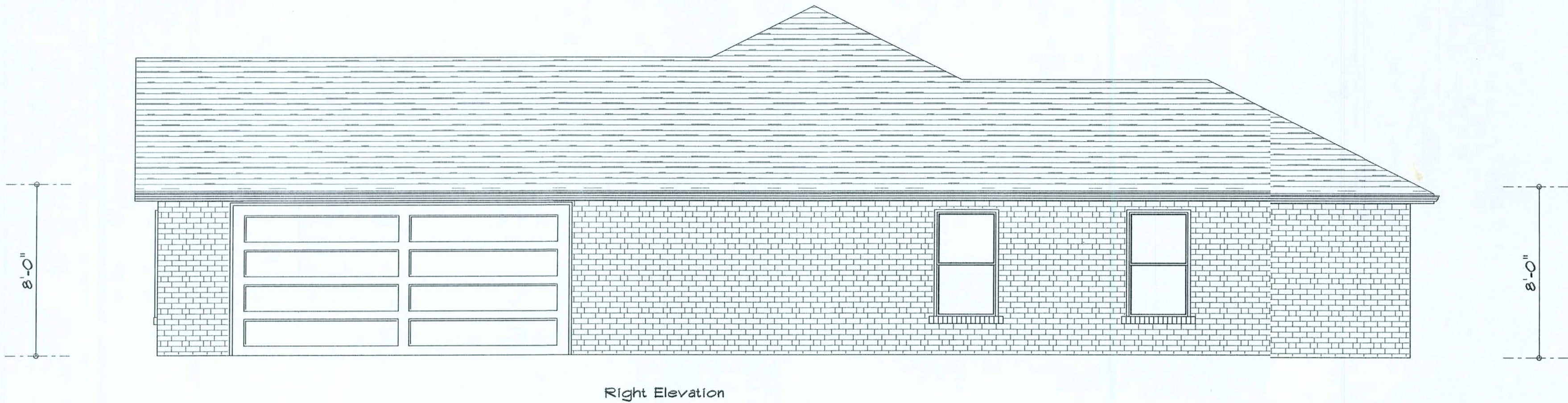
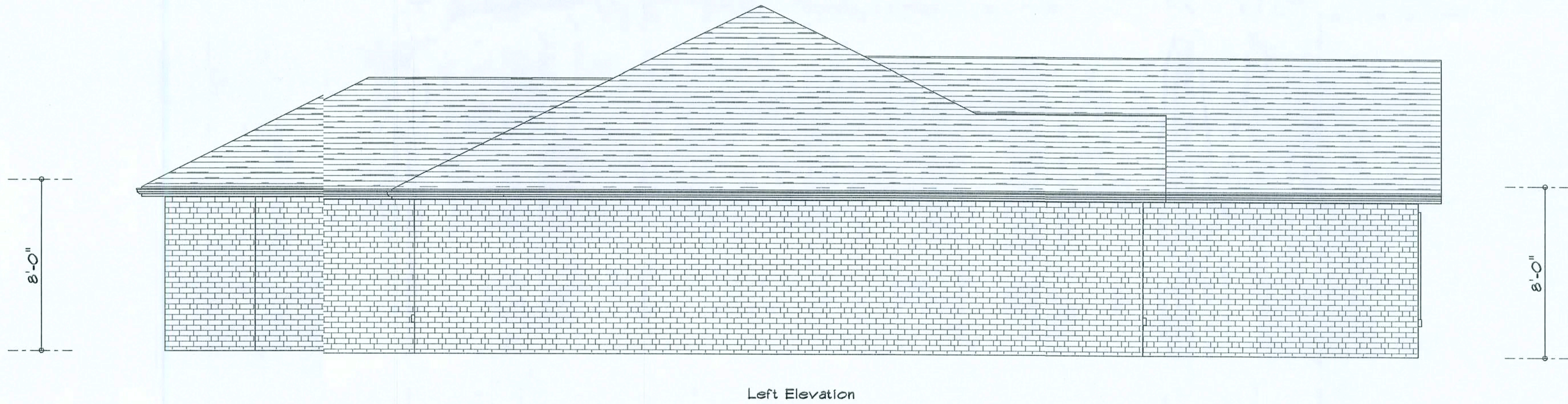
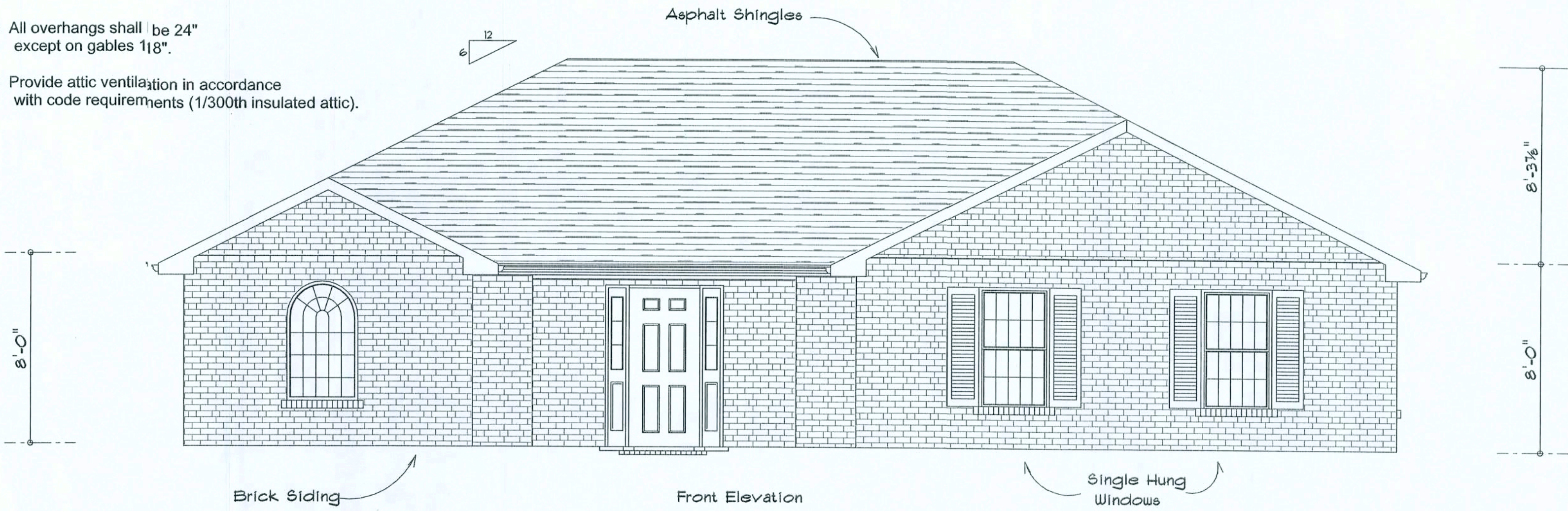


- Notes:
- R-1 All roof pitches shall be 6/12 unless otherwise noted.
  - R-2 All overhangs shall be 24" except on gables 18".
  - R-3 Provide attic ventilation in accordance with code requirements (1/300th insulated attic).

REVISIONS	

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



RESIDENCE

AnnDaniels  
Lot 64Emerald Cove

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Columbia County, Florida

Woodman Park Builders, Inc.  
Lake City, Florida  
Phone: (386) 755 - 8699  
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Email:

PRINTED DATE:  
April 9, 2006

DRAWN BY: Mark Haddox

CHECKED BY: William G. Wood

DESIGNED BY: Mark Haddox

FINALS DATE:  
19 / Apr / 06

JOB NUMBER:  
634121

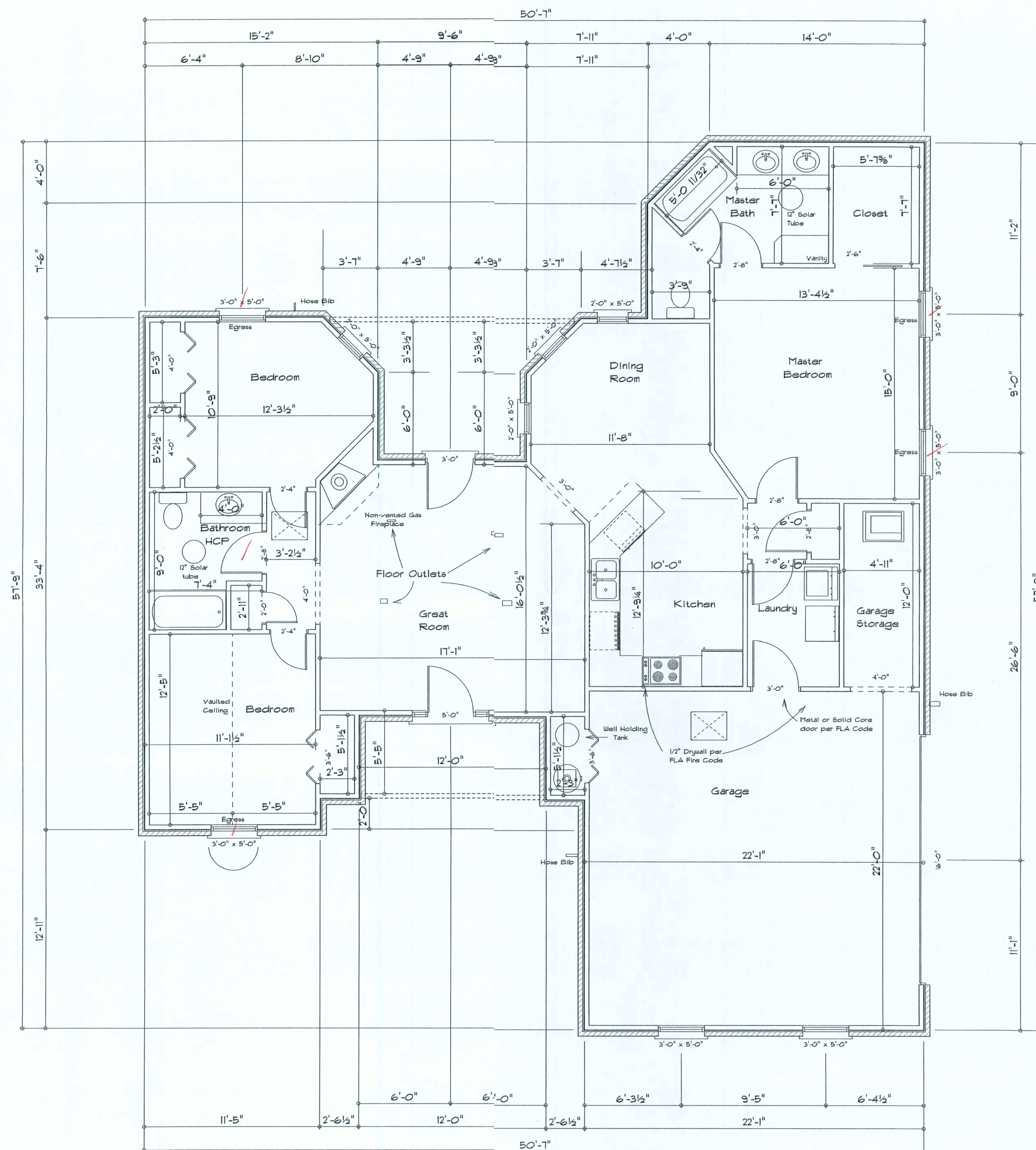
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OF 3 SHEETS



REVISIONS

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



Floor Plan  
Scale: 1/4" = 1'

Note: Septic And Well Location  
were not known at the time plans  
were engineered. Consult Contractor  
for location.

# AREA SUMMARY

Living Area	1403	S.F.
Garage Area	561	S.F.
Porch Area	169	S.F.
<b>Total Area</b>	<b>2139</b>	<b>S.F.</b>

## RESIDENCE

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DESIGNED BY:

Mark Haddox

FINALS DATE:

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604121

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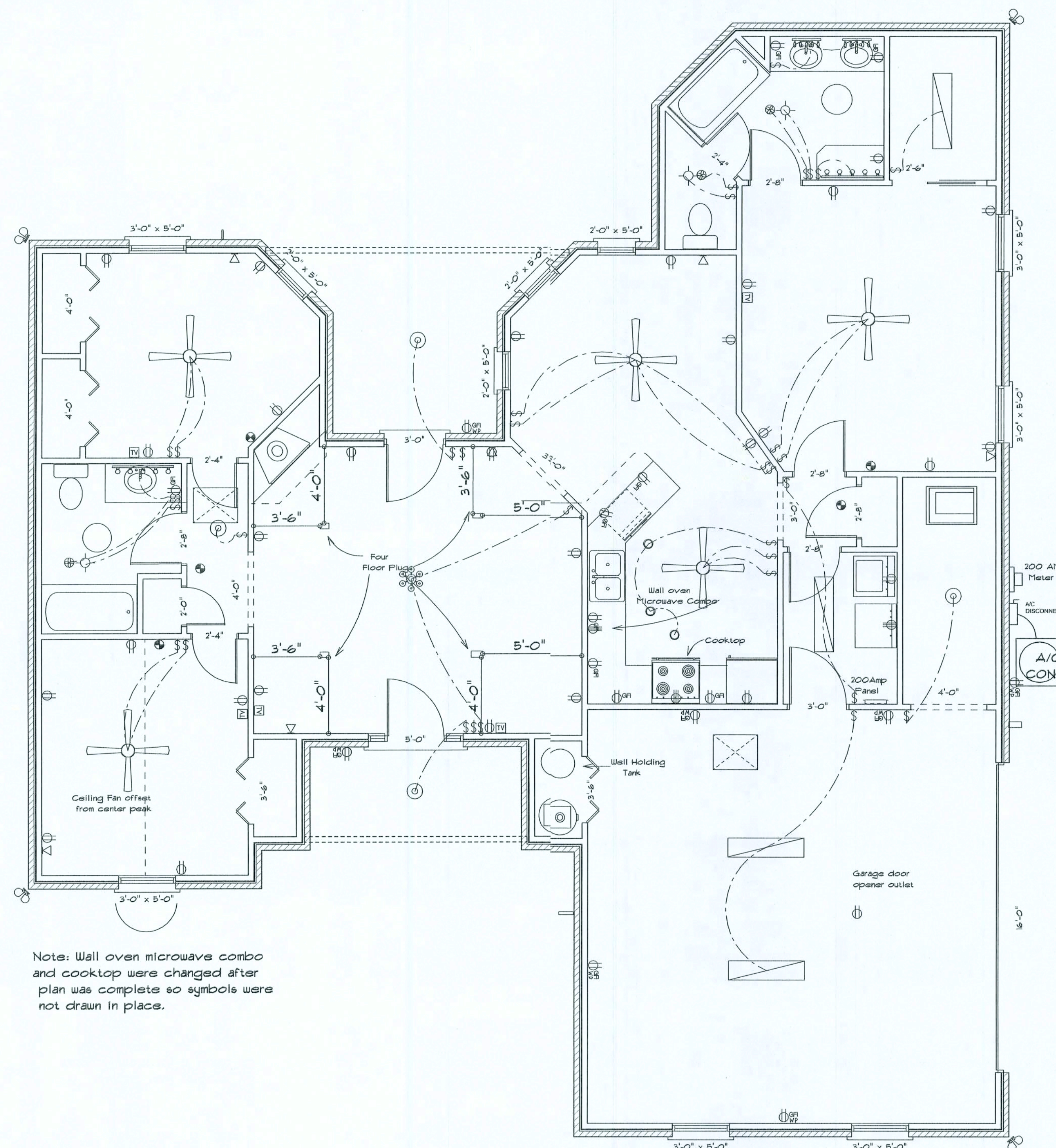


REVISIONS


SOFTPLAN  
ARCHITECTURAL DESIGN FOR FUTURE

Electrical Plan Notes:

- E-1 Wire all appliances, HVAC units and other equipment per manufactures specifications.
- E-2 Consult the owner for the number or separate telephone lines to be installed. Owner is responsible for all overages not noted on plan.
- E-3 All installations shall be per national codes.
- E-4 All smoke detectors shall be 120v with battery back-up of the photoelectric type, and shall be interlocked together. Install inside and near all bedrooms.
- E-5 Telephones, television and other low voltage devices or outlets shall be as per the owners directions and in accordance with applicable sections of the National Electric Codes latest edition. Owner is responsible for all overages not noted on plan.
- E-6 Electrical contractor shall be responsible for the design and sizing of electrical service and circuits.
- E-7 Entry of service (underground or overhead) to be determined by contractor agreement.
- E-8 All bedroom receptacles shall be AFCI (arc fault circuit interrupter).
- E-9 All outlets to be located above base floor elevation.
- E-10 All exterior GFI outlets shall be weatherproof.
- E-11 Overcurrent Protection device shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.



Note: Wall oven microwave combo and cooktop were changed after plan was complete so symbols were not drawn in place.

