

REVISIONS SOFTPLAN

ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4
 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY
 BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
 BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE
 DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
 DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC

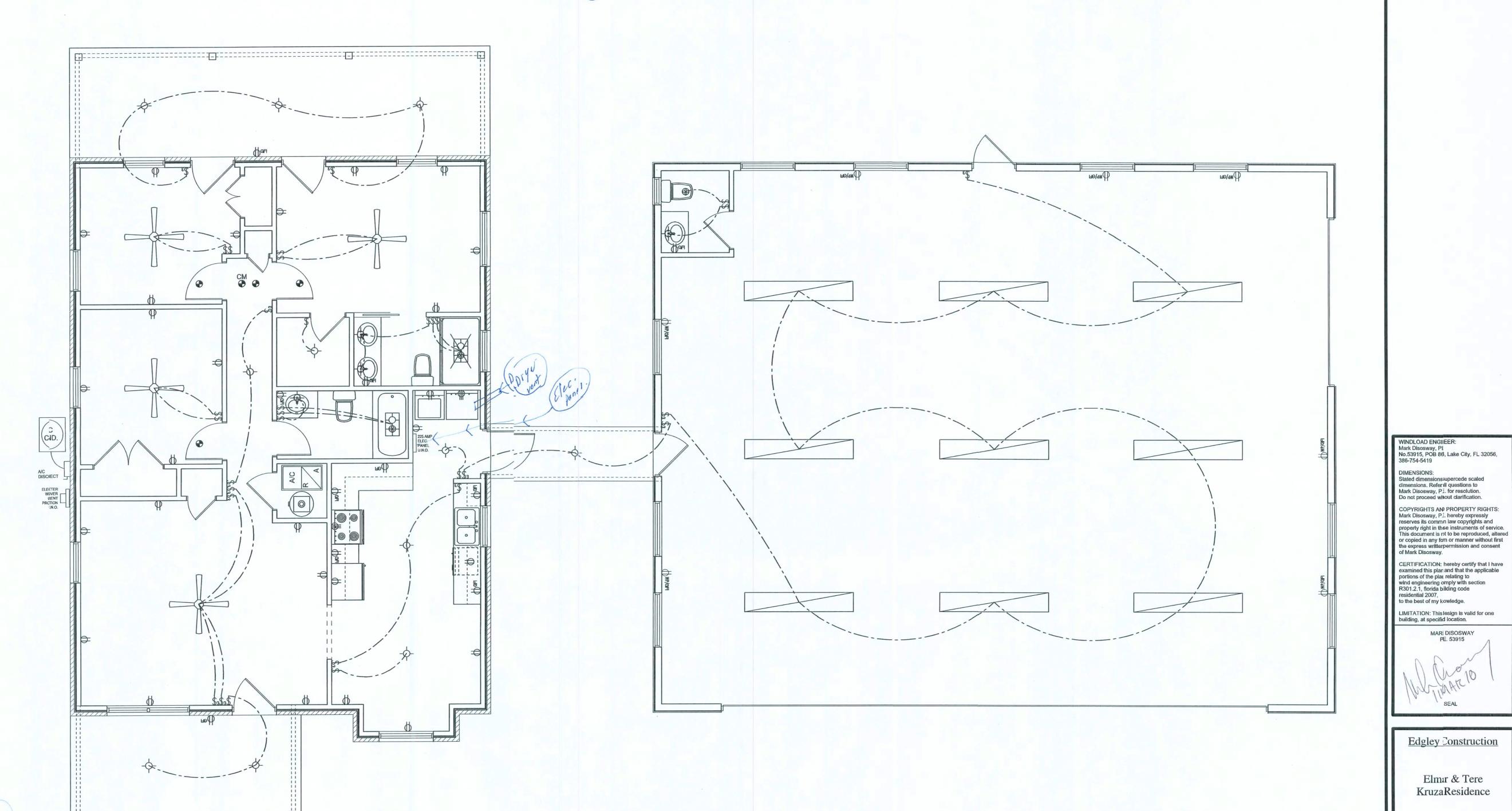
- E -10 CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL
- CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10'

 OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING
 A FOSSIL-FLIEL BUIRNING HEATER OR ARBUMANCE A FOSSIL-FLIEL BUIRNING HEATER OR ARBUMANCE. A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
QD	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
0	RECESSED CAN LIGHT
- ♦ - \	BATH EXAUST FAN WITH LIGHT
₩	BATH EXAUST FAN
- \(-	LIGHT FIXTURE
Ф	DUPLEX OUTLET
₩	220v OUTLET
⊕ _{GPI}	GFI DUPLEX OUTLET
•	SMOKE DETECTOR
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
\$4	4 WAY WALL SWITCH
∯ _{WP/GFI}	WATER PROOF GFI OUTLET
∇	PHONE JACK
0	TELEVISION JACK
頁	GARAGE DOOR OPENER
• см	CARBON MONOXIDE ALARM

ELECTRICAL PLAN

SCALE: 1/4" = 1'-0"



Edgley Construction

LIMITATION: This lesign is valid for one building, at specifid location.

WINDLOAD ENGIEER: Mark Disosway, Pt No.53915, POB 86, Lake City, FL 32056, 386-754-5419

Stated dimensions/upercede scaled dimensions. Refer III questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AN PROPERTY RIGHTS:

> Elmır & Tere KruzaResidence

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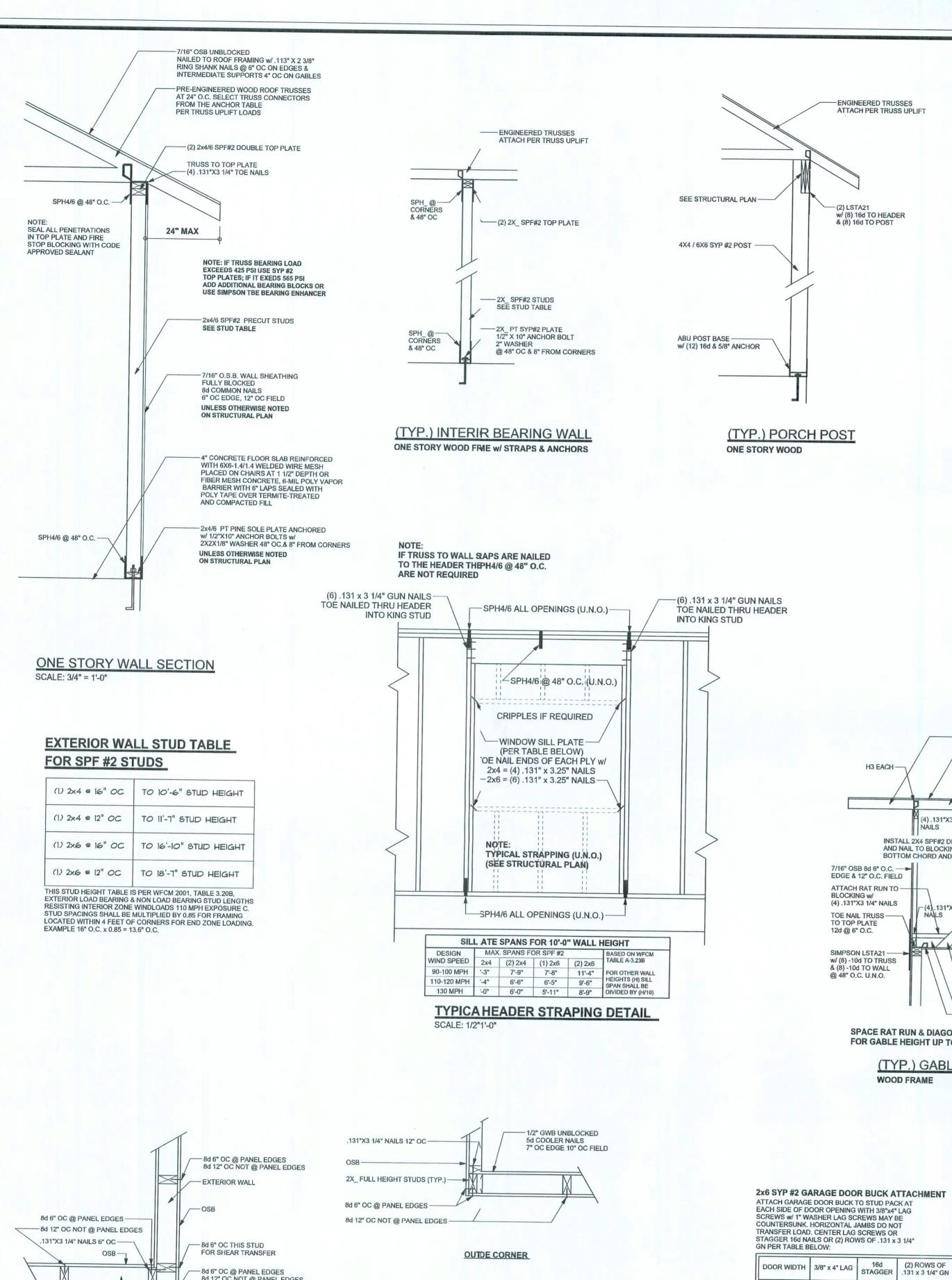
PRIITED DATE: March 11, 2010 DRAWN BY: STRUCTURAL BY:

FINALS DATE:

10Mar10 JOB NUMBER:

1(02066 DRAWNG NUMBER

OF3 SHEETS



- 1/2" GWB UNBLOCKED

7" OC EDGE 10" OC FIELD

5d COOLER NAILS

-8d 6" OC @ PANEL EDGES

INSIECORNER

8d 12" OC NOT @ PANEL EDGES -

8d 12" OC NOT @ PANEL EDGES

2X_ FULL HEIGHT STUDS (TYP.) -

.131"X3 1/4" NAILS 12" OC -

- 2X_ FULL HEIGHT STUDS (TYP.)

- 8d 6" OC @ PANEL EDGES

8d 12" OC NOT @ PANEL EDGES

INTERIOR SHEARWALL

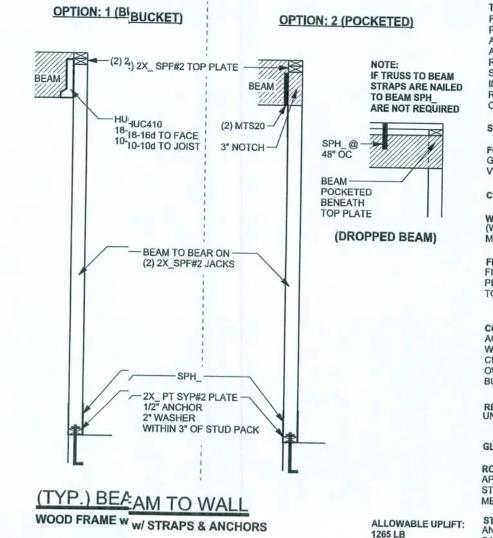
1/2" GWB UNBLOCKED -

5d COOLER NAILS 7" OC EDGE 10" OC FIELD

WOOD FRAME

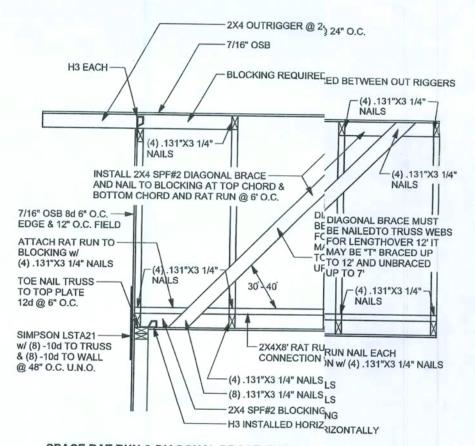
.131"X3 1/4" NAILS 12" OC ---

(TYP.) INTERSECTING WALL FRAMING



GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C. C. ENCLOSED (TYP.) GABLE BRACING DEETAIL

