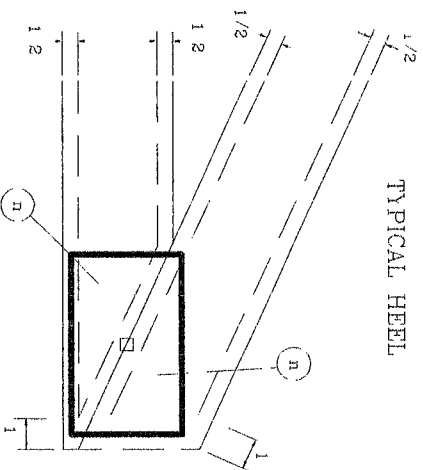
TYPICAL PANEL POINT WITHOUT SPLICE



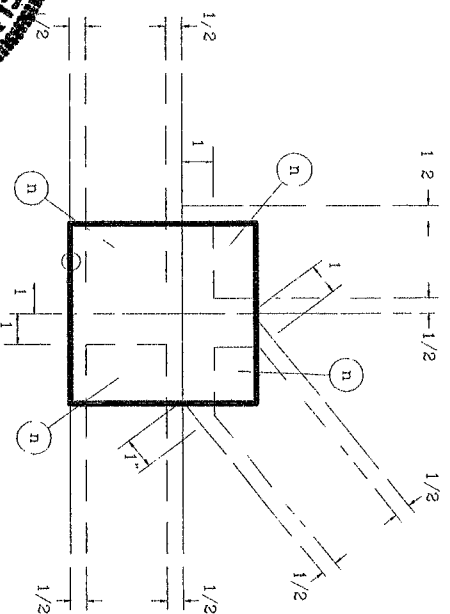
TYPICAL FILLER



TYPICAL HEEL



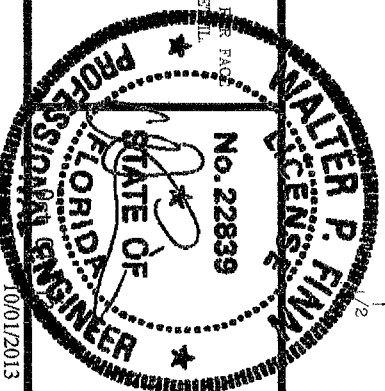
TYPICAL PANEL POINT SPLICE



- NOTES
(a) IS THE REQUIRED NUMBER OF 20 X 1 3/4" NAILS OR EQUAL, PER FACE PER PLATE AS SPECIFIED ON THE SEALED DESIGN REFERENCE THIS DETAIL
○ LOCATES PLATE CORNER OR FLUSH EDGE
□ LOCATES PLATE CENTER



Earth City, MO 63045



TRULOX PLATING

160
TL

PAGE 1 OF 1
DATE 4/1/09

(REPAIR / 13-215D -B1 /BRYAN Z - B1 Jack Scab)

REPAIR TO CHANGE EXTERIOR GEOMETRY OF TRUSS AS SHOWN

FIELD REPAIR MUST COMPLY WITH ALPINE DESIGNS AND SPECIFICATIONS.

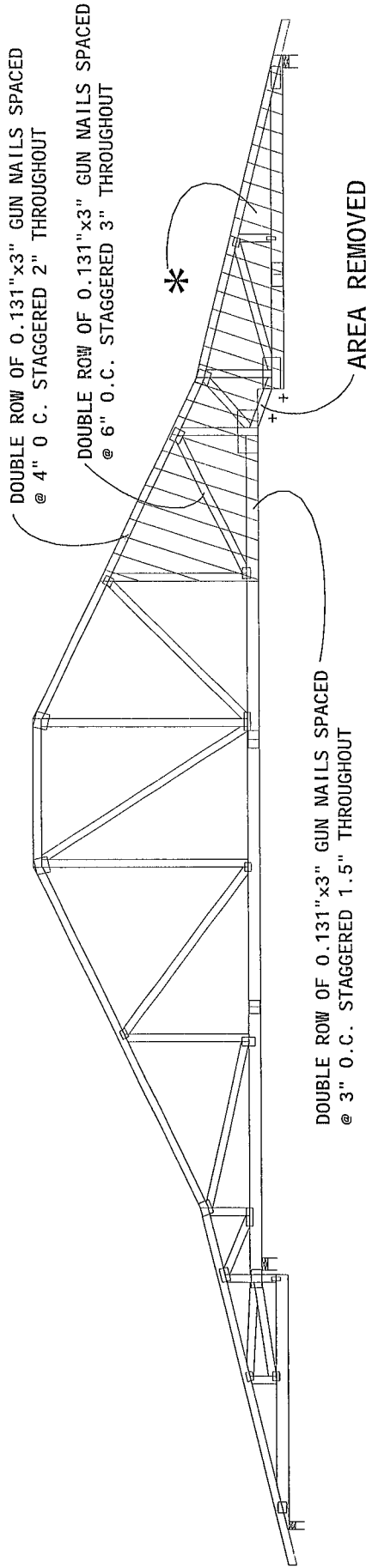
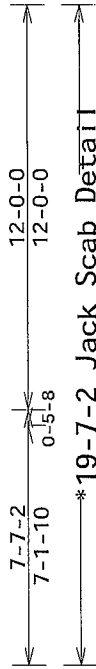
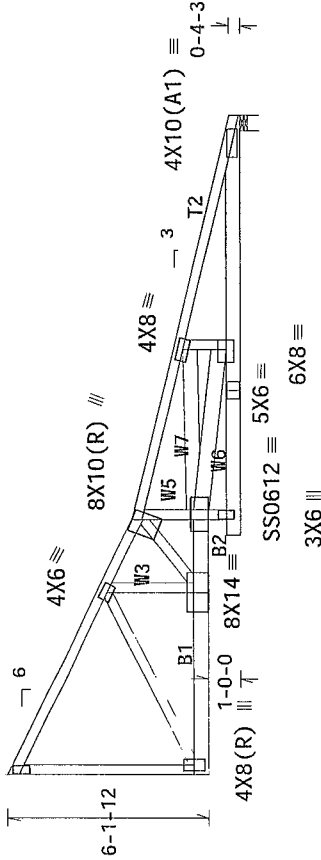
REFER TO DRAWING HCUSR9114 13282056 FOR LUMBER, PLATES AND OTHER DATA NOT GIVEN HERE.

