

Julius Lee Engineering

RE: 317841 - GIEBEIG HOMES - CAMPBELL RES.

**1109 Coastal Bay Blvd.
Boynton Beach, FL 33435**

Site Information:

Project Customer: GIEBEIG HOMES Project Name: 317841 Model: CAMPBELL RES.
Lot/Block: 5 Subdivision: REDFIELD
Address:
City: COLUMBIA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: BRIAN T. GIEBEIG License #: RR282811523
Address: 462 SW FAIRLINGTON CT
City: LAKE CITY, State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.1
Wind Code: ASCE 7-05 Wind Speed: 110 mph Floor Load: N/A psf
Roof Load: 40.0 psf

This package includes 29 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I4137444	CJ1	10/22/09	18	I4137461	T09	10/22/09
2	I4137445	CJ3	10/22/09	19	I4137462	T10	10/22/09
3	I4137446	CJ5	10/22/09	20	I4137463	T11	10/22/09
4	I4137447	EJ4	10/22/09	21	I4137464	T12	10/22/09
5	I4137448	EJ7	10/22/09	22	I4137465	T13	10/22/09
6	I4137449	HJ6	10/22/09	23	I4137466	T14	10/22/09
7	I4137450	HJ9	10/22/09	24	I4137467	T15	10/22/09
8	I4137451	HJ9A	10/22/09	25	I4137468	T16	10/22/09
9	I4137452	PB01	10/22/09	26	I4137469	T17	10/22/09
10	I4137453	T01	10/22/09	27	I4137470	T18	10/22/09
11	I4137454	T02	10/22/09	28	I4137471	T19	10/22/09
12	I4137455	T03	10/22/09	29	I4137472	T20	10/22/09
13	I4137456	T04	10/22/09				
14	I4137457	T05	10/22/09				
15	I4137458	T06	10/22/09				
16	I4137459	T07	10/22/09				
17	I4137460	T08	10/22/09				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2011.

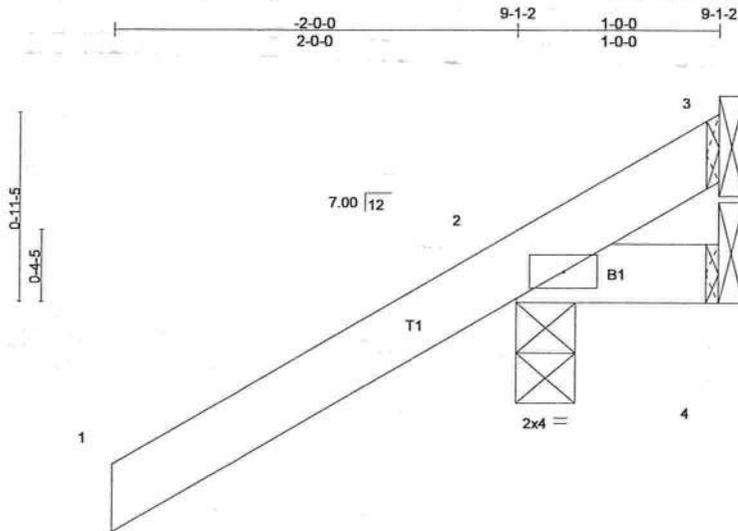
NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.





Job 317841	Truss C.J1	Truss Type JACK	Qty 14	Ply 1	GIEBIG HOMES - CAMPBELL RES.	14137444
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LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.27	Vert(LL) -0.00 2 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.01	Vert(TL) -0.00 2 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00 3 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.00 2 >999 240		Weight: 7 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

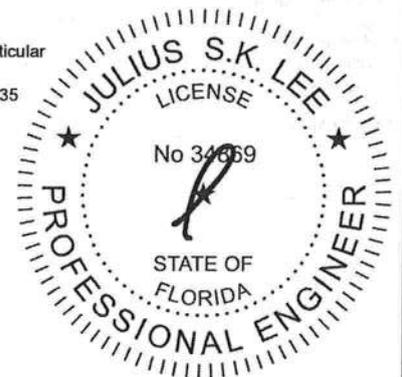
REACTIONS (lb/size) 2=289/0-3-8, 4=10/Mechanical, 3=100/Mechanical
Max Horz 2=101(LC 6)
Max Uplift 2=295(LC 6), 4=11(LC 4), 3=100(LC 1)
Max Grav 2=289(LC 1), 4=19(LC 2), 3=136(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 295 lb uplift at joint 2, 11 lb uplift at joint 4 and 100 lb uplift at joint 3.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 22, 2009

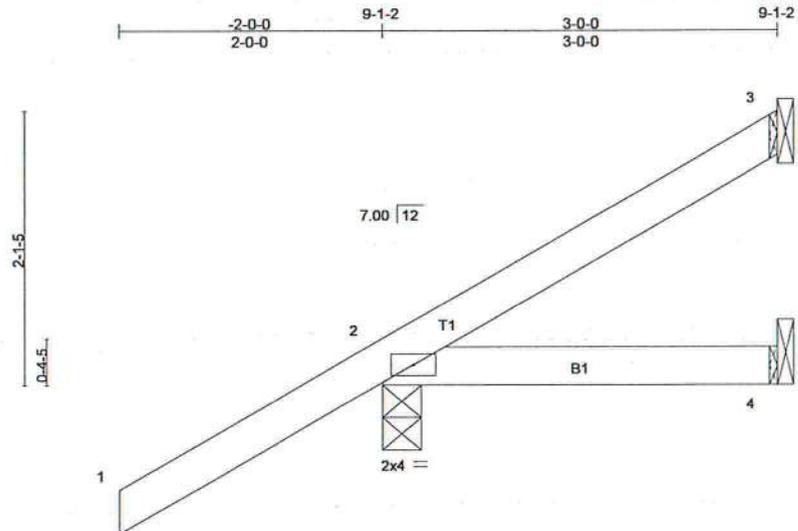
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D5B-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 317841	Truss C.J3	Truss Type JACK	Qty 14	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	I4137445
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Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 Nis Industries, Inc. Thu Oct 22 15:31:55 2009 Page 1



Scale = 1:17.0

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.28	Vert(LL) -0.00 2-4 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.08	Vert(TL) -0.01 2-4 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.01 2-4 >999 240		Weight: 13 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

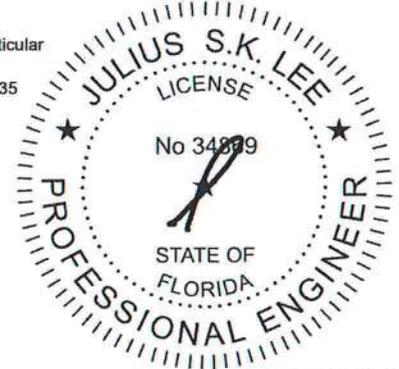
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=34/Mechanical, 2=290/0-3-8, 4=28/Mechanical
 Max Horz 2=154(LC 6)
 Max Uplift 3=31(LC 7), 2=235(LC 6), 4=33(LC 4)
 Max Grav 3=34(LC 1), 2=290(LC 1), 4=56(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (8-9)
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TC DL=4.2psf; BC DL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 3, 235 lb uplift at joint 2 and 33 lb uplift at joint 4.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



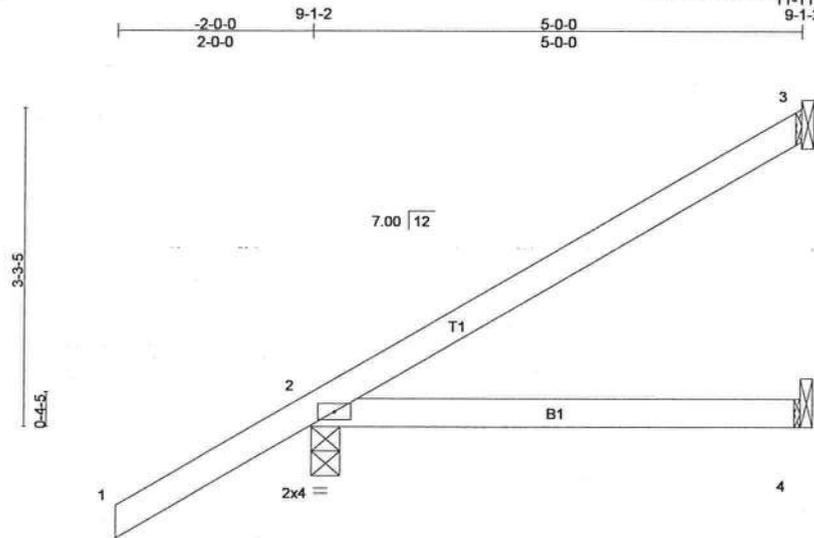
October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss CJ5	Truss Type JACK	Qty 10	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	14137446
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LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.28	Vert(LL) -0.03 2-4 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.24	Vert(TL) -0.07 2-4 >831 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.09 2-4 >663 240		Weight: 20 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

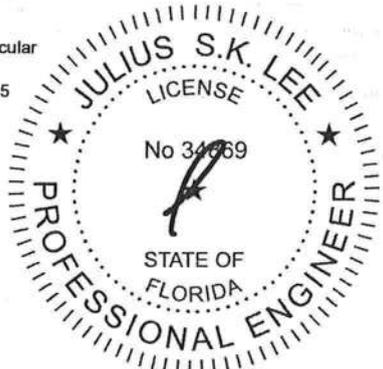
Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=115/Mechanical, 2=349/0-3-8, 4=48/Mechanical
Max Horz 2=207(LC 6)
Max Uplift 3=96(LC 6), 2=251(LC 6), 4=56(LC 4)
Max Grav 3=115(LC 1), 2=349(LC 1), 4=96(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES (8-9)**
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 3, 251 lb uplift at joint 2 and 56 lb uplift at joint 4.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



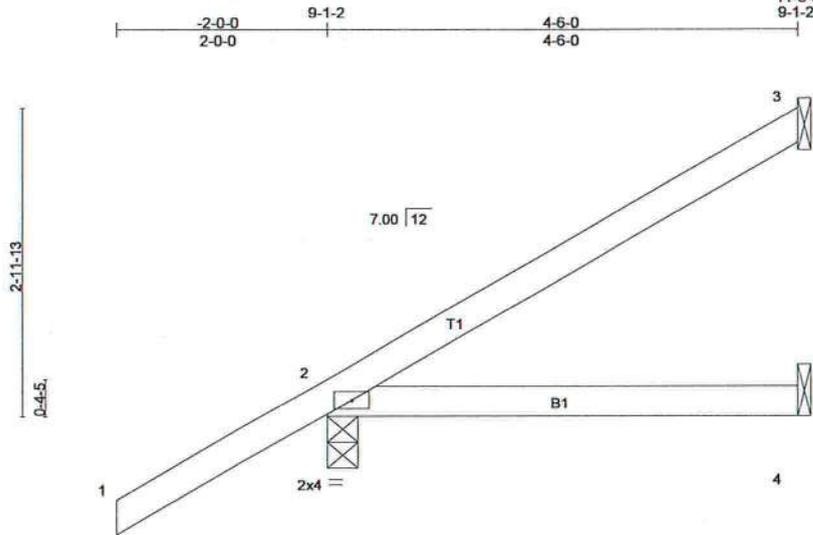
October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 317841	Truss EJ4	Truss Type JACK	Qty 8	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	I4137447
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Scale = 1:21.3

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.28	Vert(LL) -0.02 2-4 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.17	Vert(TL) -0.04 2-4 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.00 2 **** 240		
				Weight: 18 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-6-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

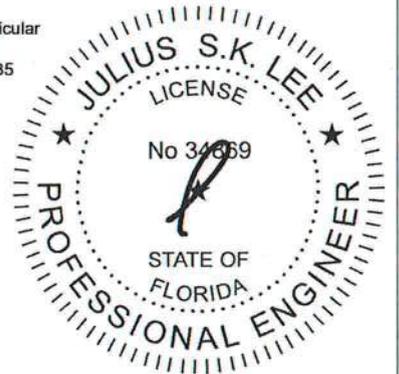
REACTIONS (lb/size) 3=97/Mechanical, 2=333/0-3-8, 4=43/Mechanical
 Max Horz 2=194(LC 6)
 Max Uplift 3=79(LC 6), 2=191(LC 6)
 Max Grav 3=97(LC 1), 2=333(LC 1), 4=86(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2 .
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 3 and 191 lb uplift at joint 2.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



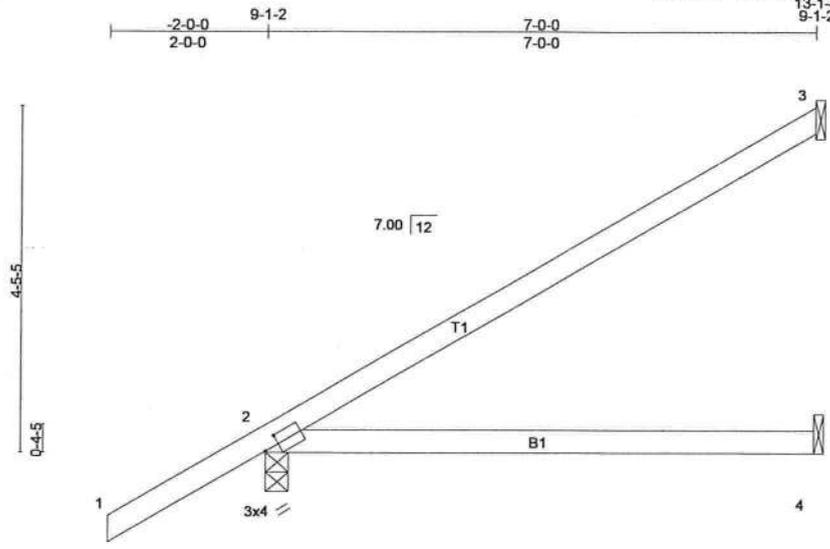
October 22, 2009

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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss EJ7	Truss Type MONO TRUSS	Qty 29	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	I4137448
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Builders FrstSource, Lake City, FL 32055 7.140 s Oct 1 2009 MITek Industries, Inc. Thu Oct 22 15:31:56 2009 Page 1
Job Reference (optional) 13-1-8 9-1-2



Scale = 1:28.4

Plate Offsets (X,Y): [2:0-2-4,0-1-8]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.47	Vert(LL) -0.09 2-4 >918 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.44	Vert(TL) -0.25 2-4 >331 240		
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.32 2-4 >258 240		Weight: 26 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD
BOT CHORD
Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

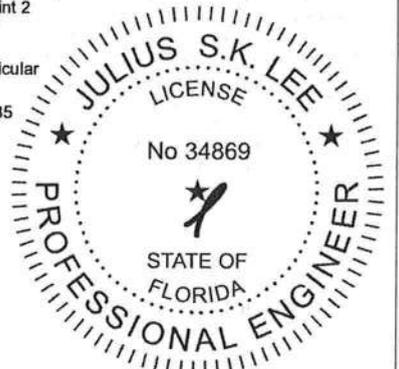
REACTIONS (lb/size) 3=177/Mechanical, 2=421/0-3-8, 4=74/Mechanical
Max Horz 2=188(LC 6)
Max Uplift 3=102(LC 6), 2=215(LC 6), 4=67(LC 4)
Max Grav 3=177(LC 1), 2=421(LC 1), 4=128(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2 .
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 3, 215 lb uplift at joint 2 and 67 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



