

Julius Lee Engineering

RE: 308959 - RICHARD KEEN - LOT 30 EE

**1109 Coastal Bay Blvd.
Boynton Beach, FL 33435**

Site Information:

Project Customer: RICHARD KEEN Project Name: 308959 Model: CUSTOM
Lot/Block: 30 Subdivision: EDGEWOOD EST.
Address:
City: COLUMBIA CTY State: FL

Name Address and License # of Structural Engineer of Record, if there is one, for the building.

Name: JAMES H. JOHNSTON III License #: CRC1328128
Address: 650 SW MAIN BLVD.
City: LAKE CITY State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.1
Wind Code: ASCE 7-05 Wind Speed: 110 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 28 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

| No. | Seal# | Truss Name | Date | No. | Seal# | Truss Name | Date |
|-----|----------|------------|---------|-----|----------|------------|---------|
| 1 | I4046145 | CJ1 | 6/29/09 | 18 | I4046162 | T11 | 6/29/09 |
| 2 | I4046146 | CJ3 | 6/29/09 | 19 | I4046163 | T12 | 6/29/09 |
| 3 | I4046147 | CJ5 | 6/29/09 | 20 | I4046164 | T13 | 6/29/09 |
| 4 | I4046148 | EJ5 | 6/29/09 | 21 | I4046165 | T14 | 6/29/09 |
| 5 | I4046149 | EJ7 | 6/29/09 | 22 | I4046166 | T15 | 6/29/09 |
| 6 | I4046150 | HJ7 | 6/29/09 | 23 | I4046167 | T16 | 6/29/09 |
| 7 | I4046151 | HJ9 | 6/29/09 | 24 | I4046168 | T17 | 6/29/09 |
| 8 | I4046152 | T01 | 6/29/09 | 25 | I4046169 | T18 | 6/29/09 |
| 9 | I4046153 | T02 | 6/29/09 | 26 | I4046170 | T19 | 6/29/09 |
| 10 | I4046154 | T03 | 6/29/09 | 27 | I4046171 | T20 | 6/29/09 |
| 11 | I4046155 | T04 | 6/29/09 | 28 | I4046172 | T21 | 6/29/09 |
| 12 | I4046156 | T05 | 6/29/09 | | | | |
| 13 | I4046157 | T06 | 6/29/09 | | | | |
| 14 | I4046158 | T07 | 6/29/09 | | | | |
| 15 | I4046159 | T08 | 6/29/09 | | | | |
| 16 | I4046160 | T09 | 6/29/09 | | | | |
| 17 | I4046161 | T10 | 6/29/09 | | | | |

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



| | | | | | |
|--|--------------|--------------------|-----------|----------|--|
| Job 308959 | Truss CJ1 | Truss Type JACK | Qty 14 | Ply 1 | RICHARD KEEN - LOT 30 EE Job Reference (optional) 7.130 s Apr 28 2009 Mitek Industries, Inc. Mon Jun 29 07:09:32 2009 Page 1 |
| Builders FrstSource, Lake City, FL 32055 | | | | | 14046145 |

| | | | | |
|---|---|---|--|---|
| LOADING (psf) TCCL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0 | SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2007/TPI2002 | CSI TC 0.34 BC 0.01 WB 0.00 (Matrix) | DEFL in (loc) l/defl L/d Vert(LL) -0.00 2 >999 360 Vert(TL) -0.00 2 >999 240 Horz(TL) 0.00 3 n/a n/a Wind(LL) 0.00 2 >999 240 | PLATES MT20 GRIP 244/190 Weight: 7 lb |
|---|---|---|--|---|

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 2=256/0-1-8 (input: 0-3-8), 4=5/Mechanical, 3=90/Mechanical
 Max Horz 2=105(LC 6)
 Max Uplift 2=355(LC 6), 4=12(LC 4), 3=90(LC 1)
 Max Grav 2=256(LC 1), 4=14(LC 2), 3=157(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

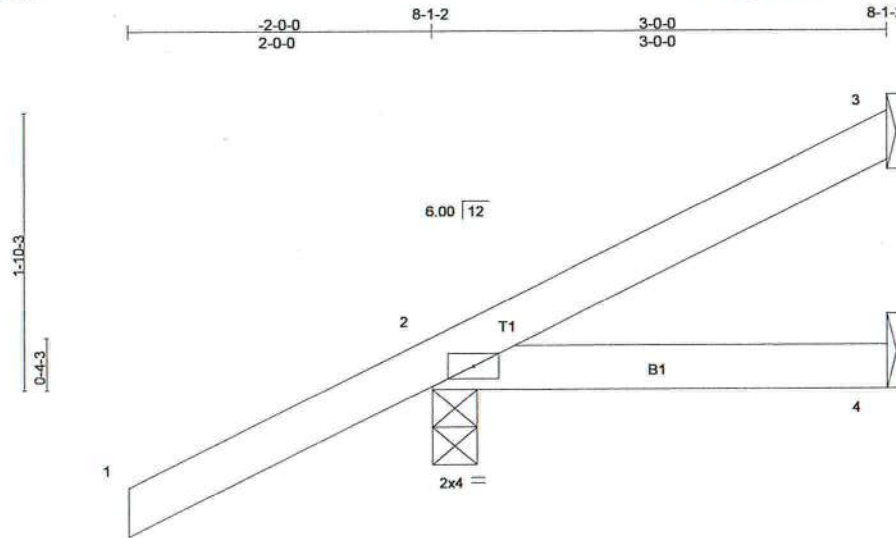
NOTES (8-9)
 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SYP No.2.
 5) Refer to girder(s) for truss to truss connections.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 355 lb uplift at joint 2, 12 lb uplift at joint 4 and 90 lb uplift at joint 3.
 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING
 TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

June 29, 2009



| LOADING (psf) | SPACING | 2-0-0 | CSI | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---------------|----------------------|-------|----------|----------|-------|-------|--------|-----|---------------|---------|
| TCLL 20.0 | Plates Increase | 1.25 | TC 0.36 | Vert(LL) | -0.00 | 2-4 | >999 | 360 | MT20 | 244/190 |
| TCDL 7.0 | Lumber Increase | 1.25 | BC 0.10 | Vert(TL) | -0.01 | 2-4 | >999 | 240 | | |
| BCLL 0.0 * | Rep Stress Incr | YES | WB 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL 5.0 | Code FBC2007/TPI2002 | | (Matrix) | Wind(LL) | 0.01 | 2-4 | >999 | 240 | | |
| | | | | | | | | | Weight: 13 lb | |

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

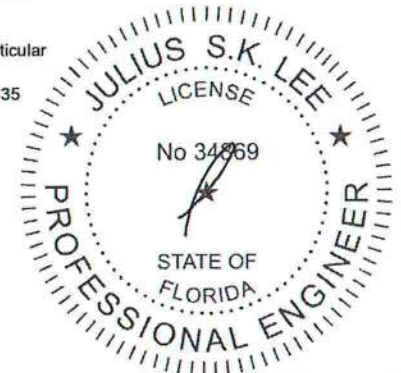
REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-1-8 (input: 0-3-8), 4=14/Mechanical
Max Horz 2=159(LC 6)
Max Uplift 3=35(LC 7), 2=298(LC 6), 4=34(LC 4)
Max Grav 3=31(LC 1), 2=250(LC 1), 4=42(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

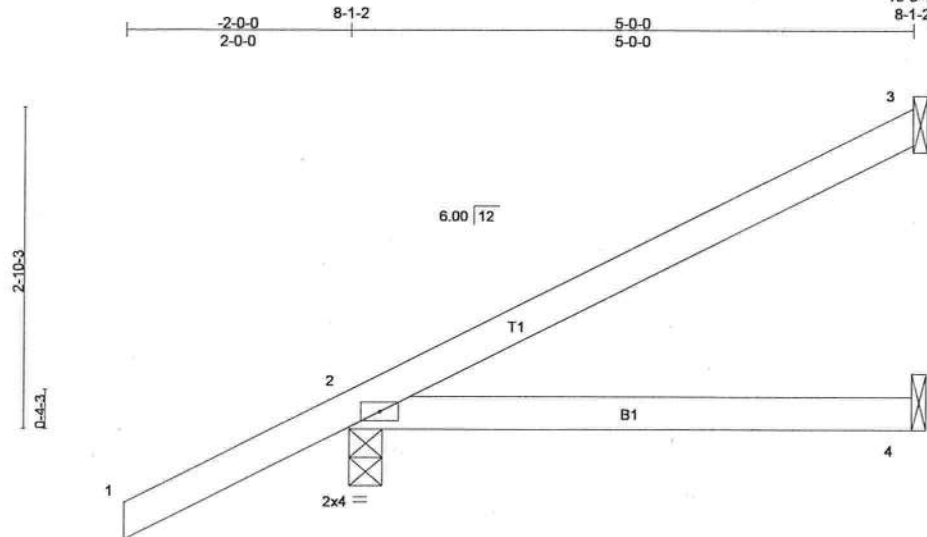
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 3, 298 lb uplift at joint 2 and 34 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



June 29, 2009

| | | | | | | |
|--------|-------|------------|-----|-----|--------------------------|----------|
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - LOT 30 EE | I4046147 |
| 308959 | CJ5 | JACK | 12 | 1 | Job Reference (optional) | |



| | | | | | | | |
|----------------------|----------------------|------------|----------------------|---------------|------------|---------------|-------------|
| LOADING (psf) | SPACING 2-0-0 | CSI | DEFL in (loc) | l/defl | L/d | PLATES | GRIP |
| TCLL 20.0 | Plates Increase 1.25 | TC 0.36 | Vert(LL) -0.03 2-4 | >999 | 360 | MT20 | 244/190 |
| TCDL 7.0 | Lumber Increase 1.25 | BC 0.30 | Vert(TL) -0.05 2-4 | >999 | 240 | | |
| BCLL 0.0 * | Rep Stress Incr YES | WB 0.00 | Horz(TL) -0.00 3 | n/a | n/a | | |
| BCDL 5.0 | Code FBC2007/TPI2002 | (Matrix) | Wind(LL) 0.11 2-4 | >547 | 240 | Weight: 19 lb | |

Structural wood sheathing directly applied or 5-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

June 29, 2009

WARNING - verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MH-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BC311 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

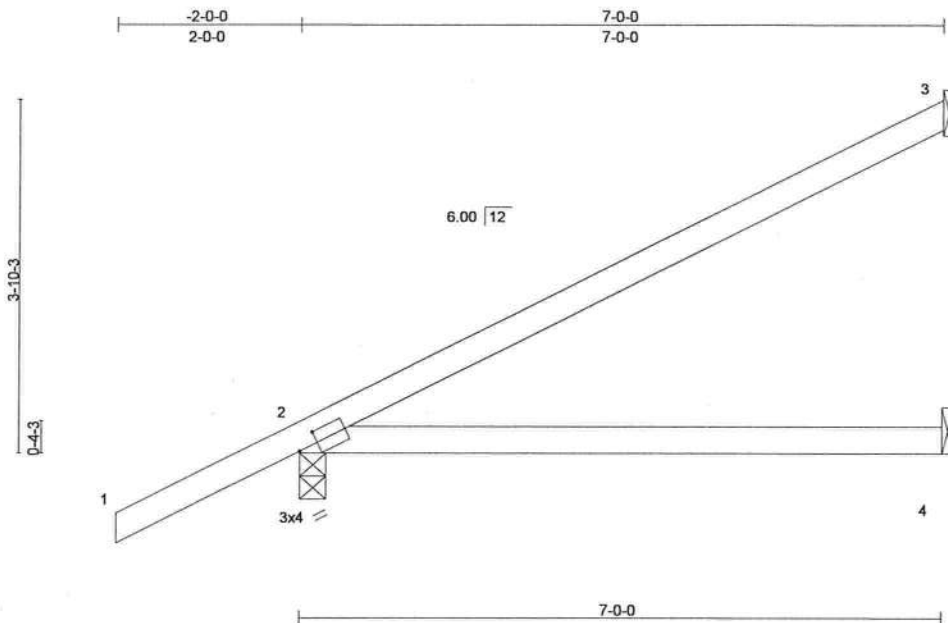
Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

| | | | | | |
|--------|-------|------------|-----|-----|--------------------------|
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - LOT 30 EE |
| 308959 | EJ7 | JACK | 33 | 1 | Job Reference (optional) |

Builders FrstSource, Lake City, FL 32055

7.110 s Dec 8 2008 MiTek Industries, Inc. Mon Jun 29 11:42:21 2009 Page 1



Scale: 1/2"=1'

Plate Offsets (X,Y): [2-0-2-10,0-1-8]

| LOADING (psf) | SPACING | 2-0-0 | CSI | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
|---------------|----------------------|-------|----------|----------|-------|-------|--------|-----|---------------|---------|
| TCLL 20.0 | Plates Increase | 1.25 | TC 0.62 | Vert(LL) | -0.09 | 2-4 | >921 | 360 | MT20 | 244/190 |
| TCDL 7.0 | Lumber Increase | 1.25 | BC 0.57 | Vert(TL) | -0.17 | 2-4 | >476 | 240 | | |
| BCLL 0.0 | Rep Stress Incr | YES | WB 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL 5.0 | Code FBC2007/TPI2002 | | (Matrix) | Wind(LL) | 0.40 | 2-4 | >203 | 240 | | |
| | | | | | | | | | Weight: 26 lb | |

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=157/Mechanical, 2=352/0-1-8 (input: 0-3-8), 4=42/Mechanical
Max Horz 2=195(LC 6)
Max Uplift 3=119(LC 6), 2=287(LC 6), 4=83(LC 5)
Max Grav 3=157(LC 1), 2=352(LC 1), 4=96(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (7-8)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 3, 287 lb uplift at joint 2 and 83 lb uplift at joint 4.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



June 29,2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

| | | | | | | |
|---|--------------|--------------------------|----------|----------|--|----------|
| Job 308959 | Truss HJ7 | Truss Type MONO TRUSS | Qty 1 | Ply 1 | RICHARD KEEN - LOT 30 EE Job Reference (optional) | 14046150 |
| Builders FirstSource, Lake City, FL 32055 | | | | | 7.130 s Apr 28 2009 MITek Industries, Inc. Mon Jun 29 07:08:34 2009 Page 1 | |

| | | | | |
|---|--|---|--|---|
| LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 5.0 | SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2007/TPI2002 | CSI TC 0.59 BC 0.29 WB 0.00 (Matrix) | DEFL in (loc) l/defl L/d Vert(LL) -0.09 2-4 >934 360 Vert(TL) -0.15 2-4 >553 240 Horz(TL) -0.00 3 n/a n/a Wind(LL) -0.03 2-4 >999 240 | PLATES GRIP MT20 244/190 Weight: 26 lb |
|---|--|---|--|---|

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 7-0-14 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=184/Mechanical, 2=338/0-1-8 (input: 0-5-11), 4=38/Mechanical

Max Horz 2=202(LC 3)

Max Uplift 3=180(LC 3), 2=316(LC 3)

Max Grav 3=184(LC 1), 2=338(LC 1), 4=99(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (9-10)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2 .
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 3 and 316 lb uplift at joint 2.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

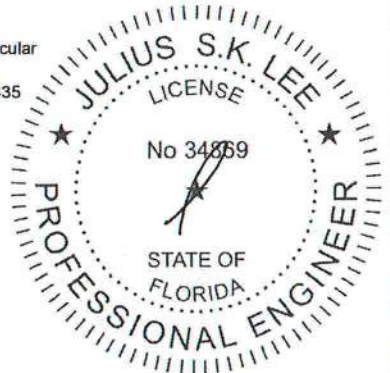
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=54

Trapezoidal Loads (plf)

Vert: 2=3(F=25, B=25)-to-3=95(F=21, B=21), 2=0(F=5, B=5)-to-4=18(F=4, B=4)



June 29, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

| | | | | | |
|--------|-------|------------|-----|-----|--------------------------|
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - LOT 30 EE |
| 308959 | HJ9 | MONO TRUSS | 6 | 1 | |

Job Reference (optional)

7.130 s Apr 28 2009 MiTek Industries, Inc. Mon Jun 29 07:09:35 2009 Page 1

Builders FrstSource, Lake City, FL 32055

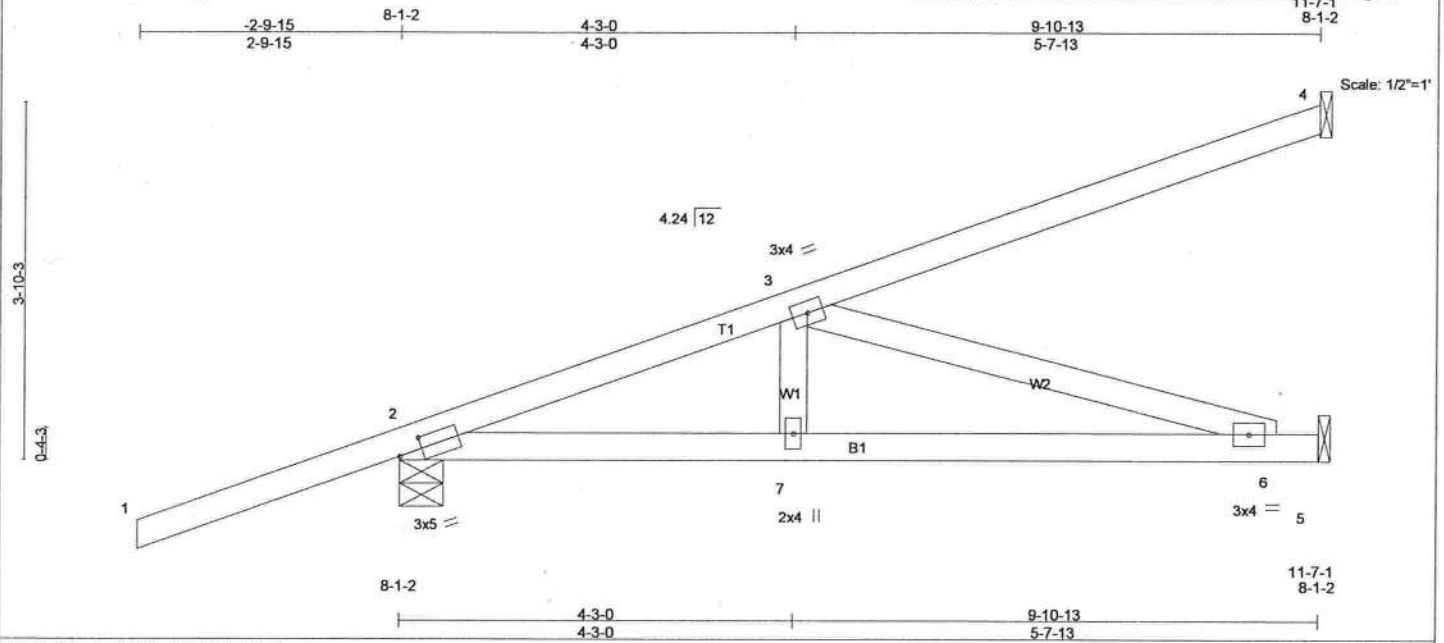


Plate Offsets (X,Y): [2:0-3-1,0-1-8]

| LOADING (psf) | SPACING | 2-0-0 | CSI | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---------------|----------------------|-------|----------|----------|-------|-------|--------|-----|---------------|---------|
| TCLL 20.0 | Plates Increase | 1.25 | TC 0.61 | Vert(LL) | -0.04 | 6-7 | >999 | 360 | MT20 | 244/190 |
| TCDL 7.0 | Lumber Increase | 1.25 | BC 0.35 | Vert(TL) | -0.11 | 6-7 | >999 | 240 | | |
| BCLL 0.0 | Rep Stress Incr | NO | WB 0.34 | Horz(TL) | 0.01 | 5 | n/a | n/a | | |
| BCDL 5.0 | Code FBC2007/TPI2002 | | (Matrix) | Wind(LL) | 0.05 | 6-7 | >999 | 240 | | |
| | | | | | | | | | Weight: 45 lb | |