ELECTRICAL	SYMBOL
ceiling fan globe 1	
ceiling globe light	
chandelier	
double spotlight	
fluorescent fixture	
pot light	
vanity bar light	
electrical panel	
AC Disconnect	
Outlet WP GFI	
cable tv outlet	
fan	
light	
outlet	
outlet 220v	
outlet gfi	
smoke detector	
switch	
telephone	

RESIDENCE

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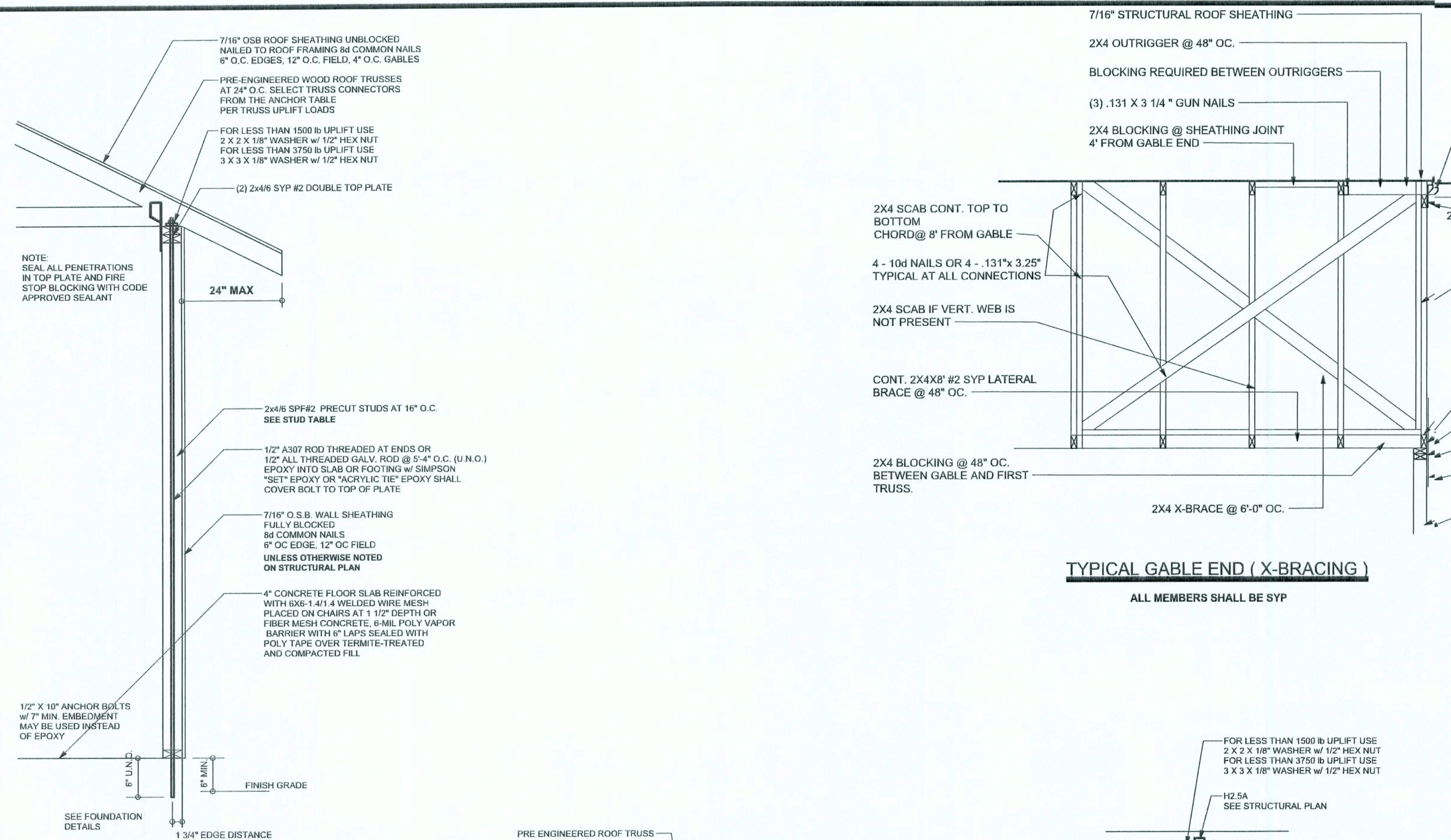
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DRAWING NUMBER

A3  
OF 1 SHEETS



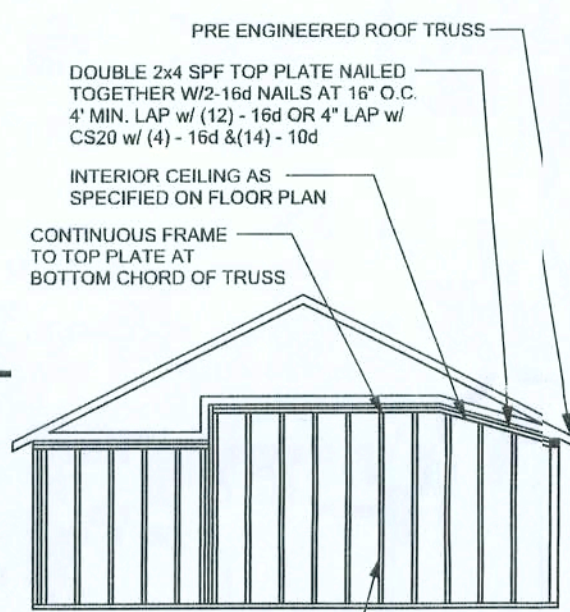


**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"

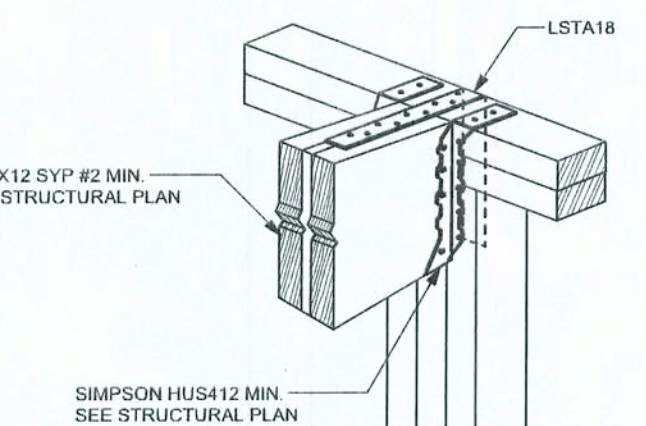
### EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

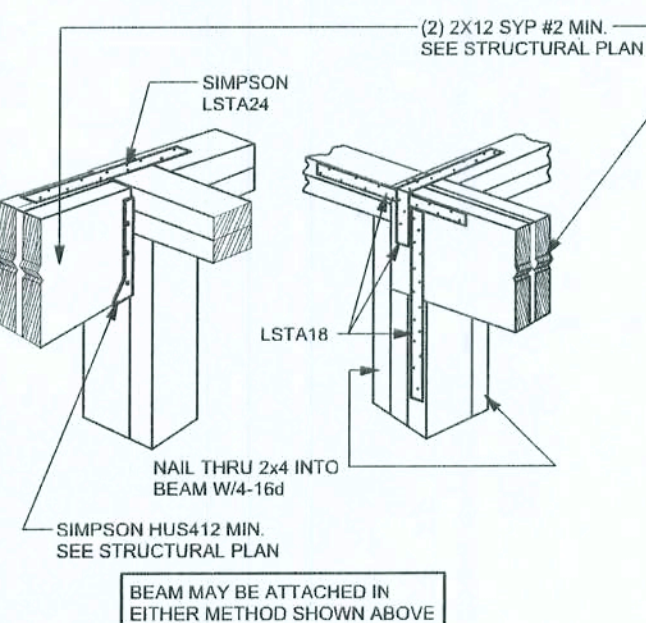
THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.208. EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS: 110 MPH EXPOSURE. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 16" O.C. X 0.85 = 13.6" O.C.



**CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL**  
SCALE: N.T.S.

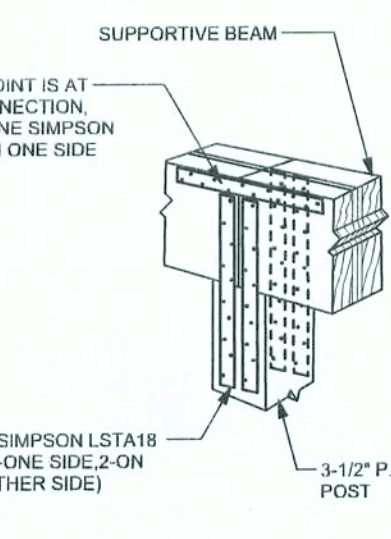


**BEAM MID-WALL CONNECTION DETAIL**  
SCALE: N.T.S.

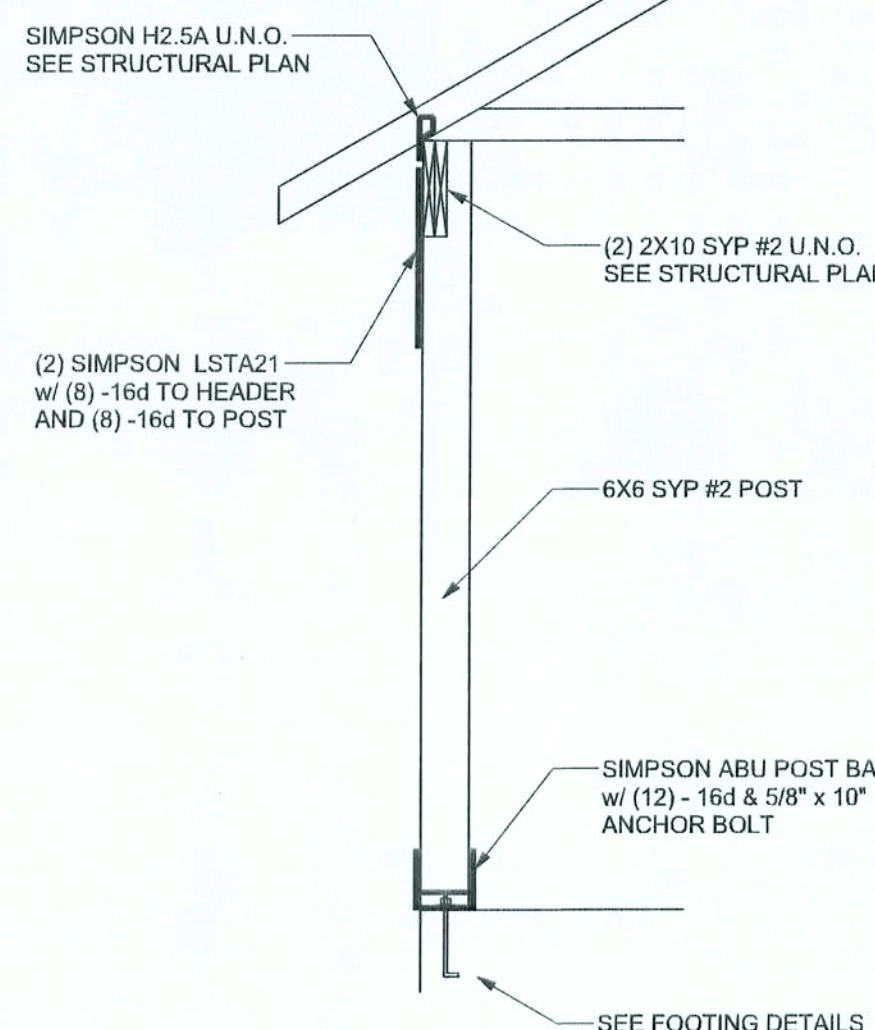


**BEAM CORNER CONNECTION DETAIL**  
SCALE: N.T.S.

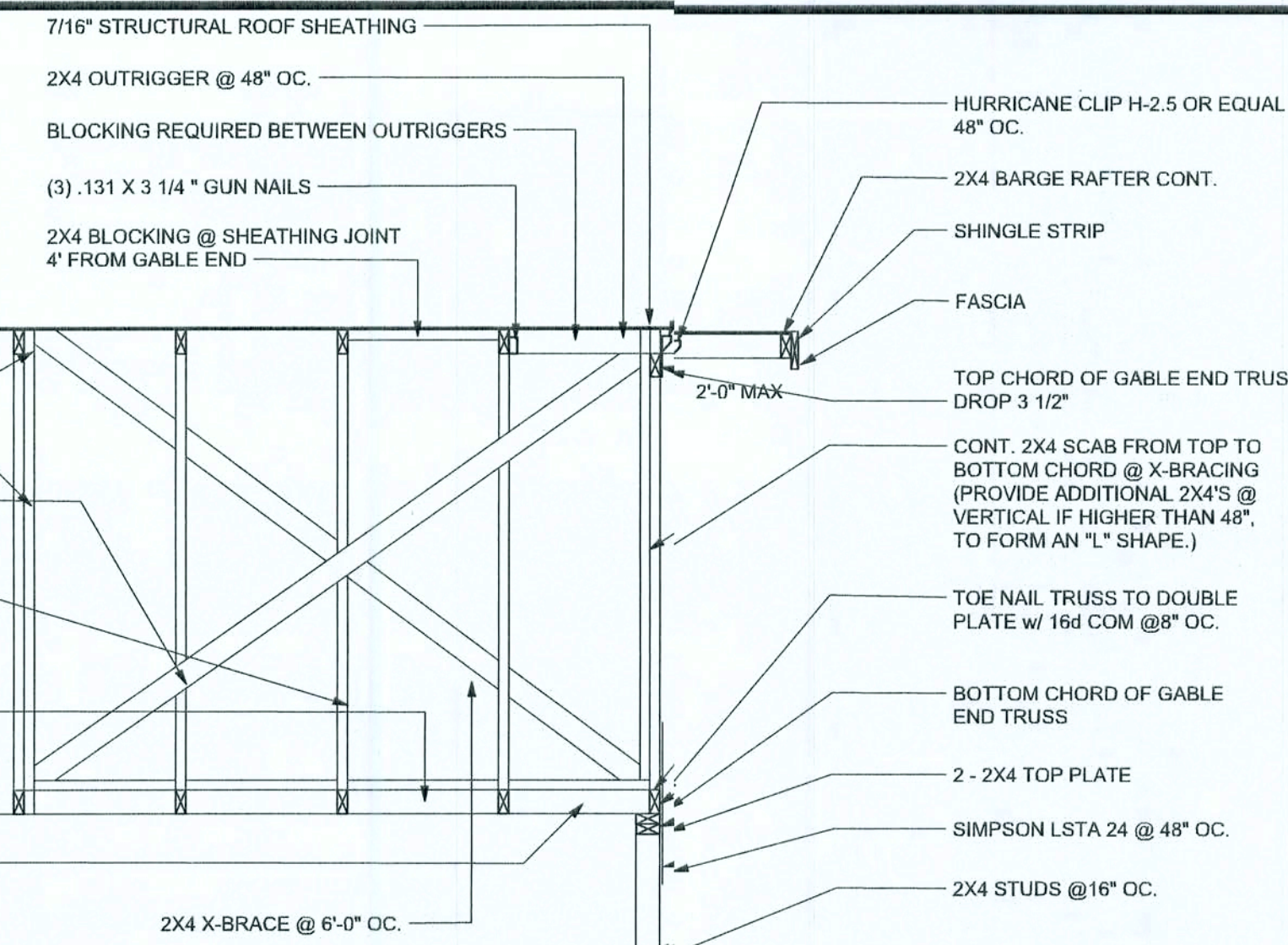
**SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM**  
SCALE: N.T.S.



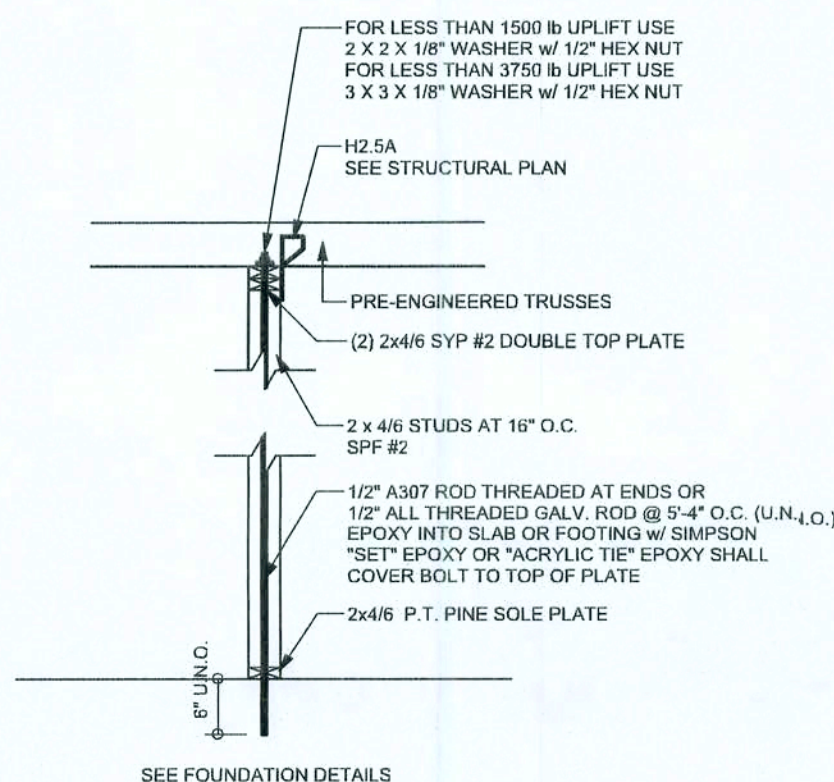
**SUPPORTIVE CENTER POST TO BEAM DETAIL**  
SCALE: N.T.S.