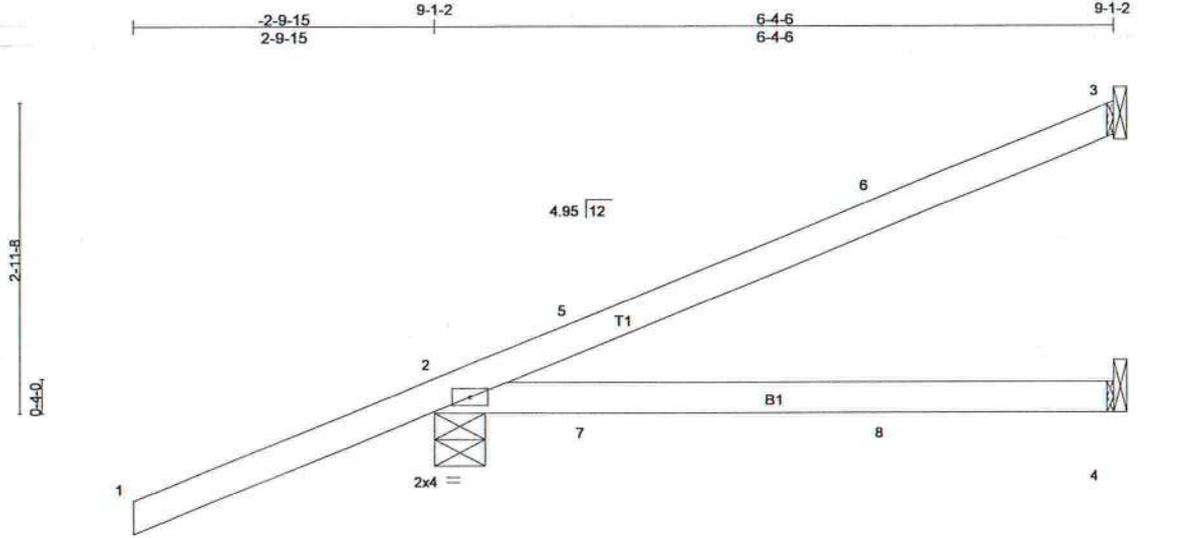
October 22, 2009

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 317841	Truss HJ6	Truss Type JACK	Qty 2	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	I4137449
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Builders FrstSource, Lake City, FL 32055 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 22 15:31:57 2009 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.60	Vert(LL) -0.07 2-4 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.41	Vert(TL) -0.18 2-4 >403 240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.08 2-4 >901 240		Weight: 24 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=79/Mechanical, 2=360/0-5-11, 4=67/Mechanical
 Max Horz 2=195(LC 5)
 Max Uplift 3=93(LC 6), 2=-377(LC 5), 4=-41(LC 3)
 Max Grav 3=79(LC 1), 2=360(LC 1), 4=134(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES (10-11)**
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 93 lb uplift at joint 3, 377 lb uplift at joint 2 and 41 lb uplift at joint 4.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 53 lb up at 1-5-12, 53 lb up at 1-5-12, and 7 lb down and 26 lb up at 4-3-11, and 7 lb down and 26 lb up at 4-3-11 on top chord, and 21 lb up at 1-5-12, 21 lb up at 1-5-12, and 16 lb down and 39 lb up at 4-3-11, and 16 lb down and 39 lb up at 4-3-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

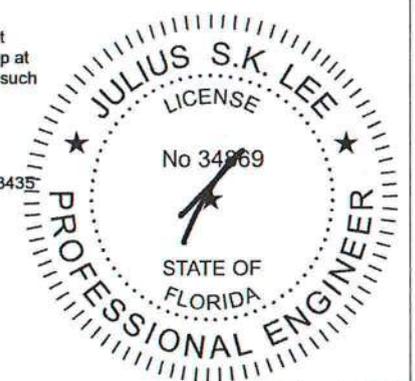
- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=60, 2-4=20

Concentrated Loads (lb)

Vert: 5=107(F=53, B=53) 6=51(F=26, B=26) 7=21(F=10, B=10) 8=16(F=-8, B=-8)



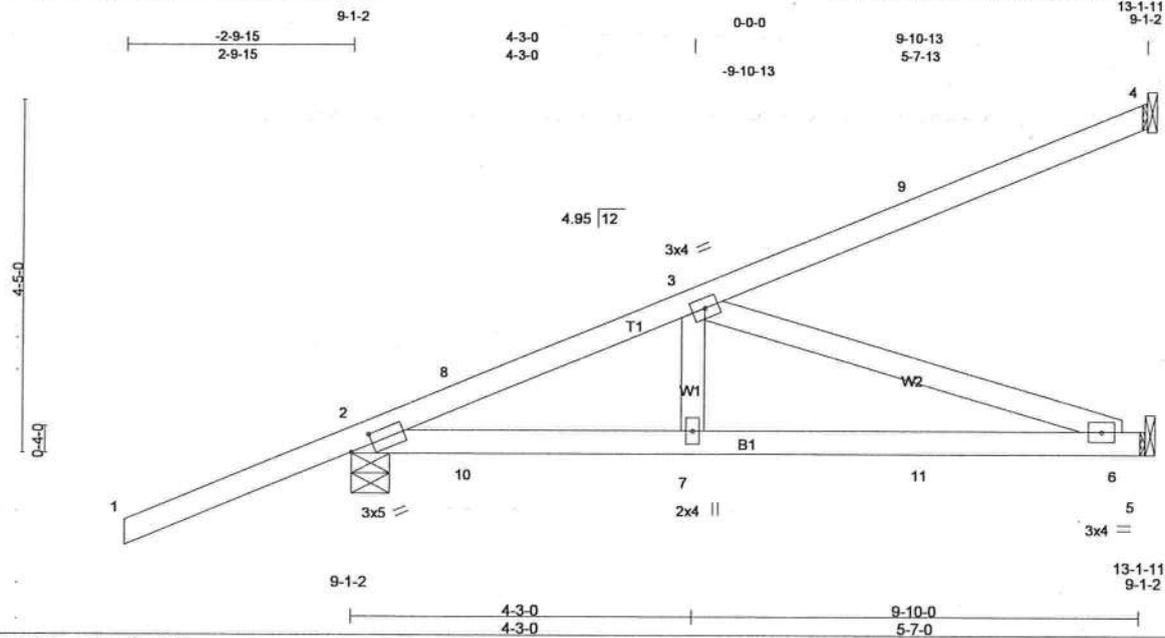
October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - CAMPBELL RES.	I4137450
317841	HJ9	MONO TRUSS	4	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055 7.140 s Oct 1 2009 MITek Industries, Inc. Thu Oct 22 15:31:57 2009 Page 1



Scale = 1:27.7

Plate Offsets (X,Y): [2:0-3-5,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.63	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.62	Vert(LL) -0.07 6-7 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.34	Vert(TL) -0.19 6-7 >606 240		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) -0.01 5 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.13 6-7 >874 240		Weight: 46 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 7-4-12 oc bracing.
WEBS 2 X 4 SYP No.3	

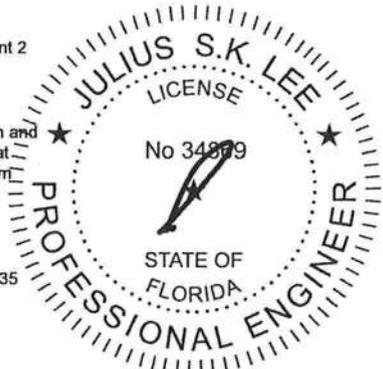
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=193/Mechanical, 2=512/0-5-11, 5=250/Mechanical
 Max Horz 2=262(LC 5)
 Max Uplift 4=158(LC 5), 2=524(LC 5), 5=274(LC 6)
 Max Grav 4=193(LC 1), 2=512(LC 1), 5=302(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-8=619/570, 3-8=632/564
 BOT CHORD 2-10=620/550, 7-10=620/550, 7-11=620/550, 6-11=620/550
 WEBS 3-7=196/330, 3-6=580/654

- NOTES (10-11)**
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 4, 524 lb uplift at joint 2 and 274 lb uplift at joint 5.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 53 lb up at 1-5-12, 53 lb up at 1-5-12, 7 lb down and 26 lb up at 4-3-11, 7 lb down and 26 lb up at 4-3-11, and 55 lb down and 84 lb up at 7-1-10, and 55 lb down and 84 lb up at 7-1-10 on top chord, and 8 lb down and 21 lb up at 1-5-12, 8 lb down and 21 lb up at 1-5-12, 16 lb down and 14 lb up at 4-3-11, 16 lb down and 14 lb up at 4-3-11, and 56 lb down and 37 lb up at 7-1-10, and 56 lb down and 37 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25



October 22, 2009

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - CAMPBELL RES.	I4137450
317841	HJ9	MONO TRUSS	4	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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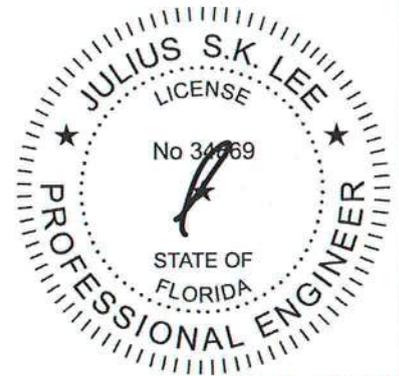
LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-60, 2-5=-20

Concentrated Loads (lb)

Vert: 3=51(F=26, B=26) 7=-16(F=-8, B=-8) 8=107(F=53, B=53) 9=-110(F=-55, B=-55) 10=21(F=10, B=10) 11=-56(F=-28, B=-28)



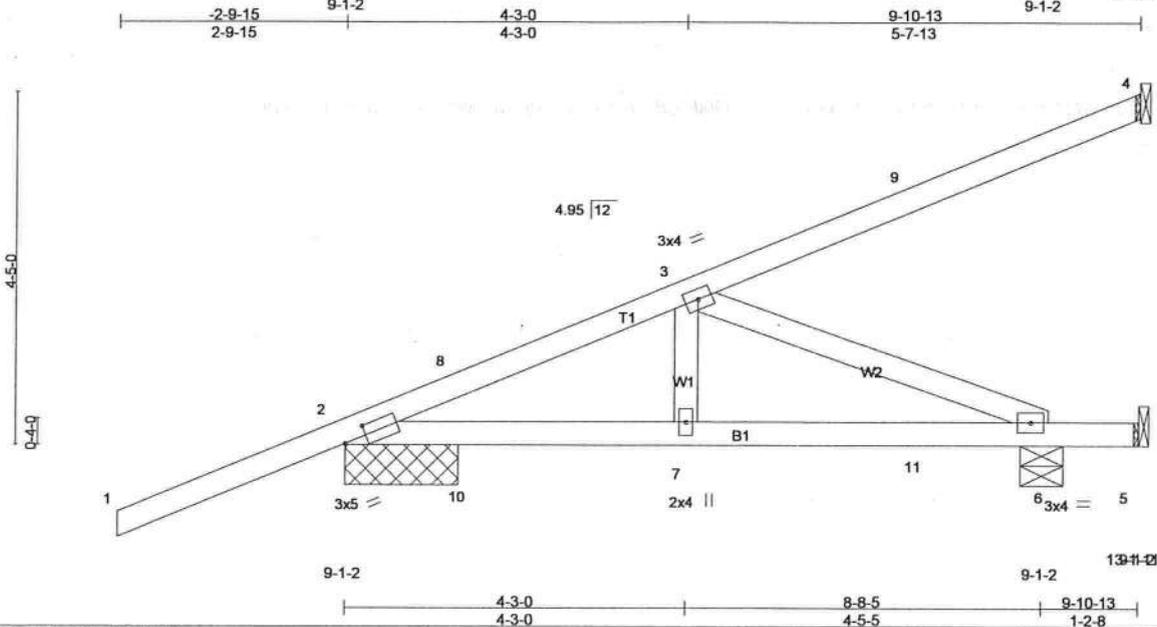
October 22, 2009

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 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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 Boynton, FL 33435

Job 317841	Truss HJ9A	Truss Type MONO TRUSS	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	14137451
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Builders FrstSource, Lake City, FL 32055 7.140 s Oct 1 2009 MITek Industries, Inc. Thu Oct 22 15:31:58 2009 Page 1 13-11-02



Scale = 1:27.7

Plate Offsets (X,Y): [2:0-3-5,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.62	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.30	Vert(LL) -0.02 6-7 >999 360		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.22	Vert(TL) -0.05 6-7 >999 240		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) -0.01 5 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.04 6-7 >999 240		Weight: 45 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD
BOT CHORD 2 X 4 SYP No.2	BOT CHORD
WEBS 2 X 4 SYP No.3	Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 8-7-8 oc bracing.

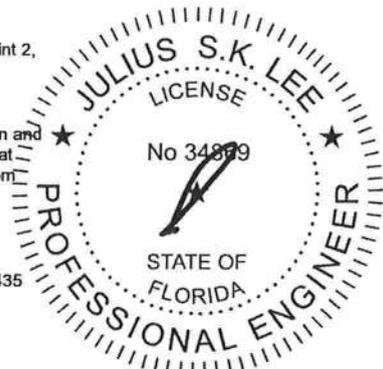
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings Mechanical except (jt=length) 2=1-5-0, 6=0-6-7.
 (lb) - Max Horz 2=262(LC 5)
 Max Uplift All uplift 100 lb or less at joint(s) except 4=155(LC 5), 2=467(LC 5), 5=115(LC 2), 6=399(LC 5)
 Max Grav All reactions 250 lb or less at joint(s) 4, 5 except 2=463(LC 1), 6=478(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-8=518/448, 3-8=471/440
 BOT CHORD 2-10=514/463, 7-10=514/463, 7-11=514/463, 6-11=514/463
 WEBS 3-6=498/552

- NOTES (10-11)**
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 4, 467 lb uplift at joint 2, 115 lb uplift at joint 5 and 399 lb uplift at joint 6.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 53 lb up at 1-5-12, 53 lb up at 1-5-12, 7 lb down and 26 lb up at 4-3-11, 7 lb down and 26 lb up at 4-3-11, and 55 lb down and 84 lb up at 7-1-10, and 55 lb down and 84 lb up at 7-1-10 on top chord, and 8 lb down and 21 lb up at 1-5-12, 8 lb down and 21 lb up at 1-5-12, 16 lb down and 14 lb up at 4-3-11, 16 lb down and 14 lb up at 4-3-11, and 56 lb down and 37 lb up at 7-1-10, and 56 lb down and 37 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25



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Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss HJ9A	Truss Type MONO TRUSS	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	I4137451
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Builders FrstSource, Lake City, FL 32055

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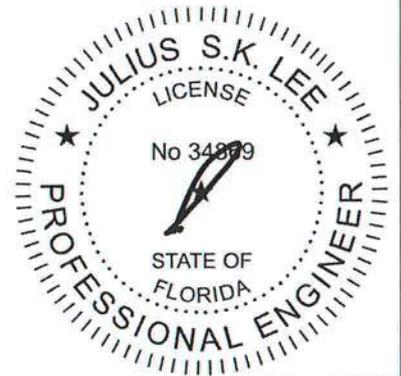
LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-60, 2-5=-20

Concentrated Loads (lb)

Vert: 3=51(F=26, B=26) 7=-16(F=-8, B=-8) 8=107(F=53, B=53) 9=-110(F=-55, B=-55) 10=21(F=10, B=10) 11=-56(F=-28, B=-28)



October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
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Job 317841	Truss PB01	Truss Type PIGGYBACK	Qty 13	Ply 1	GIEBEIG HOMES - CAMPBELL RES. 14137452
Builders FrstSource, Lake City, FL 32055					Job Reference (optional) 19-4-7

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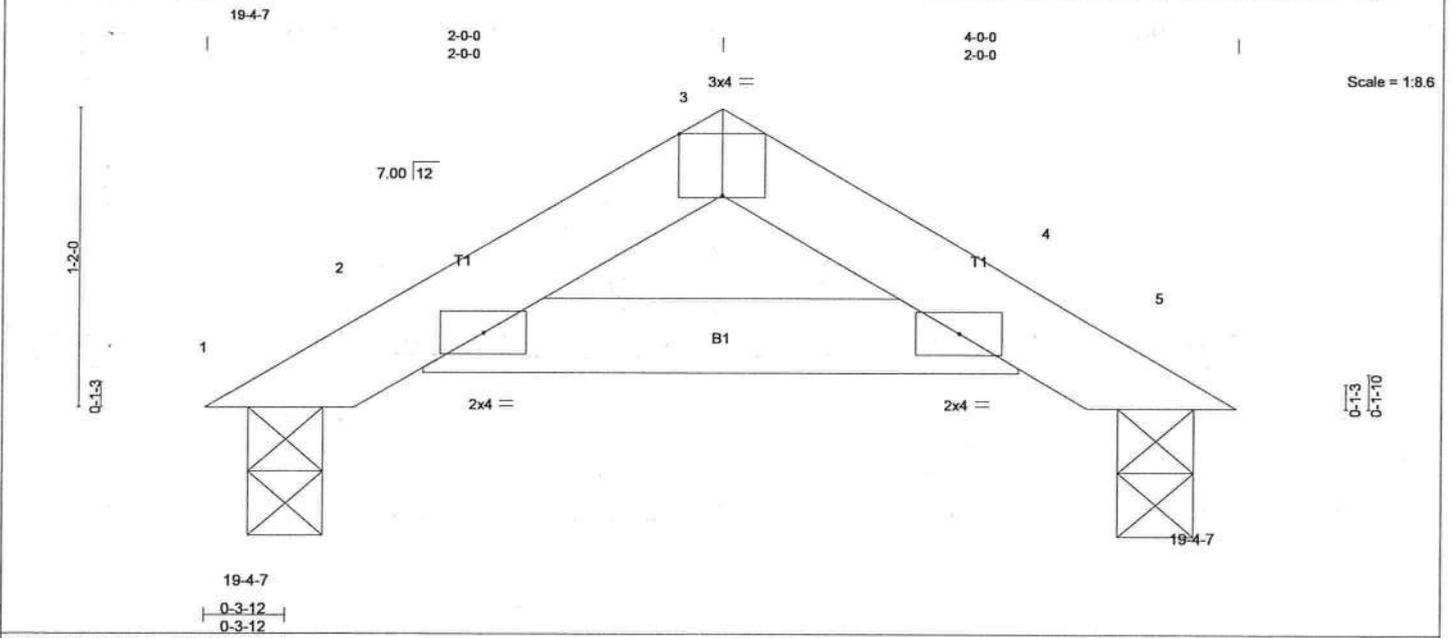