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 9-2-9 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=269/Mechanical, 2=456/0-1-8 (input: 0-5-11), 5=216/Mechanical
Max Horz 2=325(LC 3)
Max Uplift 4=292(LC 3), 2=358(LC 3), 5=88(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

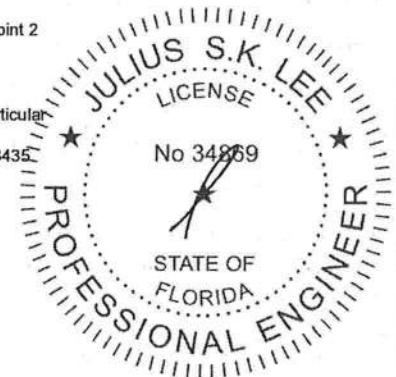
TOP CHORD 2-3=644/180
BOT CHORD 2-7=400/590, 6-7=400/590
WEBS 3-6=615/418

NOTES (9-10)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 4, 358 lb uplift at joint 2 and 88 lb uplift at joint 5.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=54
Trapezoidal Loads (plf)
Vert: 2=3(F=25, B=25)-to-4=134(F=40, B=40), 2=0(F=5, B=5)-to-5=25(F=7, B=7)



June 29, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

| | | | | | |
|--|--------------|-------------------|----------|----------|--|
| Job 308959 | Truss T01 | Truss Type HIP | Qty 1 | Ply 1 | RICHARD KEEN - LOT 30 EE Job Reference (optional) 7.130 s Apr 28 2009 MiTek Industries, Inc. Mon Jun 29 07:09:37 2009 Page 1 |
| Builders FrstSource, Lake City, FL 32055 | | | | | I4046152 |

Scale = 1:43.4

| | | | | | |
|---|----------------------|----------|------------------------------|---------------|---------|
| Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-1-11,Edge] | | | | | |
| LOADING (psf) | SPACING | CSI | DEFL | PLATES | GRIP |
| TCLL 20.0 | 2-0-0 | TC 0.53 | in (loc) l/defl L/d | MT20 | 244/190 |
| TCDL 7.0 | Plates Increase 1.25 | BC 0.60 | Vert(LL) -0.11 8-10 >999 360 | | |
| BCLL 0.0 * | Lumber Increase 1.25 | WB 0.27 | Vert(TL) -0.32 8-10 >819 240 | | |
| BCDL 5.0 | Rep Stress Incr NO | (Matrix) | Horz(TL) 0.09 6 n/a n/a | | |
| | Code FBC2007/TPI2002 | | Wind(LL) 0.12 8-10 >999 240 | | |
| | | | | Weight: 99 lb | |

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 2=1521/0-1-13 (input: 0-3-8), 6=1521/0-1-13 (input: 0-3-8)

Max Horz 2=94(LC 5)

Max Uplift 2=690(LC 5), 6=690(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2686/1101, 3-4=-2340/1028, 4-5=-2340/1028, 5-6=-2686/1101

BOT CHORD 2-10=-937/2307, 9-10=-1106/2602, 8-9=-1106/2602, 6-8=-897/2307

WEBS 3-10=-340/842, 4-10=-448/303, 4-8=-448/303, 5-8=-339/842

NOTES (12-13)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 690 lb uplift at joint 2 and 690 lb uplift at joint 6.
- Girder carries hip end with 7-0-0 end setback.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 411 lb down and 220 lb up at 15-0-0, and 411 lb down and 220 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

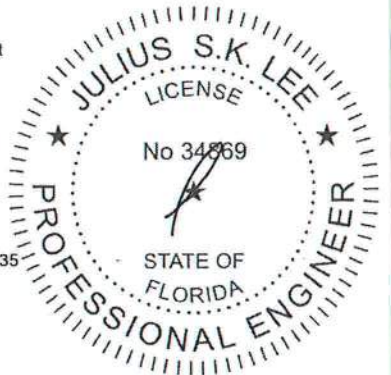
Vert: 1-3=-54, 3-5=-118(F=-64), 5-7=-54, 2-10=-10, 8-10=-22(F=-12), 6-8=-10

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-4-6 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-0-1 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



June 29, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Orafrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

| | | | | | | |
|--------|-------|------------|-----|-----|--------------------------|----------|
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - LOT 30 EE | I4046152 |
| 308959 | T01 | HIP | 1 | 1 | Job Reference (optional) | |

Builders FirstSource, Lake City, FL 32055

7.130 s Apr 28 2009 Mitek Industries, Inc. Mon Jun 29 07:09:37 2009 Page 2

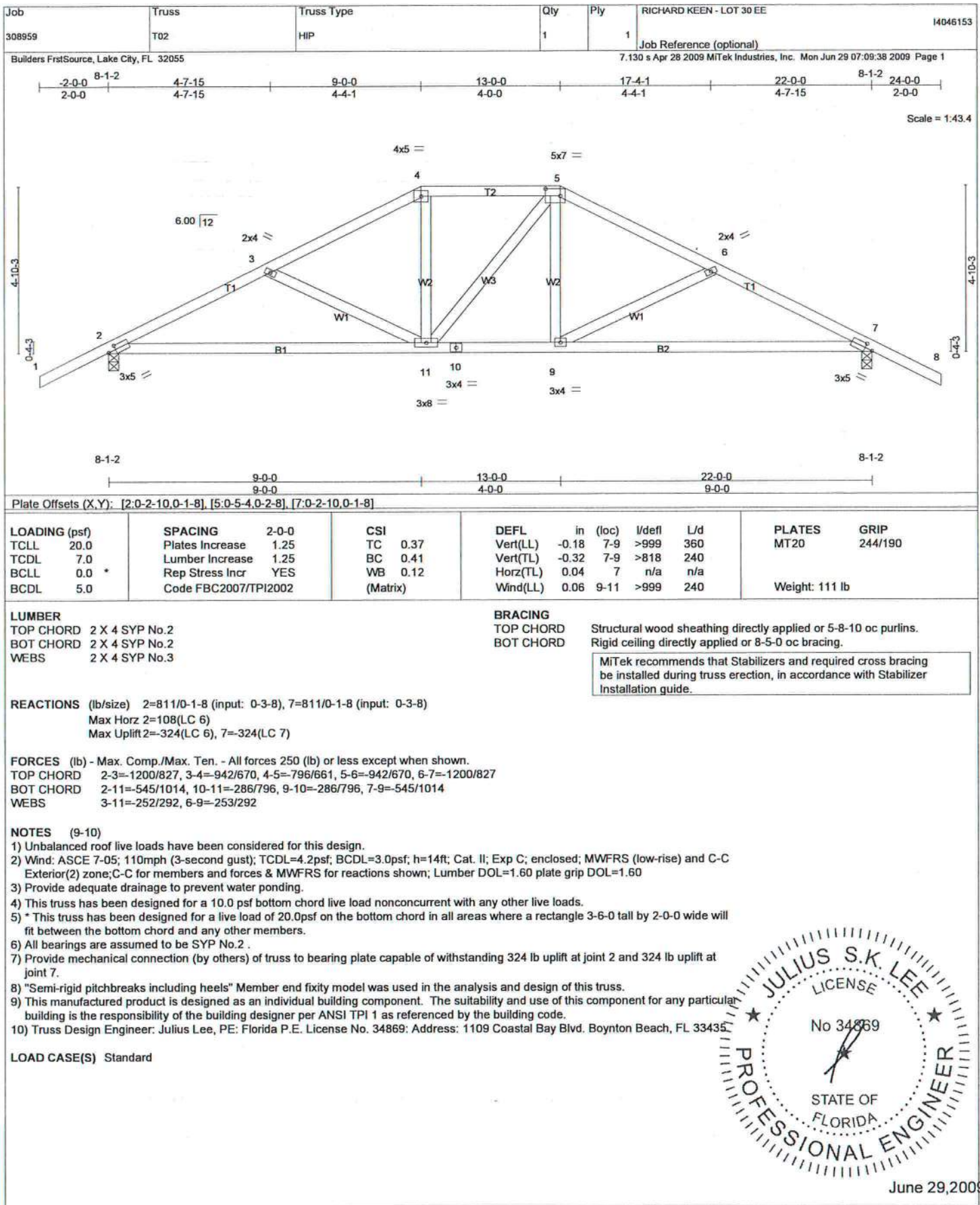
LOAD CASE(S) Standard
Concentrated Loads (lb)
Vert: 10=-411(F) 8=-411(F)



June 29, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DS8-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435



June 29,2009

| | | | | | | |
|--------|-------|------------|-----|-----|--------------------------|----------|
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - LOT 30 EE | 14046154 |
| 308959 | T03 | COMMON | 5 | 1 | Job Reference (optional) | |

Builders FirstSource, Lake City, FL 32055

7.130 s Apr 28 2009 MiTek Industries, Inc. Mon Jun 29 07:09:39 2009 Page 1

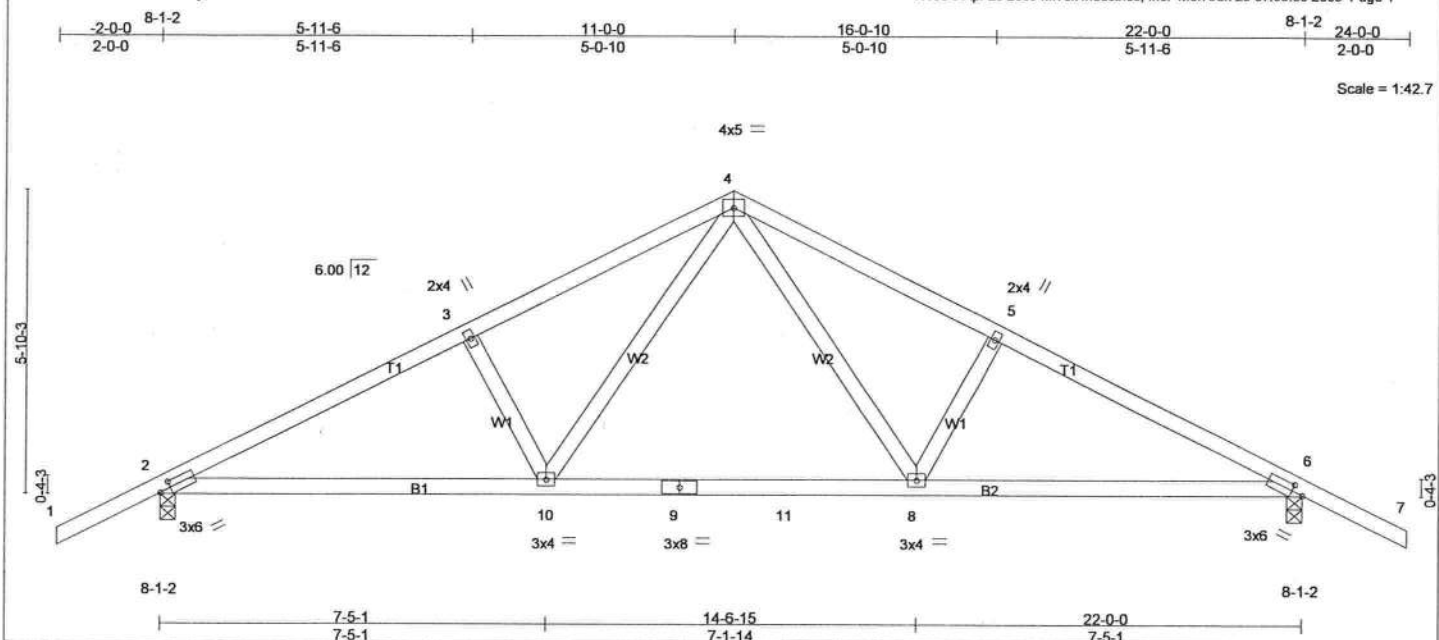


Plate Offsets (X,Y): [2-0-2-10,0-1-8], [6-0-2-10,0-1-8]

| LOADING (psf) | SPACING | 2-0-0 | CSI | DEFL | in (loc) | l/defl | L/d | PLATES | GRIP |
|---------------|----------------------|-------|----------|----------|----------|--------|------|----------------|---------|
| TCLL 20.0 | Plates Increase | 1.25 | TC 0.45 | Vert(LL) | -0.31 | 8-10 | >851 | MT20 | 244/190 |
| TCDL 7.0 | Lumber Increase | 1.25 | BC 0.78 | Vert(TL) | -0.44 | 8-10 | >587 | | |
| BCLL 0.0 | Rep Stress Incr | NO | WB 0.31 | Horz(TL) | 0.05 | 6 | n/a | | |
| BCDL 5.0 | Code FBC2007/TPI2002 | | (Matrix) | Wind(LL) | 0.22 | 8-10 | >999 | | |
| | | | | | | | | Weight: 104 lb | |

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 4-7-1 oc purlins.
Rigid ceiling directly applied or 7-4-1 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1065/0-1-8 (input: 0-3-8), 6=1065/0-1-8 (input: 0-3-8)

Max Horz 2=122(LC 6)

Max Uplift 2=387(LC 6), 6=387(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=1788/1034, 3-4=1643/1051, 4-5=1643/1051, 5-6=1788/1034

BOT CHORD 2-10=714/1521, 9-10=348/1037, 9-11=348/1037, 8-11=348/1037, 6-8=714/1521

WEBS 3-10=250/316, 4-10=405/698, 4-8=405/698, 5-8=250/316

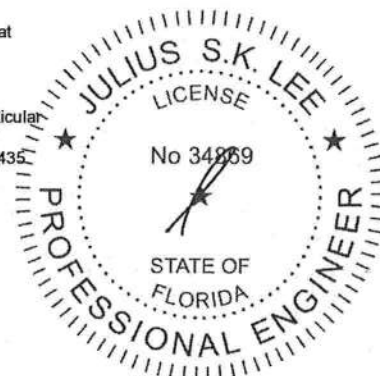
NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 387 lb uplift at joint 2 and 387 lb uplift at joint 6.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)

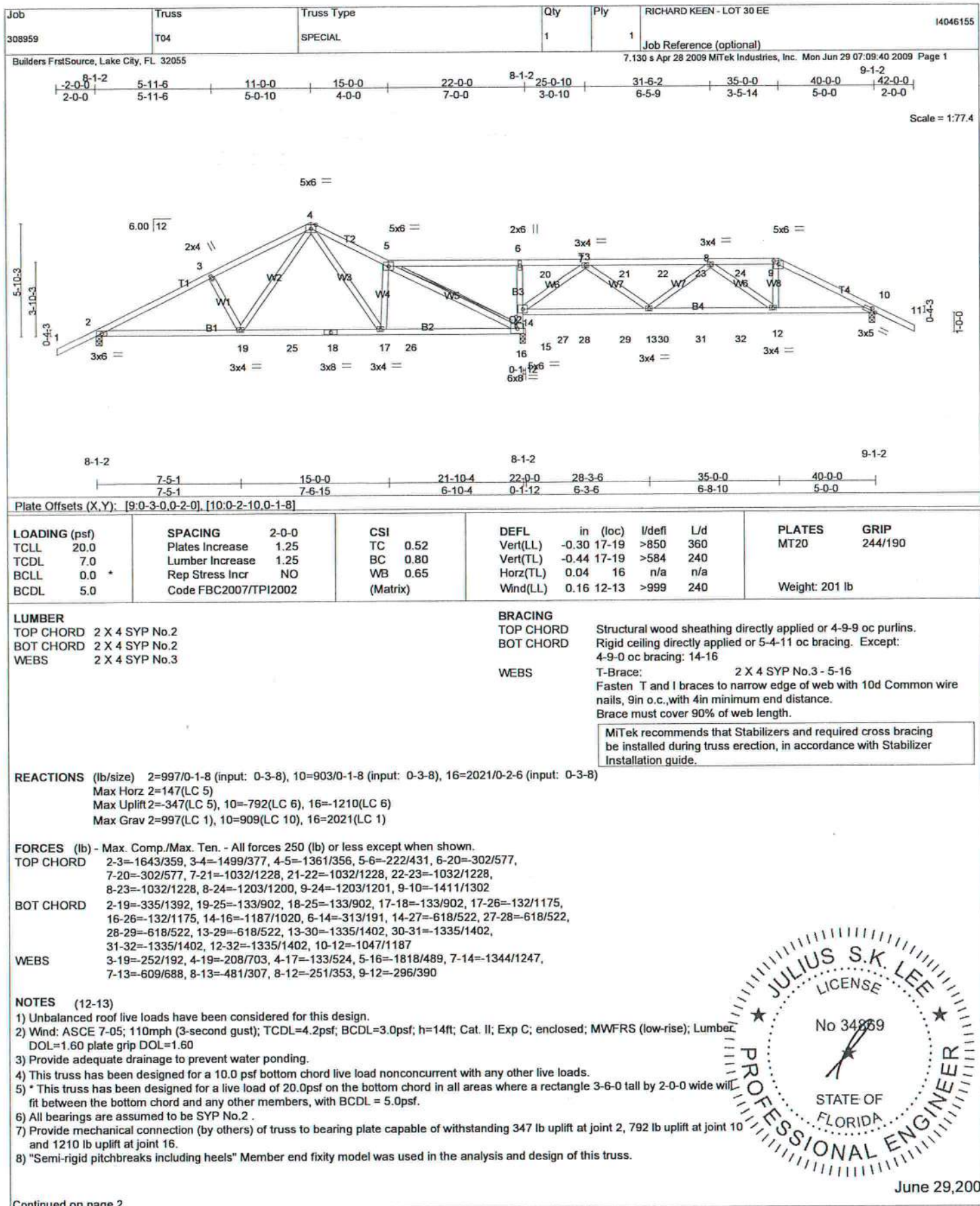
Vert: 1-4=54, 4-7=54, 2-10=10, 9-10=70(F=60), 9-11=110(F=60), 8-11=70(F=60), 6-8=10



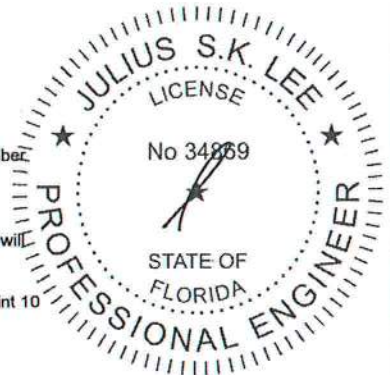
June 29, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435



Continued on page 2



June 29, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MTEK connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

| | | | | | | |
|--------|-------|------------|-----|-----|--------------------------|----------|
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - LOT 30 EE | 14046155 |
| 308959 | T04 | SPECIAL | 1 | 1 | Job Reference (optional) | |

Builders FirstSource, Lake City, FL 32055

7.130 s Apr 28 2009 Mitek Industries, Inc. Mon Jun 29 07:09:40 2009 Page 2

NOTES (12-13)

- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 89 lb down and 93 lb up at 22-11-4, 89 lb down and 93 lb up at 24-11-4, 89 lb down and 93 lb up at 26-11-4, 89 lb down and 93 lb up at 28-11-4, 89 lb down and 93 lb up at 30-11-4, and 89 lb down and 93 lb up at 32-11-4, and 259 lb down and 257 lb up at 35-0-0 on top chord, and 42 lb down and 64 lb up at 22-11-4, 42 lb down and 64 lb up at 24-11-4, 42 lb down and 64 lb up at 26-11-4, 42 lb down and 64 lb up at 28-11-4, 42 lb down and 64 lb up at 30-11-4, and 42 lb down and 64 lb up at 32-11-4, and 111 lb down and 48 lb up at 34-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

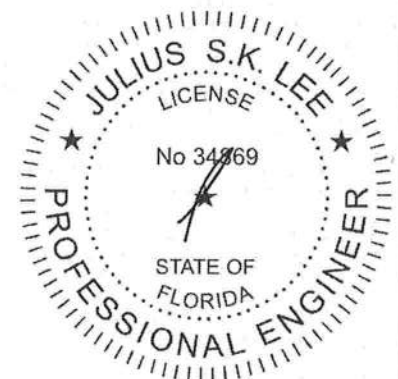
- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-5=-54, 5-9=-54, 9-11=-54, 2-19=-10, 19-25=-70(F=-60), 18-25=-110(F=-60), 18-26=-70(F=-60), 16-26=-10, 15-16=-10, 10-14=-10