PRE ENGINEERED ROOF TRUSS -DOUBLE 2x4 SPF TOP PLATE NAILED -TOGETHER W/2-16d NAILS AT 16" O.C. 4' MIN. LAP w/ (12) - 16d OR 4" LAP w/ CS20 w/ (4) - 16d &(14) - 10d INTERIOR CEILING AS -SPECIFIED ON FLOOR PLAN CONTINUOUS FRAME -TO TOP PLATE AT BOTTOM CHORD OF TRUSS

GARAGE DOOR BUCK INSTALLATION DETAIL SCALE: N.T.S.

8' - 10'

11' - 15'

2x6SYP #2 DOOR BUCK ---

24" O.C.

18" O.C.

16" O.C. 3" O.C.

5" O.C. 5" O.C.

4" O.C.

3" O.C.

4" O.C.

AND BOTTOM PLATES WITH 2-16d NAILS

CONTINUOUS FRAME TO **CEILING DIAPHRAGM DETAIL** SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO CT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W 1.4 x W 1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION

R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN

TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS

THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE

PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED

LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO

SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL

RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE

MASONRY NOTES:

ACI530.1-02 Section

Clay brick standard

Reinforcing bars, #3 - #11

2.4F Coating for corrosion protection

2.4F Coating for corrosion protection

1.4A Compressive strength

Mortar

2.3 CMU standard

3.3.E.7 Movement joints

2.2 Grout

IN WRITING.

2.3

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

3.3.E.2 Pipes, conduits, and accessories Any not shown on the project drawings

Specific Requirements

5.5"x2.75"x11.5"

ASTM C 270, Type N, UNO

8" block bearing walls F'm = 1500 psi

ASTM C 90-02, Normal weight, Hollow,

bond and 12"x12" or 16"x16" column

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap

splices min 48 bar dia. (30" for #5)

embedded in mortar or grout, ASTM

Joint reinforcement in walls exposed to

require engineering approval.

detailed on project drawings.

moisture or wire ties, anchors, sheet metal

ties not completely embedded in mortar or

grout, ASTM A153, Class B2, 1.50 oz/ft2

Contractor assumes responsibility for type

and location of movement joints if not

A525, Class G60, 0.60 oz/ft2 or 304SS

Anchors, sheet metal ties completely

medium surface finish, 8"x8"x16" running

ASTM C 476, admixtures require approval

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS

MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN

REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF

BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF

DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT

TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

THE BUILDER AN SPECIFICALLY NO	D OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE OT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE COND BACKFILL HEIGHT, WI	ITIONS, FOUNDATION BEARING CAPACITY, GRADE AND ND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS REQUIREMENTS FOR	AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUO BELIEVE THE PLAN OI THE WIND LOAD ENGI	DUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU MITS A CONTINUOUS LOAD PATH CONNECTION, CALL NEER IMMEDIATELY.
DESIGN, PLACEMENT	ANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, NNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

PLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24	12 104 1 1/2	12-100 1 1/2	
< 2050	< 1785	LGT2	14 -16d	14 -16d	
				14-100	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED RC 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED RC 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
1 105		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		(a) (a) - a) (a) (b) (b)
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		JIO AB
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		dinu ar
< 2300	< 2300	ABU66	12-16d		1/2" AB
	< 2320	. 1000	12-100		1/2" AB

386-754-5419 DIMENSIONS: Stated dimensions supercede scaled

dimensions. Reer all questions to Mark DisoswayP.E. for resolution. Do not proceedwithout clarification. COPYRIGHTS IND PROPERTY RIGHTS: Mark Disosway P.E. hereby expressly reserves its cormon law copyrights and property right inhese instruments of service

This document; not to be reproduced, altered or copied in anyform or manner without first the express writen permission and consent of Mark Disoswy. CERTIFICATIOI: I hereby certify that I have

examined this pan, and that the applicable portions of the pan, relating to wind engineerin comply with section R301.2.1, floridabuilding code residential 2007 to the best of myknowledge.

