Quote

W 100	ID. ildava	Sold To: Cash Account		Ship Date
1	Builders	Lake City, FI	JobNo. L250981	Quote 8/16/2007
Hilly,	Eirot Course	386-755-6894	P.T.	Account
10000	FirstSource		Sales No	PO
Plant:	Lake City Truss	SubDvsn: Ruby Park (Dan Magstadt)	Req'rd Engineering	Req'rd Layouts
	2525 E Duval Street	Lot: 1		
	Lake City, FL	Model/Elev Custom		
	386-755-6894	Options		
		Directions		
	Sales Rep Robert Daniel		Job Contacts	Site Office
	Sales Area		Name	
	Dist Center		Phone	386-755-6894
	Designer avm		Fax	386-755-7973

1 SET OF TRUSSES PER PLANS SALES TAX NOT INCLUDED

Accepted By Seller	Accepted By Buyer			
	Purchaser:		Truss Pkg:	\$1,400.00
By:	By: Title	e:	Tax:	\$0.00
Title:			Shipping:	\$0.00
Date Of Acceptance:	Address:		Total Price:	\$0.00
	Phone: Dat	e:		

- 1. All valleys to be conventionally framed.
- 2. Price based on premise that plans are structurally sound.
- 3. Bracing material to be supplied by contractor.
- Builders FirstSource will not be liable for back charges unless approved by representative before the work creating any charge is performed.
- 5. Builders FirstSource reserves the right to adjust price as deemed necessary after 30 days from date of estimate. Price is not to be assumed as valid if plan is repeated at a later date. New quote must be requested for advance knowledge of price unless other written contract is agreed upon for specific plan to be repeatedly fabricated at same price during specified time period.
- Bid is null and void if final plans deviate from original. Any changes to engineering is subject for rebid.
- Some walls may need to be made load-bearing, to be determined when truss details are engineered.
- 8. Jobs canceled or in house longer than 90 days are subject to a minimum of a 10% engineering fee.
- 9. Sales tax is not included.



Project Information for:

L250981

Lot:

1

Subdivision:

Ruby Park Columbia

County: Truss Count:

16

Design Program: MiTek 20/20 6.3 Building Code: FBC2004/TPI2002 **Truss Design Load Information:**

Gravity:

Wi

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

Robert Stewart Florida License No. CBC1252898

Address: P.O. Box 3001 Lake City, Florida

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

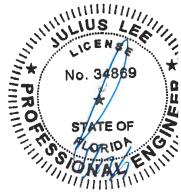
Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

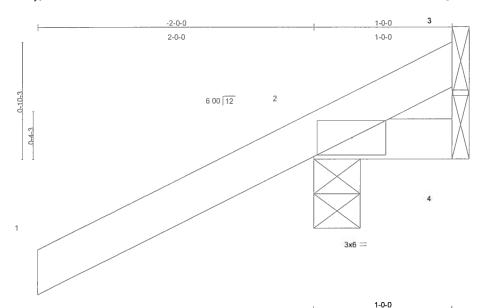
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elements in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1883175	CJ1	8/27/07
2	J1883176	CJ3	8/27/07
3	J1883177	CJ5	8/27/07
4	J1883178	EJ5	8/27/07
5	J1883179	EJ7	8/27/07
6	J1883180	HJ7	8/27/07
7	J1883181	HJ9	8/27/07
8	J1883182	T01	8/27/07
9	J1883183	T02	8/27/07
10	J1883184	T03	8/27/07
11	J1883185	T04	8/27/07
12	J1883186	T05	8/27/07
13	J1883187	T06	8/27/07
14	J1883188	T07	8/27/07
15	J1883189	T07G	8/27/07
16	J1883190	T08G	8/27/07



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883175
L250981	CJ1	JACK	6	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:17:56 2007 Page 1



1-0-0 **DEFL PLATES GRIP** LOADING (psf) **SPACING** 2-0-0 CSI (loc) I/defl L/d in TC 0.28 360 MT20 244/190 **TCLL** 20.0 Plates Increase 1.25 Vert(LL) -0.00 2 >999 2 BC >999 240 TCDL 7.0 Lumber Increase 1.25 0.01 Vert(TL) -0.00 YES **WB** 0.00 Horz(TL) 0.00 3 **BCLL** 10.0 Rep Stress Incr n/a n/a Code FBC2004/TPI2002 **BCDL** 5.0 (Matrix) Weight: 7 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=257/0-4-0, 4=5/Mechanical, 3=-91/Mechanical

Max Horz 2=87(load case 6)

Max Uplift 2=-287(load case 6), 4=-9(load case 4), 3=-91(load case 1) Max Grav 2=257(load case 1), 4=14(load case 2), 3=128(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-69/76

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2, 9 lb uplift at joint 4 and 91 lb uplift at joint 3. Continued on page 2

Truse Coston Engineer Plonas Pa No. 24866 1406 Chastal Bay Blyn Goynton Geson, FL Gosod

August 27,2007

Scale 1.5"≈1"

🛦 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an Individual building component that is installed and loaded vertically and fabricated with MITek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or IHB-91 Handling Installing and Bracing Recommendation audible from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883175
L250981	CJ1	JACK	6	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:17:56 2007 Page 2

LOAD CASE(S) Standard

Julius Les Trues Cosign Engineer Florida PE Ma. 3-1808) 100 Cassid Pay Rive Bourton Bases L. 20435



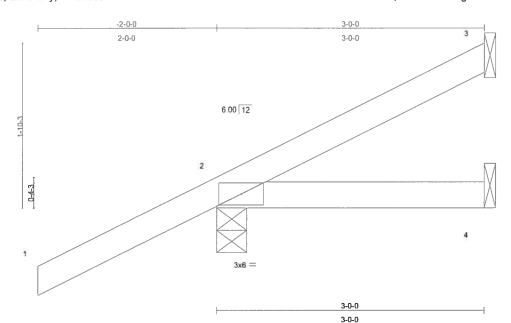
 Job
 Truss
 Truss Type
 Qty
 Ply
 CASH ACCOUNT - RUBY PARK LOT 1

 L250981
 CJ3
 JACK
 6
 1

 Job Reference (optional)
 Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:17:56 2007 Page 1



LOADING	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 13 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=251/0-4-0, 4=14/Mechanical

Max Horz 2=132(load case 6)

Max Uplift 3=-27(load case 7), 2=-240(load case 6), 4=-26(load case 4)

Max Grav 3=29(load case 1), 2=251(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-58/7

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.13

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 240 lb uplift at joint 2 and 26 lb uplift at joint 4. Continued on page 2

Julius Lees Trues Coston Chaineon Plonds PE No. 3-1800 1400 Castel Say Sivid Boyaton Weson. FL 50430

August 27,2007

Scale = 1 12.5

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters show for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation authorities the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883176
L250981	CJ3	JACK	6	1	
					Job Reference (optional)
Builders FirstSou	rce, Lake City, Fl	32055 6.	300 s Feb 15 2006 M	iTek Ind	lustries, Inc. Wed Aug 22 09:17:56 2007 Page 2

LOAD CASE(S) Standard

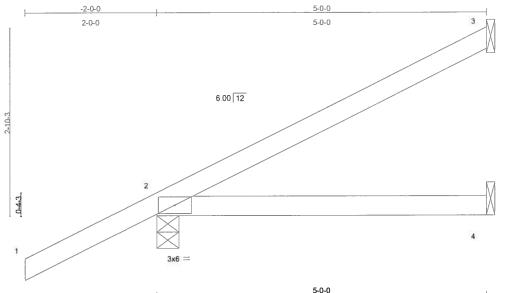


 Job
 Truss
 Truss Type
 Qty
 Ply
 CASH ACCOUNT - RUBY PARK LOT 1

 L250981
 CJ5
 JACK
 6
 1

 Builders FirstSource, Lake City, Fl 32055
 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:17:57 2007 Page 1

Builders FirstSource, Lake City, Fi 32055



LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0 SPACING Plates Increase Lumber Increase Rep Stress Code FBC	rease 1.25	CSI TC BC WB (Mat	0.30 0.24 0.00 rix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in 0.09 -0.05 -0.00	(loc) 2-4 2-4 3	l/defl >671 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 19 lb	GRIP 244/190
---	------------	-------------------------------	------------------------------	--	------------------------------	--------------------------	-------------------------------	--------------------------	---------------------------------	---------------------