Concentrated Loads (lb)

Vert: 9=-179(F) 7=-49(F) 12=-42(F) 20=-49(F) 21=-49(F) 22=-49(F) 23=-49(F) 24=-49(F) 27=-14(F) 28=-14(F) 29=-14(F) 30=-14(F) 31=-14(F) 32=-14(F)



June 29, 2009

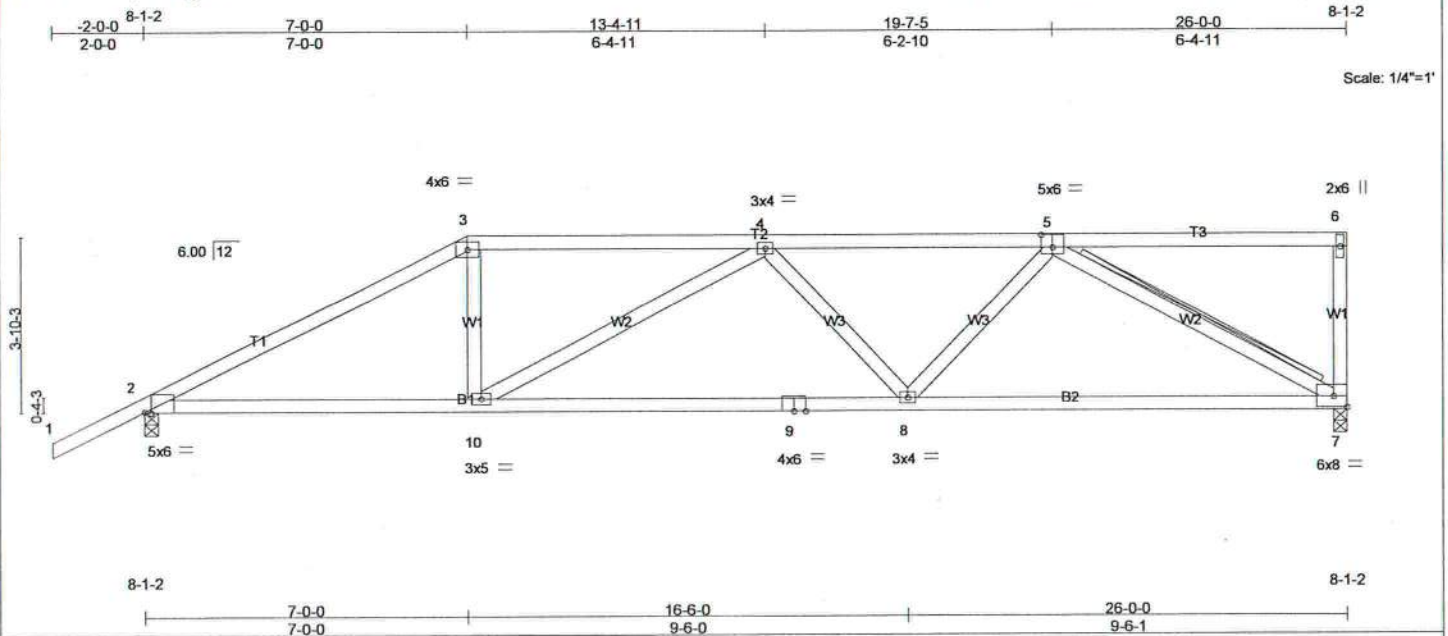
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

| | | | | | | |
|--------|-------|------------|-----|-----|--------------------------|----------|
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - LOT 30 EE | I4046156 |
| 308959 | T05 | MONO HIP | 1 | 1 | Job Reference (optional) | |

Builders FirstSource, Lake City, FL 32055

7.130 s Apr 28 2009 Mitek Industries, Inc. Mon Jun 29 07:09:41 2009 Page 1



Scale: 1/4"=1'

Plate Offsets (X,Y): [2:0-1-11,Edge], [5:0-3-0,0-3-4]

| | | | | | | | | |
|----------------------|----------------------|------------|-------------|-----------------|---------------|------------|----------------|-------------|
| LOADING (psf) | SPACING | CSI | DEFL | in (loc) | l/defl | L/d | PLATES | GRIP |
| TCLL 20.0 | 2-0-0 | TC 0.77 | Vert(LL) | -0.18 8-10 | >999 | 360 | MT20 | 244/190 |
| TCDL 7.0 | Plates Increase 1.25 | BC 0.89 | Vert(TL) | -0.50 8-10 | >613 | 240 | | |
| BCLL 0.0 * | Lumber Increase 1.25 | WB 0.93 | Horz(TL) | 0.13 7 | n/a | n/a | | |
| BCDL 5.0 | Rep Stress Incr NO | (Matrix) | Wind(LL) | 0.20 8-10 | >999 | 240 | | |
| | Code FBC2007/TPI2002 | | | | | | Weight: 126 lb | |

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-9-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-9-13 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-7
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 7=1829/0-2-3 (input: 0-3-8), 2=1762/0-2-1 (input: 0-3-8)

Max Horz 2=198(LC 5)
Max Uplift 7=851(LC 4), 2=764(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

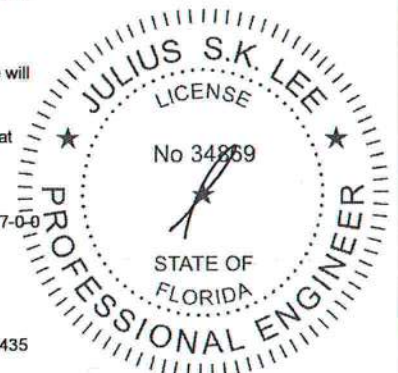
TOP CHORD 2-3=3229/1398, 3-4=2831/1296, 4-5=3101/1378, 6-7=326/213
BOT CHORD 2-10=1282/2793, 9-10=1653/3465, 8-9=1653/3465, 7-8=1226/2541
WEBS 3-10=360/920, 4-10=731/468, 4-8=554/418, 5-8=231/849, 5-7=2852/1390

NOTES (12-13)

- Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 851 lb uplift at joint 7 and 764 lb uplift at joint 2.
- Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 411 lb down and 220 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Continued on page 2



June 29, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

| | | | | | | |
|--------|-------|------------|-----|-----|--------------------------|----------|
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - LOT 30 EE | 14046156 |
| 308959 | T05 | MONO HIP | 1 | 1 | Job Reference (optional) | |

Builders FrstSource, Lake City, FL 32055

7.130 s Apr 28 2009 MiTek Industries, Inc. Mon Jun 29 07:09:41 2009 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-6=-118(F=-64), 2-10=-10, 7-10=-22(F=-12)

Concentrated Loads (lb)

Vert: 10=-411(F)



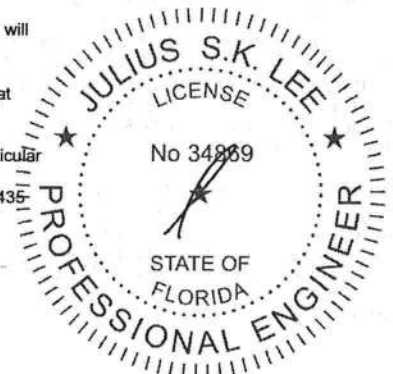
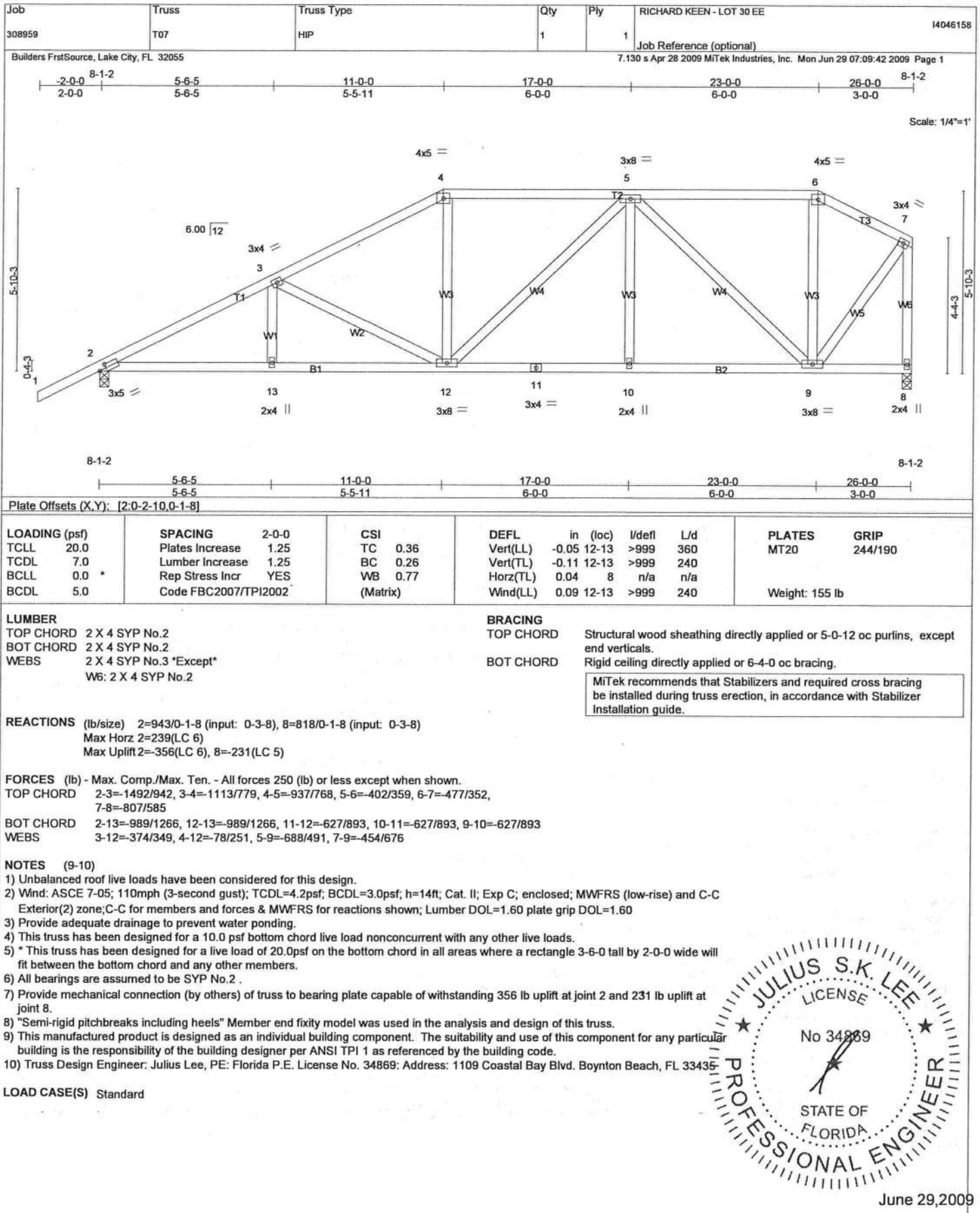
June 29, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

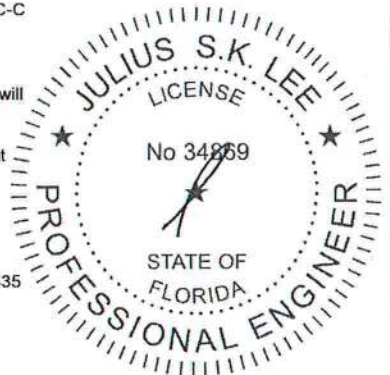


June 29, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroffia Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

| | | | | | | |
|---|--------------|--|----------|---|--|---|
| Job 308959 | Truss T08 | Truss Type HIP | Qty 1 | Ply 1 | RICHARD KEEN - LOT 30 EE | I4046159 |
| Builders FirstSource, Lake City, FL 32055 | | | | | 7.130 s Apr 28 2009 MITEK Industries, Inc. Mon Jun 29 07:09:43 2009 Page 1 | |
| <div style="display: flex; justify-content: space-between;"> -2-0-0 2-0-0 8-1-2 6-8-5 13-0-0 6-3-11 21-0-0 8-0-0 26-0-0 5-0-0 8-1-2 </div> <div style="display: flex; justify-content: space-between;"> 8-1-2 6-8-5 13-0-0 6-3-11 21-0-0 8-0-0 26-0-0 5-0-0 </div> | | | | | | |
| Plate Offsets (X,Y): [2-0-2-10,0-1-8], [4-0-6-0,0-2-8], [5-0-3-8,0-1-12] | | | | | | |
| LOADING (psf) TCLL 20.0 TCCL 7.0 BCCL 0.0 * BCDL 5.0 | | SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2007/TPI2002 | | CSI TC 0.42 BC 0.42 WB 0.40 (Matrix) | | DEFL in (loc) l/defl L/d Vert(LL) -0.11 8-10 >999 360 Vert(TL) -0.18 8-10 >999 240 Horz(TL) 0.04 7 n/a n/a Wind(LL) 0.08 10-11 >999 240 |
| | | | | PLATES MT20 | | GRIP 244/190 Weight: 147 lb |
| LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 *Except* W6: 2 X 4 SYP No.2 | | | | BRACING TOP CHORD Structural wood sheathing directly applied or 4-9-4 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. WEBS T-Brace: 2 X 4 SYP No.3 - 4-8 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length. | | |
| REACTIONS (lb/size) 2=999/0-1-8 (input: 0-3-8), 7=890/0-1-8 (input: 0-3-8) Max Horz 2=254(LC 6) Max Uplift 2=368(LC 6), 7=194(LC 7) | | | | MITEK recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. | | |
| FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=1570/936, 3-4=1131/727, 4-5=589/500, 5-6=708/478, 6-7=880/603 BOT CHORD 2-11=968/1326, 10-11=968/1326, 9-10=584/949, 9-12=584/949, 8-12=584/949 WEBS 3-10=436/438, 4-10=163/392, 4-8=503/305, 6-8=441/753 | | | | | | |
| NOTES (10-11) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf. 6) All bearings are assumed to be SYP No.2. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 368 lb uplift at joint 2 and 194 lb uplift at joint 7. 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required. 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 | | | | | | |
| LOAD CASE(S) Standard | | | | | | |



June 29, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroffo Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

| | | | | | |
|--------|-------|------------|-----|-----|--------------------------|
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - LOT 30 EE |
| 308959 | T09 | SPECIAL | 1 | 1 | Job Reference (optional) |

I4046160

Builders FirstSource, Lake City, FL 32055

7.130 s Apr 28 2009 MITek Industries, Inc. Mon Jun 29 07:09:44 2009 Page 1

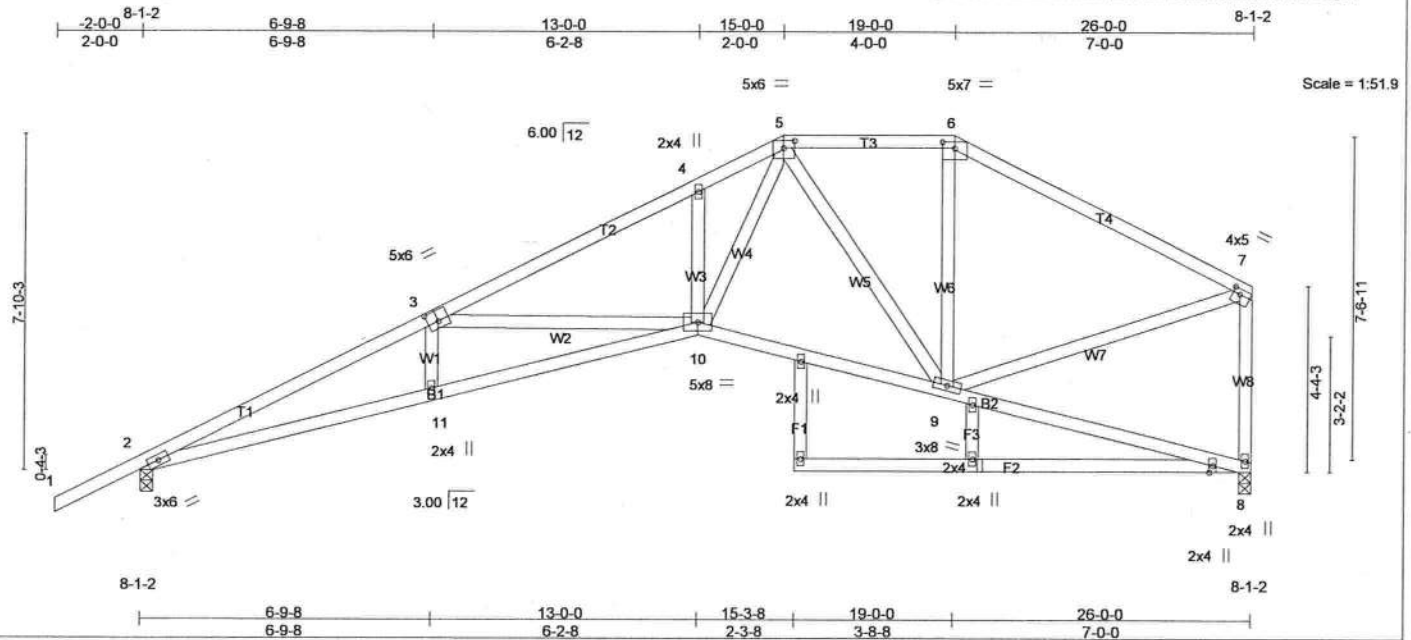


Plate Offsets (X,Y): [3:0-3-0-0-3-0], [5:0-3-0-0-2-0], [6:0-3-8-0-1-12], [7:0-2-0-0-1-8]

| LOADING (psf) | SPACING | CSI | DEFL | in (loc) | I/defl | L/d | PLATES | GRIP |
|---------------|----------------------|----------|----------|-------------|--------|-----|----------------|---------|
| TCLL 20.0 | 2-0-0 | TC 0.51 | Vert(LL) | -0.18 10-11 | >999 | 360 | MT20 | 244/190 |
| TCDL 7.0 | Plates Increase 1.25 | BC 0.51 | Vert(TL) | -0.36 10-11 | >864 | 240 | | |
| BCLL 0.0 * | Lumber Increase 1.25 | WB 0.51 | Horz(TL) | 0.21 8 | n/a | n/a | | |
| BCDL 5.0 | Rep Stress Incr YES | (Matrix) | Wind(LL) | 0.29 10-11 | >999 | 240 | | |
| | Code FBC2007/TPI2002 | | | | | | Weight: 164 lb | |

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3 *Except*
 WB: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-6-12 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 4-6-6 oc bracing. Except:
 7-4-0 oc bracing: 8-10

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=943/0-1-8 (input: 0-3-8), 8=818/0-1-8 (input: 0-3-8)
 Max Horz 2=267(LC 6)
 Max Uplift 2=377(LC 6), 8=212(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2663/1909, 3-4=-1861/1387, 4-5=-1832/1553, 5-6=-737/696, 6-7=-905/672, 7-8=-792/630

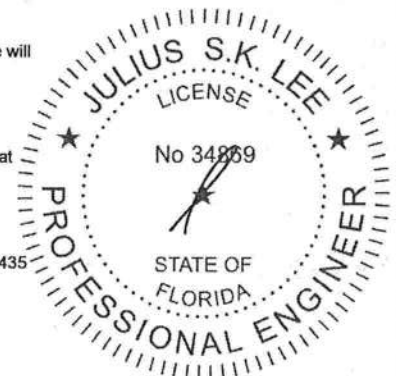
BOT CHORD 2-11=-1887/2367, 10-11=-1891/2368, 9-10=-744/1087

WEBS 3-10=-705/656, 4-10=-269/354, 5-10=-1094/1308, 5-9=-602/422, 7-9=-479/746

NOTES (10-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 377 lb uplift at joint 2 and 212 lb uplift at joint 8.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