**TYPICAL PORCH POST DETAIL**  
SCALE: 1/2" = 1'-0"

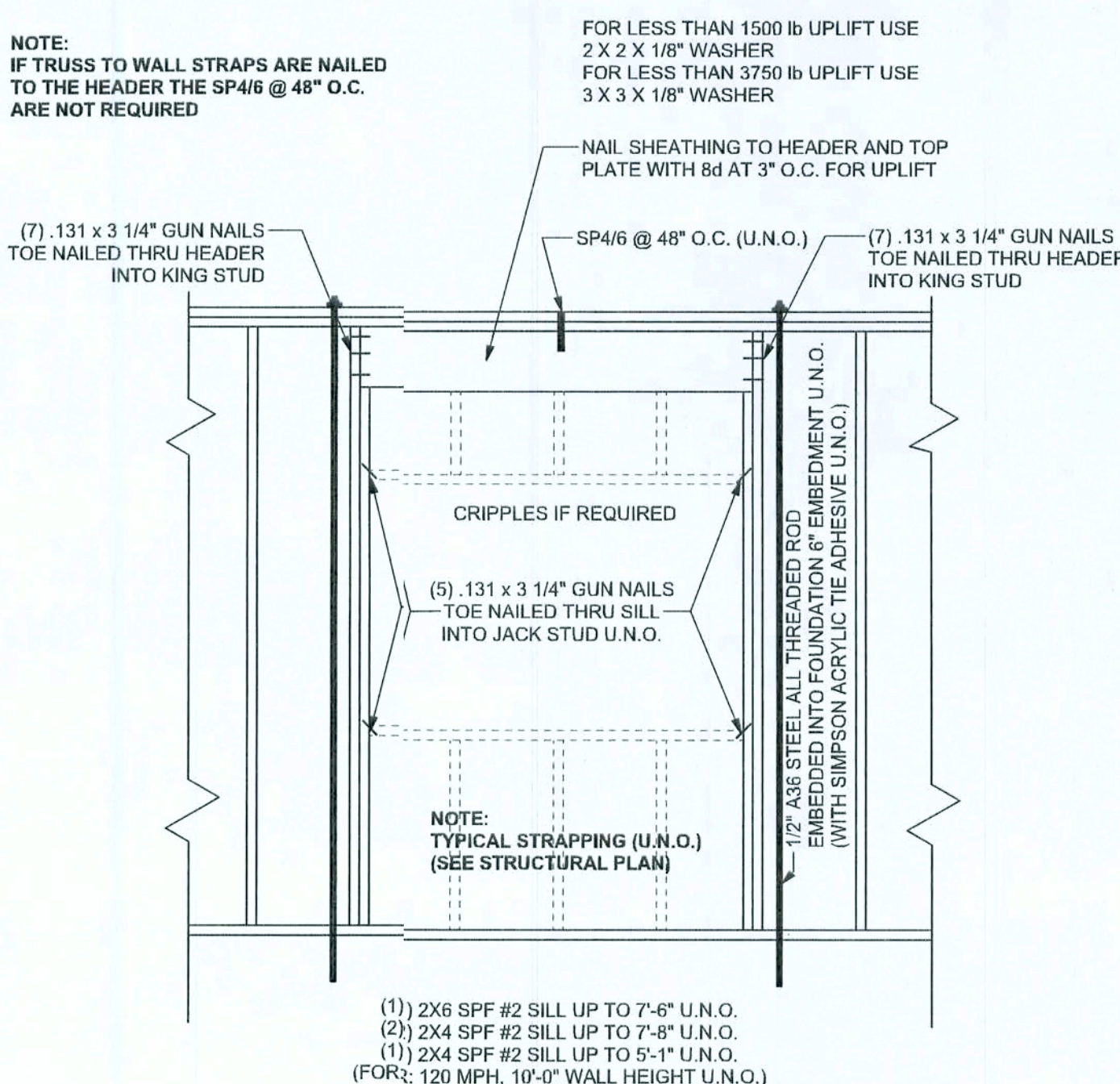


**TYPICAL GABLE END (X-BRACING)**  
ALL MEMBERS SHALL BE SYP



**INTERIOR BEARING WALL**  
SCALE: 1/2" = 1'-0"

**NOTE:**  
IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SP4/6 @ 48" O.C. ARE NOT REQUIRED



**TYPICAL 1-STORY HEADER STRAPPING DETAIL**  
SCALE: 1/2" = 1'-0"

### ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2- HTS24			
< 2050	< 1785	LGT2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					
< 3965	< 3330	MGT		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*					
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	LTT16	18-16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAH022	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

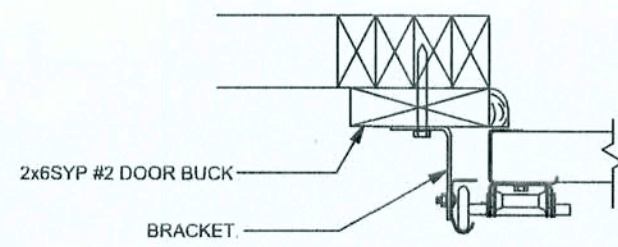
### GRADE & SPECIES TABLE

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2800	2.0
PSL	PARALAM	2800	2.0

### 2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL LAGS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" GN PER TABLE BELOW:

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8'-10"	24" O.C.	5" O.C.	5" O.C.
11'-18"	18" O.C.	4" O.C.	4" O.C.
16'-18"	16" O.C.	3" O.C.	3" O.C.



**GARAGE DOOR BUCK INSTALLATION DETAIL**  
SCALE: N.T.S.

### GENERAL NOTES:

**TRUSSES:** TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCE 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

**SITE PREPARATION:** SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

**FOUNDATION:** CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 3000 PSI.

**WELDED WIRE REINFORCED SLAB:** 6" x 6" W1 x W1.4, FB = 85KSI. WELDED REINFORCEMENT FABRIC (W1.4) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SHEATHING WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 7'.

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF SLAB AREA SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12'-0". DO NOT CUT JMW OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

**REBAR:** ASTM A 615, GRADE 60, DEFORMED BARS, F<sub>y</sub> = 60 KSI; ALL LAP SPLICES 40" DBL (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

**GLULAM BEAMS:** GLULAM BEAM, GLB, 24F-V3SP, F<sub>b</sub> = 2480, E = 1800ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC. **ROOF SHEATHING:** ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, STAGGERED, FASTENED WITH 8d COMMON NAILS (13d), 12" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4" OC, UNO.

**STRUCTURAL CONNECTORS:** MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENCOURAGEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

**ANCHOR BOLTS:** A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 9/16"; UNO.

**NAILS:** ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBCE TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

### BUILDER'S RESPONSIBILITY

**THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.**

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCE 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDING TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

### ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCE 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCE 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

### DESIGN DATA

#### WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 80FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50X HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH-VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B

3.) WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft <sup>2</sup> )	10	100
1	19.9 - 21.8	18.1	-18.1
2	19.9 - 25.5	18.1	-21.8
2 Onh	-40.6	-40.6	-40.6
3	19.9 - 25.5	18.1	-21.8
3 Onh	-68.3	-42.4	-42.4
4	21.8 - 23.6	18.5	-20.4
5	21.8 - 29.1	18.5	-22.6
Doors & Windows		21.8	-29.1
8x7 Garage Door	19.5	-22.9	
16x7 Garage Door	18.5	-21.0	

#### DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (SLEEPING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

ROOF 20 PSF (FLAT OR <4:12)

16 PSF (4:12 TO <12:12)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

### REVISIONS


**SOTPLAN**  
ARCHITECTURE DESIGN INC. (FIRM)

WINDLOAD ENGINEER: Mark Disoway  
P.E. No. 53915, P.C. 868, Lake City, FL  
32055, 386-754-810

**DIMENSIONS:** Stated dimension supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53915

*Mark Disoway*  
11/2/2006  
SEAL

### Woodman Park Builders

Am Daniels  
Residence

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PRINTED DATE:  
April 19, 2006