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

5-0-0

Structural wood sheathing directly applied or

5-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=296/0-4-0, 4=24/Mechanical

Max Horz 2=178(load case 6)

Max Uplift 3=-86(load case 6), 2=-261(load case 6), 4=-46(load case 4)

Max Grav 3=102(load case 1), 2=296(load case 1), 4=72(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-87/36

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.15

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3, 261 lb uplift at joint 2 and 46 lb uplift at joint 4. Continued on page 2

Julius Lew Truse Coelon (Indineer Plands ME No. 3-1865 1406 Cassial Ray Rive Boynton Wesch, IL Surist Boynton Wesch, IL Surist

August 27,2007

Scale = 1 16 9

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or IRIB-97 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



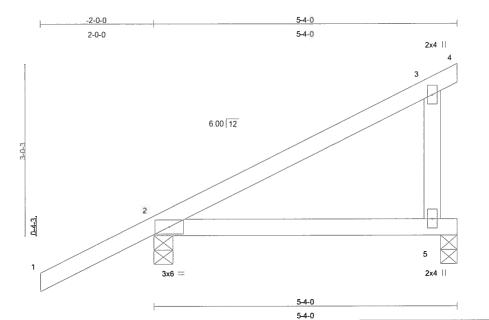
Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883177
L250981	CJ5	JACK	6	1	
					Job Reference (optional)
Builders FirstSource	, Lake City, Fl 32055	6.300 s Feb 1	5 2006 N	iTek Inc	dustries, Inc. Wed Aug 22 09:17:57 2007 Page 2

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
		ļ			J1883178
L250981	EJ5	MONO TRUSS	4	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:17:57 2007 Page 1



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.08	2-5	>691	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.23	Vert(TL)	-0.05	2-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 24 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 **WEBS**

BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-4-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

REACTIONS (lb/size) 2=294/0-4-0, 5=149/0-3-8

Max Horz 2=185(load case 6)

Max Uplift 2=-255(load case 6), 5=-149(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-106/36, 3-4=-11/0

BOT CHORD

2-5=0/0

WEBS

3-5=-126/187

JOINT STRESS INDEX

2 = 0.15, 3 = 0.10 and 5 = 0.10

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 255 Ib uplift at joint 2 and 149 lb uplift at joint 5. Continued on page 2

August 27,2007

Scale = 1:19.5

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-I1 or HIB-91 Handfing Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883178
L250981	EJ5	MONO TRUSS	4	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:17:57 2007 Page 2

LOAD CASE(S) Standard



CASH ACCOUNT - RUBY PARK LOT 1 Job Truss Truss Type Qty Ply J1883179 L250981 EJ7 MONO TRUSS 6 Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:17:58 2007 Page 1

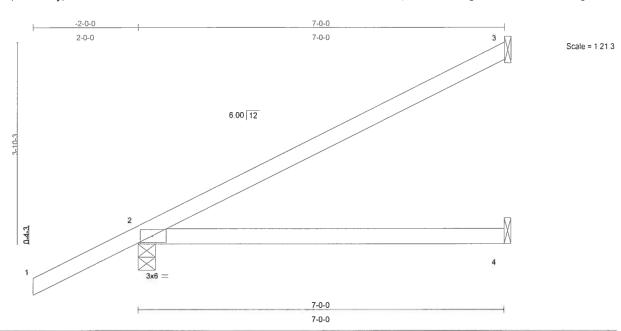


Plate Of	fsets (X,Y): [2:0-2-12,0-1-8]					***************************************				,	
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.08	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.16	2-4	>506	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 26 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-4-0, 4=44/Mechanical

Max Horz 2=161(load case 6)

Max Uplift 3=-84(load case 6), 2=-140(load case 6)

Max Grav 3=154(load case 1), 2=352(load case 1), 4=93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-119/54

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.70

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb Complited is in pagend 140 lb uplift at joint 2.

August 27,2007

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occes. For general guidance regarding storage, delivery, erection and bracing, consult BCS-I or HIB-91 Handring Installing and Bracing Recommendation avoide from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
				-	J1883179
L250981	EJ7	MONO TRUSS	6	1	
					Job Reference (optional)
Builders FirstSource	e, Lake City, FI 32055		6.300 s Feb 15 2006 M	iTek Inc	lustries, Inc. Wed Aug 22 09:17:58 2007 Page 2

LOAD CASE(S) Standard



Qty Ply CASH ACCOUNT - RUBY PARK LOT 1 Job Truss Truss Type J1883180 MONO TRUSS L250981 HJ7 1 1 Job Reference (optional) 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:17:58 2007 Page 1 Builders FirstSource, Lake City, Fl 32055 -2-9-15 7-6-8 7-6-8 2-9-15 Scale = 1 19 8 3x6 || 4.24 12 3x6 |1 7-6-8 7,8,0 0-1-8 7-6-8

LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr	2-0-0 1.25 1.25 NO	CSI TC BC WB	0.61 0.18 0.00	DEFL Vert(LL) Vert(TL) Horz(TL)	in 0.06 -0.08 0.00	(loc) 2-6 2-6 6	I/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCLL 10.0 BCDL 5.0	* Rep Stress Incr Code FBC2004/TI		(Mat		Horz(IL)	0.00	ь	n/a	n/a	Weight: 31 lb)

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 6=266/0-6-7, 2=349/0-6-7

Max Horz 2=185(load case 3)

Max Uplift 6=-238(load case 3), 2=-340(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/50, 2-3=-135/22, 3-4=-6/0, 3-6=-215/186

BOT CHORD 2-6=-78/84, 5-6=0/0

JOINT STRESS INDEX

2 = 0.53, 3 = 0.52 and 6 = 0.33

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 lb uplift at joint 6 and 340 lb uplift at joint 2.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

Julius Lee Truse Ceeton Chomer Ploride Pie No. 3-1986 Floride Pie Pier Stand Woynton Weeth, FL 20435

August 27,2007

warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation authority. Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883180
L250981	HJ7	MONO TRUSS	1	1	<u> </u>
		J			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:17:58 2007 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-2=-54 Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-3=-100(F=-23, B=-23), 3=-60(F=-23, B=-23)-to-4=-64(F=-25, B=-25), 2=0(F=5,

B=5)-to-5=-19(F=-5, B=-5)



Truss Type Qty Ply CASH ACCOUNT - RUBY PARK LOT 1 Job Truss J1883181 MONO TRUSS HJ9 L250981 1 Job Reference (optional) 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:17:59 2007 Page 1 Builders FirstSource, Lake City, FI 32055 4-3-0 9-10-13 -2-9-15 4-3-0 5-7-13 2-9-15 Scale-4 24 12 3x6 = 0-3-14 5 7 6 3x6 = 3x6 = 2x4 9-10-1 9-10-13 4-3-0 5-7-1 0-0-12 4-3-0 **GRIP SPACING** 2-0-0 CSI **DEFL** I/defl L/d **PLATES** LOADING (psf) in (loc) MT20 **TCLL** 20.0 Plates Increase 1.25 TC 0.61 Vert(LL) 0.05 6-7 >999 360 244/190 **TCDL** 7.0 Lumber Increase 1.25 BC 0.40 Vert(TL) -0.12 6-7 >986 240 0.34 5 WB Horz(TL) 0.01 n/a **BCLL** 10.0 Rep Stress Incr NO n/a Code FBC2004/TPI2002 Weight: 45 lb **BCDL** 5.0 (Matrix) **BRACING** LUMBER TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

(lb/size) 4=268/Mechanical, 2=458/0-6-7, 5=217/Mechanical REACTIONS

Max Horz 2=270(load case 3)

Max Uplift 4=-232(load case 3), 2=-284(load case 3), 5=-61(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/50, 2-3=-642/116, 3-4=-105/65

BOT CHORD

2-7=-305/593, 6-7=-305/593, 5-6=0/0

WEBS

3-7=0/189, 3-6=-618/317

JOINT STRESS INDEX

Continued on page 2

2 = 0.78, 3 = 0.16, 6 = 0.17 and 7 = 0.13

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 Ib uplift at joint 4, 284 lb uplift at joint 2 and 61 lb uplift at joint 5.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719





Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883181
L250981	HJ9	MONO TRUSS	2	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:17:59 2007 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate increase=1.25

Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)

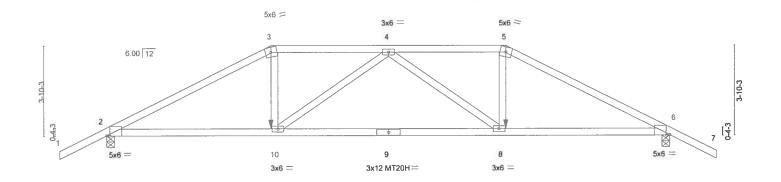
Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Les Truss (Poston Endinger Planta Pia No. 3-1995 1 100 Ensats Pay Flori Boviton Bason, Pt. 20406



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883182
L250981	T01	HIP	1	1	
23					Job Reference (optional)
Builders FirstSource	e, Lake City, FI 32055	6.300 s Feb 15	2006 M	iTek Ind	ustries, Inc. Wed Aug 22 09:17:59 2007 Page 1





	7-0-0	17-0-0	24-0-0
	7-0-0	10-0-0	7-0-0
ffsets (X,Y):	[2:0-1-11,Edge], [6:0-1-11,Edge]		

Plate Of	fsets (X,Y): [2:0-1-11,Edge], [6	6:0-1-11,E	dge]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.17	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.58	8-10	>491	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.39	Horz(TL)	0.11	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	Pl2002	(Mat	rix)						Weight: 108 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 **BRACING**

TOP CHORD

Structural wood sheathing directly applied or

3-4-5 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-3-9 oc

bracing.

REACTIONS (lb/size) 2=1657/0-4-0, 6=1657/0-4-0

Max Horz 2=77(load case 5)

Max Uplift 2=-547(load case 5), 6=-547(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/47, 2-3=-3014/914, 3-4=-2639/853, 4-5=-2639/853, 5-6=-3014/914, 6-7=0/47 TOP CHORD

BOT CHORD 2-10=-779/2603, 9-10=-979/3035, 8-9=-979/3035, 6-8=-746/2603 3-10=-248/918, 4-10=-598/318, 4-8=-598/318, 5-8=-248/918 **WEBS**

JOINT STRESS INDEX

2 = 0.74, 3 = 0.75, 4 = 0.34, 5 = 0.75, 6 = 0.74, 8 = 0.58, 9 = 0.83 and 10 = 0.58

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- E) All hearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

August 27,2007



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883182
L250981	T01	HIP	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:17:59 2007 Page 2

NOTES

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 547 lb uplift at joint 2 and 547 lb uplift at joint 6.
- 8) Girder carries hip end with 7-0-0 end setback.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

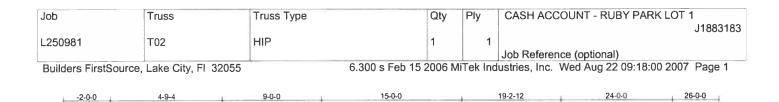
Vert: 1-3=-54, 3-5=-117(F=-63), 5-7=-54, 2-10=-10, 8-10=-22(F=-12), 6-8=-10

Concentrated Loads (lb)

Vert: 10=-411(F) 8=-411(F)

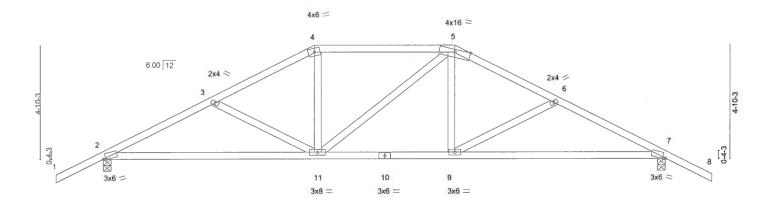
Julium Lem Trues Gestan Engineer Plantam ma No. admit 1406 Chastel May Muri Beytton tesson, 1-1 55455





6-0-0

4-2-12



-	9-0-0	15-0-0	24-0-0	
	9-0-0	6-0-0	9-0-0	
Plate Offsets (X,Y):	[2:0-1-5,0-0-7], [7:0-1-5,0-0-7]			

LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 * BCDL 5.0	SPACING Plates Increase Lumber Increase Rep Stress Incr Code FBC2004/TF	2-0-0 1.25 1.25 YES	CSI TC BC WB (Mate	0.30 0.40 0.10	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.15 -0.28 0.04	(loc) 7-9 7-9 7	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 119 lb	GRIP 244/190
---	---	------------------------------	--------------------------------	----------------------	--	------------------------------	--------------------------	-------------------------------	--------------------------	----------------------------------	---------------------

LUMBER BRACING

4-2-12

TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood signature Structure Structural wood signature Structure Structural wood signature Structure Structural wood signature Struc

Structural wood sheathing directly applied or 5-4-13 oc purlins.

4-9-4

2-0-0

Scale = 1 47.4

BOT CHORD Rigid ceiling directly applied or 9-2-5 oc bracing.

REACTIONS (lb/size) 2=874/0-4-0, 7=874/0-4-0

Max Horz 2=-89(load case 7)

Max Uplift 2=-245(load case 6), 7=-245(load case 7)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1320/697, 3-4=-1086/596, 4-5=-935/589, 5-6=-1086/597,

6-7=-1320/697, 7-8=0/47

BOT CHORD 2-11=-457/1117, 10-11=-280/935, 9-10=-280/935, 7-9=-457/1117

WEBS 3-11=-212/201, 4-11=-43/262, 5-11=-109/110, 5-9=-43/263, 6-9=-212/201

JOINT STRESS INDEX

2-0-0

2 = 0.87, 3 = 0.33, 4 = 0.57, 5 = 0.91, 6 = 0.33, 7 = 0.87, 9 = 0.34, 10 = 0.30 and 11 = 0.56

NOTES

P

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colling page 2

Truss Cosion Choinest Fichola Pin No. 3-1868 1100 Chastal Pay Rive Woynton Geson, FL 30-136

August 27,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-I1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



		J1883183
1	1	
		Job Reference (optional)
0.000 - 5-1-45	1	1 1

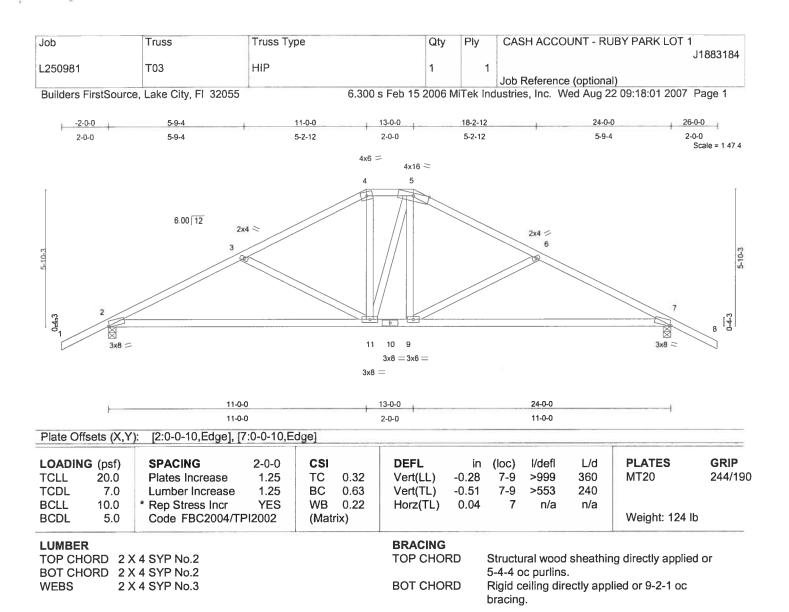
6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:18:00 2007 Page 2

NOTES

- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 245 lb uplift at joint 2 and 245 lb uplift at joint 7.

