Columbia County New Building Permit Application

For Office Use Only Application # 6 079 Date Received By 6 Permit #
Zoning Official Date Flood Zone Land Use Zoning
FEMA Map # Elevation MFE River Plans Examiner Date Comments
□ NOC □ EH □ Deed or PA □ Site Plan □ State Road Info □ Well letter □ 911 Sheet □ Parent Parcel #
□ Dev Permit # □ In Floodway □ Letter of Auth. from Contractor □ F W Comp. letter
□ Owner Builder Disclosure Statement □ Land Owner Affidavit □ Ellisville Water □ App Fee Paid □ Sub VF Form
Septic Permit No OR City Water Fax
Applicant (Who will sign/pickup the permit) Monica Cannon Phone 386-344-3385
Address 208 SW Plymoreth Are
Owners Name Manual Carren Phone 386-344-3386
911 Address 495 Sw Jordan St. F. White, Fr 32038
Contractors NamePhone
Address
Contractor Email MUSS Cannon 1 @ yahw-Con ***Include to get updates on this job.
Fee Simple Owner Name & Address
Bonding Co. Name & Address
Architect/Engineer Name & Address
Mortgage Lenders Name & Address
Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Duke Energy
Property ID Number 33-65-16-04031-000 Estimated Construction Cost #280,000
Subdivision NameLotLotLotLotPhase
Driving Directions from a Major Road For White: Right hum onto Hwy 27 from Hwy 47.
2 1/10 of a mile turn left into SW Jordan Ot. @ Property Located
You mile on the left (Pave road ends)
Construction of Single Family Home Commercial OR Residential
Proposed Use/Occupancy Primary Residence Number of Existing Dwellings on Property p
Is the Building Fire Sprinkled? KO If Yes, blueprints included Or Explain
Circle Proposed - Culvert Permit or <u>Culvert Waiver</u> or <u>D.O.T. Permit</u> or <u>Have an Existing Drive</u>
Actual Distance of Structure from Property Lines - Front Side Side Rear
Number of Stories 2 Heated Floor Area 3293 Total Floor Area 4,088 Acreage 6.38 acres
Zoning Applications applied for (Site & Development Plan, Special Exception, etc.)

Columbia County Building Permit Application - "Owner and Contractor Signature Page"

CODES: 2020 Florida Building Code 7th Edition and the 2017 National Electrical Code.

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

<u>TIME LIMITATIONS OF APPLICATION</u>: An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless pursued in good faith or a permit has been issued.

<u>TIME LIMITATIONS OF PERMITS:</u> Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO CONTRACTOR AND AGENT: *YOU ARE HEREBY NOTIFIED* as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

<u>NOTICE TO OWNER:</u> There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

Monica Cannon Printed Owners Name	Mm/o. Owners Signature	before any permit will be issued.
	ill the above written responsibili	ee that I have informed and provided this ities in Columbia County for obtaining itions.
	Contractor	's License Number
Contractor's Signature	Columbia C Competend	County cy Card Number
Affirmed and subscribed before me the	<u>Contractor</u> by means of phys	or produced ID
State of Florida Notary Signature (For the	SEAL:	

Revised 1-12-21



COLUMBIA COUNTY BUILDING DEPARTMENT

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055 Office: 386-758-1008 Fax: 386-758-2160

OWNER BUILDER DISCLOSURE STATEMENT

Florida Statutes Chapter 489.103:

- 1. I understand that state law requires construction to be done by a licensed contractor and have applied for an owner-builder permit under an exemption from the law. The exemption specifies that I, as the owner of the property listed, may act as my own contractor with certain restrictions even though I do not have a license.
- 2. I understand that building permits are not required to be signed by a property owner unless he or she is responsible for the construction and is not hiring a licensed contractor to assume responsibility.
- 3. I understand that, as an owner-builder, I am the responsible party of record on a permit. I understand that I may protect myself from potential financial risk by hiring a licensed contractor and having the permit filed in his or her name instead of my own name. I also understand that a contractor is required by law to be licensed in Florida and to list his or her license numbers on permits and contracts.
- 4. I understand that I may build or improve a one-family or two-family residence or a farm outbuilding. I may also build or improve a commercial building if the costs do not exceed \$75,000. The building or residence must be for my own use or occupancy. It may not be built or substantially improved for sale or lease, unless I am completing the requirements of a building permit where the contractor listed on the permit substantially completed the project. If a building or residence that I have built or substantially improved myself is sold or leased within 1 year after the construction is complete, the law will presume that I built or substantially improved it for sale or lease, which violates the exemption.
- 5. I understand that, as the owner-builder, I must provide direct, onsite supervision of the construction.
- 6. I understand that I may not hire an unlicensed person to act as my contractor or to supervise persons working on my building or residence. It is my responsibility to ensure that the persons whom I employ have the licenses required by law and by county or municipal ordinance.

Revision Date: 8/15/2019

- 7. I understand that it is a frequent practice of unlicensed persons to have the property owner obtain an owner-builder permit that erroneously implies that the property owner is providing his or her own labor and materials. I, as an owner-builder, may be held liable and subjected to serious financial risk for any injuries sustained by an unlicensed person or his or her employees while working on my property. My homeowner's insurance may not provide coverage for those injuries. I am willfully acting as an owner-builder and am aware of the limits of my insurance coverage for injuries to workers on my property.
- 8. I understand that I may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on my building who is not licensed must work under my direct supervision and must be employed by me, which means that I must comply with laws requiring the withholding of federal income tax and social security contributions under the Federal Insurance Contributions Act (FICA) and must provide workers' compensation for the employee. I understand that my failure to follow these laws may subject me to serious financial risk.
- 9. I agree that, as the party legally and financially responsible for this proposed construction activity, I will abide by all applicable laws and requirements that govern owner-builders as well as employers. I also understand that the construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.
- 10. I understand that I may obtain more information regarding my obligations as an employer from the Internal Revenue Service, the United States Small Business Administration, the Florida Department of Financial Services, and the Florida Department of Revenue. I also understand that I may contact the Florida Construction Industry Licensing Board at 850-487-1395 or http://www.myfloridalicense.com/ for more information about licensed contractors.
- 11. I am aware of, and consent to, an owner-builder building permit applied for in my name and understand that I am the party legally and financially responsible for the proposed construction activity at the following address:

495 SW Sordan St. For While, FL 32038
(Write in the address of jobsite property)

12. I agree to notify Columbia County Building Department immediately of any additions, deletions, or changes to any of the information that I have provided on this disclosure. Licensed contractors are regulated by laws designed to protect the public. If you contract with a person who does not have a license, the Construction Industry Licensing Board and Department of Business and Professional Regulation may be unable to assist you with any financial loss that you sustain as a result of a complaint. Your only remedy against an unlicensed contractor may be in civil court. It is also important for you to understand that, if an unlicensed contractor or employee of an individual or firm is injured while working on your property, you may be held liable for damages. If you obtain an owner-builder permit and wish to hire a licensed contractor, you will be responsible for verifying whether the contractor is properly licensed and the status of the contractor's workers' compensation coverage.

Florida Statutes Chapter 489.503:

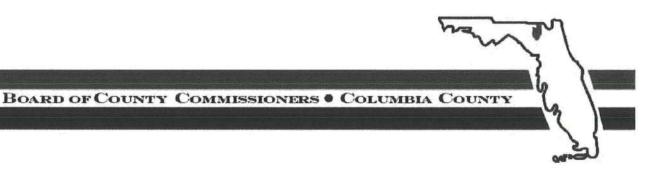
State law requires electrical contracting to be done by licensed electrical contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own electrical contractor even though you do not have a license. You may install electrical wiring for a farm outbuilding or a single-family or duplex residence. You may install electrical wiring in a commercial building the aggregate construction costs of which are under \$75,000. The home or building must be for your own use and occupancy. It may not be built for sale or lease, unless you are completing the requirements of a building permit where the contractor listed on the permit substantially completed the project. If you sell or lease more than one building you have wired yourself within 1 year after the construction is complete, the law will presume that you built it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person as your electrical contractor. Your construction shall be done according to building codes and zoning regulations. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances.

An owner of property completing the requirements of a building permit, where the contractor listed on the permit substantially completed the project as determined by the local permitting agency, for a one-family or two family residence, townhome, accessory structure of a one-family or two-family residence or townhome or individual residential condominium unit or cooperative unit. Prior to the owner qualifying for the exemption, the owner must receive approval from the local permitting agency, and the local permitting agency must determine that the contractor substantially completed the project. An owner who qualifies for the exemption under this paragraph is not required to occupy the dwelling or unit for at least 1 year after the completion of the project.

Revision Date: 8/15/2019

Before a building permit shall be issued, this notarized disclosure statement must be completed and signed by the property owner and returned to the local permitting agency responsible for issuing the permit.

TYPE OF CONSTRUCTION (i) Single Family Dwelling () Two-Family Residence () Farm Outbuilding
() Addition, Alteration, Modification or other Improvement () Electrical
() Other
() Contractor substantially completed project, of a
() Commercial, Cost of Construction for construction of
Nonical Cannon have been advised of the above disclosure (Print Property Owners Name) statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes allowing this exception for the construction permitted by Columbia County Building Permit. Signature:
NOTARY OF OWNER BUILDER SIGNATURE The above signer is personally known to me or produced identification The above signer is personally known to me or produced identification Date 6/16/23 (Seal)
Notary Public State of Florida Feresa Candelaria My Commission HH 170005 Exp. 8/26/2025



Address Assignment and Maintenance Document

To maintain the county wide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for addressing and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Services Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County

Date/Time Issued:

6/7/2022 9:42:17 AM

Address:

495 SW JORDAN St

City:

FORT WHITE

State:

FL

Zip Code

32038

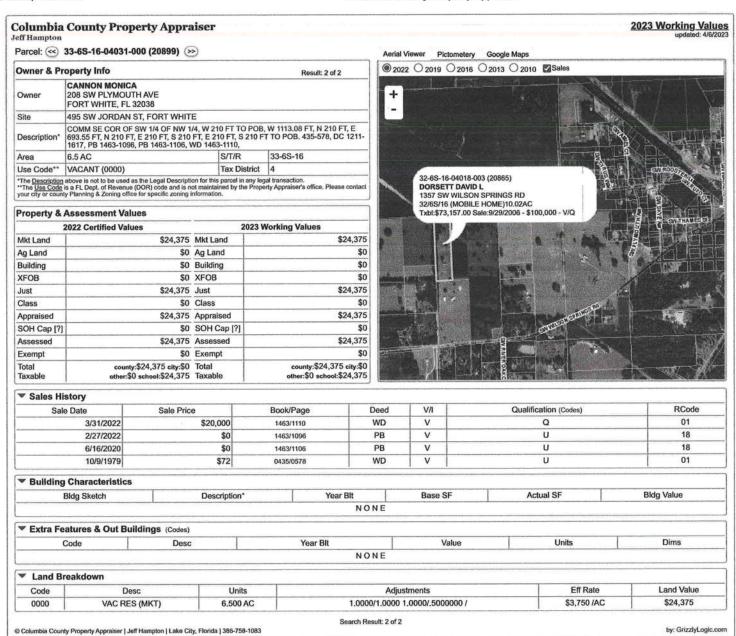
Parcel ID

04031-000

REMARKS: New address for Habitable structure (family home, business, etc.) on the parcel.

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION AND ACCESS INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION AND/OR ACCESS INFORMATION BE FOUND TO BE IN ERROR OR CHANGED, THIS ADDRESS IS SUBJECT TO CHANGE.

Address Issued By: SCHOFIELD, LINCOLN C.





COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL CHECK LIST

MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2020 EFFECTIVE 1 JANUARY 2021 AND THE NATIONAL ELECTRICAL 2017 EFFECTIVE 1 JANUARY 2021

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT FLORIDA BUILDING CODES RESIDENTIAL AND THE NATIONAL ELECTRICAL CODE. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS, FBC 1609.1 THRU 1609.6.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609.3(1)
THROUGH 1609.3(4) ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER
STRUCTURES Revised 7/1/20

Total (Sq. Ft.) under roof

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal

Items to Include-Each Box shall be

Circled as

Applicable
Selegt From Drop down

No

Yes

NA

Submit Online at- http://www.columbiacountyfla.com/BuildingandZoning.asp

GENERAL REQUIREMENTS:

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

2 All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void

shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES BUILDING 107.1.

1 Two (2) complete sets of plans containing the following:

Condition space (Sq. Ft.)

Site Plan information including

~~	2 164 mormation metading.	Name and Address of the Owner, when the Owner, where the Owner, which is the Owner,	and the same of the same of	
4	Dimensions of lot or parcel of land	-/		
5	Dimensions of all building set backs		1.2	
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	1		
7	Provide a full legal description of property.	1		
W	nd-load Engineering Summary, calculations and any details are required.			
160	GENERAL REQUIREMENTS:	Item	s to Inclu	de-
	APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	C	Box shall ircled as plicable	
8	Plans or specifications must show compliance with FBCR Chapter 3	Yes	No	NA
_		Select Fre	om Drop	down
9	Basic wind speed (3-second gust), miles per hour	~		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	~		
11	Wind importance factor and nature of occupancy	/		
12	The applicable internal pressure coefficient, Components and Cladding	1	Charles (Nove Accord	
13	The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component, cladding materials not specifally designed by the registered design professional.	V		
Ele	vations Drawing including:			
14	All side views of the structure	1-1		
15	Roof pitch	- V		
16	Overhang dimensions and detail with attic ventilation	- V		
17	Location, size and height above roof of chimneys	-/		
8	Location and size of skylights with Florida Product Approval	-V		
19	Number of stories	-V		
20	Building height from the established grade to the roofs highest peak	- 1		

	Floor Plan Including:		ern	
	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches,		· ·	T
21	deck, balconies		X	
22	Raised floor surfaces located more than 30 inches above the floor or grade		1	+
23	All exterior and interior shear walls indicated	-1/	-	-
24	Shear wall opening shown (Windows, Doors and Garage doors)	1	-	-
25	Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each		1	-
23	bedroom (net clear opening shown) and Show compliance with Section FBCR 312.2.1 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass.	-/		
26	Safety glazing of glass where needed	-1/	V	
27	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 and chapter 24 of FBCR)	./	V	
28	Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails	· V	V	
29	Identify accessibility of bathroom (see FBCR SECTION 320)	- /	1	
	GENERAL REQUIREMENTS:		to Inclu	
	APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Each I	Box sha reled as oplicable	ll be
<u>FB</u>		Each I Ci Ar	Box sha rcled as plicable	ll be
FB 30	APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Each I Ci Ar	Box sha rcled as plicable	ll be
30	APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL CR 403: Foundation Plans Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size	Each I Ci Ar	Box sha rcled as plicable	ll be
30	APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL CR 403: Foundation Plans Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	Each I	Box sha rcled as plicable	ll be
	CR 403: Foundation Plans Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing. All posts and/or column footing including size and reinforcing Any special support required by soil analysis such as piling. Assumed load-bearing valve of soil Pound Per Square Foot	Each I Ci Ar Select F	Box sha rcled as plicable	ll be
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FI	oor Framing System: First and/or second story Floor truss package shall including layout and details, signed and sealed by Florida Registered			
	Floor truss package shall including layout and details, signed and sealed by Florida Registered	1	Т	
40	Professional Engineer	1 -	1	
41	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers	-	1	
12	Girder type, size and spacing to load bearing walls, stem wall and/or priers	-	-/	
13	Attachment of joist to girder	-	V	
44	Wind load requirements where applicable	-	1	
15	Show required under-floor crawl space			
46	Show required amount of ventilation opening for under-floor spaces	-		-
47	Show required covering of ventilation opening	-	1	
18	Show the required access opening to access to under-floor spaces	-		
	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges &		/	
19	intermediate of the areas structural panel sheathing	-	V	
50	Show Draftstopping, Fire caulking and Fire blocking	_	V	F10522
51	Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6	-	/	
52	Provide live and dead load rating of floor framing systems (psf).		V	
7R	CR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION	110		
D	CR CHAI TER WOOD WALLETRAMING CONSTRUCTION	Items	to Includ	٥
	GENERAL REQUIREMENTS:	THE REST OF THE PARTY OF THE PA	Box shall	
	APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	CONTROL CONTRO	rcled as	
		CASTELLOS DE LOS DOS DESENTOS	plicable	
	Se	elect from		dow
3	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	- /		
4	Fastener schedule for structural members per table FBC 2304.10.1 are to be shown	- V		
	Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural			
55	members, showing fastener schedule attachment on the edges & intermediate of the areas structural	- /		
	panel sheathing	V		
	Show all required connectors with a max uplift rating and required number of connectors and			
6	oc spacing for continuous connection of structural walls to foundation and roof trusses or	/		
	rafter systems	V		
	Show sizes, type, span lengths and required number of support jack studs, king studs	. /		
57	for shear wall opening and girder or header per FBC 2304.3.			
8	Indicate where pressure treated wood will be placed	- /		
	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural	- /		
9	panel sheathing edges & intermediate areas			
0	A detail showing gable truss bracing, wall balloon framing details or/and wall hinge bracing detail	- V		
FI	BC :ROOF SYSTEMS:			
1	Truss design drawing shall meet section FBC 2303.1.1.1 Wood trusses	-1/		
2	Include a layout and truss details, signed and sealed by Florida Professional Engineer	- 1/		
53	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	- /		-1,1
4	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	- /		The
55	Provide dead load rating of trusses	- V	37	11.17
-				
-	BC 2304.4:Conventional Roof Framing Layout		- 1	
$\overline{}$	Rafter and ridge beams sizes, span, species and spacing	-V/		TUTCHU .
7	Connectors to wall assemblies' include assemblies' resistance to uplift rating	- V		
	Valley framing and support details	- /		
9	Provide dead load rating of rafter system	- 1		
B	SC 2304.8 ROOF SHEATHING			
0	Include all materials which will make up the roof decking, identification of structural panel	./		
	sheathing, grade, thickness	- 🗸		
71	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	- V		

ROOF ASSEMBLIES FRC Chapter 15

72 Include all materials which will make up the roof assembles covering

73 Submit Florida Product Approval numbers for each component of the roof assembles covering

FBC Energy Chapter 4

Residential construction shall comply with this code by using the following compliance methods in the FBC Chapter 4, Residential buildings compliance methods. Two of the required forms are to be submitted, N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable		
		elect from	Drop Down	
74				
	Attic space	- V		
-	Exterior wall cavity	-		
77	Crawl space	- V	64-0-6	
1	VAC information	V		
	Submit two copies of a Manual J sizing equipment or equivalent computation study	- V		
79	Exhaust fans shown in bathrooms Mechanical exhaust capacity of 50 cfm intermittent or 20 cfm continuous required	- V		
80	Show clothes dryer route and total run of exhaust duct	- 1	es - Western	
	umbing Fixture layout shown All fixtures waste water lines shall be shown on the foundation lan	I- V I		
	Show the location of water heater	- V		
	ivate Potable Water Pump motor horse power	- V_		
	Reservoir pressure tank gallon capacity	- 1		
	Rating of cycle stop valve if used	- 1/		
EI	ectrical layout shown including			
86		- V		
87	by Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A	- ~		
88	Show the location of smoke detectors & Carbon monoxide detectors	- 🗸		
89	Show service panel, sub-panel, location(s) and total ampere ratings	- V		
90	disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. For structures with foundation which establish new electrical utility companies service	· V		
	connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3			
91	Appliances and HVAC equipment and disconnects	- /		
92	Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed Combination arc-fault circuit interrupter, Protection device.	- /		

Notice Of Commencement:

A notice of commencement form RECORDED in the Columbia County Clerk Office is required to be filed with the Building Department BEFORE ANY INSPECTIONS can be performed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as
	Applicable

ITEMS 95, 96, & 98 Are Required After APPROVAL from the ZONING DEPT. Select from Drop down Building Permit Application A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted. There is a \$15.00 application fee. The completed application with attached documents and application fee can be mailed. 94 Parcel Number The parcel number (Tax ID number) from the Property Appraisers Office (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com Environmental Health Permit or Sewer Tap Approval A copy of a approved 95 Columbia County Environmental Health (386) 758-1058 96 City of Lake City A City Water and/or Sewer letter. Call 386-752-2031 97 Toilet facilities shall be provided for all construction sites 98 Town of Fort White (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit. Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations (Municpde.cpm) CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required. 101 A Flood development permit is also required for AE, Floodway & AH. Development permit cost is \$50.00 Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. County Public Works Dept. determines the size 102 and length of every culvert before instillation and completes a final inspection before permanent power is granted. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required. 911 Address: An application for a 911 address must be applied for and received through the Columbia 103 County Emergency Management Office of 911 Addressing Department (386) 758-1125.

Ordinance Sec. 90-75. - Construction debris. (e) It shall be unlawful for any person to dispose of or discard solid waste, including construction or demolition debris at any place within the county other than on an authorized disposal site or at the county's solid waste facilities. The temporary storage, not to exceed seven days of solid waste (excluding construction and demolition debris) on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance, shall not be deemed a violation of this section. The temporary storage of construction and demolition debris on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance shall not be deemed in violation of this section; provided, however, such construction and demolition debris must be disposed of in accordance with this article prior to the county's issuance of a certificate of occupancy for the premises. The burning of lumber from a construction or demolition project or vegetative trash when done so with legal and proper permits from the authorized agencies and in accordance with such agencies' rules and regulations, shall not be deemed a violation of this section. No person shall bury, throw, place, or deposit, or cause to be buried, thrown, placed, or deposited, any solid waste, special waste, or debris of any kind into or on any of the public streets, road right-of-way, highways, bridges, alleys, lanes, thoroughfares, waters, canals, or vacant lots or lands within the county. No person shall bury any vegetative trash on any of the public streets, road right-of-way, highways, bridges, lanes, thoroughfares, waters, canals, or lots less than ten acres in size within the county.

Disclosure Statement for Owner Builders:

If you as the Applicant will be acting as your own contractor or owner/builder under section 489.103(7) Florida Statutes, you must submit the required notarized Owner Builder Disclosure Statement form.

**This form can be printed from the Columbia County Website on the Building and Zoning page under

Documents. Web address is - http://www.columbiacountyfla.com/BuildingandZoning.asp

Section 105 of the Florida Building Code defines the:

Time limitation of application.

An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

Single-family residential dwelling.

Section 105.3.4 A building permit for a single-family residential dwelling must be issued within 30 working days of application therefor unless unusual circumstances require a longer time for processing the application or unless the permit application fails to satisfy the Florida Building Code or the enforcing agency's laws or ordinances.

Permit intent.

Section 105.4.1: A permit issued shall be constructed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance of a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

If work has commenced.

Section 105.4.1.1: If work has commenced and the permit is revoked, becomes null and void, or expires because of lack of progress or abandonment, a new permit covering the proposed construction shall be obtained before proceeding with the work.

New Permit.

Section 105.4.1.2: If a new permit is not obtained within 180 days from the date the initial permit became null and void, the building official is authorized to require that any work which has been commenced or completed be removed from the building site. Alternately, a new permit may be issued on application, providing the work in place and required to complete the structure meets all applicable regulations in effect at the time the initial permit became null and void and any regulations which may have become effective between the date of expiration and the date if issuance of the new permit.

Work Shall Be:

Section 105.4.1.3: Work shall be considered to be in active progress when the permit has received an approved inspection within 180 days. This provision shall not be applicable in case of civil commotion or strike or when the building work is halted due directly to judicial injunction, order or similar process.

The Fee:

Section 105.4.1.4: The fee for renewal reissuance and extension of a permit shall be set forth by the administrative authority.

Notification:

When the application is approved for permitting the applicant will be notified by phone as to the status by the Columbia County Building & Zoning Department.

Residential System Sizing Calculation

Summary Project Title:

Monica Cannon 495 SW Jordan Street Ft White, FL Project Title: Cannon Residence

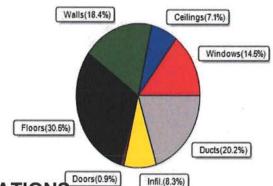
3/28/2023

Location for weather data: Gaine	esville, FL -	Defaults:	Latitude(29.7) Altitude(152 ft.) Ter	mp Range(N	1)		
Humidity data: Interior RH (50%	6) Outdoo	r wet bulb (77F) Humidity difference(51gr.)		•		
Winter design temperature(TMY3 99%) 30 F Summer design temperature(TMY3 99%) 94 F							
Winter setpoint	70	F	Summer setpoint	75	F		
Winter temperature difference	40	F	Summer temperature difference	19	F		
Total heating load calculation	49380	Btuh	Total cooling load calculation	38852	Btuh		
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh		
Total (Electric Heat Pump)	100.0	49380	Sensible (SHR = 0.70)	85.3	27196		
Heat Pump + Auxiliary(0.0kW)	100.0	49380	Latent	167.7	11656		
			Total (Electric Heat Pump)	100.0	38852		

WINTER CALCULATIONS

Winter Heating Load (for 3293 sqft)

Load compone	nt		Load	
Window total	498	sqft	7171	Btuh
Wall total	2563	sqft	9101	Btuh
Door total	24	sqft	442	Btuh
Ceiling total	3458	sqft	3510	Btuh
Floor total	3293	sqft	15041	Btuh
Infiltration	94	cfm	4119	Btuh
Duct loss			9996	Btuh
Subtotal			49380	Btuh
Ventilation	Ex:0 cfm; Sup:0	cfm	0	Btuh
TOTAL HEAT	LOSS		49380	Btuh



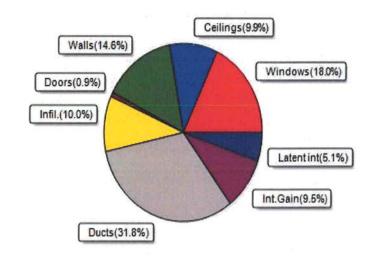
SUMMER CALCULATIONS

Summer Cooling Load (for 3293 sqft)

Load component			Load	
Window total	498	sqft	6997	Btuh
Wall total	2563	sqft	5687	Btuh
Door total	24	sqft	331	Btuh
Ceiling total	3458	sqft	3861	Btuh
Floor total		1 3350	0	Btuh
Infiltration	71	cfm	1467	Btuh
Internal gain			3700	Btuh
Duct gain			9856	Btuh
Sens. Ventilation Ex:0 o	ofm; Sup:0	cfm	0	Btuh
Blower Load			0	Btuh
Total sensible gain			31900	Btuh
Latent gain(ducts)	-	-	2517	Btuh
Latent gain(infiltration)	B	uildi	2435	Btuh
Latent gain(ventilation)	OUNTY BOY	awed	000	Btuh
Latent gain(internal/occur	ants/othe	d.	2000	Btuh
Total latent gain	-		6952	Btuh
TOTAL HEAT GAIN	-:10	Co	38852	Btuh

Code

Compliance



EnergyGauge® System Sizing PREPARED BY: 3 / 28 / 2023

8th Edition

EnergyGauge® / USRCZB v7.5.00

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Monica Cannon 495 SW Jordan Street Ft White, FL Project Title: Cannon Residence Building Type: User

3/28/2023

Reference City: Gainesville, FL (Defaults) Winter Temperature Difference: 40.0 °F (TMY3 99%) Winter Setpoint: 70 °F (Required Manual J default)

Component Loads for Whole House

Window	Panes/Type	Frame	· U	Orientation	Area(sqft) X	HTM=	Load
1	2, NFRC 0.25	TIM	0.36	N	40.0	14.4	576 Btuh
2	2, NFRC 0.25	Vinyl	0.36	N	54.0	14.4	778 Btuh
3	2, NFRC 0.25	Vinyl	0.36	N	24.0	14.4	346 Btuh
4	2, NFRC 0.25	Vinyl	0.36	N	12.0	14.4	173 Btuh
5	2, NFRC 0.25	Vinyl	0.36	N	24.0	14.4	346 Btuh
6	2, NFRC 0.25	Vinyl	0.36	N	24.0	14.4	346 Btuh
7	2, NFRC 0.25	Vinyl	0.36	W	4.0	14.4	58 Btuh
8	2, NFRC 0.25	Vinyl	0.36	W	8.0	14.4	115 Btuh
9	2, NFRC 0.25	Vinyl	0.36	S	30.0	14.4	432 Btuh
10	2, NFRC 0.25	TIM	0.36	S	48.0	14.4	691 Btuh
11	2, NFRC 0.25	Vinyl	0.36	S S S	54.0	14.4	778 Btuh
12	2, NFRC 0.25	Vinyl	0.36	S	54.0	14.4	778 Btuh
13	2, NFRC 0.25	Vinyl	0.36	S	54.0	14.4	778 Btuh
14	2, NFRC 0.25	Vinyl	0.36	E	20.0	14.4	288 Btuh
15	2, NFRC 0.25	Vinyl	0.36	E	20.0	14.4	288 Btuh
16	2, NFRC 0.25	Vinyl	0.36	E	4.0	14.4	58 Btuh
17	2, NFRC 0.25	Vinyl	0.36	N	24.0	14.4	346 Btuh
	Window Total				498.0(sqft)		7171 Btuh
Walls	Туре	Ornt. U	eff.	R-Value	Area X	HTM=	Load
				(Cav/Sh)			
1	Frame - Wood		.089)	13.0/0.0	158	3.55	561 Btuh
2	Frame - Wood	- Ext (0		13.0/0.0	60	3.55	213 Btuh
3	Frame - Wood		.089)	13.0/0.0	43	3.55	154 Btuh
4	Frame - Wood		.089)	13.0/0.0	103	3.55	365 Btuh
5	Frame - Wood		.089)	13.0/0.0	60	3.55	213 Btuh
6	Frame - Wood	500000000000000000000000000000000000000	.089)	13.0/0.0	75	3.55	265 Btuh
7	Frame - Wood	Colored Colored States	.089)	13.0/0.0	40	3.55	142 Btuh
8	Frame - Wood	The second secon	.089)	13.0/0.0	93	3.55	329 Btuh
9	Frame - Wood		.089)	13.0/0.0	127	3.55	450 Btuh
10	Frame - Wood		.089)	13.0/0.0	103	3.55	365 Btuh
11	Frame - Wood	The second secon	.089)	13.0/0.0	211	3.55	750 Btuh
12	Frame - Wood		.089)	13.0/0.0	199	3.55	708 Btuh
13	Frame - Wood		.089)	13.0/0.0	95	3.55	338 Btuh
14	Frame - Wood		.089)	13.0/0.0	93	3.55	331 Btuh
15	Frame - Wood	- Ext (0		13.0/0.0	79	3.55	282 Btuh
16	Frame - Wood	- Ext (0		13.0/0.0	100	3.55	355 Btuh
17	Frame - Wood	- Ext (0		13.0/0.0	86	3.55	305 Btuh
18	Frame - Wood	- Ext (0		13.0/0.0	60	3.55	213 Btuh
19	Frame - Wood	- Ext (0	Window Co. of Co. Co. Co.	13.0/0.0	113	3.55	400 Btuh
20	Frame - Wood	- Ext (0		13.0/0.0	443	3.55	1572 Btuh
21	Frame - Wood	- Ext (0		13.0/0.0	143	3.55	507 Btuh
22	Frame - Wood	- Ext (0		13.0/0.0	80	3.55	284 Btuh
	Wall Total		Energy	Gauge® / USRC2	ZB v 2566 (sqft)		9101 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Project Title:

Monica Cannon 495 SW Jordan Street Ft White, FL

Cannon Residence Building Type: User

3/28/2023

Doors	Type S	Storm Ueff.		Area X	HTM=	Load
1	Insulated - Garage,	n (0.460)		24	18.4	442 Btuh
	Door Total			24(sqft)		442Btuh
Ceilings	Type/Color/Surface	Ueff.	R-Value	Area X	HTM=	Load
1	Flat ceil/M/Shing	(0.025)	38.0/0.0	3458	1.0	3510 Btuh
	Ceiling Total			3458(sqft)	X.	3510Btuh
Floors	Туре	Ueff.	R-Value	Size X	HTM=	Load
1	Slab On Grade	(1.180)	0.0	318.7 ft(pe	erim.) 47.2	15041 Btuh
	Floor Total	50 65		3293 sqft	**	15041 Btuh
				Envelope Sub		35265 Btuh
Infiltration		Wholehouse AC			Market Committee	
5	Natural	0.1	17 32930	0 1.0	0 94.1	4119 Btuh
Duct load	Average sealed, R6.	0, Supply(Att)	Return(Att)) (DLI	/l of 0.254)	9996 Btuh
All Zones	,		Sensible	Subtotal All	Zones	49380 Btuh

WHOLE HOUSE TOTALS

Totals for Heating	Subtotal Sensible Heat Loss Ventilation Sens. Heat Loss Total Heat Loss	(Ex:0 cfm; Sup:0 cfm)	49380 Btuh 0 Btuh 49380 Btuh
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EQUIPMENT

Electric Heat Pump	#	49380 Btuh

Key: Window types - NFRC (Requires U-Factor and Shading coefficient(SHGC) of glass as numerical values) or - Glass as 'Clear' or 'Tint' (Uses U-Factor and SHGC defaults) U - (Window U-Factor) HTM - (ManualJ Heat Transfer Multiplier)



Version 8

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Monica Cannon 495 SW Jordan Street Ft White, FL Project Title: Cannon Residence

3/28/2023

Reference City: Gainesville, FL (Defaults)

Humidity difference: 51gr.

Temperature Difference: 19.0F(TMY3 99%)

Summer Setpoint: 75 °F (Required Manual J default)

Component Loads for Whole House

		Тур	e*			Over	hang	Window Area(sqft)			Н	ITM	Load	
Window	Panes	SHGC U	InSh	IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2 NFRC	0.25, 0.36	No	No	N	8.5ft.	2.0ft.	40.0	0.0	40.0	12	12	484	Btu
2	2 NFRC	0.25, 0.36	No	No	N	8.5ft.	2.0ft.	54.0	0.0	54.0	12	12	653	Btu
3	2 NFRC	0.25, 0.36	No	No	N	1.0ft.	3.0ft.	24.0	0.0	24.0	12	12	290	
4	2 NFRC	0.25, 0.36	No	No	N	1.5ft.	1.0ft.	12.0	0.0	12.0	12	12	145	
5	2 NFRC	0.25, 0.36	No	No	N	1.0ft.	3.0ft.	24.0	0.0	24.0	12	12	290	
6	2 NFRC	0.25, 0.36	No	No	N	1.5ft.	1.0ft.	24.0	0.0	24.0	12	12	290	Btu
7	2 NFRC	0.25, 0.36	No	No	W	1.5ft.	1.0ft.	4.0	1.0	3.0	12	31	105	
8	2 NFRC	0.25, 0.36	No	No	W	1.5ft.	1.0ft.	8.0	0.5	7.5	12	31	238	
9	2 NFRC	0.25, 0.36	No	No	S	10.5f	1.0ft.	30.0	30.0	0.0	12	14	363	
10	2 NFRC	0.25, 0.36	No	No	S	10.5f	1.0ft.	48.0	48.0	0.0	12	14	581	Btu
11	2 NFRC	0.25, 0.36	No	No	S	1.5ft.	1.0ft.	54.0	54.0	0.0	12	14	653	
12	2 NFRC	0.25, 0.36	No	No	S	7.5ft.	1.0ft.	54.0	54.0	0.0	12	14	653	
13	2 NFRC	0.25, 0.36	No	No	S	1.5ft.	1.0ft.	54.0	54.0	0.0	12	14	653	
14	2 NFRC	0.25, 0.36	No	No	E	1.5ft.	1.0ft.	20.0	1.0	19.0	12	31	600	
15		0.25, 0.36	No	No	E	1.5ft.	1.0ft.	20.0	1.0	19.0	12	31	600	
16		0.25, 0.36	No	No	E	1.5ft.	1.0ft.	4.0	1.0	3.0	12	31	105	Btu
17	COUNTY OF THE RESIDENCE OF THE PARTY OF THE	0.25, 0.36	No	No	N	1.0ft.	3.0ft.	24.0	0.0	24.0	12	12	290	Btu
	Windov							498 (s					6997	
Walls	Туре				U-	Value	₽ R-\	/alue	Area	(sqft)		НТМ	Load	
	1						Cav/S	heath					3	
1	Frame - \	Wood - Ext			0	.09	13.0		15	8.0		2.3	358	Btu
2	Frame - \	Wood - Ext			0	.09	13.0	0.0	60			2.3	136	Btu
3	Frame - \	Wood - Ext				.09	13.0		43			2.3	98	Btu
4	Frame - \	Wood - Ext				.09	13.0	* Total (1997)	102			2.3	232	Btu
5	Frame - \	Wood - Ext				.09	13.0		60			2.3	136	Btu
6	Frame - \	Wood - Ext				.09	13.0		74			2.3	169	Btu
7		Wood - Ext				.09	13.0		40			2.3	91	Btu
8	Frame - \	Wood - Ext				.09	13.0		92			2.3	210	Btu
9		Wood - Ext				.09	13.0		126			2.3	287	Btu
10	The last supplied and a second	Nood - Ext				.09	13.0	100 100 100 100 100 100 100 100 100 100	102			2.3	232	Btu
11		Nood - Ext				.09	13.0		21			2.3	478	Btul
12	The state of the s	Nood - Adj				.09	13.0		199			1.7	336	Btul
13		Nood - Ext				.09	13.0		95			2.3	216	Btul
14	The state of the s	Nood - Ext				.09	13.0		93			2.3	211	Btu
15	Chicken Sept. 75	Nood - Ext				.09	13.0		79			2.3	180	Btu
16		Nood - Ext				.09	13.0		100			2.3	226	Btul
17	The second secon	Nood - Ext				.09	13.0		86			2.3	195	Btul
18	The state of the s	Nood - Ext				.09	13.0		60			2.3	136	Btul
19	1000	Nood - Ext				.09	13.0		112			2.3	255	Btul
20		Nood - Ext				.09	13.0		442			2.3	1002	Btul
21	1227	Nood - Ext				.09	13.0		142			2.3	323	Btul
22		Nood - Ext				.09	13.0/		80			2.3	181	Btul
	Wall To					.00	10.0	0.0		3 (sqft)		2.3	5687	
Doors	Туре					- 1			Area			НТМ	Load	Diu
1	The state of the s	- Garage							24			13.8	331	Divi
	Door To									4 (sqft)		13.0	331	Btul
Ceilings	-	olor/Surfa	ace		U-	Value	F	R-Value				НТМ	Load	Diu
1		ttic/Med/Shi		В		0.025		88.0/0.0	345	0 2000000		1.12	3861	Btul
.	Ceiling		- giai (.020		0.0/0.0				1.12		
	Cennig	Total							343	8 (sqft)			3861	DIL

Manual J Summer Calculations

Residential Load - Component Details (continued)

Project Title: Climate:FL_GAINESVILLE_REGIONAL_A

Monica Cannon 495 SW Jordan Street Ft White, FL

Cannon Residence

3/28/2023

Floors	Туре	R-Va	alue	Size		HTM	Load	
1	Slab On Grade		0.0		(ft-perimeter)	0.0	0	Btuh
	Floor Total		3	293.0	(sqft)		0	Btuh
				Env	elope Subto	otal:	16877	Btuh
Infiltration	Туре	Average ACH	Volume	(cuft) V	Vall Ratio	CFM=	Load	
	Natural	0.13	33	2930	1	70.5	1467	Btuh
Internal		Occupants	Btu	ıh/occu	upant	Appliance	Load	
gain		10	X	230	+	1400	3700	Btuh
				Sen	sible Envel	ope Load:	22044	Btuh
Duct load	Average sealed,Supply	(R6.0-Attic), Return(R6.0-Attic)	((DGM of 0).447)	9856	Btuh
				Sensi	ble Load A	II Zones	31900	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Project Title: Climate:FL_GAINESVILLE_F

Monica Cannon 495 SW Jordan Street Ft White, FL

Cannon Residence

Climate:FL_GAINESVILLE_REGIONAL_A

3/28/2023

WHOLE HOUSE TOTALS			
	Sensible Envelope Load All Zones	22044	Btuh
	Sensible Duct Load	9856	Btuh
	Total Sensible Zone Loads	31900	Btuh
	Sensible ventilation (Ex:0 cfm; Sup:0 cfm)	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	31900	Btuh
Totals for Cooling	Latent infiltration gain (for 51 gr. humidity difference)	2435	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	2517	Btuh
	Latent occupant gain (10.0 people @ 200 Btuh per person)	2000	Btuh
	Latent other gain	0	Btuh
	Latent total gain	6952	Btuh
	TOTAL GAIN	38852	Btuh

EQUIPMENT	多一种是一种"产品"。	
1. Central Unit	#	38852 Btuh

*Key: Window types (Panes - Number and type of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value)

(U - Window U-Factor)

(InSh - Interior shading device: none(No), Blinds(B), Draperies(D) or Roller Shades(R))

- For Blinds: Assume medium color, half closed For Draperies: Assume medium weave, half closed

For Roller shades: Assume translucent, half closed

(IS - Insect screen: none(N), Full(F) or Half(1/2))

(Ornt - compass orientation)



Version 8

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Business and Professional Regulation - Residential Performance Method

		1 1010331011	ai Regulation - Residential Per	Tormance Method
Project Name: Street:	Cannon Residence 495 SW Jordan Street		Builder Name: Permit Office: Columbia County	11
City, State, Zip:	Ft White, FL,		Permit Office: Columbia County Permit Number:	
Owner:	Monica Cannon		Jurisdiction:	£ 14
Design Location:	FL, Gainesville		County: Columbia(Florida (Climate Zone 2)
New construction	n or existing New (F	rom Plans)	10. Wall Types (3085.3 sqft.)	Insulation Area
2. Single family or	multiple family	Detached	a. Frame - Wood, Exterior	R=13.0 2862.00 ft ²
Number of units		1	b. Frame - Wood, Adjacent c. N/A	R=13.0 223.33 ft ²
4. Number of Bedr		5	d. N/A	
5. Is this a worst ca	ase?	No	11. Ceiling Types(3457.6 sqft.) a. Flat ceiling under att (Vented)	Insulation Area R=38.0 3457.60 ft ²
	or area above grade (ft²) or area below grade (ft²)	3293 0	b. N/A c. N/A	K-36.0 3457.00 II
	sqft.) Description	Area	12. Roof(Comp. Shingles, Vented)	Deck R=0.0 3958 ft ²
a. U-Factor: SHGC:	Dbl, U=0.36	498.00 ft ²	13. Ducts, location & insulation leve	
b. U-Factor:	SHGC=0.25 N/A	ft²	a. Sup: Attic, Ret: Attic, AH: 1st Flob.	oor 6 823
SHGC:			c.	
c. U-Factor: SHGC:	N/A	ft²	14. Cooling Systems	kBtu/hr Efficiency
	erage Overhang Depth:	4.809 ft	a. Central Unit	38.9 SEER:14.00
Area Weighted Av		0.250		
8. Skylights	Description	Area	15. Heating Systems	kBtu/hr Efficiency
U-Factor:(AVG) SHGC(AVG):	N/A N/A	N/A ft ²	a. Electric Heat Pump	49.4 HSPF:8.20
9. Floor Types	Insulation	Area		
a. Slab-On-Grade		3293.00 ft ²	16. Hot Water Systems	
b. N/A	R=	ft²	a. Electric	Cap: 50 gallons EF: 0.920
c. N/A	R=	ft²	b. Conservation features	EF. 0.920
			No.	None
		45	17. Credits	CV, Pstat
Glass/Floor Area: 0.	151 Total P	roposed Modifie Total Baselin		PASS
I hereby certify that	the plans and specifications of	overed by	Review of the plans and	
this calculation are	in compliance with the Florida	Energy	specifications covered by this	OF THE STATE
Code.	1) 1 ()	7/1	calculation indicates compliance	37/00/1/9/
PREPARED BY: _	WM C-+		with the Florida Energy Code.	8
THE ANED DI.			Before construction is completed this building will be inspected for	
DATE:	3 / 28 / 2023		compliance with Section 553.908	
	this building, as designed, is in	o compliance	Florida Statutes.	11 75
with the Florida Ene		Compliance		GOD WE TRUST
OWNER/AGENT: _			BUILDING OFFICIAL:	
DATE:			DATE:	
A manufacture of the second			anufacturar that the air bandler and	

- Compliance requires certification by the air handler unit manufacturer that the air handler enclosure qualifies as certified factory-sealed in accordance with R403.3.2.1.
- Default duct leakage does not require a Duct Leakage Test Report.
- Compliance requires an Air Barrier and Insulation Inspection Checklist in accordance with R402.4.1.1 and this project requires a PERFORMANCE envelope leakage test report with envelope leakage no greater than 5.00 ACH50 (R402.4.1.2).