DRAWN BY: STRUCTURAL BY:  
David Disoway

FINALS DATE:  
18 / Apr / 06

JOB NUMBER:  
604121

DRAWING NUMBER

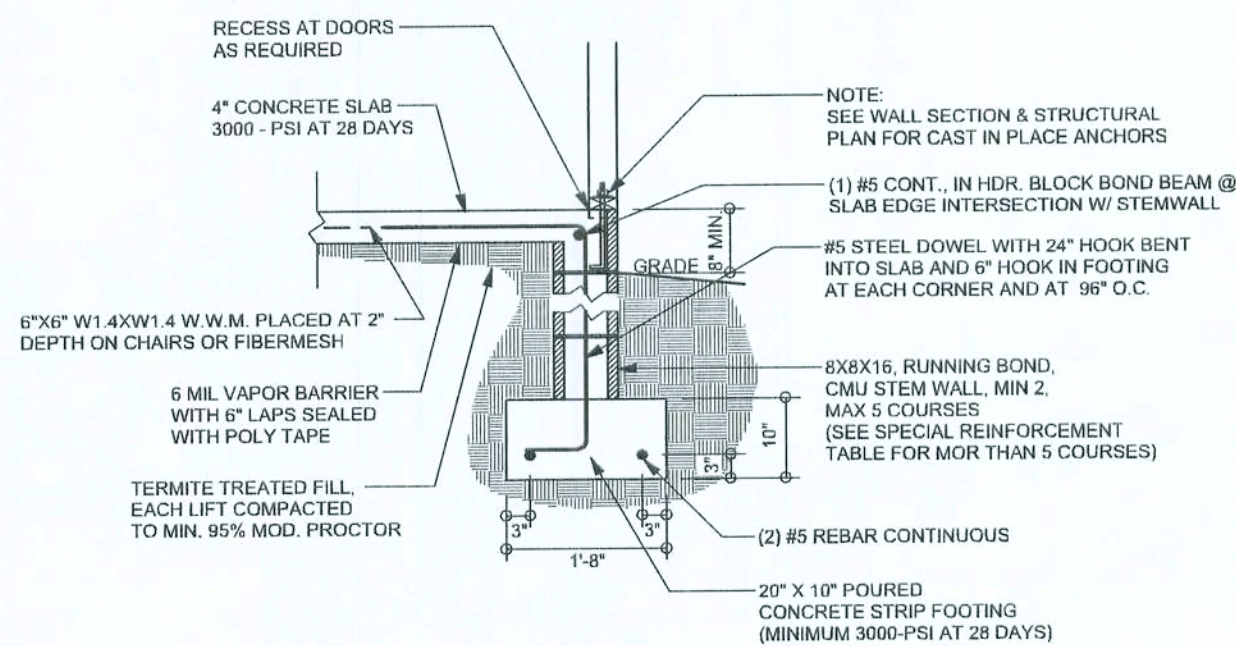
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OF 3 SHEETS

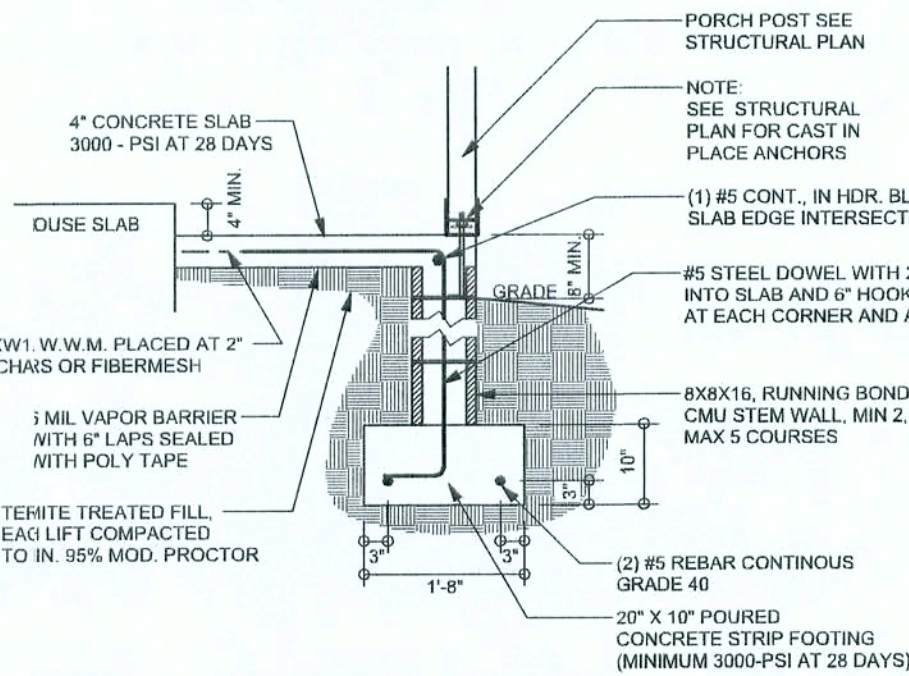


REVISIONS

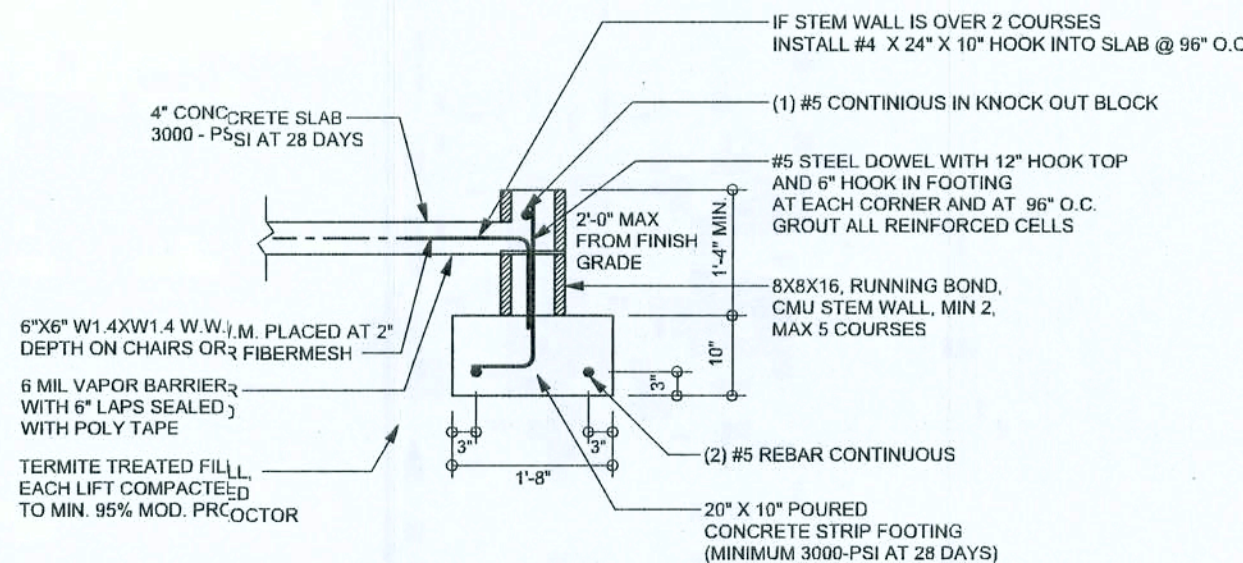
SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



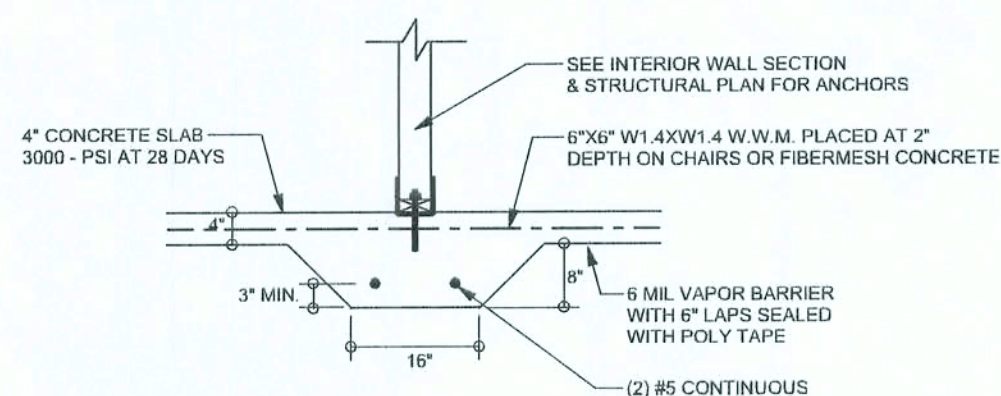
F9  
S-2 STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"