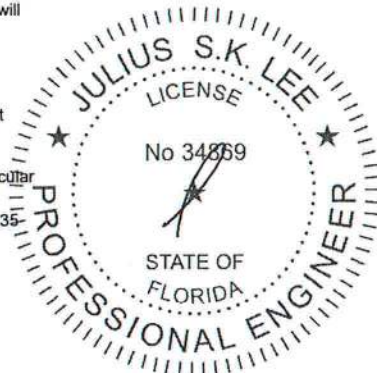
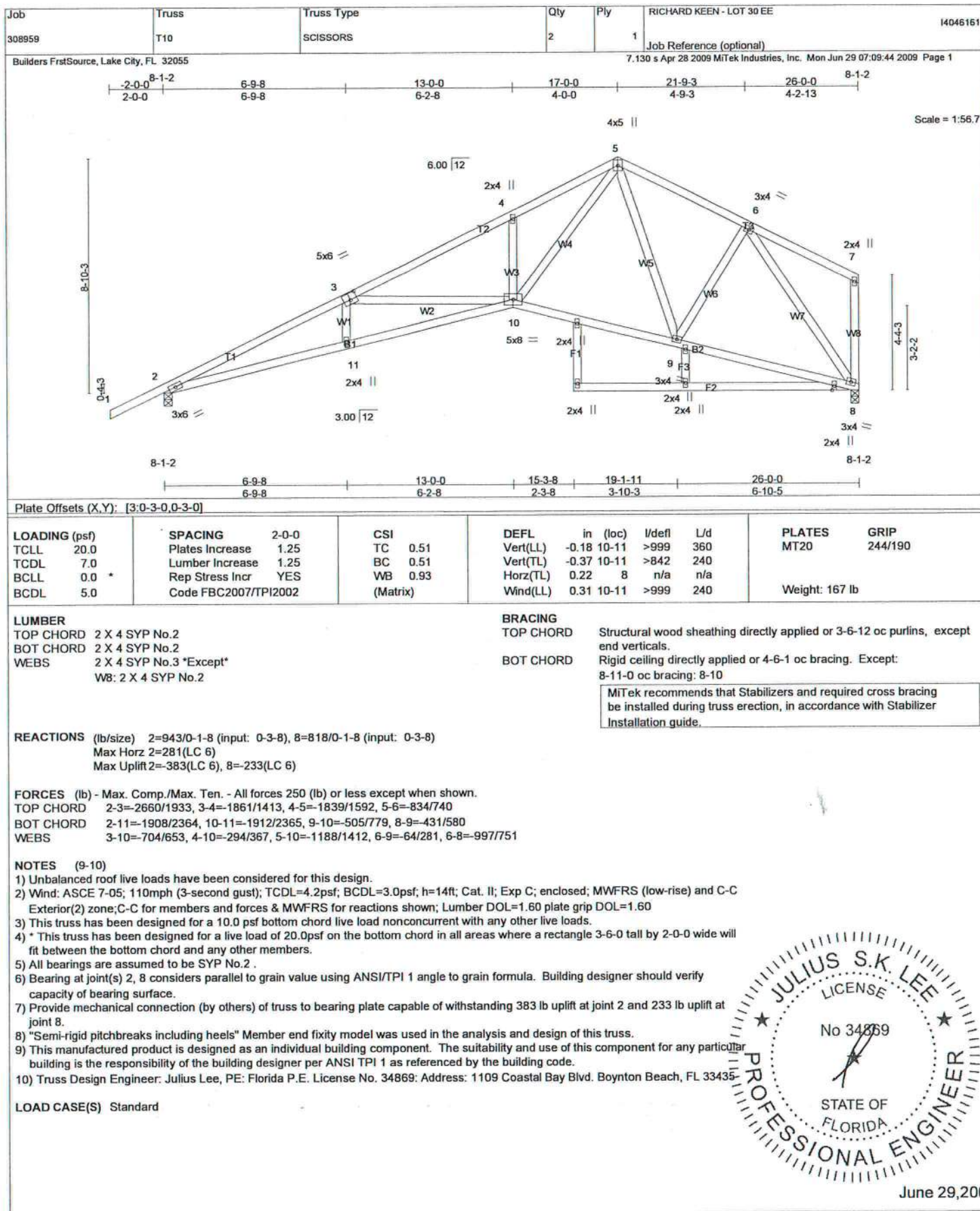


June 29, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435



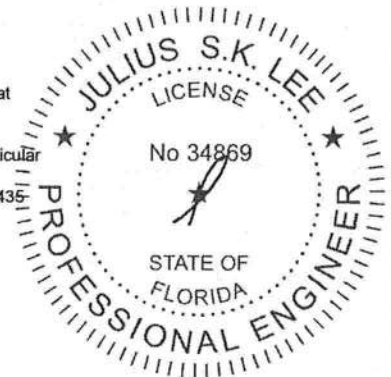
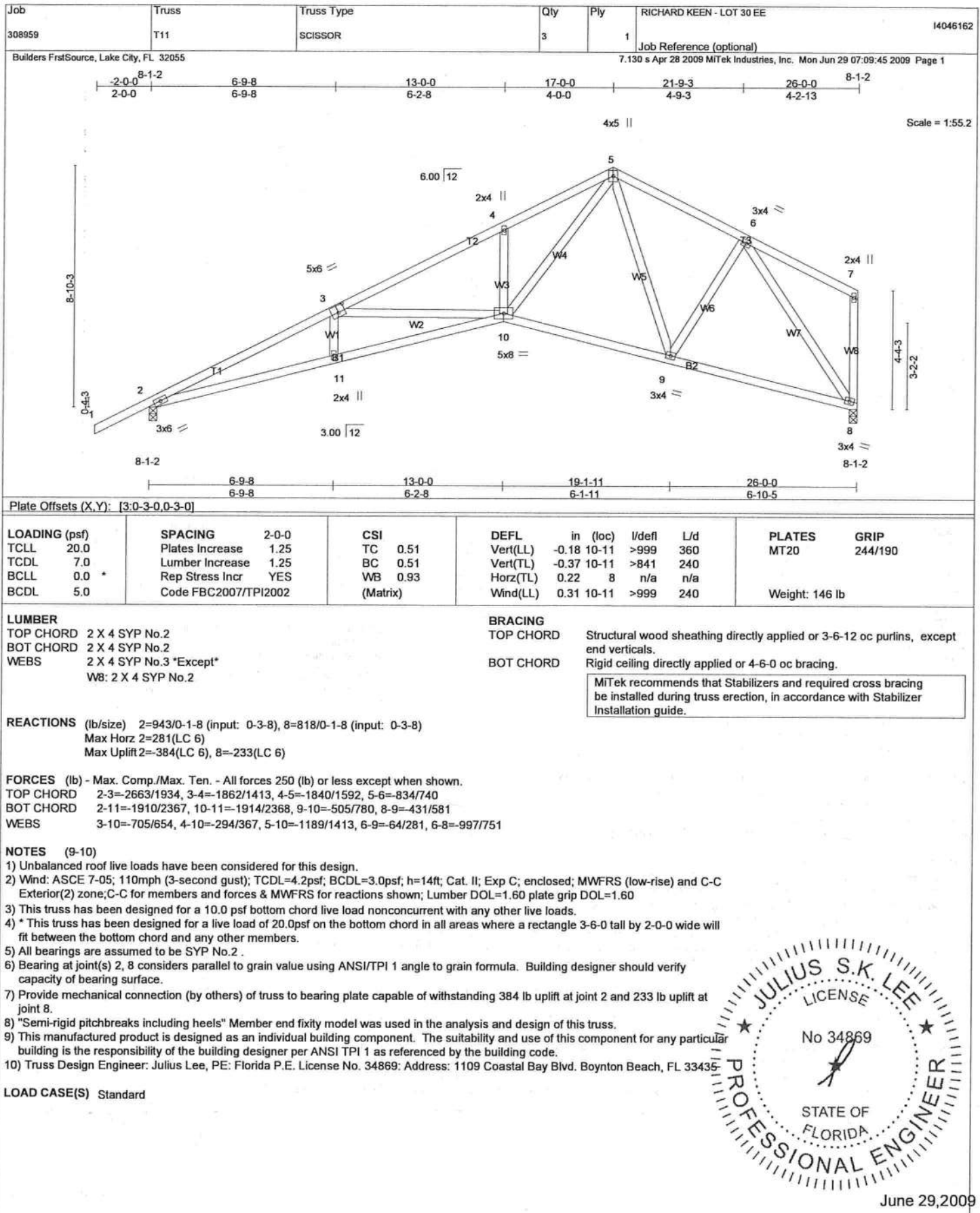
June 29, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435



June 29,2009

| | | | | | | |
|--------|-------|------------|-----|-----|--------------------------|----------|
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - LOT 30 EE | I4046164 |
| 308959 | T13 | SPECIAL | 1 | 1 | Job Reference (optional) | |

Builders FirstSource, Lake City, FL 32055

7.130 s Apr 28 2009 MITek Industries, Inc. Mon Jun 29 07:09:46 2009 Page 1

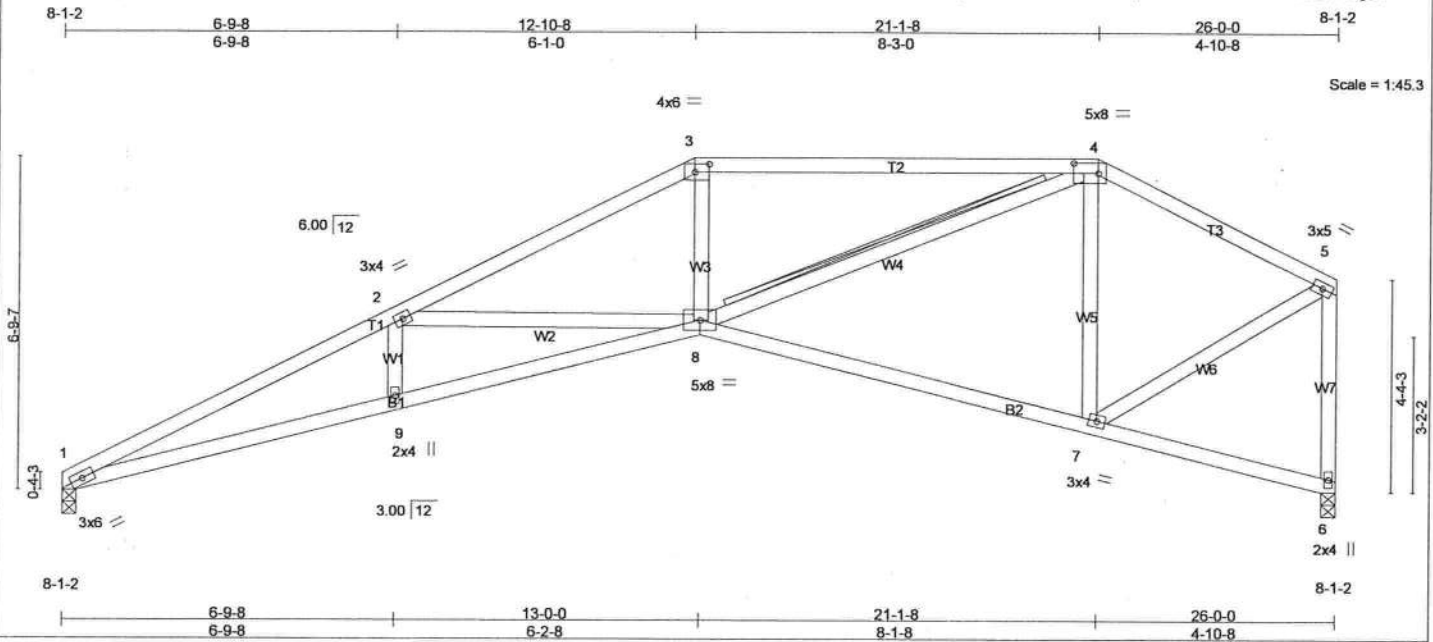


Plate Offsets (X,Y): [3:0-3-8, 0-2-0], [4:0-6-0, 0-2-8]

| LOADING (psf) | SPACING | 2-0-0 | CSI | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
|---------------|----------------------|-------|----------|----------|-------|-------|--------|-----|----------------|---------|
| TCLL 20.0 | Plates Increase | 1.25 | TC 0.65 | Vert(LL) | -0.19 | 8-9 | >999 | 360 | MT20 | 244/190 |
| TCDL 7.0 | Lumber Increase | 1.25 | BC 0.56 | Vert(TL) | -0.37 | 8-9 | >845 | 240 | | |
| BCLL 0.0 | Rep Stress Incr | YES | WB 0.50 | Horz(TL) | 0.22 | 6 | n/a | n/a | | |
| BCDL 5.0 | Code FBC2007/TPI2002 | | (Matrix) | Wind(LL) | 0.31 | 8-9 | >999 | 240 | | |
| | | | | | | | | | Weight: 131 lb | |

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-4-8 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=823/0-1-8 (input: 0-3-8), 6=823/0-1-8 (input: 0-3-8)
Max Horz 1=195(LC 6)
Max Uplift 1=248(LC 6), 6=196(LC 6)

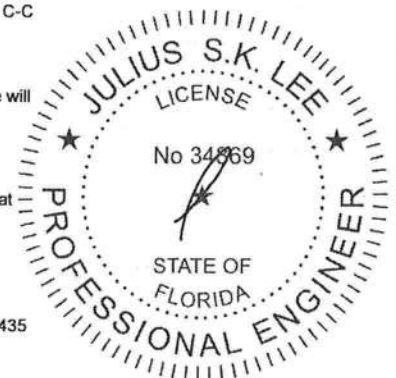
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=2705/2018, 2-3=1897/1401, 3-4=1645/1340, 4-5=771/569, 5-6=809/614
BOT CHORD 1-9=2002/2428, 8-9=2003/2428, 7-8=446/665
WEBS 2-8=722/756, 4-8=827/1094, 4-7=465/409, 5-7=499/749, 3-8=275/458

NOTES (11-12)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 248 lb uplift at joint 1 and 196 lb uplift at joint 6.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

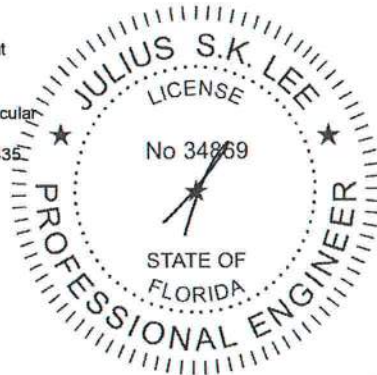
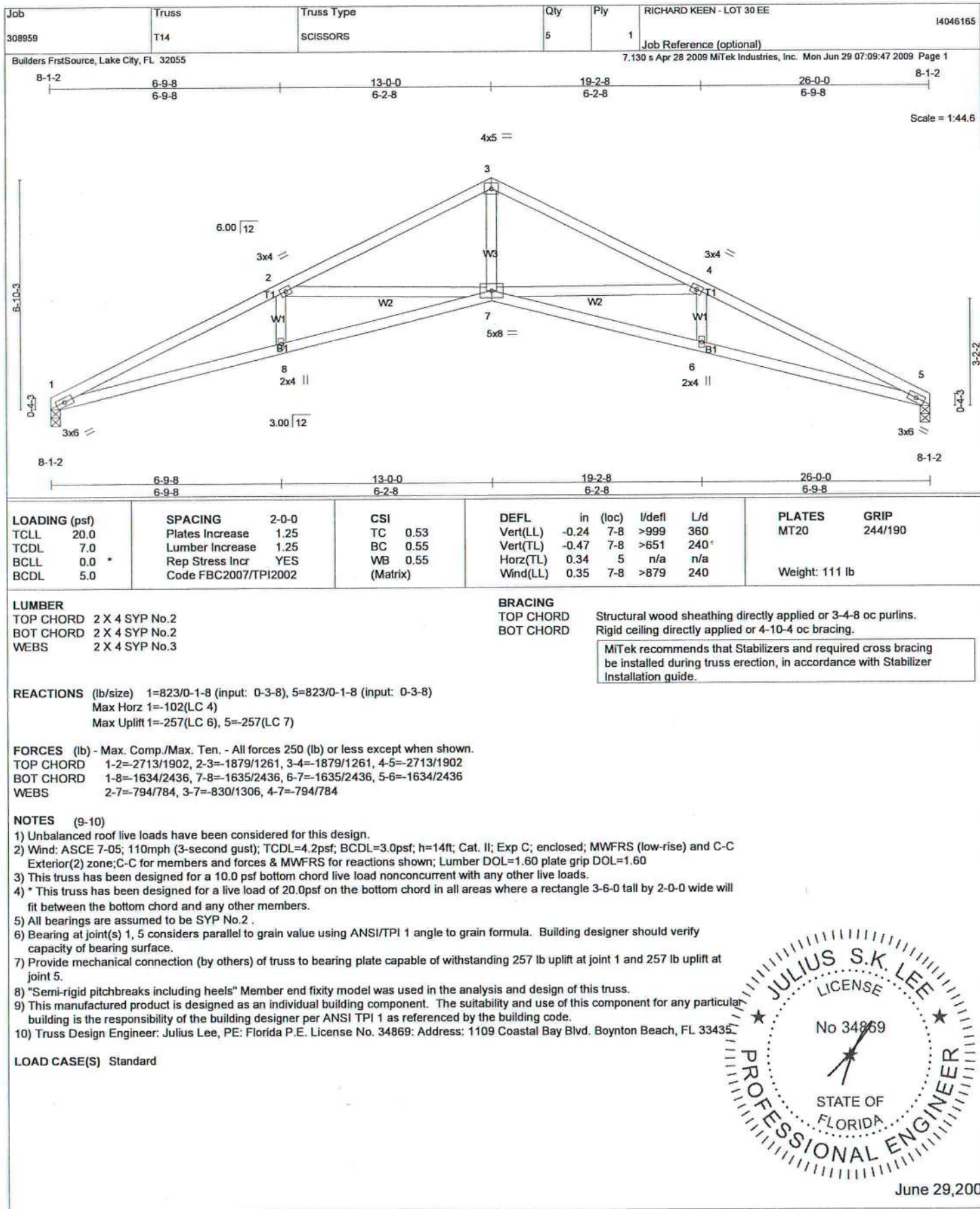
LOAD CASE(S) Standard



June 29, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435



June 29, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MH-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroffia Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

| | | | | | | |
|---------------|--------------|-------------------|----------|----------|--------------------------|----------|
| Job 308959 | Truss T15 | Truss Type HIP | Qty 1 | Ply 2 | RICHARD KEEN - LOT 30 EE | I4046166 |
|---------------|--------------|-------------------|----------|----------|--------------------------|----------|

Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

7.130 s Apr 28 2009 MiTek Industries, Inc. Mon Jun 29 07:09:50 2009 Page 1



Scale = 1:71.1

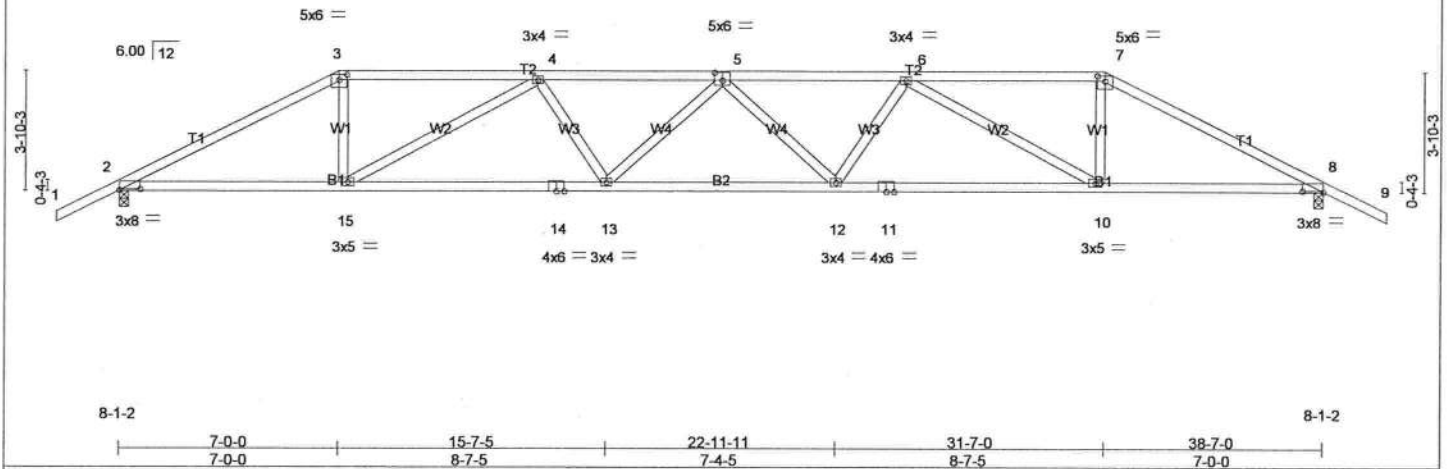


Plate Offsets (X,Y): [2-0-8-0,0-0-6], [3-0-3-0,0-2-0], [5-0-3-0,0-3-0], [7-0-3-0,0-2-0], [8-0-8-0,0-0-6]

| LOADING (psf) | SPACING | CSI | DEFL | in (loc) | I/defl | L/d | PLATES | GRIP |
|---------------|----------------------|----------|----------|----------|--------|------|----------------|---------|
| TCLL 20.0 | Plates Increase 1.25 | TC 0.42 | Vert(LL) | -0.36 | 12-13 | >999 | MT20 | 244/190 |
| TCDL 7.0 | Lumber Increase 1.25 | BC 0.76 | Vert(TL) | -0.70 | 12-13 | >661 | | |
| BCLL 0.0 * | Rep Stress Incr NO | WB 0.55 | Horz(TL) | 0.20 | 8 | n/a | | |
| BCDL 5.0 | Code FBC2007/TPI2002 | (Matrix) | Wind(LL) | 0.39 | 12-13 | >999 | | |
| | | | | | | | Weight: 362 lb | |

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-9-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-6 oc bracing.