Plate Offsets (X,Y): [3:0-2:0,Edge]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.15	Vert(LL) -0.01 2-4 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.08	Vert(TL) -0.02 2-4 >999 240		
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.01 5 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.01 2-4 >999 240		Weight: 10 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=150/0-3-8, 5=150/0-3-8
Max Horz 1=-30(LC 4)
Max Uplift 1=-25(LC 6), 5=-25(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES (10-11)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 1 and 25 lb uplift at joint 5.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - SEE MITek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34889; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

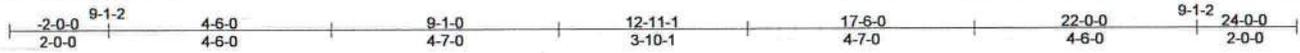


October 22, 2009

Job 317841	Truss T01	Truss Type HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	I4137453
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4x5 =

Scale = 1:45.2

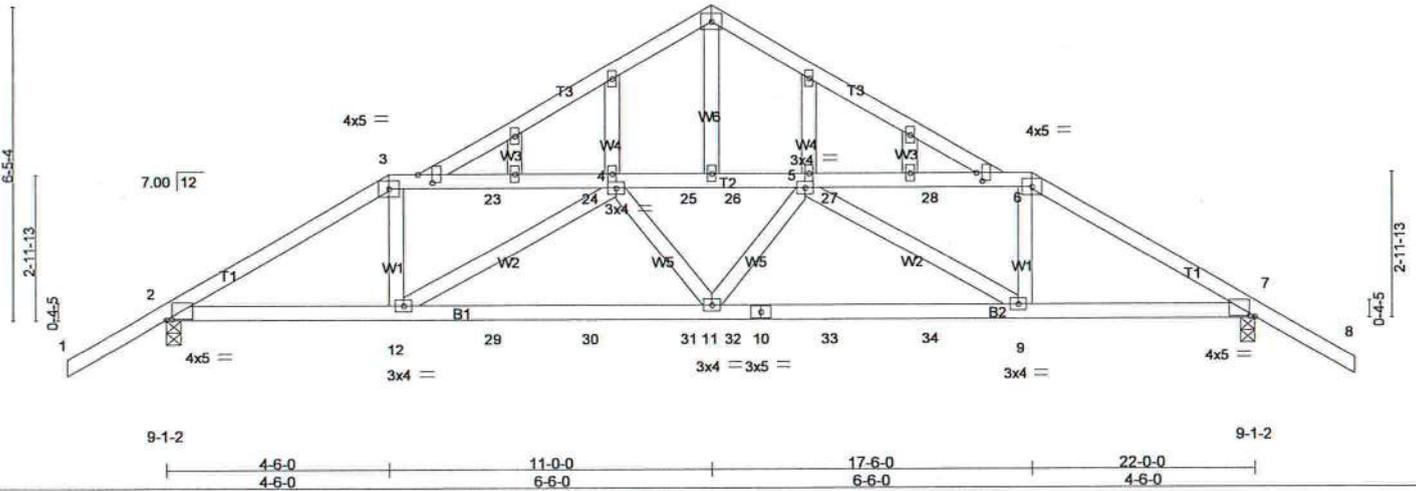


Plate Offsets (X,Y): [2:0-1-5,0-0-2], [7:0-1-5,0-0-2], [13:0-2-0,0-3-8], [15:0-2-0,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.73	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.73	Vert(LL) -0.09 11 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.49	Vert(TL) -0.28 11-12 >934 240		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.09 7 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.12 11 >999 240		Weight: 139 lb

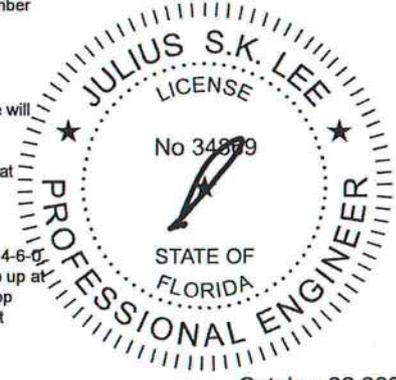
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-3-7 oc purlins. Except:
BOT CHORD 2 X 4 SYP No.2	3-9-0 oc bracing: 3-6
WEBS 2 X 4 SYP No.3	BOT CHORD Rigid ceiling directly applied or 5-11-7 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1366/0-3-8, 7=1366/0-3-8
 Max Horz 2=-73(LC 6)
 Max Uplift 2=-585(LC 5), 7=-576(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2091/891, 3-23=-1756/798, 23-24=-1755/798, 4-24=-1755/798, 4-25=-2667/1129, 25-26=-2667/1129, 5-26=-2667/1129, 5-27=-1755/799, 27-28=-1755/799, 6-28=-1756/799, 6-7=-2091/894
BOT CHORD 2-12=-767/1717, 12-29=-1138/2586, 29-30=-1138/2586, 30-31=-1138/2586, 11-31=-1138/2586, 11-32=-1127/2586, 10-32=-1127/2586, 10-33=-1127/2586, 33-34=-1127/2586, 9-34=-1127/2586, 7-9=-712/1717
WEBS 3-12=-241/794, 4-12=-1006/491, 4-11=0/277, 5-11=0/277, 5-9=-990/472, 6-9=-231/794

- NOTES (13-14)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 585 lb uplift at joint 2 and 576 lb uplift at joint 7.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 56 lb down and 148 lb up at 4-6-0, 37 lb down and 67 lb up at 6-6-12, 37 lb down and 67 lb up at 8-6-12, 37 lb down and 67 lb up at 10-6-12, 37 lb down and 67 lb up at 11-5-4, 37 lb down and 67 lb up at 13-5-4, and 37 lb down and 67 lb up at 15-5-4, and 96 lb down and 152 lb up at 17-6-0 on top chord, and 140 lb down and 40 lb up at 4-6-0, 46 lb down at 6-6-12, 46 lb down at 8-6-12, 46 lb down at 10-6-12, 46 lb down at 11-5-4, 46 lb down at 13-5-4, and 46 lb down at 15-5-4, and 140 lb down and 40 lb up at 17-5-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).



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Continued on page 2

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>Julius Lee Engineering 1109 Coastal Bay Blvd. Boynton, FL 33435</p>
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Job 317841	Truss T01	Truss Type HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	14137453
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Builders FrstSource, Lake City, FL 32055

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- 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=60, 3-6=70(F=10), 6-8=60, 2-7=20

Concentrated Loads (lb)

Vert: 3=56(F) 6=56(F) 12=70(F) 9=70(F) 23=37(F) 24=37(F) 25=37(F) 26=37(F) 27=37(F) 28=37(F) 29=23(F) 30=23(F) 31=23(F) 32=23(F) 33=23(F) 34=23(F)



October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T02	Truss Type COMMON	Qty 8	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	14137454
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Builders FrstSource, Lake City, FL 32055 7,140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 22 15:32:01 2009 Page 1

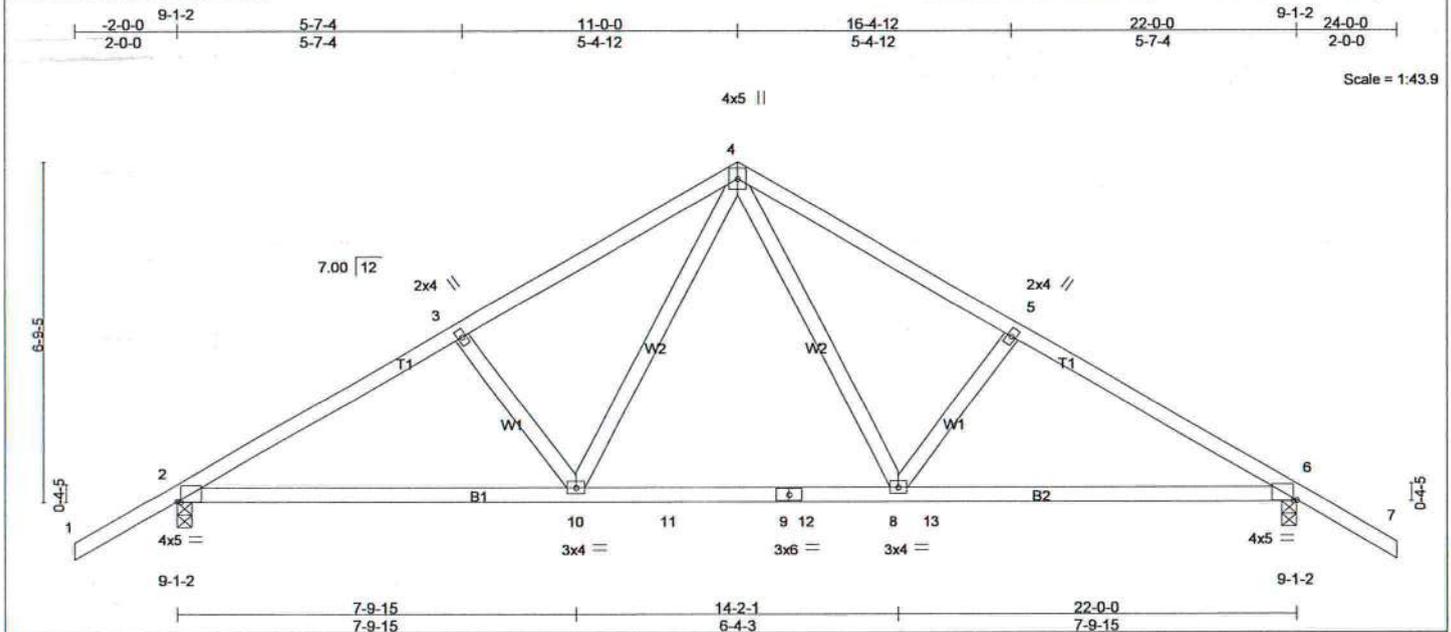


Plate Offsets (X,Y): [2:0-0-13,0-0-2], [6:0-0-13,0-0-2]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0 1.25	TC 0.30	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.81	Vert(LL) -0.21 8-10 >999 360		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.24	Vert(TL) -0.34 8-10 >772 240		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) 0.05 6 n/a n/a		
			Wind(LL) 0.10 8-10 >999 240		Weight: 110 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-5-12 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

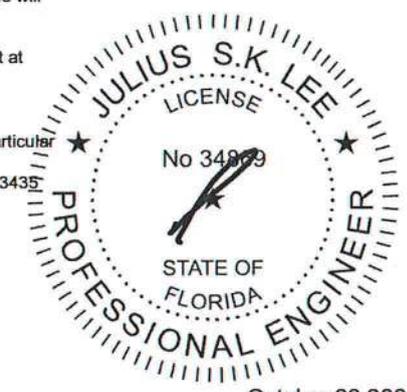
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1255/0-3-8, 6=1268/0-3-8
 Max Horz 2=-178(LC 4)
 Max Uplift 2=-274(LC 6), 6=-276(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1859/562, 3-4=-1671/567, 4-5=-1695/573, 5-6=-1883/568
 BOT CHORD 2-10=-320/1519, 10-11=-99/1058, 9-11=-99/1058, 9-12=-99/1058, 8-12=-99/1058,
 8-13=-325/1539, 6-13=-325/1539
 WEBS 4-8=-219/756, 5-8=-274/224, 4-10=-207/710, 3-10=-274/224

- NOTES** (9-10)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SYP No.2
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2 and 276 lb uplift at joint 6.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=60, 4-7=60, 2-10=20, 10-11=80(F=60), 11-12=120(F=60), 12-13=80(F=60), 6-13=20



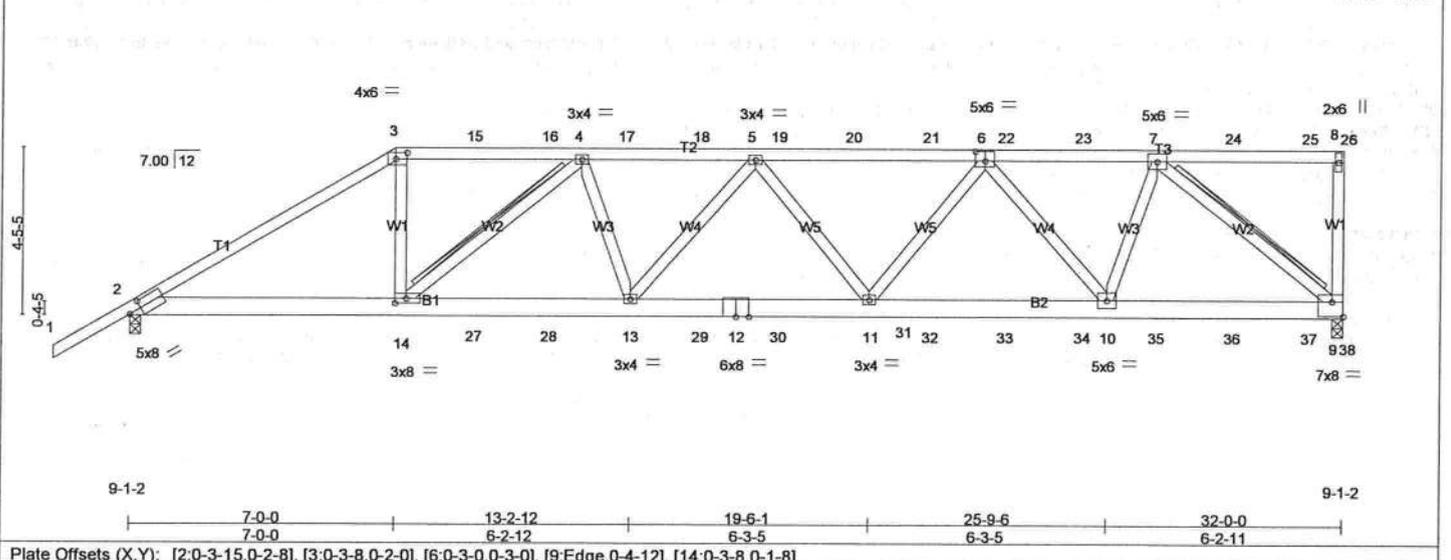
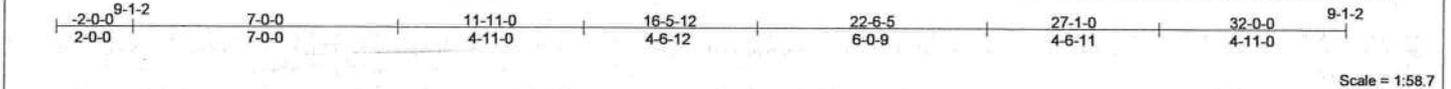
October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D5B-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T03	Truss Type MONO HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	I4137455
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Builders FrstSource, Lake City, FL 32055 Job Reference (optional) 7.140 s Oct 1 2009 MITek Industries, Inc. Thu Oct 22 15:32:02 2009 Page 1



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.92	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.62	Vert(LL) -0.20 11-13 >999 360		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.92	Vert(TL) -0.54 11-13 >706 240		
BCDL 10.0	Code FBC2007/TP12002	(Matrix)	Horz(TL) 0.13 9 n/a n/a		
			Wind(LL) 0.33 11-13 >999 240	Weight: 197 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

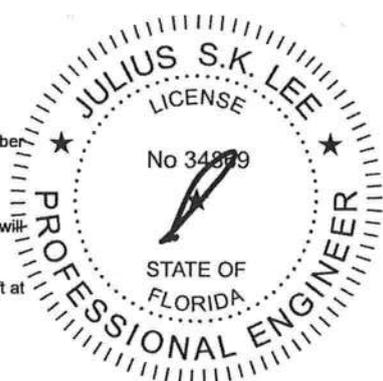
BRACING
 TOP CHORD Structural wood sheathing directly applied, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 4-9-8 oc bracing.
 WEBS T-Brace: 2 X 4 SYP No.3 - 4-14, 7-9
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