F12  
S-2 ALT. STEM WALL PORCH FOOTING  
SCALE: 1/2" = 1'-0"



F18  
S-2 STEM WALL CURB FOOTING  
SCALE: 1/2" = 1'-0"

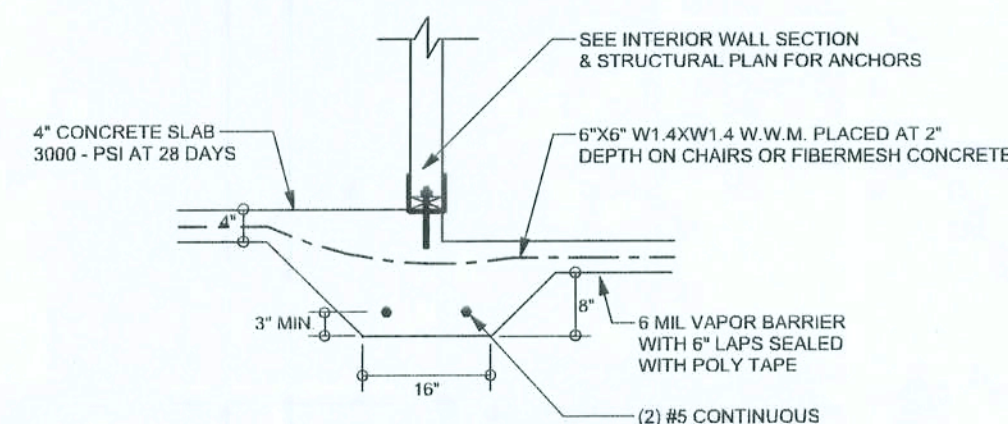


F2  
S-2 INTERIOR BEARING FOOTING  
SCALE: 1/2" = 1'-0"

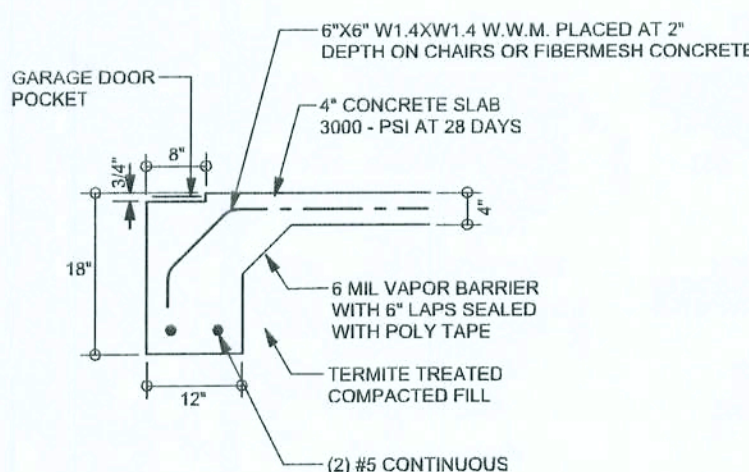
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" O.C. vertically or a horizontal bond beam with 195 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

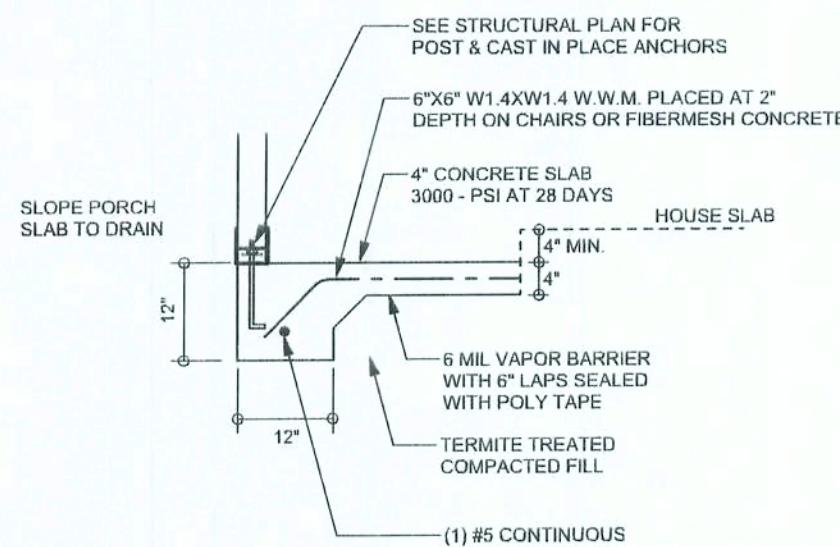
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



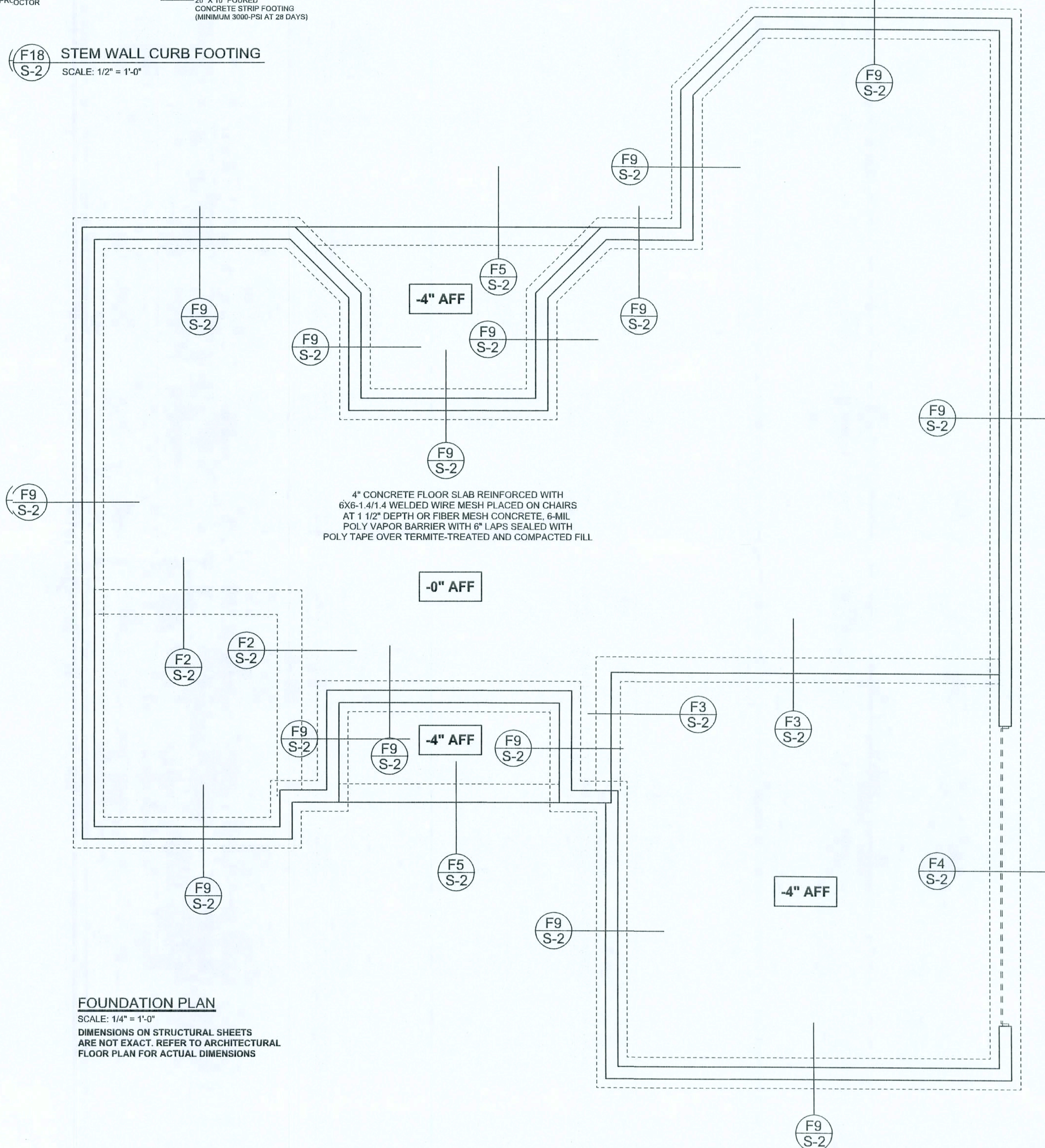
F3  
S-2 INTERIOR BEARING STEP FOOTING  
SCALE: 1/2" = 1'-0"



F4  
S-2 GARAGE DOOR FOOTING  
SCALE: 1/2" = 1'-0"



F5  
S-2 PORCH FOOTING  
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS  
ARE NOT EXACT. REFER TO ARCHITECTURAL  
FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Ask Disoway,  
P.E. No. 33915, P.O. Box 868, Lake City, FL  
32055, 386-754-5419

DIMENSIONS:  
Stated dimensions supersede scaled  
dimensions. Refer all questions to  
Mark Disoway, P.E. for resolution.  
Do not proceed without clarification.