EdgleyConstruction

Elmar & Tere

Kruzi Residence

ADDRESS: Lot 22 Country Landings S/D Columbia County, Florida

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PRITED DATE: Marcl 11, 2010 DRAWN BY: STRUCTURAL BY: David Disosway David Disosway

FINALS DATE: 10Mar10

JOB NUMBER: 1(02066 DRAWNG NUMBER

> **S-1** OF | SHEETS

DESIGN DATA

	HURRICANE ZON	IE			
BUILDING IS NOT IN THE WIND-BORNE DE	BRIS REGION				
1.) BASIC WIND SPEED = 110 MPH					
2.) WIND EXPOSURE = C					
3.) WIND IMPORTANCE FACTOR = 1.0					
4.) BUILDING CATEGORY = II					
5.) ROOF ANGLE = 10-45 DEGREES					
6.) MEAN ROOF HEIGHT = <30 FT					
7.) INTERNAL PRESSURE COEFFICIENT =	N/A (ENCLOSED	BUILD	ING)		
8.) COMPONENTS AND CLADDING DESIGN				R301 20	(2))
A B	Zone			ind Ar	ea (ft2
	-	_	0		100
2 2	1 2	27.8	-30.5 -35.7	_	-25.3
1	7 2 O'hg	27.0	-56.8	25.3	-30.5
2 2 2 1	3	27.8	-35.7	25.3	-30.5
9 \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \					
4 3 4	3 O'hg		-95.6		-59.3
3 4	4	30.5	-95.6 -33.0	25.9	-
3 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5		30.5 30.5		25.9 25.9	-28.5
3 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	4	30.5	-33.0 -40.7		-28.5 -31.6
3 4 55 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	4 5 Doors a	30.5 & Wind	-33.0 -40.7 lows	25.9	-28.5 -31.6
3 3 4 3 4 5 5 5 5 5 5	4 5 Doors a Wors (Zone	30.5 & Wind st Case 5, 10	-33.0 -40.7 lows et2)	25.9	-28.5 -31.6
4 3 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	4 5 Doors a Wors (Zone 8x7 Gara	30.5 & Wind st Case 5, 10 f	-33.0 -40.7 lows (t2)	25.9 30.5 27.3	-28.5 -31.6 -40.7
4 3 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	4 5 Doors a Wors (Zone	30.5 & Wind st Case 5, 10 f	-33.0 -40.7 lows (t2)	25.9 30.5	-28.5 -31.6 -40.7
3 4 3 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	4 5 Doors a Wors (Zone 8x7 Gara	30.5 & Wind st Case 5, 10 f	-33.0 -40.7 lows (t2)	25.9 30.5 27.3	-59.3 -28.5 -31.6 -40.7 -32.0 -29.4
3 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	4 5 Doors a Wors (Zone 8x7 Gara	30.5 & Wind st Case 5, 10 f	-33.0 -40.7 lows (t2)	25.9 30.5 27.3	-28.5 -31.6 -40.7
4 3 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	4 5 Doors a Wors (Zone 8x7 Gara	30.5 & Wind st Case 5, 10 f	-33.0 -40.7 lows (t2)	25.9 30.5 27.3	-28.5 -31.6 -40.7

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSI

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

REVISIONS

SOFTPIAN

LIMITATION: The design is valid for one building, at specied location. P.E. 53915