REACTIONS (lb/size) 9=2841/0-3-8, 2=2585/0-3-8
 Max Horz 2=191(LC 5)
 Max Uplift 9=1706(LC 3), 2=1444(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=4405/2587, 3-15=3750/2291, 15-16=3749/2291, 4-16=3749/2291, 4-17=5113/3041, 17-18=5113/3041, 5-18=5113/3041, 5-19=4925/2884, 19-20=4925/2884, 20-21=4925/2884, 6-21=4925/2884, 6-22=3255/1907, 22-23=3255/1907, 7-23=3255/1907, 8-9=374/216
 BOT CHORD 2-14=2258/3695, 14-27=2952/4955, 27-28=2952/4955, 13-28=2952/4955, 13-29=3092/5266, 12-29=3092/5266, 12-30=3092/5266, 30-31=3092/5266, 11-31=3092/5266, 11-32=2548/4367, 32-33=2548/4367, 33-34=2548/4367, 10-34=2548/4367, 10-35=1582/2709, 35-36=1582/2709, 36-37=1582/2709, 37-38=1582/2709, 9-38=1582/2709
 WEBS 3-14=977/1519, 4-14=1585/972, 4-13=326/586, 5-11=571/349, 6-11=579/936, 6-10=1781/1030, 7-10=1062/1784, 7-9=3508/2049

NOTES (11-12)
 1) Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCCL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
 2) Provide adequate drainage to prevent water ponding.
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 5) All bearings are assumed to be SYP No.2
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1706 lb uplift at joint 9 and 1444 lb uplift at joint 2.
 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



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Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T03	Truss Type MONO HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	14137455
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Builders FrstSource, Lake City, FL 32055

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NOTES (11-12)

- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 250 lb down and 237 lb up at 7-0-0, 117 lb down and 90 lb up at 9-0-12, 117 lb down and 90 lb up at 11-0-12, 117 lb down and 90 lb up at 13-0-12, 117 lb down and 90 lb up at 15-0-12, 117 lb down and 90 lb up at 17-0-12, 117 lb down and 90 lb up at 19-0-12, 117 lb down and 90 lb up at 21-0-12, 117 lb down and 90 lb up at 23-0-12, 117 lb down and 90 lb up at 25-0-12, 117 lb down and 90 lb up at 27-0-12, 117 lb down and 90 lb up at 29-0-12, and 117 lb down and 90 lb up at 31-0-12, and 117 lb down and 90 lb up at 31-5-4 on top chord, and 351 lb down and 353 lb up at 7-0-0, 88 lb down and 73 lb up at 9-0-12, 88 lb down and 73 lb up at 11-0-12, 88 lb down and 73 lb up at 13-0-12, 88 lb down and 73 lb up at 15-0-12, 88 lb down and 73 lb up at 17-0-12, 88 lb down and 73 lb up at 19-0-12, 88 lb down and 73 lb up at 21-0-12, 88 lb down and 73 lb up at 23-0-12, 88 lb down and 73 lb up at 25-0-12, 88 lb down and 73 lb up at 27-0-12, 88 lb down and 73 lb up at 29-0-12, and 88 lb down and 73 lb up at 31-0-12, and 88 lb down and 73 lb up at 31-5-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

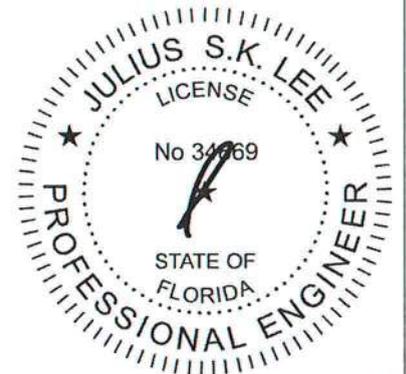
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=60, 3-8=60, 2-9=20

Concentrated Loads (lb)

Vert: 3=250(F) 14=284(F) 13=54(F) 7=117(F) 15=117(F) 16=117(F) 17=117(F) 18=117(F) 19=117(F) 20=117(F) 21=117(F) 22=117(F) 23=117(F) 24=117(F) 25=117(F) 26=117(F) 27=54(F) 28=54(F) 29=54(F) 30=54(F) 31=54(F) 32=54(F) 33=54(F) 34=54(F) 35=54(F) 36=54(F) 37=54(F) 38=54(F)

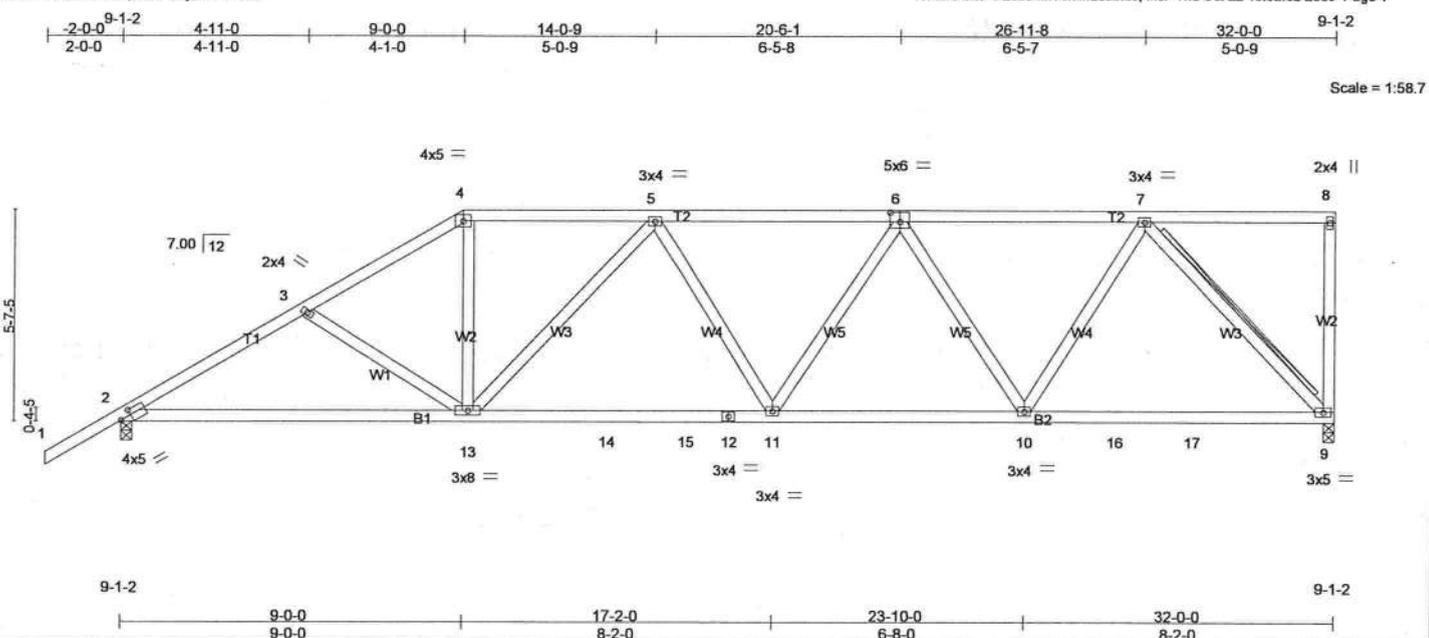


October 22, 2009

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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T04	Truss Type MONO HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	14137456
Builders FrstSource, Lake City, FL 32055					Job Reference (optional)	



Scale = 1:58.7

Plate Offsets (X,Y): [2:0-3-8,0-1-12], [6:0-3-0,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.31	Vert(LL) -0.19 11-13 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.63	Vert(TL) -0.42 2-13 >912 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.51	Horz(TL) 0.09 9 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.10 11-13 >999 240		Weight: 179 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-1-11 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-11-4 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS T-Brace: 2 X 4 SYP No.3 - 7-9
	Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
	Brace must cover 90% of web length.

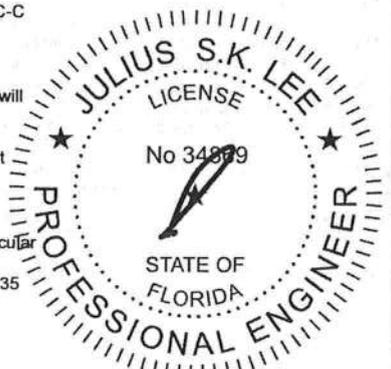
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 9=1368/0-3-8, 2=1459/0-3-8
 Max Horz 2=227(LC 6)
 Max Uplift 9=-320(LC 4), 2=-264(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-2213/777, 3-4=-1971/715, 4-5=-1662/667, 5-6=-2001/779, 6-7=-1535/580
 BOT CHORD 2-13=-813/1826, 13-14=-819/1998, 14-15=-819/1998, 12-15=-819/1998, 11-12=-819/1998,
 10-11=-760/1893, 10-16=-440/1076, 16-17=-440/1076, 9-17=-440/1076
 WEBS 4-13=-201/712, 5-13=-496/249, 6-10=-674/340, 7-10=-276/907, 7-9=-1558/647

- NOTES (9-10)**
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) Provide adequate drainage to prevent water ponding.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 5) All bearings are assumed to be SYP No.2.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 320 lb uplift at joint 9 and 264 lb uplift at joint 2.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T05	Truss Type HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	14137457
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Builders FrstSource, Lake City, FL 32055

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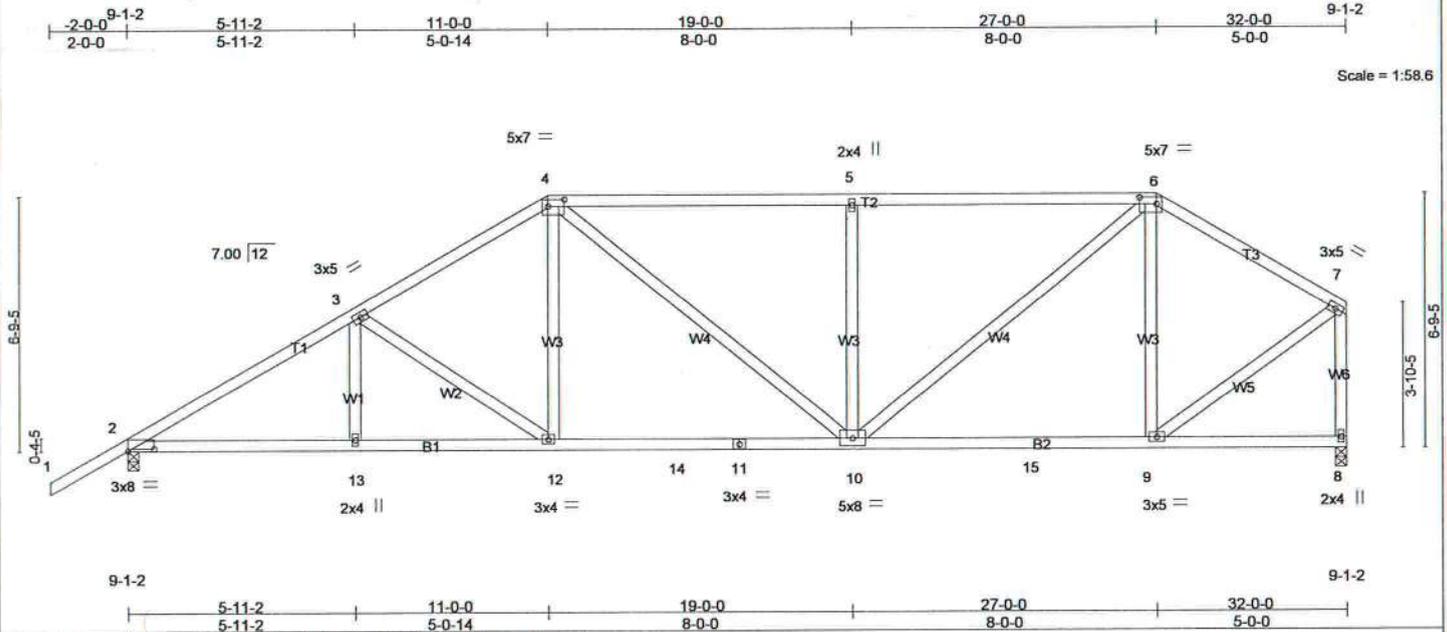


Plate Offsets (X,Y): [2:0-8-1,0-0-10], [4:0-5-4,0-2-4], [6:0-5-4,0-2-4]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.56	Vert(LL) -0.13 10-12 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.58	Vert(TL) -0.32 10-12 >999 240		
BCLL 0.0	Rep Stress Incr YES	WB 0.71	Horz(TL) 0.07 8 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.08 10-12 >999 240		Weight: 187 lb

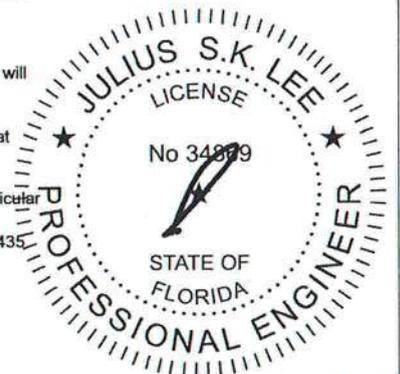
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-11-4 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 7-5-13 oc bracing.
WEBS 2 X 4 SYP No.3 *Except* W6: 2 X 4 SYP No.2	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1505/0-3-8, 8=1416/0-3-8
Max Horz 2=207(LC 6)
Max Uplift 2=282(LC 6), 8=233(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=2306/757, 3-4=1925/710, 4-5=1754/734, 5-6=1754/734, 6-7=1192/436,
7-8=1393/510
BOT CHORD 2-13=714/1897, 12-13=714/1897, 12-14=543/1603, 11-14=543/1603, 10-11=543/1603,
10-15=305/963, 9-15=305/963
WEBS 3-12=367/208, 4-12=81/457, 4-10=197/313, 5-10=551/352, 6-10=392/1047,
6-9=442/260, 7-9=370/1184

- NOTES** (9-10)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 6) All bearings are assumed to be SYP No.2
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 282 lb uplift at joint 2 and 233 lb uplift at joint 8.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 317841	Truss T06	Truss Type HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	4137458
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Builders FrstSource, Lake City, FL 32055 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 22 15:32:04 2009 Page 1

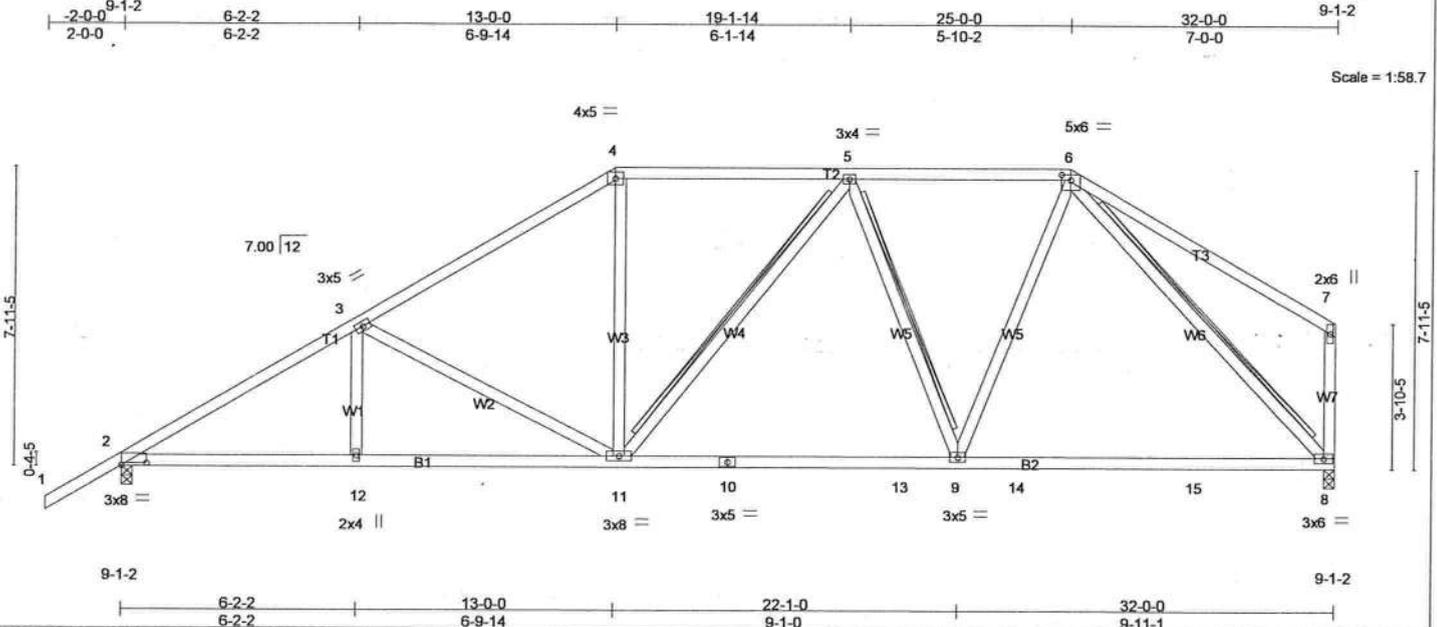


Plate Offsets (X,Y): [2:0-8-1,0-0-10], [6:0-3-0,0-1-12]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.37	Vert(LL) -0.22 8-9 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.48	Vert(TL) -0.50 8-9 >768 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.92	Horz(TL) 0.07 8 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.07 11-12 >999 240		Weight: 188 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP M 31
WEBS 2 X 4 SYP No.3 *Except*
W7: 2 X 4 SYP No.2

