GENERAL NOTES

DESIGN CRITERIA

D1 ALL WORK SHALL CONFORM TO AT LEAST THE MINIMUM STANDARD OF THE FLORIDA BUILDING CODE, LATEST EDITION.

D2 DESIGN LOAD VALUES:

ROOF LIVE LOADS	20 PSF
ASSUMED ALLOWABLE SOIL BEARING CAPACITY	2000 PSF

D3 THE STRUCTURAL PLANS AND WIND SPEED HAVE BEEN DESIGNED IN ACCORDANCE WITH SECTION 1606 OF THE FLORIDA BUILDING CODE LATEST EDITION.

GENERAL

G1 THE GENERAL CONTRACTOR SHALL REVIEW AND DETERMINE THAT DIMENSIONS ARE COORDINATED BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO FABRICATION OR START OF CONSTRUCTION.

G2 THE GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, THE WORK PERSONS, AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION & FOR ALL JOBSITE SAFETY.

G3 NO STRUCTURAL MEMBER SHALL BE CUT NOTCHED OR OTHERWISE REDUCED IN STRENGTH.

G4 THE GENERAL CONTRACTOR SHALL COORDINATE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR ANCHORED, EMBEDDED AND SUPPORTED ITEMS WHICH AFFECT THE STRUCTRAL DRAWINGS AND NOTIFY THE ARCHITECT/ENGINEER ON ANY DISCREPANCIES

G5 ANY SUBMITTALS RECEIVED BY A/E THAT HAVE NOT BEEN CHECKED BY THE GC AND HIS SUBCONTRACTOR SHALL BE RETURNED WITHOUT REVIEW.

G6 ALL SECTIONS AND DETAILS SHALL BE CONSTRUED TO BE TYPICAL OR SIMILAR UNLESS ANOTHER SECTION OR DETAIL IS NOTED.

G7 THE CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING ALL EXISTING CONDITIONS, ELEVATIONS, AND DIMENSIONS PRIOR TO COMMENCING CONSTRUCTION, INCLUDING FABRICATION. ALL DISCREPANCIES SHALL BE REPORTED TO THE A/E FOR RESOLUTION.

FOUNDATIONS

F1 A GEOTECHNICAL REPORT FOR THIS PROJECT WAS NOT PROVIDED BY THE OWNER. THESE PLANS ARE BASED UPON AN ASSUMED ALLOWABLE BEARING CAPACITY OF 2000 PSF. THE CONTRACTOR SHALL ENGAGE A QUALIFIED AND CERTIFIED GEOTECHNICAL ENGINEER TO DETERMINE THE ALLOWABLE SOIL BEARING CAPACITY. A COPY OF THE REPORT SHALL BE PROVIDED TO THE A/E. IF THE DETERMINED ALLOWABLE BEARING CAPACITY IS LESS THAN THE ASSUMED VALUE, MODIFICATIONS TO THE FOUNDATIONS MAY BE REQUIRED.

F2 THE GEOTECHNICAL ENGINEER SHALL MAKE A FIELD INVESTIGATION TO DETERMINE IF ANY SOIL CONDITIONS ARE PRESENT THAT MAY ADVERSELY AFFECT THE PROJECT. THE CONTRACTOR SHALL REMOVE ALL SUCH MATERIAL AND REPLACE IT WITH APPROVED FILL.

F3 SUBGRADE UNDER FOOTINGS AND SLABS SHALL BE COMPACTED TO AT LEAST 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D1557). SUBMIT DENSITY TESTS TO THE A/E.

SLABS ON GRADE

S1 PROVIDE 6 MIL POLYETHYLENE SHEETING UNDER ALL SLABS ON GRADE.

S2 PROVIDE CONTROL JOINTS (1/4" WIDE BY 3/8" DEEP) AS INDICATED ON PLAN. FILL JOINTS WITH POURED RUBBER. IF JOINTS ARE SAWCUT, SAWCUTTING MUST BE DONE THE SAME DAY THE CONCRETE IS PLACED.

S3 PROVIDE 1/2" EXPANSION JOINTS AT ALL LOCATIONS WHERE SLABS ABUT STRUCTURES (WALLS, COLUMNS, ETC.)

CONCRETE AND REINFORCING

C1 THE GENERAL CONTRACTOR SHALL ENGAGE A CERTIFIED TESTING AGENCY TO PERFORM INDUSTRY STANDARD TESTING INCLUDING SLUMP TESTS AND CYLINDER BREAKS TO ENSURE CONFORMANCE WITH PLANS. SUBMIT REPORTS TO A/E.