REACTIONS

(lb/size) 2=2676/0-1-9 (input: 0-3-8), 8=2676/0-1-9 (input: 0-3-8)
Max Horz 2=94(LC 5)
Max Uplift 2=1122(LC 4), 8=1122(LC 3)

FORCES

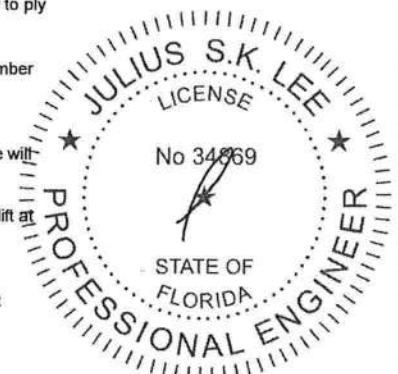
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=5213/2296, 3-4=4630/2111, 4-5=7176/3248, 5-6=7176/3248, 6-7=4630/2111, 7-8=5213/2296
BOT CHORD 2-15=2001/4555, 14-15=3110/6866, 13-14=3110/6866, 12-13=3376/7511, 11-12=3070/6866, 10-11=3070/6866, 8-10=1961/4555
WEBS 3-15=778/1853, 4-15=2643/1312, 4-13=142/610, 5-13=493/322, 5-12=493/322, 6-12=142/610, 6-10=2643/1312, 7-10=778/1853

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1122 lb uplift at joint 2 and 1122 lb uplift at joint 8.
- Girder carries hip end with 7-0-0 end setback.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 411 lb down and 220 lb up at 31-7-0, and 411 lb down and 220 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Continued on page 2



June 29, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

| | | | | | | |
|--------|-------|------------|-----|-----|--------------------------|----------|
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - LOT 30 EE | I4046166 |
| 308959 | T15 | HIP | 1 | 2 | Job Reference (optional) | |

Builders FirstSource, Lake City, FL 32055

7.130 s Apr 28 2009 MiTek Industries, Inc. Mon Jun 29 07:09:50 2009 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-7=-118(F=-64), 7-9=-54, 2-15=-10, 10-15=-22(F=-12), 8-10=-10

Concentrated Loads (lb)

Vert: 15=411(F) 10=411(F)



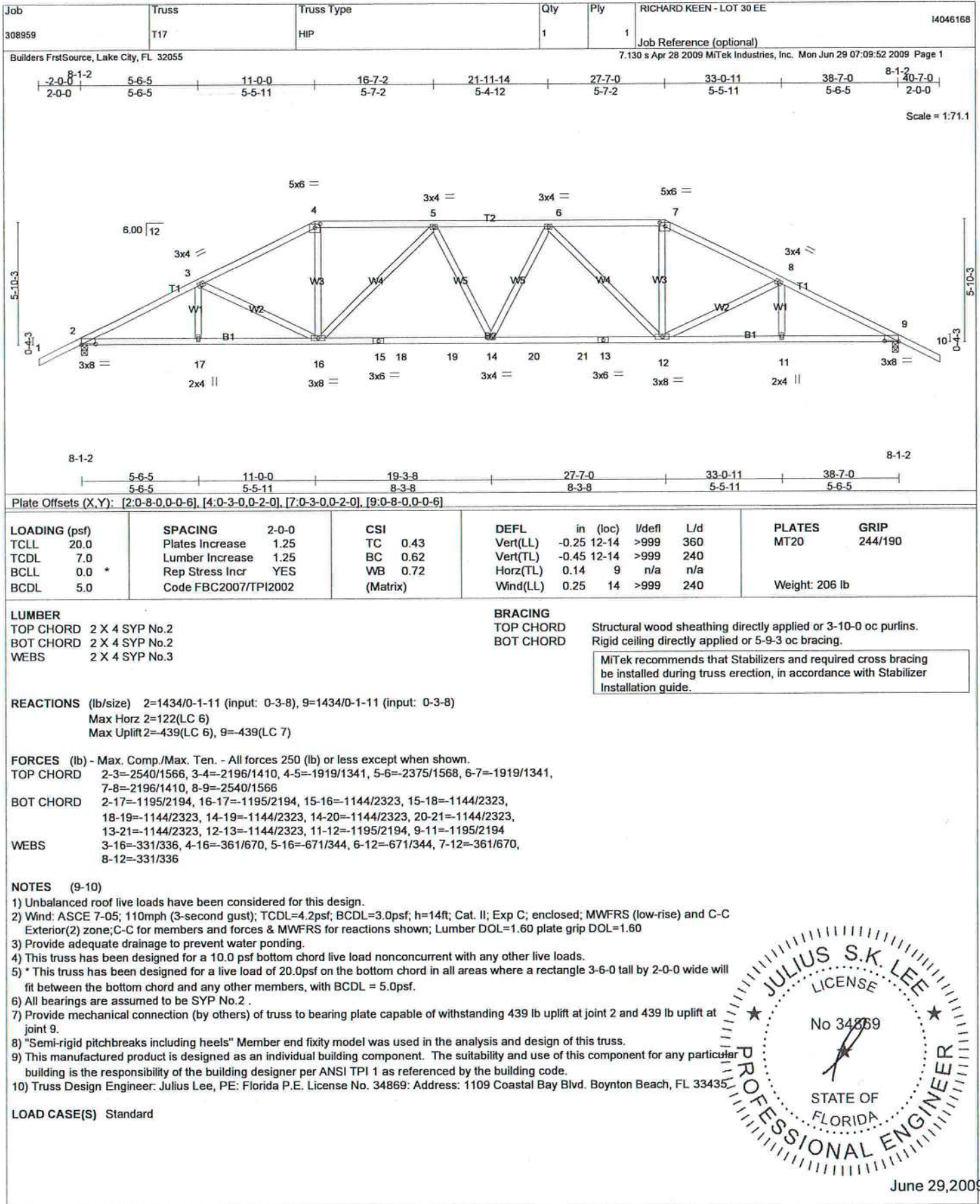
June 29, 2009



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D3B-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-T473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSR-89 and BCSII Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

| | | | | | | |
|---------------|--------------|------------------------|----------|----------|--|----------|
| Job 308959 | Truss T19 | Truss Type MONO HIP | Qty 1 | Ply 1 | RICHARD KEEN - LOT 30 EE Job Reference (optional) 7.130 s Apr 28 2009 MiTek Industries, Inc. Mon Jun 29 07:09:53 2009 Page 1 | I4046170 |
|---------------|--------------|------------------------|----------|----------|--|----------|

Builders FirstSource, Lake City, FL 32055

7.130 s Apr 28 2009 MiTek Industries, Inc. Mon Jun 29 07:09:53 2009 Page 1

Scale = 1:25.9

Plate Offsets (X,Y): [2-0-2-10,0-1-8]

| | | | | | | | | | |
|---------------|----------------------|-------|----------|----------|----------|--------|------|---------------|---------|
| LOADING (psf) | SPACING | 2-0-0 | CSI | DEFL | in (loc) | l/defl | L/d | PLATES | GRIP |
| TCLL 20.0 | Plates Increase | 1.25 | TC 0.50 | Vert(LL) | -0.06 | 2-7 | >999 | MT20 | 244/190 |
| TCDL 7.0 | Lumber Increase | 1.25 | BC 0.29 | Vert(TL) | -0.10 | 2-7 | >999 | | |
| BCLL 0.0 * | Rep Stress Incr | NO | WB 0.42 | Horz(TL) | 0.01 | 6 | n/a | | |
| BCDL 5.0 | Code FBC2007/TPI2002 | | (Matrix) | Wind(LL) | 0.10 | 2-7 | >999 | | |
| | | | | | | | | Weight: 65 lb | |

| | |
|--|--|
| LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 | BRACING TOP CHORD Structural wood sheathing directly applied or 5-9-15 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 7-8-13 oc bracing. |
|--|--|

REACTIONS (lb/size) 6=917/0-1-8 (input: 0-3-8), 2=764/0-1-8 (input: 0-3-8)
 Max Horz 2=198(LC 5)
 Max Uplift 6=611(LC 4), 2=555(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=1113/630, 3-4=955/599, 4-5=834/563, 5-6=862/582
 BOT CHORD 2-7=642/935
 WEBS 5-7=660/972

NOTES (11-12)

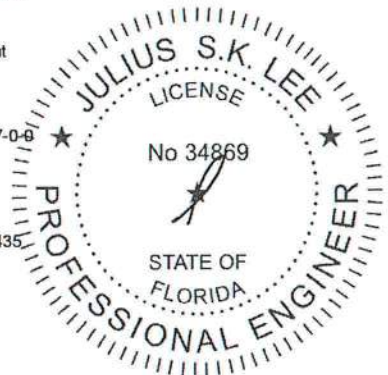
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SYP No.2.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 611 lb uplift at joint 6 and 555 lb uplift at joint 2.
- 7) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 411 lb down and 220 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=54, 4-5=118(F=64), 2-7=10, 6-7=22(F=12)

June 29, 2009

Continued on page 2

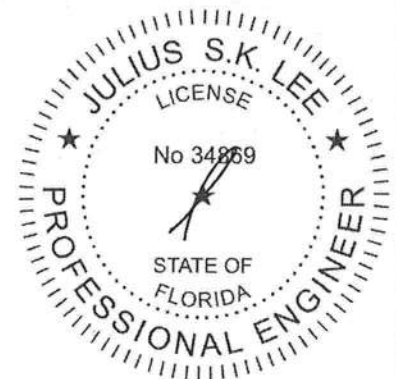


| | | | | | | |
|--------|-------|------------|-----|-----|--------------------------|----------|
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - LOT 30 EE | I4046170 |
| 308959 | T19 | MONO HIP | 1 | 1 | Job Reference (optional) | |

Builders FrstSource, Lake City, FL 32055

7.130 s Apr 28 2009 MiTek Industries, Inc. Mon Jun 29 07:09:53 2009 Page 2

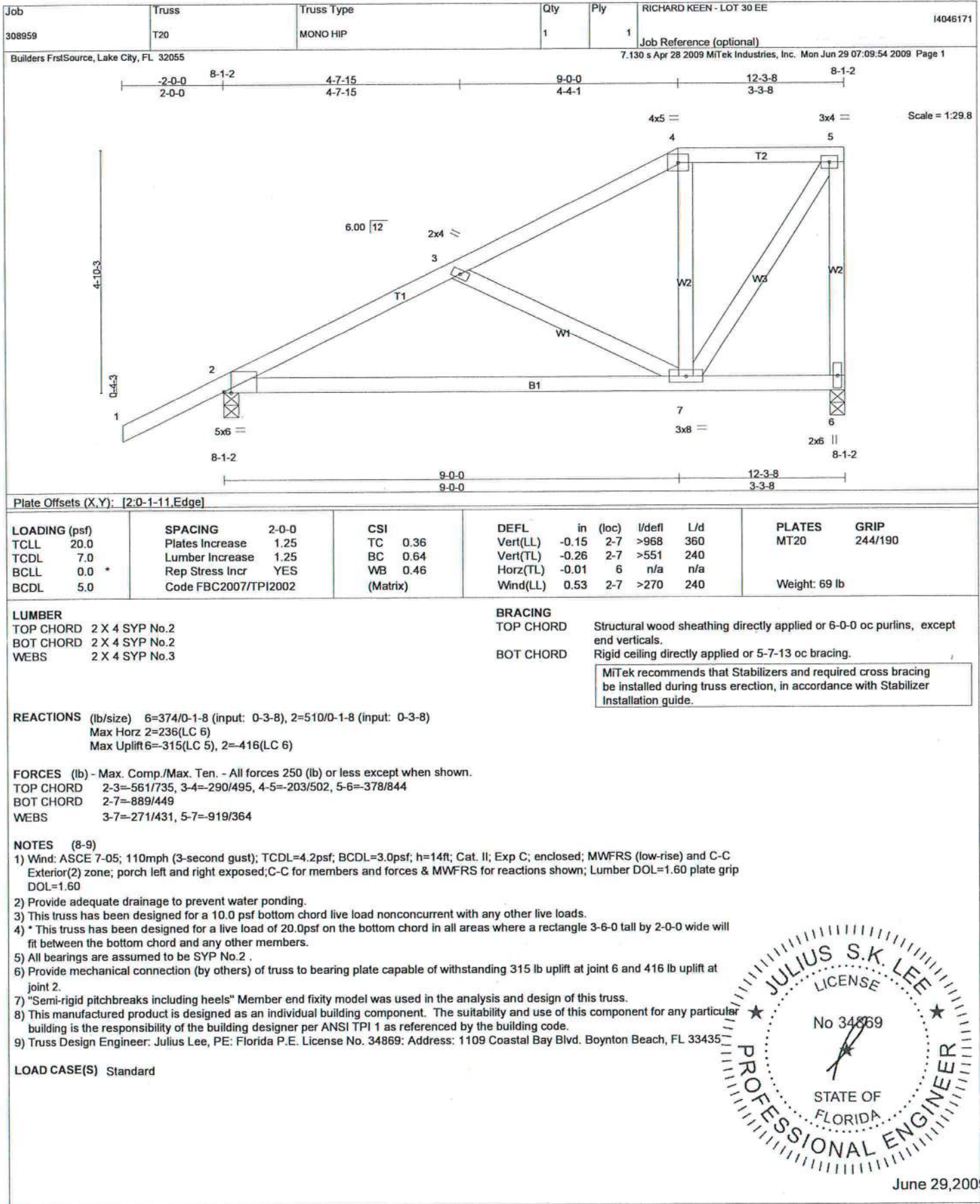
LOAD CASE(S) Standard
Concentrated Loads (lb)
Vert: 7=-411(F)



June 29,2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

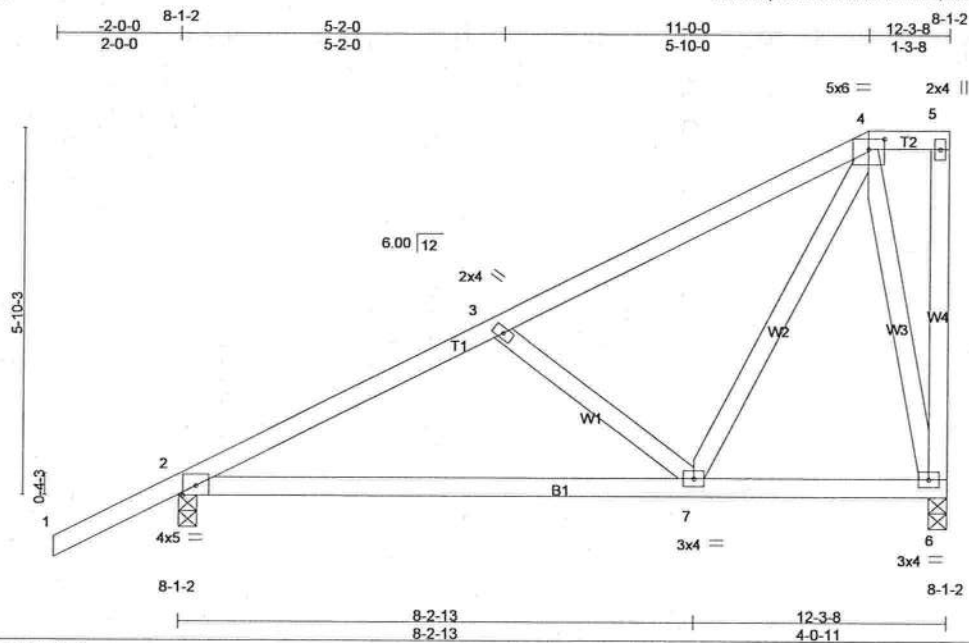


| | | | | | |
|--------|-------|------------|-----|-----|--------------------------|
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - LOT 30 EE |
| 308959 | T21 | MONO HIP | 1 | 1 | Job Reference (optional) |

I4046172

Builders FrstSource, Lake City, FL 32055

7.130 s Apr 28 2009 MITek Industries, Inc. Mon Jun 29 07:09:54 2009 Page 1



Scale = 1:35.4

Plate Offsets (X,Y): [4:0-3:0,0-2-0]

| LOADING (psf) | SPACING | CSI | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---------------|----------------------|----------|----------------|-----|-------|--------|-----|---------------|---------|
| TCLL 20.0 | Plates Increase 1.25 | TC 0.36 | Vert(LL) -0.10 | 2-7 | >999 | 360 | | MT20 | 244/190 |
| TCDL 7.0 | Lumber Increase 1.25 | BC 0.49 | Vert(TL) -0.18 | 2-7 | >798 | 240 | | | |
| BCLL 0.0 | Rep Stress Incr YES | WB 0.60 | Horz(TL) -0.01 | 6 | n/a | n/a | | | |
| BCDL 5.0 | Code FBC2007/TPI2002 | (Matrix) | Wind(LL) 0.36 | 2-7 | >395 | 240 | | | |
| | | | | | | | | Weight: 72 lb | |

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-9-14 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=374/0-1-8 (input: 0-3-8), 2=510/0-1-8 (input: 0-3-8)
 Max Horz 2=275(LC 6)
 Max Uplift 6=351(LC 6), 2=404(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-566/707, 3-4=-317/540
 BOT CHORD 2-7=-926/453
 WEBS 3-7=-306/460, 4-7=-884/324, 4-6=-407/851

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SYP No.2.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 351 lb uplift at joint 6 and 404 lb uplift at joint 2.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



June 29, 2009

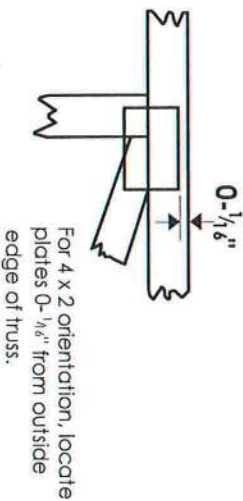
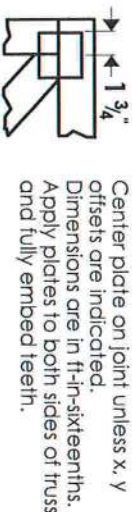
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION

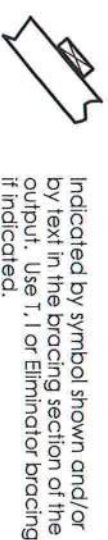


* Plate location details available in Mitek 20/20 software or upon request.