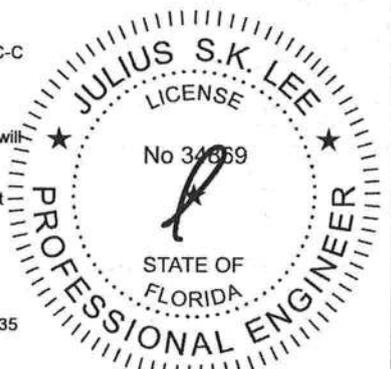
BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-2-9 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-11, 5-9, 6-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1514/0-3-8, 8=1520/0-3-8
Max Horz 2=228(LC 5)
Max Uplift 2=-294(LC 6), 8=-178(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2335/755, 3-4=-1827/658, 4-5=-1490/638, 5-6=-1394/559
BOT CHORD 2-12=-716/1927, 11-12=-716/1927, 10-11=-471/1522, 10-13=-471/1522, 9-13=-471/1522, 9-14=-322/1087, 14-15=-322/1087, 8-15=-322/1087
WEBS 3-12=0/268, 3-11=-506/287, 4-11=-101/504, 5-9=-408/245, 6-9=-175/905, 6-8=-1516/445

- NOTES (10-11)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 294 lb uplift at joint 2 and 178 lb uplift at joint 8.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



October 22, 2009

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 317841	Truss T07	Truss Type SPECIAL	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	14137459
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Builders FirstSource, Lake City, FL 32055 7,140 s Oct 1 2009 MITEK Industries, Inc. Thu Oct 22 15:32:05 2009 Page 1

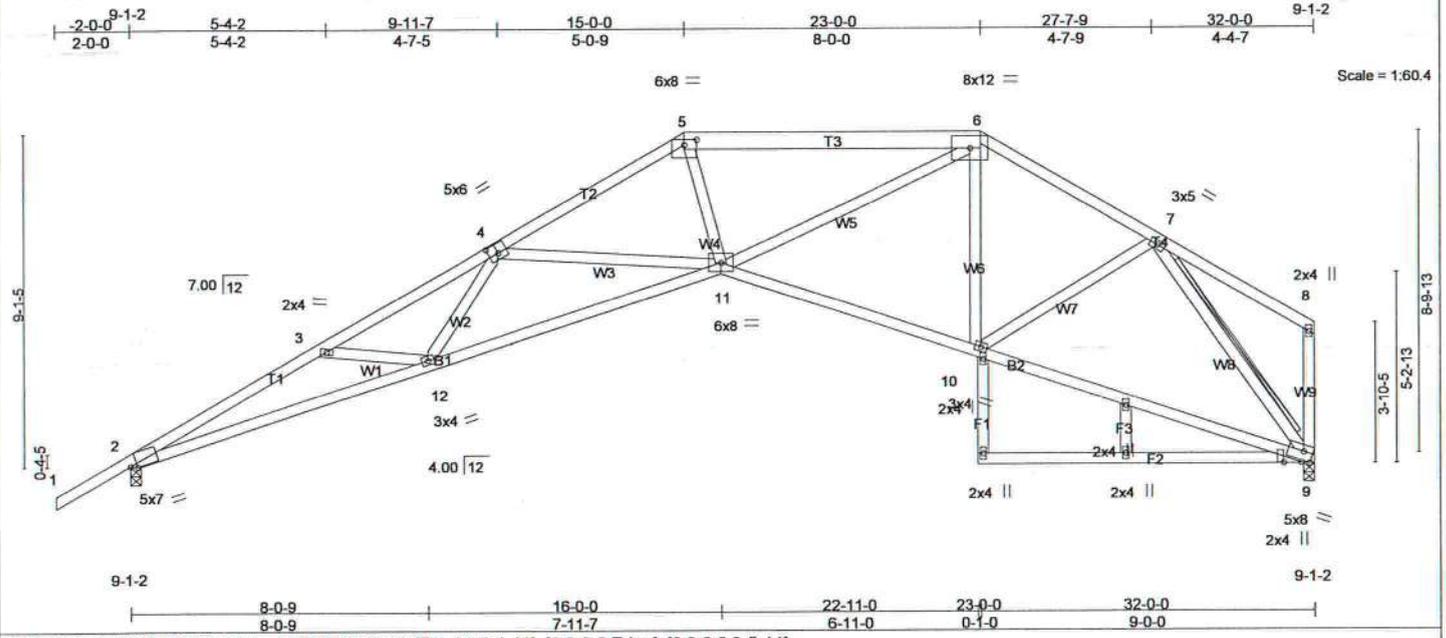


Plate Offsets (X,Y): [2:0-2-2,Edge], [4:0-3-0,0-3-0], [5:0-4-0,0-1-11], [9:0-3-0,Edge], [9:0-0-0,5-11]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.44	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.86	Vert(LL) -0.29 11-12 >999 360		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.55	Vert(TL) -0.82 11-12 >465 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.56 9 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.33 11-12 >999 240	Weight: 199 lb	

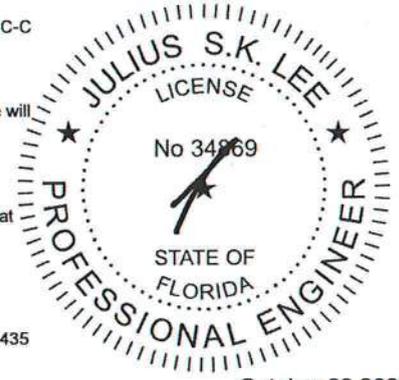
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2 *Except* T3: 2 X 6 SYP No.1D	TOP CHORD Structural wood sheathing directly applied or 2-8-2 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 4-11-4 oc bracing.
WEBS 2 X 4 SYP No.3 *Except* W9: 2 X 4 SYP No.2	WEBS T-Brace: 2 X 4 SYP No.3 - 7-9 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1401/0-3-8, 9=1264/0-3-8
Max Horz 2=260(LC 5)
Max Uplift 2=303(LC 6), 9=175(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-4366/1692, 3-4=-4064/1540, 4-5=-2981/1160, 5-6=-2817/1151, 6-7=-1717/715
BOT CHORD 2-12=-1597/3865, 11-12=-1340/3466, 10-11=-475/1514, 9-10=-397/1039
WEBS 4-12=-59/513, 4-11=-761/431, 5-11=-348/1106, 6-11=-543/1564, 6-10=-279/170, 7-10=-144/571, 7-9=-1659/656

- NOTES** (11-12)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Bearing at joint(s) 2, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at joint 2 and 175 lb uplift at joint 9.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



LOAD CASE(S) Standard October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 317841	Truss T08	Truss Type SPECIAL	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	14137460
Builders FrstSource, Lake City, FL 32055					Job Reference (optional)	

7.140 s Oct 1 2009 MITek Industries, Inc. Thu Oct 22 15:32:06 2009 Page 1

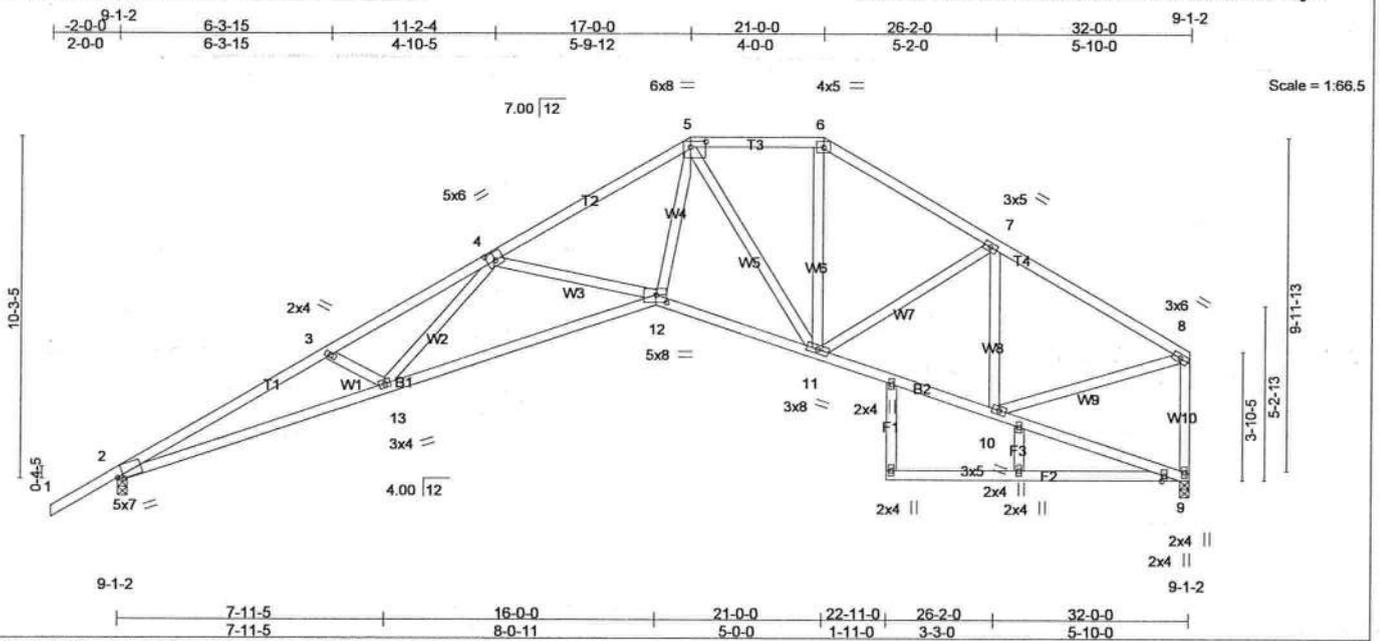


Plate Offsets (X,Y): [2:0-2-2,Edge], [4:0-3-0,0-3-0], [5:0-5-8,0-2-0], [12:0-3-12,0-2-12]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.54	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.81	Vert(LL) -0.29 12-13 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 1.00	Vert(TL) -0.85 12-13 >447 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.57 9 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.34 12-13 >999 240		Weight: 201 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W10: 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-0-5 oc bracing. Except: 9-6-0 oc bracing: 9-11

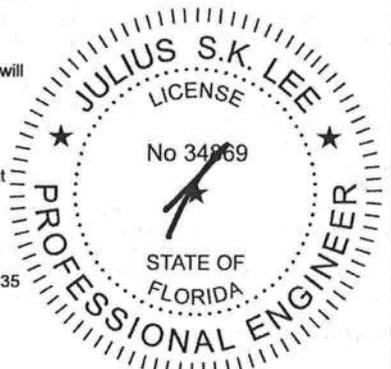
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1401/0-3-8, 9=1264/0-3-8
Max Horz 2=294(LC 5)
Max Uplift 2=311(LC 6), 9=187(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=4374/1642, 3-4=4093/1568, 4-5=2882/1102, 5-6=1442/686, 6-7=1745/735, 7-8=1469/568, 8-9=1219/491
BOT CHORD 2-13=1546/3872, 12-13=1240/3297, 11-12=626/2101, 10-11=445/1285
WEBS 4-13=187/670, 4-12=731/451, 5-12=688/2054, 5-11=1036/347, 6-11=219/589, 7-11=137/330, 7-10=646/301, 8-10=423/1235

- NOTES (10-11)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2 .
 - Bearing at joint(s) 2, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 187 lb uplift at joint 9.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 317841	Truss T09	Truss Type SPECIAL	Qty 2	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	14137461
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Builders FrstSource, Lake City, FL 32055 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 22 15:32:07 2009 Page 1

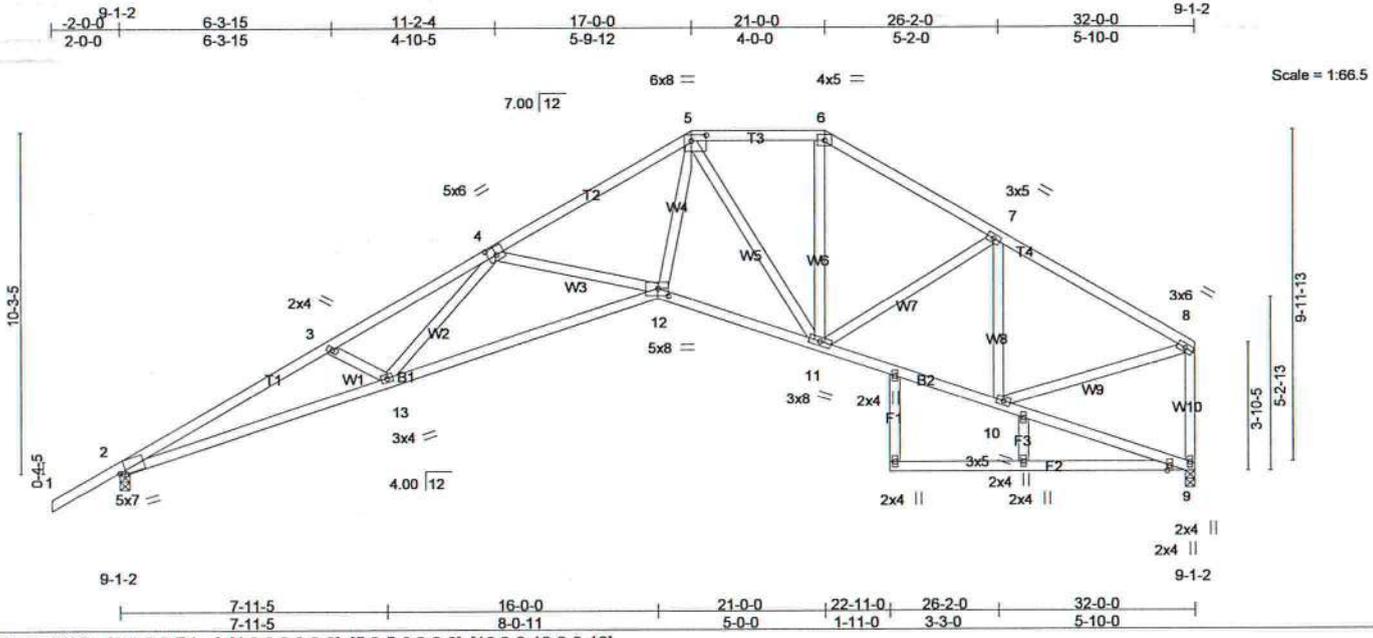


Plate Offsets (X,Y): [2:0-2-2,Edge], [4:0-3-0,0-3-0], [5:0-5-8,0-2-0], [12:0-3-12,0-2-12]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.54	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.81	Vert(LL) -0.29 12-13 >999 360		
BCLL 0.0 *	Lumber Increase 1.25	WB 1.00	Vert(TL) -0.85 12-13 >447 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.57 9 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.34 12-13 >999 240	Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W10: 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-15 oc purlins, except end verticals, and 2-0-0 oc purlins (5-2-5 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 5-0-5 oc bracing. Except: 9-6-0 oc bracing: 9-11