			25744		PRO	JECT				L E		
Build Perri Juris Fam New Year	ding Type:	Cannon Residence User Monica Cannon Columbia County Detached New (From Plans) 2023	9	Total Sto Worst Co Rotate A Cross Vo	ned Area: ories: ase: angle: entilation: louse Fan	1 No 0 Yes	Lot Blo Pla Str Co Cit	dress type: #: ck/SubDivis tBook: eet: unty: y, State, Zip:	ion: 495 S Colum		Street	=
					CLIN	MATE						
	sign cation		Tmy Site		Des 97.5%	sign Temp 2.5%		ign Temp Summer	Heating Degree D			Daily temp Range
FL	., Gainesville		FL_GAINESVILLE_	REGION	A 32	92	70	75	1305.5	51	М	edium
100					BLO	CKS						
/ Nur	mber	Name	Area	Vo	lume		7					
1		Block1	3293	32	930 cu ft							
					SPA	CES				122 11 212		
Nur	mber	Name	Area	Volume	Kitchen	Occupant	s Bed	Irooms	Finished	d	Cooled	Heated
1		1st Floor	3293	32930	Yes	10		5	Yes		Yes	Yes
		4		1.	FLO	ORS		Total Ex	kposed	Area =	3293 9	sq.ft.)
/#	Floor Type	9	Space	Exposed	Perim	Perimeter R-V	alue Are	a U-Facto	or Joist R-\	Value Til	e Wood	Carpet
1	Slab-On-Gra	ade Edge Ins	1st Floor	318.66	57	0	3293	3 ft 0.304	4	- 0.0	0.0	00 1.00
					RO	OF						
/#	Туре	÷	Materials		Roof krea	Gable Roo Area Colo			SA I Tested	Emitt Em Tes		
1	Hip	(Composition shingle	s 39	58 ft²	0 ft ² Mediu	m Y	0.96	No	0.9 N	0 0	33.69
					AT	TIC						
/#	Туре		Ventilation		Vent F	Ratio (1 in)	Area	RBS	II	RCC		
1	Partial cathe	dral ceiling	Vented			300	3293 ft²	Υ		N		
				·	CEIL	ING	(Total Ex	posed	Area =	3458 s	sq.ft.)
/#	Ceiling Typ	pe	5	pace	R-V	alue Ins. Ty				ming Frac		uss Type

							W	ALLS	3			(Tot	al Exp	osed	Area =	308	5 sq.:	ft.)
√# Ornt		acent To	Wall Type		Space)		avity -Value	Width Ft			ight In	Area sq.ft		Sheath R-Value		Solar Absor.	Below Grade
1 N		Exterior	Frame - Wood		1st	Floor		13.0	24.0	0	10.0	0 6	252.0	0.084		0.23	0.75	0 %
2 N		Exterior	Frame - Wood	Ľ.	1st	Floor		13.0	6.0	0	10.0	0 0	60.0	0.084	1	0.23	0.75	0 %
3 E		Exterior	Frame - Wood		1st	Floor		13.0	4.0	4	10.0	0 0	43.3			0.23	0.75	0 %
4 N		Exterior	Frame - Wood		1st	Floor		13.0	12.0	8	10.0	0 0	126.7	0.084	ı	0.23	0.75	0 %
5 W		Exterior	Frame - Wood		1st	Floor		13.0	6.0	0	10.0	0 0	60.0	0.084		0.23	0.75	0 %
6 N		Exterior	Frame - Wood	Ē	1st	Floor		13.0	8.0	8	10.0	0	86.7	0.084	į.	0.23	0.75	0 %
7 E		Exterior	Frame - Wood		1st	Floor		13.0	4.0	0	10.0	0	40.0	0.084	1	0.23	0.75	0 %
8 N		Exterior	Frame - Wood			Floor		13.0	11.0	8	10.0		116.7		Į.	0.23	0.75	0 %
9 W		Exterior	Frame - Wood			Floor		13.0	12.0	8	10.0		126.7			0.23	0.75	0 %
10 N		Exterior	Frame - Wood			Floor		13.0	12.0	8	10.0		126.7			0.23	0.75	0 %
11 W		Exterior	Frame - Wood			Floor		13.0	22.0	4	10.0		223.3			0.23	0.75	0 %
12 S		Garage	Frame - Wood			Floor		13.0	22.0	4	10.0		223.3			0.23	0.75	0 %
13 S		Exterior	Frame - Wood			Floor		13.0	17.0	4	10.0		173.3			0.23	0.75	0 %
14 W		Exterior	Frame - Wood			Floor		13.0	9.0	4	10.0		93.3			0.23	0.75	0 %
15 S		Exterior	Frame - Wood			Floor		13.0	13.0	4	10.0		133.3			0.23	0.75	0 %
17 S		Exterior Exterior	Frame - Wood			Floor		13.0	10.0	0	10.0		100.0			0.23	0.75	0 %
$\frac{17.5}{18.W}$		Exterior	Frame - Wood Frame - Wood			Floor		13.0	14.0	0	10.0		140.0			0.23	0.75	0 %
18 W		Exterior	Frame - Wood					13.0	6.0	0	10.0		60.0			0.23	0.75	0 %
E		Exterior	Frame - Wood			Floor		13.0 13.0	16.0 48.0	8	10.0		166.7 486.7			0.23	0.75	0 %
		Exterior	Frame - Wood			Floor		13.0	16.0	8	10.0		166.7			0.23	0.75	0 %
22 W		Exterior	Frame - Wood			Floor		13.0	8.0	0	10.0		80.0			0.23 0.23	0.75 0.75	0 % 0 %
						1 1001		10.0	0.0		10.0		00.0	0.004		0.23	0.75	0 76
							DC	ORS	<u> </u>			(T	otal E	xpose	d Area	a = 2	4 sq.f	t.)
√# Ornt		Adjacent	To Door Type		Space			Stor	ms		U-V	alue		Vidth Ft In		ight In	Are	a
1 S		Garage	Insulated		1st Flo	or		No	one		0	.46	3.00	0 0	8.00	0	24.0)ft²
					7	W	/IN	DOW	IS			/To	tal Ex	20000	Aroa	- 40	9 ca f	+ \
								DOT				(10	THE REAL PROPERTY.			- 43	o sq.i	ι.)
/	Wall ID	Frame	Panes	NFRC	U-Factor	SHGC	Imp	Storm	Total Area (ft²)	Sar Un		Vidth (ft)	Height (ft)	Overh Depth (ft)		nterior	Shade	Screen
1 N	1	TIM	Low-E Double	Υ	0.36	0.25	Ν	N	40.0	2		2.50	8.00	8.5	2.0	Nor	ne	None
2 N	1	Vinyl	Low-E Double	Y	0.36	0.25	N	N	54.0	3		3.00	6.00	8.5	2.0	Nor		None
3 N	4	Vinyl	Low-E Double	Y	0.36	0.25	N	N	24.0	1		4.00	6.00	1.0	3.0	Nor		None
4 N	6	Vinyl	Low-E Double	Y	0.36	0.25	N	N	12.0	1		2.00	6.00	1.5	1.0	Nor		None
5 N	8	Vinyl	Low-E Double	Y	0.36	0.25	N	N	24.0	1		4.00	6.00	1.0	3.0	Nor		None
6 N	10	Vinyl	Low-E Double	Y	0.36	0.25	N	N	24.0	_ 1		4.00	6.00	1.5	1.0	Nor		None
7 W	11	Vinyl	Low-E Double	Υ	0.36	0.25	N	N	4.0	1		4.00	1.00	1.5	1.0	Nor		None
8 W	11	Vinyl	Low-E Double	Y	0.36	0.25	Ν	N	8.0	1		2.00	4.00	1.5	1.0	Nor		None
9 S	13	Vinyl	Low-E Double	Y	0.36	0.25	Ν	N	30.0	2		3.00	5.00		1.0	Nor		None
10S	13	TIM	Low-E Double	Y	0.36	0.25	N	N	48.0	2		3.00	8.00		1.0	Nor		None
11S	15	Vinyl	Low-E Double	Y	0.36	0.25	Ν	N	54.0	3		3.00	6.00		1.0	Non		None
— 12S	17	Vinyl	Low-E Double	Y	0.36	0.25	N	N	54.0	3		3.00	6.00		1.0	Non		None
13S	19	Vinyl	Low-E Double	Y	0.36	0.25	N	N	54.0	3		3.00	6.00	1.5	1.0	Non		None
14E	20	Vinyl	Low-E Double	Y	0.36	0.25	N	N	20.0	2		2.00	5.00		1.0	Non		None
15E	20	Vinyl	Low-E Double	Y	0.36	0.25	N	N	20.0	1		1.00	5.00		1.0	Non		None
— 16E	20	Vinyl	Low-E Double	Y	0.36	0.25	N	N	4.0	1		1.00	1.00		1.0	Non		None
— ^{17N}	21	Vinyl	Low-E Double	Υ	0.36	0.25	N	N	24.0	1	4	1.00	6.00	1.0	3.0	Non	е	None
	19200-1-1	-700			10-07/2		_	_		-		_						

å pa	e 2 at	Paris I	es a	INF	ILTRA	TION			11.77		53.60
/#	Scope	Method	SLA	CFM50	ELA	EqLA	ACH	ACH50	Space(s)	Infiltration	n Test Volum
_	1 Wholehouse	Proposed ACH(50)	0.00032	2744	150.55	282.65	0.1071	5.0	All	32930 cu	ı ft
				G	SARAG	E					
/#	Floo	r Area	Roof Area	Exp	osed Wall	Perimeter	Avç	j. Wall Heig	ht Ex	xposed Wall	Insulation
—	1 49	1 ft²	491 ft²		66 ft			10 ft		1	
					MASS						
/#	Mass Type		Area		Thickne	ss	Furniture Fr	action	Space		+
	1 Default(8 lbs/s	sq.ft.)	0 ft²		0 ft		0.30		1st Flo	or	
,				HEAT	ING SY	STEM					
/#	System Type		Subtype/Speed	AHRI	# Effic		Capacity kBtu/hr Er	Geother	mal HeatPu ver Volt	mp Duc Current	ts Block
	1 Electric Heat I	Pump	None/Single		HSP	F: 8.20	49.4	0.0	0.00	0.00 sys#	# 1 1
				COOL	ING SY	STEM					
/#	System Type		Subtype/Speed	AHRI	# E	fficiency	Capacit kBtu/hr		Flow ofm	SHR Duc	ct Block
	Central Unit		None/Single	E	SE	ER:14.0	38.9	1	170	0.70 sys#	# 1 1
				HOT W	ATER S	SYSTE	M				
/#	System Type	Subtype	Location	EF(L	JEF) Ca	ap Us	se SetPnt	Fixture	Flow Pip	e Ins. P	ipe length
1	Electric	None	1st Floor	0.92 ((0.92) 50.00	0 gal 40 g	gal 120 deg	Stand	lard N	one	12
	Recirculation System	Recirc Contro Type		_oop Brai			HR Facilit Connec			VHR Oth	ner Credits
1	No			NA N	A N	A No	NA	NA	NA NA	No.	one
					DUCTS	3					
/Du #	2222	oply R-Value Area I	Retur ocation R-	n Value Area	a Leaka	ge Type	Air Handler	CFM 25 TOT	CFM 25 OUT	QN RLF	HVAC# Heat Cool
1	Attic	6.0 823 ft ² Attic		6.0 165 ft²	Default	Leakage	1st Floor	(Default) (D	Default)		1 1
				TEMP	ERAT	URES					
Cod	gramable Thermo bling [] Jan ating [X] Jan nting [] Jan	ostat: Y [] Feb [] Mai [X] Feb [X] Mai [] Feb [X] Ma	r []Apr	Ceiling [] May [] May [] May	Fans: N [X] Jun [] Jun [] Jun	[X] Jul [] Jul [] Jul	[X] Aug [] Aug [] Aug	[X] Sep [] Sep [] Sep	[] Oct [] Oct [X] Oct	[] Nov [X] Nov [X] Nov	[] Dec [X] Dec [] Dec

FORM R405-2020

				TE	MPE	RATU	RES(C	ontinu	ued)					1.0
,	Thermostat Schedu	le: HERS 2	006 Refer	ence				Но	ours					
\vee	Schedule Type		1	2	3	4	5	6	7	8	9	10	11	12
	Cooling (WD)	AM PM	78 80	78 80	78 78	78 78	78 78	78 78	78 78	78 78	80 78	80 78	80 78	80 78
-	Cooling (WEH)	AM PM	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78
_	_ Heating (WD)	AM PM	66 68	66 68	66 68	66 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	68 66
	_Heating (WEH)	AM PM	66 68	66 68	66 68	66 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	68 66

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD ESTIMATED ENERGY PERFORMANCE INDEX* = 97

The lower the EnergyPerformance Index, the more efficient the home.

495 SW Jordan Street, Ft White, FL,

1. New construction or ex	xisting N	lew (From Plans)	10. Wall Types (3085.3 sqft.)	Insulation Area
2. Single family or multip	le family	Detached	a. Frame - Wood, Exterior	R=13.0 2862.00 ft ²
3. Number of units, if mu	Itiple family	1	b. Frame - Wood, Adjacent c. N/A	R=13.0 223.33 ft ²
4. Number of Bedrooms		5	d. N/A	
5. Is this a worst case?		No	11. Ceiling Types(3457.6 sqft.)	Insulation Area
Conditioned floor area Conditioned floor area		3293 0	a. Flat ceiling under att (Vented)b. N/Ac. N/A	R=38.0 3457.60 ft ²
7. Windows** a. U-Factor: SHGC:	Description Dbl, U=0.36 SHGC=0.25	Area 498.00 ft ²	 Roof(Comp. Shingles, Vented) Ducts, location & insulation leve Sup: Attic, Ret: Attic, AH: 1st Flo 	I R ft ²
b. U-Factor: SHGC:	N/A	ft²	b. C.	0 623
c. U-Factor: SHGC:	N/A	ft²	 Cooling Systems Central Unit 	kBtu/hr Efficiency 38.9 SEER:14.00
Area Weighted Average Area Weighted Average		4.809 ft 0.250		
 Skylights U-Factor:(AVG) SHGC(AVG): 	Description N/A N/A	Area N/A ft²	 Heating Systems Electric Heat Pump 	kBtu/hr Efficiency 49.4 HSPF:8.20
 Floor Types Slab-On-Grade Edge N/A 	Insulation R= 0 R=	.0 3293.00 ft ² ft ²	Hot Water Systems a. Electric	Cap: 50 gallons EF: 0.920
c. N/A	R=	ft²	b. Conservation features	
			17 Cradita	None
			17. Credits	CV, Pstat

I certify that this home has complied with the Florida Energy Efficiency Code for Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: 495 SW Jordan Street

City/FL Zip: Ft White,FL,

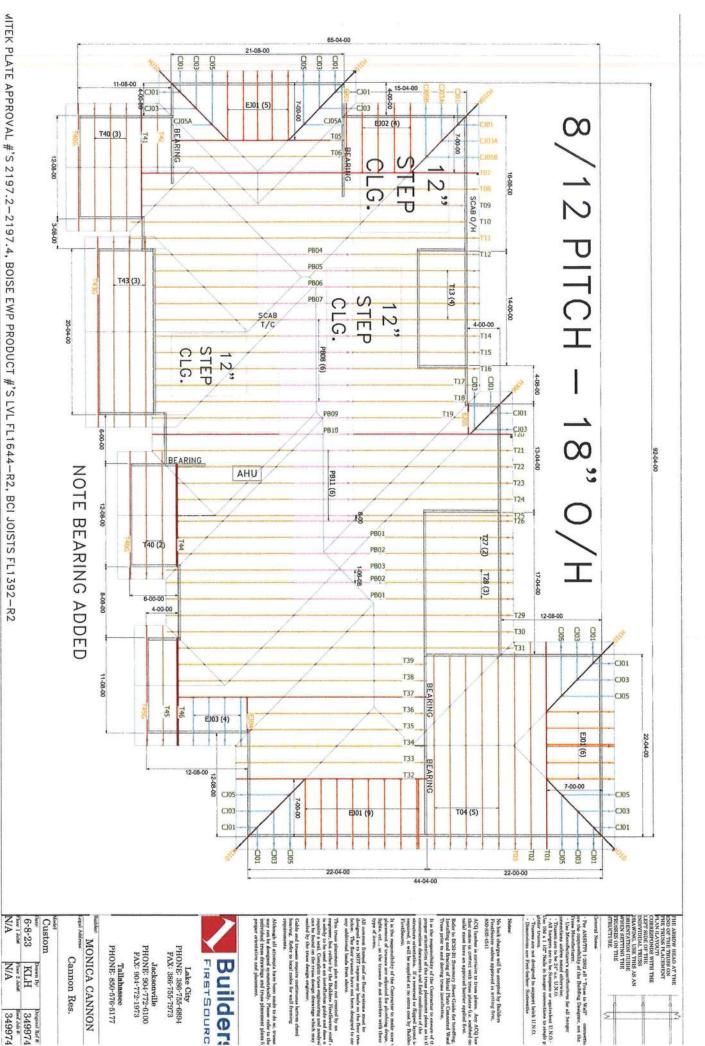
OF THE STATION OF THE

*Note: This is not a Building Energy Rating. If your Index is below 70, your home may qualify for energy efficient mortgage (EEM) incentives if you obtain a Florida Energy Rating. For information about the Florida Building Code, Energy Conservation, contact the Florida Building Commission's support staff.

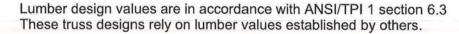
**Label required by Section R303.1.3 of the Florida Building Code, Energy Conservation, if not DEFAULT.

Envelope Leakage Test Report (Blower Door Test) Residential Prescriptive, Performance or ERI Method Compliance 2020 Florida Building Code, Energy Conservation, 7th Edition

Jurisdiction:	Permit #:
Job Information	
Builder: Community:	Lot: NA
Address: 495 SW Jordan Street	
City: Ft White State	: FL Zip:
Air Leakage Test Results Passing results must meet	either the Performance, Prescriptive, or ERI Method
PRESCRIPTIVE METHOD-The building or dwelling unit shall be test changes per hour at a pressure of 0.2 inch w.g. (50 Pascals) in Climic PERFORMANCE or ERI METHOD-The building or dwelling unit shat the selected ACH(50) value, as shown on Form R405-2020 (Performance) ACH(50) specified on Form R405-2020-Energy Calc	Il be tested and verified as having an air leakage rate of not exceeding or R406-2020 (ERI), section labeled as infiltration, sub-section ACH50.
CFM(50)	Method for calculating building volume: Retrieved from architectural plans Code software calculated
R402.4.1.2 Testing. Testing shall be conducted in accordance with ANSI/RE Testing shall be conducted by either individuals as defined in Section 553.94 489.105(3)(f), (g), or (i) or an approved third party. A written report of the resprovided to the ode official. Testing shall be performed at any time after creat During testing: 1. Exterior windows and doors, fireplace and stove doors shall be closed, but control measures. 2. Dampers including exhaust, intake, makeup air, back draft and flue dampeneasures. 3. Interior doors, if installed at the time of the test, shall be open. 4. Exterior doors for continuous ventilation systems and heat recovery ventil 5. Heating and cooling systems, if installed at the time of the test, shall be tu 6. Supply and return registers, if installed at the time of the test, shall be fully	23(5) or (7F/orida Statues.or individuals licensed as set forth in Section sults of the test shall be signed by the party conducting the test and tion of all penetrations of the uilding thermal envelope. It not sealed, beyond the intended weatherstripping or other infiltration ers shall be closed, but not sealed beyond intended infiltration control ators shall be closed and sealed. The sealed in the test and the tes
Testing Company	
Company Name: I hereby verify that the above Air Leakage results are in accordant Energy Conservation requirements according to the compliance in	ce with the 2020 7th Edition Florida Building Code
Signature of Tester:	Date of Test:
Printed Name of Tester:	
License/Certification #:	_ Issuing Authority:



MITEK PLATE APPROVAL #'S 2197.2-2197.4, BOISE EWP PRODUCT #'S LVL FL1644-R2, BCI JOISTS FL1392-R2





RE: 2718981 - DETAILS

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Date

Site Information:

Customer Info: DETAILS Project Name: N/A Model: N/A

Lot/Block: N/A

Subdivision: N/A

Address: N/A, N/A

State: N/A

City: N/A

Name Address and License # of Structural Engineer of Record, If there is one, for the building. Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014

Design Program: MiTek 20/20 8.4

Wind Code: ASCE 7-16 Roof Load: 37.0 psf

Wind Speed: 130 mph Floor Load: N/A psf

This package includes 20 individual, General Truss Details and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

Seal#

T23399820

T23399821

T23399822

T23399823 T23399824

No.

16

17

18

No.	Seal#	Detail Name	Date
1	T23399806	MII-REP10	4/2/21
2	T23399807	MII-T-BRACE 2	4/2/21
3	T23399808	MII-SCAB-BRACE	4/2/21
4	T23399809	MII-REP05	4/2/21
1 2 3 4 5 6 7 8 9	T23399810	MII-GE130-D-SP	4/2/21
6	T23399811	MII-GE130-SP	4/2/21
7	T23399812	MII-GE140-001	4/2/21
8	T23399813	MII-GE170-D-SP	4/2/21
9	T23399814	MII-GE180-D-SP	4/2/21
10	T23399815	MII-GE180-D-SP	4/2/21
11	T23399816	MII-PIGGY-ALT-7-16	4/2/21
12	T23399817	MII-REP01A1	4/2/21
13	T23399818	MII-TOENAIL SP	4/2/21
14	T23399819	MII-VALLEY FIGH WIND1	4/2/21

MII-STRGBCK T23399825 Building Reviewed 9

Detail Name

MII-VALLEY HIGH WIND2 4/2/21 MII-VALLEY SP 4/2/21 MII-VALLEY SP 4/2/21 MII-GE146-001 4/2/21

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature verified on any electronic copies Plans

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: ORegan, Philip

My license renewal date for the state of Florida is February 28, 2023.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

ORegan, Philip

April 2,2021



RE: \$JOBNAME - \$JOBDESC

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: \$SI_CUSTOMER Project Name: \$SI_JOBNAME Model: \$SI_MODEL

Subdivision: \$SI_SUBDIV

Lot/Block: \$SI_LOTNUM Address: \$SI_SITEADDR City: \$SI_SITECITY State: \$SI_SITESTATE OCTOBER 28, 2016

STANDARD REPAIR FOR ADDING A FALSE BOTTOM CHORD

MII-REP10 T23399806

MiTek USA, Inc.

Page 1 of 1



MAIN TRUSS MANUFACTURED WITHOUT FALSE BOTTOM CHORD.

MAIN TRUSS (SPACING = 24" O.C.)

MiTek USA, Inc. REFER TO THE BOTTOM CHORD BRACING SECTION OF THE INDIVIDUAL TRUSS DESIGN FOR MAXIMUM SPACING OF CONTINUOUS LATERAL BRACING WHENEVER RIGID CEILING MATERIAL IS NOT DIRECTLY ATTACHED TO THE VERTICAL STUDS @ 48" O.C.. ATTACHED BOTTOM CHORD. WITH (3) - 10d (0.131" X 3") NAILS AT EACH END OF VERTICAL (TYP.). VERTICAL STUDS TO BE 2 x 4 STUD GRADE (OR BETTER) SPF, HF, DF OR SP (BOARD SIZE SPECIFIED IS MINIMUM. LARGER SIZE MAY BE USED) 2 x 4 NO. 2 (OR BETTER) SPF, HF, DF OR SP FALSE BOTTOM CHORD (BOARD SIZE SPECIFIED IS MINIMUM, LARGER SIZE MAY BE USED) FALSE BOTTOM TRUSS SPAN

NOTES:

- LOADING: TOP CHORD: (REFER TO THE MAIN TRUSS DESIGN FOR TOP CHORD LOADING). BOTTOM CHORD: LL = 0 PSF, DL = 10 PSF.
- 2. REFER TO THE MAIN TRUSS DESIGN FOR LUMBER AND PLATING REQUIREMENTS.
- 3. MAXIMUM BOTTOM CHORD PITCH = 6/12.
- 4. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
- 5. FALSE BOTTOM CHORD ONLY DESIGNED TO CARRY VERTICAL LOAD. NO LATERAL (SHEAR) LOAD ALLOWED.
- 6. FILLER MAY EXTEND FOR FULL LENGTH OF TRUSS.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6834 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021



Nails

Web

Nails

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2 T23399807

MiTek USA, Inc.

Proce Size

Page 1 of 1



Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

	Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing	
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.	

Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

Nails
SPACING
WEB HER STATE OF THE STATE OF T
T-BRACE
Nails Section Detail
T-Brace Web

I-Brace

for One-Ply Truss			
Specified Continuous Rows of Lateral Bracing			
1	2		
2x4 T-Brace	2x4 I-Brace		
2x6 T-Brace	2x6 I-Brace		
2x8 T-Brace	2x8 I-Brace		
	Specified Rows of La 1 2x4 T-Brace 2x6 T-Brace		

	Brace Size for Two-Ply Truss Specified Continuous Rows of Lateral Bracing		
Web Size	1	2	
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace	
2x6	2x6 T-Brace	2x6 I-Brace	
2x8	2x8 T-Brace	2x8 I-Brace	

T-Brace / I-Brace must be same species and grade (or better) as web member.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



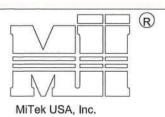
6904 Parke East Blvd. Tampa, FL 36610

SCAB-BRACE DETAIL

MII-SCAB-BRACE T23399808

MiTek USA, Inc.

Page 1 of 1



Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.

Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS ***
REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

SCAB TO ONE FACE OF WEB WITH APPLY 2x 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB. MAXIMUM WEB AXIAL FORCE = 2500 lbs MAXIMUM WEB LENGTH = 12'-0" 2x4 MINIMUM WEB SIZE SCAB BRACE MINIMUM WEB GRADE OF #3 Nails Section Detail Scab-Brace Web

Scab-Brace must be same species grade (or better) as web member.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021



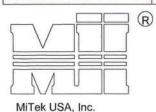


6904 Parke East Blvd. Tampa, FL 36610 AUGUST 1, 2016

STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05 T23399809

MiTek USA, Inc. Page 1 of 1



1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

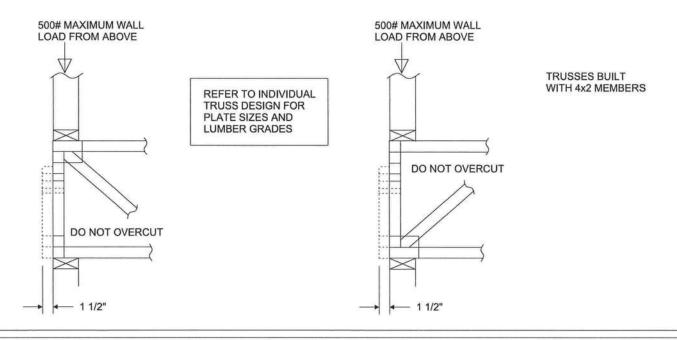
ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
 THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE

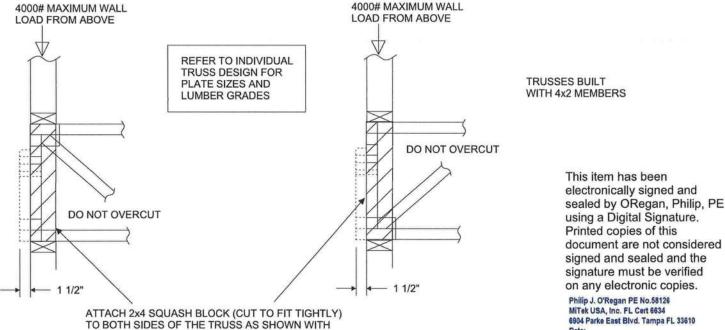
SUCH AS TO AVOID SPLITTING OF THE WOOD.

4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.

5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ ORIENTATION ONLY.

CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.





MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

April 2,2021



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

10d (0.131" X 3") NAILS SPACED 3" O.C.

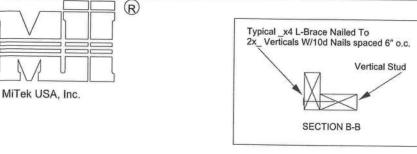


APRIL 12, 2019

Standard Gable End Detail

MII-GE130-D-SP T23399810

MiTek USA, Inc.



Page 1 of 2 Vertical Stud DIAGONAL (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A

DIAGONAL BRACE TRUSS GEOMETRY AND CONDITIONS 4'-0" O.C. MAX SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA ** 3x4 = **Diagonal Bracing** - L-Bracing Refer ** Refer to Section A-A to Section B-B

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

- 10d

NAILS'

Roof Sheathing

1'-3"

Max.

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

NOTE

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
- CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
- 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
- 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
- DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE

11. NAILS DESIGNA NAILS DESIGNA Minimum Stud Size	Stud Spacing	RE (0.131" X RE (0.131" X Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
Species and Grade		Maximum Stud Len			ngth	1/01 011113
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7
2x4 SP No. 3 / Stud		3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

24" O.C. | 2-9-11 | 2-10-11 | 4-1-1 Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING

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April 2,2021

(2) - 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C.

ATTACHED TO VERTICAL WITH (4) -16d

HORIZONTAL BRACE (SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

NAILS AND ATTACHED

ASCE 7.02 ASCE 7.05 130 MPH

ASCE 7.10 160 MPH

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.

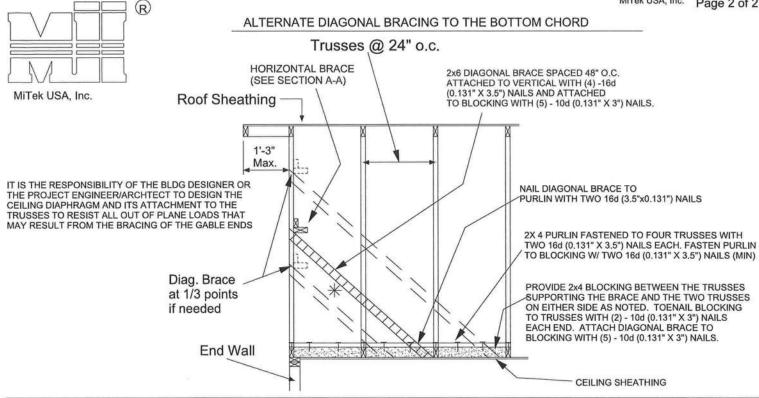
DURATION OF POAD INCREASE 1.10 READ NOT CONNECTION OF BRACING'S BASED ON MWRG'S. 5192020 BEFORE USE.

Design valid for use only with MTake connectors. This design is based only upon parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



MiTek USA, Inc. Page 2 of 2



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED: METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE

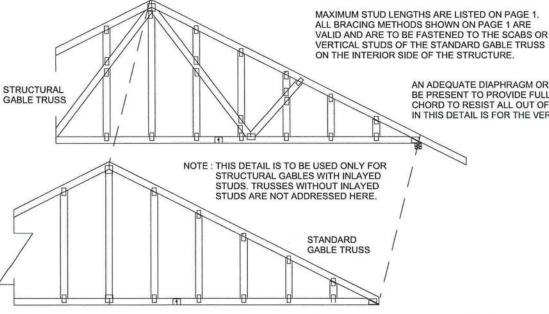
FOLLOWING NAILING SCHEDULE

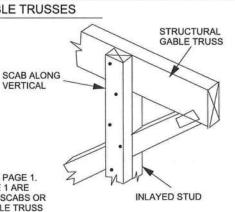
METHOD 2: ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.

FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

> This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 8634 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTekey connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



6904 Parke East Blvd.

APRIL 12, 2019

Standard Gable End Detail

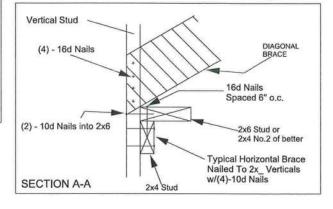
MII-GE130-SP T23399811

Page 1 of 2

MiTek USA, Inc.



Typical x4 L-Brace Nailed To Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B



DIAGONAL BRACE TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. 4'-0" O.C. MAX Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA ** 3x4 =Diagonal Bracing ** - L-Bracing Refer Refer to Section A-A to Section B-B

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

24" Max

2 DIACONAL

NOTE:

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
- 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
- 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
- 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
 GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR
- TYPE TRUSSES.
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
- NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

	N N	47		DAY /
1	1'-3" Max.	(2) - 10d NAILS		(2) - 10d NAILS
	1		Truss	ses @ 24" o.c
Diag. Brac at 1/3 poin if needed		ATTAC NAILS		
End V	Vall V		HORIZON	TAL BRACE TION A-A)

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	BRACES AT 1/3 POINTS				
and Grade		Maximum Stud Length								
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6				
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1				
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15				

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C

ASCE 7-98, ASCE 7-02, ASCE 7-05, 130 MPH SCE,7-10, ASCE 7-16 160 MPH

electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

This item has been

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

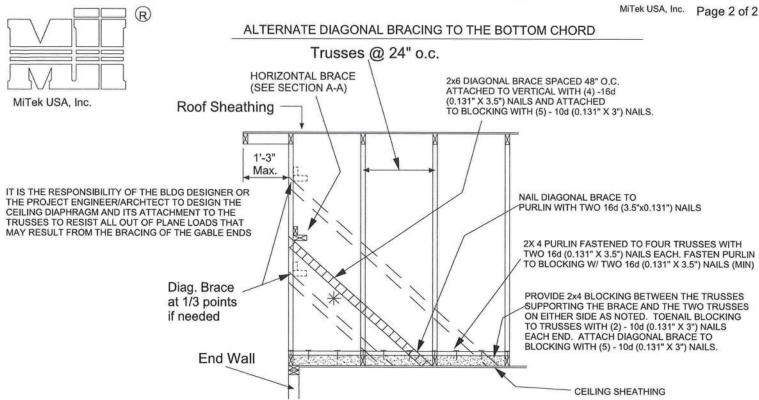
April 2,2021

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.

Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Tampa, FL 36610



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

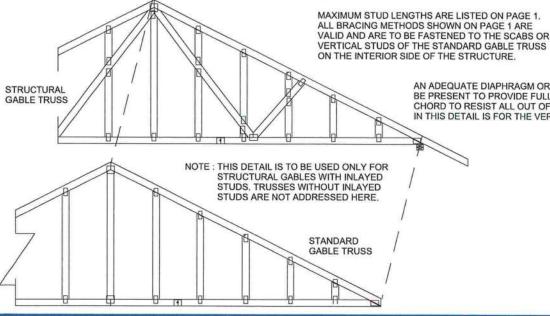
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED: METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

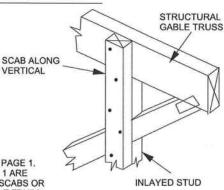
METHOD 2: ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL

MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL
MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

VERTICAL

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTIEKS connectors. This design is based only upon parameters shown, and is for on individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

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JANUARY 6, 2017

Standard Gable End Detail

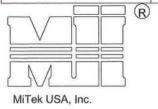
MII-GE140-001 T23399812

Page 1 of 2

MiTek USA, Inc.

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST

TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH



Typical _x4 L-Brace Nailed To Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

Vertical Stud DIAGONAL (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x Verticals w/(4)-10d Nails SECTION A-A

(5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD DF/SPF BLOCK

DIAGONAL BRACE TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. 4'-0" O.C. MAX Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA 3x4 =- Diagonal Bracing - L-Bracing Refer Refer to Section A-A to Section B-B 24"

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT. 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.

CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTIÓN MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES

10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Roof Sheathir	ng _
1'-3" Max.	(2) - 10d NAILS
1	Trusses @ 24" o.c.
Diag. Brace at 1/3 points if needed	2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.
End Wall	HORIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade			Maximu	n Stud Lei	ngth	
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum

MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING

end distance. Brace must cover 90% of diagonal length.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

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ASCE 7-98, ASCE 7-02, ASCE 7-05 STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.

ASCE 7-98, ASCE 7-02, ASCE 7-05

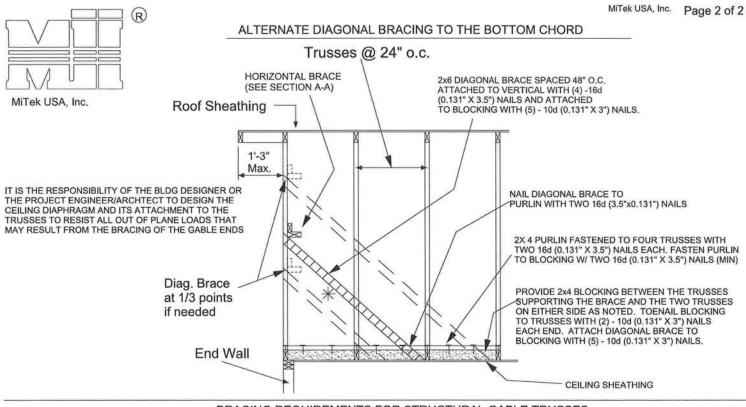
DURINON-OFLOAD INGREASE IN 6.0 READ CONNECTION OF BRACING IS BASED ON COMPONENTS AND CLADDING,

Design valid for use only with MTexe connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web anafor chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

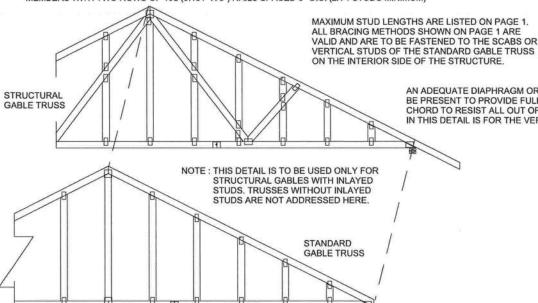
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED: METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

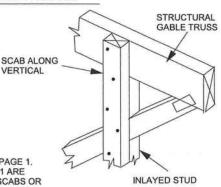
METHOD 2 : ATTACH 2X _ SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL

MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY

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April 2,2021

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ANSITEPT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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Standard Gable End Detail

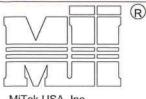
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Page 1 of 2

2X6 SP OR SPF No. 2 DIAGONAL BRACE

16d Nails Spaced 6" o.c.

MiTek USA, Inc.



MiTek USA, Inc.

Typical 2x4 L-Brace Nailed To 2x4 Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss ** 3x4 =B

SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

24" Max

End Wall

(2) - 10d Nails into 2x6 2X6 SP OR SPF No. 2 Typical Horizontal Brace Nailed To 2x4 Verticals w/(4)-10d Nails SECTION A-A 2X4 SP OR SPF No. 2 PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

(5) - 10d NAILS.

Vertical Stud

(4) - 16d Nails

- Diagonal Bracing ** - L-Bracing Refer Refer to Section A-A to Section B-B

DIAGONAL BRACE

4'-0" O.C. MAX

NOTE:

- MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
- CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND
- WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
- DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES
- DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
- 11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS				
Species and Grade		Maximum Stud Length							
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4				
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3				
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13				
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7				
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5				
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14				

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

Roof Sheathing 1'-0" (2) - 10d Max. NAILS (2) - 10d NAILS Trusses @ 24" o.c. Diag. Brace at 1/3 points 2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH if needed (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

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HORIZONTAL BRACE

(SEE SECTION A-A)

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April 2,2021

MAX MEAN ROOF HEIGHT = 30 FEET

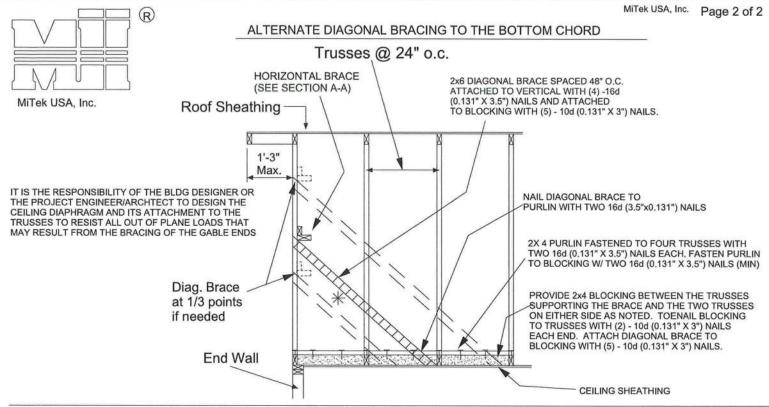
DURATION OF LOAD INCREASE: 1.60 READ NOTE SCHUNG STICKLOF BRACING IS BASED ON MWERS 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not Design Valid for use only with wire lever connectors. Into cessign is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Tampa, FL 36610



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

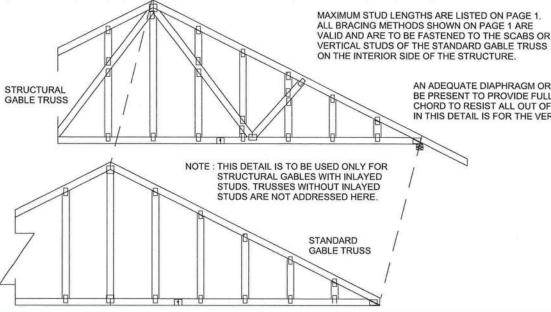
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED: METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

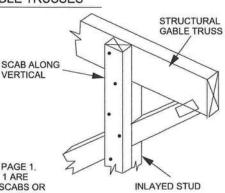
METHOD 2: ATTACH 2X_SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C. FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL

MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

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April 2,2021

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available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Standard Gable End Detail

MII-GE180-D-SP T23399814



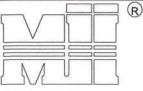
Page 1 of 2

2X6 SP OR SPF No. 2 DIAGONAL BRACE

2X6 SP OR SPF No. 2

Typical Horizontal Brace Nailed To 2x4 Verticals w/(4)-10d Nails

16d Nails Spaced 6" o.c.



DIAGONAL BRACE

4'-0" O.C. MAX

MiTek USA, Inc.

Typical 2x4 L-Brace Nailed To 2x4 Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

> SECTION A-A Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA **

> > 3x4 =

24" Max

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

2X4'SP OR SPF No. 2

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Vertical Stud

(4) - 16d Nails

(2) - 10d Nails into 2x6

Roof Sheathing

 Diagonal Bracing Refer to Section A-A ** - L-Bracing Refer to Section B-B

NOTE

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
- 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT
- 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3
 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
 DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF
- DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
- DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC
- NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS				
and Grade		Maximum Stud Length							
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3				
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7				
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4				
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1				
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7				
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9				

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

1'-0" (2) - 10d Max. NAILS (2) - 10d NAILS Trusses @ 24" o.c. Diag. Brace at 1/3 points 2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH if needed (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS. HORIZONTAL BRACE End Wall (SEE SECTION A-A)

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April 2,2021

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D

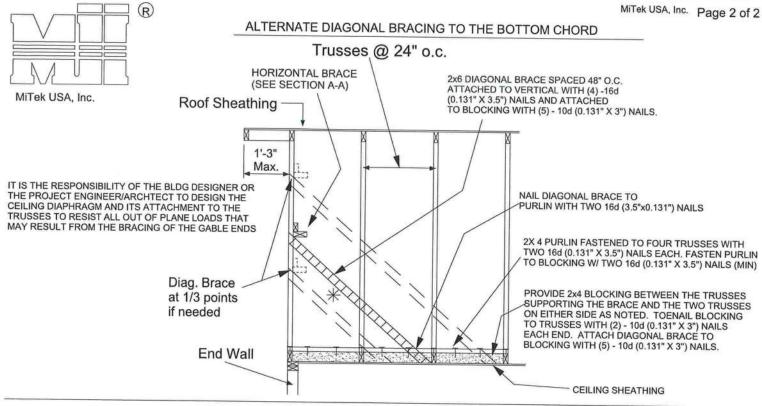
DURATION OF LOAD, INCREASE 1.60 READ NOTES CONNECTION OF BRACING IS BASED ON IMWERS. 5/19/2020 BEFORE USE

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6904 Parke East Blvd.



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:
METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE
FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE
FOLLOWING NAILING SCHEDULE.

METHOD 2: ATTACH 2X _ SCABS TO THE FACE OF EACH VERTICAL

MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING

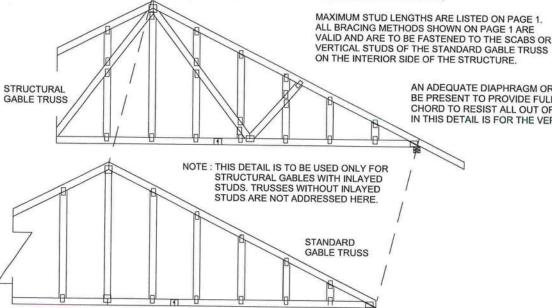
NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE

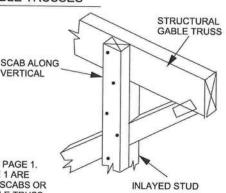
AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

 FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.

- FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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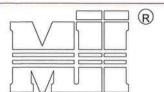
ANSITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C **ENCLOSED BUILDING** LOADING = 5 PSF TCDL ASCE 7-10, ASCE 7-16

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING

DURATION OF LOAD INCREASE: 1.60

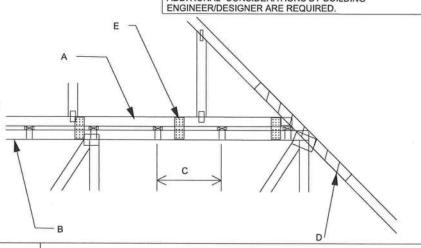
- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN
- SHALL BE CONNECTED TO EACH PURLIN
 WITH (2) (0.131" X 3.5") TOE-NAILED.

 B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

 C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
 UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
 CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.

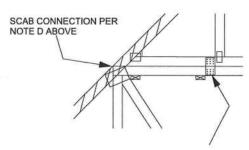
 D 2 X __ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF
 PIGGYBACK TRUSS, MIN GRADE 2. ATTACHED TO ONE FACE, CENTERED.

 ON INTERSECTION MINTH (2) POWS OF (1.414" X 3") NAILS & A " O C.
- ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND
 - 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 180 MPH WITH A MAXIMUM
- PIGGYBACK SPAN OF 12 ft. E FOR WIND SPEEDS BETWEEN 116 AND 180 MPH, ATTACH MITEK NP37 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" NAIL EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

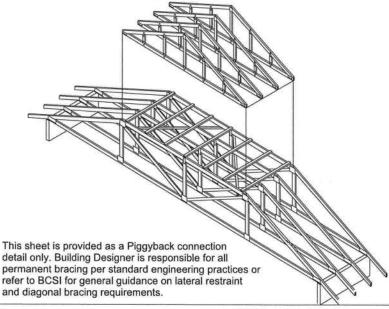


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

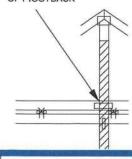
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK NP37 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" NAIL EDGE DISTANCE.



VERTICAL WEB TO **EXTEND THROUGH BOTTOM CHORD** OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL
- ATTACH 2 x x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

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available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



6904 Parke East Blvd

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT-7-16 T23399816

MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C.

CATEGORY II BUILDING EXPOSURE B or C ENCLOSED BUILDING

LOADING = 5 PSF TCDL MINIMUM ASCE 7-10, ASCE 7-16 DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED



A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN

WITH (2) 0(0.131" X 3.5") TOE-NAILED.