LOAD CASE(S) Standard





REACTIONS (lb/size) 2=874/0-4-0, 7=874/0-4-0

Max Horz 2=-101(load case 7)

Max Uplift 2=-256(load case 6), 7=-256(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1299/712, 3-4=-972/557, 4-5=-812/559, 5-6=-971/557,

6-7=-1299/712, 7-8=0/47

BOT CHORD 2-11=-463/1097, 10-11=-198/810, 9-10=-198/810, 7-9=-463/1097

WEBS 3-11=-330/300, 4-11=-93/256, 5-11=-136/143, 5-9=-94/257, 6-9=-332/301

JOINT STRESS INDEX

2 = 0.84, 3 = 0.33, 4 = 0.51, 5 = 0.58, 6 = 0.33, 7 = 0.85, 9 = 0.34, 10 = 0.78 and 11 = 0.66

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Collygue 10 page 2

Julius (Pesian Enginesi Plohas Pis No. 3-1985 Pioga Chemis (Rey Sivi Goyaton Geson, P., 35-135

August 27,2007



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883184
L250981	T03	HIP	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:18:01 2007 Page 2

NOTES

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

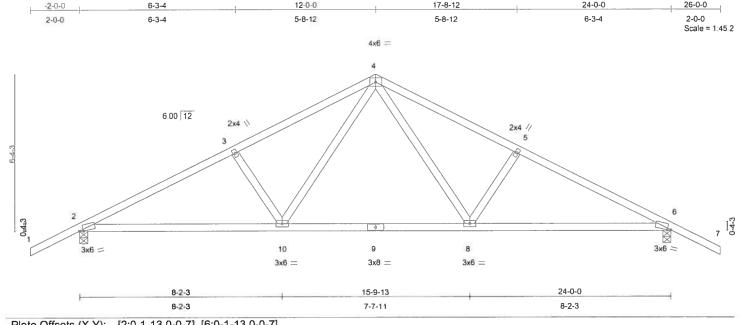
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 256 lb uplift at joint 2 and 256 lb uplift at joint 7.

LOAD CASE(S) Standard

Julium Law Truse Geston Endinser Florida FM No. Silvas 1100 Casalal Bay Blvd Boynton Weson, FL börlös



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883185
L250981	T04	COMMON	5	1	
					Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 N	1iTek Ind	dustries, Inc. Mon Aug 27 13:50:45 2007 Page 1



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.32	8-10	>888	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.49	8-10	>585	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.34	Horz(TL)	0.06	6	n/a	n/a		
BCDL	DL 5.0 Code FBC2004/TPI2002		(Matrix)							Weight: 113 lb		

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 4-5-15

oc purlins.

BOT CHORD Rigid ceiling directly applied or 7-3-4 oc bracing.

(lb/size) 2=1103/0-4-0, 6=1103/0-4-0 **REACTIONS**

Max Horz 2=-107(load case 7)

Max Uplift 2=-324(load case 6), 6=-324(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1838/1000, 3-4=-1664/998, 4-5=-1664/998, 5-6=-1838/1000, 6-7=0/47

2-10=-711/1561, 9-10=-376/1078, 8-9=-376/1078, 6-8=-711/1561 **BOT CHORD**

3-10=-268/257, 4-10=-370/671, 4-8=-370/671, 5-8=-268/257 **WEBS**

JOINT STRESS INDEX

2 = 0.79, 3 = 0.34, 4 = 0.71, 5 = 0.34, 6 = 0.79, 8 = 0.51, 9 = 0.97 and 10 = 0.51

NOTES

1) Unbalanced roof live loads have been considered for this design.

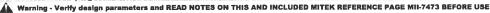
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 324 lb uplift at joint 2 and 324 lb uplift at joint 6.

August 27,2007

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).







Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883185
L250981	T04	COMMON	5	1	
			}		Job Reference (optional)

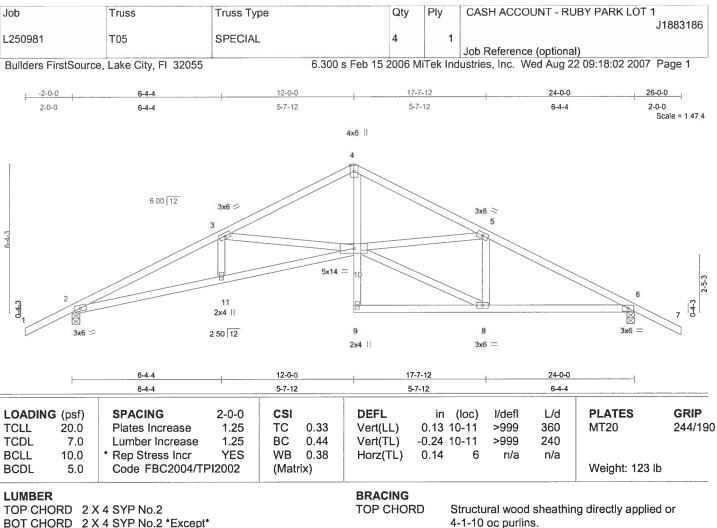
6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Aug 27 13:50:45 2007 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10





4-9 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

BOT CHORD

Rigid ceiling directly applied or 7-5-11 oc

bracing.

REACTIONS (lb/size) 2=874/0-4-0, 6=874/0-4-0

Max Horz 2=-107(load case 7)

Max Uplift 2=-261(load case 6), 6=-261(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/46, 2-3=-2082/977, 3-4=-1445/689, 4-5=-1473/704, 5-6=-1308/676, 6-7=0/47

BOT CHORD

2-11=-707/1818, 10-11=-709/1817, 9-10=0/74, 4-10=-374/941, 8-9=-8/25,

6-8=-426/1094

WEBS

3-11=0/201, 3-10=-573/401, 8-10=-457/1169, 5-10=-16/222, 5-8=-410/232

JOINT STRESS INDEX

2 = 0.68, 3 = 0.39, 4 = 0.68, 5 = 0.39, 6 = 0.58, 8 = 0.65, 9 = 0.60, 10 = 0.92 and 11 = 0.33

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other

 All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

August 27,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BGSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883186
L250981	T05	SPECIAL	4	1	
					Job Reference (optional)

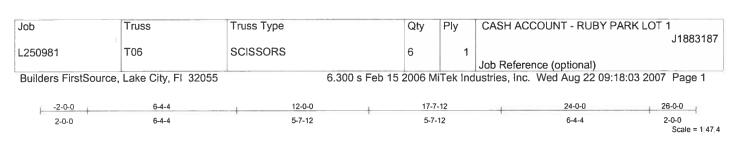
6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:18:02 2007 Page 2

NOTES

- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 2 and 261 lb uplift at joint 6.

LOAD CASE(S) Standard





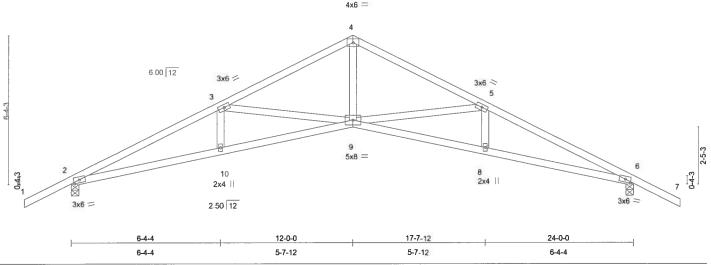


Plate Of	ffsets (X,Y): [2:0-2-15,0-1-8], [3	3:0-0-0,0-0)-0], [4:0)-0-0,0-0	-0], [5:0-0-0,0	-0-0], [6	:0-2-15	,0-1-8]			
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	0.15	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.40	Vert(TL)	-0.28	8-9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.19	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	P12002	(Mat	rix)						Weight: 110 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		4-1-11 oc purlins.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 7-5-9 oc

REACTIONS (lb/size) 2=874/0-4-0, 6=874/0-4-0

Max Horz 2=-106(load case 7)

Max Uplift 2=-261(load case 6), 6=-261(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-2077/976, 3-4=-1471/699, 4-5=-1471/699, 5-6=-2077/976, 6-7=0/46

BOT CHORD 2-10=-708/1814, 9-10=-711/1815, 8-9=-711/1815, 6-8=-708/1814

WEBS 3-10=0/184, 3-9=-563/391, 4-9=-374/946, 5-9=-563/391, 5-8=0/184

JOINT STRESS INDEX

2 = 0.67, 3 = 0.39, 4 = 0.60, 5 = 0.39, 6 = 0.67, 8 = 0.33, 9 = 0.52 and 10 = 0.33

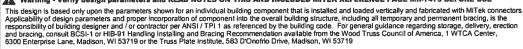
NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Truss Coelon Chamber Plands Per No. 3-1886 1:100 Chastal Bay Blvd Coynton Geson, FL 36-196







Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883187
L250981	T06	SCISSORS	6	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:18:03 2007 Page 2

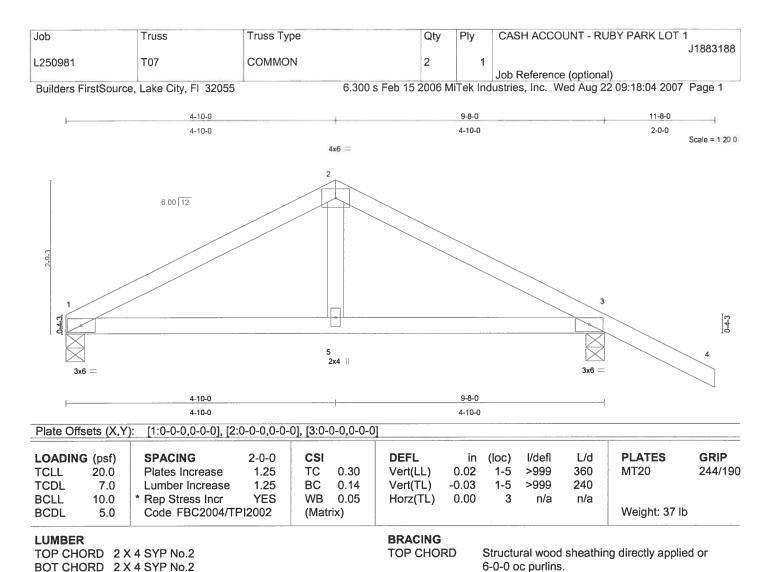
NOTES

- 5) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 2 and 261 lb uplift at joint 6.

LOAD CASE(S) Standard







BOT CHORD

bracing.

REACTIONS (lb/size) 1=285/0-4-0, 3=429/0-4-0

2 X 4 SYP No.3

Max Horz 1=-78(load case 7)

Max Uplift 1=-63(load case 6), 3=-169(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-404/238, 2-3=-410/247, 3-4=0/47

BOT CHORD

1-5=-55/311, 3-5=-55/311

WEBS

WEBS

2-5=0/159

JOINT STRESS INDEX

Continued on page 2

1 = 0.40, 2 = 0.49, 3 = 0.40 and 5 = 0.11

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

August 27,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Rigid ceiling directly applied or 10-0-0 oc

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883188
L250981	T07	COMMON	2	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:18:04 2007 Page 2

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 1 and 169 lb uplift at joint 3.

LOAD CASE(S) Standard

Julius Lees Truss Costan Engineer Fladds Fill Did. 3-1965 1466 Chastal Bay Alva Boynton Goson, 4L 25430



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1		
					J188		
L250981	T07G	GABLE	1	1	1		
				-	Job Reference (optional)		
Builders FirstSource, Lake City, FI 32055			00 s Feb 15 2006 N	ndustries, Inc. Wed Aug 22 09:18:05 2007 Page			
4-10-0				11-8-0			
4-10-0							

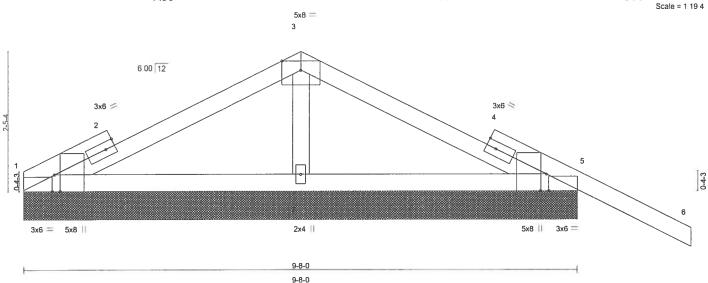


Plate Of	fsets (X,Y): [1:0-3-8,Edge], [1:	0-0-8,Edg	e], [5:0-	3-8,Edge	e], [5:0-0-8,Ed	ge]				7	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.02	6	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.19	Vert(TL)	-0.04	6	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.15	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 40 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		9-8-0 oc purlins.
OTHERS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc

oc O bracing.

REACTIONS (lb/size) 1=133/9-8-0, 5=408/9-8-0, 7=886/9-8-0

Max Horz 1=-92(load case 7)

Max Uplift 1=-46(load case 6), 5=-255(load case 7), 7=-318(load case 6) Max Grav 1=153(load case 10), 5=442(load case 11), 7=886(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-179/257, 2-3=-220/412, 3-4=-203/398, 4-5=-155/207, 5-6=-30/99

BOT CHORD 1-7=-254/294, 5-7=-254/294

WEBS 3-7=-797/592

JOINT STRESS INDEX

1 = 0.63, 1 = 0.00, 2 = 0.00, 2 = 0.36, 3 = 0.68, 4 = 0.00, 4 = 0.36, 5 = 0.63, 5 = 0.00 and 7 = 0.33

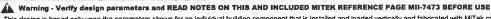
NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal Coffithe (1968) page MiTek "Standard Gable End Detail"

August 27,2007



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883189
L250981	T07G	GABLE	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:18:05 2007 Page 2

NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1, 255 lb uplift at joint 5 and 318 lb uplift at joint 7.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

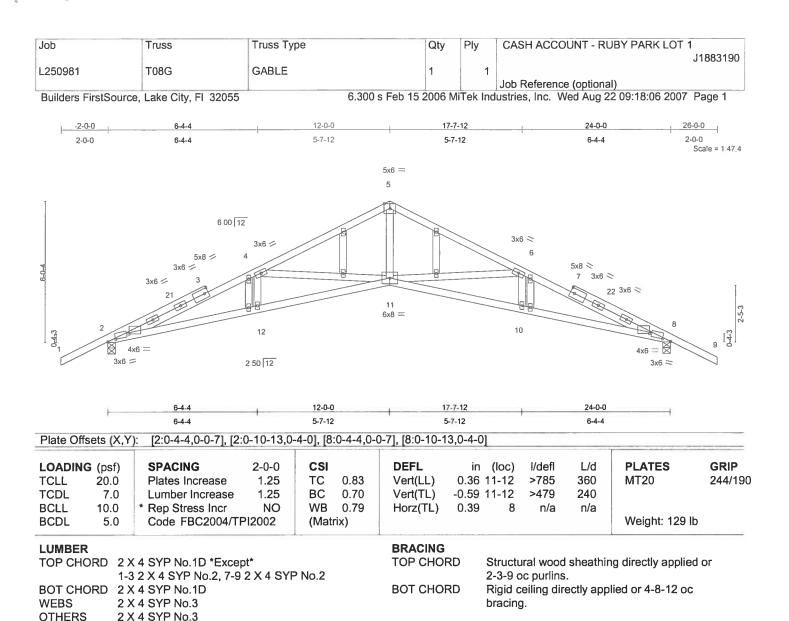
LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-114(F=-60), 3-6=-114(F=-60), 1-5=-10

Julius Lee Trues Ceston Choinear Planda Pie No. 3-1888 1406 Cessial May Mivd Govnion Weson, HL 20438





REACTIONS (lb/size) 2=1414/0-4-0, 8=1414/0-4-0

Max Horz 2=117(load case 6)

Max Uplift 2=-718(load case 6), 8=-718(load case 7)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-21=-4511/2329, 3-21=-4404/2319, 3-4=-4381/2312, 4-5=-3100/1584,

5-6=-3100/1584, 6-7=-4381/2312, 7-22=-4404/2319, 8-22=-4511/2329, 8-9=0/46

BOT CHORD 2-12=-1988/4093, 11-12=-1992/4099, 10-11=-1992/4099, 8-10=-1988/4093

WEBS 4-12=0/163, 4-11=-1372/851, 5-11=-937/1980, 6-11=-1372/851, 6-10=0/163

JOINT STRESS INDEX

2 = 0.80, 2 = 0.92, 3 = 0.00, 3 = 0.40, 3 = 0.40, 3 = 0.82, 4 = 0.39, 5 = 0.75, 6 = 0.39, 7 = 0.00, 7 = 0.82, 7 = 0.40, 7 = 0.40, 8 = 0.80, 8 = 0.92, 10 = 0.33, 11 = 0.90, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33, 18 = 0.33, 19 = 0.33 and 20 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

Trues Coston Engineer Planda PB No. 34868 9406 Grastal Pay Myn Goynton Goson, FL 20426

> Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 1
					J1883190
L250981	T08G	GABLE	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:18:06 2007 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 718 lb uplift at joint 2 and 718 lb uplift at joint 8.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-21=-54, 5-21=-114(F=-60), 5-22=-114(F=-60), 9-22=-54, 2-11=-10, 8-11=-10

Julius Les Prints Election Engineer Prints et Pic. 3-1868 1400 Cassis Pay Sive Rounds Baste

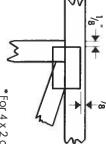


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



'For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



'This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 × 4

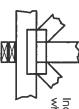
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



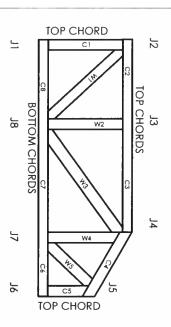
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

ICBO

96-31, 96-67

ВОСА

SBCCI

3907, 4922

WISC/DILHR

9667, 9432A HR 960022-W, 970036-N

561

NER N





MiTek Engineering Reference Sheet: MII-7473

m 🛕 General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)

Ċ

4.