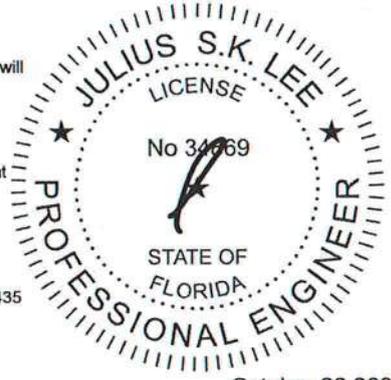
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1401/0-3-8, 9=1264/0-3-8
Max Horz 2=294(LC 5)
Max Uplift 2=311(LC 6), 9=187(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-4374/1642, 3-4=-4093/1568, 4-5=-2882/1102, 5-6=-1442/686, 6-7=-1745/735, 7-8=-1469/568, 8-9=-1219/491
BOT CHORD 2-13=-1546/3872, 12-13=-1240/3297, 11-12=-626/2101, 10-11=-445/1285
WEBS 4-13=-187/670, 4-12=-731/451, 5-12=-688/2054, 5-11=-1036/347, 6-11=-219/589, 7-11=-137/330, 7-10=-646/301, 8-10=-423/1235

- NOTES (11-12)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2 .
 - Bearing at joint(s) 2, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 187 lb uplift at joint 9.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 22, 2009

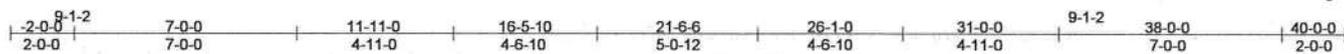
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

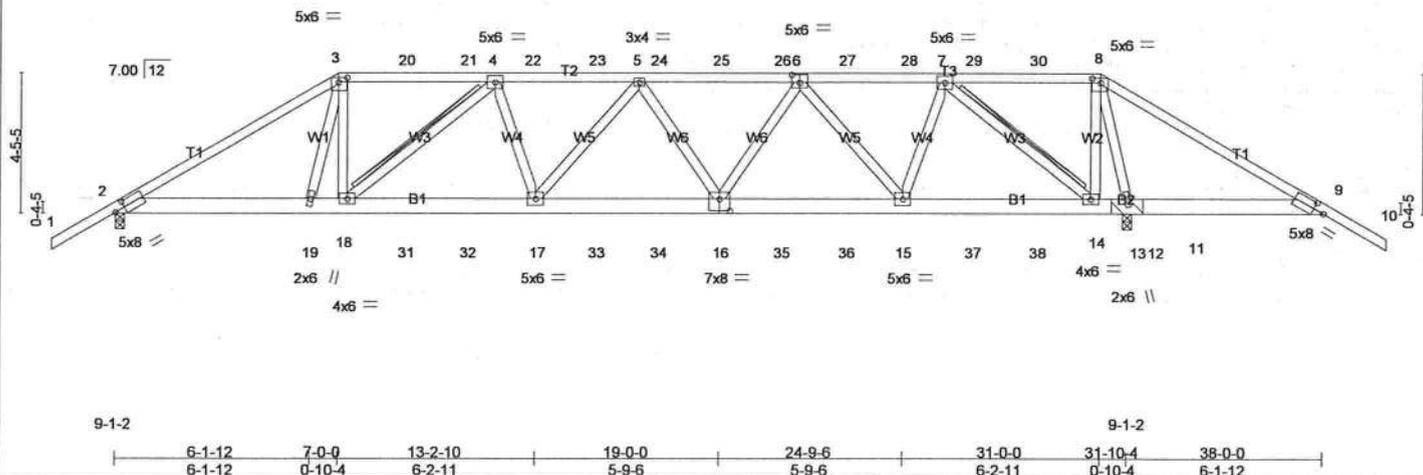
Job 317841	Truss T10	Truss Type HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	M4137462
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Builders FrstSource, Lake City, FL 32055

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Scale = 1:70.2



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.90	Vert(LL) -0.21 16-17 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.61	Vert(TL) -0.53 16-17 >714 240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.88	Horz(TL) 0.13 12 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.34 16-17 >999 240		Weight: 238 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 4-9-2 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-18, 7-14
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=2519/0-3-8, 12=3396/0-4-0 (0-3-8 + bearing block)
Max Horz 2=-111(LC 3)
Max Uplift 2=-1453(LC 5), 12=-1997(LC 6)
Max Grav 2=2561(LC 9), 12=3396(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=4369/2630, 3-20=3703/2354, 20-21=3703/2354, 4-21=3702/2353, 4-22=5033/3105, 22-23=5033/3105, 5-23=5033/3105, 5-24=4896/2996, 24-25=4896/2996, 25-26=4896/2996, 6-26=4896/2996, 6-27=3478/2140, 27-28=3478/2140, 7-28=3478/2140, 7-29=440/404, 29-30=441/404, 8-30=442/404, 8-9=374/708
BOT CHORD 2-19=2282/3663, 18-19=2297/3650, 18-31=3004/4890, 31-32=3004/4890, 17-32=3004/4890, 17-33=3136/5156, 33-34=3136/5156, 16-34=3136/5156, 16-35=2710/4474, 35-36=2710/4474, 15-36=2710/4474, 15-37=1814/2975, 37-38=1814/2975, 14-38=1814/2975, 13-14=266/377, 12-13=266/377, 11-12=530/443, 9-11=530/443
WEBS 3-18=1114/1471, 4-18=1560/958, 4-17=303/541, 5-16=542/304, 6-16=504/845, 6-15=1653/947, 7-15=998/1700, 7-14=3378/1984, 8-14=1146/1788, 8-12=2754/1599

- NOTES (13-14)**
- 2 X 6 SYP No.1D bearing block 12" long at jt. 12 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. Bearing is assumed to be SYP.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1453 lb uplift at joint 2 and 1997 lb uplift at joint 12



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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - CAMPBELL RES.	14137462
317841	T10	HIP	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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NOTES (13-14)

- 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 250 lb down and 237 lb up at 7-0-0, 117 lb down and 90 lb up at 9-0-12, 117 lb down and 90 lb up at 11-0-12, 117 lb down and 90 lb up at 13-0-12, 117 lb down and 90 lb up at 15-0-12, 117 lb down and 90 lb up at 17-0-12, 117 lb down and 90 lb up at 19-0-0, 117 lb down and 90 lb up at 20-11-4, 117 lb down and 90 lb up at 22-11-4, 117 lb down and 90 lb up at 24-11-4, 117 lb down and 90 lb up at 26-11-4, and 117 lb down and 90 lb up at 28-11-4, and 287 lb down and 238 lb up at 31-0-0 on top chord, and 351 lb down and 353 lb up at 7-0-0, 88 lb down and 73 lb up at 9-0-12, 88 lb down and 73 lb up at 11-0-12, 88 lb down and 73 lb up at 13-0-12, 88 lb down and 73 lb up at 15-0-12, 88 lb down and 73 lb up at 17-0-12, 88 lb down and 73 lb up at 19-0-0, 88 lb down and 73 lb up at 20-11-4, 88 lb down and 73 lb up at 22-11-4, 88 lb down and 73 lb up at 24-11-4, 88 lb down and 73 lb up at 26-11-4, and 88 lb down and 73 lb up at 28-11-4, and 7 lb down and 99 lb up at 30-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

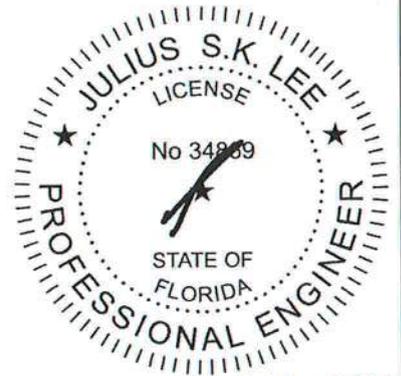
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-60, 3-8=-60, 8-10=-60, 2-9=-20

Concentrated Loads (lb)

Vert: 3=-250(B) 8=-247(B) 16=-54(B) 18=-284(B) 17=-54(B) 15=-54(B) 14=27(B) 20=-117(B) 21=-117(B) 22=-117(B) 23=-117(B) 24=-117(B) 25=-117(B) 26=-117(B) 27=-117(B) 28=-117(B) 29=-117(B) 30=-117(B) 31=-54(B) 32=-54(B) 33=-54(B) 34=-54(B) 35=-54(B) 36=-54(B) 37=-54(B) 38=-54(B)



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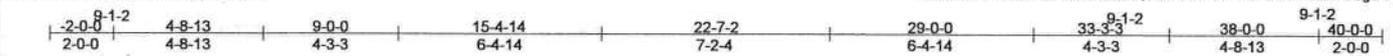
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-B9 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T11	Truss Type HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	4137463
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Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MITek Industries, Inc. Thu Oct 22 15:32:10 2009 Page 1



Scale = 1:70.2

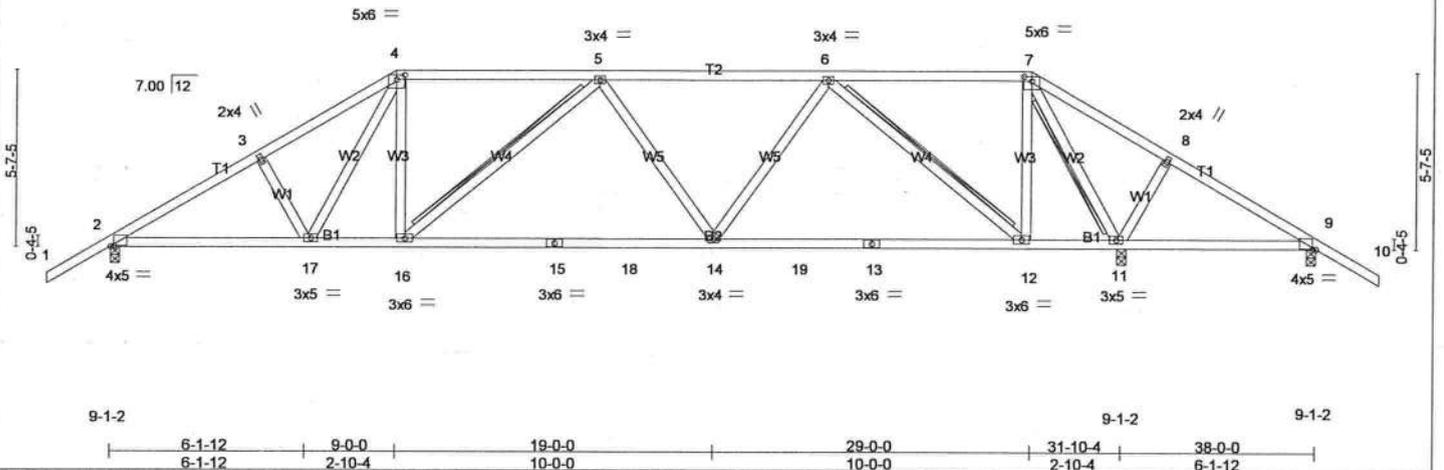


Plate Offsets (X,Y): [2:0-1-5,0-0-2], [4:0-3-0,0-1-12], [7:0-3-0,0-1-12], [9:0-1-5,0-0-2]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	TC 0.35	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.74	Vert(LL) -0.18 14-16 >999 360		
BCLL 0.0	Lumber Stress Incr YES	WB 0.60	Vert(TL) -0.49 14-16 >778 240		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) 0.08 11 n/a n/a		
			Wind(LL) 0.08 14 >999 240		Weight: 207 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
 BOT CHORD
 WEBS

Structural wood sheathing directly applied or 4-3-8 oc purlins.
 Rigid ceiling directly applied or 6-0-0 oc bracing.
 T-Brace: 2 X 4 SYP No.3 - 5-16, 6-12, 7-11
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

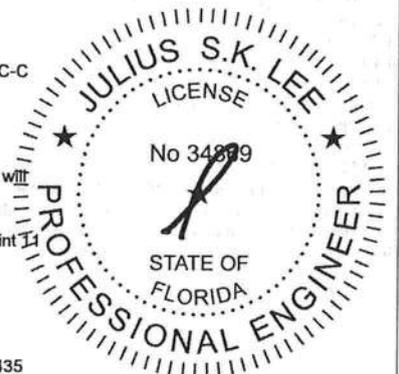
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1382/0-3-8, 11=2154/0-3-8, 9=77/0-3-8
 Max Horz 2=145(LC 5)
 Max Uplift 2=268(LC 5), 11=505(LC 4), 9=284(LC 10)
 Max Grav 2=1382(LC 1), 11=2154(LC 1), 9=37(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=2080/735, 3-4=1943/757, 4-5=1535/647, 5-6=1713/689, 6-7=257/185, 7-8=205/951, 8-9=228/831
BOT CHORD 2-17=475/1712, 16-17=370/1522, 15-16=488/1833, 15-18=488/1833, 14-18=488/1833, 14-19=345/1363, 13-19=345/1363, 12-13=345/1363, 9-11=686/322
WEBS 4-16=114/516, 5-16=481/241, 6-14=113/627, 6-12=1479/578, 7-12=284/1141, 7-11=2113/632, 8-11=256/201

NOTES (10-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SYP No.2
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 268 lb uplift at joint 2, 505 lb uplift at joint 11 and 284 lb uplift at joint 9.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



October 22, 2009

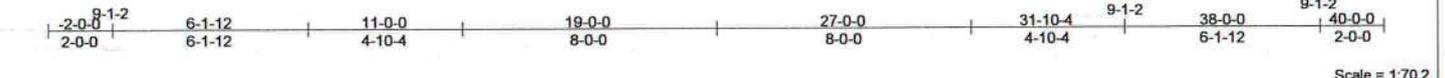
LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T12	Truss Type HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	14137464
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Builders FirstSource, Lake City, FL 32055 7,140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 22 15:32:11 2009 Page 1



Scale = 1:70.2

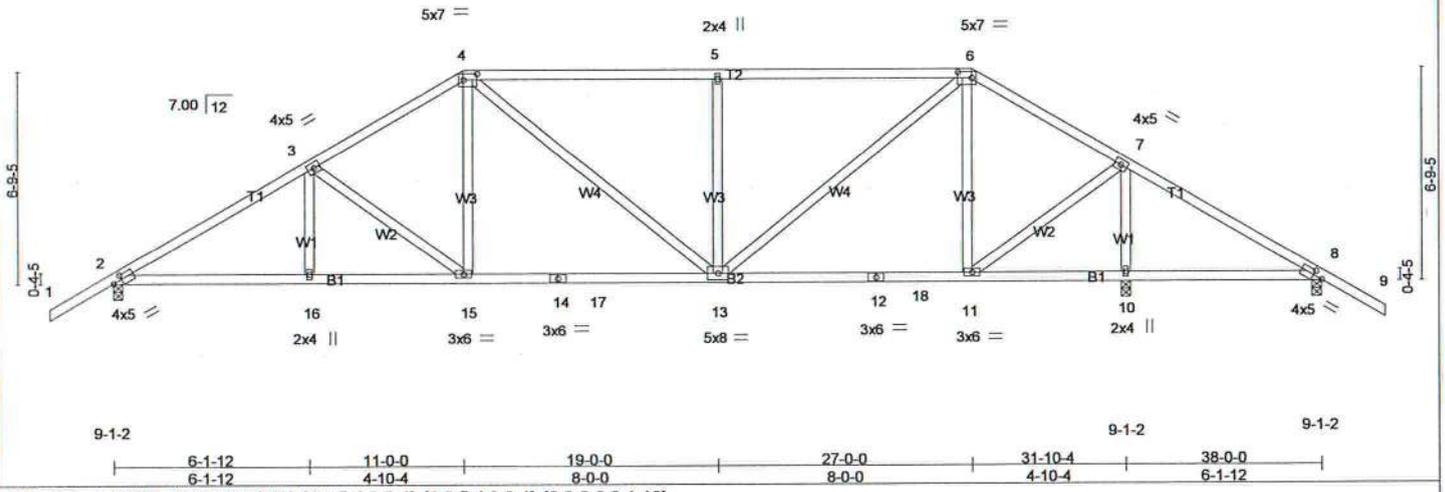


Plate Offsets (X,Y): [2:0-3-8,0-1-12], [4:0-5-4,0-2-4], [6:0-5-4,0-2-4], [8:0-3-8,0-1-12]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.56	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.56	Vert(LL) -0.13 13-15 >999 360		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.74	Vert(TL) -0.31 13-15 >999 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.06 10 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.07 13 >999 240		Weight: 210 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