C2 CONCRETE WORK & MIX DESIGN SHALL CONFORM TO ACI 301 (LATEST EDITION) "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS". MIXING SHALL BE IN ACCORDANCE WITH ASTM C94

C3 MINIMUM 28-DAY COMPRESSIVE STRENGTH:

FOOTINGS AND TIE BEAMS	2,500 PSI
COLUMNS AND SLABS	2,500 PSI

C4 SPLICES OF REINFORCING (EXCEPT AS SHOWN ON PLANS)

#4 BARS	20 INCHES
#5 BARS	27 INCHES
WELDED WIRE MESH	6"

C5 AT ALL CORNERS OF TIE BEAMS AND WALL FOOTINGS, PROVIDE CORNER BARS (30 INCH MINIMUM LEGS) TO MATCH HORIZONTAL BARS.

C6 REINFORCING BARS SHALL CONFORM TO ASTM A615-96a GRADE 40. WELDED WIRE MESH SHALL CONFORM TO ASTM A-185. LAP WELDED WIRE MESH ON MESH + 2" WHERE SPLICED.

C7 MINIMUM COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHEWISE NOTED.

LOCATION	MIN. COVER
BOTTOM OF FOOTINGS	3"
SIDES OF FOOTINGS	3"
COLUMNS	1 1/2"
TIE BEAMS	1 1/2"
SLABS	AS NOTED

STRUCTURAL STEEL

S1 GENERAL CONTRACTOR SHALL ENGAGE A CERTIFIED TESTING AGENCY TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS. SUBMIT REPORT TO A/E.

S2 STEEL WORK SHALL CONFORM TO THE AISC "SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL BUILDINGS", LATEST EDITION

S3 MATERIAL

BEAMS & CHANNELS	ASTM A572, GRADE 50
STEEL TUBING	ASTM A500, GRADE B
PLATES	ASTM A36
BOLTS	ASTM A325
ANCHOR BOLTS	ASTM A36 THREADED ROD - 6" MIN. EMBEDMENT WITH 2" HOOK
EXPANSION ANCHORS	RAMSET/REDHEAD TRUBOLT OR APPORVED EQUIVALENT
NON-SHRINK GROUT	5000 PSI

S4 BRACE AND MAINTAIN ALL STEEL IN ALIGNMENT UNTIL OTHER PARTS OF CONSTRUCTION NECESSARY FOR PERMANENT SUPPORT ARE COMPLETED. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING TEMPORARY SHORING AS REQUIRED FOR THE STABILITY OF THE STEEL FRAME UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN COMPLETED AND BUILDING IS ENCLOSED.

S5 ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF "THE STANDARD CODE FOR WELDING IN BUILDING CONSTRUCTION: OF THE AMERICAN WELDING SOCIETY. WELDING ELECTRODES SHALL BE E70XX-LOW HYDROGEN FOR SHIELD AND METAL ARC WELDING.

S6 PROVIDE NUT & WASHER FOR ALL BOLTS AND ANCHOR BOLTS

S7 ALL WELDED CONNECTIONS SHALL BE 1/4 " FILLET ALL AROUND, UNO. ALL BOLTED CONNECTIONS SHALL BE 1/2 " DIA. A325 BOLTS, UNO.

PRE-FABRICATED WOOD TRUSSES

WT1 GENERAL CONTRACTOR SHALL ENGAGE A CERTIFIED TESTING AGENCY TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS. SUBMIT REPORTS TO A/E.

WT2 WOOD TRUSSES SHALL BE DESIGNED, SIGNED & SEALED BY A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. TRUSSES SHALL BE FABRICATED IN CONFORMANCE WITH THE THE "QUALITY CONTROL MANUAL" BY TRUSS PLATE INSTITUTE (TPI).

WT3 HANDLING, ERECTION AND BRACING OF WOOD TRUSSES SHALL BE IN ACCORDANCE WITH "HANDLING AND ERECTING WOOD TRUSSES" (HET80) AND "BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS" (BWT-76) BY THE TRUSS PLATE INSTITUTE (TPI).

WT4 PERMANENT BRACING SHALL BE INDICATED IN THE TRUSS LAYOUT DRAWINGS AND SHALL BE SUPPLIED AND INSTALLED BY THE FRAMING CONTRACTOR.