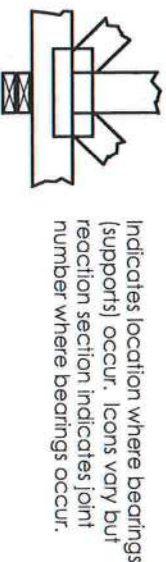
PLATE SIZE

4 X 4
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



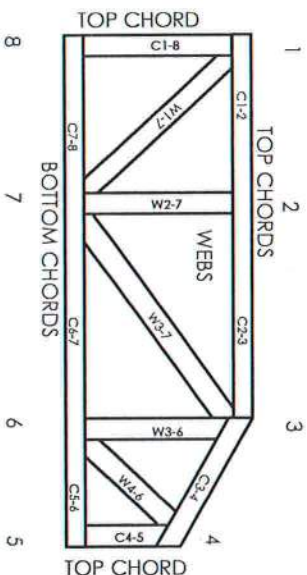
BEARING



Industry Standards:

ANSI/TP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B,
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

© 2006 Mitek® All Rights Reserved



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by ANSI/TP11.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP11.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP11 Quality Criteria.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

STEPPED DOWN CORNER SET

TOP CHORD 2X4 SO. PINE #2 or Better
BOT CHORD 2X4 SO. PINE #2 or Better
WEBS 2X4 SO. PINE #3 or Better

120 MPH MAX

Setback 7' or Less

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND
SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE

UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND
SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

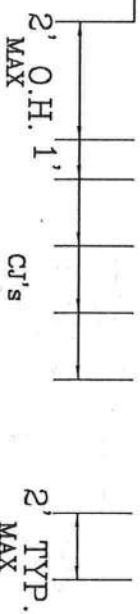
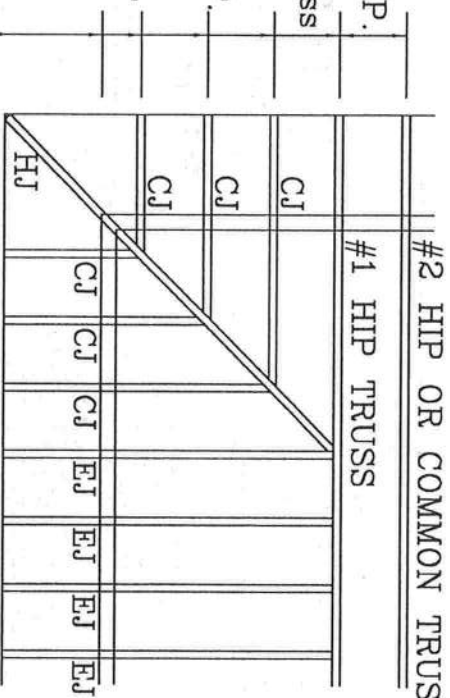
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND
SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

2' TYP. MAX
#2 HIP OR COMMON TRUSS

2' TYP. MAX
#1 HIP TRUSS

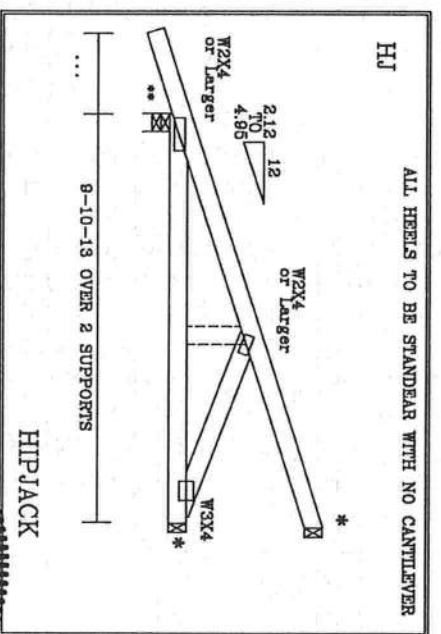
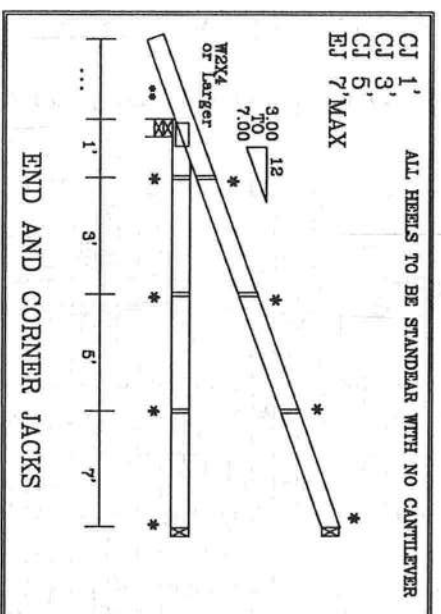
2' TYP. MAX
CJ's

1'



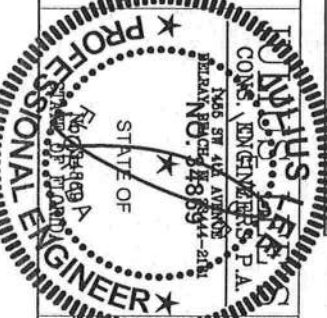
*(3) 16d TOENAILS

** SEE EOR FOR TIE DOWN



| UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED | BC LIVE LOAD IS NON CONCURRENT 10* |
|--|---|
| REF | 7' MAX STBK CS |
| DATE | Jun./27/2008 |
| DRWG | ENG |
| REVIEWED | By Julius Lee at 10:52 am, Jun 27, 2008 |

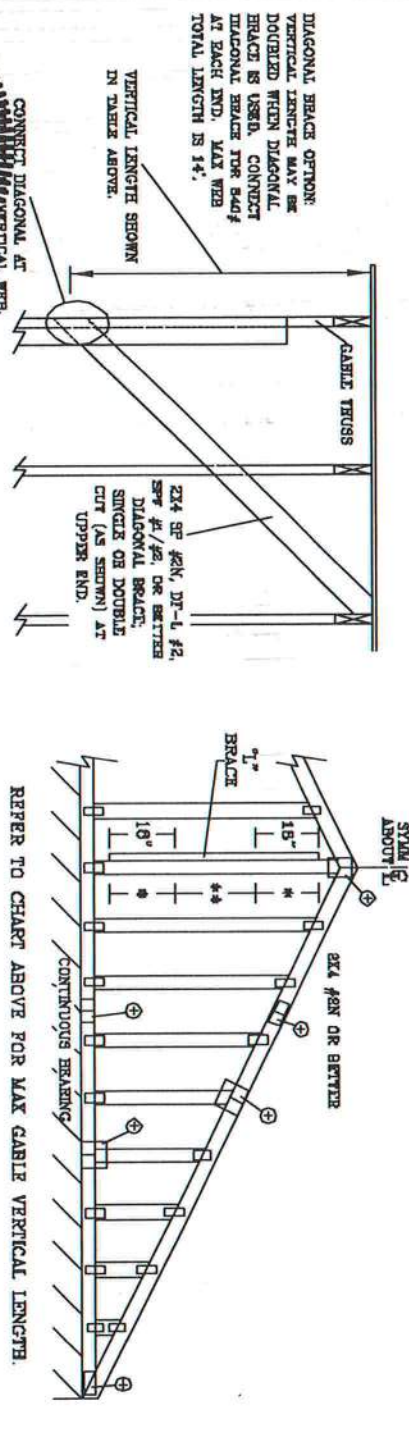
CORNER SET
SETBACK
7'0" MAX



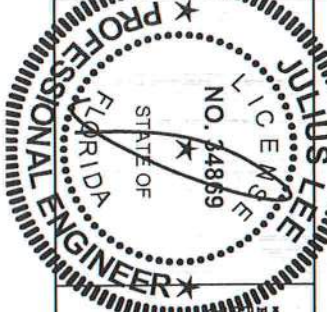
| UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED | BC LIVE LOAD IS NON CONCURRENT 10* |
|--|---|
| REF | 7' MAX STBK CS |
| DATE | Jun./27/2008 |
| DRWG | ENG |
| REVIEWED | By Julius Lee at 10:52 am, Jun 27, 2008 |

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

| MAX GABLE VERTICAL LENGTH | | BRACE | | NO BRACES | | (1) 1X4 "L" BRACE * | | (1) 2X4 "L" BRACE * | | (2) 2X4 "L" BRACE ** | | (1) 2X6 "L" BRACE * | | (2) 2X6 "L" BRACE ** | |
|---------------------------|-----------------|-------|-----------|-----------|--------|---------------------|--------|---------------------|--------|----------------------|-------|---------------------|--------|----------------------|---------|
| CABLE VERTICAL SPACING | SPECIES / GRADE | BRACE | NO BRACES | GROUP A | | GROUP B | | GROUP A | | GROUP B | | GROUP A | | GROUP B | |
| | | | | #1 / #2 | #3 | #1 / #2 | #3 | #1 / #2 | #3 | #1 / #2 | #3 | #1 / #2 | #3 | #1 / #2 | #3 |
| 12" O.C. | SPF | SPF | SPF | 3' 4" | 3' 4" | 6' 10" | 6' 0" | 6' 11" | 7' 1" | 8' 3" | 8' 6" | 10' 10" | 11' 2" | 12' 11" | 13' 3" |
| | HF | HF | HF | 3' 3" | 3' 3" | 4' 11" | 4' 11" | 6' 6" | 6' 6" | 8' 3" | 8' 3" | 10' 1" | 10' 1" | 12' 11" | 12' 11" |
| | DFL | DFL | DFL | 3' 3" | 3' 3" | 4' 2" | 4' 2" | 5' 6" | 5' 6" | 7' 5" | 7' 5" | 8' 8" | 8' 8" | 11' 6" | 11' 6" |
| | SP | SP | SP | 3' 3" | 3' 3" | 5' 10" | 6' 3" | 6' 3" | 6' 3" | 8' 3" | 8' 3" | 10' 10" | 11' 6" | 12' 11" | 13' 11" |
| 16" O.C. | SPF | SPF | SPF | 3' 6" | 3' 6" | 6' 10" | 6' 0" | 6' 11" | 7' 1" | 8' 3" | 8' 6" | 10' 10" | 11' 2" | 12' 11" | 13' 7" |
| | HF | HF | HF | 3' 6" | 3' 6" | 5' 0" | 5' 0" | 6' 6" | 6' 6" | 8' 3" | 8' 3" | 10' 3" | 10' 3" | 12' 11" | 13' 7" |
| | DFL | DFL | DFL | 3' 6" | 3' 6" | 4' 3" | 4' 3" | 5' 8" | 5' 8" | 7' 8" | 7' 8" | 9' 10" | 10' 4" | 12' 11" | 13' 7" |
| | SP | SP | SP | 3' 6" | 3' 6" | 6' 10" | 6' 0" | 6' 11" | 7' 1" | 8' 3" | 8' 6" | 10' 10" | 11' 2" | 12' 11" | 13' 7" |
| 24" O.C. | SPF | SPF | SPF | 3' 10" | 3' 10" | 6' 8" | 6' 8" | 7' 11" | 8' 1" | 9' 6" | 9' 6" | 12' 4" | 12' 4" | 14' 0" | 14' 0" |
| | HF | HF | HF | 3' 10" | 3' 10" | 6' 0" | 6' 0" | 7' 11" | 7' 11" | 9' 5" | 9' 5" | 12' 4" | 12' 4" | 14' 0" | 14' 0" |
| | DFL | DFL | DFL | 3' 10" | 3' 10" | 5' 2" | 5' 2" | 6' 10" | 6' 10" | 8' 2" | 8' 2" | 10' 2" | 10' 2" | 12' 4" | 12' 4" |
| | SP | SP | SP | 3' 10" | 3' 10" | 6' 8" | 6' 8" | 7' 11" | 8' 1" | 9' 6" | 9' 6" | 12' 4" | 12' 4" | 14' 0" | 14' 0" |



| CABLE TRUSS DETAIL NOTES: | |
|---|--|
| LIVE LOAD DEFLECTION CRITERIA IS L/240. | |
| PROVIDE UPLIFT CONNECTIONS FOR 136 PSF OVER CONTINUOUS BEAMING (6 PSF TO DEAD LOAD). | |
| CABLE END SUPPORTS LOAD FROM 4' 0" OUTLINE WITH 8' 0" OVERHANG, OR 12' PLYWOOD OVERHANG. | |
| ATTACH EACH "L" BRACE WITH 10# NAILS. | |
| * FOR (1) "L" BRACE, SPACE NAILS AT 8" O.C. IN 16" END ZONES AND 4" O.C. BETWEEN ZONES. | |
| ** FOR (2) "L" BRACES, SPACE NAILS AT 3" O.C. IN 16" END ZONES AND 6" O.C. BETWEEN ZONES. | |
| "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH. | |



REVIEWED
By Julius Lee at 12:00 pm, Jun 11, 2008

Julius Lee's
CONS. ENGINEERS P.A.
1435 ST. 4th AVENUE
DELAN BEACH, FL 33441-9161

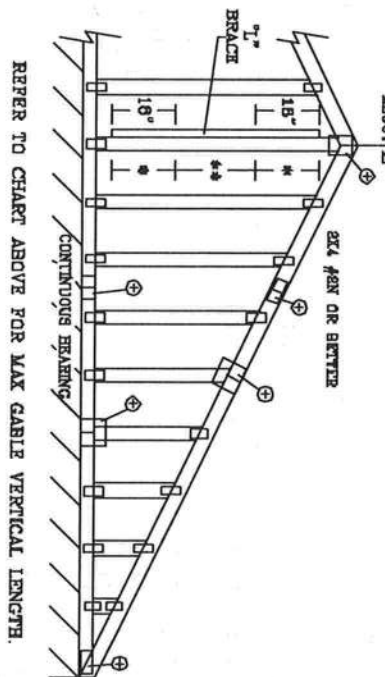
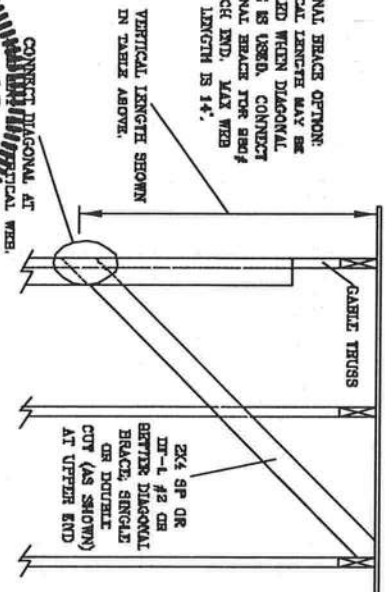
REF: ASCE7-02-CAB13015
DATE: 11/26/03
DRWG: MTRX STD CABLE 15 E ET
-ENG-

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

| CABLE VERTICAL SPACING | BRACE SPECIES | BRACE NO. | (1) 1X4 T" BRACE * | | | | | | (2) 2X4 T" BRACE ** | | | | | | (3) 2X6 T" BRACE * | | | | | | (4) 2X8 T" BRACE * | | | | | |
|------------------------|---------------|-----------|--------------------|--------|---------|--------|---------|--------|---------------------|--------|---------|--------|---------|--------|--------------------|--------|---------|--------|---------|--------|--------------------|--------|---------|--------|---------|--------|
| | | | GROUP A | | GROUP B | | GROUP A | | GROUP B | | GROUP A | | GROUP B | | GROUP A | | GROUP B | | GROUP A | | GROUP B | | GROUP A | | GROUP B | |
| | | | #1 | #2 | #1 | #2 | #1 | #2 | #1 | #2 | #1 | #2 | #1 | #2 | #1 | #2 | #1 | #2 | #1 | #2 | #1 | #2 | #1 | #2 | #1 | #2 |
| 12" O.C. | SPF | #1 / #2 | 3' 2" | 5' 6" | 6' 8" | 6' 8" | 6' 9" | 7' 10" | 8' 0" | 10' 3" | 10' 7" | 12' 3" | 12' 7" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" |
| | | #3 | 3' 1" | 4' 5" | 4' 5" | 6' 10" | 5' 10" | 7' 10" | 7' 10" | 8' 1" | 9' 1" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" |
| | | STUD | 3' 1" | 4' 6" | 4' 5" | 5' 10" | 6' 10" | 7' 10" | 7' 10" | 9' 1" | 9' 1" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" |
| | HF | STANDARD | 2' 11" | 3' 6" | 3' 9" | 6' 0" | 5' 0" | 6' 9" | 8' 9" | 7' 10" | 10' 7" | 10' 7" | 10' 7" | 10' 7" | 10' 7" | 10' 7" | 10' 7" | 10' 7" | 10' 7" | 10' 7" | 10' 7" | 10' 7" | 10' 7" | 10' 7" | 10' 7" | 10' 7" |
| | | #1 | 3' 6" | 5' 6" | 5' 11" | 6' 8" | 7' 0" | 7' 10" | 8' 5" | 10' 3" | 11' 1" | 12' 3" | 13' 2" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" |
| | | #2 | 3' 6" | 5' 6" | 5' 11" | 6' 8" | 7' 0" | 7' 10" | 8' 5" | 10' 3" | 11' 1" | 12' 3" | 13' 2" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" |
| 16" O.C. | SPF | #1 / #2 | 3' 3" | 4' 8" | 4' 8" | 6' 11" | 5' 11" | 6' 11" | 8' 0" | 9' 4" | 9' 4" | 12' 3" | 12' 6" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" |
| | | #3 | 3' 3" | 4' 8" | 4' 8" | 6' 11" | 5' 11" | 6' 11" | 8' 0" | 9' 4" | 9' 4" | 12' 3" | 12' 6" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" |
| | | STUD | 3' 3" | 4' 8" | 4' 8" | 6' 11" | 5' 11" | 6' 11" | 8' 0" | 9' 4" | 9' 4" | 12' 3" | 12' 6" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" |
| | HF | STANDARD | 3' 7" | 5' 6" | 5' 5" | 7' 2" | 7' 2" | 8' 11" | 8' 11" | 11' 2" | 11' 2" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #1 | 3' 7" | 5' 6" | 5' 5" | 7' 2" | 7' 2" | 8' 11" | 8' 11" | 11' 2" | 11' 2" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #2 | 3' 7" | 5' 6" | 5' 5" | 7' 2" | 7' 2" | 8' 11" | 8' 11" | 11' 2" | 11' 2" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| 24" O.C. | SPF | #1 / #2 | 3' 0" | 4' 10" | 4' 10" | 6' 11" | 5' 11" | 6' 11" | 8' 0" | 9' 4" | 9' 4" | 12' 3" | 12' 6" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" |
| | | #3 | 3' 0" | 4' 10" | 4' 10" | 6' 11" | 5' 11" | 6' 11" | 8' 0" | 9' 4" | 9' 4" | 12' 3" | 12' 6" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" |
| | | STUD | 3' 0" | 4' 10" | 4' 10" | 6' 11" | 5' 11" | 6' 11" | 8' 0" | 9' 4" | 9' 4" | 12' 3" | 12' 6" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" | 12' 3" |
| | HF | STANDARD | 3' 7" | 5' 6" | 5' 5" | 7' 2" | 7' 2" | 8' 11" | 8' 11" | 11' 2" | 11' 2" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #1 | 3' 7" | 5' 6" | 5' 5" | 7' 2" | 7' 2" | 8' 11" | 8' 11" | 11' 2" | 11' 2" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #2 | 3' 7" | 5' 6" | 5' 5" | 7' 2" | 7' 2" | 8' 11" | 8' 11" | 11' 2" | 11' 2" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |

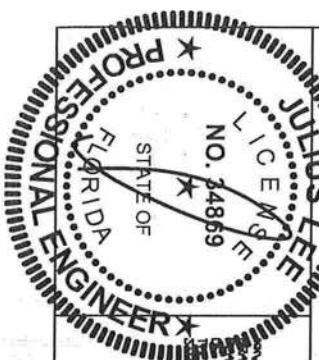


| BRACING GROUP SPECIES AND GRADES: | | | |
|-----------------------------------|----|----------|----|
| GROUP A: | | | |
| SPF | #1 | SPF | #2 |
| STUD | #3 | STUD | #4 |
| STANDARD | | STANDARD | |
| GROUP B: | | | |
| SPF | #1 | SPF | #2 |
| STUD | #3 | STUD | #4 |
| STANDARD | | STANDARD | |

CABLE TRUSS DETAIL NOTES:
 LIVE LOAD DEADLOAD COMBINATION IS 1.2/1.0.
 PROVIDE UPLIFT CONNECTIONS PER 160 PSF OVER CONTINUOUS BEARING (6 PSF TO DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4' 0" OUTLINE WITH 3' 0" OVERHANG, OR 12' PLYWOOD OVERHANG.
 ATTACH EACH T" BRACE WITH 104 NAILS.
 * FOR (1) T" BRACE, SPACE NAILS AT 8" O.C. IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.
 ** FOR (2) T" BRACES, SPACE NAILS AT 3" O.C. IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.
 T" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

| CABLE VERTICAL PLATE SIZES | | | |
|---|------------|-----------|-------|
| VERTICAL LENGTH | NO. BRACE | THICKNESS | GRADE |
| LESS THAN 4' 0" | 1X4 OR 2X4 | 1/2" | SPF |
| GREATER THAN 4' 0" BUT LESS THAN 11' 8" | 2X4 | 1/2" | SPF |
| GREATER THAN 11' 8" | 2X6 | 1/2" | SPF |

REFER TO COMMON TRUSS DESIGN FOR PEAK, SPICE, AND BRG. PLATES.