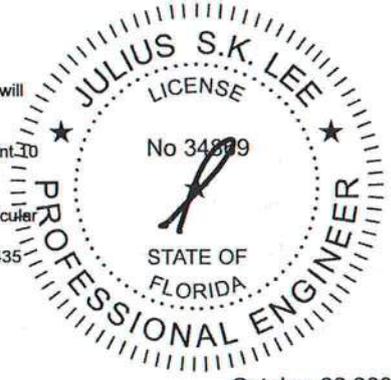
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1448/0-3-8, 10=1992/0-3-8, 8=90/0-3-8
Max Horz 2=178(LC 5)
Max Uplift 2=280(LC 6), 10=381(LC 4), 8=231(LC 7)
Max Grav 2=1448(LC 1), 10=1992(LC 1), 8=131(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=2188/740, 3-4=1810/697, 4-5=1590/713, 5-6=1590/713, 6-7=932/407, 7-8=117/601
BOT CHORD 2-16=464/1794, 15-16=464/1794, 14-15=340/1506, 14-17=340/1506, 13-17=340/1506, 13-18=111/721, 12-18=111/721, 11-12=111/721, 10-11=448/239, 8-10=448/239
WEBS 3-15=369/210, 4-15=87/465, 5-13=553/355, 6-13=412/1140, 6-11=609/267, 7-11=347/1442, 7-10=1884/659

- NOTES (9-10)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SYP No.2
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2, 381 lb uplift at joint 10 and 231 lb uplift at joint 8.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D5B-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

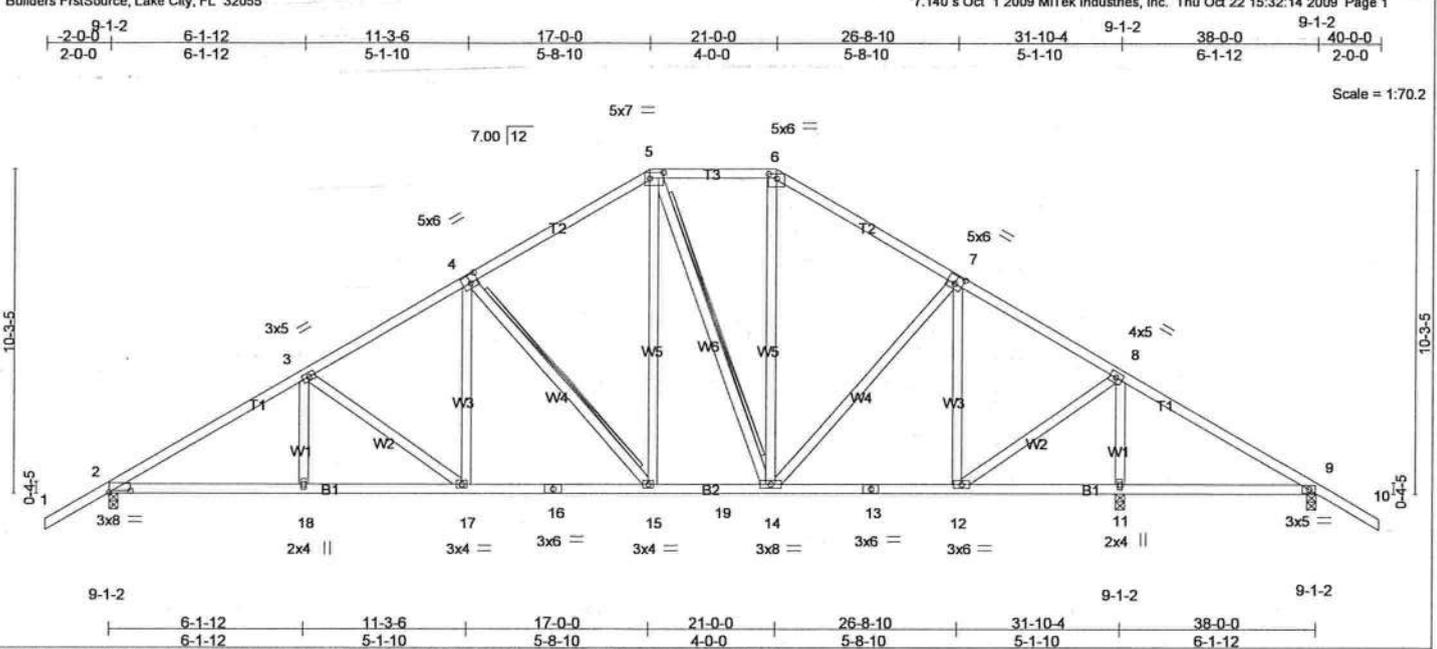


Plate Offsets (X,Y): [2:0-8-1,0-0-10], [4:0-3-0,0-3-0], [5:0-5-4,0-2-4], [6:0-3-0,0-1-12], [7:0-3-0,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.31	Vert(LL) -0.10 15-17 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.47	Vert(TL) -0.23 15-17 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.48	Horz(TL) 0.06 11 n/a n/a		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.07 9-11 >997 240		Weight: 244 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

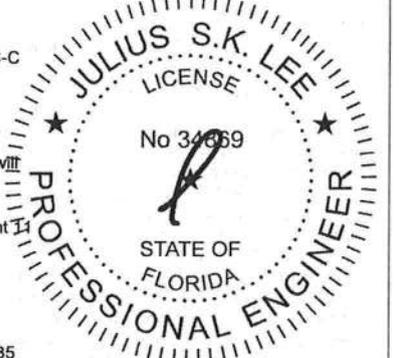
BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-11-6 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS T-Brace: 2 X 4 SYP No.3 - 4-15, 5-14
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1480/0-3-8, 11=1958/0-3-8, 9=141/0-3-8
 Max Horz 2=275(LC 5)
 Max Uplift 2=307(LC 6), 11=306(LC 7), 9=225(LC 7)
 Max Grav 2=1480(LC 1), 11=1958(LC 1), 9=209(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=2253/710, 3-4=1856/669, 4-5=1355/587, 5-6=973/529, 6-7=-1216/546, 7-8=1009/405, 8-9=-86/500
 BOT CHORD 2-18=-438/1849, 17-18=-438/1849, 16-17=-272/1535, 15-16=-272/1535, 15-19=-138/1098, 14-19=-138/1098, 13-14=-43/806, 12-13=-43/806, 11-12=-346/215, 9-11=-346/215
 WEBS 3-17=-384/202, 4-17=-86/427, 4-15=-677/332, 5-15=-217/686, 5-14=-433/156, 6-14=-102/312, 7-14=-127/323, 7-12=-620/244, 8-12=-314/1406, 8-11=-1833/615

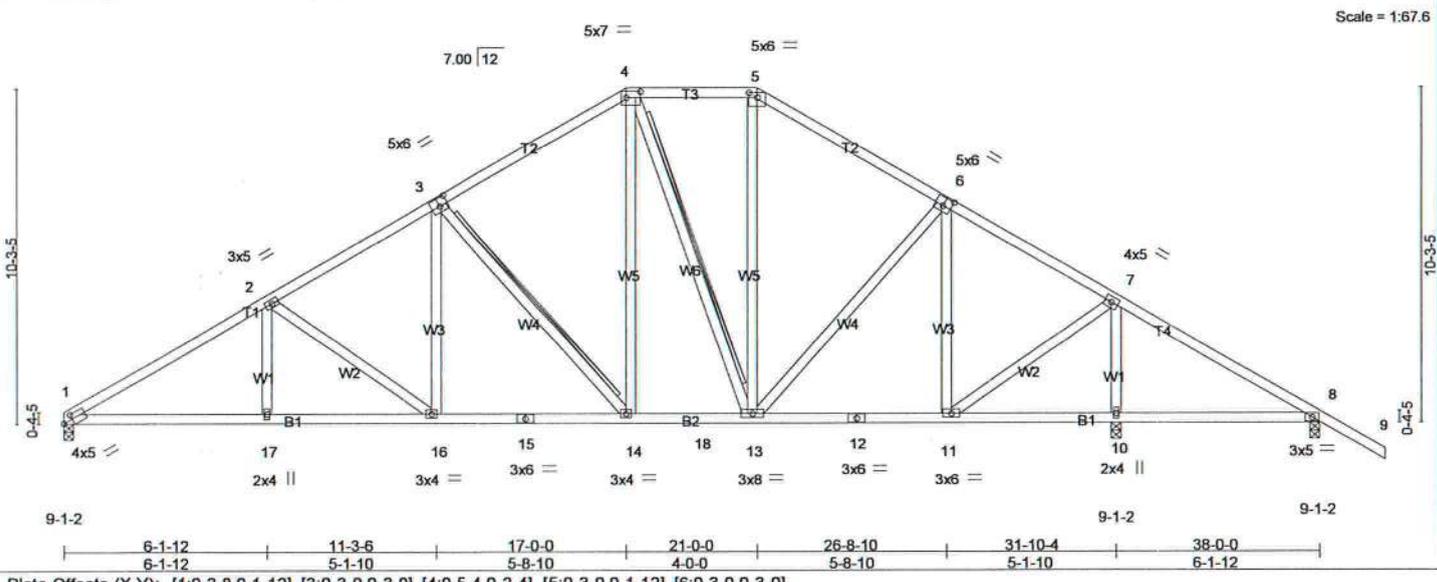
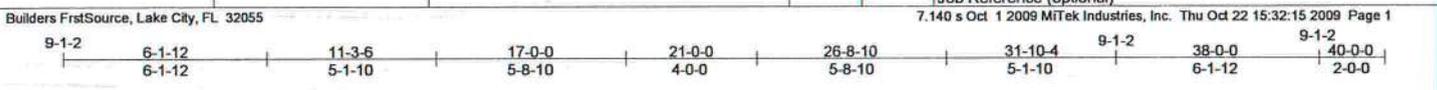
- NOTES** (10-11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SYP No.2
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 307 lb uplift at joint 2, 306 lb uplift at joint 11 and 225 lb uplift at joint 9.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



October 22, 2009

LOAD CASE(S) Standard

Job 317841	Truss T16	Truss Type HIP	Qty 2	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	I4137468
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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.36	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.49	Vert(LL) -0.11 14-16 >999 360		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.49	Vert(TL) -0.23 14-16 >999 240		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) 0.06 10 n/a n/a		
			Wind(LL) 0.07 8-10 >998 240		Weight: 241 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

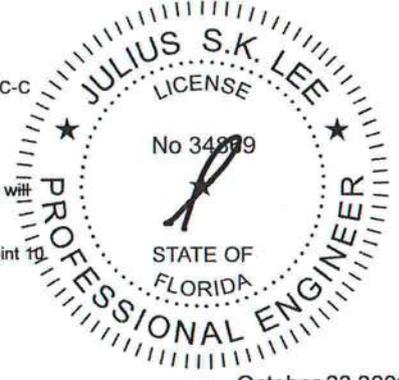
BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-9 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-5.
Rigid ceiling directly applied or 6-0-0 oc bracing.
BOT CHORD
WEBS T-Brace: 2 X 4 SYP No.3 - 3-14, 4-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1346/0-3-8, 10=1967/0-3-8, 8=138/0-3-8
Max Horz 1=-298(LC 4)
Max Uplift 1=-212(LC 6), 10=-308(LC 6), 8=-224(LC 7)
Max Grav 1=1346(LC 1), 10=1967(LC 1), 8=208(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-2270/756, 2-3=-1873/690, 3-4=-1361/594, 4-5=-977/534, 5-6=-1220/551,
6-7=-1010/406, 7-8=-93/506
BOT CHORD 1-17=-489/1889, 16-17=-489/1889, 15-16=-286/1546, 14-15=-286/1546, 14-18=-139/1104,
13-18=-139/1104, 12-13=-44/807, 11-12=-44/807, 10-11=-351/222, 8-10=-351/222
WEBS 2-16=-420/248, 3-16=-108/444, 3-14=-686/342, 4-14=-226/693, 4-13=-439/158,
5-13=-105/315, 6-13=-128/327, 6-11=-624/250, 7-11=-324/1414, 7-10=-1842/625

- NOTES (11-12)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide with fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 212 lb uplift at joint 1, 308 lb uplift at joint 10 and 224 lb uplift at joint 8.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



October 22, 2009

Continued on page 2
Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34889; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - CAMPBELL RES.	I4137468
317841	T16	HIP	2	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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LOAD CASE(S) Standard



October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T17	Truss Type SPECIAL	Qty 2	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	14137469
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Builders FrstSource, Lake City, FL 32055

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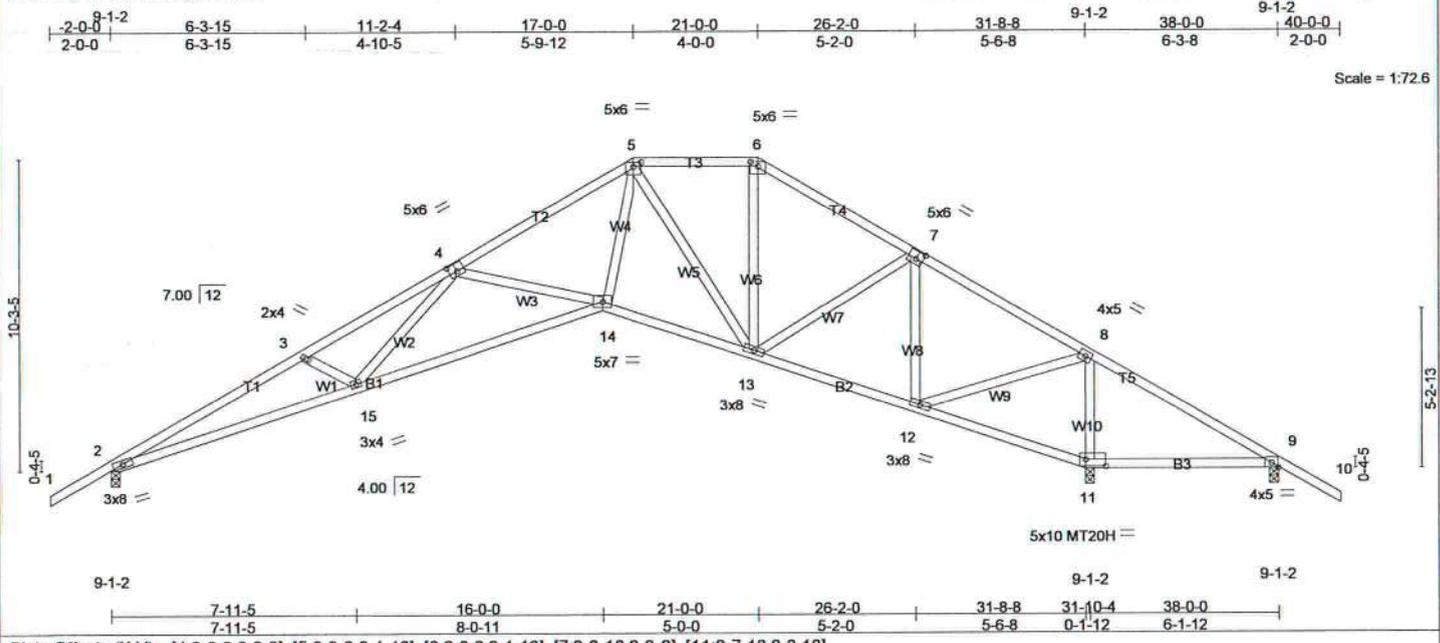


Plate Offsets (X,Y): [4:0-3-0,0-3-0], [5:0-3-0,0-1-12], [6:0-3-0,0-1-12], [7:0-2-12,0-3-0], [11:0-7-12,0-2-12]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.65	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.71	Vert(LL) -0.22 14-15 >999 360	MT20H	187/143
BCLL 0.0 *	Lumber Increase 1.25	WB 0.97	Vert(TL) -0.67 14-15 >566 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.38 11 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.21 14-15 >999 240		Weight: 204 lb

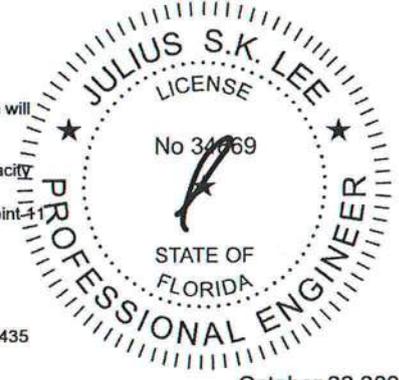
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-0-13 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 4-4-7 oc bracing.
WEBS 2 X 4 SYP No.3	