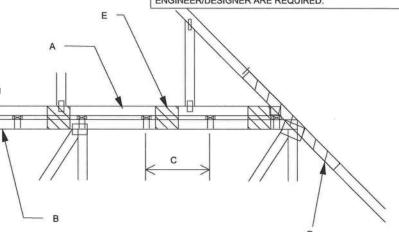
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH. 2 X __ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF

PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131* X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:

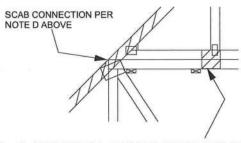
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 180 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

- FOR WIND SPEED IN THE RANGE 116 MPH - 180 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

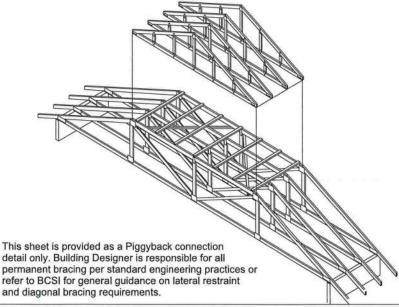


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

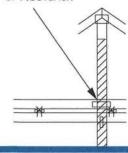
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH **BOTTOM CHORD** OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL

ATTACH 2 x ___ x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH ATTACH 2 x VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)

THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS

GREATER THAN 4000 LBS.

4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH

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April 2,2021

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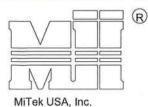
Tampa, FL 36610

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1 T23399817

MiTek USA, Inc.

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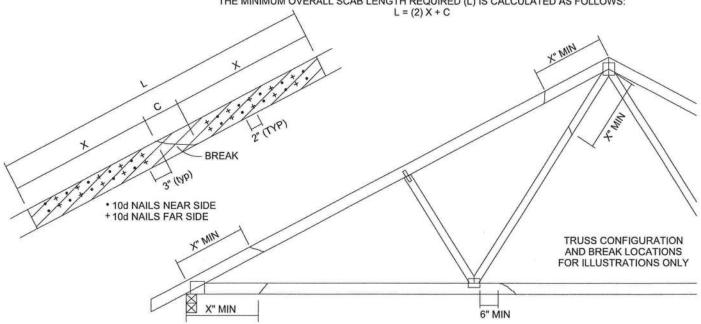


TOTAL NUMBER OF			MAXIMUM FORCE (lbs) 15% LOAD DURATION									
NAILS EACH SIDE OF BREAK *		X INCHES	SP		DF		SPF		HF			
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6		
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028		
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608		
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187		
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767		
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347		

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

NOTES:

- THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED, THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED. THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
- 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR
- THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x_ ORIENTATION ONLY.

 6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

April 2,2021

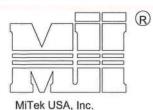


LATERAL TOE-NAIL DETAIL

MII-TOENAIL_SP T23399818

MiTek USA, Inc.

Page 1 of 1



NOTES:

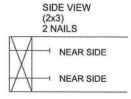
- TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
- 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

	DIAM.	SP	DF	HF	SPF	SPF-S	
O	.131	88.0	80.6	69.9	68.4	59.7	
LONG	.135	93.5	85.6	74.2	72.6	63.4	
3.5"L	.162	108.8	99.6	86.4	84.5	73.8	
SG	.128	74.2	67.9	58.9	57.6	50.3	
LONG	.131	75.9	69.5	60.3	59.0	51.1	
3.25"	.148	81.4	74.5	64.6	63.2	52.5	

ILLUSTRATION PURPOSES ONLY

VIEWS SHOWN ARE FOR



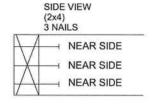
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

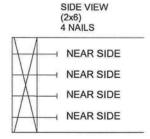
EXAMPLE:

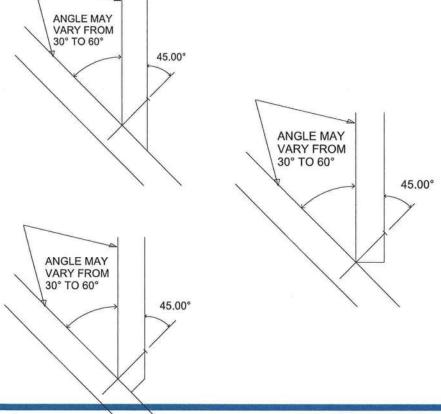
(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity







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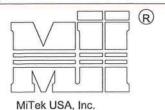
6904 Parke East Blvd. Tampa, FL 36610 APRIL 12, 2019

TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND1 T23399819

MiTek USA, Inc.

Page 1 of 1



GENERAL SPECIFICATIONS

1. NAIL SIZE 10d (0.131" X 3")

 WOOD SCREW = 3" WS3 ÚSP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW

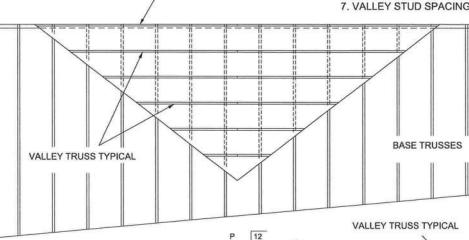
 INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A

 BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.

BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.

6. NAILING DONE PER NDS - 01

7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



SEE DETAIL A BELOW (TYP.)

GABLE END, COMMON TRUSS

OR GIRDER TRUSS

GABLE END, COMMON TRUSS OR GIRDER TRUSS

SECURE VALLEY TRUSS
W/ ONE ROW OF 10d
NAILS 6" O.C.

ATTACH 2x4 CONTINUOUS NO.2 SP
TO THE ROOF W/ TWO USP WS3 (1/4" X 3")
WOOD SCREWS INTO EACH BASE TRUSS.

DETAIL A

N.T.S.

(NO SHEATHING)

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10, ASCE 7-16 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF

ON THE TRUSSES

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021

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available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



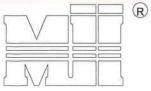
6904 Parke East Blvd. Tampa, FL 36610

TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND2 T23399820

MiTek USA, Inc.

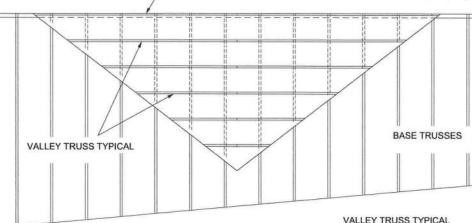
Page 1 of 1



MiTek USA, Inc.

GENERAL SPECIFICATIONS

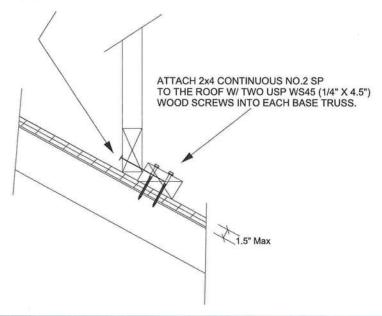
- 1. NAIL SIZE 10d (0.131" X 3") 2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT 3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
- 4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A
- 5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 6. NAILING DONE PER NDS-01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS OR GIRDER TRUSS

> GABLE END, COMMON TRUSS VALLEY TRUSS TYPICAL OR GIRDER TRUSS 12 SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF

ON THE TRUSSES

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

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ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



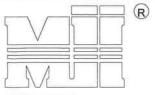
APRIL 12, 2019

TRUSSED VALLEY SET DETAIL

MII-VALLEY SP T23399821

MiTek USA, Inc.

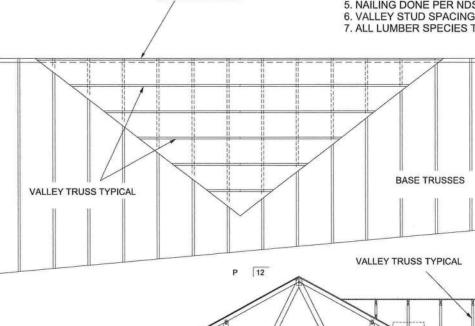
Page 1 of 1



MiTek USA, Inc.

GENERAL SPECIFICATIONS

- 1. NAIL SIZE 16d (0.131" X 3.5") 2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.



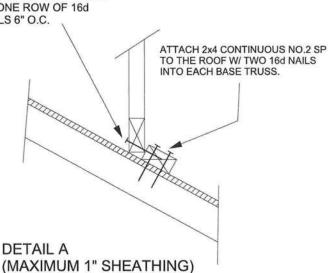
SEE DETAIL A BELOW (TYP.)

GABLE END, COMMON TRUSS

OR GIRDER TRUSS

GABLE END, COMMON TRUSS OR GIRDER TRUSS

SECURE VALLEY TRUSS W/ ONE ROW OF 16d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF

ON THE TRUSSES

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April 2,2021

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



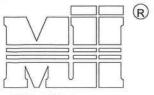
APRIL 12, 2019

TRUSSED VALLEY SET DETAIL

MII-VALLEY SP T23399822

MiTek USA, Inc.

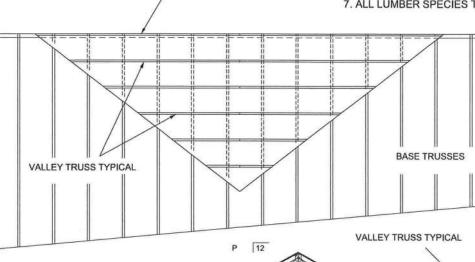
Page 1 of 1



MiTek USA, Inc.

GENERAL SPECIFICATIONS

- 1. NAIL SIZE 16d (0.131" X 3.5")
- 2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.

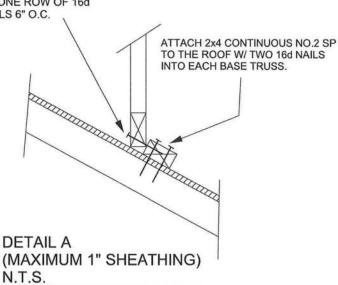


SEE DETAIL A BELOW (TYP.)

GABLE END, COMMON TRUSS OR GIRDER TRUSS

GABLE END, COMMON TRUSS OR GIRDER TRUSS

SECURE VALLEY TRUSS W/ ONE ROW OF 16d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF

MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES

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April 2,2021

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ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



6904 Parke East Blvd. Tampa, FL 36610

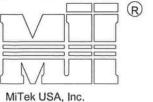
AUGUST 1, 2016

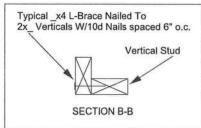
Standard Gable End Detail

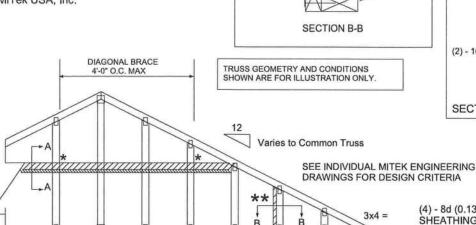
MII-GE146-001 T23399823

MiTek USA, Inc.

Page 1 of 2







- L-Bracing Refer

to Section B-B

Vertical Stud

(4) - 16d Nails

DIAGONAL BRACE

16d Nails

Spaced 6" o.c.

(2) - 10d Nails into 2x6

2x6 Stud or
2x4 No.2 of better

Typical Horizontal Brace Nailed To 2x_ Verticals W/(4)-10d Nails

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK

Roof Sheathing

NOTE:

Diagonal Bracing

Refer to Section A-A

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
- 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
- BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
- DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- 7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- 8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES
- DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
- NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS				
and Grade		Maximum Stud Length							
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10				
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11				
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7				

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING 24" Max 1'-3" - 10d Max. NAILS (2) - 10d NAILS Trusses @ 24" o.c. 2x6 DIAGONAL BRACE SPACED 48" O.C. Diag. Brace ATTACHED TO VERTICAL WITH (4) -16d at 1/3 points NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS. if needed HORIZONTAL BRACE End Wall (SEE SECTION A-A)

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April 2,2021

ASCE 7-98, ASCE 7-02, ASCE 7-05
STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
DURANON-OFILOAD, INCREASE:::4.60 READ CONNECTION OF BRACING IS BASED ON MWFRS473 rev. 5/19/2020

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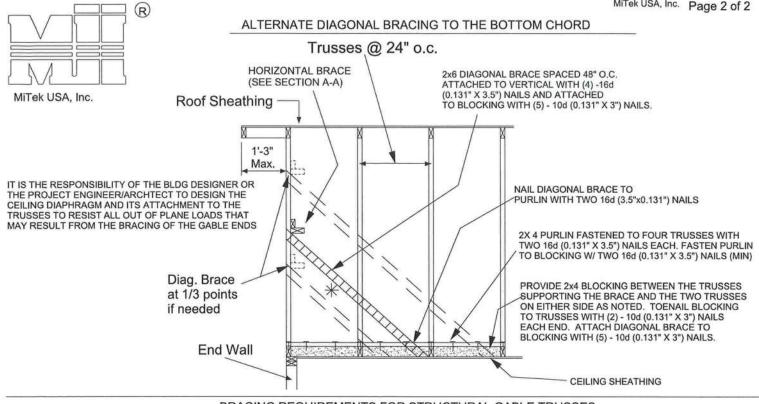
ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

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MiTek USA, Inc. Page 2 of 2



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

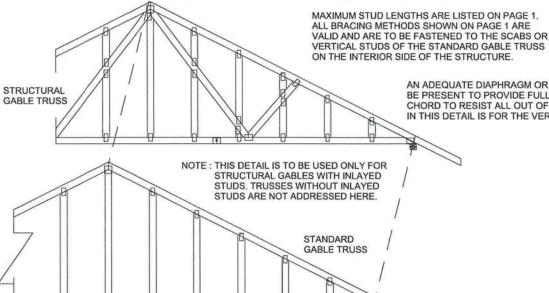
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:
METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE
FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE METHOD 2: ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL

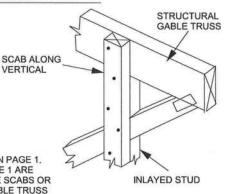
MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C. FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL

MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

VERTICAL

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April 2,2021

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Tampa, FL 36610

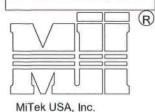
OCTOBER 5, 2016

REPLACE BROKEN OVERHANG

MII-REP13B T23399824

MiTek USA, Inc.

Page 1 of 1



TRUSS CRITERIA:

LOADING: 40-10-0-10 DURATION FACTOR: 1.15 SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12

HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

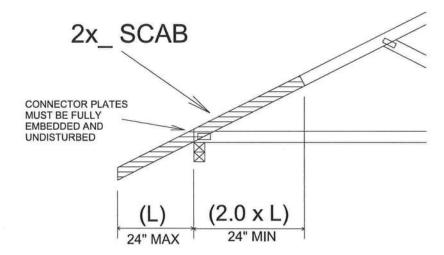
END BEARING CONDITION

NOTES:

1. ATTACH 2x_SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.

2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.

Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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April 2,2021



Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

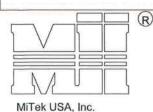
April 2,2021



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MiTek USA, Inc.

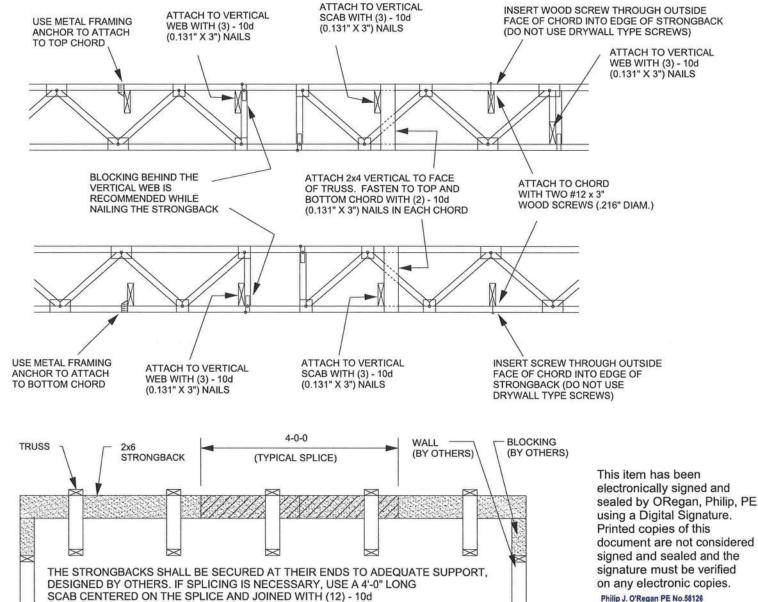
Page 1 of 1



TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END. CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.



ALTERNATE METHOD OF SPLICING: OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d (0.131" X 3") NAILS STAGGERED AND EQUALLY SPACED.

(0.131" X 3") NAILS EQUALLY SPACED.

Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

document are not considered signed and sealed and the signature must be verified Philip J. O'Regan PE No.58126

MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021

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ANSITYPI Quality Criteria, DSB-89 and BCSI Building Component

O BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL



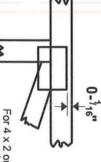
Tampa, FL 36610

Symbols

PLATE LOCATION AND ORIENTATION



Apply plates to both sides of truss offsets are indicated. Center plate on joint unless x, y and fully embed teeth. Dimensions are in ft-in-sixteenths



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

00

required direction of slots in connector plates. This symbol indicates the

Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

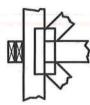
to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



if indicated. output. Use T or I bracing by text in the bracing section of the Indicated by symbol shown and/or

BEARING



Min size shown is for crushing only number where bearings occur. reaction section indicates joint (supports) occur. Icons vary but Indicates location where bearings

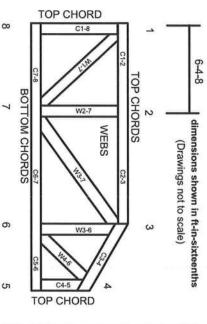
Industry Standards:

ANSI/TPI1: National Design Specification for Metal

DSB-89:

Connected Wood Trusses Guide to Good Practice for Handling, **Building Component Safety Information** Plate Connected Wood Truss Construction. Installing & Bracing of Metal Plate Design Standard for Bracing

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values established by others. Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered
- ω stack materials on inadequately braced trusses. Never exceed the design loading shown and never
- Provide copies of this truss design to the building all other interested parties. designer, erection supervisor, property owner and
- Cut members to bear tightly against each other.
- locations are regulated by ANSI/TPI 1. Place plates on each face of truss at each oint and embed fully. Knots and wane at joint

0 Ġ

- the environment in accord with ANSI/TPI 1. Design assumes trusses will be suitably protected from
- shall not exceed 19% at time of fabrication. Unless otherwise noted, moisture content of lumber
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing. or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- 19 Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.



RE: 3499744 - CANNON RES.

MiTek USA, Inc.

16023 Swingley Ridge Rd Chesterfield, MO 63017

Site Information:

Customer Info: MONICA CANNON Project Name: Cannon Res. Model: Custom

Lot/Block: N/A

Subdivision: N/A

Address: 495 SW Jordan Street, N/A

City: Columbia Cty

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014

Design Program: MiTek 20/20 8.5

Wind Code: ASCE 7-16

Wind Speed: 130 mph

Roof Load: 37.0 psf

Floor Load: N/A psf

This package includes 74 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name PB01	Date
1	T30766082	CJ01 CJ03	6/9/23 6/9/23	15 16	T30766096 T30766097	PB02	6/9/23 6/9/23
2	T30766083 T30766084	CJ03A	6/9/23	16 17	T30766098	PB03	6/9/23
	T30766085	CJ05	6/9/23	18	T30766099 T30766100	PB04 PB05	6/9/23 6/9/23
4 5 6	T30766086 T30766087	CJ05A CJ05B	6/9/23 6/9/23	19 20	T30766101	PB06	6/9/23
7	T30766088	EJ01	6/9/23	20 21 22	T30766102	PB07	6/9/23
8	T30766089	EJ02	6/9/23	22 23	T30766103 T30766104	PB08 PB09	6/9/23 6/9/23
9	T30766090 T30766091	EJ03 EJ04	6/9/23 6/9/23	24	T30766105	PB10	6/9/23
11	T30766092	EJ05	6/9/23	25	T30766106	PB11	6/9/23
12 13	T30766093	HJ06	6/9/23 6/9/23	26 27	T30766107 T30766108	T01 T02	6/9/23 6/9/23
13	T30766094	HJ10 HJ10A	6/9/23	28	T30766109	T03	6/9/23

Reviewed Code Compliance ans Exam

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Lake City, FL.

Truss Design Engineer's Name: ORegan, Philip

My license renewal date for the state of Florida is February 28, 2025.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023



RE: 3499744 - CANNON RES.

MiTek USA, Inc.

16023 Swingley Ridge Rd Chesterfield, MO 63017

Site Information:

Customer Info: MONICA CANNON Project Name: Cannon Res. Model: Custom

Subdivision: N/A

Lot/Block: N/A Address: 495 SW Jordan Street, N/A

State: FL City: Columbia Cty

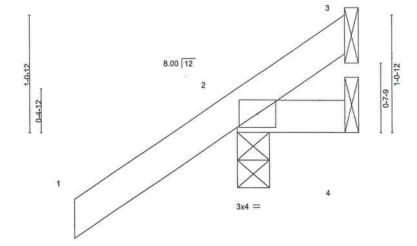
No. 23012334567899012344567890123456789012345	Seal# T30766110 T30766111 T30766111 T30766113 T30766114 T30766116 T30766116 T30766119 T30766120 T30766121 T30766121 T30766122 T30766123 T30766124 T30766125 T30766127 T30766128 T30766130 T30766131	Truss Name T04 T05 T06 T07 T08 T09 T10 T11 T12 T13 T14 T15 T16 T17 T18 T19 T20 T21 T22 T23 T24 T25 T26 T27 T28 T29 T30 T31 T32 T33 T34 T35 T36 T37 T38 T39 T40	Date 6/9/23
61	T30766142	T36	6/9/23
62	T30766143	T37	6/9/23
63	T30766144	T38	6/9/23

CANNON RES. Job Truss Type Truss T30766082 3499744 CJ01 Jack-Open Job Reference (optional) Builders FirstSource, Lake City, FL 32055

Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:44 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-HWUA_Tb77TlpfVMST9leQrPG4WQZ7HmUvuzvMkz83DT

1-0-0 -1-6-0

Scale = 1:10.5



LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.19	Vert(LL)	0.00	7	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.05	Vert(CT)	0.00	7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MP	Section 1997					Weight: 6 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD

REACTIONS.

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=52(LC 12)

Max Uplift 3=-5(LC 1), 2=-69(LC 12), 4=-20(LC 1) Max Grav 3=7(LC 8), 2=179(LC 1), 4=21(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 3, 69 lb uplift at joint 2 and 20 lb uplift at joint 4.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Rogan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



16023 Swingley Ridge Rd Chesterfield, MO 63017

Truss Type CANNON RES Job Truss 12 Jack-Open CJ03 3499744 Job Reference (optional)

1-6-0

Builders FirstSource, Lake City, FL 32055

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Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Scale = 1:17.2

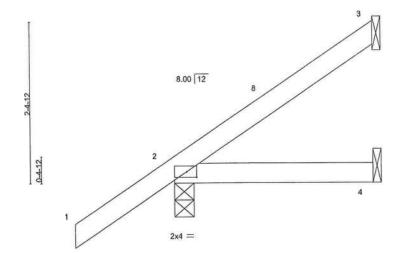


Plate Offsets (X,Y) [2:0-1-13,0-1-0]							
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-4 Plate Grip DOL 1.2-1 Lumber DOL 1.2-1 Rep Stress Incr YES Code FBC2020/TPI2014	CSI. TC 0.16 BC 0.08 WB 0.00 Matrix-MP	DEFL. i Vert(LL) -0.00 Vert(CT) -0.0 Horz(CT) 0.00	4-7	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 13 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD**

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=97(LC 12)

Max Uplift 3=-44(LC 12), 2=-49(LC 12)

Max Grav 3=65(LC 19), 2=210(LC 1), 4=51(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 2-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 3 and 49 lb uplift at joint 2.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

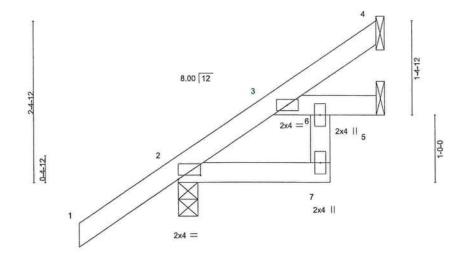
Philip J. O'Regan PE. No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023



Job Truss Truss Type Qty CANNON RES. T30766084 CJ03A Jack-Open 3499744 Job Reference (optional) Builders FirstSource, Lake City, FL 32055 Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:44 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-HWUA_Tb77TIpfVMST9leQrPGTWO67HmUvuzvMkz83ĎT 3-0-0 1-6-0

Scale = 1:17.2



	2-3-0	0-8-6	
	220	000	
1	2-3-0	1 3-0-0 1	

Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Plate Of	fsets (X,Y)	[2:0-4-0,0-0-11]										
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1,25	TC	0.16	Vert(LL)	-0.00	10	>999	240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.14	Vert(CT)	-0.00	7	>999	180	700-100-107	
3CLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	5	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MR	2 5					Weight: 15 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* **BOT CHORD**

6-7: 2x4 SP No.3

REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical

Max Horz 2=97(LC 12)

Max Uplift 4=-25(LC 12), 2=-47(LC 12), 5=-14(LC 12)

Max Grav 4=45(LC 19), 2=216(LC 1), 5=66(LC 3)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-5-2, Interior(1) 1-5-2 to 2-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 4, 47 lb uplift at joint 2 and 14 lb uplift at joint 5.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regau PE No.58126 MITek Inc. DBA MITek USA FL Cert 8634 16033 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

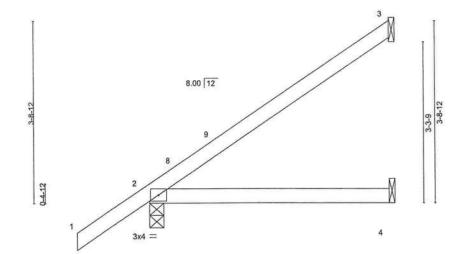
ANSITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



16023 Swingley Ridge Rd Chesterfield, MO 63017

Scale = 1:23.7



LOADIN TCLL TCDL	G (psf) 20.0 7.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25	CSI. TC BC	0.28 0.24	DEFL. Vert(LL) Vert(CT)	in 0.03 -0.06	(loc) 4-7 4-7	l/defl >999 >999	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code FBC2020/T	YES PI2014	WB Matri	0.00 x-MP	Horz(CT)	0.00	3	n/a	n/a	Weight: 19 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=143(LC 12)

Max Uplift 3=-81(LC 12), 2=-49(LC 12), 4=-1(LC 12) Max Grav 3=120(LC 19), 2=276(LC 1), 4=89(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ff; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 4-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

1-6-0

- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 81 lb uplift at joint 3, 49 lb uplift at joint 2 and 1 lb uplift at joint 4.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16025 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

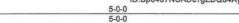
Job	Truss	Truss Type	Qty	Ply	CANNON RES.	
3499744	CJ05A	Jack-Open	2	1	T30766	5086
3499744	CJUSA	Jack-Open	2.	3.	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

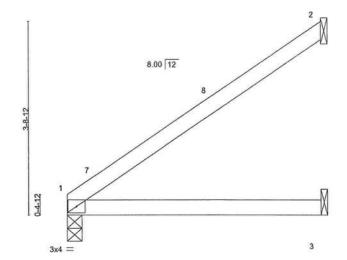
Run: 8.530 s. Jan. 6 2022 Print: 8.530 s. Jan. 6 2022 MITek Industries, Inc. Fri Jun. 9 09:11:45 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-li2YCocmunQgHfxe1tpty2xPswjcsk0e8YjSuBz83DS

Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



Scale = 1:22.3



LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.31	Vert(LL)	0.04	3-6	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.25	Vert(CT)	-0.06	3-6	>964	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	1	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MP	20 (20)					Weight: 17 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

(size) 1=0-3-8, 2=Mechanical, 3=Mechanical

Max Horz 1=113(LC 12)

Max Uplift 1=-10(LC 12), 2=-84(LC 12), 3=-5(LC 12) Max Grav 1=183(LC 1), 2=126(LC 19), 3=91(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 4-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 1, 84 lb uplift at joint 2 and 5 lb uplift at joint 3.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

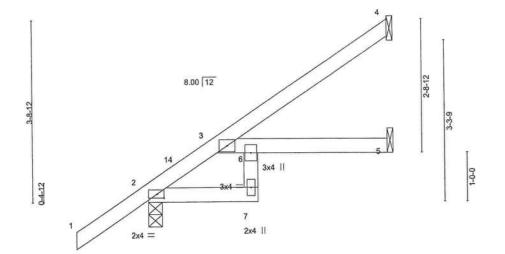
June 9,2023



CANNON RES. Truss Truss Type Job T30766087 CJ05B Jack-Open 3499744 Builders FirstSource, Lake City, FL 32055

1-6-0

Scale = 1:23.7



2-8-8

BRACING-

TOP CHORD

BOT CHORD

SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
Plate Grip DOL	1.25	TC	0.21					370,0000	M120	244/190
Lumber DOL	1.25	BC	0.31	Vert(CT)	-0.05	5-6	>999	180	1	
Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.02	5	n/a	n/a	Weight: 22 lb	FT = 20%
	Plate Grip DOL Lumber DOL Rep Stress Incr	Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES	Plate Grip DOL 1.25 TC Lumber DOL 1.25 BC Rep Stress Incr YES WB	Plate Grip DOL 1.25 TC 0.21 Lumber DOL 1.25 BC 0.31 Rep Stress Incr YES WB 0.00	Plate Grip DOL 1.25 TC 0.21 Vert(LL) Lumber DOL 1.25 BC 0.31 Vert(CT) Rep Stress Incr YES WB 0.00 Horz(CT)	Plate Grip DOL 1.25 TC 0.21 Vert(LL) 0.03 Lumber DOL 1.25 BC 0.31 Vert(CT) -0.05	Plate Grip DOL 1.25 TC 0.21 Vert(LL) 0.03 5-6 Lumber DOL 1.25 BC 0.31 Vert(CT) -0.05 5-6 Rep Stress Incr YES WB 0.00 Horz(CT) 0.02 5	Plate Grip DOL 1.25 TC 0.21 Vert(LL) 0.03 5-6 >999 Lumber DOL 1.25 BC 0.31 Vert(CT) -0.05 5-6 >999 Rep Stress Incr YES WB 0.00 Horz(CT) 0.02 5 n/a	Plate Grip DOL 1.25 TC 0.21 Vert(LL) 0.03 5-6 >999 240 Lumber DOL 1.25 BC 0.31 Vert(CT) -0.05 5-6 >999 180 Rep Stress Incr YES WB 0.00 Horz(CT) 0.02 5 n/a n/a	Plate Grip DOL 1.25 TC 0.21 Vert(LL) 0.03 5-6 >999 240 MT20 Lumber DOL 1.25 BC 0.31 Vert(CT) -0.05 5-6 >999 180 Rep Stress Incr YES WB 0.00 Horz(CT) 0.02 5 n/a n/a

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 **BOT CHORD**

2x4 SP No.2 *Except*

6-7: 2x4 SP No.3

(size) 4=Mechanical, 2=0-3-8, 5=Mechanical

Max Horz 2=143(LC 12)

Max Uplift 4=-64(LC 12), 2=-46(LC 12), 5=-16(LC 12) Max Grav 4=106(LC 19), 2=285(LC 1), 5=93(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-5-3, Interior(1) 1-5-3 to 4-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 4, 46 lb uplift at joint 2 and 16 lb uplift at joint 5.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Philip J. O'Regau PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and propry damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



-1-6-0 3-7-0 TO:Bpo4d/NCADc?gZDQu4Ayb\

Scale = 1:30.1

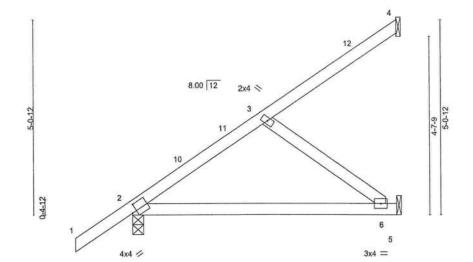


Plate Off	sets (X,Y) [2:0-1-9,0-2-5]										
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.35	Vert(LL)	-0.08	6-9	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.44	Vert(CT)	-0.16	6-9	>526	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.08	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MS	Entertail A					Weight: 32 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS.

(size) 4=Mechanical, 2=0-3-8, 5=Mechanical

Max Horz 2=182(LC 12)

Max Uplift 4=-47(LC 12), 2=-55(LC 12), 5=-59(LC 12) Max Grav 4=75(LC 19), 2=346(LC 1), 5=186(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 6-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 4, 55 lb uplift at joint 2 and 59 lb uplift at joint 5.

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Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Philip J. O'Regau PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023





Truss Type Qty CANNON RES Truss Job Jack-Partial EJ02 3499744 Job Reference (optional) Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MITek Industries, Inc. Fri Jun 9 09:11:46 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-DucxP8dOf5YXvpWrbaK6VGUXUJ1ebAgnNBS0Qdz83DR Builders FirstSource, Lake City, FL 32055 7-0-0 -1-6-0 Scale = 1:30.1 8.00 12 2x4 = 4-0-12 3x4 || 0.4-12 9 2x4 ||

OADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.25	TC	0.45	Vert(LL)	-0.04	7-8	>999	240	MT20	244/190
CDL 7.0	Lumber DOL	1.25	BC	0.33	Vert(CT)	-0.10	7-8	>847	180	- ALLOCATIONS	
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.10	Horz(CT)	0.03	6	n/a	n/a		
3CDL 10.0	Code FBC2020/T	PI2014	Matri	x-MS						Weight: 33 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* **BOT CHORD**

8-9: 2x4 SP No.3

WEBS 2x4 SP No.3

REACTIONS.

(size) 5=Mechanical, 2=0-3-8, 6=Mechanical

Max Horz 2=182(LC 12)

Max Uplift 5=-46(LC 12), 2=-55(LC 12), 6=-60(LC 12) Max Grav 5=73(LC 19), 2=346(LC 1), 6=187(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

3-4=-310/85 TOP CHORD **BOT CHORD** 7-8=-241/344 WEBS 4-7=-371/261

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-5-2, Interior(1) 1-5-2 to 6-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 5, 55 lb uplift at joint 2 and 60 lb uplift at joint 6.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Philip J. O'Regau PE. No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

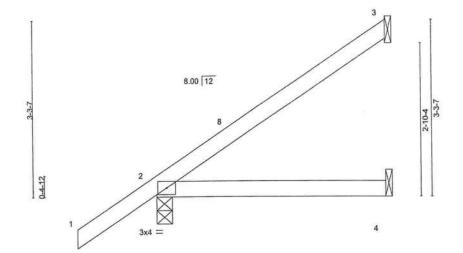


CANNON RES Job Truss Truss Type T30766090 EJ03 Jack-Open 3499744 Job Reference (optional)

Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:46 2023 Page
ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-DucxP8dOf5YXvpWrbaK6VGUbOJ44bBGnNBS0Qdz83DR Builders FirstSource, Lake City, FL 32055

1-6-0

Scale = 1:21.5



LOADING TCLL TCDL	20.0 7.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25	CSI. TC BC	0.20 0.18	Vert(LL)	0.02	(loc) 4-7 4-7	Vdefl >999 >999	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL	0.0 *	Rep Stress Incr Code FBC2020/T	YES PI2014	WB Matri	0.00 x-MP	Horz(CT)	0.00	3	n/a	n/a	Weight: 17 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=127(LC 12)

Max Uplift 3=-69(LC 12), 2=-48(LC 12)

Max Grav 3=102(LC 19), 2=253(LC 1), 4=77(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 4-3-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 3 and 48 lb uplift at joint 2.

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Philip J. O'Regna PE No.58126 MiTek lur. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023



Structural wood sheathing directly applied or 4-4-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Builders FirstSource, Lake City, FL 32055

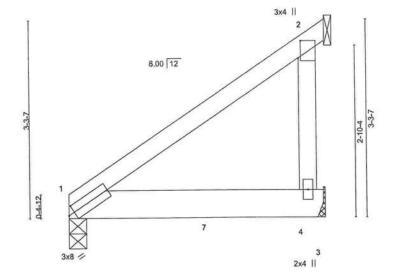
Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MTek Industries, Inc. Fri Jun 9 09:11:46 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-DucxP8dOf5YXvpWrbaK6VGUZ1J0WbBGnNBS0Qdz83DR 44-0

Structural wood sheathing directly applied or 4-4-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

4-4-0

Scale = 1:19.2



			1000		Vietowie, vo						+0.11-0.010+1:
OADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.25	TC	0.35	Vert(LL)	-0.04	4-6	>999	240	MT20	244/190
CDL 7.0	Lumber DOL	1.25	BC	0.40	Vert(CT)	-0.07	4-6	>708	180	M(N.2020)	
BCLL 0.0 *	Rep Stress Incr	NO	WB	0.00	Horz(CT)	-0.00	2	n/a	n/a		
3CDL 10.0	Code FBC2020/TI	PI2014	Matri	x-MP						Weight: 22 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP M 26 WEBS 2x4 SP No.3

WEBS 2x4 SP I

REACTIONS.

(size) 1=0-3-8, 4=Mechanical, 2=Mechanical

Max Horz 1=93(LC 8)

Max Uplift 1=-219(LC 8), 4=-124(LC 8), 2=-67(LC 8) Max Grav 1=1218(LC 2), 4=670(LC 2), 2=123(LC 29)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 lb uplift at joint 1, 124 lb uplift at joint 4 and 67 lb uplift at joint 2.
- 8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 873 lb down and 179 lb up at 0-4-12, and 858 lb down and 176 lb up at 2-4-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-2=-54, 1-3=-20 Concentrated Loads (lb)

Vert: 6=-807(F) 7=-801(F)

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Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. ChesterBeld, MO 63017 Date:

June 9,2023



 Job
 Truss
 Truss Type
 Qty
 Ply
 CANNON RES.
 T30766092

 3499744
 EJ05
 Jack-Open Girder
 1
 1
 1
 Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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3-8-0 3-8-0

Scale = 1:17.1

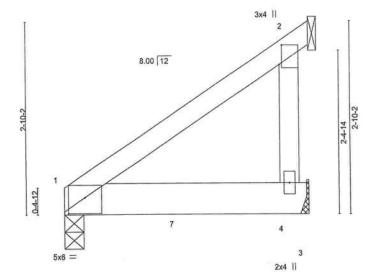


Plate Offsets (X,Y)	1:0-3-0,0-2-13], [2:0-3-7,	0-0-8]									
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/T	2-0-0 1.25 1.25 NO PI2014	CSI. TC BC WB Matri:	0.35 0.46 0.00 x-MP	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.03 -0.05 -0.00	(loc) 4-6 4-6 2	l/defl >999 >828 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 18 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP M 26 WEBS 2x4 SP No.3 **BRACING-**

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-8-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=0-3-8, 4=Mechanical, 2=Mechanical

Max Horz 1=77(LC 8)

Max Uplift 1=-153(LC 8), 4=-152(LC 8), 2=-58(LC 8) Max Grav 1=848(LC 2), 4=787(LC 2), 2=114(LC 29)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 153 lb uplift at joint 1, 152 lb uplift at joint 4 and 58 lb uplift at joint 2.
- Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1518 lb down and 311 lb up at 1-8-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-54, 1-3=-20

Concentrated Loads (lb)
Vert: 7=-1384(F)

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Philip J. O'Regau PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

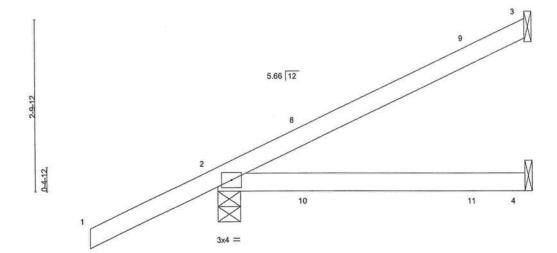
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
ANSITTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Sulte 203 Waldorf, MD 20601



-2-1-7 5-1-8 2-1-7 5-1-8

Scale = 1:18.9



LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.		(loc)	l/defi	L/d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.25	TC	0.32	Vert(LL)	-0.04	4-7	>999	240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.29	Vert(CT)	-0.07	4-7	>875	180	000000000	
CLL	0.0	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MP						Weight: 20 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

(size) 3=Mechanical, 2=0-4-9, 4=Mechanical

Max Horz 2=112(LC 8)

Max Uplift 3=-97(LC 8), 2=-128(LC 8), 4=-3(LC 8) Max Grav 3=117(LC 1), 2=329(LC 1), 4=103(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 97 lb uplift at joint 3, 128 lb uplift at joint 2 and 3 lb uplift at joint 4.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 62 lb down and 73 lb up at 1-6-1, 62 lb down and 73 lb up at 1-6-1, and 71 lb down and 48 lb up at 4-4-0, and 71 lb down and 48 lb up at 4-4-0 on top chord, and 21 lb down and 45 lb up at 1-6-1, 21 lb down and 45 lb up at 1-6-1, and 27 lb down at 4-4-0, and 27 lb down at 4-4-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 4-5=-20

Concentrated Loads (lb)

Vert: 9=-8(F=-4, B=-4) 11=-13(F=-7, B=-7)

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Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023



Structural wood sheathing directly applied or 5-1-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

CANNON RES. Truss Type Truss Job T30766094 Diagonal Hip Girder HJ10 3499744 Job Reference (optional) Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:47 2023 Page ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-h5AJdUe0POgOWz519IrL1T1g1jHmKZCwcrCZ23z83DQ Builders FirstSource, Lake City, FL 32055 9-10-1 Scale = 1:29.2 14-9 12 5.66 12 3x4 = 0-4-12 15 6 14 7 3x4 = 52x4 || 3x4 = 9-10-1 4-6-0 4-6-0 GRIP PLATES DEFL (loc) I/defl L/d CSI SPACING-2-0-0 LOADING (psf) 244/190 >999 240 MT20 Vert(LL) -0.06 6-7 0.59 Plate Grip DOL 1.25 TC 20.0 TCLL Vert(CT) -0.14 6-7 >836 180 Lumber DOL 1.25 BC 0.66 7.0 TCDI n/a Horz(CT) 0.01 5 n/a Rep Stress Incr NO WB 0.34 0.0 BCLL Weight: 45 lb FT = 20%Code FBC2020/TPI2014 Matrix-MS BCDL 10.0 **BRACING-**LUMBER-Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (siz

(size) 4=Mechanical, 2=0-4-9, 5=Mechanical

Max Horz 2=182(LC 8)

Max Uplift 4=-94(LC 8), 2=-191(LC 8), 5=-113(LC 8) Max Grav 4=150(LC 1), 2=526(LC 1), 5=298(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-671/211

BOT CHORD 2-7=-288/556, 6-7=-288/556 WEBS 3-7=-2/302, 3-6=-613/318

NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 4, 191 lb uplift at joint 2 and 113 lb uplift at joint 5.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 62 lb down and 73 lb up at 1-6-1, 62 lb down and 73 lb up at 1-6-1, 80 lb down and 46 lb up at 4-4-0, 80 lb down and 46 lb up at 4-4-0, and 109 lb down and 92 lb up at 7-1-15, and 109 lb down and 92 lb up at 7-1-15 on top chord, and 21 lb down and 45 lb up at 1-6-1, 21 lb down and 45 lb up at 1-6-1, 25 lb down at 4-4-0, 25 lb down at 4-4-0, and 47 lb down and 16 lb up at 7-1-15, and 47 lb down and 16 lb up at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 5-8=-20

Concentrated Loads (lb)

Vert: 7=-4(F=-2, B=-2) 12=-73(F=-36, B=-36) 15=-59(F=-29, B=-29)

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Phillip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16925 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

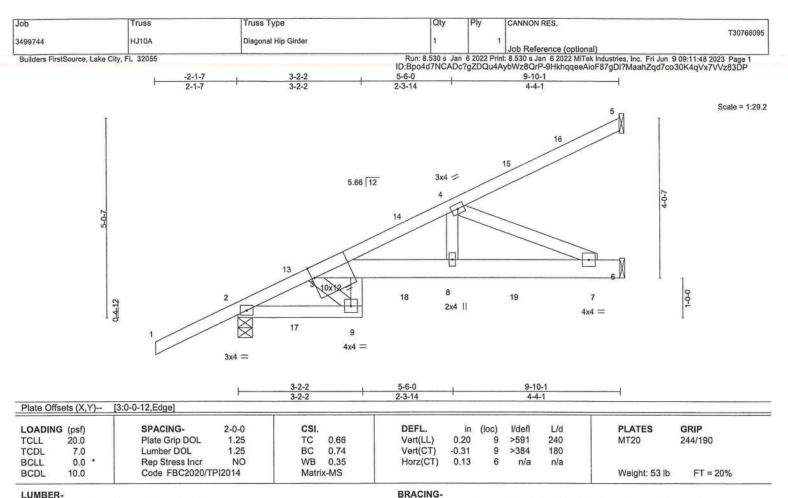
June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP 2850F 2.0E or 2x4 SP M 31

BOT CHORD 2x4 SP No.2 *Except*

3-9: 2x4 SP No.3, 3-6: 2x6 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (size) 5=Mechanical, 2=0-4-9, 6=Mechanical

Max Horz 2=182(LC 8)

Max Uplift 5=-58(LC 8), 2=-207(LC 8), 6=-161(LC 8) Max Grav 5=103(LC 1), 2=566(LC 1), 6=377(LC 1)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

TOP CHORD 3-11=-255/24, 3-4=-1006/387 3-8=-452/897, 7-8=-455/902 **BOT CHORD** WEBS 4-8=-174/469, 4-7=-981/495

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 58 lb uplift at joint 5, 207 lb uplift at joint 2 and 161 lb uplift at joint 6.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 62 lb down and 73 lb up at 1-6-1, 62 lb down and 73 lb up at 1-6-1, 67 lb down and 25 lb up at 4-4-0, 67 lb down and 25 lb up at 4-4-0, and 103 lb down and 74 lb up at 7-1-15, and 103 lb down and 74 lb up at 7-1-15 on top chord, and 21 lb down and 45 lb up at 1-6-1, 21 lb down and 45 lb up at 1-6-1, 46 lb down and 27 lb up at 4-4-0, 46 lb down and 27 lb up at 4-4-0, and 59 lb down and 34 lb up at 7-1-15, and 59 lb down and 34 lb up at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 9-10=-20, 3-6=-20

Concentrated Loads (lb)

Vert: 15=-46(F=-23, B=-23) 18=-52(F=-26, B=-26) 19=-96(F=-48, B=-48)

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Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 2-9.