PRE-FABRICATED WOOD TRUSSES (CONTINUED)

WT5 TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS:

DEAD LOAD	10 PSF
LIVE LOAD	20 PSF
WIND	110 MPH 3 SECOND WIND GUST

WT6 PRE-FABRICATED WOOD TRUSSES SHALL BE FABRICATED FROM SOUTHERN PINE (SPIB) KILN DRIED #2 GRADE OR BETTER FOR CHORD AND #3 GRADE OR BETTER FOR WEBS.

WT7 TRUSS BEARING SHALL BE 4" NOMINAL UNLESS NOTED OTHERWISE. BEARING LOCATIONS MUST BE MARKED ON TRUSS BY FABRICATOR TO INSURE PROPER INSTALLATION.

WT8 SHOP DRAWINGS SHALL BE SUBMITTED WHICH INDICATE DESIGN LOADS, DURATION FACTOR TRUSS LAYOUT, TRUSS CONFIGURATION AND TRUSS TO TRUSS CONNECTION. SHOP DRAWINGS SHALL SHOW PIECE MARKS, MEMBER SIZE AND GRADE AND CONNECTION DETAILS.

WT9 NO WANE KNOTS, SKIPS OR OTHER DEFECTS SHALL OCCUR IN THE PLATE CONTACT AREA OR SCARFED AREA OF WEB MEMBERS. PLATES SHALL BE CENTERED WITH ONE REQUIRED EACH SIDE OR TRUSS.

WT10 DESIGN OF METAL CONNECTED WOOD ROOF TRUSSES TO COMPLY WITH STANDARD BLDG. CODE NFPA'S "NATIONAL DESIGN SPECIFICATIONS FOR STRESS GRADED LUMBER AND ITS FASTENINGS", AND TRUSS PLATE INSTITUTE'S "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES".

WT11 WOOD BLOCKING AT TRUSS BEARING SHALL BE LAP SPLICED 4'-0" MIN. AND NAILED WITH (20) 10d NAILS AT SPLICE, 10d NAILS @ 16" O.C. ELSEWHERE.

MASONRY

M1 MASONRY CONSTRUCTION SHALL CONFORM TO ACI STANDARD BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES (ACI 530-88/ASCE 5-88) AND SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1-88/ASCE 6-88)

NOTE:
ALL WIND LOADS ARE IN ACCORDANCE WITH SECTION 1606. FLORIDA BUILDING CODE, 2001 EDITION.

BASIC WIND SPEED		110 MPH
IMPORTANCE FACTOR		1.0
BUILDING CATEGORY		2
EXPOSURE		B
INTERNAL PRESSURE COEFFICIENT		+/- 0.18
COMPONENT AND CLADDING PRESSURE	WALLS	+21.8/-29.1 PSF
	ROOF	+12.5/-29.1 PSF
	OVERHANGS	-71.6 PSF
TYPE OF STRUCTURE		ENCLOSED

NOTE:
EXTERIOR WINDOWS AND GLASS DOORS SHALL BE TESTED BY AN APPROVED INDEPENDENT TESTING LABORATORY, AND BEAR AN AAMA OR WDMA OR OTHER APPROVED LABEL IDENTIFYING THE MANUFACTURER, PERFORMANCE CHARACTERISTICS AND APPROVED PRODUCT EVALUATION ENTITY TO INDICATE COMPLIANCE WITH THE REQUIREMENTS OF THE FOLLOWING SPECIFICATION.

ANSI/AAMA/NWWDA 101/IS2 2/97

THE CONSTRUCTION SHALL BE TESTED IN ACCORDANCE WITH ASTM E 330, STANDARD TEST METHODS FOR STRUCTURAL PERFORMANCE OF EXTERIOR WINDOWS, CURTAIN WALLS, AND DOORS BY UNIFORM STATIC AIR PRESSURE.

BUILDING USE, CLASSIFACATION & OCCUPANCY AS PER TABLES 500 & 1003.1, FLORIDA BUILDING CODE, 2001 ED.	
BUILDING GROUP OCCUPANCY	GROUP B - AMBULATORY
TABLE 500 TYPE OF CONSTRUCTION	TYPE VI - UNPRO.
TABLE 500 AREA/HEIGHT LIMITATIONS	9.0 KSF/2 STORY/40 FEET
OCCUPANCY	
BUSINESS OFFICE: 1:100 SF GROSS 6510 SF/100 = 65.10 OR 66	66 OCCUPANTS

Walter H. Fee 9/20/04

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CERTIFICATE OF AUTHORIZATION # 00008701

Freeman Design Group inc

DATE 6/22/04 | DRAWN BY W.H.F.

REVISIONS

SHEET A-1
OF 12
PROJECT NO