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permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have  
examined this plan, and that the applicable  
portions of the plan, relating to wind engineering  
comply with section F3212.1, Florida building  
code residential 2004, to the best of my  
knowledge.

LIMITATION: This design is valid for one  
building, at specified location.

MARK DISOWAY  
P.E. 3315

SEA

Woodman Park Builders

Ann Daniels  
Residence

ADDRESS:  
Lot 64 Emerald Cove S/D  
Columbia County, Florida

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P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE:  
April 19, 2006

DRAWN BY: STRUCTURAL BY:  
David Disoway

FINALS DATE:  
18 / Apr / 06

JOB NUMBER:  
604121

DRAWING NUMBER

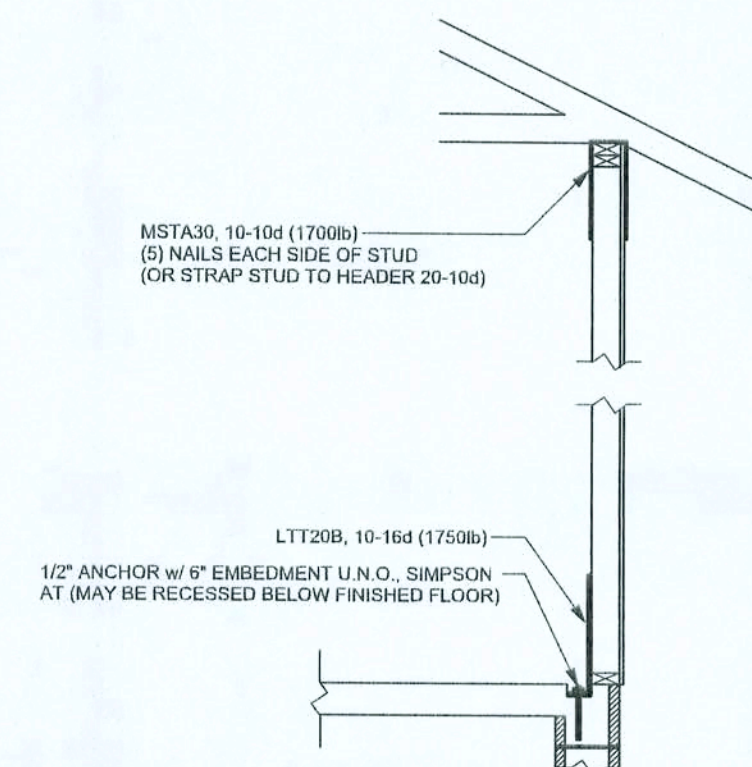
S-2

OF 3 SHEETS

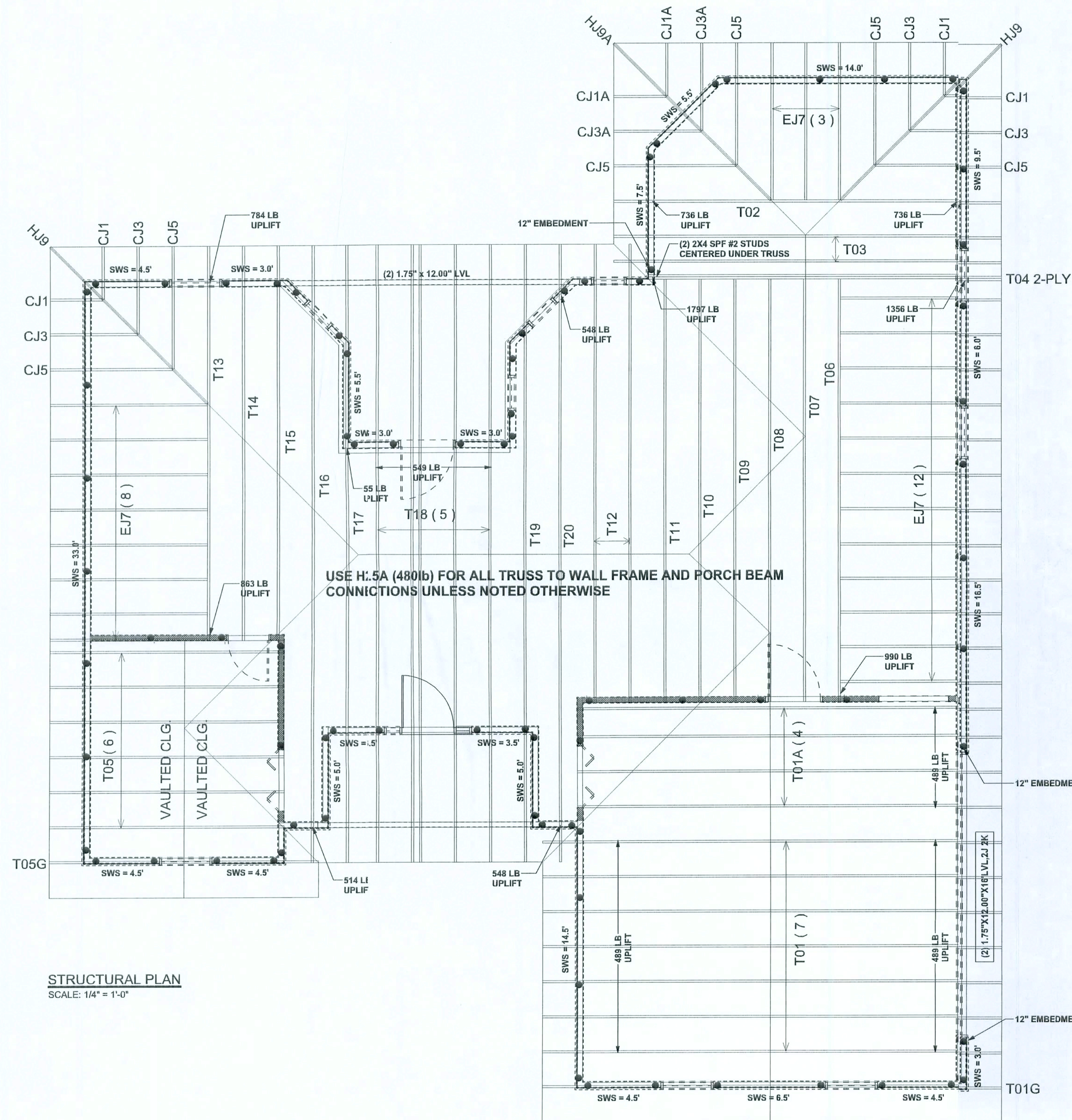


# REVISIONS

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



ALTERNATE WALL TIE CONNECTION WHERE  
THREADED ROD CANNOT BE PLACED IN WALL.  
SCALE: 1/2" = 1'-0"



STRUCTURAL PLAN  
SCALE: 1/4" = 1'-0"

## STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

## WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

## THREADED ROD LEGEND

- INDICATES LOCATION OF:  
1ST FLOOR 1/2" A307 ALL THREADED ROD
- ⊗ INDICATES LOCATION OF:  
2ND FLOOR 1/2" A307 ALL THREADED ROD

## HEADER LEGEND

- (2) 2X12X0', 1J 1K → HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

## TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	34.5'	105.5'
LONGITUDINAL	32.5'	64.5'

WINDLOAD ENGINEER: Mark Disosway,  
P.E. No. 53915, P.O. Box 868, Lake City, FL  
32056, 386-7545415

DIMENSIONS:  
Stated dimensions supersede scaled  
dimensions. Refer all questions to  
Mark Disosway, P.E. for resolution.  
Do not proceed without clarification.

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permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have  
examined this plan, and that the applicable  
portions of the plan, relating to wind engineering  
comply with section F301.2.1, Florida building  
code residential2004, to the best of my  
knowledge.

LIMITATION: This design is valid for one  
building, at specified location.

MARK DISOSWAY  
P.E. 53915

*Mark Disosway*  
4 APR 06  
SEAL

## Woodm Park Builders

Am Daniels  
Residence

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DRAWING NUMBER

S-3

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED  
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING  
FURNISHED BY BUILDER. BUILDERS FIRST SOURCE  
JOB #L158385