Philip J. O'Regna PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2570 Crain Highway, Suite 203 Waldorf, MD 20601



 Job
 Truss
 Truss Type
 Qty
 Ply
 CANNON RES.
 T30766096

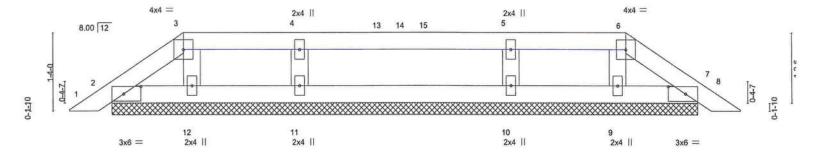
 3499744
 PB01
 GABLE
 2
 1
 Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

| Job Reference (optional)
| Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:48 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-9HkhqqeeAioF87gDi?MaahZzU7m735G4qVx7VVz83DP

11-8-0

Scale = 1:19.7



-				-1-17		11-8-0 11-8-0						
Plate Off	sets (X,Y)	[2:0-3-9,0-1-8], [7:0-3-9,0	-1-8]									
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.10	Vert(LL)	0.00	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.07	Vert(CT)	0.00	7	n/r	120		
BCLL	0.0	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	7	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-S	000000000000000000000000000000000000000					Weight: 37 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 10-1-12.

(lb) - Max Horz 2=-26(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 7, 10, 11, 12, 9

Max Grav All reactions 250 lb or less at joint(s) 2, 7, 10, 11, 12, 9

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-3-5 to 2-0-0, Exterior(2R) 2-0-0 to 6-2-15, Interior(1) 6-2-15 to 9-8-0, Exterior(2E) 9-8-0 to 11-4-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) Provide adequate drainage to prevent water ponding.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 4-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 7, 10, 11, 12, 9.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J, O'Rogan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16025 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Scale = 1:19.7

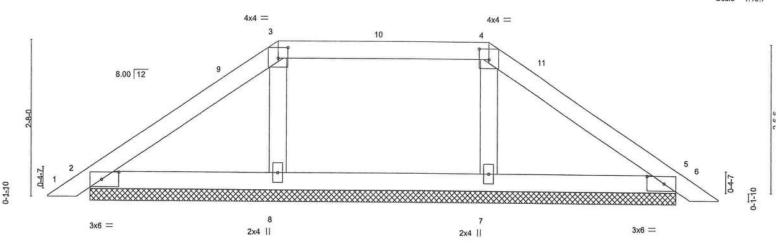


Plate Offsets (X,Y)	[2:0-3-9,0-1-8], [3:0-2-0,0-2-3], [4	.0-2-0,0-2-3], [0.0-3-9,0-1-0]							
LOADING (psf) TCLL 20.0 TCDL 7.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25	TC 0.13 BC 0.09	DEFL. Vert(LL) Vert(CT)	in 0.00 0.00	(loc) 6 6	l/defl n/r n/r	L/d 120 120	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code FBC2020/TPI2014	WB 0.04 Matrix-S	Horz(CT)	0.00	5	n/a	n/a	Weight: 41 lb	FT = 20%

11-8-0

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 10-1-12.

(lb) - Max Horz 2=-55(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 5, 7, 8

Max Grav All reactions 250 lb or less at joint(s) 2, 5 except 7=265(LC 24), 8=265(LC 23)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-3-5 to 3-3-5, Interior(1) 3-3-5 to 4-0-0, Exterior(2E) 4-0-0 to 11-4-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) Provide adequate drainage to prevent water ponding.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 4-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members,
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5, 7, 8.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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Philip J. O'Regan PE No.58126 MiTek far. DBA MITek USA FL Cert 6634 16023 Swingley Bidge Rd. Chesterfield, MO 63017

June 9,2023

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



CANNON RES. Truss Type Job Truss T30766098 PB03 GABLE 3499744 | Job Reference (optional)
| Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:49 2023 Page 1
| ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-dTH31AfGx0w6mGFQGjtp6u68AX6LoYKD39hg1yz83DO Builders FirstSource, Lake City, FL 32055 11-8-0 Scale: 1/2"=1" 4x8 = 4 2x4 || 5 2x4 || 8.00 12 3 13 0-4-Z 1-1-10 1-1-10 0-1-10 3x6 = 3x6 = 2x4 || 2x4 || 2x4 || 11-8-0 11-8-0 [2:0-3-9,0-1-8], [6:0-3-9,0-1-8] Plate Offsets (X,Y)-PLATES GRIP DEFL CSI. in (loc) I/defi 1/4 SPACING-LOADING (psf) 244/190 Plate Grip DOL 1.25 TC 0.10 Vert(LL) 0.00 n/r 120 MT20 TCLL 20.0 1.25 BC 0.08 Vert(CT) 0.00 n/r 120 Lumber DOL TCDL 7.0 Horz(CT) 0.00 6 WB 0.04 n/a n/a Rep Stress Incr BCLL 0.0 YES Weight: 46 lb FT = 20% Code FBC2020/TPI2014 Matrix-S BCDL 10.0

LUMBER-

2x4 SP No.2 TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 **OTHERS**

BRACING-

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 10-1-12.

Max Horz 2=-82(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 8=-117(LC 13), 10=-118(LC 12) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9 except 8=260(LC 20), 10=260(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-3-5 to 3-3-5, Interior(1) 3-3-5 to 5-10-0, Exterior(2R) 5-10-0 to 10-0-15, Interior(1) 10-0-15 to 11-4-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 4-0-0 oc.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 8=117, 10=118
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regen PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023





Job Truss Type CANNON RES. T30766099 PB04 GABLE 3499744 Job Reference (optional) Builders FirstSource, Lake City, FL 32055 Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:49 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-dTH31AfGx0w6mGFQGjlp6u699X70oYhD39hg1yz83DO

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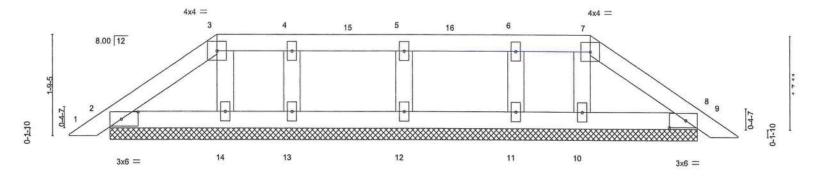


Plate Offsets	(X Y) [2:0-3-9,0-1-8], [8:0-3-9,0-	-1-81			12-0-0						
iato Onobio	1	2.0 0 0,0 1 0,1 (0.0 0 0,0	1-0]					_				
LOADING (p	sf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20	0.0	Plate Grip DOL	1.25	TC	0.04	Vert(LL)	0.00	8	n/r	120	MT20	244/190
TCDL 7	7.0	Lumber DOL	1.25	BC	0.03	Vert(CT)	0.00	8	n/r	120		
BCLL (0.0 *	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	8	n/a	n/a		
BCDL 10	0.0	Code FBC2020/TF	PI2014	Matrix	k-S					321102	Weight: 43 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD** WEBS 2x4 SP No.3 2x4 SP No.3 OTHERS

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 10-5-12.

(lb) - Max Horz 2=35(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 12, 11, 13, 14, 10 Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 11, 13, 14, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-3-5 to 2-8-0, Exterior(2R) 2-8-0 to 6-10-15, Interior(1) 6-10-15 to 9-4-0, Exterior(2E) 9-4-0 to 11-8-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Provide adequate drainage to prevent water ponding.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 4-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 12, 11, 13,
- 12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regnu PE No.58126 Millek lur. DBA Millek USA FL Cert 6634 16025 Swingley Ridge Rd. Chesterfield, MO 63017 Dates:

June 9,2023

eters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

**available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Joh Truss Truss Type CANNON RES T30766100 3499744 PB05 GABLE | Job Reference (optional)
| Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Frl Jun 9 09:11:49 2023 Page 1
| ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-dTH31AfGx0w6mGFQGjtp6u687X6MoYPD39hg1yz83DO Builders FirstSource, Lake City, FL 32055 12-0-0 2-8-0 Scale = 1:20.6 3x6 / 2x4 II 3x6 > 5 6 2x4 || 7 2x4 || 3 8.00 12 0-4-7 12 10 3x6 = 3x6 = 2x4 || 2x4 || 2x4 || [2:0-3-9,0-1-8], [4:0-3-0,0-0-2], [6:0-3-0,0-0-2], [8:0-3-9,0-1-8] LOADING (psf) SPACING-CSI DEFL in I/defl L/d PLATES (loc) GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.10 Vert(LL) 0.00 n/r 120 MT20 244/190 TCDL 7.0 1.25 BC Lumber DOL 0.07 Vert(CT) 0.00 9 n/r 120 BCLL 0.0 Rep Stress Incr YES WB 0.04 Horz(CT) 0.00 8 n/a n/a BCDL Code FBC2020/TPI2014 10.0 Matrix-S Weight: 46 lb FT = 20% LUMBER-**BRACING-**TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD 2x4 SP No.2 **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing. **OTHERS** 2x4 SP No.3

REACTIONS.

All bearings 10-5-12.

(lb) - Max Horz 2=-65(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 11 except 10=-103(LC 13), 12=-107(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 2, 8, 11 except 10=252(LC 1), 12=256(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown,

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-3-5 to 3-3-5, Interior(1) 3-3-5 to 4-8-0, Exterior(2E) 4-8-0 to 11-8-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) Provide adequate drainage to prevent water ponding.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 4-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 11 except (jt=lb) 10=103, 12=107.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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Philip J. O'Regau PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for slability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type CANNON RES T30766101 3499744 PB06 GABLE Job Reference (optional)

Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Frl Jun 9 09:11:49 2023 Page
ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-dTH31AfGx0w6mGFQGjtp6u685X6MoYUD39hg1yz83DO Builders FirstSource, Lake City, FL 32055 12-0-0 Scale = 1:20.3 2x4 || 5x6 = 5x6 = 5 6 14 8.00 12 13 9 0-1-10 12 10 3x6 = 2x4 || 3x6 =2x4 || 12-0-0 Plate Offsets (X,Y)--[2:0-3-9,0-1-8], [4:0-3-8,0-1-12], [6:0-3-8,0-1-12], [8:0-3-9,0-1-8] LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.11 Vert(LL) 0.00 9 n/r 120 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.07 Vert(CT) 0.00 9 n/r 120 BCLL 0.0 Rep Stress Incr YES WB 0.03 Horz(CT) 0.00 8 n/a BCDL 10.0 Code FBC2020/TPI2014 Matrix-S Weight: 45 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 10-5-12.

(lb) - Max Horz 2=-58(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 11, 10, 12

Max Grav All reactions 250 lb or less at joint(s) 2, 8, 11 except 10=251(LC 1), 12=251(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-3-5 to 3-3-5, Interior(1) 3-3-5 to 4-2-8, Exterior(2E) 4-2-8 to 11-8-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) Provide adequate drainage to prevent water ponding.
- 6) Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 11, 10, 12.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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Philip J. O'Rogna PE No.58126 MITek Inc. DBA MiTek USA FL Cert 6634 16025 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023



Job Truss Type Qty CANNON RES. Trus T30766102 PB07 3499744 Piggyback Job Reference (optional)

Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:50 2023 Page 1
ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-5frRFWguiJ2zNQqcQQO2f6flvwS_X?INIpQDZOz83DN Builders FirstSource, Lake City, FL 32055 12-0-0 4-8-0 Scale = 1:21.2 4x4 = 3 8.00 12 4x4 = 4x4 = -5-10 6 0-1-10 10 3x6 = 3x6 = 2x4 || 2x4 || 2x4 || 12-0-0 [2:0-3-9,0-1-8], [6:0-3-9,0-1-8] Plate Offsets (X,Y)--LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) **V**defl L/d PLATES GRIP Plate Grip DOL TC 0.17 Vert(LL) 0.00 TCLL 20.0 1.25 6 n/r 120 MT20 244/190 BC 7.0 Lumber DOL 1.25 0.11 Vert(CT) 0.00 6 120 TCDL n/r BCLL 0.0 Rep Stress Incr YES WB 0.03 Horz(CT) 0.00 6 n/a n/a Code FBC2020/TPI2014 BCDL 10.0 Matrix-S Weight: 42 lb FT = 20% LUMBER-**BRACING-**2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD 2x4 SP No.2 **BOT CHORD BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2x4 SP No.3 6-0-0 oc bracing: 8-9. WEBS 2x4 SP No.3 **OTHERS**

REACTIONS. All bearings 10-5-12.

(lb) - Max Horz 2=-64(LC 10) Max Uplift All uplift 100

Max Uplift All uplift 100 lb or less at joint(s) 2, 6, 10, 9, 8

Max Grav All reactions 250 lb or less at joint(s) 2, 6, 10, 9, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-3-5 to 3-3-5, Interior(1) 3-3-5 to 4-8-0, Exterior(2E) 4-8-0 to 7-1-8, Interior(1) 7-1-8 to 9-9-8, Exterior(2E) 9-9-8 to 11-8-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6, 10, 9, 8.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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Philip J. O'Rogan PE No. 88126 MiTek Inc. DBA MITek USA FL Cort 5634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



T30766103 **PB08** Piggyback 3499744 Job Reference (optional) Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MITek Industries, Inc. Fri Jun 9 09:11:50 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-5frRFWguiJ2zNQqcqQO2f6flwwRSX?gNlpQDZOz83DN Builders FirstSource, Lake City, FL 32055 9-4-0 4-8-0 Scale = 1:20.1 4x4 = 3 8.00 12 0-4-7 6 2x4 = 2x4 = 244 11 9-4-0 GRIP PLATES DEFL (loc) I/defl L/d CSI SPACING-2-0-0 LOADING (psf) 244/190 120 MT20 Vert(LL) 0.00 5 n/r 0.17 Plate Grip DOL 1.25 TC 20.0 TCLL Vert(CT) 0.01 5 n/r 120 Lumber DOL 1.25 BC 0.15 7.0 TCDL Horz(CT) 0.00 4 n/a n/a Rep Stress Incr YES WB 0.04 BCLL 0.0 Weight: 32 lb FT = 20% Matrix-S Code FBC2020/TPI2014 BCDL 10.0

LUMBER-

Job

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD** 2x4 SP No.3 **OTHERS**

BRACING-

Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

CANNON RES.

REACTIONS.

(size) 2=7-9-12, 4=7-9-12, 6=7-9-12

Max Horz 2=64(LC 11)

Truss

Max Uplift 2=-48(LC 12), 4=-57(LC 13), 6=-39(LC 12) Max Grav 2=169(LC 1), 4=169(LC 1), 6=293(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-3-5 to 3-3-5, Interior(1) 3-3-5 to 4-8-0, Exterior(2R) 4-8-0 to 7-8-0, Interior(1) 7-8-0 to 9-0-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Gable requires continuous bottom chord bearing.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.

Truss Type

8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building

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June 9,2023





Qty Job Truss Truss Type CANNON RES T30766104 PB09 GABLE 3499744 Job Reference (optional) Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:50 2023 Page ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-5frRFWguiJ2zNQqcqQO2f6fJywSXX?sNIpQDZOz83DN Builders FirstSource, Lake City, FL 32055 3-10-8 Scale = 1:18,0 4x4 = 4x4 = 10 8.00 12 5 0-4-7 2x4 || 2x4 || 2x4 = 2x4 = [3:0-2-0,0-2-3], [4:0-2-0,0-2-3] Plate Offsets (X,Y)--CSI. DEFL. l/defl L/d **PLATES** GRIP LOADING (psf) SPACING-(loc) Plate Grip DOL 1.25 TC 0.10 Vert(LL) 0.00 n/r 120 MT20 244/190 TCII 20 0 BC 0.08 Vert(CT) 0.00 6 n/r 120 7.0 Lumber DOL 1.25 TCDL WB Horz(CT) 0.00 5

LUMBER-

BCLL

BCDL

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD** 2x4 SP No.3 OTHERS

0.0

10.0

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Weight: 33 lb

FT = 20%

Rigid ceiling directly applied or 10-0-0 oc bracing.

n/a

n/a

REACTIONS. All bearings 7-9-12.

(lb) - Max Horz 2=-53(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 5, 7, 8 Max Grav All reactions 250 lb or less at joint(s) 2, 5, 7, 8

YES

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

Rep Stress Incr

Code FBC2020/TPI2014

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-3-5 to 3-3-5, Interior(1) 3-3-5 to 3-10-8, Exterior(2E) 3-10-8 to 9-0-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

0.03

Matrix-S

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) Provide adequate drainage to prevent water ponding.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 4-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5, 7, 8.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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Philip J. O'Regau PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

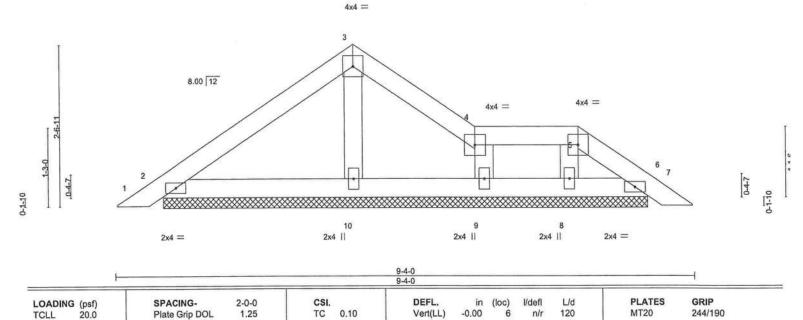
sters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guildinace regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Co. Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component



Scale = 1:18.3



Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

0.00

0.00

6

6

n/r

n/a

6-0-0 oc bracing: 8-9.

120

n/a

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

Weight: 32 lb

FT = 20%

LUMBER-

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

7.0

0.0

10.0

WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

REACTIONS. All bearings 7-9-12.

(lb) - Max Horz 2=-52(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 6, 10, 9, 8

Max Grav All reactions 250 lb or less at joint(s) 2, 6, 10, 9, 8

1.25

YES

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Lumber DOL

Rep Stress Incr

Code FBC2020/TPI2014

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-3-5 to 3-3-5, Interior(1) 3-3-5 to 3-10-0, Exterior(2E) 3-10-0 to 5-9-8, Interior(1) 5-9-8 to 7-5-8, Exterior(2E) 7-5-8 to 9-0-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

BC

WB

Matrix-S

0.07

0.03

- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6, 10, 9, 8.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regau PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16025 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see __ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information __available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



CANNON RES. Truss Type Job Truss T30766106 Piggyback 3499744 PR11 | Job Reference (optional)
| Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:51 2023 Page | ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-asPpSshWTdAq?aPo08wHCJBT?KoVGS6WWTAn6qz83DM Builders FirstSource, Lake City, FL 32055 7-8-0 Scale = 1:16.8 4x4 = 3 8.00 12 0-4-7 0-1-10 6 2x4 = 2x4 || 2x4 = 7-8-0 PLATES GRIP CSI. DEFL in (loc) **V**defl L/d SPACING-2-0-0 LOADING (psf) 244/190 Vert(LL) 0.00 n/r 120 MT20 TC 0.14 Plate Grip DOL 1.25 TCLL 20.0 Vert(CT) 0.01 120 5 n/r BC 0.10 TCDL 7.0 Lumber DOL 1.25 Horz(CT) 0.00 n/a n/a WR 0.03 0.0 Rep Stress Incr YES BCLL FT = 20%Weight: 25 lb Code FBC2020/TPI2014 Matrix-P BCDL 10.0 BRACING-LUMBER-TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD**

2x4 SP No.3 **OTHERS**

(size) 2=6-1-12, 4=6-1-12, 6=6-1-12

Max Horz 2=-52(LC 10)

Max Uplift 2=-48(LC 12), 4=-55(LC 13), 6=-15(LC 12) Max Grav 2=150(LC 1), 4=150(LC 1), 6=207(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

REACTIONS.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-3-5 to 3-3-5, Interior(1) 3-3-5 to 3-10-0, Exterior(2R) 3-10-0 to 6-10-14, Interior(1) 6-10-14 to 7-4-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Rogau PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEX REPERENCE PAGE MIL-74/3 fev. 5/19/2/20 BEFORE USE.

Design valid for use only with MITEX® connectors. This design is based only upon parameters shown, and is for on individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

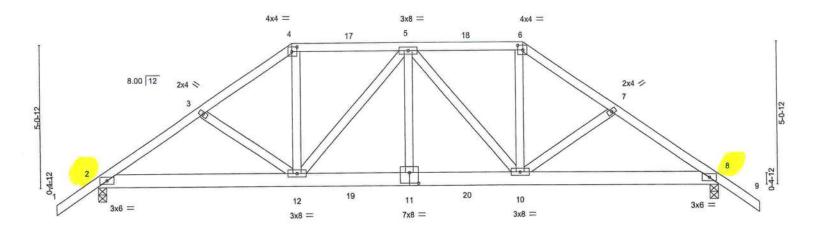
ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Rigid ceiling directly applied or 10-0-0 oc bracing.

CANNON RES. Truss Truss Type Job T30766107 T01 Hip Girder 3499744 Job Reference (optional) Run: 8,530 s Jan 6 2022 Print: 8,530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:51 2023 Page ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-asPpSshWTdAq?aPoO8wHCJBSJKhSGLVWWTAn6qz83DM Builders FirstSource, Lake City, FL 32055 23-10-0 22-4-0 18-6-5 11-2-0 15-4-0 4-2-0 4-2-0

Scale = 1:41.0



-	7-0-0 7-0-0		+	4-2-0 4-2-0		4-2-0		+		7-0-0	
Plate Offsets (X,Y)		-2-0], [11:0-4-0	,0-4-8]	760							
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/T	2-0-0 1.25 1.25 NO PI2014	CSI. TC BC WB Matrix	0.25 0.55 0.45 -MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.09 -0.16 0.05	(loc) 11 11 8	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 143 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x6 SP No.2 **BOT CHORD** 2x4 SP No.3 WEBS

REACTIONS.

(size) 2=0-3-8, 8=0-3-8

Max Horz 2=-125(LC 25) Max Uplift 2=-629(LC 8), 8=-643(LC 9) Max Grav 2=1722(LC 1), 8=1749(LC 1)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

2-3=-2696/1024, 3-4=-2545/997, 4-5=-2107/872, 5-6=-2146/892, 6-7=-2594/1023, TOP CHORD

7-8=-2745/1051

2-12=-851/2206, 11-12=-934/2481, 10-11=-934/2481, 8-10=-793/2246 **BOT CHORD**

4-12=-435/1176, 5-12=-626/322, 5-11=-209/532, 5-10=-555/261, 6-10=-392/1130 WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=629, 8=643.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 65 lb down and 50 lb up at 7-0-0, 65 lb down and 47 lb up at 9-0-12, 65 lb down and 41 lb up at 11-0-12, 65 lb down and 41 lb up at 11-3-4, and 65 lb down and 47 lb up at 13-3-4, and 172 lb down and 155 lb up at 15-4-0 on top chord, and 427 lb down and 220 lb up at 7-0-0, 157 lb down and 79 lb up at 9-0-12, 157 lb down and 79 lb up at 11-0-12, 157 lb down and 79 lb up at 11-3-4, and 157 lb down and 79 lb up at 13-3-4, and 427 lb down and 220 lb up at 15-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 6-9=-54, 2-8=-20

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regau PE No. 38126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

rameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guldance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Structural wood sheathing directly applied or 3-5-7 oc purlins.

Rigid ceiling directly applied or 7-7-7 oc bracing.

Truss Type Qty Job Truss Ply CANNON RES. T30766107 3499744 T01 Hip Girder

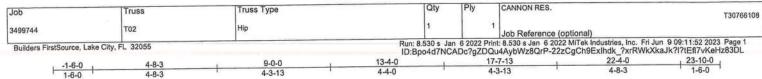
Builders FirstSource, Lake City, FL 32055

| Job Reference (optional)
| Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Frl Jun 9 09:11:51 2023 Page 2
| ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-asPpSshWTdAq?aPo08wHCJBSJKhSGLVWWTAn6qz83DM

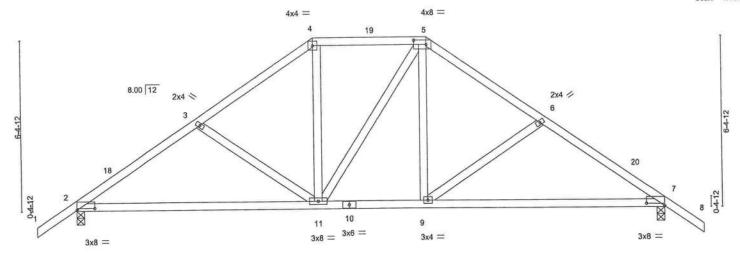
LOAD CASE(S) Standard

Concentrated Loads (lb)
Vert: 4=-17(F) 6=-90(F) 12=-427(F) 11=-315(F) 5=-33(F) 10=-427(F) 17=-17(F) 18=-17(F) 19=-157(F) 20=-157(F)





Scale = 1:43.3



H	9.	-0-0 -0-0	1010	4-4-0	-1			9-0-0		-
Plate Offsets (X,Y) LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/TF	2-0-0 1.25 1.25 YES	CSI. TC 0.43 BC 0.67 WB 0.16 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.16 -0.32 0.03	(loc) 9-17 9-17 7	l/defl >999 >826 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 121 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

13.4.0

22-4-0

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 5-2-10 oc purlins.

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 2x4 SP No.3 WEBS

(size) 2=0-3-8, 7=0-3-8 REACTIONS.

Max Horz 2=-155(LC 10) Max Uplift 2=-202(LC 12), 7=-202(LC 13)

Max Grav 2=907(LC 1), 7=907(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1170/258, 3-4=-961/215, 4-5=-742/217, 5-6=-961/215, 6-7=-1170/258 TOP CHORD

2-11=-226/947, 9-11=-52/742, 7-9=-121/947 **BOT CHORD**

3-11=-279/171, 4-11=-52/327, 5-9=-59/327, 6-9=-280/171 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 9-0-0, Exterior(2E) 9-0-0 to 13-4-0, Exterior(2R) 13-4-0 to 17-9-6, Interior(1) 17-9-6 to 23-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=202, 7=202.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.88126 MiTek Inc. DBA MiTek USA FL Cert 6634 16623 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

🚵 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MIL-7473 rev. S192/2020 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters and properly incorporate this design into the overall a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



CANNON RES. Truss Type Truss Job T30766109 FINK T03 3499744 Job Reference (optional) Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MTek Industries, Inc. Fri Jun 9 09:11:52 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-22zCgCh9ExIhdk_2xrRWkXkbdkxo?nVfl7vKeHz83DL Builders FirstSource, Lake City, FL 32055 16-1-5 22-4-0 23-10-0 6-2-11 Scale: 1/4"=1" 4x6 || 8.00 12 2x4 || 17 2x4 11 16 3 9 19 20 8 10 3x6 =4x6 = 3x6 = 4x4 = 4x4 = 22-4-0 16-1-5 6-2-11 9-10-10 6-2-11 GRIP DEFL. l/defl L/d **PLATES** in (loc) SPACING-2-0-0 CSI LOADING (psf) MT20 244/190 -0.21 8-10 >999 240 Vert(LL) TC 0.34 Plate Grip DOL 1.25 TCLL 20.0

-0.40

0.03

8-10

6

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

>666

n/a

180

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 4-0-2 oc purlins.

LUMBER-

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 2x6 SP No.2 BOT CHORD

7.0

0.0

10.0

2x4 SP No.3 WEBS

> (size) 2=0-3-8, 6=0-3-8 Max Horz 2=185(LC 11)

Max Uplift 2=-277(LC 12), 6=-277(LC 13) Max Grav 2=1322(LC 19), 6=1322(LC 20)

Lumber DOL

Rep Stress Incr

Code FBC2020/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

REACTIONS.

2-3=-2033/387, 3-4=-2057/537, 4-5=-2057/537, 5-6=-2033/387

1.25

NO

BOT CHORD

2-10=-330/1744, 8-10=-133/1049, 6-8=-235/1640

WEBS

4-8=-354/1249, 5-8=-292/231, 4-10=-354/1249, 3-10=-292/231

NOTES-

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 11-2-0, Exterior(2R) 11-2-0 to 15-4-15, Interior(1) 15-4-15 to 23-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

BC

WB

Matrix-MS

0.92

0.53

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Vert: 1-4=-54, 4-7=-54, 2-10=-20, 8-10=-80(F=-60), 6-8=-20

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FT = 20%

Weight: 135 lb

Philip J. O'Rogan PE. No.58126 Mil'ek Inc. DBA Mil'ek USA FL. Cert 6534 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

eters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



CANNON RES Qty Truss Type Truss Job T30766110 5 T04 Common Job Reference (optional)

Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:52 2023 Page 1
ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-22zCgCh9Exlhdk_?xrRWkXkbdkxd?n7fl7vKeHz83DL 3499744 Builders FirstSource, Lake City, FL 32055 22-4-0 6-2-11 Scale = 1:47.2 4x6 || 8.00 12 2x4 || 2x4 || 17 8 16 7 9 3x6 = 4x6 = 3x6 =4x4 = 4x4 = 16-1-5 6-2-11 9-10-11 6-2-11 Plate Offsets (X,Y)-- [2:0-3-9,0-1-8], [6:0-3-9,0-1-8] GRIP **PLATES** L/d CSI. DEFL. in (loc) **Vdefl** SPACING-2-0-0 LOADING (psf) 244/190 >999 240 MT20 Vert(LL) -0.21 7-9 Plate Grip DOL 1.25 TC 0.34

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

-0.40

0.03

7-9

6

>665

n/a

180

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 3-11-5 oc purlins.

LUMBER-

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x6 SP No.2

20.0

7.0

0.0

10.0

2x4 SP No.3 WEBS

(size) 6=0-3-8, 2=0-3-8

Max Horz 2=179(LC 9)

Max Uplift 6=-243(LC 13), 2=-277(LC 12) Max Grav 6=1245(LC 20), 2=1324(LC 19)

Lumber DOL

Rep Stress Incr

Code FBC2020/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-2035/388, 3-4=-2071/542, 4-5=-2085/552, 5-6=-2047/396 2-9=-346/1736, 7-9=-147/1034, 6-7=-254/1637

BOT CHORD

WEBS

REACTIONS.

4-7=-370/1285, 5-7=-314/238, 4-9=-359/1268, 3-9=-311/236

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 11-2-0, Exterior(2R) 11-2-0 to 14-2-0, Interior(1) 14-2-0 to 22-4-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

0.93

0.55

BC

WB

Matrix-MS

- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1 25

NO

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=243, 2=277
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 2-9=-20, 7-9=-80(F=-60), 6-7=-20

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

FT = 20%

Weight: 132 lb

Philip J. O'Regan PE No.58126 MiTek Int. DBA MITek USA FL Cert 5634 16025 Swingley Ridge Rd. Chestorfield, MO 63017 Date:

June 9,2023

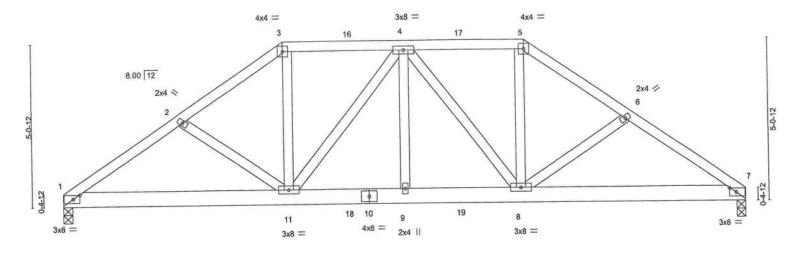
neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MIL-7473 rev. 5/19/2020 BEFORE USE
Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not
a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall
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fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component
Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



CANNON RES Truss Type Job Truss T30766111 Hip Girde T05 Job Reference (optional)

Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:52 2023 Page 1
ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-22zCgCh9ExIhdk_?xrRWkXkdSk2U?plff7vKeHz83DL 3499744 Builders FirstSource, Lake City, FL 32055 21-8-0 17-10-5 14-8-0 3-10-0 3-9-11

Scale = 1:36.0



	7-0-0		1	3-10-0	1	3-10-0				7-0-0	
Plate Offsets (X,Y)	[1:0-2-9,0-1-8], [7:0-2-9,0	-1-8]				_	_				
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/T	2-0-0 1.25 1.25 NO	CSI. TC BC WB Matrix	0.23 0.50 0.41	Vert(LL) Vert(CT) Horz(CT)	in 0.08 -0.14 0.05	(loc) 9 9 7	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 135 lb	GRIP 244/190 FT = 20%

10-10-0

LUMBER-

2x4 SP No.2 TOP CHORD 2x6 SP No.2 **BOT CHORD** 2x4 SP No.3 BRACING-

14-8-0

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-6-13 oc purlins. Rigid ceiling directly applied or 8-1-7 oc bracing.

21-8-0

REACTIONS.

TOP CHORD

(size) 1=0-3-8, 7=0-3-8 Max Horz 1=103(LC 5)

Max Uplift 1=-545(LC 8), 7=-558(LC 9) Max Grav 1=1530(LC 1), 7=1556(LC 1)

7-0-0

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 1-2=-2509/944, 2-3=-2364/915, 3-4=-1952/802, 4-5=-1988/820, 5-6=-2410/938,

6-7=-2556/968

1-11=-801/2062, 9-11=-823/2219, 8-9=-823/2219, 7-8=-751/2101 **BOT CHORD**

3-11=-392/1085, 4-11=-490/262, 4-9=-157/377, 4-8=-419/201, 5-8=-349/1038 WEBS

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=545, 7=558.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 65 lb down and 50 lb up at 7-0-0, 65 lb down and 47 lb up at 9-0-12, 65 lb down and 40 lb up at 10-10-0, and 65 lb down and 47 lb up at 12-7-4, and 172 lb down and 155 lb up at 14-8-0 on top chord, and 427 lb down and 220 lb up at 7-0-0, 157 lb down and 79 lb up at 9-0-12, 157 lb down and 79 Ib up at 10-10-0, and 157 lb down and 79 lb up at 12-7-4, and 427 lb down and 220 lb up at 14-7-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 5-7=-54, 1-7=-20

Concentrated Loads (lb)

Vert: 3=-17(F) 5=-90(F) 11=-427(F) 9=-157(F) 4=-17(F) 8=-427(F) 16=-17(F) 17=-17(F) 18=-157(F) 19=-157(F)

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

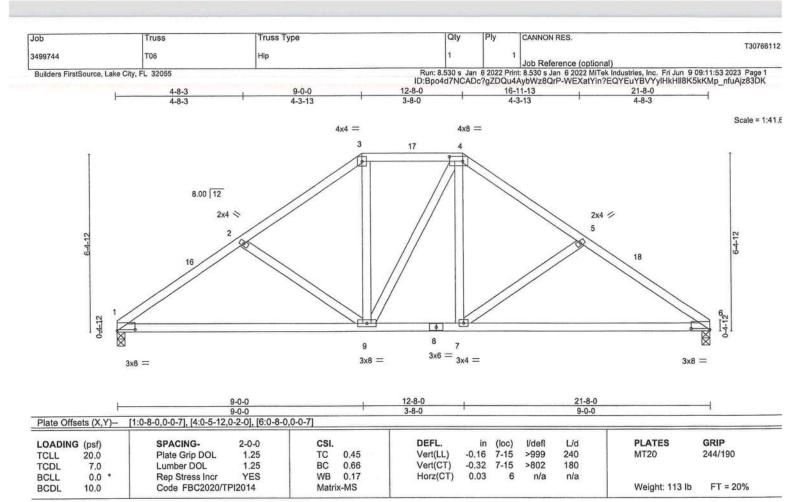
MARNING - Verify dasign parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 5/19/2020 BEFORE USE.

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ANSITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS

2x4 SP No.3

(size) 1=0-3-8, 6=0-3-8

Max Horz 1=133(LC 9) Max Uplift 1=-164(LC 12), 6=-164(LC 13)

Max Grav 1=802(LC 1), 6=802(LC 1) FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1146/257, 2-3=-932/211, 3-4=-715/213, 4-5=-932/212, 5-6=-1146/257

1-9=-243/932, 7-9=-66/715, 6-7=-153/932 **BOT CHORD** 2-9=-291/179, 3-9=-58/317, 4-7=-69/317, 5-7=-292/179 WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 9-0-0, Exterior(2E) 9-0-0 to 12-8-0, Exterior(2R) 12-8-0 to 17-1-6, Interior(1) 17-1-6 to 21-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=164, 6=164.

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Philly J. O'Regan PE No.58126 MITek Iuc. DBA MITek USA FL Cort 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

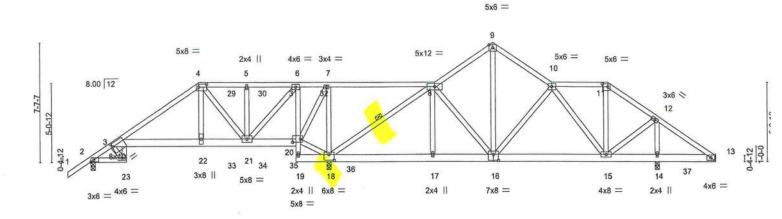


Structural wood sheathing directly applied or 5-1-14 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

CANNON RES Job Truss Truss Type Qtv T30766113 ROOF SPECIAL GIRDER T07 3499744 | ' | Job Reference (optional)
Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:53 2023 Page 1
ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-WEXatYin?EQYEuYBVYylHkHdH8l3kCjp_nfuAjz83DK Builders FirstSource, Lake City, FL 32055 36-10-4 33-8-0 3-10-0 3-10-0

Scale = 1:73.9



	2-3-8		10-2-0 3-2-0	13-4-0 15-5-1 3-2-0 2-1-12			26-2-0 3-10-0	-		33-8-0 7-6-0		9-12
Plate Offse		[3:0-0-12,0-2-8], [4:0-6-4	THE RESERVE AND PARTY AND PERSONS ASSESSMENT OF THE PARTY AND PARTY.			the second second second	and the second second second	,0-2-8]			021	V 112
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.93	Vert(LL)	0.15	23	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.79	Vert(CT)	-0.25	23	>742	180	W115-H5175	
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.66	Horz(CT)	0.14	18	n/a	n/a		
BCDL	10.0	Code FBC2020/	TPI2014	Mati	ix-MS						Weight: 281 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

1-4: 2x4 SP 2850F 2.0E or 2x4 SP M 31

2x6 SP No.2 *Except* **BOT CHORD**

2-23: 2x4 SP No.2, 3-23,6-19: 2x4 SP No.3

WEBS 2x4 SP No.3

REACTIONS. All bearings 0-3-8 except (jt=length) 18=0-3-10 (input: 0-3-8), 13=Mechanical.

Max Horz 2=174(LC 5)

Max Uplift All uplift 100 lb or less at joint(s) 13 except 2=-176(LC 9), 18=-1086(LC 8), 14=-381(LC 28) All reactions 250 lb or less at joint(s) 13 except 2=572(LC 19), 18=3056(LC 1), 14=839(LC 16)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown

TOP CHORD 3-27=-357/221, 3-4=-636/425, 4-5=-220/459, 5-6=-220/459, 6-7=-518/1397,

7-8=-646/1700, 8-9=-490/497, 9-10=-462/476, 10-11=-260/234, 11-12=-347/247

3-22=-327/517, 21-22=-335/547, 20-21=-1375/564, 6-20=-1311/557, 17-18=-590/403, **BOT CHORD** 16-17=-591/400, 15-16=-265/406

4-22=-326/862, 4-21=-1101/457, 6-21=-701/1728, 18-20=-1791/752, 7-20=-347/644, WEBS

7-18=-951/434, 8-18=-1514/413, 8-16=-306/755, 9-16=-459/363, 10-16=-255/153,

10-15=-309/322, 12-15=-286/497, 12-14=-774/463

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) WARNING: Required bearing size at joint(s) 18 greater than input bearing size.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13 except (jt=lb) 2=176, 18=1086, 14=381.
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 64 lb down and 48 lb up at 7-0-0, 64 lb down and 46 lb up at 9-0-12, 64 lb down and 46 lb up at 11-0-12, and 64 lb down and 46 lb up at 13-0-12, and 65 lb down and 47 lb up at 15-0-12 on top chord, and 507 lb down and 269 lb up at 7-0-0, 159 lb down and 80 lb up at 9-0-12, 159 lb down and 80 lb up at 11-0-12, 159 lb down and 80 lb up at 13-0-12, and 157 lb down and 79 lb up at 15-0-12, and 225 lb down and 118 lb up at 38-8-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

This item has been electronically signed and sealed by ORegan, Philip, Pl using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Rogen PE No.58126 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

eters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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*ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing, Except:

10-0-0 oc bracing: 3-23,3-22,21-22.

1 Row at midpt

Job	Truss	Truss Type	Qty	Ply	CANNON RES.	T30766113
3499744	Т07	ROOF SPECIAL GIRDER	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:53 2023 Page 2 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-WEXatYin?EQYEuYBVYyIHkHdH8l3kCjp_nfuAjz83DK

LOAD CASE(S) Standard

Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 4-8=-54, 8-9=-54, 9-10=-54, 10-11=-54, 11-13=-54, 23-26=-20, 3-20=-20, 13-19=-20

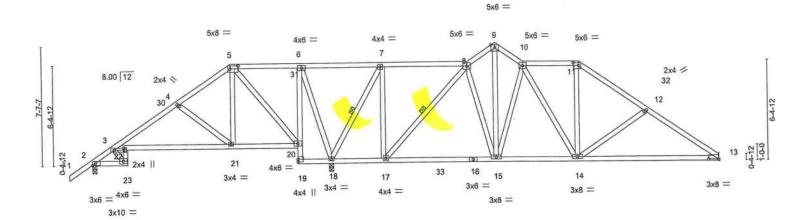
Concentrated Loads (lb)

Vert: 4=-15(F) 22=-507(F) 29=-15(F) 30=-15(F) 31=-15(F) 32=-17(F) 33=-159(F) 34=-159(F) 35=-159(F) 36=-157(F) 37=-225(F)



Job	Truss		Truss Type		Qty	Ply	CANNON RES.			T30766114
3499744	Т08		ROOF SPECIAL	L	1		Job Reference (o	ptional)		1 10 20 70 70 70 70 70 70 70 70 70 70 70 70 70
Builders FirstSource, Lake	City, FL 32055				Run: 8.530 s ID:Bpo4d7N	Jan 6 2022 Pri CADc?gZDQu	nt: 8.530 s Jan 6 2022 4AybWz8QrPR5y	MiTek Industries, Inc 5tjPmYYPs27N3G	Fri Jun 9 09:11:54 2 pypsfYciTfXyDRC	023 Page 1 0Rj9z83DJ
-1-6-0, 2-3-	8 . 5-7-12	9-0-0	13-4-0	18-10-0	24-4-0	26-2-0,28		35-11-13	40-8-0	
1-6-0 2-3-		3-4-4	4-4-0	5-6-0	5-6-0	1-10-0 1-	10-0 3-8-0	4-3-13	4-8-3	

Scale = 1:73.9



2-3 2-3 Plate Offsets (X,Y)		13-4-0 4-4-0 -2-4], [11:0-4-4,0	15-5-12 2-1-12 -2-4], [13:0-8	18-10-0 3-4-4 3-0,0-0-7]	26-2-0 7-4-0		31-8- 5-6-		40-8-0 9-0-0	
LOADING (psf) TCLL 20.0 TCDL 7.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25	CSI. TC	0.69 0.96	DEFL. Vert(LL) Vert(CT)	in (loc) -0.22 21-22 -0.40 21-22	Vdefl >862 >459	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr Code FBC2020/TI	YES PI2014	WB Matrix-	0.69 MS	Horz(CT)	0.07 13	n/a	n/a	Weight: 261 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No 2 TOP CHORD

2x4 SP No.2 *Except* **BOT CHORD**

22-23,6-19: 2x4 SP No.3

2x4 SP No.3 WEBS

(size) 13=Mechanical, 2=0-3-8, 18=0-3-8 REACTIONS.

Max Horz 2=174(LC 9)

Max Uplift 13=-226(LC 13), 2=-111(LC 12), 18=-453(LC 12) Max Grav 13=875(LC 20), 2=423(LC 23), 18=2089(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

3-28=-399/132, 3-4=-310/137, 5-6=-72/590, 6-7=-104/782, 8-9=-710/328, 9-10=-686/317, TOP CHORD 10-11=-768/305, 11-12=-969/320, 12-13=-1180/365

2-23=-137/307, 22-23=-134/384, 21-22=-120/279, 6-20=-80/569, 18-19=-485/122, **BOT CHORD**

15-17=-102/487, 14-15=-103/692, 13-14=-242/965

4-21=-346/177, 5-21=-80/489, 5-20=-826/174, 6-18=-799/227, 7-18=-1280/230,

7-17=-67/871, 8-17=-875/152, 8-15=-6/260, 9-15=-306/647, 10-15=-555/233,

11-14=-67/356, 12-14=-315/177, 3-23=-418/174

NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-6-13, Interior(1) 2-6-13 to 9-0-0, Exterior(2R) 9-0-0 to 13-0-13, Interior(1) 13-0-13 to 26-2-0, Exterior(2E) 26-2-0 to 28-0-0, Interior(1) 28-0-0 to 31-8-0, Exterior(2R) 31-8-0 to 35-8-13, Interior(1) 35-8-13 to 40-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 13=226, 2=111, 18=453.

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Structural wood sheathing directly applied or 4-10-1 oc purlins.

7-18, 8-17

Rigid ceiling directly applied or 4-6-8 oc bracing.