REVIEWED

By Julius Lee at 12:00 pm, Jun 11, 2008

NO. 34869

STATE OF FLORIDA

PROFESSIONAL ENGINEER

RECEIVED

DATE 11/26/03

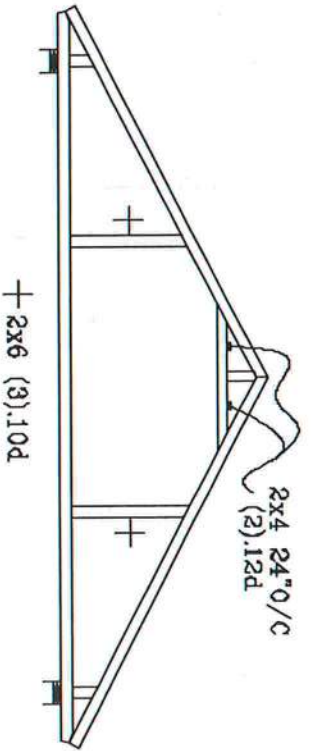
DWG. DATE STD. CABLE 30' x 17'

ENG

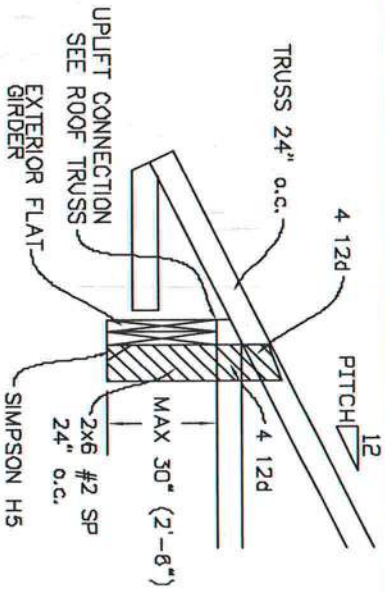
MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

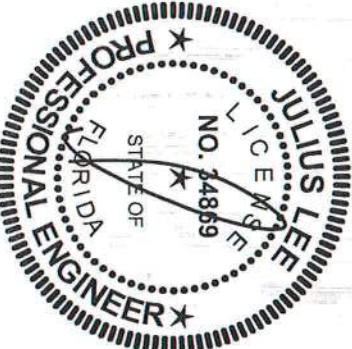
TYPICAL ATTIC TRUSS BRACING



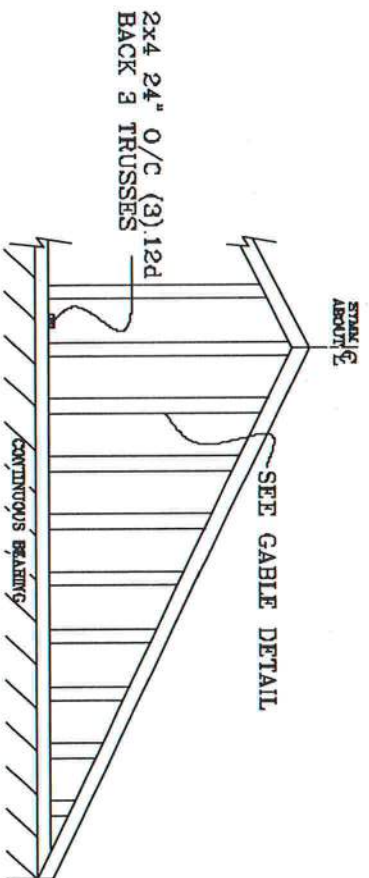
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



REVIEWED
By Julius Lee at 1:59 am, Jun 11, 2008

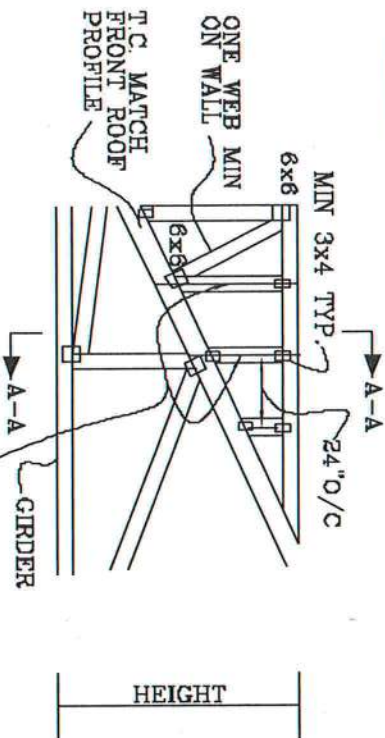


GABLE END TRUSS DETAIL



MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR BOB

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL

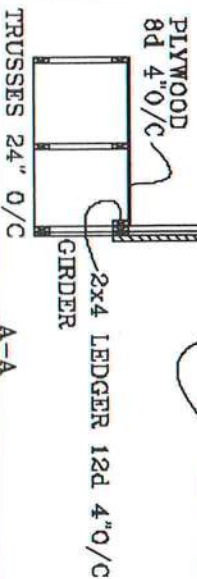


SEE ROOF TRUSSES FOR UPLIFT

SEE ROOF TRUSSES FOR UPLIFT

ROOF 24" O/C

SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
MIAMI BEACH, FL 33444-2161

No. 34869
STATE OF FLORIDA

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPICES MUST BE STAGGERED SO THAT ONE SPICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

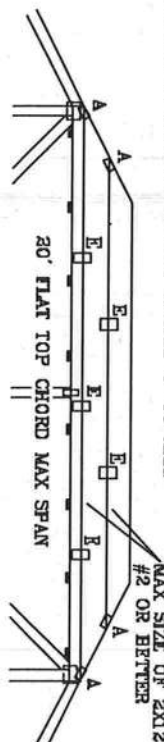
CAT 1, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, ETC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF

WIND TC DL=5 PSF, WIND BC DL=5 PSF

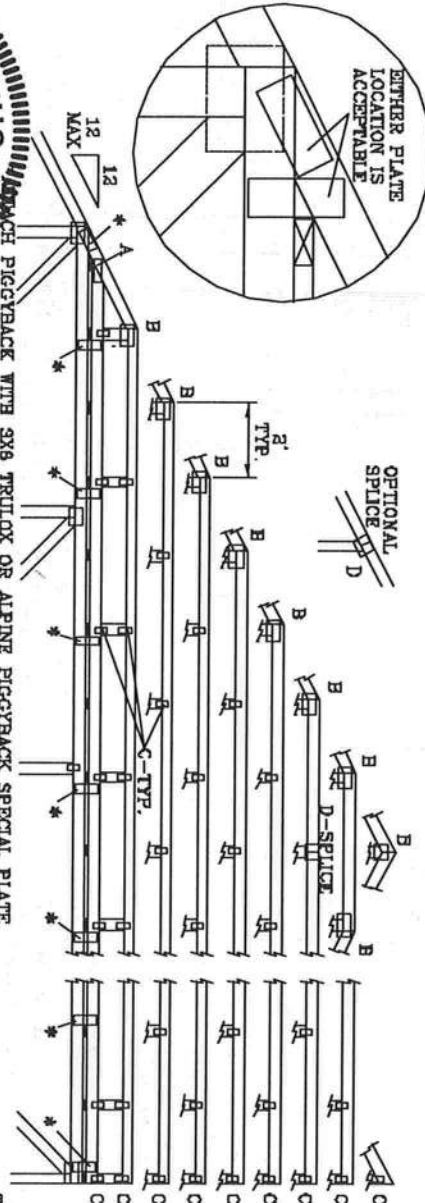
FRONT FACE (E*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT 1, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF



EITHER PLATE LOCATION IS ACCEPTABLE

OPTIONAL SPICE

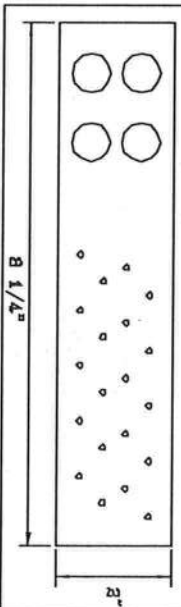


ATTACH TRUSS PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

| JOINT TYPE | SPANS UP TO | | |
|------------|---|-------|-------|
| | 30' | 36' | 62' |
| A | 2X4 | 2.6X4 | 3X6 |
| B | 4X6 | 6X6 | 6X6 |
| C | 1.5X3 | 1.5X4 | 1.5X4 |
| D | 5X4 | 6X6 | 6X6 |
| E | 4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY | | |

| WEB LENGTH | WEB BRACING CHART |
|--------------|--|
| 0' TO 7'-9" | NO BRACING |
| 7'-9" TO 10' | 1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4' OC. |
| 10' TO 14' | 2X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC. |

* PIGGYBACK SPECIAL PLATE
ATTACH TEETH TO THE PIGGYBACK AT THE TIE UP FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 847.045

JULIUS LEE'S
CONS. ENGINEERS P.A.
1404 SW 42ND AVENUE
DEERBEEK, FL 33442-261

No. 34866
STATE OF FLORIDA

MAX LOADING

65 PSF AT

1.33 DUR. FAC.

50 PSF AT

1.25 DUR. FAC.

47 PSF AT

1.15 DUR. FAC.

SPACING

24.0"

REF PIGGYBACK

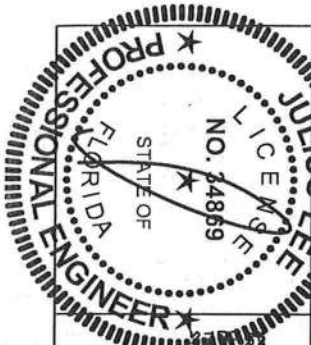
DATE 09/12/07

DRWG/ITEK STD PIGGY

-ENG JL

REVIEWED

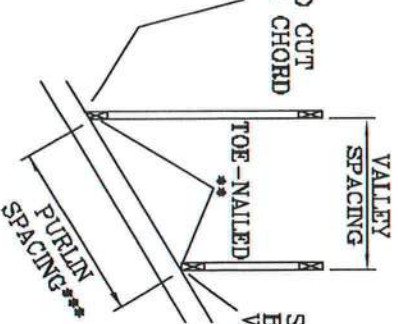
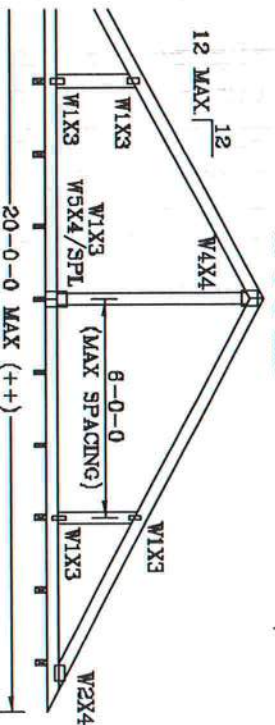
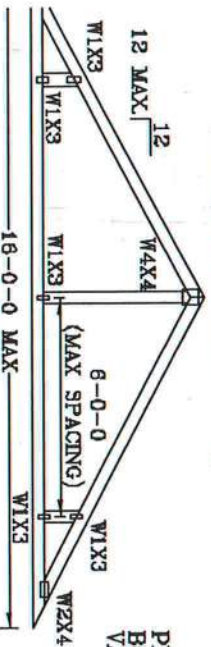
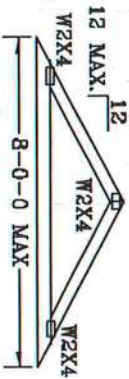
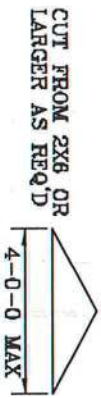
By Julius Lee at 11:59 am, Jun 11, 2008



VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

- * 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- ** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF.



*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.

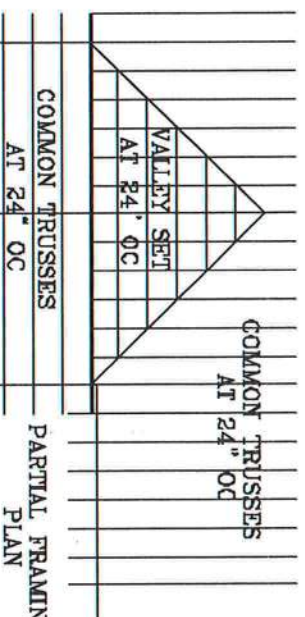
UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS
INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON
ENGINEERS' SEALED DESIGN.

SQUARE CUT
BOTTOM CHORD
VALLEY

OPTIONAL STUB
END DETAIL

OPTIONAL HIP
JOINT DETAIL



COMMON TRUSSES
AT 24" OC

PARTIAL FRAMING
PLAN

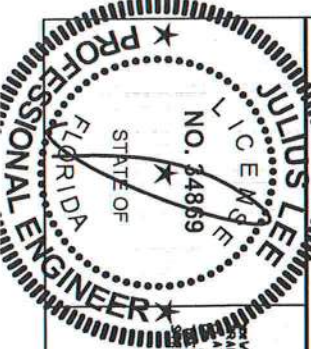
THIS DRAWING REPLACES DRAWING A105

JULIUS LEE'S
CONS. ENGINEERS P.A.

1465 SW 4th AVENUE
DEALY BRICK, FL 33444-7801

| TC LL | 20 | 20 | PSF | REF | VALLEY DETAIL |
|----------|----|----|-----|------|---------------|
| TC DL | 7 | 15 | PSF | DATE | 11/26/03 |
| BC DL | 5 | 5 | PSF | DRWG | VALTRUSS1103 |
| BC LL | 0 | 0 | PSF | -ENG | JL |
| TOT. LD. | 32 | 40 | PSF | | |

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008



TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

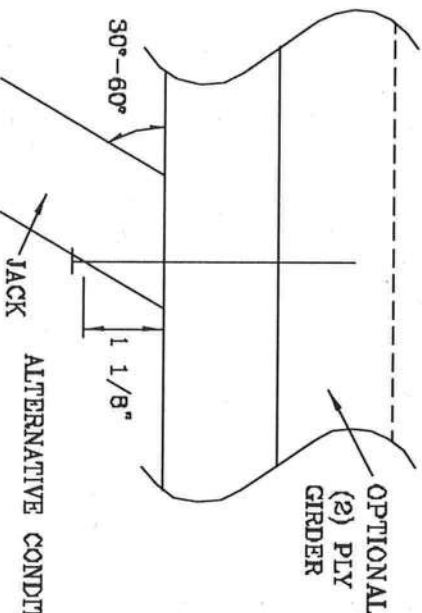
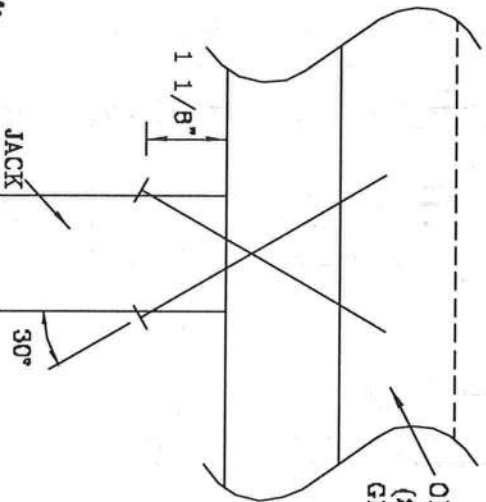
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

| NUMBER OF TOE-NAILS | SOUTHERN PINE | | DOUGLAS FIR-LARCH | | HEM-FIR | | SPRUCE PINE FIR | |
|---------------------|---------------|---------|-------------------|---------|---------|---------|-----------------|---------|
| | 1 PLY | 2 PILES | 1 PLY | 2 PILES | 1 PLY | 2 PILES | 1 PLY | 2 PILES |
| 2 | 187# | 256# | 181# | 234# | 156# | 203# | 154# | 189# |
| 3 | 296# | 383# | 271# | 361# | 234# | 304# | 230# | 298# |
| 4 | 394# | 511# | 361# | 468# | 312# | 406# | 307# | 397# |
| 5 | 493# | 639# | 452# | 585# | 390# | 507# | 384# | 496# |

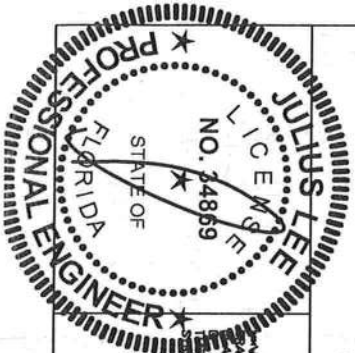
ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS BRACING. TRUSS BRACING SHALL BE PERFORMED BY THE TRUSS MANUFACTURER. TRUSS BRACING SHALL BE PERFORMED BY THE TRUSS MANUFACTURER. TRUSS BRACING SHALL BE PERFORMED BY THE TRUSS MANUFACTURER. TRUSS BRACING SHALL BE PERFORMED BY THE TRUSS MANUFACTURER.



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.