 Unless expressly noted, this design is not applicable for use with fire retardant or

Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

- applicable for use with fire retardant or preservative treated lumber.

 7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General
- Plate type, size and location dimensions shown indicate minimum plating requirements

practice is to camber for dead load deflection

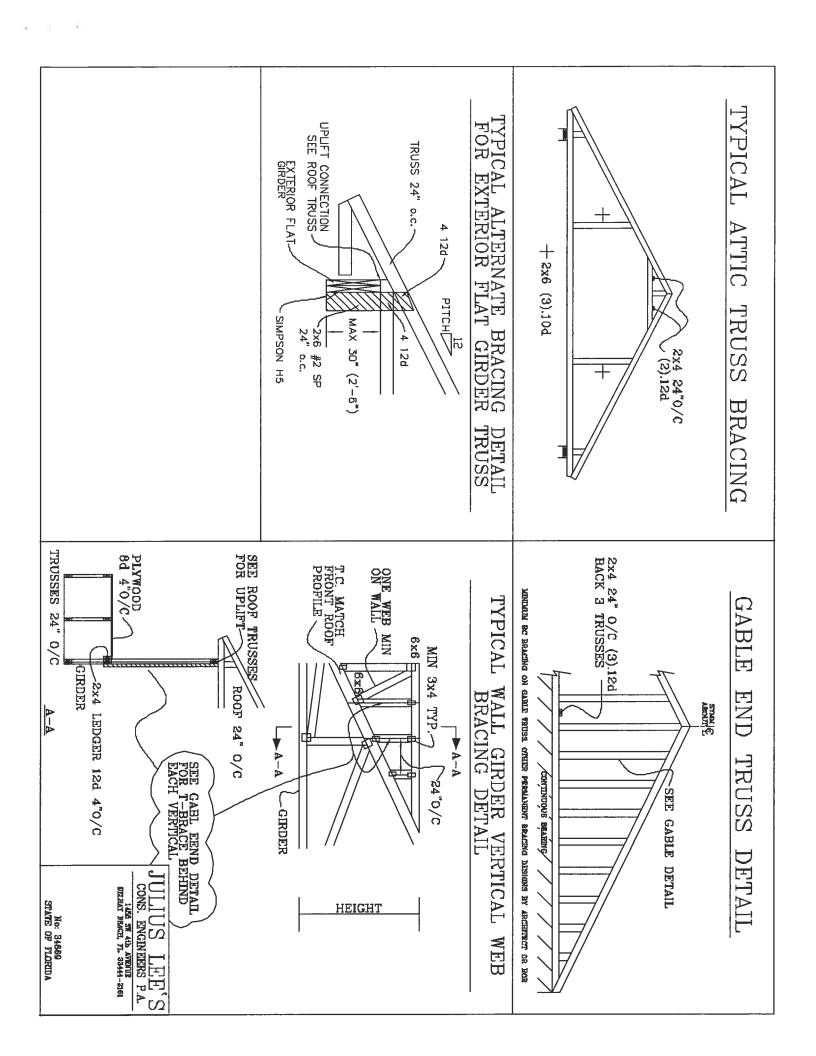
- Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
- 13. Do not overload roof or floor trusses with stacks of construction materials.
- 14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
- 15. Care should be exercised in handling, erection and installation of trusses.© 1993 MiTek® Holdings, Inc.

THE INSTITUTE, SET TO JESS 1-403 GROUNDES CAPE IN FABRURATION, PRINCIPSED BY TO TRUSS COUNS. PLATE INSTITUTE, SEE TO JESS 1-403 GROUNDES CAPE IN FABRURATION, PRINCIPSES TO THE TO TRUSS COUNCIL. OF MERICIA, STORE STOREMED RE, SAITE 200, MADISON, VI. SETZES AND VITA EVOID TRUSS COUNCIL. THESE FARCITIONS. UNICES OFFERVISE (MODERNE), TOP COORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTON CHARG SHALL HAVE A PROPERLY ATTACHED CEILING. STRUCTURAL PANELS AND BOTTON CHARG SHALL HAVE A PROPERLY ATTACHED RECEDED. NO: 34869 STATE OF FALARING.	POR MAX POP		4' 7" 7' 4" 7' 11" 8' 9" 9' 5" 10' 6" 11' 2" 13 8" 14' 0" D 4' 4" 7' 2" 7' 2" 8' 9" 9' 2" 10' 5" 10' 11" 13' 8" 14' 0" D 4' 4" 7' 1" 7' 1" 8' 9" 9' 2" 10' 5" 10' 11" 13' 8" 14' 0" ARD 4' 3" 6' 1" 6' 1" 8' 0" 8' 0" 10' 5" 10' 8" 12' 6" 12' 6"	C HH SIANDARD 4 B 7 4 7 7 1 8 9 8 11 10 6 10 8 13 8 13 1	SP #2 4 2" 8 8" 7 2" 7 11" 8 8 8" 10 2" 12 5" 13 5" SP #2 4 2" 8 8" 7 2" 7 11" 8 8" 9 6" 10 2" 12 6" 13 6" HI STUD 4 0" 8 1" 7 11" 8 2" 9 6" 9 11" 12 6" 13 6" STUD 4 0" 8 1" 7 11" 8 1 8 9 6" 9 11" 12 6" 13 6" STUD 4 0" 8 1" 8 1" 7 11" 8 1 8 9 6" 9 11" 12 6" 12 6"	ARD 5 9 6 7 7 11 7 11 9 5 9 5 12 4 12 4 ARD 5 9 6 7 6 10 6 10 7 11 7 11 9 5 8 9 8 10 7 10 7 10 7 10 7 10 7 10 7 10 7 10	SPH #1 #6 34 6 10 10 10 11 2 12 HH STUD 3 3 4 11 4 11 6 6 6 6 6 8 8 3 8 8 10 10 10 11 2 12 STUD 3 3 4 11 4 11 6 6 6 6 6 8 8 3 8 8 3 10 1 1 10 10 11 12 STANDARD 3 3 4 2 2 4 2 6 6 7 6 7 6 7 6 8 3 8 11 10 10 11 8 12 SP #2 3 7 5 10 6 3 6 11 7 5 8 3 8 11 10 10 11 8 12	ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOS GRACE PRACE OF GROUP B G GROUP B GROUP B G G G G G G G G G G G G G G G G G G	
MAX. TOT. LD. 60 PSF MAX. SPACING 24.0" DATE 11/26/03 DRWG MIX STD GMAX 15 E HT -ENG		DUTALIDAGES WITH 2' O" DYEMBANG, DE 12" PLYMODID OVERHANG. ATIACH RACH "L" BRACES RITH 104 NAILS. * POR (1) "L" REACE RITH 104 NAILS AF 2" O.C. IN 16" END ZONES AND 4" O.C. HETWIEN ZONES. ** POR (2) "L" BRACES: EFACE NAILS AT 3" O.C. IN 16" END ZONES AND 6" O.C. HETWIEN ZONES. T. BRACING ADST HE A MINIMUM OF BOX OF WEB MEMBER LENGTH.	14.07		14° D" 5007HDEN PNB DOUGLAS	11 13 7 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	13 3 DEGLING STOUP STELLE 11 11 12 11 12 11 13	I = 1.00, B 'L' HEACE ** P A GROUP B	