1 Row at midpt

Philip J. O'Regan PE No.58126 MiTek Iur. DBA MITek USA FL Cort 6634 16025 Swingley Ridge Rd. Chexterfield, MO 63017

June 9,2023

eters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 5/19/2020 BEPURE USE.

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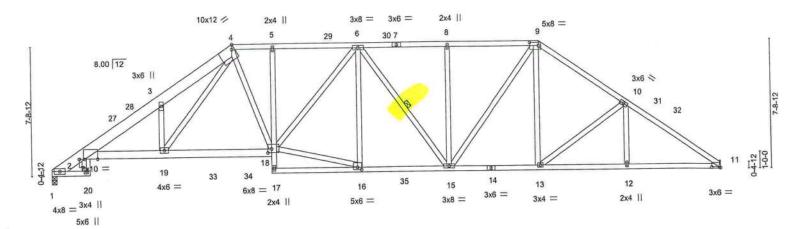
ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



CANNON RES Qty Truss Type Truss T30766115 Job Hip T09 Job Reference (optional)

Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:54 2023 Page 1
ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-_R5y5ijPmYYPs27N3GT_pypu4YgJTc2yDRORj9z83DJ Builders FirstSource, Lake City, FL 32055 40-8-0 29-8-0 34-11-13 24-1-2 18-8-0 5-4-0 5-3-13

Scale = 1:69.3



	6-7-12 13-4-0 4-4-4 6-8-4 [2:0-9-2,0-0-0], [2:0-0-0,0-3-15	18-8-0 5-4-0 i], [4:0-1-15,Edge], [9:0-6-4,	24-1-2 5-5-2 ,0-2-4], [11:0-6-0,0-0-7], [1	29-8-0 5-6-14 [6:0-3-0,0-1-12],	[18:0-2-1	34-11-13 5-3-13 12,0-2-12]	40-8-0 5-8-3	
LOADING (psf) TCLL 20.0 TCDL 7.0	SPACING- 2- Plate Grip DOL 1 Lumber DOL 1	0-0 CSI. .25 TC 0.5 .25 BC 0.6	DEFL. 54 Vert(LL) 56 Vert(CT)	in (loc) -0.22 15-16 -0.37 15-16	l/defl >999 >999	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr Y Code FBC2020/TPI20	TES WB 0.8 14 Matrix-MS		0.24 11	n/a	n/a	Weight: 287 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

1-4: 2x8 SP 2400F 2.0E

2x4 SP No.2 *Except* **BOT CHORD**

2-20: 2x6 SP No.2, 2-18: 2x6 SP M 26, 5-17: 2x4 SP No.3

2x4 SP No.3 WEBS

REACTIONS.

(size) 1=0-3-8, 11=Mechanical

Max Horz 1=164(LC 9)

Max Uplift 1=-317(LC 12), 11=-321(LC 13) Max Grav 1=1672(LC 2), 11=1658(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-23=-993/215, 2-3=-3402/650, 3-4=-3712/860, 4-5=-2361/517, 5-6=-2357/518,
6-8=-2241/480, 8-9=-2241/480, 9-10=-2268/478, 10-11=-2648/513

2-19=-595/2961, 18-19=-396/2078, 15-16=-406/2299, 13-15=-217/1833, 12-13=-353/2154, **BOT CHORD**

11-12=-353/2154

3-19=-824/342, 4-19=-436/1611, 4-18=-226/831, 16-18=-387/2219, 6-16=-295/114, WEBS

8-15=-319/158, 9-15=-237/740, 9-13=-82/509, 10-13=-492/202

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 4-2-9, Interior(1) 4-2-9 to 11-0-0, Exterior(2R) 11-0-0 to 16-9-0, Interior(1) 16-9-0 to 29-8-0, Exterior(2R) 29-8-0 to 35-5-0, Interior(1) 35-5-0 to 40-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=317, 11=321.

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Structural wood sheathing directly applied or 3-4-12 oc purlins.

Rigid ceiling directly applied or 9-3-9 oc bracing.

1 Row at midpt

Philip J. O'Regan PE No.58126 MiTek lur. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

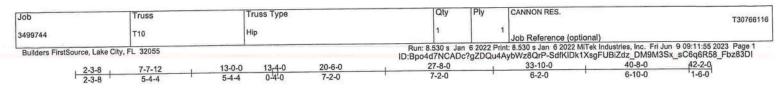
June 9,2023

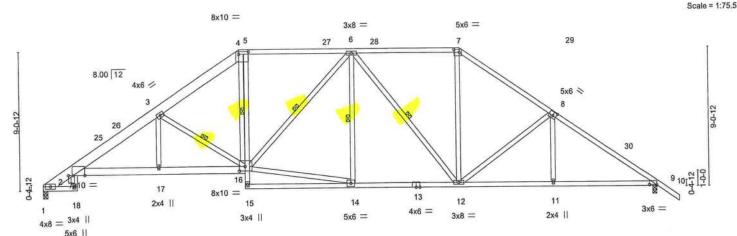
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 5/1972/020 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







2-3-8	7-7-12 5-4-4	13-4-0 5-8-4	20-6-0 7-2-0	27-8-0		6-	10-0	40-8-0 6-10-0	H
Plate Offsets (X,Y)	[2:0-0-0,0-3-15], [2:0-9-2,	0-0-0], [4:0-7-12	,0-2-0], [7:0-4-4,0-2	-4], [8:0-2-12,0-3-0], [9:	0-6-0,0-0-3]	[16:0-4-8,	0-3-8]		
LOADING (psf) TCLL 20.0 TCDL 7.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25	CSI. TC 0.56 BC 0.77	DEFL. Vert(LL) Vert(CT)	in (loc -0.23 12-1 -0.39 12-1	4 >999	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr Code FBC2020/T	YES PI2014	WB 0.68 Matrix-MS	Horz(CT)	0.23	9 n/a	n/a	Weight: 290 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

1 Row at midpt

1 Row at midpt

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

1-4: 2x8 SP 2400F 2.0E 2x4 SP No.2 *Except* **BOT CHORD**

2-18: 2x6 SP No.2, 2-16: 2x6 SP M 26, 5-15: 2x4 SP No.3

2x4 SP No.3 WEBS

REACTIONS.

(size) 1=0-3-8, 9=0-3-8

Max Horz 1=-207(LC 10) Max Uplift 1=-314(LC 12), 9=-348(LC 13)

Max Grav 1=1641(LC 2), 9=1703(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-21=-979/254, 2-3=-3162/620, 3-4=-2348/504, 4-5=-1906/470, 5-6=-1907/472, TOP CHORD

6-7=-1693/427, 7-8=-2106/449, 8-9=-2556/487

2-17=-559/2736, 16-17=-559/2736, 12-14=-282/1957, 11-12=-287/2074, 9-11=-288/2069 **BOT CHORD** WEBS

3-17=-23/480, 3-16=-1168/345, 4-16=-217/972, 14-16=-269/1794, 6-12=-507/200,

7-12=-131/863, 8-12=-585/233, 8-11=0/265

NOTES-

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 4-2-9, Interior(1) 4-2-9 to 13-0-0, Exterior(2R) 13-0-0 to 18-9-0, Interior(1) 18-9-0 to 27-8-0, Exterior(2R) 27-8-0 to 33-5-0, Interior(1) 33-5-0 to 42-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=314, 9=348.

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Structural wood sheathing directly applied or 3-3-12 oc purlins.

3-16, 6-16, 6-14, 6-12

Rigid ceiling directly applied or 10-0-0 oc bracing. Except:

5-16

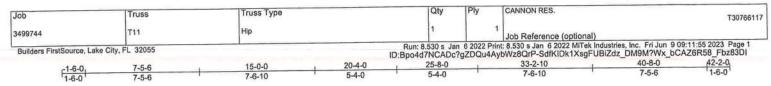
Philip J. O'Regan PE No.58116 MITek Inc. DBA MITek USA FL Cert 6634 16025 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

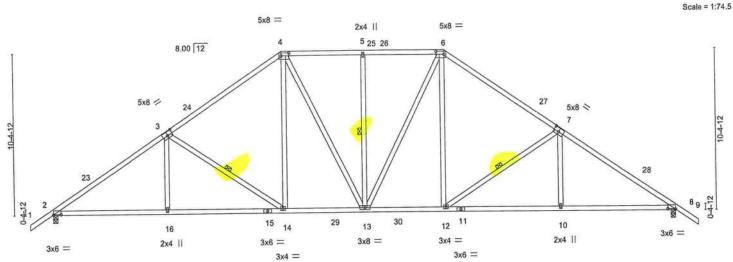
June 9,2023

elers and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







F	7-5-6 7-5-6	15-0-0 7-6-10	20-4-0 5-4-0	25-8	-0		2-10 3-10	40-8-0 7-5-6	
Plate Offsets (X,Y)-	[2:0-6-0,0-0-4], [3:0-4-0,0)-3-0], [4:0-6-4,0-2-	4], [6:0-6-4,0-2-4], [7:0-4-0	,0-3-0], [8:0-6	-0,0-0-3]				
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/1	2-0-0 1.25 1.25 YES	CSI. TC 0.81 BC 0.79 WB 0.44 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.16 10-12 -0.31 10-12 0.12 8	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 253 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied.

1 Row at midpt

Rigid ceiling directly applied or 8-8-9 oc bracing.

3-14, 5-13, 7-12

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

2x4 SP No.3 WEBS

> 2=0-3-8, 8=0-3-8 (size) Max Horz 2=243(LC 11)

Max Uplift 2=-345(LC 12), 8=-345(LC 13) Max Grav 2=1719(LC 2), 8=1719(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-2574/479, 3-4=-2014/425, 4-5=-1706/394, 5-6=-1706/394, 6-7=-2014/425, TOP CHORD

7-8=-2574/479

2-16=-438/2164, 14-16=-438/2163, 13-14=-207/1595, 12-13=-125/1595, 10-12=-274/2080, **BOT CHORD**

8-10=-274/2080

3-16=0/321, 3-14=-702/279, 4-14=-108/650, 4-13=-181/354, 5-13=-312/162, **WEBS**

6-13=-181/354, 6-12=-108/650, 7-12=-702/279, 7-10=0/321

NOTES-

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-6-13, Interior(1) 2-6-13 to 15-0-0, Exterior(2R) 15-0-0 to 20-9-0, Interior(1) 20-9-0 to 25-8-0, Exterior(2R) 25-8-0 to 31-5-0, Interior(1) 31-5-0 to 42-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=345, 8=345,

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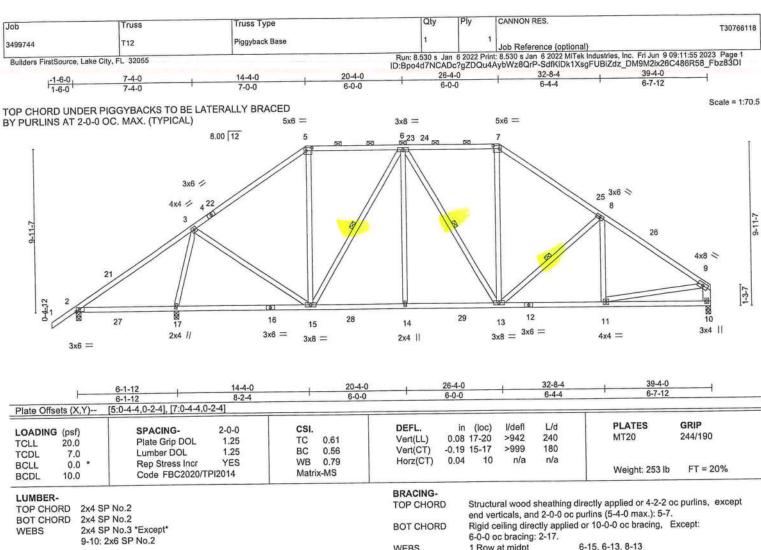
Philip J. O'Regau PE No.38126 MITek Iar. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and propry damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





WEBS

1 Row at midpt

REACTIONS.

(size) 2=0-3-8, 17=0-3-8, 10=0-3-8

Max Horz 2=223(LC 9) Max Uplift 2=-46(LC 12), 17=-347(LC 12), 10=-266(LC 13) Max Grav 2=214(LC 23), 17=1805(LC 2), 10=1315(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD

2-3=-86/370, 3-5=-1097/256, 5-6=-834/270, 6-7=-1095/350, 7-8=-1397/357,

8-9=-1654/346, 9-10=-1210/282

BOT CHORD WEBS

2-17=-268/131, 15-17=-154/260, 14-15=-174/1133, 13-14=-174/1133, 11-13=-209/1317 3-17=-1541/380, 3-15=-93/809, 5-15=-73/341, 6-15=-623/177, 6-14=0/321, 7-13=-78/487,

8-13=-399/200, 9-11=-156/1187

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-5-3, Interior(1) 2-5-3 to 14-4-0, Exterior(2R) 14-4-0 to 19-10-12, Interior(1) 19-10-12 to 26-4-0, Exterior(2R) 26-4-0 to 31-10-12, Interior(1) 31-10-12 to 39-1-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 17=347, 10=266.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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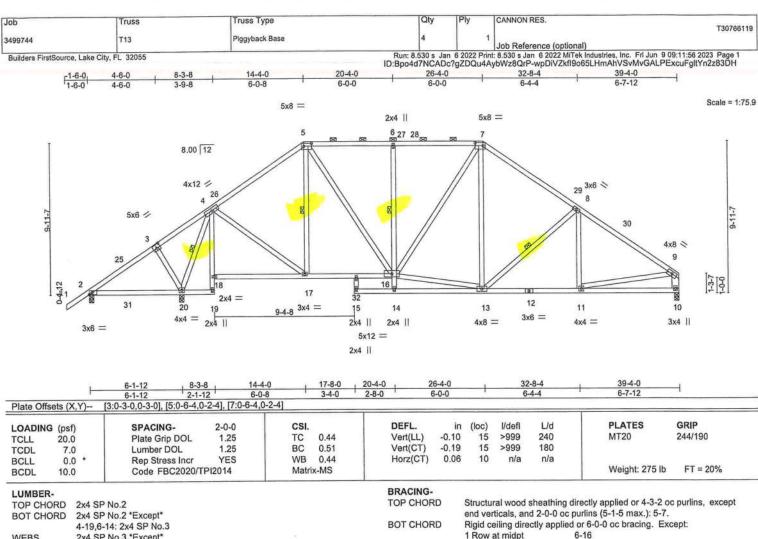
Philip J. O'Regau PE No.58126 MITek Ier. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

elers and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 (ev. 519/2020 BEFORE USE.)

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS//TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





WEBS

10-0-0 oc bracing: 14-16

1 Row at midpt

4-20, 5-17, 8-13

2x4 SP No.3 *Except* WEBS

9-10: 2x6 SP No.2

REACTIONS.

(size) 2=0-3-8, 20=0-3-8, 10=0-3-8

Max Horz 2=223(LC 9)

Max Uplift 2=-189(LC 26), 20=-418(LC 12), 10=-269(LC 13) Max Grav 2=65(LC 9), 20=2118(LC 2), 10=1276(LC 2)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown

TOP CHORD

2-3=-221/749, 3-4=-199/825, 4-5=-1074/268, 5-6=-1194/327, 6-7=-1191/328, 7-8=-1335/363, 8-9=-1598/349, 9-10=-1170/285

2-20=-485/38, 16-17=-120/820, 6-16=-367/184, 11-13=-212/1271 **BOT CHORD** 4-20=-1845/348, 4-17=-81/901, 5-17=-302/86, 5-16=-153/689, 13-16=-52/1060,

WEBS 7-16=-138/343, 7-13=-84/314, 8-13=-405/200, 9-11=-159/1142

NOTES-

- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-5-3, Interior(1) 2-5-3 to 14-4-0, Exterior(2R) 14-4-0 to 19-10-12, Interior(1) 19-10-12 to 26-4-0, Exterior(2R) 26-4-0 to 31-10-12, Interior(1) 31-10-12 to 39-1-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=189, 20=418, 10=269.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

This item has been electronically signed and sealed by ORegan, Philip, Pl using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regau PE No.58126 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

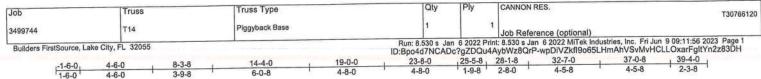
June 9,2023

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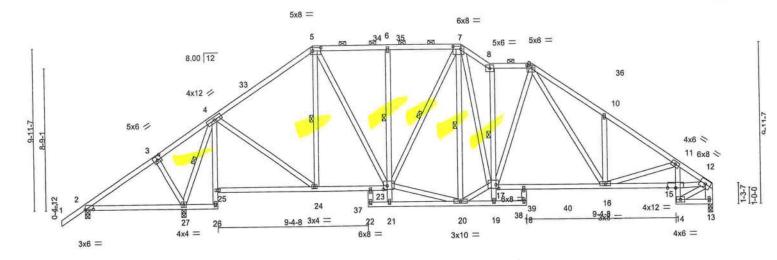
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Scale = 1:71.3



	-	6-1-12 8-3 6-1-12 2-1-	12 6-0	-8	17-8-0 3-4-0	19-0-0	23-8-0 4-8-0	1-		2-2-8	32-7-0 4-11-0	4-5-8	2-3-8
Plate Offsets (X	Y)		,0-3-0], [5:0-6-4,	0-2-4], [7:0-5-12	2,0-2-0],	[9:0-3-12,0-2	2-0], [17:0)-2-0,0-2	2-12], [23:0-2-1	2,0-2-12]	DANCES ASSOCIA	SAMPLES OF
LOADING (psf) TCLL 20.0 TCDL 7.0		SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25	BC ().37).75	Ve Ve	ert(CT)	in -0.20 1 -0.34 1		l/defl >999 >999	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL 0.0 BCDL 10.0	٠	Rep Stress Incr Code FBC2020	YES TPI2014	WB (Matrix-l).57 MS	Ho	orz(CT)	0.12	13	n/a	n/a	Weight: 316 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

1 Row at midpt

1 Row at midpt

10-0-0 oc bracing: 21-23

LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* **BOT CHORD**

4-26,6-21,11-14: 2x4 SP No.3

2x4 SP No.3 *Except* WEBS

12-13: 2x6 SP No.2

(size) 2=0-3-8, 13=0-3-8, 27=0-3-8 REACTIONS.

Max Horz 2=223(LC 9)

Max Uplift 2=-251(LC 26), 13=-240(LC 13), 27=-345(LC 12) Max Grav 2=110(LC 9), 13=1320(LC 2), 27=2213(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-229/849, 3-4=-206/924, 4-5=-1039/266, 5-6=-1113/312, 6-7=-1111/313, TOP CHORD

7-8=-1700/419, 8-9=-1392/329, 9-10=-1997/493, 10-11=-1985/357, 11-12=-1980/369,

12-13=-1296/248

2-27=-576/58, 23-24=-118/790, 6-23=-280/145, 16-17=-121/1282, 15-16=-318/1715 BOT CHORD WEBS

4-27=-1916/292, 4-24=-72/954, 5-24=-336/77, 5-23=-138/717, 20-23=-87/1059,

7-20=-605/74, 17-20=-99/1152, 8-17=-950/263, 9-17=-90/401, 10-16=-290/206,

7-17=-266/1365, 12-15=-262/1499, 9-16=-233/735

NOTES-

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-5-3, Interior(1) 2-5-3 to 14-4-0, Exterior(2R) 14-4-0 to 18-3-3, Interior(1) 18-3-3 to 23-8-0, Exterior(2E) 23-8-0 to 25-5-8, Interior(1) 25-5-8 to 28-1-8, Exterior(2R) 28-1-8 to 32-0-12, Interior(1) 32-0-12 to 39-1-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

This item has been electronically signed and sealed by ORegan, Philip, P using a Digital Signature. Printed copies of this document are not considere signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regau PE No.58126 MiTek Inc. DBA MITek USA FL. Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 ray. 5/19/2020 BEFORE USE
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent collapse with possible personal injury and property general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component
Safety Information
available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



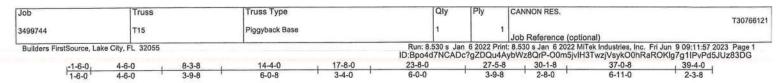
Structural wood sheathing directly applied or 4-0-7 oc purlins, except

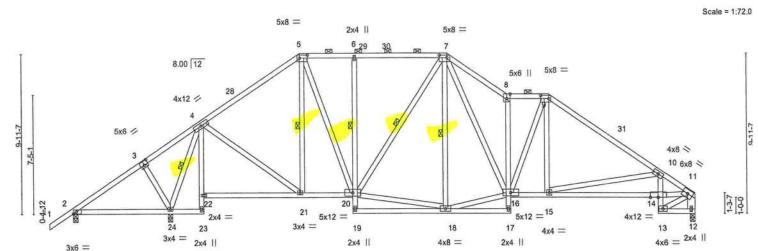
4-27, 5-24, 7-23, 7-20, 8-19

end verticals, and 2-0-0 oc purlins (4-10-5 max.): 5-7, 8-9.

Rigid ceiling directly applied or 6-0-0 oc bracing. Except:

6-23





	76	6-1-12	8-3-8	14-4-0		17-8-0	23-8-0	- 46	27-8-	0 ,	30-1-8	37-0-8	39-4-0
		6-1-12	2-1-12	6-0-8		3-4-0	6-0-0	3.	4-0-0	, ,	2-5-8	6-11-0	2-3-8
Plate Offs	ets (X,Y)-	- [3:0-3-0,0-3	3-0], [5:0-6-4,0-2	-4], [7:0-5-12,0-2	2-0], [8:0-3-	-8,Edge], [9	:0-6-4,0-2-4]						
LOADING	(psf)	SPA	CING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
CLL	20.0	Plat	e Grip DOL	1.25	TC	0.60	Vert(LL)	-0.10	14-15	>999	240	MT20	244/190
CDL	7.0	Lum	ber DOL	1.25	BC	0.79	Vert(CT)	-0.21	14-15	>999	180	Salvaniova:	
BCLL	0.0 *	Rep	Stress Incr	YES	WB	0.56	Horz(CT)	0.10	12	n/a	n/a		
BCDL	10.0	Cod	e FBC2020/TPI	2014	Matrix-	-MS	0.000.000.000					Weight: 305 lb	FT = 20%

LUMBER-

WEBS

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* **BOT CHORD**

4-23,6-19,8-17,10-13: 2x4 SP No.3

2x4 SP No.3 *Except*

11-12: 2x6 SP No.2

BRACING-

WEBS

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-7-7 oc purlins, except

end verticals, and 2-0-0 oc purlins (4-10-11 max.): 5-7, 8-9. Rigid ceiling directly applied or 6-0-0 oc bracing. Except:

1 Row at midpt 1 Row at midpt 6-20

4-24, 5-21, 7-20, 7-18

REACTIONS.

(size) 2=0-3-8, 12=0-3-8, 24=0-3-8

Max Horz 2=224(LC 9)

Max Uplift 2=-202(LC 24), 12=-249(LC 13), 24=-360(LC 12) Max Grav 2=65(LC 9), 12=1145(LC 1), 24=1911(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-221/647, 3-4=-199/732, 4-5=-918/266, 5-6=-887/309, 6-7=-889/311, 7-8=-1662/495,

8-9=-1346/361, 9-10=-1583/345, 10-11=-1768/398, 11-12=-1135/259

BOT CHORD 2-24=-466/49, 20-21=-119/672, 6-20=-293/152, 8-16=-1025/328, 15-16=-168/1225,

14-15=-414/1646

4-24=-1689/286, 4-21=-78/779, 5-21=-314/77, 5-20=-166/629, 18-20=-101/866,

7-18=-256/73, 16-18=-84/864, 7-16=-327/1044, 9-16=-101/310, 9-15=-18/351,

10-15=-488/279, 11-14=-280/1365

NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-5-3, Interior(1) 2-5-3 to 14-4-0, Exterior(2R) 14-4-0 to 18-3-3, Interior(1) 18-3-3 to 23-8-0, Exterior(2E) 23-8-0 to 27-5-8, Interior(1) 27-5-8 to 30-1-8, Exterior(2R) 30-1-8 to 34-0-12, Interior(1) 34-0-12 to 39-1-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=202, 12=249, 24=360.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

This item has been electronically signed and sealed by ORegan, Philip, Pl using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Phillp J. O'Regau PE No.58126 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

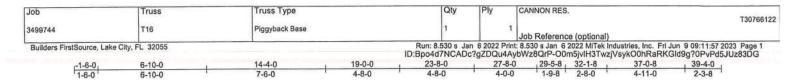
June 9,2023

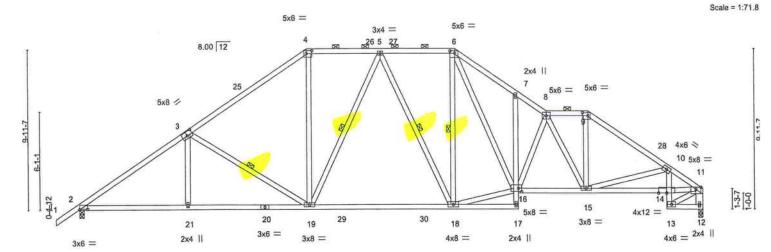
irs and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 (ev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







	1	6-10-0	14-4-0	1		23-8-0	1	27-8-	0 ,	32-1-8	37-0-8	39-4-0
	1	6-10-0	7-6-0			9-4-0		4-0-0) '	4-5-8	4-11-0	2-3-8
Plate Offse	ets (X,Y)	[2:0-6-0,0-0-4], [3:0-4-0,0	-3-0], [4:0-4-4,0	-2-4], [6:0-3	-8,0-1-12], [9	:0-4-4,0-2-4], [16:0	1-2-8,0-	2-8]			To the second second	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.25	TC	0.86	Vert(LL)	-0.34	18-19	>999	240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.98	Vert(CT)	-0.57	18-19	>823	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.70	Horz(CT)	0.18	12	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MS	65 50					Weight: 275 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* BOT CHORD

7-17,10-13: 2x4 SP No.3

2x4 SP No.3 *Except* WEBS

11-12: 2x6 SP No.2

(size) 2=0-3-8, 12=0-3-8

Max Horz 2=223(LC 9)

Max Uplift 2=-289(LC 12), 12=-267(LC 13) Max Grav 2=1652(LC 2), 12=1581(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2475/448, 3-4=-1955/423, 4-5=-1551/407, 5-6=-1588/416, 6-7=-2623/628,

TOP CHORD

7-8=-2568/537, 8-9=-1958/422, 9-10=-2382/455, 10-11=-2404/444, 11-12=-1558/291

2-21=-403/2076, 19-21=-403/2081, 18-19=-206/1617, 15-16=-374/2396, 14-15=-402/2107 BOT CHORD 3-21=0/282, 3-19=-656/274, 4-19=-101/774, 5-19=-315/184, 16-18=-162/1577, WEBS

6-16=-352/1354, 8-16=-654/185, 8-15=-943/183, 9-15=-160/1097, 10-15=-273/167,

11-14=-321/1845

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-5-3, Interior(1) 2-5-3 to 14-4-0, Exterior(2R) 14-4-0 to 18-3-3, Interior(1) 18-3-3 to 23-8-0, Exterior(2R) 23-8-0 to 27-7-4, Interior(1) 27-7-4 to 32-1-8, Exterior(2R) 32-1-8 to 36-0-12, Interior(1) 36-0-12 to 39-1-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=289, 12=267
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

This item has been electronically signed and sealed by ORegan, Philip, PI using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Structural wood sheathing directly applied, except end verticals, and

3-19, 5-19, 5-18, 6-18

2-0-0 oc purlins (4-2-0 max.): 4-6, 8-9.

1 Row at midpt

Rigid ceiling directly applied or 2-2-0 oc bracing.

Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

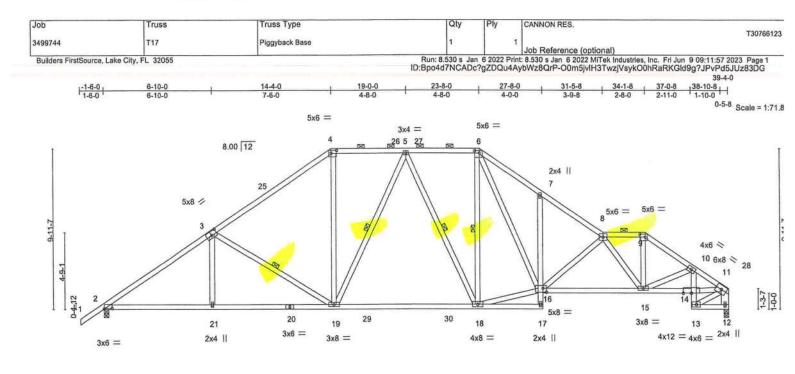
June 9.2023

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L	6-10-0	14-4-0		23-8-0	21-8-	0	34-1-8		39-4-0
	6-10-0	7-6-0		9-4-0	4-0-0) '	6-5-8	2-11-0	2-3-8
sets (X,Y)	[2:0-6-0,0-0-4], [3:0-4-0,0)-3-0], [4:0-4-4,0	-2-4], [6:0-3-8,0-1	12], [9:0-4-4,0-2-4], [16	:0-2-8,0-3-0]				
G (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
20.0	Plate Grip DOL	1.25	TC 0.86	Vert(LL)	-0.35 18-19	>999	240	MT20	244/190
7.0	Lumber DOL	1.25	BC 0.98	Vert(CT)	-0.58 18-19	>803	180		
0.0 *	Rep Stress Incr	YES	WB 0.68	Horz(CT)	0.18 12	n/a	n/a		
10.0	Code FBC2020/T	PI2014	Matrix-MS	27 0				Weight: 269 lb	FT = 20%
	3 (psf) 20.0 7.0 0.0 *	sets (X,Y) [2:0-6-0,0-0-4], [3:0-4-0,0 (psf) SPACING- 20.0 Plate Grip DOL 7.0 Lumber DOL 0.0 * Rep Stress Incr	6-10-0 7-6-0 sets (X,Y) [2:0-6-0,0-0-4], [3:0-4-0,0-3-0], [4:0-4-4,0 3 (psf) SPACING- 2-0-0 20.0 Plate Grip DOL 1.25 7.0 Lumber DOL 1.25 0.0 * Rep Stress Incr YES	6-10-0 7-6-0 sets (X,Y)- [2:0-6-0,0-0-4], [3:0-4-0,0-3-0], [4:0-4-4,0-2-4], [6:0-3-8,0-1- (psf) SPACING- 2-0-0 CSI. 20.0 Plate Grip DOL 1.25 TC 0.86 7.0 Lumber DOL 1.25 BC 0.98 0.0 * Rep Stress Incr YES WB 0.68	6-10-0 7-6-0 9-4-0 sets (X,Y) [2:0-6-0,0-0-4], [3:0-4-0,0-3-0], [4:0-4-4,0-2-4], [6:0-3-8,0-1-12], [9:0-4-4,0-2-4], [16 (psf) SPACING- 2-0-0 CSI. DEFL. 20.0 Plate Grip DOL 1.25 TC 0.86 Vert(LL) 7.0 Lumber DOL 1.25 BC 0.98 Vert(CT) 0.0 * Rep Stress Incr YES WB 0.68 Horz(CT)	6-10-0 7-6-0 9-4-0 4-0-0 sets (X,Y) [2:0-6-0,0-0-4], [3:0-4-0,0-3-0], [4:0-4-4,0-2-4], [6:0-3-8,0-1-12], [9:0-4-4,0-2-4], [16:0-2-8,0-3-0] (psf) SPACING- 2-0-0 CSI. DEFL. in (loc) 20.0 Plate Grip DOL 1.25 TC 0.86 Vert(LL) -0.35 18-19 7.0 Lumber DOL 1.25 BC 0.98 Vert(CT) -0.58 18-19 0.0 * Rep Stress Incr YES WB 0.68 Horz(CT) 0.18 12	6-10-0 7-6-0 9-4-0 4-0-0 sets (X,Y)- [2:0-6-0,0-0-4], [3:0-4-0,0-3-0], [4:0-4-4,0-2-4], [6:0-3-8,0-1-12], [9:0-4-4,0-2-4], [16:0-2-8,0-3-0] 3 (psf) SPACING- 2-0-0 CSI. DEFL. in (loc) l/defl 20.0 Plate Grip DOL 1.25 TC 0.86 Vert(LL) -0.35 18-19 >999 7.0 Lumber DOL 1.25 BC 0.98 Vert(CT) -0.58 18-19 >803 0.0 * Rep Stress Incr YES WB 0.68 Horz(CT) 0.18 12 n/a	6-10-0 7-6-0 9-4-0 4-0-0 6-5-8 sets (X,Y)- [2:0-6-0,0-0-4], [3:0-4-0,0-3-0], [4:0-4-4,0-2-4], [6:0-3-8,0-1-12], [9:0-4-4,0-2-4], [16:0-2-8,0-3-0] 3 (psf) SPACING- 2-0-0 CSI. DEFL. in (loc) l/defl L/d 20.0 Plate Grip DOL 1.25 TC 0.86 Vert(LL) -0.35 18-19 >999 240 7.0 Lumber DOL 1.25 BC 0.98 Vert(CT) -0.58 18-19 >803 180 0.0 * Rep Stress Incr YES WB 0.68 Horz(CT) 0.18 12 n/a n/a	6-10-0 7-6-0 9-4-0 4-0-0 6-5-8 2-11-0 sets (X,Y)- [2:0-6-0,0-0-4], [3:0-4-0,0-3-0], [4:0-4-4,0-2-4], [6:0-3-8,0-1-12], [9:0-4-4,0-2-4], [16:0-2-8,0-3-0] 3 (psf) SPACING- 2-0-0 CSI. DEFL. in (loc) l/defl L/d PLATES 20.0 Plate Grip DOL 1.25 TC 0.86 Vert(LL) -0.35 18-19 >999 240 MT20 7.0 Lumber DOL 1.25 BC 0.98 Vert(CT) -0.58 18-19 >803 180 0.0 * Rep Stress Incr YES WB 0.68 Horz(CT) 0.18 12 n/a n/a

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

7-17,10-13: 2x4 SP No.3

2x4 SP No.3 *Except* WEBS

11-12: 2x6 SP No.2

(size) 2=0-3-8, 12=0-3-8

Max Horz 2=223(LC 9)

Max Uplift 2=-289(LC 12), 12=-267(LC 13) Max Grav 2=1652(LC 2), 12=1581(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2475/446, 3-4=-1955/420, 4-5=-1551/406, 5-6=-1588/412, 6-7=-2636/621, TOP CHORD

7-8=-2621/525, 8-9=-2077/412, 9-10=-2468/459, 10-11=-2376/440, 11-12=-1559/292

2-21=-403/2076, 19-21=-403/2081, 18-19=-197/1617, 7-16=-269/187, 15-16=-469/2796, **BOT CHORD**

14-15=-367/2023

WEBS 3-21=0/282, 3-19=-656/274, 4-19=-99/774, 5-19=-315/184, 16-18=-153/1620,

6-16=-355/1377, 8-16=-841/233, 8-15=-1222/254, 9-15=-193/1220, 11-14=-320/1794

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-5-3, Interior(1) 2-5-3 to 14-4-0, Exterior(2R) 14-4-0 to 18-3-3, Interior(1) 18-3-3 to 23-8-0, Exterior(2R) 23-8-0 to 27-7-4, Interior(1) 27-7-4 to 34-1-8, Exterior(2R) 34-1-8 to 38-0-12, Interior(1) 38-0-12 to 39-1-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

This item has been electronically signed and sealed by ORegan, Philip, Pl using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek Iuc. DBA MiTek USA FL Cert 5634 16923 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building appropriate the property of the property

Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



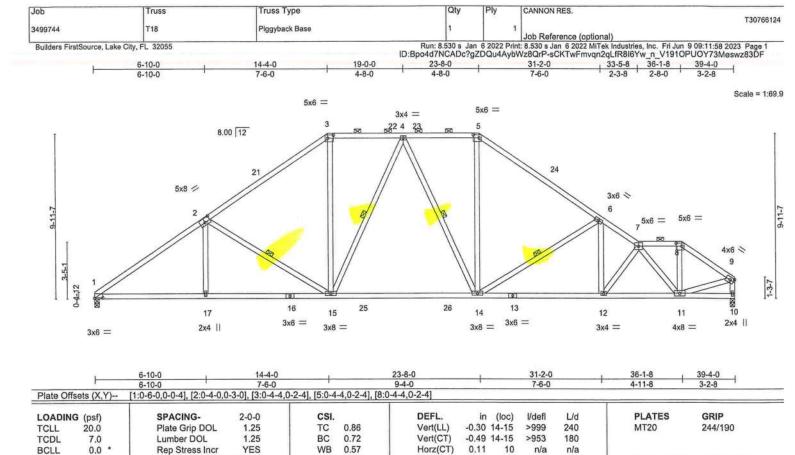
Structural wood sheathing directly applied, except end verticals, and

3-19, 5-19, 5-18, 6-18

2-0-0 oc purlins (4-0-13 max.): 4-6, 8-9.

1 Row at midpt

Rigid ceiling directly applied or 2-2-0 oc bracing.



BRACING-TOP CHORD

BOT CHORD

WEBS

LUMBER-

BCDL

TOP CHORD 2x4 SP No.2

2x4 SP No.1 *Except* **BOT CHORD** 1-16: 2x4 SP No.2

WFBS 2x4 SP No.3

REACTIONS. (size) 1=0-3-8, 10=0-3-8 Max Horz 1=209(LC 9)

Max Uplift 1=-257(LC 12), 10=-269(LC 13)

Max Grav 1=1588(LC 2), 10=1585(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 1-2=-2493/452, 2-3=-1965/421, 3-4=-1558/408, 4-5=-1605/412, 5-6=-2017/434, 6-7=-2669/510, 7-8=-1507/305, 8-9=-1809/335, 9-10=-1527/287

Code FBC2020/TPI2014

BOT CHORD 1-17=-411/2095, 15-17=-411/2100, 14-15=-195/1626, 12-14=-378/2258, 11-12=-420/2459 **WEBS**

2-17=0/286, 2-15=-669/281, 3-15=-98/777, 4-15=-315/182, 5-14=-105/805

6-14=-839/309, 6-12=-18/479, 7-12=-340/70, 7-11=-1519/272, 8-11=-111/843,

9-11=-241/1486

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-11-3, Interior(1) 3-11-3 to 14-4-0, Exterior(2R) 14-4-0 to 18-3-3. Interior(1) 18-3-3 to 23-8-0, Exterior(2R) 23-8-0 to 27-7-3, Interior(1) 27-7-3 to 36-1-8, Exterior(2E) 36-1-8 to 39-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-MS

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=257, 10=269.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Philip J. O'Regan PE No.58126 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chasterfield, MO 63017

June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Weight: 244 lb

Structural wood sheathing directly applied, except end verticals, and

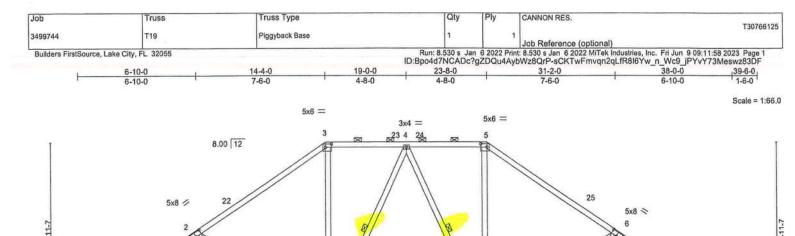
2-15, 4-15, 4-14, 6-14

2-0-0 oc purlins (4-6-8 max.): 3-5, 7-8.

1 Row at midpt

Rigid ceiling directly applied or 9-0-6 oc bracing.

FT = 20%



	1	6-10-0	14-4-0			23-8-0			31-	Total Control Control	38-0-0	
	1	6-10-0	7-6-0			9-4-0			7-6	-0	6-10-0	
Plate Offs	sets (X,Y)	[1:0-6-0,0-0-4], [2:0-4-0,0	-3-0], [3:0-4-4,0)-2-4], [5:0-4	-4,0-2-4], [6:0	0-4-0,0-3-0], [7:0-6	-0,0-0-4					
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.82	Vert(LL)	-0.32	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.95	Vert(CT)	-0.52	11-12	>885	180	200000000	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.28	Horz(CT)	0.11	7	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MS	THE REAL PROPERTY.					Weight: 222 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

28

11

3x8 =

10

3x6 =

9

Structural wood sheathing directly applied, except

Rigid ceiling directly applied or 2-2-0 oc bracing.

2-0-0 oc purlins (4-8-12 max.): 3-5.

1 Row at midpt

2x4 ||

2-12, 4-12, 4-11, 6-11

3x6 =

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

3x6 =

WEBS 2x4 SP No.3

(size) 1=Mechanical, 7=0-3-8

Max Horz 1=-226(LC 8)

Max Uplift 1=-291(LC 12), 7=-324(LC 13) Max Grav 1=1538(LC 2), 7=1607(LC 2)

14

2x4 ||

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2406/458, 2

1-2=-2406/458, 2-3=-1877/389, 3-4=-1484/389, 4-5=-1484/387, 5-6=-1875/387,

13

3x6 =

12

3x8 =

6-7=-2397/45

BOT CHORD 1-14=-427/2047, 12-14=-426/2052, 11-12=-167/1528, 9-11=-261/1946, 7-9=-262/1941

WEBS 2-14=0/286, 2-12=-670/279, 3-12=-114/728, 4-12=-268/179, 4-11=-270/179,

5-11=-114/726, 6-11=-658/272, 6-9=0/284

NOTES-

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-9-10, Interior(1) 3-9-10 to 14-4-0, Exterior(2R) 14-4-0 to 19-8-8, Interior(1) 19-8-8 to 23-8-0, Exterior(2R) 23-8-0 to 29-0-8, Interior(1) 29-0-8 to 39-6-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Refer to girder(s) for truss to truss connections.

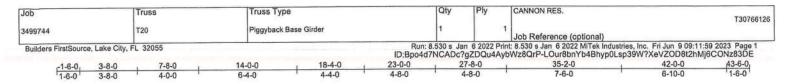
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=291, 7=324.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Philip J, O'Regan PE No.58126 MiTok luc, DBA MiTok USA FL Cert 5634 16025 Swingley Ridge Rd, Chesterfield, MO 63017

June 9,2023





Scale = 1:73.9 5x6 = 5x6 = 3x4 = 3x6 / 5 5x8 > 8x10 || 5x6 = 8.00 12 10 24 13

15

4x8 =

16

7x8 =

	74	3-8-	0 , 7-8-0 ,	14-0-0	18-4-0	27-8-0		1		35-2-0	42-0-0)
	1	3-8-		6-4-0	4-4-0	9-4-0		- 1		7-6-0	6-10-0)
Plate Offse	ets (X,	Y)	[3:0-2-4,0-6-4], [6:0-4-4,0	-2-4], [8:0-4-4,0	-2-4], [9:0-4-0,0-3-0], [16:0-4-0,0-4-12]						
LOADING	(psf)		SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plate Grip DOL	1.25	TC 0.94	Vert(LL)	-0.28 16	-17	>999	240	MT20	244/190
TCDL	7.0		Lumber DOL	1.25	BC 0.68	Vert(CT)	-0.49 16	-17	>999	180	24/20/2019	
BCLL	0.0	*	Rep Stress Incr	NO	WB 0.80	Horz(CT)	0.11	10	n/a	n/a		
BCDL	10.0		Code FBC2020/T	PI2014	Matrix-MS						Weight: 295 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

4-6: 2x4 SP 2850F 2.0E or 2x4 SP M 31, 8-9: 2x4 SP No.1

17

4x6 =

BOT CHORD 2x6 SP No.2 *Except*

18

3x8 ||

2-16: 2x6 SP M 26

WEBS 2x4 SP No.3

4x6 =

REACTIONS. (size) 2=0-3-8, 10=0-3-8

Max Horz 2=-233(LC 25)

Max Uplift 2=-520(LC 8), 10=-320(LC 9) Max Grav 2=2529(LC 2), 10=1843(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-4165/812, 3-4=-5239/968, 4-5=-3339/583, 5-6=-2540/488, 6-7=-2071/436, TOP CHORD

7-8=-1854/381, 8-9=-2316/379, 9-10=-2857/451 2-18=-743/3421, 17-18=-753/3478, 16-17=-1024/5299, 15-16=-451/2741, 13-15=-220/2011,

BOT CHORD 12-13=-260/2333, 10-12=-260/2328

3-18=-174/968, 3-17=-412/2112, 4-17=-1043/284, 4-16=-2745/614, 5-16=-205/1182, WEBS

5-15=-1236/394, 6-15=-204/1213, 7-13=-502/212, 8-13=-127/973, 9-13=-679/277,

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Provide adequate drainage to prevent water ponding.

5) The Fabrication Tolerance at joint 3 = 4%

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=520, 10=320.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 99 lb down and 63 lb up at 3-8-0 on top chord, and 78 lb down and 13 lb up at 3-8-0, and 752 lb down and 172 lb up at 3-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

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Phillip J. O'Regan PE No.55126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building company of the control of the Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



12

2x4 ||

4-16, 5-15, 7-15, 7-13, 9-13

Structural wood sheathing directly applied, except

2-0-0 oc purlins (2-2-10 max.): 3-4, 6-8. Rigid ceiling directly applied or 9-10-4 oc bracing.