Prior to and during repair operation, this truss and any supported spans must be temporarily braced and shored. The design and positioning of such shall be designed and supplied by others.

+ USE A SHARP METAL CUTTING SAW BLADE TO CAREFULLY REMOVE UNWANTED MATERIAL FROM THE TRUSS AS SHOWN DASHED. REMAINING PORTIONS OF TRUSS MUST BE FREE FROM DAMAGE

* ONE (1) JACK SCAB. CHORDS AND WEBS AS SHOWN IN DETAIL ONE (1) JACK SCAB REQUIRED. ATTACH JACK SCAB TO ONE FACE OF TRUSS WITH A DOUBLE ROW OF 0.131"x3" GUN NAILS SPACED AT 24" O.C. STAGGERED 12" THROUGHOUT MATCHING MEMBERS, UNLESS OTHERWISE NOTED REFER TO DRAWING CENAILSPOT09 FOR ADDITIONAL NAIL SPACING INFORMATION

Top chord 2x4 SP #1 T2 2x4 SP 2850f-2.3E
Bot chord 2x6 SP #1 Dense :B1 2x6 SP #2 :B2 2x6 SP SS
Webs 2x4 SP #3 W3 2x4 SP #2.
3X6 || W5 2x4 SP 2850f-2.3E: W6, W7 2x6 SP SS.



PLT TYP. 18 Gauge HS.Wave

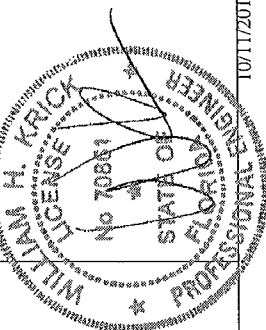
Design Crit. FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

13.02.00.0926 19 QTY 1 FL/-/3/-/-/R/- Scale = .1875"/Ft.

TRUSS REPAIR

DAMAGED TRUSSES MUST BE CAREFULLY EVALUATED TO DETERMINE THE EXTENT OF DAMAGE AND THE FEASIBILITY OF REPAIR. IN SOME CASES THE PRUDENT SOLUTION IS TO SCRAP THE DAMAGED TRUSSES AND REBUILD. INTERNAL WOOD FIBER DAMAGE AND EXCESSIVE CONNECTOR STRESS FROM BENDING OR SHOCK CANNOT BE READILY DETECTED THEREFORE IT IS VITAL THAT THE TRUSS FABRICATOR AND BUILDING CONTRACTOR CONSIDER THE CAUSE OF THE DAMAGE IN THEIR DECISION WHETHER TO REPAIR OR REBUILD

REPAIR WORK SHOWN ON THIS DRAWING APPLIES ONLY TO THOSE SECTIONS OF THE TRUSS REPORTED BY THE TRUSS MANUFACTURER TO HAVE BEEN DAMAGED. A QUALIFIED THIRD PARTY INSPECTOR SHALL CHECK TRUSSES TO DETERMINE THE EXTENT OF ANY FURTHER DAMAGE IF ANY AND VERIFY THAT REPAIRS HAVE BEEN PERFORMED AS INDICATED ON THIS DRAWING



ALPINE

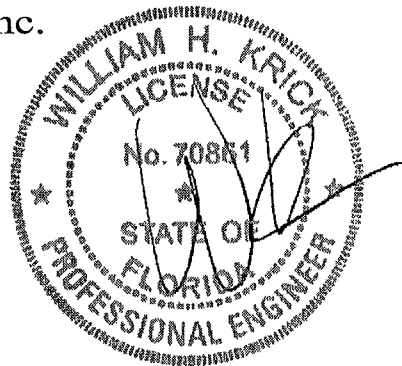
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0278

TC LL	20.0 PSF	REF	R9114- 17151
TC DL	7.0 PSF	DATE	10/11/13
BC DL	10.0 PSF	DRW	HCUSR9114 13284001
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	1625
DUR.FAC.	1.25	FROM	CVB
SPACING	24.0"	JREF-	1V0D9114Z01

10/11/2013

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V0D9114Z0111111002



Truss Fabricator **Anderson Truss Company**
Job Identification. **REPAIR / 13-215D -B1 /BRYAN Z (REPAIR / 13-215D-)**
Truss Count **1**
Model Code **Florida Building Code 2010**
Truss Criteria. **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 13.02.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61615-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

10/11/2013

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H. Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: -

#	Ref	Description	Drawing#	Date
1	17151--B1	Jack Scab	13284001	10/11/13