2.0

٠

MAX. TOT. LD. No: 34869 STATE OF FLUNDA MAX. SPACING	BANADOGIN TRUSSIS REBUIRE EXTREME CARE IN FARRICATING, HANDLING, SUPPING, DISTALLING AND RANGE ENTERPOSE IN 1-03 CONDUCTOR SAFETY (NETBOANTING, HANDLING, SUPPING, DISTALLING AND REACTIONS OF THE CONSTRUCTION OF THE SAFETY PROFESSION OF THE SAFETY	ANCE 7 OZ: 130 MPH MIND SPEED, 30 MEAN HEIGHT. ENCLOSED, 1 = 1.00. H SPLANE SPEEDS GAMES MADE 1 SACE 1 OZ	2 09: 190 MDU WIND COEED 90' MEAN HEIGHT ENGLOSED I
ID. 60 PSF NG 24.0"	REF ASCE?—02-GAB13030 DATE 11/26/03 DWG with 5th gable 30' e hi -eng	BRACING GROUP SPECIES AND GRADES: GROUP A: SPRICE-PRE-PRE-PRE 4. / 42 STANDARD DOUGLAS FR-LARCH STUDD STANDARD DOUGLAS FR-LARCH SOUTHERN PORE 42 STUDD STANDARD DOUGLAS FR-LARCH FROUP B: HEM-PR HEM-PR HEM-PR HEM-PR HEM-PR GROUP B: HEM-PR H	EVDACIDE



BOT CHORD CHORD WEBS 2X4 2X4 2X4 800 888 BETTER
BETTER
BETTER

PIGGYBACK DETAIL

TYPE

SWAGS

â

8

2

8 궣

DE DE

Ħ

4X8

6X6

9

ĐX6 226

.bx3

1.5X4

1.5%

1.5X4 **5**X6

5X6

5

>

2X4

2.5X4

2.6X4

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED FURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE POLLOWING WIND CONDITIONS: 110 MPH WIND, 30' MBAN HGT, ASCE 7-93, CLOSED BILDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DI=5 PSF, WIND BC DI=5 PSF

110 MPH WIND, 30' MBAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TO DL-5 PSF, WIND BC DL-5 PSF

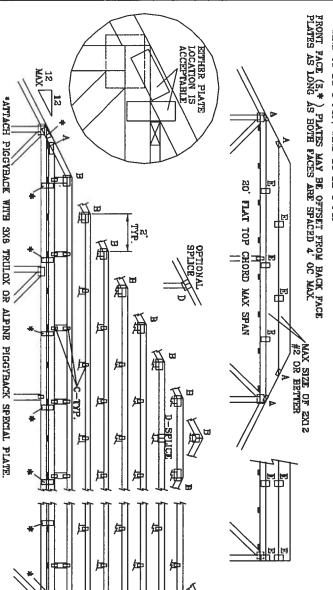
BLDG, LOCATED WIND TC DL=6 F 30° MEAN HGT, ASCE 7-98, CLOSED ANYWHERE IN ROOF, CAT II, EXP. C, PSF, WIND HC DL=6 PSF

> Ħ Ħ C

4XB

OR SX6 TRULOX AT 4'
HOTATED VERTICALLY

50



ATTACH TRULOX PLATES WITH (6) 0.120° X 1.375" NAILS, EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBES BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULO INFORMATION.
JER TO

O' TO 7'8" NO BRACING 1x4 "T" BRACE. SAME GRADE. SPECIES AS WEB 7'9" TO 10' MEMBER. OR BETTER. AND 80% LENGTH OF WEB 10' TO 14' MEMBER. OR BETTER. AND 80% LENGTH OF WEB 10' TO 14' MEMBER. OR BETTER. AND 80% LENGTH OF WEB 10' TO 14' MEMBER. OR BETTER. AND 80% LENGTH OF WEB	WEB LINGTH WEB BRACING CHART REQUIRED BRACING
IX4 "I" BRACE. SAME GRADE, SPECIES AS MEMBER, OR HEITTER, AND 80% LENGTH OF MEMBER. ATTACH WITH 84 NAILS AT 4" OC 2X4 "I" BRACE. SAME GRADE, SPECIES AS MEMBER. OR HEITTER, AND 80% LENGTH OF MEMBER. ATTACH WITH 184 NAILS AT 4" O	o' To 7'e" NO BRACING
2x4 "t" brace. Same grade, species as member, or better, and box length of member. Attach with 16d nails at 4° o	1x4 "T" BRACE. SAME MEMBER, OR HETTER, AL MEMBER. ATTACH WITH
	Zx4 "I" BRACE. SAME MEMBER, OR BETTER, A MEMBER. ATTACH WITH

		00
1 -kg	THE PIGGYBACK AT THE TIME OF ACH TO SUPPORTING TRUSS WITH VAILS PER FACE PER PLY. APPLY L PLATE TO EACH TRUSS FACE AND ESS.	ATTACH TEETH TO THE FABRICATION. ATTACH (4) 0.120° X 1.375° NJ PIGGYBACK SPECIAL PI SPACE 4° OC OR LESS
	A TINGITIECT OF ECTER & THIEF	, 116

ı	
	THUS
	DRAWING
	REPLACES
	DRAWINGS
	634,016
	634,017
	8
	347,0

No: 34868	:		Average was and the second	1455 SW 4th AVENUE	보		THIS DRAWIN
SPACING 24 0"	47 PSF AT 1.15 DUR. FAC.	1.25 DUK. FAC.	50 PSF AT	1.33 DUR. FAC.	55 PSF AT	MAX LOADING	G REPLACES DRAWINGS 6
			-ENG JL	DRWGMITEK STD PIGGY	DATE 11/26/09	REF PIGGYBACK	THIS DRAWING REPLACES DRAWINGS 634,016 834,017 & 847,045

STRUCTURAL PARLS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CHORD STRUCTURAL PARLS AND STRUCTURAL AND BOTTON AND BOTTON AND ADDRESS AND ACTION ACTIO

ح

VALLEYTRUSS DETAIL

TOP CHORD CHORD WEES 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR 2X4 SP #3 OR BETTER. BETTER.

- 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d HOX (0.135" X 3.5") NAILS TOE-NAILED FOR BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF SBC 110 MPH, ASCE 7-93 110 MPH WIND OR (3) 16d FOR ASCE 7-98 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED ENCLOSED

EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9" WITH 8d BOX (0.113" X 2.6") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING. UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80%

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

ENGINEERS' SEALED DESIGN. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

CUT FROM 2X6 OR LARGER AS REQ'D

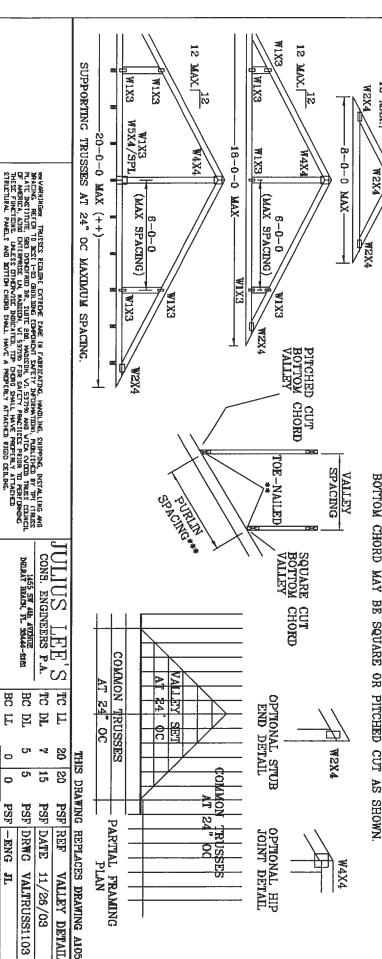
4-0-0

XAM

12 MAX.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



No: 34868 STATE OF PLORIDA

SPACING DURFAC 1.25 TOT. LD

24. 1.25 BC

o

PSF PSF

-ENG

32

40

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

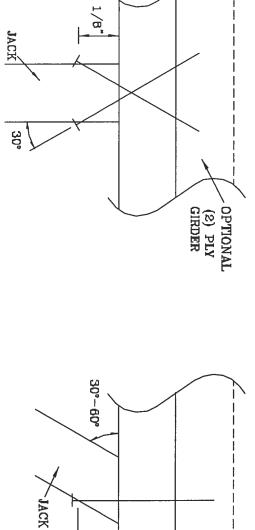
PER ANSI/AF&PA NDS-1997 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A FRAMING INTO A SINGLE TOE-NAILED CONNECTION FOR JACK OR DOUBLE PLY SUPPORTING GIRDER