4x8 =

1 Row at midpt

4x8 =

4x6 =

Job	Truss	Truss Type	Qty	Ply	CANNON RES.	T2070010
3499744	T20	Piggyback Base Girder	1		Job Reference (optional)	T3076612
Builders FirstSource, La	ke City, FL 32055		Run: 8.530 s Jar	6 2022 P	int: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:59 2	023 Page 2

Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:11:59 2023 Page 2 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-LOur8bnYb4Bhyp0Lsp39W?XeVZOD8t2hMj6CONz83DE

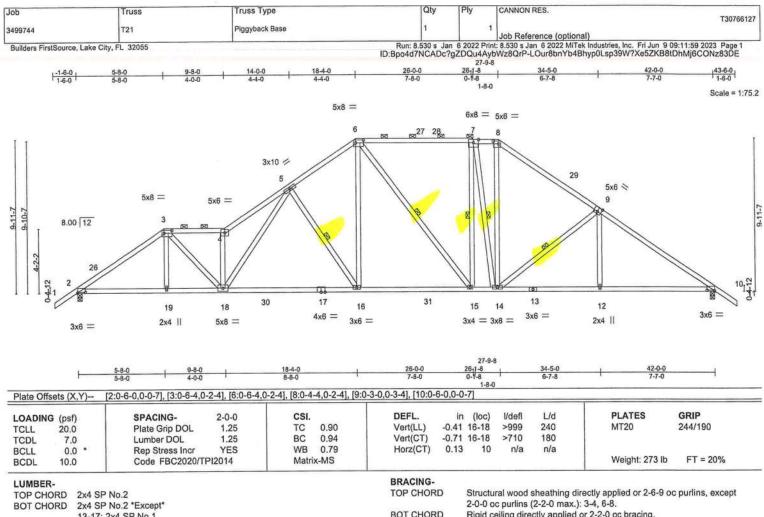
LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 4-6=-54, 6-8=-54, 8-11=-54, 2-10=-20

Concentrated Loads (lb) Vert: 3=-59(F) 18=-743(F)





WEBS

2x4 SP No.2 *Except*

13-17: 2x4 SP No.1

WEBS 2x4 SP No.3

Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Row at midpt

5-16, 6-15, 7-14, 9-14, 7-15

REACTIONS.

(size) 2=0-3-8, 10=0-3-8

Max Horz 2=-234(LC 10)

Max Uplift 2=-337(LC 12), 10=-300(LC 13) Max Grav 2=1797(LC 2), 10=1777(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-2726/487, 3-4=-3349/610, 4-5=-4152/802, 5-6=-2353/512, 6-7=-1821/453,

7-8=-1726/435, 8-9=-2154/468, 9-10=-2661/472

BOT CHORD 2-19=-431/2220, 18-19=-431/2225, 16-18=-368/2416, 15-16=-196/1926, 14-15=-156/1816, 12-14=-288/2146, 10-12=-288/2149

3-18=-248/1614, 4-18=-2418/504, 5-18=-385/1849, 5-16=-950/326, 6-16=-220/1201, WEBS

6-15=-263/108, 7-14=-666/256, 8-14=-205/1032, 9-14=-652/262, 9-12=0/306,

7-15=-49/356

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-8-6, Interior(1) 2-8-6 to 5-8-0, Exterior(2E) 5-8-0 to 9-8-0, Interior(1) 9-8-0 to 18-4-0, Exterior(2R) 18-4-0 to 22-6-6, Interior(1) 22-6-6 to 27-9-8, Exterior(2R) 27-9-8 to 31-11-14, Interior(1) 31-11-14 to 43-6-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Philip J. O'Regau PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16923 Swingley Ridge Rd. Chesterfield, MO 63017

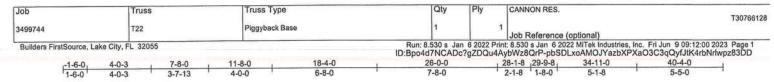
June 9,2023

ers and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE

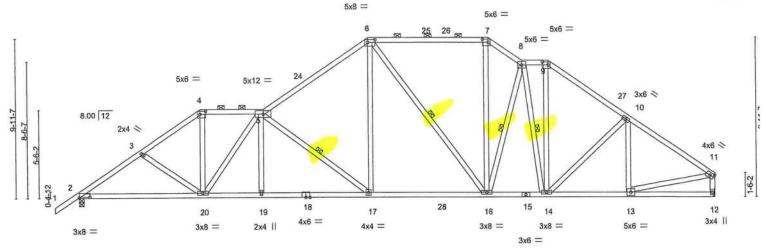
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	7	7-8-0	11-8-0	18-4-0	26-0-0	29-9-8	34-11-0 40)-4-0
	-	7-8-0	4-0-0	6-8-0	7-8-0	3-9-8	5-1-8 5	-5-0
Plate Offse	ets (X,Y)	[2:0-8-0,0-0-7], [4:0-4-4,0	-2-4], [6:0-5-12	0-2-0], [7:0-4-4,0-2-4], [9:0	-4-4,0-2-4]			
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL. in (loc)	l/defl L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC 0.87	Vert(LL) -0.23 16-17	>999 240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC 0.95	Vert(CT) -0.40 16-17	>999 180	1	
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.81	Horz(CT) 0.12 12	n/a n/a	2.50	
BCDL	10.0	Code FBC2020/T	PI2014	Matrix-MS			Weight: 282 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

6-7: 2x4 SP No.1

BOT CHORD 2x4 SP No.2

2x4 SP No.3 **WEBS**

BRACING-

WEBS

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied, except end verticals, and

2-0-0 oc purlins (3-8-6 max.): 4-5, 6-7, 8-9.

Rigid ceiling directly applied or 2-2-0 oc bracing. 1 Row at midpt 5-17, 6-16, 8-16, 8-14

REACTIONS.

BOT CHORD

(size) 2=0-3-8, 12=Mechanical

Max Horz 2=223(LC 9)

Max Uplift 2=-329(LC 12), 12=-260(LC 13) Max Grav 2=1687(LC 2), 12=1620(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2540/496, 3-4=-2399/471, 4-5=-1982/428, 5-6=-2142/471, 6-7=-1588/423, TOP CHORD

7-8=-1874/470, 8-9=-1549/395, 9-10=-1917/435, 10-11=-1971/376, 11-12=-1534/301

2-20=-508/2131, 19-20=-487/2608, 17-19=-488/2604, 16-17=-227/1734, 14-16=-230/1668,

13-14=-260/1591

4-20=-189/1152, 5-20=-1053/202, 5-17=-1115/341, 6-17=-161/1006, 6-16=-333/121, WEBS

7-16=-133/766, 8-16=-416/207, 8-14=-661/140, 9-14=-149/820, 11-13=-243/1548

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-6-6, Interior(1) 2-6-6 to 7-8-0, Exterior(2E) 7-8-0 to 11-8-0. Interior(1) 11-8-0 to 18-4-0, Exterior(2R) 18-4-0 to 22-4-6, Interior(1) 22-4-6 to 26-0-0, Exterior(2E) 26-0-0 to 28-1-8, Interior(1) 28-1-8 to 29-9-8, Exterior(2R) 29-9-8 to 33-9-14, Interior(1) 33-9-14 to 40-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=329, 12=260.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

This item has been electronically signed and sealed by ORegan, Philip, P using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

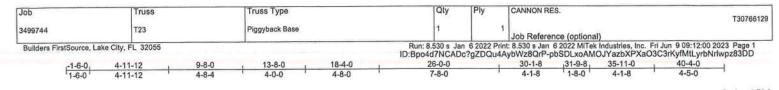
Philip J. O'Regan PE No.58126 MITek Iuc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

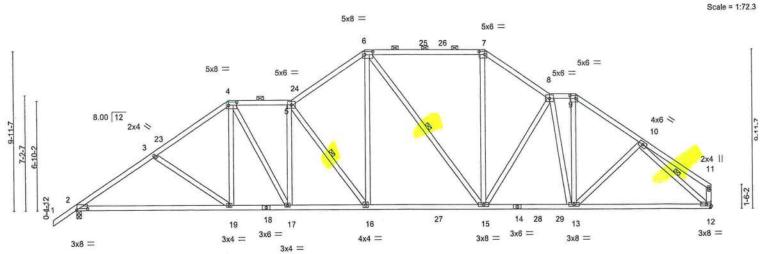
June 9,2023

ters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







	l .	9-8-0	4-0-) '	4-8-0	7-8-0	0	ı	5	-9-8	8-6-8	- 1
Plate Offs	ets (X,Y)	[2:0-8-4,0-0-15], [4:0-6-4,0	0-2-4], [6:0-6-4,0	1-2-4], [7:0-4	1-4,0-2-4], [9	:0-4-4,0-2-4]					,	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.81	Vert(LL)	-0.23	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.94	Vert(CT)	-0.44	19-22	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.75	Horz(CT)	0.12	12	n/a	n/a		
BCDL	10.0	Code FBC2020/TF	PI2014	Matri	k-MS	82 53					Weight: 276 lb	FT = 20%

26-0-0

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 *Except*

6-7: 2x4 SP No.1

BOT CHORD 2x4 SP No.2

2x4 SP No.3 WEBS

(size) 2=0-3-8, 12=Mechanical

Max Horz 2=223(LC 9)

Max Uplift 2=-329(LC 12), 12=-260(LC 13) Max Grav 2=1696(LC 2), 12=1645(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2510/498, 3-4=-2318/465, 4-5=-2252/503, 5-6=-2126/489, 6-7=-1613/424, TOP CHORD 7-8=-1948/466, 8-9=-1649/381, 9-10=-2016/423

2-19=-497/2111, 17-19=-345/1877, 16-17=-390/2265, 15-16=-228/1746, 13-15=-269/1819, BOT CHORD

12-13=-266/1488

3-19=-314/184, 4-19=-53/475, 4-17=-155/739, 5-17=-592/142, 5-16=-903/300, WEBS

6-16=-198/1035, 6-15=-325/115, 7-15=-120/777, 8-15=-474/203, 8-13=-794/184,

9-13=-169/932, 10-12=-1879/366

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-6-6, Interior(1) 2-6-6 to 9-8-0, Exterior(2E) 9-8-0 to 13-8-0, Interior(1) 13-8-0 to 18-4-0, Exterior(2R) 18-4-0 to 22-4-6, Interior(1) 22-4-6 to 26-0-0, Exterior(2E) 26-0-0 to 30-1-8, Interior(1) 30-1-8 to 31-9-8, Exterior(2R) 31-9-8 to 36-0-8, Interior(1) 36-0-8 to 40-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Refer to girder(s) for truss to truss connections.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=329, 12=260.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

This item has been electronically signed and sealed by ORegan, Philip, Pl using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regau PE No.58126 MITek lur. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TPI1 Quality Criteria, DSB-99 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



40-4-0

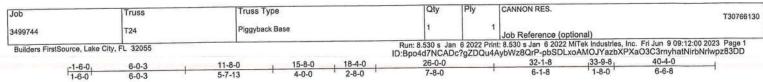
Structural wood sheathing directly applied or 3-3-11 oc purlins,

Rigid ceiling directly applied or 2-2-0 oc bracing.

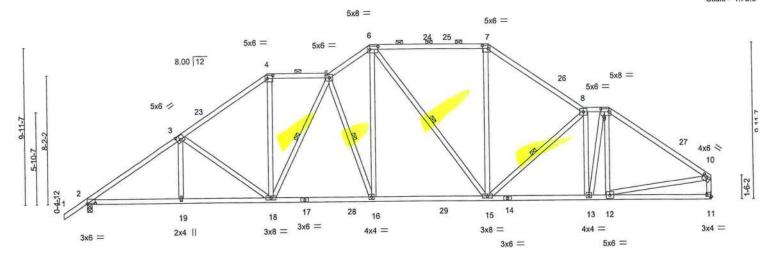
1 Row at midpt

except end verticals, and 2-0-0 oc purlins (3-5-2 max.): 4-5, 6-7, 8-9.

5-16, 6-15, 10-12



Scale = 1:73.6



	1	6-0-3	11-8-0	18-4-0	26-0-0	-		32-1-8	33-9-8	40-4-0 6-6-8	
Plate Offse	ets (X,Y)	6-0-3 [2:0-6-0,0-0-7], [3:0-2-12	5-7-13 0-3-0], [4:0-4-4	6-8-0 0-2-4], [6:0-6-4,0-2-4], [7:0-	7-8-0 4-4,0-2-4], [9:0-6-4,0-2	-4], [10:		6-1-8 -8], [11:Edg	1-8-0 ' ge,0-1-8]	0-0-0	
LOADING TCLL TCDL	20.0 7.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI. TC 0.78 BC 0.80 WB 0.58	Vert(LL) -0.21	(loc) 15-16 15-16	l/defl >999 >999 n/a	L/d 240 180 n/a	PLA MT2	3	GRIP 244/190
BCLL BCDL	0.0 *	Code FBC2020/T		Matrix-MS	, , ,			o consumer Work for	Weig	ght: 275 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

6-7: 2x4 SP No.1

BOT CHORD 2x4 SP No.2

2x4 SP No.3 WEBS

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 3-0-12 oc purlins, except end verticals, and 2-0-0 oc purlins (3-6-12 max.): 4-5, 6-7, 8-9.

Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 8-6-11 oc bracing: 2-19

8-7-12 oc bracing: 18-19.

5-18, 5-16, 6-15, 8-15 WEBS 1 Row at midpt

REACTIONS.

(size) 2=0-3-8, 11=Mechanical

Max Horz 2=223(LC 9)

Max Uplift 2=-329(LC 12), 11=-260(LC 13) Max Grav 2=1714(LC 2), 11=1637(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2605/470, 3-4=-2206/456, 4-5=-1786/426, 5-6=-2107/496, 6-7=-1618/426, TOP CHORD 7-8=-1998/450, 8-9=-1870/416, 9-10=-2042/386, 10-11=-1529/311

2-19=-470/2157, 18-19=-470/2162, 16-18=-310/2024, 15-16=-222/1769, 13-15=-305/1919, **BOT CHORD**

12-13=-248/1627 3-18=-513/209, 4-18=-146/964, 5-18=-543/146, 5-16=-809/294, 6-16=-233/1091, WEBS

6-15=-347/115, 7-15=-99/752, 8-15=-477/216, 8-13=-929/208, 9-13=-192/1017,

10-12=-209/1519

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-6-6, Interior(1) 2-6-6 to 11-8-0, Exterior(2E) 11-8-0 to 15-8-0, Interior(1) 15-8-0 to 18-4-0, Exterior(2R) 18-4-0 to 22-4-6, Interior(1) 22-4-6 to 26-0-0, Exterior(2R) 26-0-0 to 30-0-6, Interior(1) 30-0-6 to 33-9-8, Exterior(2R) 33-9-8 to 37-9-14, Interior(1) 37-9-14 to 40-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=329, 11=260,
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

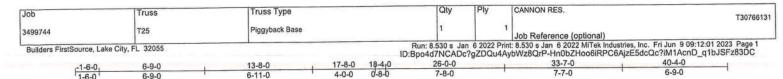
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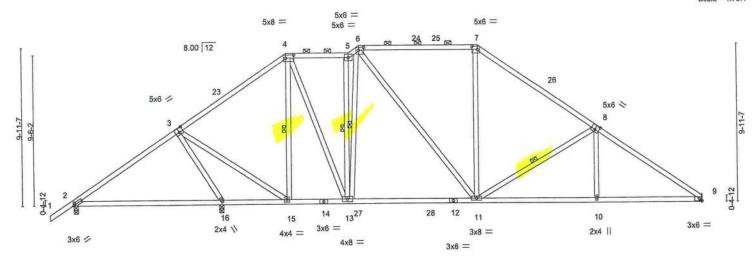
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	-	9-5-12 9-5-12	13-8-	4-	-0-0 0-1		0	0 Ed1	33-7-0 7-7-0	6-9-0	
Plate Offsets	(X,Y)	[2:0-1-5,0-1-8], [3:0-3-0,0-3	4], [4:0-6-4,0-2	-4], [6:0-4-0,0	J-2-0], [7:0	-4-4,0-2-4], [8:0-3	0,0-3-4], [9:0-2	-s,Eugej			
	osf) 0.0 7.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25		.83 .78	DEFL. Vert(LL) Vert(CT)	in (loc) 0.32 16-19 -0.37 16-19	l/defl >354 >311	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL (0.0 * 0.0	Rep Stress Incr Code FBC2020/TPI	YES 2014	WB 0. Matrix-M	.82 1S	Horz(CT)	0.05 9	n/a	n/a	Weight: 257 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2

WEBS

BOT CHORD 2x4 SP No.2 2x4 SP No.3 **BRACING-**TOP CHORD

BOT CHORD

WEBS

1 Row at midpt

2-0-0 oc purlins (3-7-0 max.): 4-5, 6-7. Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

Structural wood sheathing directly applied or 3-9-14 oc purlins, except

6-0-0 oc bracing: 15-16.

4-15, 5-13, 6-13, 8-11

REACTIONS.

BOT CHORD

(size) 2=0-3-8, 16=0-3-8, 9=Mechanical

Max Horz 2=-219(LC 10)

Max Uplift 2=-100(LC 12), 16=-248(LC 12), 9=-231(LC 13) Max Grav 2=516(LC 23), 16=1570(LC 2), 9=1267(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-424/177, 3-4=-754/224, 4-5=-874/283, 5-6=-941/297, 6-7=-1058/329, 7-8=-1367/324, 8-9=-1894/357

2-16=-181/339, 15-16=-564/211, 13-15=-121/579, 11-13=-135/885, 10-11=-230/1539, 9-10=-230/1538

WEBS

3-16=-1449/336, 3-15=-93/1237, 4-15=-686/127, 4-13=-159/914, 5-13=-450/129,

6-11=-99/355, 7-11=-17/397, 8-11=-679/282, 8-10=0/294

NOTES-

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-6-6, Interior(1) 2-6-6 to 13-8-0, Exterior(2E) 13-8-0 to 17-8-0, Interior(1) 17-8-0 to 18-4-0, Exterior(2R) 18-4-0 to 22-4-6, Interior(1) 22-4-6 to 26-0-0, Exterior(2R) 26-0-0 to 30-0-6, Interior(1) 30-0-6 to 40-4-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=100, 16=248, 9=231.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16923 Swingley Ridge Rd. Chesterfield, MO 63017

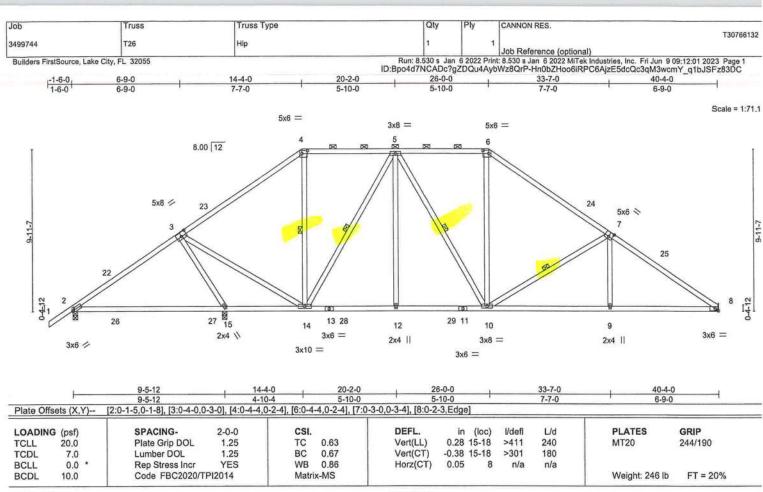
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LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

2x4 SP No.3

BRACING-

WEBS

TOP CHORD

Structural wood sheathing directly applied or 3-10-14 oc purlins,

2-0-0 oc purlins (5-6-2 max.): 4-6 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 14-15. 1 Row at midpt

4-14, 5-14, 5-10, 7-10

REACTIONS.

(size) 2=0-3-8, 15=0-3-8, 8=Mechanical

Max Horz 2=226(LC 9)

Max Uplift 2=-104(LC 12), 15=-315(LC 12), 8=-268(LC 13) Max Grav 2=501(LC 25), 15=1608(LC 2), 8=1269(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-398/154, 3-4=-818/224, 4-5=-593/219, 5-6=-1056/355, 6-7=-1366/349, TOP CHORD

7-8=-1928/416

BOT CHORD 2-15=-179/334, 14-15=-606/243, 12-14=-143/987, 10-12=-143/987, 9-10=-264/1554,

8-9=-264/1554

3-15=-1525/414, 3-14=-156/1327, 5-14=-809/180, 5-12=0/336, 6-10=-49/432, WEBS

7-10=-697/278, 7-9=0/309

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-6-6, Interior(1) 2-6-6 to 14-4-0, Exterior(2R) 14-4-0 to 20-2-0, Interior(1) 20-2-0 to 26-0-0, Exterior(2R) 26-0-0 to 31-8-7, Interior(1) 31-8-7 to 40-4-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

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7) Refer to girder(s) for truss to truss connections.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=104, 15=315, 8=268.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



CANNON RES Qtv Truss Type Truss Joh T30766133 T27 Piggyback Base 3499744 Job Reference (optional) Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:01 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-Hn0bZHoo6iRPC6AjzE5dcQc3qM3wcmY_q1bJSFz83DC Builders FirstSource, Lake City, FL 32055 40-4-0 26-0-0 5-10-0 33-7-0 7-7-0 20-2-0 5-10-0 7-7-0 Scale = 1:71.1 5x6 = 3x8 = 5x6 = 6 8.00 12 5x8 / 5x6 > 15 29 11 27 13 28 9 12 10 3x6 = 3x6 = 2x4 \\ 2x4 || 2x4 || 3x8 = 3x6 / 3x10 = 3x6 = 26-0-0 5-10-0 6-9-0 5-10-0 [2:0-1-5,0-1-8], [3:0-4-0,0-3-0], [4:0-4-4,0-2-4], [6:0-4-4,0-2-4], [7:0-3-0,0-3-4], [8:0-2-3,Edge] Plate Offsets (X,Y)-GRIP PLATES DEFL I/defl L/d CSI. SPACING-2-0-0 LOADING (psf) 244/190 Vert(LL) 0.28 15-18 >411 240 MT20 TC 0.63 Plate Grip DOL 1.25 TCLL 20.0 BC 0.67 Vert(CT) -0.38 15-18 >301 180 1.25 TCDL 7.0 Lumber DOL WB Horz(CT) 0.05 8 n/a n/a 0.86 YES BCLL 0.0 Rep Stress Incr FT = 20% Weight: 246 lb Code FBC2020/TPI2014 Matrix-MS BCDL 10.0 BRACING-LUMBER-Structural wood sheathing directly applied or 3-10-14 oc purlins, TOP CHORD TOP CHORD 2x4 SP No.2 2x4 SP No.2 except BOT CHORD 2-0-0 oc purlins (5-6-2 max.): 4-6. 2x4 SP No.3 WEBS Rigid ceiling directly applied or 10-0-0 oc bracing, Except: BOT CHORD 6-0-0 oc bracing: 14-15. 4-14, 5-14, 5-10, 7-10 WEBS 1 Row at midpt 2=0-3-8, 15=0-3-8, 8=Mechanical REACTIONS. (size) Max Horz 2=226(LC 11) Max Uplift 2=-104(LC 12), 15=-315(LC 12), 8=-268(LC 13)

Max Grav 2=501(LC 25), 15=1608(LC 2), 8=1269(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-398/154, 3-4=-818/224, 4-5=-593/219, 5-6=-1056/355, 6-7=-1366/349,

TOP CHORD 7-8=-1928/416

2-15=-179/334, 14-15=-606/243, 12-14=-143/987, 10-12=-143/987, 9-10=-264/1554, BOT CHORD

8-9=-264/1554

3-15=-1525/414, 3-14=-156/1327, 5-14=-809/180, 5-12=0/336, 6-10=-49/432,

7-10=-697/278, 7-9=0/309

NOTES-

WEBS

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-6-6, Interior(1) 2-6-6 to 14-4-0, Exterior(2R) 14-4-0 to 20-2-0, Interior(1) 20-2-0 to 26-0-0, Exterior(2R) 26-0-0 to 31-8-7, Interior(1) 31-8-7 to 40-4-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=104, 15=315, 8=268.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

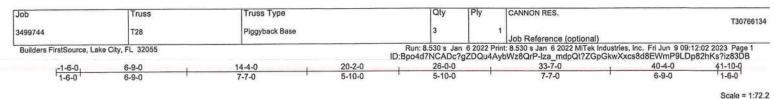
elers and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

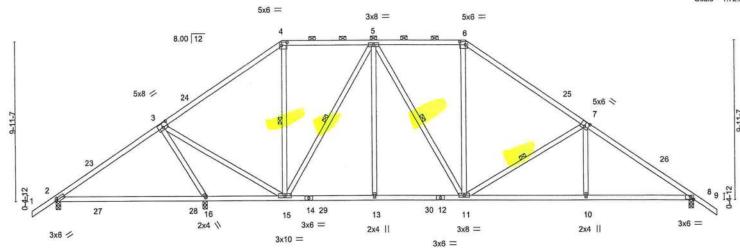
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2870 Crain Highway, Suite 203 Waldorf, MD 20601







		9-5-12	14-4-0	1	20-2-0	26-	-0-0	- 1		33-7-0	40-4-0	
		9-5-12	4-10-4		5-10-0		0-0			7-7-0	6-9-0	
Plate Offse	ets (X,Y)	[2:0-1-5,0-1-8], [3:0-4-0,0	-3-0], [4:0-4-4,0-2	2-4], [6:0-4-	4,0-2-4], [7:0-3	3-0,0-3-4], [8:0-2	-3,Edge	1				
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.64	Vert(LL)	0.28	16-19	>411	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.67	Vert(CT)	-0.38	16-19	>301	180		
BCLL BCDL	0.0 *	Rep Stress Incr Code FBC2020/T	YES PI2014	WB Matri	0.86 x-MS	Horz(CT)	0.05	8	n/a	n/a	Weight: 248 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-TOP CHORD

Structural wood sheathing directly applied or 3-11-11 oc purlins,

2-0-0 oc purlins (5-6-4 max.): 4-6. **BOT CHORD**

Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing; 15-16.

WEBS 4-15, 5-15, 5-11, 7-11 1 Row at midpt

REACTIONS.

(size) 2=0-3-8, 16=0-3-8, 8=0-3-8

Max Horz 2=-233(LC 10)

Max Uplift 2=-107(LC 12), 16=-310(LC 12), 8=-303(LC 13) Max Grav 2=501(LC 25), 16=1608(LC 2), 8=1345(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-398/161, 3-4=-817/230, 4-5=-591/223, 5-6=-1053/357, 6-7=-1362/351, TOP CHORD

BOT CHORD 2-16=-173/341, 15-16=-609/255, 13-15=-135/985, 11-13=-135/985, 10-11=-230/1543,

8-10=-230/1542

3-16=-1524/408, 3-15=-149/1326, 5-15=-807/178, 5-13=0/336, 6-11=-50/429,

7-11=-685/270, 7-10=0/307

NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-6-6, Interior(1) 2-6-6 to 14-4-0, Exterior(2R) 14-4-0 to 20-2-0, Interior(1) 20-2-0 to 26-0-0, Exterior(2R) 26-0-0 to 31-8-7, Interior(1) 31-8-7 to 41-10-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=107, 16=310, 8=303. 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Philip J. O'Regon PE No.58126 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

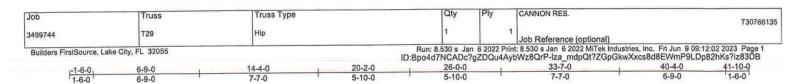
June 9,2023

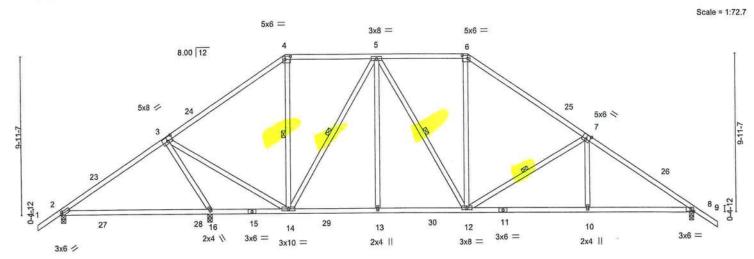
ters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for on individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design, Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







1	9-5-12 9-5-12	14-4-0					33-7-0 7-7-0	40-4-0 6-9-0	
Plate Offsets (X,Y)	[2:0-1-5,0-1-8], [3:0-4-0,0	-3-0], [4:0-4-4,0-2	?-4], [6:0-4-4,0-2-4], [7	7:0-3-0,0-3-4], [8:0-2-	3,Edge]				
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/T	2-0-0 1.25 1.25 YES	CSI. TC 0.64 BC 0.67 WB 0.86 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.28 16-19 -0.38 16-19 0.05 8	l/defl >411 >301 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 248 lb	GRIP 244/190 FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

2x4 SP No.3 WEBS

(size) 2=0-3-8, 16=0-3-8, 8=0-3-8

Max Horz 2=-233(LC 10) Max Uplift 2=-107(LC 12), 16=-310(LC 12), 8=-303(LC 13) Max Grav 2=501(LC 25), 16=1608(LC 2), 8=1345(LC 20)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown. TOP CHORD 2-3=-398/161, 3-4=-817/230, 4-5=-591/223, 5-6=-1053/357, 6-7=-1362/351,

7-8=-1916/412

2-16=-173/341, 14-16=-609/255, 13-14=-135/985, 12-13=-135/985, 10-12=-230/1543, BOT CHORD

8-10=-230/1542

3-16=-1524/408, 3-14=-149/1326, 5-14=-807/178, 5-13=0/336, 6-12=-50/429,

7-12=-685/270, 7-10=0/307

NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-6-6, Interior(1) 2-6-6 to 14-4-0, Exterior(2R) 14-4-0 to 20-2-0, Interior(1) 20-2-0 to 26-0-0, Exterior(2R) 26-0-0 to 31-8-7, Interior(1) 31-8-7 to 41-10-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=107, 16=310, 8=303.

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Structural wood sheathing directly applied or 3-11-11 oc purlins.

4-14, 5-14, 5-12, 7-12

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 14-16.

1 Row at midpt

Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

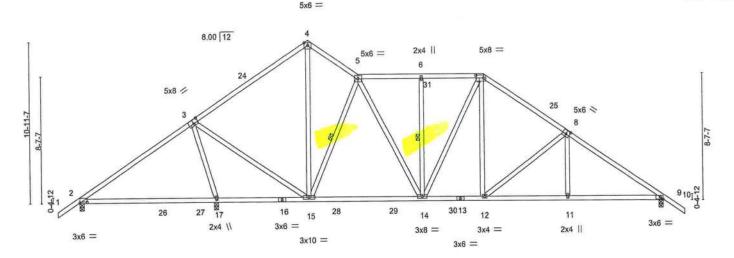
elers and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design, Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



CANNON RES. Ply Truss Type Job Truss T30766136 Roof Special T30 3499744 | ' | Job Reference (optional)
Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:02 2023 Page ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-Iza_mdpQt?ZGpGkwXxcs8d8BTmOoLCK82hKs?iz63DB Builders FirstSource, Lake City, FL 32055 40-4-0 6-7-0 28-0-0 23-8-0 19-4-0 3-6-0

Scale = 1:78.7



		9-5-12 9-5-12		15-10-0 6-4-4	1	23-8-0 7-10-0	-	4-4-0	-1-	5-9-0	6-7-0	
Plate Offsets	(X,Y)	[2:0-6-0,0-0-12], [3:0-4-0,	,0-3-4], [7:0-6-4	,0-2-4], [8:0-3-0	,0-3-0], [9:0-2-	3,Edgej						
TCDL BCLL	osf) 0.0 7.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/T	2-0-0 1.25 1.25 YES	BC 0.	.77 .75 .95	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.38 -0.48 0.04	(loc) 17-20 17-20 9	l/defl >301 >236 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 253 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD** 2x4 SP No.3 WEBS

BRACING-

TOP CHORD **BOT CHORD** WEBS

Structural wood sheathing directly applied or 3-11-14 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt 5-15, 6-14

REACTIONS.

2=0-3-8, 17=0-3-8, 9=0-3-8 (size)

Max Horz 2=-255(LC 10)

Max Uplift 2=-80(LC 12), 17=-294(LC 12), 9=-307(LC 13) Max Grav 2=415(LC 23), 17=1823(LC 2), 9=1302(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 3-4=-767/264, 4-5=-682/272, 5-6=-1139/359, 6-7=-1139/359, 7-8=-1387/385, TOP CHORD

8-9=-1833/415

15-17=-551/286, 14-15=-91/945, 12-14=-54/1092, 11-12=-229/1469, 9-11=-229/1469 **BOT CHORD**

3-17=-1603/341, 3-15=-174/1212, 4-15=-184/519, 5-15=-988/352, 5-14=-109/487, **WEBS**

6-14=-258/140, 7-12=-106/494, 8-12=-573/222, 8-11=0/273

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-6-6, Interior(1) 2-6-6 to 15-10-0, Exterior(2E) 15-10-0 to 19-4-0, Interior(1) 19-4-0 to 28-0-0, Exterior(2R) 28-0-0 to 32-0-6, Interior(1) 32-0-6 to 41-10-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 17=294, 9=307.

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Philip J. O'Regau PE No.58126 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

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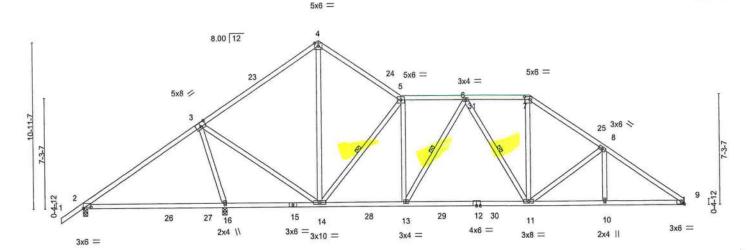
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

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CANNON RES. Truss Type Qty Ply Truss Joh T30766137 Roof Special 3499744 | Job Reference (optional)
Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:03 2023 Page ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-DA8Mzyq2eJh7RQJ65f75hrhM0AjR4e8HHL4PX8z83DA Builders FirstSource, Lake City, FL 32055 30-0-0 34-11-10 40-4-0 25-8-0 15-10-0 4-11-10

Scale = 1:76.4



H	9-5-12		6-4-4	5-6	3-0	8-8-0			1-10	5-4-6
Plate Offsets (X,Y)	[2:0-6-0,0-0-11], [3:0-4-0,	0-3-4], [7:0-4-4	,0-2-4], [9:0-2	2-3,Edge]						
LOADING (psf) FCLL 20.0 FCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/Ti	2-0-0 1.25 1.25 YES PI2014	CSI. TC BC WB Matri:	0.78 0.79 0.98 x-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	(loc) 16-22 16-22 9	l/defl >301 >249 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 24	GRIP 244/190 0 lb FT = 20%

21_4_0

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS

BRACING-

TOP CHORD **BOT CHORD** WEBS

30-0-0

Structural wood sheathing directly applied or 4-1-10 oc purlins.

34-11-10

40-4-0

Rigid ceiling directly applied or 6-0-0 oc bracing. 5-14, 6-13, 6-11 1 Row at midpt

REACTIONS.

9=Mechanical, 2=0-3-8, 16=0-3-8 (size)

Max Horz 2=247(LC 11)

Max Uplift 9=-269(LC 13), 2=-70(LC 12), 16=-307(LC 12) Max Grav 9=1202(LC 2), 2=385(LC 23), 16=1900(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-163/290, 3-4=-710/252, 4-5=-646/244, 5-6=-1239/339, 6-7=-1214/366, TOP CHORD

7-8=-1516/389, 8-9=-1848/427

14-16=-645/290, 13-14=-135/1231, 11-13=-156/1287, 10-11=-285/1495, 9-10=-285/1495 **BOT CHORD** 3-16=-1649/357, 3-14=-204/1269, 4-14=-135/405, 5-14=-1203/357, 5-13=-33/439, WEBS

7-11=-100/621, 8-11=-438/195

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-6-6, Interior(1) 2-6-6 to 15-10-0, Exterior(2R) 15-10-0 to 19-10-6, Interior(1) 19-10-6 to 30-0-0, Exterior(2R) 30-0-0 to 34-0-6, Interior(1) 34-0-6 to 40-4-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 9=269, 16=307.

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Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

elers and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-7473 fev. 5192/2020 BEFORE USE.

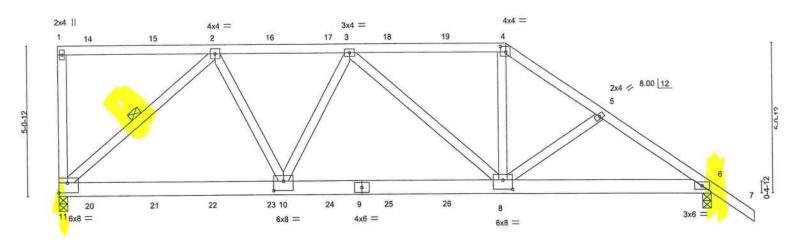
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, eraction and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2570 Crain Highway, Suite 203 Waldorf, MD 20601



CANNON RES. Truss Type Job Truss T30766138 Roof Special Girder T32 3499744 | ' | Job Reference (optional)
Run: 8,530 s Jan 6 2022 Print: 8,530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:03 2023 Page 1
ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-DA8Mzyq2eJh7RQJ65f75hrhRdAhC4kzHHL4PX8z83DA Builders FirstSource, Lake City, FL 32055 23-10-0 18-6-5 9-11-9 15-4-0

Scale = 1:39.0



1		7-8-1				15-4-0		-			22-4-0	
Plate Offsets (Y V_	7-8-1 [4:0-2-4,0-2-4], [8:0-4-0,0	-3-121 [10:0-4-	0.0-3-12], [1	:Edge.0-4-0	7-8-0 01		-			7-0-0	
LOADING (ps	sf)	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25	CSI. TC BC	0.49 0.93	DEFL. Vert(LL) Vert(CT)	in 0.11 -0.19	(loc) 8-10 8-10	l/defl >999 >999	L/d 240 180	PLATES MT20	GRIP 244/190
	.0 *	Rep Stress Incr Code FBC2020/T	NO	WB Matri	0.61	Horz(CT)	0.05	6	n/a	n/a	Weight: 143 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

end verticals.

1 Row at midpt

LUMBER-

2x4 SP No 2 TOP CHORD 2x6 SP No.2 BOT CHORD 2x4 SP No.3 WEBS

REACTIONS.

11=0-3-8, 6=0-3-8 (size)

Max Horz 11=-191(LC 24)

Max Uplift 11=-795(LC 4), 6=-583(LC 9)

Max Grav 11=1982(LC 1), 6=1652(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2093/814, 3-4=-2031/810, 4-5=-2454/931, 5-6=-2597/951

BOT CHORD

10-11=-609/1576, 8-10=-822/2183, 6-8=-720/2118

2-11=-2068/834, 2-10=-407/1178, 4-8=-317/1008 WEBS

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl.,
- GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=795, 6=583,
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 65 lb down and 47 lb up at 1-0-12, 65 lb down and 47 lb up at 1-3-4, 65 lb down and 47 lb up at 3-3-4, 65 lb down and 47 lb up at 5-3-4, 65 lb down and 47 lb up at 7-3-4, 65 lb down and 47 lb up at 11-3-4, and 65 lb down and 47 lb up at 13-3-4, and 155 lb down and 155 lb up at 15-4-0 on top chord, and 158 lb down and 78 lb up at 1-0-12, 157 lb down and 79 lb up at 1-3-4, 157 lb down and 79 lb up at 3-3-4, 157 lb down and 79 lb up at 5-3-4, 157 lb down and 79 lb up at 7-3-4, 157 lb down and 79 lb up at 9-3-4, 157 lb down and 79 lb up at 11-3-4, and 157 lb down and 79 lb up at 13-3-4, and 427 lb down and 220 lb up at 15-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 6-11=-20

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Structural wood sheathing directly applied or 3-7-0 oc purlins, except

2-11

Rigid ceiling directly applied or 8-0-6 oc bracing.

Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

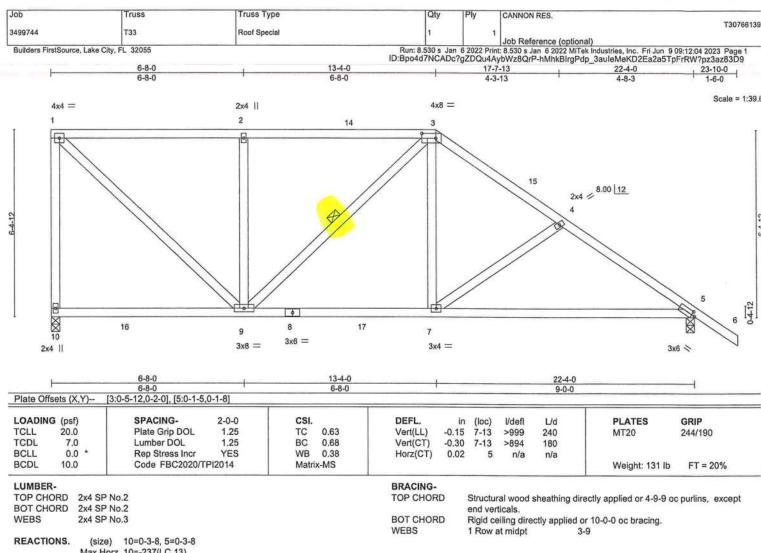


Job	Truss	Truss Type	Qty	Ply	CANNON RES.	T3076613
3499744	T32	Roof Special Girder	1		Job Reference (optional)	13070013
Builders FirstSource,	Lake City, FL 32055				rint: 8.530 s Jan 6 2022 MiTek Industries, Inc. AybWz8QrP-DA8Mzyq2eJh7RQJ65f75hr	

LOAD CASE(S) Standard

Concentrated Loads (lb)
Vert: 4=-90(B) 2=-17(B) 8=-427(B) 14=-36(B) 15=-17(B) 16=-17(B) 17=-17(B) 18=-17(B) 19=-17(B) 20=-315(B) 21=-157(B) 22=-157(B) 23=-157(B) 24=-157(B) 25=-157(B) 26=-157(B) 26=





Max Horz 10=-237(LC 13)

Max Uplift 10=-211(LC 8), 5=-168(LC 13) Max Grav 10=925(LC 2), 5=966(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-10=-795/227, 1-2=-738/172, 2-3=-738/172, 3-4=-1063/179, 4-5=-1240/207

BOT CHORD 7-9=-47/842, 5-7=-98/1020

WEBS 1-9=-231/988, 2-9=-414/204, 3-7=-51/483, 4-7=-297/173

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 13-4-0, Exterior(2R) 13-4-0 to 16-4-0, Interior(1) 16-4-0 to 23-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=211, 5=168.

This item has been electronically signed and sealed by ORegan, Philip, Pl using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regau PE No. 88126 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

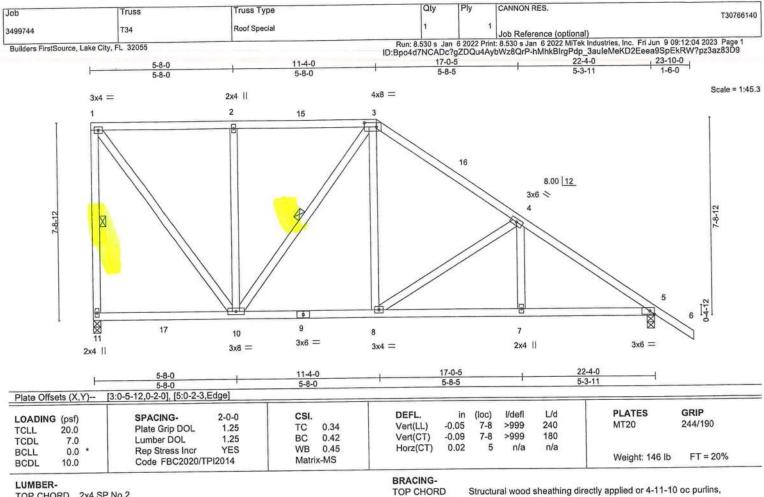
June 9,2023

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

**available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BOT CHORD

WEBS

except end verticals.

1 Row at midpt

Rigid ceiling directly applied or 10-0-0 oc bracing.

1-11, 3-10

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 2x4 SP No.3

(size) 11=0-3-8, 5=0-3-8

Max Horz 11=-283(LC 13) Max Uplift 11=-205(LC 8), 5=-170(LC 13) Max Grav 11=936(LC 2), 5=988(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD **BOT CHORD**

REACTIONS.

1-11=-825/218, 1-2=-545/120, 2-3=-545/120, 3-4=-915/153, 4-5=-1320/186

10-11=-146/282, 8-10=-17/722, 7-8=-65/1056, 5-7=-65/1056

WEBS

1-10=-199/901, 2-10=-346/176, 3-10=-325/123, 3-8=-73/496, 4-8=-505/205

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 11-4-0, Exterior(2R) 11-4-0 to 14-4-0, Interior(1) 14-4-0 to 23-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=205, 5=170.

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June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



CANNON RES Truss Type Job Truss T30766141 Hip T35 | ' | Job Reference (optional) Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:04 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-hMhkBlrgPdp_3auleMeKD2Ea9a8Cp9rRW?pz3az83D9 3499744 Builders FirstSource, Lake City, FL 32055 15-11-6 Scale = 1:52.9 4x8 = 4x6 = 8.00 12 2 14 15 16 17 3x6 / 3x6 > 6-7-7 8 20 6 9 7 10 3x6 = 2x4 || 3x6 = 3x8 = 2x4 || 3x4 = 5-8-0 [2:0-5-12,0-2-0], [3:0-3-12,0-2-0], [5:0-2-3,Edge] Plate Offsets (X,Y)--L/d PLATES GRIP DEFL I/defl CSI. (loc) SPACING-2-0-0 LOADING (psf) 244/190 MT20 6-13 >999 240 Vert(LL) -0.06 Plate Grip DOL 1.25 TC 0.63 TCLL 20.0 -0.11 6-13 >999 180 Vert(CT) 1.25 BC 0.50 Lumber DOL TCDL 7.0 0.76 Horz(CT) 0.02 n/a Rep Stress Incr YES WR BCLL 0.0 FT = 20% Weight: 149 lb Code FBC2020/TPI2014 Matrix-MS BCDL 10.0 BRACING-LUMBER-Structural wood sheathing directly applied or 4-10-11 oc purlins, TOP CHORD 2x4 SP No.2 TOP CHORD except end verticals. 2x4 SP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD**

WEBS

BOT CHORD WEBS

2x4 SP No.3

(size) 5=Mechanical, 10=0-3-8

Max Horz 10=-253(LC 13) Max Uplift 5=-165(LC 13), 10=-185(LC 13) Max Grav 5=918(LC 20), 10=911(LC 2)

TOP CHORD

REACTIONS.