1405 BT 4th AVENUE
DELRAY BEACH, FL 33444-2161

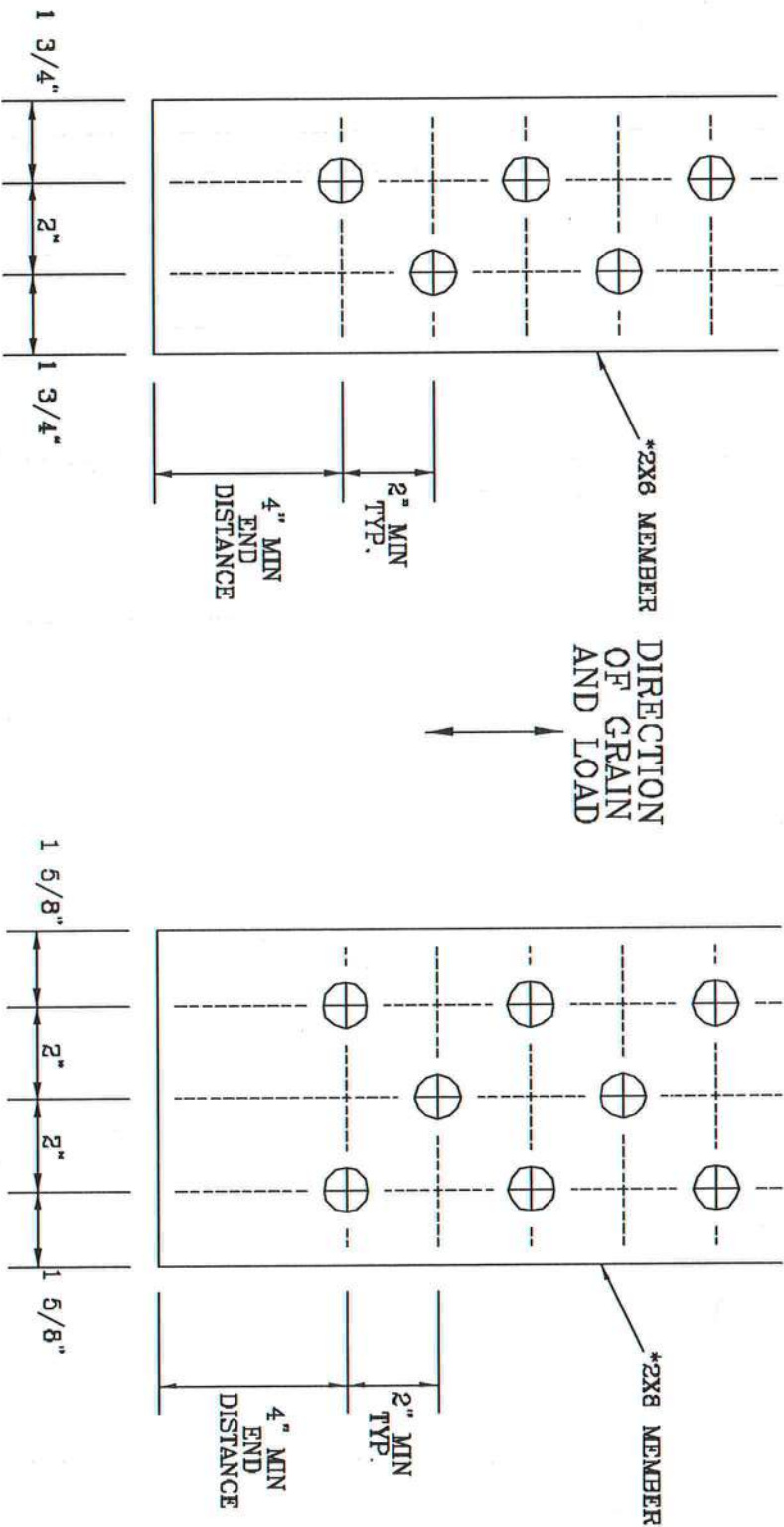
No. 34689
STATE OF FLORIDA

| | | | |
|-----------|------|------|--------------|
| TC IL | PSF | REF | TOE-NAIL |
| TC DL | PSF | DATE | 09/12/07 |
| BC DL | PSF | DRWG | CNTONAIL1103 |
| BC IL | PSF | -ENG | JL |
| TOT. LD. | PSF | | |
| DUR. FAC. | 1.00 | | |
| SPACING | | | |

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

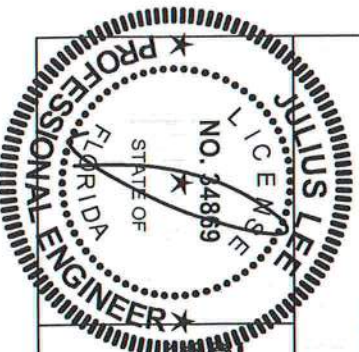
TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A828.016



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTION. REFER TO DETAIL T-203 GUIDING DEPENDENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS ALIANCE INSTITUTE, 280 DOWNSIDE DR., SUITE 204, MADISON, VT. 05750 AND WICA CIVIL TRUSS COUNCIL OF AMERICA, 6000 ENTERPRISE LN, MADISON, VT. 05750 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1420 ST 4TH AVENUE
DELMAR BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

| | | | |
|-----------|-----|------|--------------|
| TC LL | PSF | REF | BOLT SPACING |
| TC DL | PSF | DATE | 11/26/03 |
| BC DL | PSF | DRWG | CNBOLTSPI103 |
| BC LL | PSF | -ENG | JL |
| TOT. LD. | PSF | | |
| DUR. FAC. | | | |
| SPACING | | | |

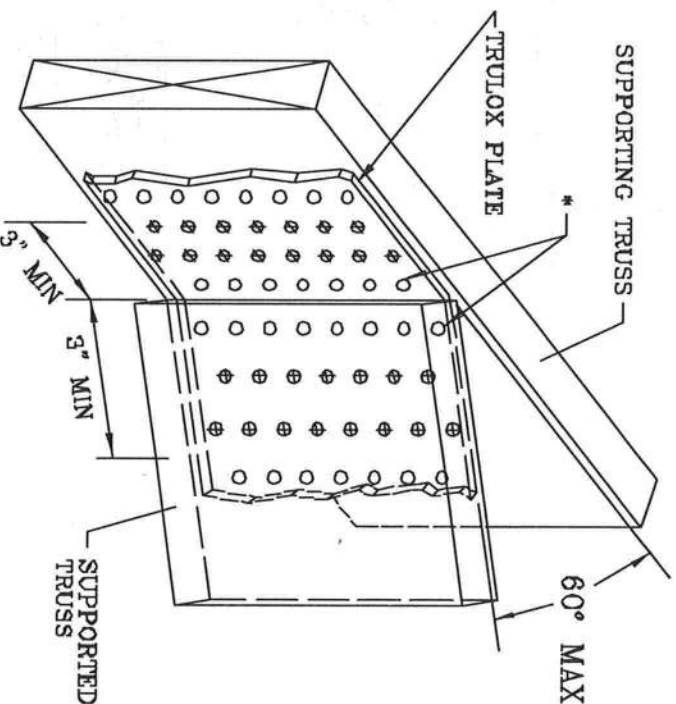
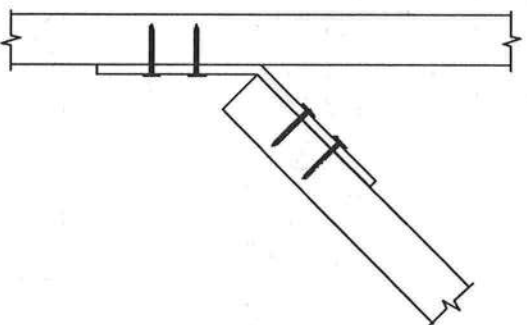
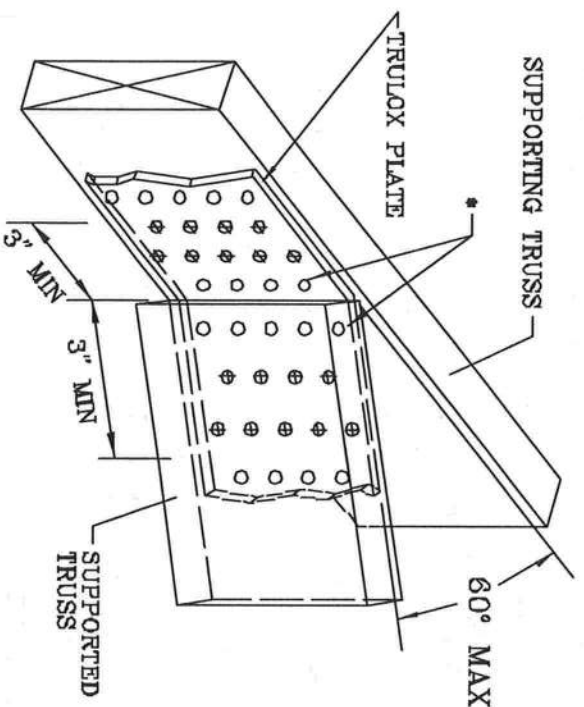
TRULOX CONNECTION DETAIL

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.
REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE

| TRULOX PLATE SIZE | REQUIRED NAILS PER TRUSS | MAXIMUM LOAD UP OR DOWN |
|-------------------|--------------------------|-------------------------|
| 3X6 | 9 | 350 # |
| 5X6 | 15 | 990 # |

REVIEWED

By Julius Lee at 11:58 am, Jun 11, 2008

MINIMUM 5X6 TRULOX PLATE

THIS DRAWING REPLACES DRAWINGS 1,156,989 1,158,989/R
1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO AISC 308 BUILDING DEPARTMENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 3861 DUNSTON DR., SUITE 200, MARIETTA, GA 30067 AND VITA CYCLO TRUSS COUNCIL, 1000 N. 10TH AVE., SUITE 100, DENVER, CO 80202 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SW 4th AVE
DELRAY BEACH, FL 33444-2801

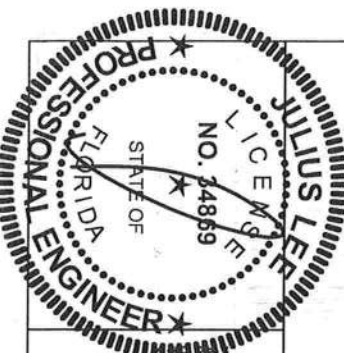
REF TRULOX

DATE 11/26/03

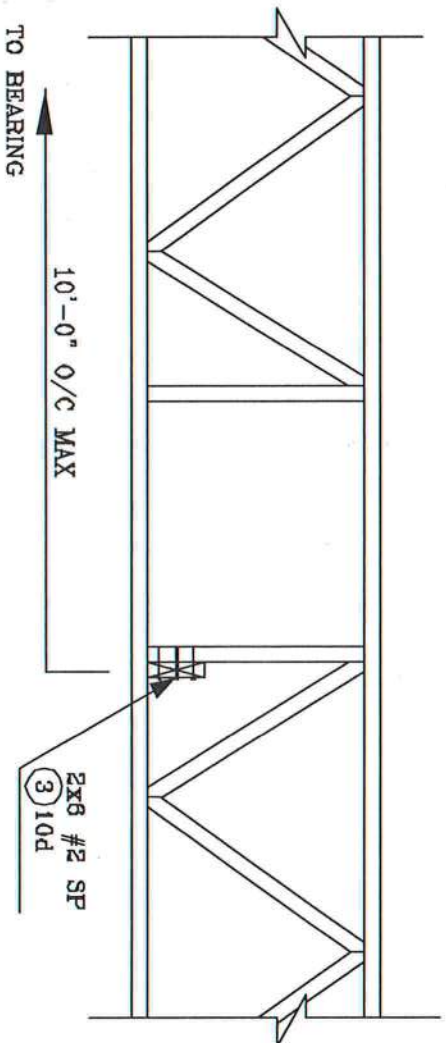
DRWG CTRULOX1103

-ENG JL

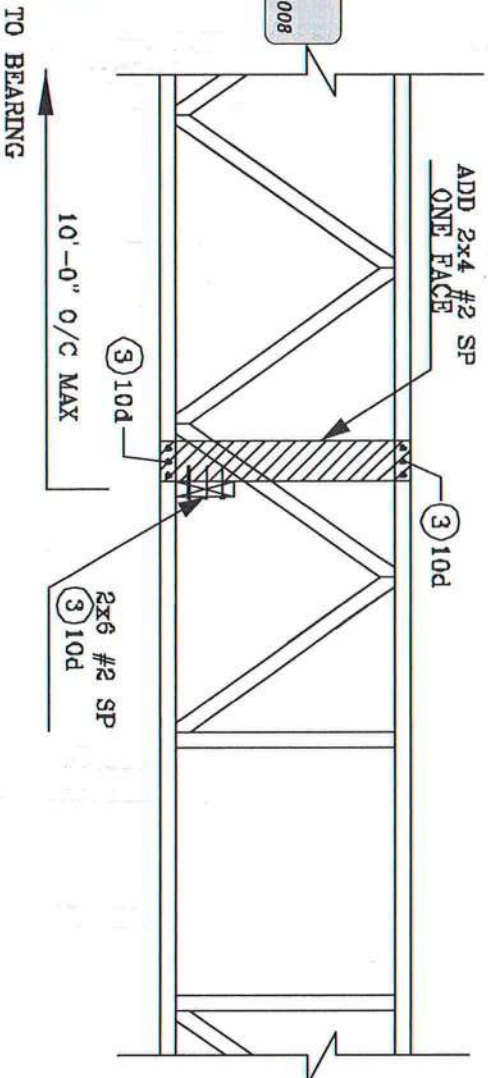
No: 344869
STATE OF FLORIDA



STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS

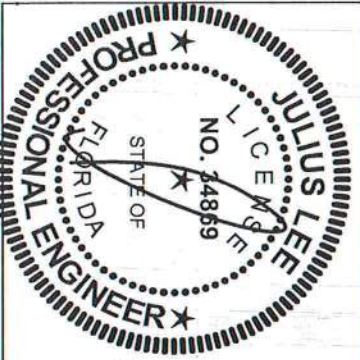


ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED

By Julius Lee at 11:58 am, Jun 11, 2008



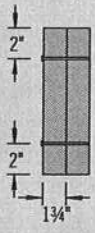
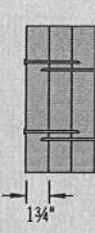
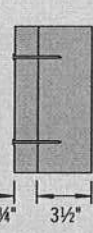
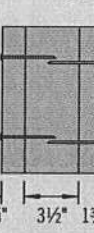
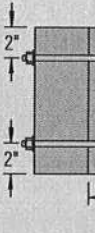

JULIUS LEE'S
CONS. ENGINEERS P.A.

1426 SW 4th AVENUE
DEER BEACH, FL 33442-2601

No. 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

| Connector Type | Number of Rows | Connector On-Center Spacing | Connector Pattern | | | | | |
|---|----------------|-----------------------------|---|---|---|--|---|---|
| | | | Assembly A | Assembly B | Assembly C | Assembly D | Assembly E | Assembly F |
| | | |  |  |  |  |  |  |
| | | | 3 1/2" 2-ply | 5 1/4" 3-ply | 5 1/4" 2-ply | 7" 3-ply | 7" 2-ply | 7" 4-ply |
| 10d (0.128" x 3") Nail ⁽¹⁾ | 2 | 12" | 370 | 280 | 280 | 245 | | |
| | 3 | 12" | 555 | 415 | 415 | 370 | | |
| 1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾ | 2 | 24" | 505 | 380 | 520 | 465 | 860 | 340 |
| | | 19.2" | 635 | 475 | 655 | 580 | 1,075 | 425 |
| | | 16" | 760 | 570 | 785 | 695 | 1,290 | 505 |
| SDS 1/4" x 3 1/2" ⁽⁴⁾ | 2 | 24" | 680 | 510 | 510 | 455 | | |
| | | 19.2" | 850 | 640 | 640 | 565 | | |
| | | 16" | 1,020 | 765 | 765 | 680 | | |
| SDS 1/4" x 6" ⁽³⁾⁽⁴⁾ | 2 | 24" | | | | 455 | 465 | 455 |
| | | 19.2" | | | | 565 | 580 | 565 |
| | | 16" | | | | 680 | 695 | 680 |
| USP WS35 ⁽⁴⁾ | 2 | 24" | 480 | 360 | 360 | 320 | | |
| | | 19.2" | 600 | 450 | 450 | 400 | | |
| | | 16" | 715 | 540 | 540 | 480 | | |
| USP WS6 ⁽³⁾⁽⁴⁾ | 2 | 24" | | | | 350 | 525 | 350 |
| | | 19.2" | | | | 440 | 660 | 440 |
| | | 16" | | | | 525 | 790 | 525 |
| 3 3/8" TrussLok ⁽⁴⁾ | 2 | 24" | 635 | 475 | 475 | 425 | | |
| | | 19.2" | 795 | 595 | 595 | 530 | | |
| | | 16" | 955 | 715 | 715 | 635 | | |
| 5" TrussLok ⁽⁴⁾ | 2 | 24" | | 500 | 500 | 445 | 480 | 445 |
| | | 19.2" | | 625 | 625 | 555 | 600 | 555 |
| | | 16" | | 750 | 750 | 665 | 725 | 665 |
| 6 3/4" TrussLok ⁽⁴⁾ | 2 | 24" | | | | 445 | 620 | 445 |
| | | 19.2" | | | | 555 | 770 | 555 |
| | | 16" | | | | 665 | 925 | 665 |

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 1/16" maximum.

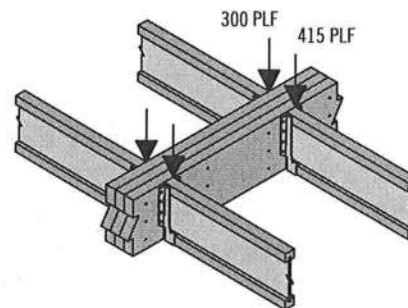
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

| Connector Type | Number of Connectors | Connector Pattern | | | | | |
|--|----------------------|-------------------|----------------------|--------------|------------|----------------------|----------------------|
| | | Assembly A | Assembly B | Assembly C | Assembly D | Assembly E | Assembly F |
| | | | | | | | |
| | | 3 1/2" 2-ply | 5 1/4" 3-ply | 5 1/4" 2-ply | 7" 3-ply | 7" 2-ply | 7" 4-ply |
| 10d (0.128" x 3") Nail | 6 | 1,110 | 835 | 835 | 740 | | |
| | 12 | 2,225 | 1,670 | 1,670 | 1,485 | | |
| | 18 | 3,335 | 2,505 | 2,505 | 2,225 | | |
| | 24 | 4,450 | 3,335 | 3,335 | 2,965 | | |
| SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6 ⁽¹⁾ | 4 | 1,915 | 1,435 ⁽⁴⁾ | 1,435 | 1,275 | 1,860 ⁽²⁾ | 1,405 ⁽²⁾ |
| | 6 | 2,870 | 2,150 ⁽⁴⁾ | 2,150 | 1,915 | 2,785 ⁽²⁾ | 2,110 ⁽²⁾ |
| | 8 | 3,825 | 2,870 ⁽⁴⁾ | 2,870 | 2,550 | 3,715 ⁽²⁾ | 2,810 ⁽²⁾ |
| 3 3/4" or 5" TrussLok™ | 4 | 2,545 | 1,910 ⁽⁴⁾ | 1,910 | 1,695 | 1,925 ⁽³⁾ | 1,775 ⁽²⁾ |
| | 6 | 3,815 | 2,860 ⁽⁴⁾ | 2,860 | 2,545 | 2,890 ⁽³⁾ | 2,665 ⁽²⁾ |
| | 8 | 5,090 | 3,815 ⁽⁴⁾ | 3,815 | 3,390 | 3,855 ⁽³⁾ | 3,550 ⁽²⁾ |

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

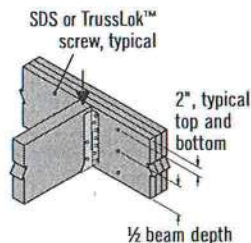
(2) 6" long screws required.

(3) 5" long screws required.

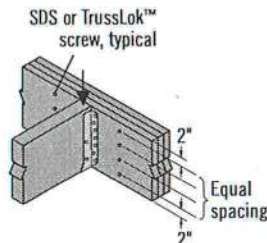
(4) 3 1/2" and 3 3/4" long screws must be installed on both sides.

Connections

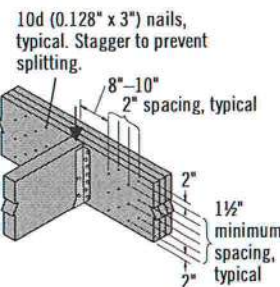
4 or 6 or Screw Connection



8 Screw Connection

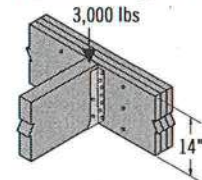


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

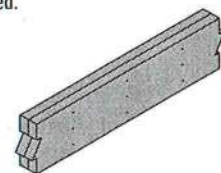
on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

L6