MAXIMUM LATERAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF		SOUTHERN PINE	DOUGLAS	DOUGLAS FIR-LARCH		HEM-FIR	SPRUCE	SPRUCE PINE FIR
TOE-NAILS	1 PLY	2 PLIES 1 PLY		2 PLIES	1 P LY	2 PLIES	1 PLY	2 PLIES
N	197#	256#	181#	234#	156#	203#	154#	199#
ယ	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#
ALL VALU	TYM SE	ALL VALUES MAY BE MILITIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR	AD AA U	ROPRIATE	DURATION	OF LOAD FO	CTOR	



1/8"

GIRDER

OPTIONAL (2) PLY

ALTERNATIVE CONDITION

SIHE
DRAWING
REPLACES
DRAWING
784040

WARDEL TRUSSES REQUIRE EXTREME CARE IN FARRICATING, HANDLING, SHEPPING, DISTALLING AND BRACING, RETER TO BEST 1-43 CHIMING COMPIDENT SAFETY INTERNATION, PUBLISHED BY FPI CHRISS PLATE INSTITUTE, 383 PONCERGO PA, SUTE 280, HANDLING, SOTIE) AND UTCA (ANDID TRUSS COLACIO. OF ANERICA, GODD ENTERPOSE LIM, MORIEDH, VI 2019) FOR SAFETY PRACIFICES PRIDE TO PREFEDENCY THESE FAUFUTENS, UNILESS CHIMENTS, INDICATED, TOP CHOOD SHALL HAVE PRIPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHOED SHALL HAVE A PROPERLY ATTACHED RIGID CILING.							
No: 34869 STATE OF FLORIDA				1455 SV 411 AVENUE DELRAY BEACH, PL SCH44-2161	CONS. ENGINEERS P.A.	S, HH'I SOL'INL	
SPACING	DUR. FAC. 1.00	TOT. LD.	BC LL	BC DL	TC DL	TC LL	
	1.00	PSF	PSF	PSF	PSF	PSF	
	•		-ENG JL	DRWG	DATE	REF	
			Л	CNTONAIL1103	DATE 11/26/03	TOE-NAIL	

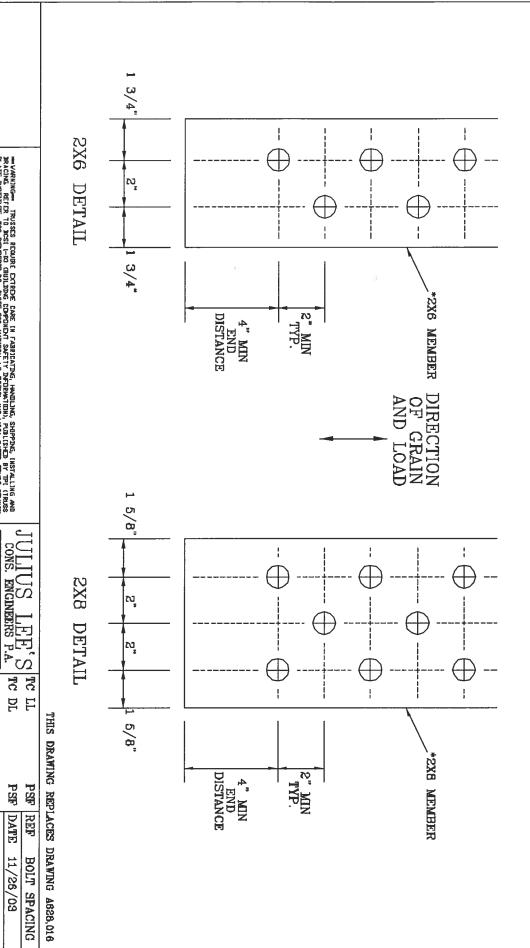
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PAITERNS SHOWN BELOW. BOLT APPLIED

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



MYARINIG* TRUSSES REQUIRE ECTREDE EARE IN FAREINATING, HANDLING, SHOPPING, INST.

**BACING, REFER TO EXCL FOR BUILDING CEPPRING SAFETY PARENATION, PUBLICISTO BY

**PARENA INSTITUTE, 383 OTHORRED SP. SUITE ERM, MAIISON, VJ. 33739 AND MICA VICTO TR

**CFERCA, GAID ENTERPORTS LM, MAINSON, MI 33739 FOR SAFETY PARACTICES PRIBE TO PE

**THESE FUNCTIONS. UNLESS OTHERPORTS. DOMICATED, TOTO CHERD SAALL HAVE PREFEREN ATTACH

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CELLING

**STRUCTURAL PARELS AND MOTITON CHORD SHALL HAVE A PROPERTY ATTACHED MUID CHORD SHALL HAVE A PROPERTY ATTACHED MUID CHORD CHO

CONS.

DELEAT SEACH, FL 33444-2161

BC DL TC DL

DATE

11/26/03

CNBOLTSP1103

PSF PSF PSF PSF

> -ENG DRWG

No: 34869 STATE OF FLORIDA

SPACING DUR. FAC. TOT. LD. BC LL

TRULOX CONNECTION DETAIL

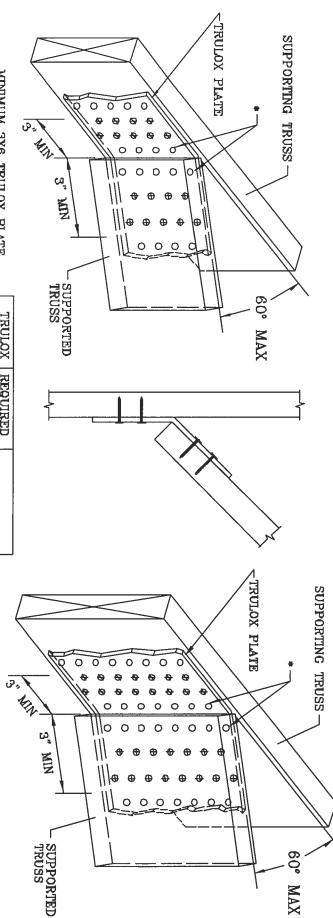
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (\(\phi \)).

NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE

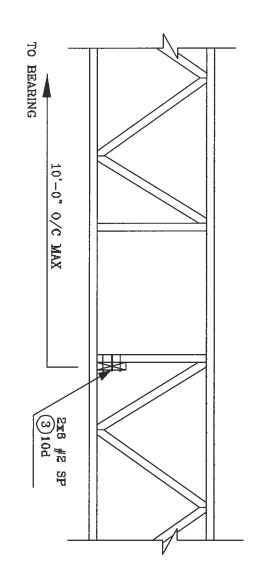
6X6	3X6	TRULOX PLATE SIZE
15	9	REQUIRED NAILS PER TRUSS
990#	350#	MAXIMUM LOAD UP OR DOWN

MINIMUM 5X6 TRULOX PLATE

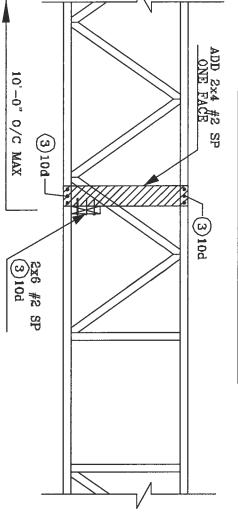
THIS DRAWING REPLACES DRAWINGS 1,168,989 1,158,989/R 1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524

No: 34869 STATE OF FLORIDA	JULIUS LEE'S cons. engineers P.A. 1455 97 444 APRICE persay many, 72 3844-216.	Leaft Arit				
		ここくばいして こうきいきんてい したんじんりじ				
	REF TRULOX DATE 11/26/03 DRWG CNTRULOX1103 -ENG JL	1,100,017 1,100,104 0, 1,101,004				

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



CONS. ENGINEERS P.A.

1455 ST 4th AVEUIT

1555 ST 4th AVEUIT

1555

No: 34869 STATE OF FLORIDA TO BEARING