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-439/116, 2-3=-572/204, 3-4=-776/180, 4-5=-1279/235, 1-10=-865/193

BOT CHORD

9-10=-129/252, 7-9=-66/408, 6-7=-116/1012, 5-6=-116/1012

WEBS

2-9=-419/143, 2-7=-172/507, 4-7=-625/251, 4-6=0/281, 1-9=-134/663

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 3-8-0, Exterior(2R) 3-8-0 to 7-10-15, Interior(1) 7-10-15 to 9-4-0, Exterior(2R) 9-4-0 to 13-6-15, Interior(1) 13-6-15 to 22-4-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=165, 10=185.

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2-9

1 Row at midpt

Philip J. O'Regan PE No. 58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

ers and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Compositions available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

16023 Swingley Ridge Rd Chesterfield, MO 63017

MiTek

Truss Type CANNON RES. Joh Truss T30766142 T36 Hip 3499744 Job Reference (optional)

Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:04 2023 Page
ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-hMhkBlrgPdp_3auleMeKD2EZ1a5tpFWRW?pz3az83D9 Builders FirstSource, Lake City, FL 32055 14-8-14 7-4-14 Scale = 1:61.0 4x8 = 4x4 = 2 8.00 12 16 3x6 / 5x8 > 19 7 10 9 8 6 2x4 || 3x6 = 3x6 = 3x4 = 3x8 =2x4 || 7-4-0 5-8-0 Plate Offsets (X,Y)--[2:0-5-12,0-2-0], [3:0-2-4,0-2-4], [4:0-4-0,0-3-0], [5:0-2-3,Edge] LOADING (psf) SPACING-2-0-0 CSI. DEFL **V**defl PLATES GRIP (loc) 20.0 Plate Grip DOL 1.25 TC 0.70 Vert(LL) -0.11 6-13 >999 240 MT20 244/190 TCLL TCDL 7.0 Lumber DOL 1.25 BC 0.65 Vert(CT) -0.206-13 >999 180 0.0 Rep Stress Incr YES WB 0.33 Horz(CT) 0.02 BCLL n/a n/a Weight: 159 lb FT = 20%BCDL 10.0 Code FBC2020/TPI2014 Matrix-MS BRACING-LUMBER-TOP CHORD TOP CHORD 2x4 SP No.2 Structural wood sheathing directly applied or 4-7-15 oc purlins, 2x4 SP No.2 except end verticals. BOT CHORD **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

WEBS

1 Row at midpt

2-9, 3-8, 4-8

2x4 SP No.3 WEBS

REACTIONS.

10=0-3-8, 5=Mechanical (size)

Max Horz 10=-273(LC 13)

Max Uplift 10=-193(LC 13), 5=-156(LC 13) Max Grav 10=931(LC 20), 5=925(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 1-2=-546/162, 2-3=-450/184, 3-4=-629/149, 4-5=-1231/209, 1-10=-819/209

BOT CHORD

9-10=-157/270, 8-9=-39/456, 6-8=-79/960, 5-6=-79/960

WEBS 2-9=-299/98, 2-8=-168/471, 4-8=-730/289, 4-6=0/334, 1-9=-129/604

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 5-8-0, Exterior(2E) 5-8-0 to 7-4-0, Exterior(2R) 7-4-0 to 11-6-15, Interior(1) 11-6-15 to 22-4-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=193, 5=156.

This item has been electronically signed and sealed by ORegan, Philip, Pl using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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June 9,2023

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3499744 T37 Roof Special Girder Builders FirstSource, Lake City, FL 32055 18-0-0 5-9-0 22-4-9 31-0-0 4-2-13 Scale = 1:65,5 4x4 = 2 8.00 12 3x8 > 3x8 3 2x4 || 4x12 = 2x4 || 5x12 = 6 16 19 10 22 9 24 20 21 12 14 13 11 2x4 || 6x8 = 6x8 = 4x8 = 5x6 = 2x4 || 4x12 = 2x4 || 12-3-0 18-0-0 22-4-9 5-9-0 4-2-13 4-4-9 LOADING (psf) SPACING-2-0-0 CSI. DEFL l/defl L/d PLATES GRIP (loc) 20.0 Plate Grip DOL 1.25 TC 0.68 Vert(LL) -0.26 240 TCLL 11 >999 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.90 Vert(CT) -0.4611 180 >801 BCLL 0.0 Rep Stress Incr NO WB 0.83 Horz(CT) 0.08 8 n/a Code FBC2020/TPI2014 BCDL 10.0 Matrix-MS Weight: 228 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

end verticals.

1 Row at midpt

CANNON RES.

T30766143

Qty

LUMBER-

Job

TOP CHORD 2x4 SP No.2 2x6 SP No.2

BOT CHORD 2x4 SP No.3 WEBS

REACTIONS.

(size) 8=Mechanical, 15=0-3-8

Max Horz 15=-180(LC 9)

Truss

Max Uplift 8=-552(LC 9), 15=-334(LC 9) Max Grav 8=1940(LC 2), 15=1502(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-985/275, 2-3=-986/256, 3-4=-2233/480, 4-5=-4345/1107, 5-6=-4345/1107,

1-15=-1378/349

13-14=-332/1813, 11-13=-1063/4578, 10-11=-1061/4586, 9-10=-711/2564, 8-9=-711/2564 **BOT CHORD** WEBS

2-14=-177/788, 3-14=-1623/514, 3-13=-339/1645, 4-13=-3098/819, 4-10=-861/454,

Truss Type

5-10=-337/209, 6-10=-479/2162, 6-9=0/280, 6-8=-3074/848, 1-14=-244/1072

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=552, 15=334.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 105 lb down and 72 lb up at 22-4-12, 89 lb down and 73 lb up at 23-2-6, 89 lb down and 73 lb up at 25-2-6, and 89 lb down and 73 lb up at 27-2-6, and 89 lb down and 73 lb up at 29-2-6 on top chord, and 635 lb down and 144 lb up at 22-4-12, 42 lb down and 13 lb up at 23-2-6, 42 lb down and 13 lb up at 25-2-6, and 42 lb down and 13 lb up at 27-2-6, and 42 lb down and 13 lb up at 29-2-6 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-2=-54, 2-4=-54, 4-7=-54, 8-15=-20

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Structural wood sheathing directly applied or 2-6-2 oc purlins, except

3-14, 4-13, 6-8, 1-15

Rigid ceiling directly applied or 7-2-6 oc bracing.

Philip J. O'Regan PE. No. 58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

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Job	Truss	Truss Type	Qty	Ply	CANNON RES.
3499744	Т37	Roof Special Girder	1	8	T3076614 Job Reference (optional)
Builders FirstSource	, Lake City, FL 32055				nt: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:05 2023 Page 2 bWz8QrP-9YF6OerlAwxrgkTUC4AZmGmk0zNCYb2akfZWb0z83D8

LOAD CASE(S) Standard

Concentrated Loads (lb)
Vert: 5=-65(B) 10=-614(B) 16=-42(B) 17=-42(B) 18=-42(B) 19=-42(B) 22=-28(B) 23=-28(B) 24=-28(B) 25=-28(B)

Job Truss Truss Type CANNON RES T30766144 3499744 T38 Roof Special | Job Reference (optional)
| Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:05 2023 Page ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-9YF6OerlAwxrgkTUC4AZmGmmvzMpYbgakfZWb0z83D8 Builders FirstSource, Lake City, FL 32055 11-10-4 31-0-0 6-4-0 20-4-0 24-8-0 4x4 = Scale = 1:69.2 8.00 12 2 4x8 > 3x6 / 4x4 = 10-11-7 4x6 = 3x4 = 4-7-7 20 22 10 2x4 || 3x6 = 3x8 = 3x6 3x4 = 6x8 = 16-0-0 24-8-0 31-0-0 Plate Offsets (X,Y)--[6:0-2-4,0-2-4], [7:0-2-3,Edge] LOADING (psf) SPACING-CSI. 2-0-0 DEFL. (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.56 Vert(LL) -0.369-11 >999 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.99 -0.60Vert(CT) 9-11 >616 180 BCLL 0.0 Rep Stress Incr YES WB 0.79 Horz(CT) 0.06

BRACING-

TOP CHORD

BOT CHORD

WEBS

n/a

end verticals.

1 Row at midpt

n/a

Rigid ceiling directly applied or 2-2-0 oc bracing.

Weight: 191 lb

Structural wood sheathing directly applied or 3-3-6 oc purlins, except

3-11, 1-12

FT = 20%

LUMBER-

REACTIONS.

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 **WEBS** 2x4 SP No.3

(size) 7=Mechanical, 12=0-3-8

Max Horz 12=-280(LC 13) Max Uplift 7=-259(LC 13), 12=-264(LC 13) Max Grav 7=1243(LC 2), 12=1295(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-846/222, 2-3=-839/212, 3-4=-2868/600, 4-5=-2316/442, 5-6=-1529/376,

Code FBC2020/TPI2014

6-7=-1883/383, 1-12=-1195/277

BOT CHORD 11-12=-170/277, 9-11=-95/1300, 8-9=-318/2024, 7-8=-227/1503

WEBS 2-11=-119/637, 3-11=-1090/379, 3-9=-440/2067, 4-9=-1708/415, 5-9=-13/447,

5-8=-711/127, 6-8=-70/782, 1-11=-183/915

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-2-15, Interior(1) 3-2-15 to 6-6-0, Exterior(2R) 6-6-0 to 9-7-3, Interior(1) 9-7-3 to 24-8-0, Exterior(2R) 24-8-0 to 27-9-3, Interior(1) 27-9-3 to 31-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-MS

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

- 4) Provide adequate drainage to prevent water ponding.5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (|t=|b|) 7=259, 12=264.

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Philip J. O'Regan PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

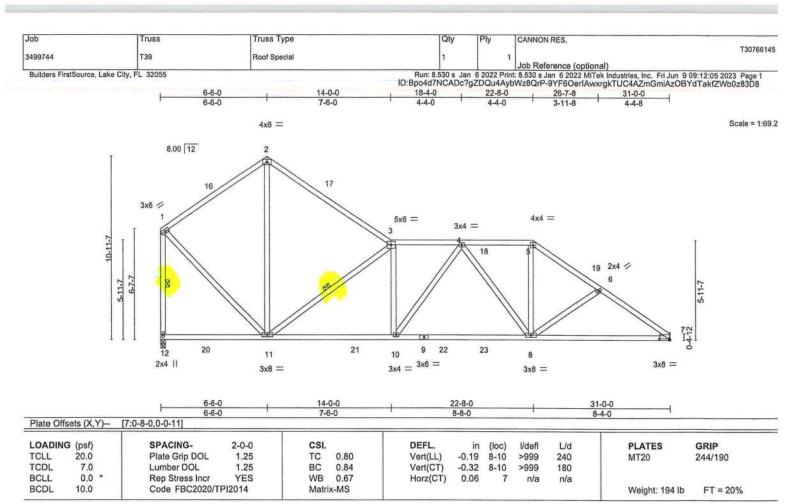
ers and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 REFORE USE

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BRACING-

TOP CHORD

BOT CHORD

WEBS

end verticals.

1 Row at midpt

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS.

(size) 7=Mechanical, 12=0-3-8

Max Horz 12=-280(LC 13) Max Uplift 7=-259(LC 13), 12=-264(LC 13) Max Grav 7=1257(LC 2), 12=1292(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-830/223, 2-3=-853/197, 3-4=-1785/376, 4-5=-1427/356, 5-6=-1752/382,

6-7=-1913/423, 1-12=-1169/279

BOT CHORD 11-12=-170/278, 10-11=-184/1781, 8-10=-209/1675, 7-8=-294/1575 WEBS

2-11=-64/554, 3-11=-1439/409, 4-8=-416/118, 5-8=-103/777, 6-8=-266/162,

1-11=-193/889

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-2-15, Interior(1) 3-2-15 to 6-6-0, Exterior(2R) 6-6-0 to 9-7-3, Interior(1) 9-7-3 to 22-8-0, Exterior(2R) 22-8-0 to 25-9-3, Interior(1) 25-9-3 to 31-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except ((t=lb) 7=259, 12=264.

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Philip J. O'Regan PE No. \$\$126 MITek Inc. DBA MITek USA FL Cert 6634 16923 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

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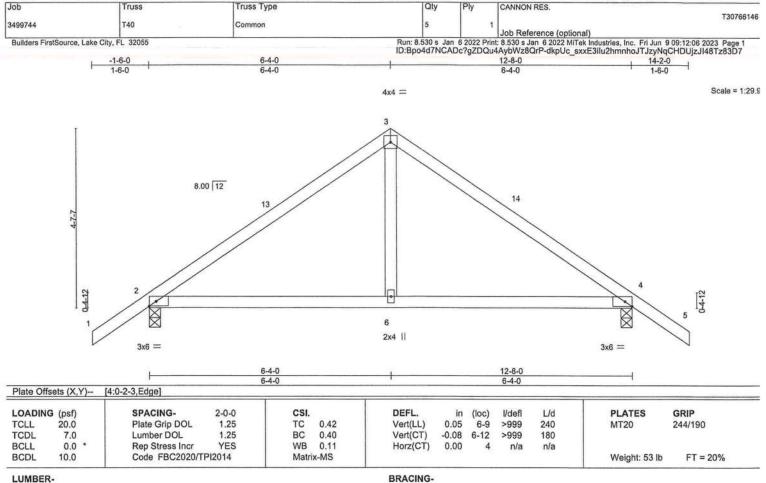
ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Structural wood sheathing directly applied or 2-2-0 oc purlins, except

3-11, 1-12

Rigid ceiling directly applied or 10-0-0 oc bracing.



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 2x4 SP No.3 WEBS

REACTIONS. (size) 2=0-3-8, 4=0-3-8 Max Horz 2=-115(LC 10)

Max Uplift 2=-126(LC 12), 4=-126(LC 13) Max Grav 2=550(LC 1), 4=550(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-553/156, 3-4=-553/156 **BOT CHORD** 2-6=-30/388, 4-6=-30/388

WEBS 3-6=0/291

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 6-4-0, Exterior(2R) 6-4-0 to 9-4-0, Interior(1) 9-4-0 to 14-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=126, 4=126.

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Philip J. O'Regan PE No.58126 MITek Iar. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

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Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Job Truss Type Qty CANNON RES. Truss T30766147 3499744 T40G Common Supported Gable | Job Reference (optional)
| Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:06 2023 Page 1 |
| ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-dkpUc_sxxE3ilu2hmnhoJTJ1uNwwHEgjzJl48Tz83D7 Builders FirstSource, Lake City, FL 32055 12-8-0 14-2-0 1-6-0 Scale = 1:27.5 4x4 = 8.00 12 0-4-12 16 15 14 13 12 4x8 || 4x8 11 Plate Offsets (X,Y)-- [2:0-3-8,Edge], [10:0-3-8,Edge] LOADING (psf) DEFL. PLATES GRIP SPACING-2-0-0 CSI. in l/defl L/d

Vert(LL)

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

-0.01

-0.01

0.00

11

11

10

120

120

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

n/r

n/r

n/a

2-0-0 oc purlins (6-0-0 max.).

LUMBER-

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 **OTHERS**

20.0

7.0

0.0

10.0

2x4 SP No.3

REACTIONS. All bearings 12-8-0. (lb) -Max Horz 2=-107(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 15, 16, 13, 12 All reactions 250 lb or less at joint(s) 2, 10, 14, 15, 16, 13, 12

1.25

1.25

YES

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

Plate Grip DOL

Rep Stress Incr

Code FBC2020/TPI2014

Lumber DOL

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-6-0 to 1-6-0, Exterior(2N) 1-6-0 to 6-4-0, Corner(3R) 6-4-0 to 9-4-0, Exterior(2N) 9-4-0 to 14-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip

TC

BC

WB

Matrix-S

0.16

0.04

0.04

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 15, 16, 13,
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 10.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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244/190

FT = 20%

MT20

Weight: 68 lb

Philip J. O'Regau PE No.58126 MITek Iuc. DBA MITek USA FL Ceri 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9.2023

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ANSITTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information

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Joh Truss Truss Type Qty CANNON RES. T30766148 3499744 T41 Common Girder | ' | Job Reference (optional) Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:06 2023 Page ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-dkpUc_sxxE3ilu2hmnhoJTJ?TNqYH5EjzJl48Tz83D7 Builders FirstSource, Lake City, FL 32055 -1-6-0 12-8-0 3-5-11 1-6-0 2-10-5 4x4 = Scale = 1:28 5 3x8 / 8.00 12 3x8 > 3 0-4-12 14 15 9 8 7 3x8 || 4x8 = 3x8 || 6x8 12-8-0 3-5-11 [2:0-1-14,Edge], [6:0-1-14,Edge], [7:0-6-0,0-1-8], [9:0-6-0,0-1-8] Plate Offsets (X,Y)--LOADING SPACING-DEFL CSL (psf) 2-0-0 in (loc) I/defi L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.32 Vert(LL) -0.04>999 240 MT20 244/190

-0.07

0.02

7-11

6

>999

n/a

180

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 3-3-10 oc purlins.

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x8 SP 2400F 2.0E 2x4 SP No.3

7.0

0.0

10.0

WEBS REACTIONS.

(size) 6=0-3-8, 2=0-3-8

Max Horz 2=108(LC 26)

Max Uplift 6=-590(LC 9), 2=-268(LC 8) Max Grav 6=2485(LC 2), 2=1013(LC 1)

Lumber DOL

Rep Stress Incr

Code FBC2020/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1437/360, 3-4=-1327/386, 4-5=-1329/384, 5-6=-2797/718 TOP CHORD **BOT CHORD** 2-9=-310/1164, 8-9=-310/1164, 7-8=-555/2324, 6-7=-555/2324

WEBS 4-8=-362/1278, 5-8=-1616/456, 5-7=-389/1679

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

BC

WB

Matrix-MS

0.45

0.64

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

NO

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=590, 2=268.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 144 lb down and 86 lb up at 7-0-12, and 810 lb down and 246 lb up at 9-0-12, and 1638 lb down and 341 lb up at 11-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 2-6=-20

Concentrated Loads (lb)

Vert: 7=-756(B) 14=-144(B) 15=-1480(B)

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FT = 20%

Weight: 85 lb

Philip J. O'Regas PE No.58126 MiTek Iuc. DBA MiTek USA FL Cert 5634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



3499744 T42 Common | Job Reference (optional)
| Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:06 2023 Page ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-dkpUc_sxxE3ilu2hmnhoJTJz5Nq9HDNjzJl48Tz83D7 Builders FirstSource, Lake City, FL 32055 7-0-0 -1-6-0 1-6-0 Scale = 1:31.1 4x6 = 2x4 || 8.00 12 0.4-12 5 3x6 = 3x4 / Plate Offsets (X,Y)--[2:0-1-5,0-1-8] PLATES GRIP LOADING (psf) DEFL SPACING-2-0-0 CSI in (loc) I/def L/d 244/190 Vert(LL) -0.075-8 >999 240 TCLL 20.0 Plate Grip DOL 1.25 TC 0.41 MT20 >531 180

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

-0.15

0.01

5-8

n/a

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 38 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

FT = 20%

Qty

CANNON RES.

LUMBER-

REACTIONS.

TCDL

BCLL

BCDL

Job

Truss

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

7.0

0.0

10.0

2x4 SP No.3 WEBS

(size) 2=0-3-8, 5=Mechanical

Max Horz 2=168(LC 12)

Max Uplift 2=-63(LC 12), 5=-98(LC 12)

Lumber DOL

Rep Stress Incr

Code FBC2020/TPI2014

Max Grav 2=343(LC 1), 5=248(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1.25

YES

WEBS 3-5=-329/325

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 6-4-0, Exterior(2E) 6-4-0 to 6-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

BC

WB

Matrix-MS

0.41

0.12

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.

Truss Type

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T30766149

Philip J. O'Regau PE No.58126 MITek Inc. DRA MITek USA FL Cort 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qtv CANNON RES T30766150 T43 3499744 Common | Job Reference (optional)
Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Frl Jun 9 09:12:07 2023 Page 1
ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-6xNtpKtZiYBYw1dtKVC1rhsBEn420WctCz2dgvz83D6 Builders FirstSource, Lake City, FL 32055 10-2-0 5-0-6 5-1-10 Scale = 1:54.3 4x4 = 8.00 12 3x4 > 5 3x4 || 3x4 II 13 14 15 16 17 10 6x8 = 5x12 = 6x8 = 10-2-0 Plate Offsets (X,Y)--[9:0-6-0,0-3-0] LOADING (psf) SPACING-2-0-0 CSI. DEFL L/d **PLATES** GRIP I/defl (loc) 20.0 Plate Grip DOL 1.25 0.25 0.27 9-10 240 TCLL TC Vert(LL) >891 MT20 244/190 -0.41 TCDL 7.0 Lumber DOL 1.25 BC 0.81 Vert(CT) 8-9 >584 180 BCLL 0.0 Rep Stress Incr YES WB 0.76 Horz(CT) 0.01 n/a n/a

BRACING-

TOP CHORD

BOT CHORD

end verticals

LUMBER-

BCDL

TOP CHORD 2x4 SP No.2 2x4 SP No.1 **BOT CHORD**

10.0

2x4 SP No.3 *Except* WEBS

2-10,6-8: 2x6 SP No.2

REACTIONS. (size) 10=0-3-8, 8=0-3-8

Max Horz 10=-243(LC 10)

Max Uplift 10=-174(LC 12), 8=-174(LC 13) Max Grav 10=902(LC 2), 8=902(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code FBC2020/TPI2014

TOP CHORD 3-4=-679/563, 4-5=-679/563, 2-10=-281/198, 6-8=-280/198

BOT CHORD 9-10=-351/591, 8-9=-333/523

WEBS 4-9=-506/468, 3-10=-660/383, 5-8=-660/383

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 10-2-0, Exterior(2R) 10-2-0 to 13-2-0, Interior(1) 13-2-0 to 21-10-0 zone; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-MS

- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=174, 8=174.

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Weight: 136 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

Rigid ceiling directly applied or 9-5-1 oc bracing.

FT = 20%

Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

WARNING - Varify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/ITP/11 Quality Criteria, DSB-89 and BCSI Building Components Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Type CANNON RES. Truss T30766151 T43G GABLE | Job Reference (optional)
| Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MITek Industries, Inc. Fri Jun 9 09:12:07 2023 Page 1
| D:Bpo4d7NCADc?gZDQu4AybWz8QrP-6xNtpKtZiYBYw1dtKVC1rhsA2n4N0UVtCz2dgvz83D6 Builders FirstSource, Lake City, FL 32055 20-4-0 1-6-0 15-2-6

> 8.00 12 4x4 / 6 3x4 / 3x4 > 3x4 > 3x4 / 5x12 || D 5x12 || 37 38 39 40 41 11 12 10

> > 5x12 =

BRACING-

TOP CHORD

BOT CHORD

4x4 =

Plate Offsets (X,Y)-- [11:0-6-0,0-3-0]

LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.27	Vert(LL)	0.31	10-11	>765	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.79	Vert(CT)	-0.40	10-11	>603	180	555500722	
BCLL	0.0	Rep Stress Incr	YES	WB	0.89	Horz(CT)	0.02	10	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MS	100000 MODEL M					Weight: 198 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.1

WEBS 2x4 SP No.3 *Except*

2-12,8-10: 2x6 SP No.2

OTHERS 2x4 SP No.3

REACTIONS.

(size) 12=0-3-8, 10=0-3-8

Max Horz 12=230(LC 11)

Max Uplift 12=-176(LC 12), 10=-176(LC 13) Max Grav 12=895(LC 2), 10=895(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-4=-200/297, 4-5=-698/716, 5-6=-698/716, 6-8=-200/297, 2-12=-294/301,

6x8

8-10=-294/301

BOT CHORD 11-12=-452/613, 10-11=-442/553

5-11=-647/484, 4-12=-645/461, 6-10=-645/461 **WEBS**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-6-0 to 1-6-0, Exterior(2N) 1-6-0 to 10-2-0, Corner(3R) 10-2-0 to 13-2-0, Exterior(2N) 13-2-0 to 21-10-0 zone; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=176, 10=176,
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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6x8 =

2-0-0 oc purlins (6-0-0 max.). except end verticals.

Rigid ceiling directly applied or 8-1-5 oc bracing.

Scale = 1:52.3

Philip J. O'Regan PE No.58126 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



CANNON RES. Job Truss Truss Type Qtv T30766152 Common Girder T44 3499744 | Z Job Reference (optional)
Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Frl Jun 9 09:12:08 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-a7xF1guBTrJPXBC3tCjGOu0L8BY3l?p0RdnACLz83D5 Builders FirstSource, Lake City, FL 32055 6-4-0 2-10-5 9-2-5 2-10-5 Scale = 1:27.7 4x6 || 3 8.00 12 3x6 / 3x6 < 13 14 15 8 6 3x8 || 8x10 = 3x8 || 4x8 3-5-11 2-10-5 Plate Offsets (X,Y)--[1:0-1-4,Edge], [5:0-1-4,Edge], [6:0-5-12,0-1-8], [7:0-5-0,0-5-8], [8:0-5-12,0-1-8] LOADING (psf) SPACING-2-0-0 CSI. DEFL l/defl L/d **PLATES** GRIP (loc) Plate Grip DOL -0.06 240 244/190 TCLL 20.0 1.25 TC 0.31 Vert(LL) >999 MT20 TCDL 7.0 Lumber DOL 1.25 BC 0.31 Vert(CT) -0.11 >999 180 0.0 WB 0.63 Horz(CT) 0.03 5 BCLL Rep Stress Incr NO BCDL 10.0 Code FBC2020/TPI2014 Matrix-MS Weight: 165 lb FT = 20% BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x8 SP 2400F 2.0E BOT CHORD 2x4 SP No.3 *Except* WEBS

3-7: 2x4 SP No.2

REACTIONS.

(size) 1=0-3-8, 5=0-3-8 (reg. 0-3-9)

Max Horz 1=93(LC 24)

Max Uplift 1=-953(LC 8), 5=-1088(LC 9) Max Grav 1=4617(LC 2), 5=5999(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-6884/1406, 2-3=-5247/1062, 3-4=-5248/1064, 4-5=-7301/1371

BOT CHORD 1-8=-1182/5710, 7-8=-1182/5710, 6-7=-1098/6060, 5-6=-1098/6060

WEBS 3-7=-1107/5617, 4-7=-2167/425, 4-6=-365/2368, 2-7=-1722/461, 2-8=-405/1868

NOTES-

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-7-0 oc. Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-6-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

- 4) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) WARNING: Required bearing size at joint(s) 5 greater than input bearing size.

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=953, 5=1088,
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1241 lb down and 288 lb up at 1-7-4, 1241 lb down and 288 lb up at 3-7-4, 1241 lb down and 288 lb up at 5-7-4, 1235 lb down and 251 lb up at 6-3-4, 1617 lb down and 280 lb up at 8-3-4, and 1625 lb down and 280 lb up at 10-3-4, and 1606 lb down and 274 lb up at 12-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

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Structural wood sheathing directly applied or 4-5-5 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Philip J. O'Regan PE No.58126 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

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ANSITYPII Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	CANNON RES.	
3499744	T44	Common Girder	1	2	Job Reference (optional)	152
Builders FirstSource, Lake City, FL 32055		Run: 8.530 s Jan	6 2022 Print	t: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:08 2023 Page 2		

Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:08 2023 Page 2 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-a7xF1guBTrJPXBC3tCjGOuOL8BY3l?p0RdnACLz83D5

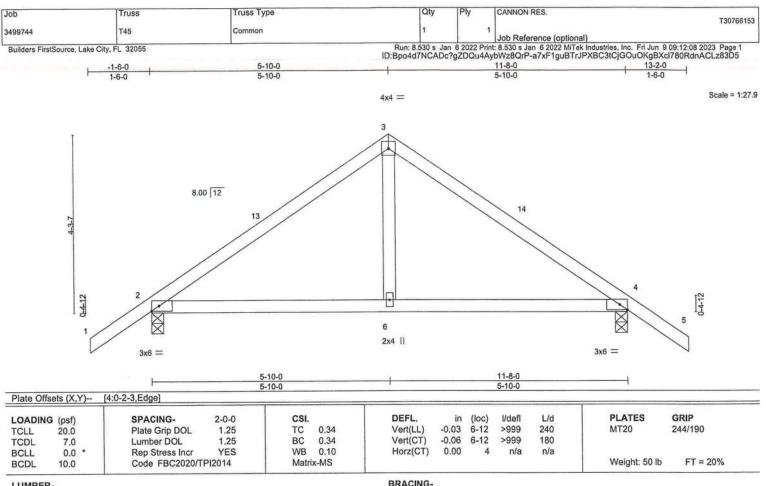
LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 1-5=-20

Concentrated Loads (lb)

Vert: 7=-1136(F) 8=-1129(F) 13=-1129(F) 14=-1129(F) 15=-1465(F) 16=-1465(F) 17=-1471(F)



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 WEBS 2x4 SP No.3

REACTIONS.

(size) 2=0-3-8, 4=0-3-8

Max Horz 2=108(LC 11) Max Uplift 2=-119(LC 12), 4=-119(LC 13) Max Grav 2=513(LC 1), 4=513(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-504/150, 3-4=-504/150 **BOT CHORD** 2-6=-25/355, 4-6=-25/355

WEBS 3-6=0/267

NOTES-

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 5-10-0, Exterior(2R) 5-10-0 to 8-10-0, Interior(1) 8-10-0 to 13-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=119, 4=119,

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Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Philip J. O'Regau PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

sters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Truss Type Qty CANNON RES Joh T30766154 T45G Common Supported Gable 3499744 Job Reference (optional) Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MITek Industries, Inc. Fri Jun 9 09:12:08 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-a7xF1guBTrJPXBC3tCjGOuONJBcQl890RdnACLz83D5 Builders FirstSource, Lake City, FL 32055 11-8-0 13-2-0 5-10-0 5-10-0 Scale = 1:26.3 4x4 = 7 2x4 || 8.00 12 2x4 || 4x6 || 4x6 || 0-4-12 XXXXX 14 13 12 16 15 4x8 || 4x8 11 2x4 || 2x4 || 2x4 || 2x4 || 2x4 || 11-8-0 [2:0-3-8,Edge], [3:0-1-5,0-2-4], [9:0-1-5,0-2-4], [10:0-3-8,Edge] Plate Offsets (X,Y)--DEFL PLATES GRIP SPACING-2-0-0 CSI. I/defl L/d LOADING (psf) (loc)

-0.01

-0.01

0.00

11

11

10

n/r

n/r

n/a

2-0-0 oc purlins (6-0-0 max.).

120

120

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

Vert(LL)

Vert(CT)

Horz(CT)

Rep Stress Incr 0.0 BCLL Code FBC2020/TPI2014

LUMBER-TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD** 2x4 SP No.3 **OTHERS**

20.0

7.0

10.0

BRACING-TOP CHORD **BOT CHORD**

TC

BC

WB

Matrix-S

0.17

0.03

0.04

All bearings 11-8-0.

Plate Grip DOL

Lumber DOL

Max Horz 2=-100(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 15, 16, 13, 12 Max Grav All reactions 250 lb or less at joint(s) 2, 10, 14, 15, 16, 13, 12

1.25

1.25

YES

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

REACTIONS.

TCLL

TCDL

BCDL

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-6-0 to 1-6-0, Exterior(2N) 1-6-0 to 5-10-0, Corner(3R) 5-10-0 to 8-10-0, Exterior(2N) 8-10-0 to 13-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 15, 16, 13,
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

This item has been electronically signed and sealed by ORegan, Philip, Pt using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

244/190

FT = 20%

MT20

Weight: 62 lb

Philip J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 9,2023

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ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.



Job Truss Truss Type Qty Ply CANNON RES T30766155 3499744 T46 Common Girder Run: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 Print: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:09 2023 Page 1 ID:Bpo4d7NCADc?gZDQu4AybWz8QrP-2JVdE0vpE9RG9LnGRvEVw6xYpbvTUQEAfGXkkoz83D4 Builders FirstSource, Lake City, FL 32055 11-8-0 1-6-0 Scale = 1:26.8 4x4 || 3x6 / 3x6 > 8.00 12 0-4-12 14 16 9 8 7 3x8 II 7x8 = 3x8 || 4x8 = 11-8-0 Plate Offsets (X,Y)--[2:0-4-0,0-1-9], [6:0-4-0,0-1-9], [7:0-5-8,0-1-8], [8:0-4-0,0-5-0], [9:0-5-8,0-1-8]

DEFL

Vert(LL)

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

(loc)

8 >999

6

-0.04

-0.07

0.02

l/def

>999 8

n/a

L/d

240

180

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

LOADING (psf)

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x8 SP 2400F 2.0E

2x4 SP No.3 WEBS

20.0

7.0

0.0

REACTIONS.

(size) 6=0-3-8. 2=0-3-8

Max Horz 2=100(LC 5) Max Uplift 6=-877(LC 9), 2=-691(LC 8) Max Grav 6=3712(LC 2), 2=2664(LC 2)

SPACING-

Plate Grip DOL

Rep Stress Incr

Code FBC2020/TPI2014

Lumber DOL

1.25

1.25

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-4462/1126, 3-4=-3726/947, 4-5=-3729/946, 5-6=-5116/1215

BOT CHORD 2-9=-943/3676, 8-9=-943/3676, 7-8=-970/4242, 6-7=-970/4242

WEBS 4-8=-981/3945, 5-8=-1470/374, 5-7=-319/1587, 3-8=-754/275, 3-9=-224/809

NOTES-

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-5-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

CSI.

TC

BC

WB

Matrix-MS

0.19

0.24

0.75

3) Unbalanced roof live loads have been considered for this design.

- 4) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except ((t=lb) 6=877, 2=691.

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1920 lb down and 572 lb up at 4-4-12, 1223 lb down and 279 lb up at 6-4-12, and 1237 lb down and 279 lb up at 8-4-12, and 1182 lb down and 289 lb up at 10-4-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 2-6=-20

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.38126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 9,2023

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PLATES

Weight: 157 lb

MT20

Structural wood sheathing directly applied or 5-5-12 oc purlins.

GRIP

244/190

FT = 20%

Job	Truss	Truss Type	Qty	Ply	CANNON RES.
3499744	T46	Common Girder	1	2	T30766155 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055					t: 8.530 s Jan 6 2022 MiTek Industries, Inc. Fri Jun 9 09:12:09 2023 Page 2 Nz8QrP-2JVdE0vpE9RG9LnGRvEVw6xYpbvTUQEAfGXkkoz83D4

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 7=-1122(F) 14=-1845(F) 15=-1122(F) 16=-1073(F)

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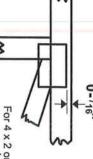


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y and fully embed teeth. Apply plates to both sides of truss offsets are indicated. Dimensions are in ft-in-sixteenths.



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

œ

6

5

connector plates. required direction of slots in This symbol indicates the

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE



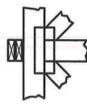
the length parallel to slots to slots. Second dimension is width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



if indicated. output. Use T or I bracing Indicated by symbol shown and/or by text in the bracing section of the

BEARING



Min size shown is for crushing only. number where bearings occur. Indicates location where bearings reaction section indicates joint (supports) occur. Icons vary but

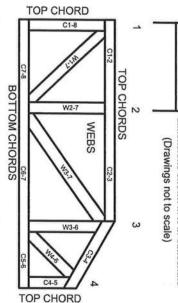
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.

DSB-89:

Guide to Good Practice for Handling, Connected Wood Trusses Installing & Bracing of Metal Plate Building Component Safety Information,

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

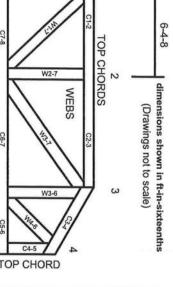
CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

ICC-ES Reports:

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values established by others. Lumber design values are in accordance with ANSI/TPI 1





PRODUCT CODE APPROVALS

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered
- Never exceed the design loading shown and never stack materials on inadequately braced trusses
- designer, erection supervisor, property owner and Provide copies of this truss design to the building all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each locations are regulated by ANSI/TPI 1. joint and embed fully. Knots and wane at joint
- the environment in accord with ANSI/TPI 1. Design assumes trusses will be suitably protected from
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection. responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.



Address Assignment and Maintenance Document

To maintain the county wide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for addressing and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Services Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County

Date/Time Issued:

6/7/2022 9:42:17 AM

Address:

495 SW JORDAN St

City:

FORT WHITE

State:

FL

Zip Code

32038

Parcel ID

04031-000

REMARKS: New address for Habitable structure (family home, business, etc.) on the parcel.

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION AND ACCESS INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION AND/OR ACCESS INFORMATION BE FOUND TO BE IN ERROR OR CHANGED, THIS ADDRESS IS SUBJECT TO CHANGE.

Address Issued By: SCHOFIELD, LINCOLN C.

Columbia County Department of Information Technology 135 NE Hernando Ave. Lake City, FL 32055 Telephone 386-719-1456

COLUMBIA COUNTY BUILDING DEPARTMENT 135 NE Hernando Ave., Suite B-21, Lake City, FL 32055 Office: 3

Office: 386-758-1008 Fax: 386-758-2160

www. columbia county fla. com/Building and Zoning. asp

NEW CONSTRUCTION RESIDENTIAL OR COMMERCIAL

Subn	nit Permit Applications Online at: https://www.columbiacountyfla.com/PermitSearch/MyBNZPortalLogin.a
	2 nd pg Permit Application with <i>PROPERTY OWNER'S Signature</i> & <i>Notarized</i> Contractor Signature + \$15.00
The D	eeded Property owner must sign page 2 of the Application. If the customer has a notarized Power of
Attor	ney from the Deeded Property Owner, then that named person can sign for the owner.
	1 -Notes:
\Box	Subcontractors Verification Form, signed by the license holder/contractor that is subcontracted the job.
_	2 -Notes:
	License Holders (Contractors) must complete a "Letter of Authorization" for who signs the permit.
	3 -Notes:
V	If an Owner Builder, Notarized Disclosure Statement (Owner Builders must sign for the Permit)
	4 -Notes:
	Recorded deed or Property Appraiser's parcel details printout; and if
	5 -Notes:
	Owner is Corporation or Trust, provide corporate articles listing the signor, trust executor or POA forms.
	6 -Notes:
	Site plan with actual distances of the structure to each property line
	8 -Notes:
	911 Address form, Phone 386-758-1125 #1 ALL CONSTRUCTION REQUIRES VERIFICATION
	9 -Notes:
	Residential or Commercial Checklist completed including Product Approval Code Spec sheet.
	10 -Notes:
	Recorded Notice of Commencement; before the 1st inspection.
	11 -Notes:
V	2 sets of plans (blueprints) folded to 9 x 12 size with Signed & Sealed Engineering
	13 -Notes:
	2 sets of Signed & Sealed truss engineering, if not included within the building blueprints
	45 -Notes:
V	2 sets of energy code & Manual J forms, if required.
	15 -Notes:
\prod	Provide information on Development Permits/Zoning Applications applied for, if applicable.
	16 -Notes:
Neede	ed AFTER Zoning Review and Approval has been allowed for this project.
	Approved and Signed Site Plan from Environmental Health on the septic 386-758-1058
	Notes:
Ц	New Wells need a letter from the well driller (Well Letter); or if on City Water provide City Water Letter;
	or if the property is in the Ellisville Water System area contact 386-719-7565 for review.
	Notes:

Applications mailed, include the \$15.00 fee, checks to BCC or Board of County Commissioners.



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Address Issued By: SCHOFIELD, LINCOLN C.



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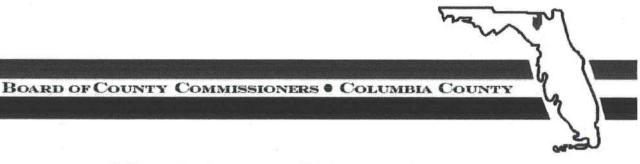
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FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Business and Professional Regulation - Residential Performance Method

		1 1010001011	arregulation - residential re	orientianee wearea
- 1. The file of the control of the	annon Residence 95 SW Jordan Street		Builder Name:	
	White, FL,		Permit Office: Columbia County Permit Number:	у
	onica Cannon		Jurisdiction:	
	., Gainesville			a Climate Zone 2)
d Name and the second	N. /F	DI	L 40 W. II T 40005 0 . 0 .	
New construction or	11 (10 × 10 × 10 × 10 × 10 × 10 × 10 × 1	rom Plans)	10. Wall Types (3085.3 sqft.) a. Frame - Wood, Exterior	Insulation Area R=13.0 2862.00 ft ²
Single family or mul		Detached	b. Frame - Wood, Exterior	R=13.0 223.33 ft ²
Number of units, if r	multiple family	1	c. N/A	10.0
Number of Bedroom	ns	5	d. N/A	
5. Is this a worst case?	?	No	11. Ceiling Types(3457.6 sqft.)	
6. Conditioned floor ar	ea above grade (ft²)	3293	a. Flat ceiling under att (Vented) b. N/A	R=38.0 3457.60 ft ²
Conditioned floor ar		0	c. N/A	
7. Windows(498.0 sqf	ft.) Description	Area	12. Roof(Comp. Shingles, Vented)) Deck R=0.0 3958 ft ²
a. U-Factor:	Dbl, U=0.36	498.00 ft ²	13. Ducts, location & insulation lev	
SHGC:	SHGC=0.25	6.2	a. Sup: Attic, Ret: Attic, AH: 1st F	Floor 6 823
b. U-Factor: SHGC:	N/A	ft²	b.	
c. U-Factor:	N/A	ft²	c. 14. Cooling Systems	kBtu/hr Efficiency
SHGC:		*	a. Central Unit	38.9 SEER:14.00
Area Weighted Avera		4.809 ft		
Area Weighted Average	The course was a second	0.250	45 11 11 0	15. " = "
8. Skylights	Description	Area	Heating Systems a. Electric Heat Pump	kBtu/hr Efficiency 49.4 HSPF:8.20
U-Factor:(AVG) SHGC(AVG):	N/A N/A	N/A ft ²	a. Electric Fleat Fulfip	49.4 HSFF.0.20
		A	- 1	
Floor Types Slab-On-Grade Ed	Insulation Insulation R= 0.0	Area 3293.00 ft ²	16. Hot Water Systems	
b. N/A	R=	ft ²	a. Electric	Cap: 50 gallons
c. N/A	R=	ft²	b. Conservation features	EF: 0.920
			b. Conservation reatures	None
			17. Credits	CV, Pstat
Glass/Floor Area: 0.151	Total Pr	oposed Modifie	ed Loads: 81.40	
Transcription (Control of the Control of the Contro		Total Baselin		PASS
I hereby certify that the	plans and specifications co	overed by	Review of the plans and	
A	ompliance with the Florida	Energy	specifications covered by this	STOR THE STATE OF
Code.	1).1190	M	calculation indicates compliance	
PREPARED BY:	WM C-+	I \mathcal{U}	with the Florida Energy Code. Before construction is completed	5 1111 6
			this building will be inspected for	Varia
DATE:	3 / 28 / 2023		compliance with Section 553.908	O A
			Florida Statutes.	
	building, as designed, is in	compliance	₩	COD WE TRU
with the Florida Energy	Code.		BUILDING OFFICIAL:	
DATE:		•	DATE:	
 Compliance requires 	s certification by the air h	andler unit ma	anufacturer that the air handler encl	losure qualifies as

- Compliance requires certification by the air handler unit manufacturer that the air handler enclosure qualifies as certified factory-sealed in accordance with R403.3.2.1.
- Default duct leakage does not require a Duct Leakage Test Report.
- Compliance requires an Air Barrier and Insulation Inspection Checklist in accordance with R402.4.1.1 and this project requires a PERFORMANCE envelope leakage test report with envelope leakage no greater than 5.00 ACH50 (R402.4.1.2).

				PRO	JECT								
Title: Building Type: Owner: Builder Name: Permit Office: Jurisdiction: Family Type: New/Existing: Year Construct: Comment:	Cannon Residence User Monica Cannon Columbia County Detached New (From Plans) 2023		Bedroom Condition Total Sto Worst Ca Rotate A Cross Ve Whole Ho Terrain: Shielding	ned Area ries: ase: ngle: entilation: ouse Fan	1 No 0 Yes		Lot #: Block/ PlatBo Street Count		n: 495 Colu	SW Journal of the second of th	ress rdan Stree	t	
				CLIN	/IATE								
/ Design Location		Tmy Site		Des 97.5%	sign Temp 6 2.5%		Design nter Su		Heatir Degree		Design Moisture		ily temp
FL, Gainesville		FL_GAINESVILLE_	REGIONA	32	92	7	0	75	1305.	5	51	Medi	um
				BLC	CKS								
Number	Name	Area	Vo	lume									
1	Block1	3293	329	930 cu ft			h						
				SPA	CES								3
Number	Name	Area	Volume	Kitchen	Occupan	ts	Bedro	oms	Finish	ed	Coole	d F	leated
1	1st Floor	3293	32930	Yes	10		5		Yes		Yes	Ř	Yes
				FLO	ORS		(To	otal Exp	osec	d Are	a = 329	93 sq	.ft.)
# Floor Typ	е	Space	Exposed	Perim	Perimeter R-	/alue	Area	U-Factor	Joist R	R-Value	Tile V	lood	Carpet
1 Slab-On-Gr	ade Edge Ins	1st Floor	318.66	7	0	119	3293 ft	0.304			0.00	0.00	1.00
				RO	OF								
/# Type		Materials		oof rea	Gable Roo Area Col		Rad Barr	Solar Absor.	SA rested	Emitt	Emitt Tested	Deck Insul.	Pitch (deg)
1 Hip	C	Composition shingle	s 39	58 ft²	0 ft² Medi	um	Υ	0.96	No	0.9	No	0	33.69
				AT	TIC								
/ _{# Type}		Ventilation		Vent I	Ratio (1 in)	Area	9	RBS		IRCC			
1 Partial cathe	edral ceiling	Vented			300	3293	ft²	Υ		N			
				CEIL	ING		(To	otal Exp	oseo	Area	a = 345	8 sq.	ft.)
# Ceiling Ty	ре	S	pace	R-V	alue Ins. T	/ре	Area	U-Fac	tor F	raming	Frac.	Truss	з Туре
									_				

							W	ALLS	3			(Tot	al Exp	osed	Area	= 308	5 sq.1	t.)
√# Ornt		acent Γο	Wall Type		Space)		avity -Value	Width Ft			eight In	Area sq.ft.		Sheath R-Valu		Solar Absor.	Belov Grad
1 N	3	Exterior	Frame - Wood	i		Floor		13.0	24.0	0	10.	0 6	252.0	0.084		0.23	0.75	0 %
2 N		Exterior	Frame - Wood			Floor		13.0	6.0	0	10.	0 0	60.0	0.084	100	0.23	0.75	0 %
3 E		Exterior	Frame - Wood	i	1st	Floor		13.0	4.0	4	10.	0 0	43.3	0.084	6)	0.23	0.75	0 %
4 N		Exterior	Frame - Wood	i		Floor		13.0	12.0	8	10.	0 0	126.7	0.084	107	0.23	0.75	0 %
5 W		Exterior	Frame - Wood	i	1st	Floor		13.0	6.0	0	10.	0 0	60.0			0.23	0.75	0 %
6 N		Exterior	Frame - Wood	i	1st	Floor		13.0	8.0	8	10.		86.7	0.084		0.23	0.75	0 %
7 E		Exterior	Frame - Wood	1	1st	Floor		13.0	4.0	0	10.		40.0			0.23	0.75	0 %
8 N		Exterior	Frame - Wood	1	1st	Floor		13.0	11.0	8	10.		116.7			0.23	0.75	0 %
9 W		Exterior	Frame - Wood	1	1st	Floor		13.0	12.0	8	10.		126.7			0.23	0.75	0 %
10 N		Exterior	Frame - Wood	1		Floor		13.0	12.0	8	10.		126.7			0.23	0.75	0 %
11 W		Exterior	Frame - Wood			Floor		13.0	22.0		10.		223.3			0.23	0.75	0 %
12 S		Garage	Frame - Wood			Floor		13.0	22.0		10.		223.3			0.23	0.75	0 %
13 S		Exterior	Frame - Wood			Floor		13.0	17.0		10.		173.3					
14 W		Exterior	Frame - Wood													0.23	0.75	0 %
15 S						Floor		13.0	9.0	4	10.		93.3	0.084		0.23	0.75	0 %
		Exterior	Frame - Wood			Floor		13.0		4	10.		133.3			0.23	0.75	0 %
16 E		Exterior	Frame - Wood			Floor		13.0		0	10.		100.0			0.23	0.75	0 %
17 S		Exterior	Frame - Wood			Floor		13.0	14.0	0	10.		140.0			0.23	0.75	0 %
18 W		Exterior	Frame - Wood			Floor		13.0	6.0	0	10.		60.0	0.084		0.23	0.75	0 %
19 S		Exterior	Frame - Wood			Floor		13.0		8	10.		166.7			0.23	0.75	0 %
20 E		Exterior	Frame - Wood		1st	Floor		13.0	48.0	8	10.	0 0	486.7	0.084		0.23	0.75	0 %
21 N		Exterior	Frame - Wood		1st	Floor		13.0	16.0	8	10.	0 0	166.7	0.084		0.23	0.75	0 %
22 W		Exterior	Frame - Wood		1st	Floor		13.0	8.0	0	10.	0 0	80.0	0.084		0.23	0.75	0 %
							DO	ORS	3			(T	otal E	xpose	d Are	a = 2	4 sq.f	t.)
√# Ornt		Adjacent	To Door Type		Space			Stor	ms		U-\	/alue		/idth t In		eight In	Are	а
1 S		5																
' 0		Garage	Inculated		1et Flo	or		NI	no.		-	0.46	2.00		0.00	_	24.0	612
		Garage	Insulated	111 - 234	1st Flo	ence.			ne		(0.46	3.00	0	8.00	0	24.0	ft²
		Garage	e Insulated		1st Flo	ence.	/IN	DOW			(3.00 tal Exp			-		
√# Ornt	Wall ID	Garage	Panes	NFRC	1st Flo	ence.		DOW		Sar	ne \		tal Exp		Area	-	8 sq.f	
√ # Ornt		Frame	Panes		U-Factor	SHGC	Imp	DOW Storm	Total Area (ft²)	Uni	me \interior	(To Width (ft)	tal Exp Height (ft)	Overh Depth (ft)	Area nang Sep. (ft)	= 49	8 sq.f	t.) Screen
/ # Ornt	ID 1	Frame	Panes Low-E Double	Y	U-Factor	SHGC	Imp N	Storm N	Total Area (ft²)	Uni 2	me \	(To Width (ft)	Height (ft)	Overh Depth (ft)	Area nang Sep. (ft)	= 49	8 sq.f	Screen
/ # Ornt 1 N 2 N	1 1	Frame TIM Vinyl	Panes Low-E Double Low-E Double	Y Y	U-Factor 0.36 0.36	SHGC 0.25 0.25	Imp N N	Storm N N	Total Area (ft²) 40.0 54.0	Uni	me \its	(To Width (ft)	Height (ft)	Overh Depth (ft) 8.5 8.5	Area nang Sep. (ft) 2.0 2.0	= 49	8 sq.f	Screen None
# Ornt 1 N 2 N 3 N	1 1 4	Frame TIM Vinyl Vinyl	Panes Low-E Double Low-E Double Low-E Double	Y Y Y	U-Factor 0.36 0.36 0.36	0.25 0.25 0.25 0.25	Imp N N	Storm N N N	Total Area (ft²) 40.0 54.0 24.0	2 3 1	me \interior	(To Width (ft) 2.50 3.00 4.00	Height (ft) 8.00 6.00 6.00	Overh Depth (ft) 8.5 8.5 1.0	Area pang Sep. (ft) 2.0 2.0 3.0	= 49	8 sq.f	Screen None None None
# Ornt 1 N 2 N 3 N 4 N	1 1 4 6	TIM Vinyl Vinyl Vinyl	Panes Low-E Double Low-E Double Low-E Double Low-E Double	Y Y Y	U-Factor 0.36 0.36 0.36 0.36	0.25 0.25 0.25 0.25 0.25	Imp N N N	Storm N N N N	Total Area (ft²) 40.0 54.0 24.0 12.0	Uni 2	me \its	(To Width (ft) 2.50 3.00 4.00 2.00	Height (ft) 8.00 6.00 6.00 6.00	Overh Depth (ft) 8.5 8.5 1.0 1.5	Area nang Sep. (ft) 2.0 2.0 3.0 1.0	= 49	8 sq.f	Screen None None None None None
# Ornt 1 N 2 N 3 N 4 N 5 N	1 1 4 6 8	Frame TIM Vinyl Vinyl Vinyl Vinyl Vinyl	Panes Low-E Double Low-E Double Low-E Double Low-E Double Low-E Double	Y Y Y Y	U-Factor 0.36 0.36 0.36 0.36 0.36	0.25 0.25 0.25 0.25 0.25 0.25	Imp N N N N	Storm N N N N N N N N N N N N N N N N N N	Total Area (ft²) 40.0 54.0 24.0 12.0 24.0	2 3 1	me \its	(To Width (ft) 2.50 3.00 4.00 2.00 4.00	Height (ft) 8.00 6.00 6.00 6.00 6.00	Overh Depth (ft) 8.5 8.5 1.0 1.5	Area nang Sep. (ft) 2.0 2.0 3.0 1.0 3.0	Non Non Non Non Non Non	8 sq.f	None None None None None None
# Ornt 1 N 2 N 3 N 4 N 5 N 6 N	1 1 4 6 8 10	TIM Vinyl Vinyl Vinyl Vinyl Vinyl	Panes Low-E Double Low-E Double Low-E Double Low-E Double Low-E Double Low-E Double	Y Y Y Y	U-Factor 0.36 0.36 0.36 0.36 0.36 0.36	0.25 0.25 0.25 0.25 0.25 0.25	Imp N N N N N N N	Storm N N N N N N N N N N N N N N N N N N	Total Area (ft²) 40.0 54.0 24.0 12.0 24.0 24.0	2 3 1 1 1	me \its	(To Width (ft) 2.50 3.00 4.00 2.00 4.00 4.00	Height (ft) 8.00 6.00 6.00 6.00 6.00 6.00	Overh Depth (ft) 8.5 8.5 1.0 1.5 1.0	Area lang- Sep. (ft) 2.0 2.0 3.0 1.0 3.0 1.0	Non Non Non Non Non Non Non Non	8 sq.f	None None None None None None None
# Ornt 1 N 2 N 3 N 4 N 5 N 6 N 7 W	1 1 4 6 8 10 11	Frame TIM Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl	Panes Low-E Double	Y Y Y Y Y	U-Factor 0.36 0.36 0.36 0.36 0.36 0.36 0.36	0.25 0.25 0.25 0.25 0.25 0.25 0.25	Imp N N N N N N N N N N N N N N N N N N N	Storm N N N N N N N N N N N N N N N N N N	Total Area (ft²) 40.0 54.0 24.0 12.0 24.0 4.0	2 3 1 1 1 1	me Nits	(To Width (ft) 2.50 3.00 4.00 2.00 4.00 4.00 4.00	Height (ft) 8.00 6.00 6.00 6.00 6.00 6.00 1.00	Overh Depth (ft) 8.5 8.5 1.0 1.5 1.0 1.5	Area tang- Sep. (ft) 2.0 2.0 3.0 1.0 3.0 1.0 1.0	Non Non Non Non Non Non Non Non Non Non	8 sq.f	None None None None None None None
# Ornt 1 N 2 N 3 N 4 N 5 N 6 N 7 W 8 W	1 1 4 6 8 10 11 11	Frame TIM Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl	Panes Low-E Double	Y Y Y Y Y Y	U-Factor 0.36 0.36 0.36 0.36 0.36 0.36 0.36 0.3	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25	Imp ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	Storm N N N N N N N N N N N N N N N N N N	Total Area (ft²)	2 3 1 1 1 1 1	me \its	(To Width (ft) 2.50 3.00 4.00 2.00 4.00 4.00 4.00 2.00	Height (ft) 8.00 6.00 6.00 6.00 6.00 6.00 4.00	Overh Depth (ft) 8.5 8.5 1.0 1.5 1.5 1.5	Area tang Sep. (ft) 2.0 2.0 3.0 1.0 3.0 1.0 1.0 1.0	Non Non Non Non Non Non Non Non Non Non	8 sq.f	None None None None None None None None
# Ornt 1 N 2 N 3 N 4 N 5 N 6 N 7 W 8 W 9 S	1 1 4 6 8 10 11 11 13	Frame TIM Vinyl	Panes Low-E Double	Y Y Y Y Y Y	U-Factor 0.36 0.36 0.36 0.36 0.36 0.36 0.36 0.3	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25	Imp ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	Storm N N N N N N N N N N N N N N N N N N	Total Area (ft²) 40.0 54.0 24.0 12.0 24.0 4.0	2 3 1 1 1 1 1 1 2	me \text{its}	(To Width (ft) 2.50 3.00 4.00 2.00 4.00 4.00 4.00 2.00 3.00	Height (ft) 8.00 6.00 6.00 6.00 6.00 1.00 4.00 5.00	Overh Depth (ft) 8.5 8.5 1.0 1.5 1.5 1.5	Area tang- Sep. (ft) 2.0 2.0 3.0 1.0 3.0 1.0 1.0	Non Non Non Non Non Non Non Non Non Non	8 sq.f	None None None None None None None
# Ornt 1 N 2 N 3 N 4 N 5 N 6 N 7 W 8 W 9 S 10S	1 1 4 6 8 10 11 11 13 13	Frame TIM Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl TIM	Panes Low-E Double	Y Y Y Y Y Y Y	U-Factor 0.36 0.36 0.36 0.36 0.36 0.36 0.36 0.3	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25	Imp ZZZZZZZZZZZZZZ	Storm N N N N N N N N N N N N N N N N N N	Total Area (ft²)	2 3 1 1 1 1 1 2 2	me \its	(To Width (ft) 2.50 3.00 4.00 2.00 4.00 4.00 4.00 2.00 3.00 3.00 3.00	Height (ft) 8.00 6.00 6.00 6.00 6.00 6.00 4.00 5.00 8.00	Overh Depth (ft) 8.5 8.5 1.0 1.5 1.0 1.5 1.5 1.5	Area tang Sep. (ft) 2.0 2.0 3.0 1.0 3.0 1.0 1.0 1.0	Non Non Non Non Non Non Non Non Non Non	8 sq.f	None None None None None None None None
# Ornt 1 N 2 N 3 N 4 N 5 N 6 N 7 W 8 W 9 S 10S 11S	1 1 4 6 8 10 11 11 13 13 15	Frame TIM Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl TIM Vinyl	Panes Low-E Double	Y Y Y Y Y Y Y Y	U-Factor 0.36 0.36 0.36 0.36 0.36 0.36 0.36 0.3	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25	Imp ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	Storm N N N N N N N N N N N N N N N N N N	Total Area (ft²)	2 3 1 1 1 1 1 1 2	me \its	(To Width (ft) 2.50 3.00 4.00 2.00 4.00 4.00 4.00 2.00 3.00	Height (ft) 8.00 6.00 6.00 6.00 6.00 1.00 4.00 5.00	Overh Depth (ft) 8.5 8.5 1.0 1.5 1.0 1.5 1.5 1.5 1.5	Area sang Sep. (ft) 2.0 2.0 3.0 1.0 3.0 1.0 1.0 1.0 1.0	Non Non Non Non Non Non Non Non Non Non	8 sq.f	None None None None None None None None
# Ornt 1 N 2 N 3 N 4 N 5 N 6 N 7 W 8 W 9 S 10S 11S 12S	1 1 4 6 8 10 11 11 13 13 15 17	Frame TIM Vinyl	Panes Low-E Double	Y Y Y Y Y Y Y Y Y	U-Factor 0.36 0.36 0.36 0.36 0.36 0.36 0.36 0.3	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25	Imp ZZZZZZZZZZZZZZ	Storm N N N N N N N N N N N N N N N N N N	Total Area (ft²)	2 3 1 1 1 1 1 2 2	me \its	(To Width (ft) 2.50 3.00 4.00 2.00 4.00 4.00 4.00 2.00 3.00 3.00 3.00	Height (ft) 8.00 6.00 6.00 6.00 6.00 6.00 4.00 5.00 8.00	Overh Depth (ft) 8.5 8.5 1.0 1.5 1.0 1.5 1.5 1.5 1.5 1.5	Area sang Sep. (ft) 2.0 2.0 3.0 1.0 3.0 1.0 1.0 1.0 1.0 1.0 1.0	Non Non Non Non Non Non Non Non Non Non	8 sq.f	None None None None None None None None
# Ornt 1 N 2 N 3 N 4 N 5 N 6 N 7 W 8 W 9 S 10S 11S 12S 13S	1 1 4 6 8 10 11 11 13 13 15	Frame TIM Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl TIM Vinyl	Panes Low-E Double	Y Y Y Y Y Y Y Y	U-Factor 0.36 0.36 0.36 0.36 0.36 0.36 0.36 0.3	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25	Imp ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	Storm N N N N N N N N N N N N N N N N N N	Total Area (ft²) 40.0 54.0 24.0 12.0 24.0 4.0 8.0 30.0 48.0 54.0	Uni 2 3 1 1 1 1 1 2 2 3	me Mits	(To Width (ft) 2.50 3.00 4.00 4.00 4.00 4.00 2.00 3.00 3.00 3.00	Height (ft) 8.00 6.00 6.00 6.00 6.00 6.00 4.00 5.00 8.00 6.00	Overh Depth (ft) 8.5 8.5 1.0 1.5 1.0 1.5 1.5 1.5 10.5 10.5 1.5 7.5	Area sang Sep. (ft) 2.0 2.0 3.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0	Non Non Non Non Non Non Non Non Non Non	8 sq.f	None None None None None None None None
# Ornt 1 N 2 N 3 N 4 N 5 N 6 N 7 W 8 W 9 S 10S 11S 12S 13S 14E	1 1 4 6 8 10 11 11 13 13 15 17	Frame TIM Vinyl	Panes Low-E Double	Y Y Y Y Y Y Y Y Y	U-Factor 0.36 0.36 0.36 0.36 0.36 0.36 0.36 0.3	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25	Imp N N N N N N N N N N N N N N N N N N N	Storm N N N N N N N N N N N N N	Total Area (ft²) 40.0 54.0 24.0 12.0 24.0 4.0 8.0 30.0 48.0 54.0 54.0	Uni 2 3 1 1 1 1 1 2 2 3 3	me Mits	(To Width (ft) 2.50 3.00 4.00 2.00 4.00 4.00 4.00 2.00 3.00 3.00 3.00 3.00 3.00	Height (ft) 8.00 6.00 6.00 6.00 6.00 4.00 5.00 8.00 6.00 6.00 6.00 6.00 6.00	Overh Depth (ft) 8.5 8.5 1.0 1.5 1.0 1.5 1.5 10.5 10.5 1.5 1.5 1.5	Area sang Sep. (ft) 2.0 2.0 3.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1	Non Non Non Non Non Non Non Non Non Non	8 sq.f	None None None None None None None None
# Ornt 1 N 2 N 3 N 4 N 5 N 6 N 7 W 8 W 9 S 10S 11S 12S 13S	1 1 4 6 8 10 11 11 13 13 15 17	Frame TIM Vinyl	Panes Low-E Double	Y Y Y Y Y Y Y Y Y Y	U-Factor 0.36 0.36 0.36 0.36 0.36 0.36 0.36 0.3	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25	Imp	Storm N N N N N N N N N N N N N	Total Area (ft²) 40.0 54.0 24.0 12.0 24.0 24.0 30.0 4.0 8.0 30.0 54.0 54.0 54.0 20.0	2 3 1 1 1 1 1 2 2 3 3 3	me \text{its}	(To Width (ft) 2.50 3.00 4.00 2.00 4.00 4.00 4.00 2.00 3.00 3.00 3.00 3.00 3.00 2.00	Height (ft) 8.00 6.00 6.00 6.00 6.00 4.00 5.00 6.00 6.00 6.00 6.00 6.00 6.00 6	Overh (ft) 8.5 8.5 1.0 1.5 1.0 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5	Area eang Sep. (ft) 2.0 2.0 3.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1	Nor Nor Nor Nor Nor Nor Nor Nor Nor Nor	8 sq.f	None None None None None None None None
# Ornt 1 N 2 N 3 N 4 N 5 N 6 N 7 W 8 W 9 S 10S 11S 12S 13S 14E 15E	1 1 4 6 8 10 11 11 13 13 15 17 19 20 20	Frame TIM Vinyl	Panes Low-E Double	Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y	U-Factor 0.36 0.36 0.36 0.36 0.36 0.36 0.36 0.3	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25	Imp ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	Storm N N N N N N N N N N N N N	Total Area (ft²) 40.0 54.0 24.0 12.0 24.0 30.0 44.0 8.0 54.0 54.0 54.0 20.0 20.0	2 3 1 1 1 1 1 2 2 3 3 3	me \text{its}	(To Width (ft) 2.50 3.00 4.00 2.00 4.00 4.00 2.00 3.00 3.00 3.00 3.00 3.00 4.00	Height (ft) 8.00 6.00 6.00 6.00 6.00 4.00 5.00 6.00 6.00 6.00 5.00 5.00 5.00	Overh (ft) 8.5 8.5 1.0 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5	Area sang Sep. (ft) 2.0 2.0 3.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1	Nor Nor Nor Nor Nor Nor Nor Nor Nor Nor	8 sq.f	None None None None None None None None
# Ornt 1 N 2 N 3 N 4 N 5 N 6 N 7 W 8 W 9 S 10S 11S 12S 13S 14E	1 1 4 6 8 10 11 11 13 13 15 17 19 20	Frame TIM Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl Vinyl TIM Vinyl Vinyl	Panes Low-E Double	Y Y Y Y Y Y Y Y Y Y Y Y	U-Factor 0.36 0.36 0.36 0.36 0.36 0.36 0.36 0.3	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25	Imp ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	Storm N N N N N N N N N N N N N	Total Area (ft²) 40.0 54.0 24.0 12.0 24.0 30.0 4.0 8.0 54.0 54.0 54.0 54.0 20.0	2 3 1 1 1 1 1 2 2 3 3 3	me !	(To Width (ft) 2.50 3.00 4.00 2.00 4.00 4.00 4.00 2.00 3.00 3.00 3.00 3.00 3.00 2.00	Height (ft) 8.00 6.00 6.00 6.00 6.00 4.00 5.00 6.00 6.00 6.00 6.00 6.00 6.00 6	Overh (ft) 8.5 8.5 1.0 1.5 1.0 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5	Area eang Sep. (ft) 2.0 2.0 3.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1	Nor Nor Nor Nor Nor Nor Nor Nor Nor Nor	8 sq.f	None None None None None None None None

FORM R405-2020

				INF	ILTRATI	ON		2.0			
/#	Scope	Method	SLA	CFM50	ELA	EqLA	ACH	ACH50	Space(s)	Infiltrat	ion Test Volu
_ 1	Wholehouse	Proposed ACH(50)	0.00032	2744	150.55	282.65	0.1071	5.0	All	32930	cu ft
,				G	SARAGE						
/#	Floor	Area	Roof Area	Exp	osed Wall Pe	rimeter	A	vg. Wall Heig	ht E	Exposed Wa	II Insulation
_1	491	l ft²	491 ft²		66 ft			10 ft		1	4
					MASS						
#	Mass Type	4	Area	67	Thickness		Furniture F	raction	Space	е	
_1	Default(8 lbs/s	q.ft.)	0 ft²		O ft		0.30		1st Fl	oor	
				HEAT	ING SYS	STEM					
#	System Type		Subtype/Speed	AHRI	# Efficie		Capacity kBtu/hr E	Geother Entry Pow			ucts Block
_1	Electric Heat F	ump	None/Single		HSPF:	8.20	49.4	0.0	0.00	0.00 sy	rs#1 1
				COOL	ING SYS	STEM					
#	System Type		Subtype/Speed	AHRI	# Effic	iency	Capac kBtu/h		Flow	SHR D	ouct Block
1	Central Unit		None/Single		SEEF	R:14.0	38.9	1	170	0.70 sy	s#1 1
				HOT W	ATER SY	STE	VI				E
/#	System Type	Subtype	Location	EF(L	JEF) Cap	Us	e SetPr	nt Fixture	Flow Pi	ipe Ins.	Pipe length
_ 1	Electric	None	1st Floor	0.92 (0.92) 50.00 g	al 40 g	jal 120 de	eg Stand	lard I	None	12
	Recirculation System	Recirc Contro Type		oop Brai	7.000 PM		HR Facil Conn			WHR (Other Credits
_1	No			NA N	A NA	No	N	A NA	N N	IA	None
					DUCTS						
/Duc				n Value Area	a Leakage	Туре	Air Handler	CFM 25 TOT	CFM 25 OUT	QN RLF	HVAC #
_ 1,4	Attic	6.0 823 ft ² Attic		6.0 165 ft²	Default Le	akage	1st Floor	(Default) ([Default)		1 1
	an es salitani			TEMP	ERATU	RES					
Progr Cooli Heati		stat: Y [] Feb [] Ma [X] Feb [X] Ma		Ceiling [] May [] May	Fans: N [X] Jun [] Jun	[X] Jul [] Jul	[X] Aug	[X] Sep	[] Oct	[] Nov [X] Nov	

FORM R405-2020

TEMPERATURES(Continued)													
/ Thermostat Schedu	ule: HERS 2	2006 Refer	rence				Но	ours					
✓ Schedule Type		1	2	3	4	5	6	7	8	9	10	11	12
Cooling (WD)	AM PM	78 80	78 80	78 78	78 78	78 78	78 78	78 78	78 78	80 78	80 78	80 78	80
Cooling (WEH)	AM PM	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78
Heating (WD)	AM PM	66 68	66 68	66 68	66 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	68 66
Heating (WEH)	AM PM	66 68	66 68	66 68	66 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	68 66

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD ESTIMATED ENERGY PERFORMANCE INDEX* = 97

The lower the EnergyPerformance Index, the more efficient the home.

495 SW Jordan Street, Ft White, FL,

1.	New construction or ex	isting	New (Fro	om Plans)		Wall Types (3085.3 sqft.)	Insulation	n	Area
2.	Single family or multiple	e family		Detached		Frame - Wood, Exterior	R=13.0		.00 ft ²
3.	Number of units, if mult	tiple family		1		Frame - Wood, Adjacent	R=13.0	223.	.33 ft ²
4.	Number of Bedrooms			5	700	. N/A			
5.	Is this a worst case?			No		Ceiling Types(3457.6 sqft.)	Insulation	n	Area
6.	Conditioned floor area Conditioned floor area			3293 0	b.	Flat ceiling under att (Vented) N/A N/A	R=38.0	3457.	60 ft ²
	Windows** a. U-Factor: SHGC:	Description Dbl, U=0.36 SHGC=0.25		Area 498.00 ft ²	12. 13.	Roof(Comp. Shingles, Vented) Do Ducts, location & insulation level Sup: Attic, Ret: Attic, AH: 1st Floor		39 R 6	58 ft ² ft ² 823
b	o. U-Factor: SHGC:	N/A		ft²	b. c.		Ų	U	023
-	. U-Factor: SHGC:	N/A		ft²		Cooling Systems Central Unit	kBtu/hr 38.9	Effic SEER:	iency 14.00
	Area Weighted Average of Area Weighted Average		oth:	4.809 ft 0.250					
	Skylights U-Factor:(AVG) SHGC(AVG):	Description N/A N/A		Area N/A ft²		Heating Systems Electric Heat Pump	kBtu/hr 49.4	Effic HSPF	iency :8.20
a	Floor Types . Slab-On-Grade Edge . N/A	Insulation R:		Area 3293.00 ft ² ft ²		Hot Water Systems Electric	Сар	: 50 ga	allons 0.920
С	. N/A	R:	=	ft ²	b.	Conservation features			
					17.	Credits			None Pstat

I certify that this home has complied with the Florida Energy Efficiency Code for Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: 495 SW Jordan Street

City/FL Zip: Ft White,FL,



*Note: This is not a Building Energy Rating. If your Index is below 70, your home may qualify for energy efficient mortgage (EEM) incentives if you obtain a Florida Energy Rating. For information about the Florida Building Code, Energy Conservation, contact the Florida Building Commission's support staff.

**Label required by Section R303.1.3 of the Florida Building Code, Energy Conservation, if not DEFAULT.

Envelope Leakage Test Report (Blower Door Test) Residential Prescriptive, Performance or ERI Method Compliance 2020 Florida Building Code, Energy Conservation, 7th Edition

Jurisdiction:	Permit #	t	
Job Information			
Builder: C	community:	Lot:	NA
Address: 495 SW Jordan Street			
City: Ft White	State: FL	Zip:	2.7
Air Leakage Test Results Passing	results must meet either the Perf	ormance, Prescriptive,	or ERI Method
PRESCRIPTIVE METHOD-The building or dwe changes per hour at a pressure of 0.2 inch w.g. PERFORMANCE or ERI METHOD-The buildin the selected ACH(50) value, as shown on Form R405	(50 Pascals) in Climate Zones 1 and g or dwelling unit shall be tested and v	2. verified as having an air le	akage rate of not exceeding
ACH(50) specified on Form R4 x 60 ÷ 32930 Building Volume PASS When ACH(50) is less than 3, Mechanust be verified by building departments be verified by building departments be conducted by either individuals as defined as a specified by building departments be conducted by either individuals as defined as a specified by building departments be verified by building departments be veri	anical Ventilation installation ent. ACH(50) A	Method for calculation Retrieved from Code software Field measure and reported at a pressure Statuesor individuals lice hall be signed by the party ations of the uilding thermal wond the intended weather ed, but not sealed beyond	ating building volume: n architectural plans e calculated ad and calculated e of 0.2 inch w.g. (50 Pascals). ensed as set forth in Section or conducting the test and d envelope.
Exterior doors for continuous ventilation systems and 5. Heating and cooling systems, if installed at the time 6. Supply and return registers, if installed at the time of Testing Company	of the test, shall be turned off.	sed and sealed.	
Company Name:	Ph	one:	
I hereby verify that the above Air Leakage resu Energy Conservation requirements according to	Its are in accordance with the 20	20 7th Edition Florida I	Building Code
Signature of Tester:	Da	ite of Test:	
Printed Name of Tester:			
License/Certification #:	Issuing Au	thority:	

Residential System Sizing Calculation

Summary

Monica Cannon 495 SW Jordan Street Ft White, FL Project Title: Cannon Residence

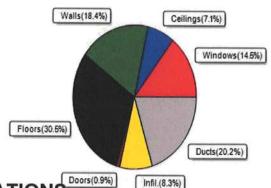
3/28/2023

Location for weather data: Gaine	sville, FL -	Defaults:	Latitude(29.7) Altitude(152 ft.) Ter	mp Range(N	1)				
Humidity data: Interior RH (50%	6) Outdoo	r wet bulb (77F) Humidity difference(51gr.)						
Winter design temperature(TMY3 99%) 30 F Summer design temperature(TMY3 99%) 94 F									
Winter setpoint	70	F	Summer setpoint	75					
Winter temperature difference	40	F	Summer temperature difference	19	F				
Total heating load calculation	49380	Btuh	Total cooling load calculation	38852	Btuh				
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh				
Total (Electric Heat Pump)	100.0	49380	Sensible (SHR = 0.70)	85.3	27196				
Heat Pump + Auxiliary(0.0kW)	100.0	49380	Latent	167.7	11656				
	3.4500.0000.0000		Total (Electric Heat Pump)	100.0	38852				

WINTER CALCULATIONS

Winter Heating Load (for 3293 soft)

Load componen	it		Load	
Window total	498	sqft	7171	Btuh
Wall total	2563	sqft	9101	Btuh
Door total	24	sqft	442	Btuh
Ceiling total	3458	sqft	3510	Btuh
Floor total	3293	sqft	15041	Btuh
Infiltration	94	cfm	4119	Btuh
Duct loss			9996	Btuh
Subtotal			49380	Btuh
Ventilation	Ex:0 cfm; Sup:0) cfm	0	Btuh
TOTAL HEAT L	OSS		49380	Btuh

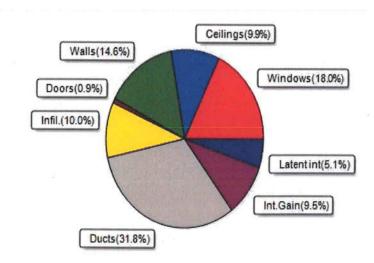


SUMMER CALCULATIONS

Summer Cooling Load (for 3293 sqft)

8th Edition

Load component	***************************************		Load	
Window total	498	sqft	6997	Btuh
Wall total	2563	sqft	5687	Btuh
Door total	24	sqft	331	Btuh
Ceiling total	3458	sqft	3861	Btuh
Floor total			0	Btuh
Infiltration	71	cfm	1467	Btuh
Internal gain			3700	Btuh
Duct gain			9856	Btuh
Sens.Ventilation	Ex:0 cfm; Sup:0	cfm	0	Btuh
Blower Load			0	Btuh
Total sensible gai	n		31900	Btuh
Latent gain(ducts)			2517	Btuh
Latent gain(infiltrati	on)	A.	2435	Btuh
Latent gain(ventilat	ion)	UNI	On	Btuh
Latent gain(interna	l/occupants/othe	1	Reviezago	Blub
Latent gain(ventilat Latent gain(interna Total latent gain	6.0		for6952	But
TOTAL HEAT GAI	N is		38852	Bruh
	131	161	a Cor)Vã:



EnergyGauge® System Sizing PREPARED BY:

DATE: 3 / 28 / 2023

Exa@herryGauge® / USRCZB v7.5.00

Code

Compliance

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Monica Cannon 495 SW Jordan Street Ft White, FL Project Title: Cannon Residence Building Type: User

3/28/2023

Reference City: Gainesville, FL (Defaults) Winter Temperature Difference: 40.0 °F (TMY3 99%) Winter Setpoint: 70 °F (Required Manual J default)

Component Loads for Whole House

Window	Panes/Type	Frame	U	Orientation	Area(sqft) X	HTM=	Load
1	2, NFRC 0.25	TIM	0.36	N	40.0	14.4	576 Btuh
2	2, NFRC 0.25	Vinyl	0.36	N	54.0	14.4	778 Btuh
3	2, NFRC 0.25	Vinyl	0.36	N	24.0	14.4	346 Btuh
4	2, NFRC 0.25	Vinyl	0.36	N	12.0	14.4	173 Btuh
5	2, NFRC 0.25	Vinyl	0.36	N	24.0	14.4	346 Btuh
6	2, NFRC 0.25	Vinyl	0.36	N	24.0	14.4	346 Btuh
7	2, NFRC 0.25	Vinyl	0.36	W	4.0	14.4	58 Btuh
8	2, NFRC 0.25	Vinyl	0.36	W	8.0	14.4	115 Btuh
9	2, NFRC 0.25	Vinyl	0.36	S	30.0	14.4	432 Btuh
10	2, NFRC 0.25	TIM	0.36	S S	48.0	14.4	691 Btuh
11	2, NFRC 0.25	Vinyl	0.36	S	54.0	14.4	778 Btuh
12	2, NFRC 0.25	Vinyl	0.36		54.0	14.4	778 Btuh
13	2, NFRC 0.25	Vinyl	0.36	S	54.0	14.4	778 Btuh
14	2, NFRC 0.25	Vinyl	0.36	S E E	20.0	14.4	288 Btuh
15	2, NFRC 0.25	Vinyl	0.36	E	20.0	14.4	288 Btuh
16	2, NFRC 0.25	Vinyl	0.36	E	4.0	14.4	58 Btuh
17	2, NFRC 0.25	Vinyl	0.36	N	24.0	14.4	346 Btuh
	Window Total				498.0(sqft)		7171 Btuh
Walls	Туре	Ornt. Ue	eff.	R-Value	Area X	HTM=	Load
		11.2237 (V 9025) I	1400000000	(Cav/Sh)		Van Control	
1	Frame - Wood	- Ext (0.		13.0/0.0	158	3.55	561 Btuh
2	Frame - Wood		089)	13.0/0.0	60	3.55	213 Btuh
3	Frame - Wood		089)	13.0/0.0	43	3.55	154 Btuh
4	Frame - Wood		089)	13.0/0.0	103	3.55	365 Btuh
5	Frame - Wood		089)	13.0/0.0	60	3.55	213 Btuh
6	Frame - Wood		089)	13.0/0.0	75	3.55	265 Btuh
7	Frame - Wood		089)	13.0/0.0	40	3.55	142 Btuh
8	Frame - Wood		089)	13.0/0.0	93	3.55	329 Btuh
9	Frame - Wood		089)	13.0/0.0	127	3.55	450 Btuh
10	Frame - Wood	7.0	089)	13.0/0.0	103	3.55	365 Btuh
11	Frame - Wood	1.00	089)	13.0/0.0	211	3.55	750 Btuh
12	Frame - Wood	The second secon	089)	13.0/0.0	199	3.55	708 Btuh
13	Frame - Wood		089)	13.0/0.0	95	3.55	338 Btuh
14	Frame - Wood		089)	13.0/0.0	93	3.55	331 Btuh
15 16	Frame - Wood		089)	13.0/0.0	79	3.55	282 Btuh
16 17	Frame - Wood	UNIVERSITY CONTRACTOR	089)	13.0/0.0	100	3.55	355 Btuh
17 18	Frame - Wood	- Ext (0.		13.0/0.0	86	3.55	305 Btuh
	Frame - Wood	- Ext (0.0		13.0/0.0	60	3.55	213 Btuh
19	Frame - Wood	- Ext (0.0		13.0/0.0	113	3.55	400 Btuh
20	Frame - Wood	- Ext (0.0		13.0/0.0	443	3.55	1572 Btuh
21 22	Frame - Wood	- Ext (0.0		13.0/0.0	143	3.55	507 Btuh
22	Frame - Wood	- Ext (0.0		13.0/0.0	80	3.55	284 Btuh
	Wall Total		Energy	Gauge® / USRC	ZB v2500(sqft)		9101 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Monica Cannon 495 SW Jordan Street Ft White, FL Project Title: Cannon Residence Building Type: User

3/28/2023

Doors		torm Ueff.		Area X	HTM=	Load
1	Insulated - Garage,	n (0.460)		24	18.4	442 Btuh
	Door Total			24(sqft)	-	442Btuh
Ceilings	Type/Color/Surface	Ueff.	R-Value	Area X	HTM=	Load
1	Flat ceil/M/Shing	(0.025)	38.0/0.0	3458	1.0	3510 Btuh
	Ceiling Total	6000 0000		3458(sqft)	3510Btuh
Floors	Туре	Ueff.	R-Value	Size X	HTM=	Load
1	Slab On Grade	(1.180)	0.0	318.7 ft(p	erim.) 47.2	15041 Btuh
	Floor Total			3293 sqft		15041 Btuh
		11		Envelope Sub	ototal:	35265 Btuh
Infiltration	Type V	Vholehouse AC	CH Volume	(cuft) Wall R	atio CFM=	
	Natural	0.1	3293	0 1.0	00 94.1	4119 Btuh
Duct load	Average sealed, R6.	0, Supply(Att)	Return(Att) (DL	M of 0.254)	9996 Btuh
All Zones	-		Sensible	Subtotal All	Zones	49380 Btuh

WHOLE HOUSE TOTALS

Totals for Heating Subtotal Sensible Heat Loss Ventilation Sens. Heat Loss Total Heat Loss	(Ex:0 cfm; Sup:0 cfm)	49380 Btuh 0 Btuh 49380 Btuh
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EQUIPMENT

1. Electric Heat Pump	#	49380 Btuh
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Key: Window types - NFRC (Requires U-Factor and Shading coefficient(SHGC) of glass as numerical values) or - Glass as 'Clear' or 'Tint' (Uses U-Factor and SHGC defaults) U - (Window U-Factor)

HTM - (ManualJ Heat Transfer Multiplier)



Version 8

System Sizing Calculations - Summer

Residential Load - Whole House Component Details Project Title:

Monica Cannon 495 SW Jordan Street Ft White, FL

Cannon Residence

3/28/2023

Reference City: Gainesville, FL (Defaults)

Humidity difference: 51gr.

Temperature Difference: 19.0F(TMY3 99%)

Summer Setpoint: 75 °F (Required Manual J default)

Component Loads for Whole House

	Type* Overhang W		Wind	dow Area(sqft)		НТМ		Load						
Window	Panes SHG	72.5	InSh	IS	Ornt	Len	Hgt			CONTROL OF COMME		Unshaded		
1	2 NFRC 0.25,		No	No	N	8.5ft.	2.0ft.	40.0	0.0	40.0	12	12	484	Btul
2	2 NFRC 0.25,	0.36	No	No	N	8.5ft.	2.0ft.	54.0	0.0	54.0	12	12	653	
3	2 NFRC 0.25,	0.36	No	No	N	1.0ft.	3.0ft.	24.0	0.0	24.0	12	12	290	
4	2 NFRC 0.25,	0.36	No	No	N	1.5ft.	1.0ft.	12.0	0.0	12.0	12	12	145	
5	2 NFRC 0.25,	0.36	No	No	N	1.0ft.	3.0ft.	24.0	0.0	24.0	12	12	290	
6	2 NFRC 0.25,	0.36	No	No	N	1.5ft.	1.0ft.	24.0	0.0	24.0	12	12	290	
7	2 NFRC 0.25,	0.36	No	No	W	1.5ft.	1.0ft.	4.0	1.0	3.0	12	31	105	
8	2 NFRC 0.25,	0.36	No	No	W	1.5ft.	1.0ft.	8.0	0.5	7.5	12	31	238	
9	2 NFRC 0.25,		No	No	S	10.5f	1.0ft.	30.0	30.0	0.0	12	14	363	
10	2 NFRC 0.25,	0.36	No	No	S	10.5f	1.0ft.	48.0	48.0	0.0	12	14	581	
11	2 NFRC 0.25,	0.36	No	No	S	1.5ft.	1.0ft.	54.0	54.0	0.0	12	14	653	
12	2 NFRC 0.25,		No	No	S	7.5ft.	1.0ft.	54.0	54.0	0.0	12	14	653	
13	2 NFRC 0.25,		No	No	S	1.5ft.	1.0ft.	54.0	54.0	0.0	12	14	653	
14	2 NFRC 0.25,		No	No	E	1.5ft.	1.0ft.	20.0	1.0	19.0	12	31	600	
15	2 NFRC 0.25,		No	No	E	1.5ft.	1.0ft.	20.0	1.0	19.0	12	31	600	
16	2 NFRC 0.25,		No	No	E	1.5ft.	1.0ft.	4.0	1.0	3.0	12	31	105	
17	2 NFRC 0.25,			No	N	1.0ft.	3.0ft.	24.0	0.0	24.0	12	12	290	
5626	Window Tot		(10.000)	11.70			0.0	498 (s		21.0			6997	
Walls	Туре				U.	-Value	R-V	/alue	Area	(saft)		НТМ	Load	-
							Cav/S			. ,				
1	Frame - Wood	- Ext			(0.09	13.0	/0.0	15	8.0		2.3	358	Btu
2	Frame - Wood	- Ext				0.09	13.0		60			2.3	136	Btu
3	Frame - Wood	- Ext				0.09	13.0			3.3		2.3	98	Btu
4	Frame - Wood	- Ext				0.09	13.0			2.7		2.3	232	
5	Frame - Wood					0.09	13.0		60			2.3	136	Btu
6	Frame - Wood	- Ext				0.09	13.0		74			2.3	169	Btu
7	Frame - Wood					0.09	13.0		40			2.3	91	Btu
8	Frame - Wood					0.09	13.0		92			2.3	210	
9	Frame - Wood					0.09	13.0		120			2.3	287	
10	Frame - Wood					0.09	13.0		102			2.3	232	Btu
11	Frame - Wood					0.09	13.0		21			2.3	478	Btu
12	Frame - Wood					0.09	13.0		199			1.7	336	Btu
13	Frame - Wood	0.00				0.09	13.0		95			2.3	216	Btu
14	Frame - Wood	2720				0.09	13.0		93			2.3	211	Btu
15	Frame - Wood					0.09	13.0		79			2.3	180	Btu
16	Frame - Wood					0.09	13.0		100			2.3	226	Btu
17	Frame - Wood -					0.09	13.0		86			2.3	195	Btu
18	Frame - Wood -					.09	13.0/		60			2.3	136	Btu
19	Frame - Wood -					0.09	13.0/		112			2.3	255	Btu
20	Frame - Wood -					.09	13.0/		442			2.3	1002	Btu
21	Frame - Wood -					.09	13.0/		142			2.3	323	
22	Frame - Wood -					.09	13.0/		80			2.3		Btu
~~	Wall Total	LAL			·	.03	13.0/	0.0		.0 3 (sqft)		2.3	181 5687	Btu
Doors	Туре	-							Area			НТМ	Load	Diu
1	Insulated - Gara	200							24					D4.
	Door Total	age								.0 4 (sqft)		13.8	331	
Ceilings	Type/Color/s	Surf	ace		П	Value		R-Value				НТМ	331	Dlu
1	Vented Attic/Me			R		0.025			345	N 250 2500			Load	Divis
- 15			ilgierk		1	0.025	3	8.0/0.0				1.12	3861	
	Ceiling Total	L							345	8 (sqft)			3861	Btul

Manual J Summer Calculations

Residential Load - Component Details (continued)

Project Title: Climate:FL_GAINESVILLE_REGIONAL_A

Monica Cannon 495 SW Jordan Street Ft White, FL

Cannon Residence

3/28/2023

Floors	Туре	R-V	alue	Size		НТМ	Load	
1	Slab On Grade	9	0.0	3293	(ft-perimeter)	0.0	0	Btuh
	Floor Total			3293.0 ((sqft)		0	Btuh
				Enve	elope Subt	otal:	16877	Btuh
Infiltration	Туре	Average ACH	Volume	e(cuft) V	Vall Ratio	CFM=	Load	
	Natural	0.13	3	2930	1	70.5	1467	Btuh
Internal		Occupants	Bt	uh/occu	pant	Appliance	Load	
gain		10	X	230	+	1400	3700	Btuh
	y - 4			Sens	sible Envel	ope Load:	22044	Btuh
Duct load	Average sealed,Supply(R	t6.0-Attic), Return(R6.0-Attic)			(DGM of 0	0.447)	9856	Btuh
				Sensil	ole Load A	II Zones	31900	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Project Title: Climate:FL_GAINESVILLE_F

Monica Cannon 495 SW Jordan Street Ft White, FL

Cannon Residence

Climate:FL_GAINESVILLE_REGIONAL A

3/28/2023

WHOLE HOUSE TOTALS

			a billing
	Sensible Envelope Load All Zones	22044	Btuh
	Sensible Duct Load	9856	Btuh
	Total Sensible Zone Loads	31900	Btuh
	Sensible ventilation (Ex:0 cfm; Sup:0 cfm)	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	31900	Btuh
Totals for Cooling	Latent infiltration gain (for 51 gr. humidity difference)	2435	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	2517	Btuh
	Latent occupant gain (10.0 people @ 200 Btuh per person)	2000	Btuh
	Latent other gain	0	Btuh
	Latent total gain	6952	Btuh
i t	TOTAL GAIN	38852	Btuh

EQUIPMENT		
1. Central Unit	#	38852 Btuh

*Key: Window types (Panes - Number and type of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value) (U - Window U-Factor)

(InSh - Interior shading device: none(No), Blinds(B), Draperies(D) or Roller Shades(R))

- For Blinds: Assume medium color, half closed

For Draperies: Assume medium weave, half closed

For Roller shades: Assume translucent, half closed

(IS - Insect screen: none(N), Full(F) or Half(1/2))

(Ornt - compass orientation)



Version 8