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1168/0-3-8, 11=2880/0-3-8, 9=773/0-3-8
 Max Horz 2=-276(LC 4)
 Max Uplift 2=-280(LC 6), 11=-465(LC 6), 9=873(LC 10)
 Max Grav 2=1168(LC 1), 11=2880(LC 1), 9=94(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-3409/1107, 3-4=-3115/1026, 4-5=-1801/521, 5-6=-680/383, 6-7=-872/385, 7-8=-294/261, 8-9=-537/2271
 BOT CHORD 2-15=-821/3006, 14-15=-482/2342, 13-14=-192/1283, 11-12=-2067/670, 9-11=-1858/599
 WEBS 4-15=-220/735, 4-14=-763/464, 5-14=-237/1466, 5-13=-1011/279, 7-13=-209/810, 7-12=-1144/342, 8-12=-476/2040, 8-11=-2107/703

- NOTES (12-13)**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) All plates are MT20 plates unless otherwise indicated.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 7) All bearings are assumed to be SYP No.2 .
 - 8) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2, 465 lb uplift at joint 11 and 873 lb uplift at joint 9.
 - 10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 11) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - 12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



LOAD CASE(S) Standard October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 317841	Truss T18	Truss Type SPECIAL	Qty 7	Ply 1	GIEBEIG HOMES - CAMPBELL RES.	14137470
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Builders FrstSource, Lake City, FL 32055 7,140 s Oct 1 2009 Mitek Industries, Inc. Thu Oct 22 15:32:16 2009 Page 1

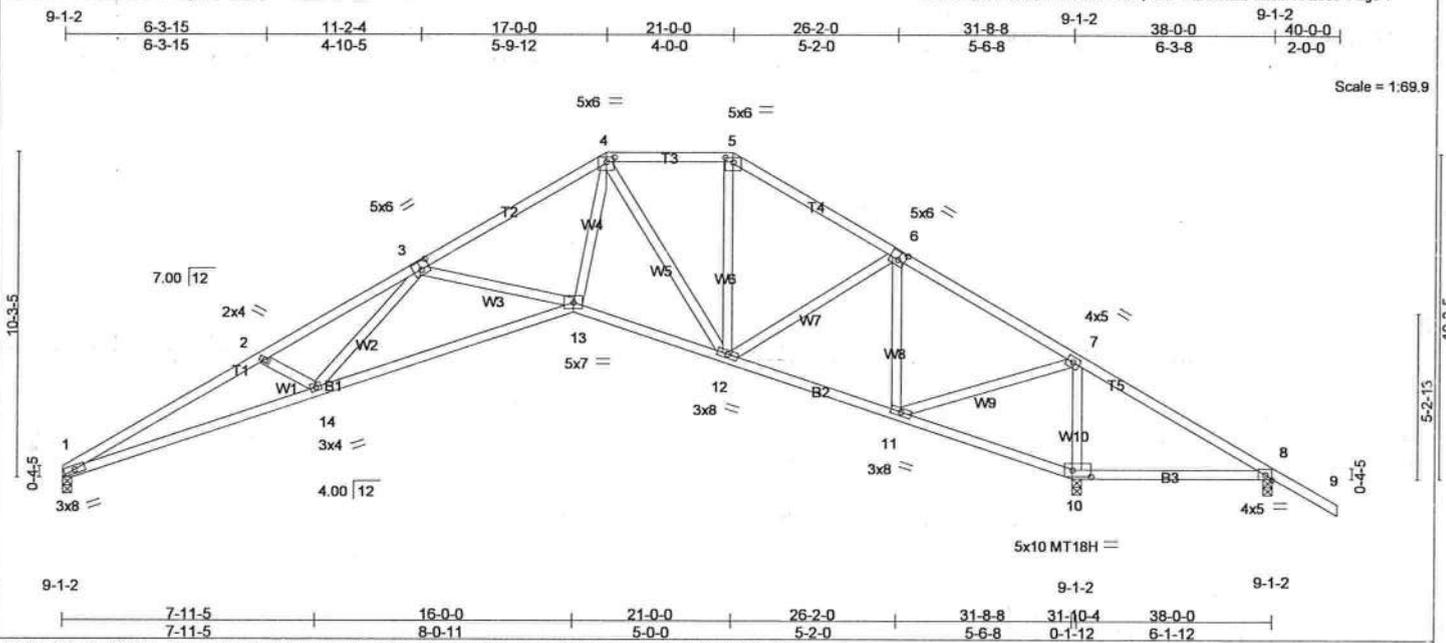


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [4:0-3-0,0-1-12], [5:0-3-0,0-1-12], [6:0-2-12,0-3-0], [10:0-7-0,0-2-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.66	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.74	Vert(LL) -0.22 13-14 >999 360	MT18H	244/190
BCLL 0.0	Lumber Increase 1.25	WB 0.98	Vert(TL) -0.67 13-14 >566 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.38 10 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.21 13-14 >999 240		Weight: 201 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-6 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 4-4-2 oc bracing.

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

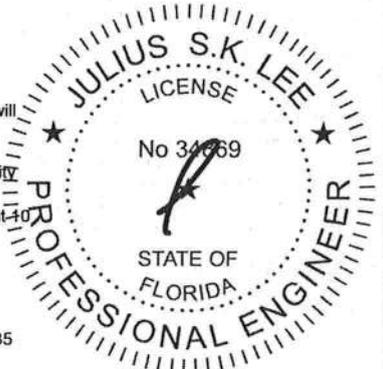
REACTIONS (lb/size) 1=1031/0-3-8, 10=2903/0-3-8, 8=789/0-3-8
Max Horz 1=-298(LC 4)
Max Uplift 1=-183(LC 6), 10=-487(LC 6), 8=889(LC 10)
Max Grav 1=1031(LC 1), 10=2903(LC 1), 8=99(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-3462/1198, 2-3=-3175/1101, 3-4=-1805/526, 4-5=-677/380, 5-6=-869/381, 6-7=-290/257, 7-8=-575/2302
BOT CHORD 1-14=-917/3083, 13-14=-500/2361, 12-13=-192/1285, 10-11=-2095/706, 8-10=-1884/631
WEBS 2-14=-261/257, 3-14=-282/751, 3-13=-778/482, 4-13=-239/1475, 4-12=-1021/281, 6-12=-212/820, 6-11=-1154/354, 7-11=-496/2056, 7-10=-2121/721

- NOTES (12-13)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2
 - Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 183 lb uplift at joint 1, 487 lb uplift at joint 10 and 889 lb uplift at joint 8.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

October 22, 2009

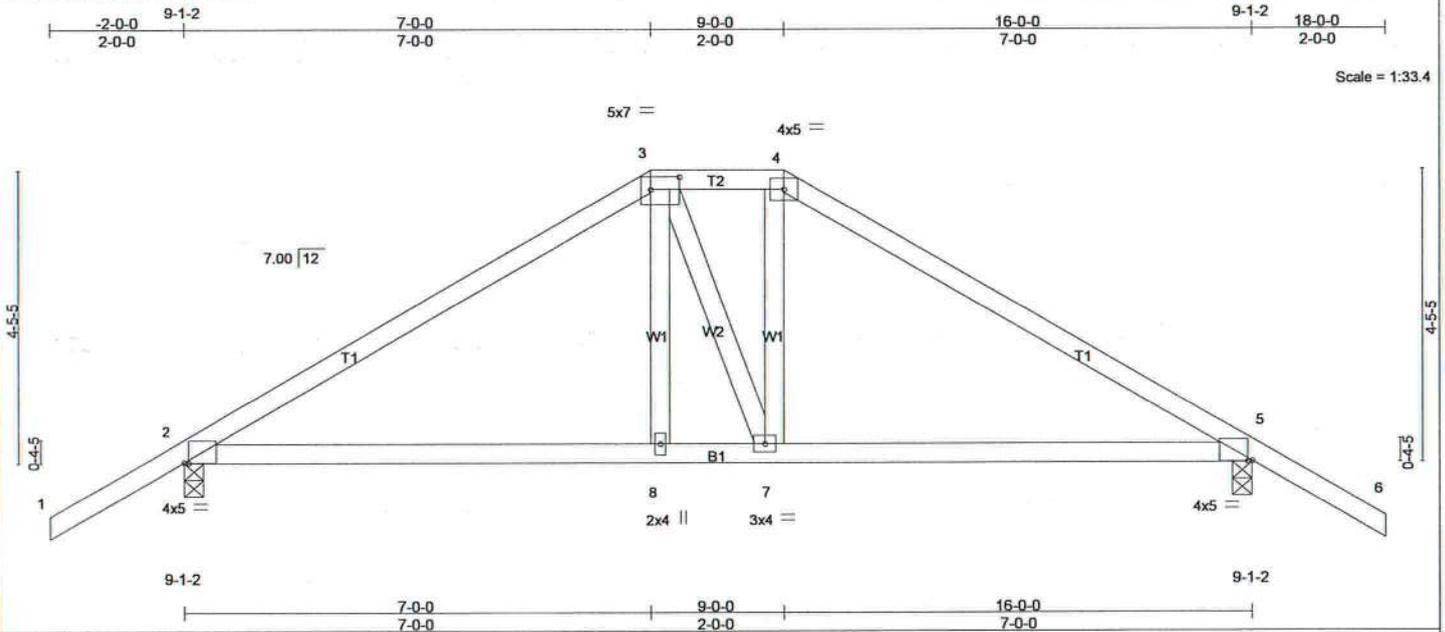


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Builders FrstSource, Lake City, FL 32055

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Scale = 1:33.4

Plate Offsets (X,Y): [2:0-0-13,0-0-2], [3:0-5-4,0-2-4], [5:0-0-13,0-0-2]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.48	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.56	Vert(LL) -0.07 2-8 >999 360		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.17	Vert(TL) -0.21 2-8 >889 240		
BCDL 10.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) 0.04 5 n/a n/a		
			Wind(LL) 0.15 2-8 >999 240		Weight: 76 lb

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 4-0-11 oc purlins. BOT CHORD Rigid ceiling directly applied or 5-7-15 oc bracing. <div style="border: 1px solid black; padding: 5px; margin-top: 5px;"> MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </div>
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REACTIONS (lb/size) 2=1291/0-3-8, 5=1291/0-3-8
 Max Horz 2=-113(LC 3)
 Max Uplift 2=-937(LC 5), 5=-930(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1841/1370, 3-4=-1497/1227, 4-5=-1843/1380
 BOT CHORD 2-8=-1142/1481, 7-8=-1157/1495, 5-7=-1113/1483
 WEBS 3-8=-426/536, 4-7=-479/541

- NOTES** (11-12)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) All bearings are assumed to be SYP No.2.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 937 lb uplift at joint 2 and 930 lb uplift at joint 5.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 250 lb down and 237 lb up at 7-0-0 and 290 lb down and 241 lb up at 9-0-0 on top chord, and 351 lb down and 303 lb up at 7-0-0, and 351 lb down and 303 lb up at 8-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=60, 3-4=60, 4-6=60, 2-5=20



October 22, 2009

Continued on page 2

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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 317841	Truss T19	Truss Type HIP	Qty 1	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	14137471
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Builders FrstSource, Lake City, FL 32055

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LOAD CASE(S) Standard
 Concentrated Loads (lb)
 Vert: 3=-250(F) 4=-250(F) 8=-284(F) 7=-284(F)



October 22, 2009

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and RCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>Julius Lee Engineering 1109 Coastal Bay Blvd. Boynton, FL 33435</p>
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Job 317841	Truss T20	Truss Type COMMON	Qty 3	Ply 1	GIEBEIG HOMES - CAMPBELL RES. Job Reference (optional)	I4137472
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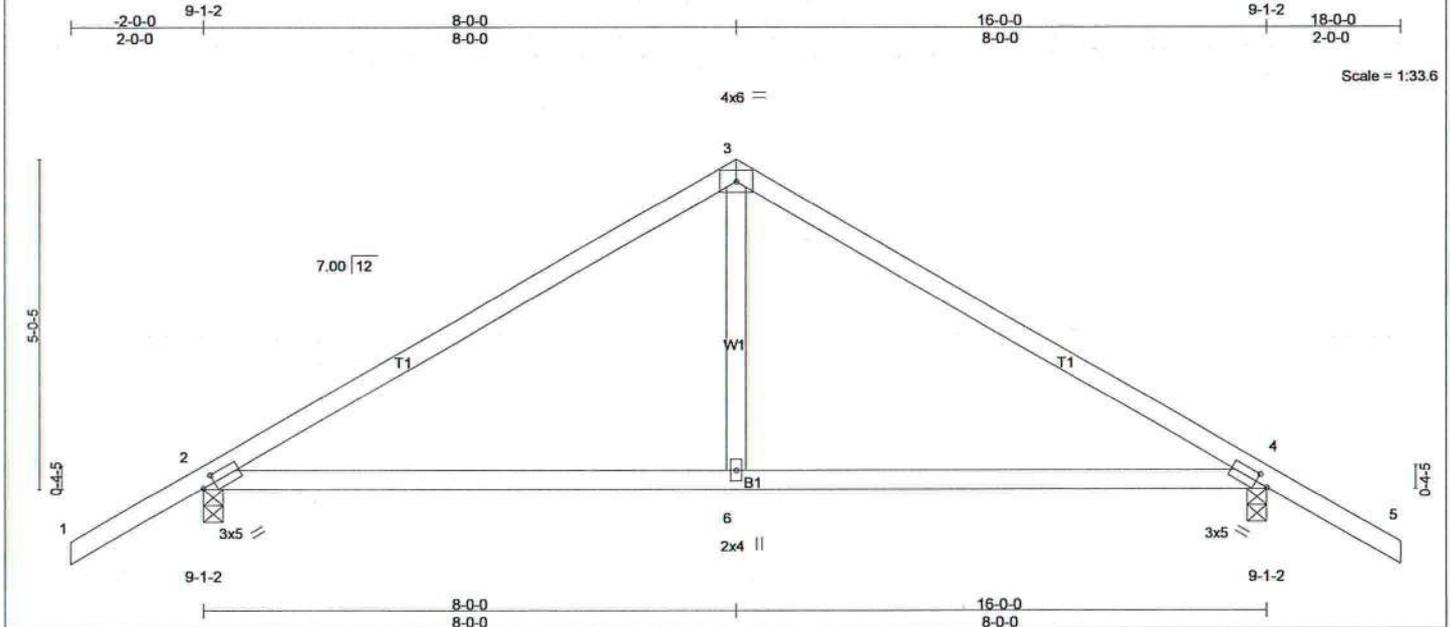


Plate Offsets (X,Y): [2:0-2-4,0-1-8], [4:0-2-4,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.41	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.25	BC 0.47	Vert(LL) -0.08 4-6 >999 360		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.20	Vert(TL) -0.22 4-6 >876 240		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.02 4 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.26 4-6 >721 240		Weight: 65 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 8-3-0 oc bracing.
WEBS 2 X 4 SYP No.3	

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=757/0-3-8, 4=757/0-3-8
 Max Horz 2=129(LC 5)
 Max Uplift 2=402(LC 6), 4=402(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=793/792, 3-4=793/792
 BOT CHORD 2-6=487/576, 4-6=487/576
 WEBS 3-6=513/380

- NOTES (8-9)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 402 lb uplift at joint 2 and 402 lb uplift at joint 4.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



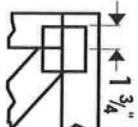
October 22, 2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

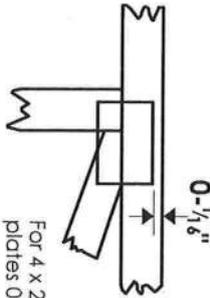
Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 X 2 orientation, locate plates 0- $\frac{1}{8}$ " from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in **MiTek 20/20 software** or upon request.

PLATE SIZE

4 X 4

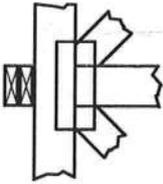
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

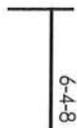


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

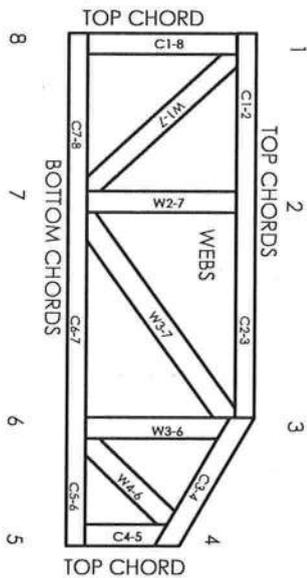
Industry Standards:

ANSI/FP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9804B,
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by ANSI/FP 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/FP 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or pultrus provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (from, back, words and pictures) before use. Reviewing pictures done is not sufficient.
20. Design assumes manufacture in accordance with ANSI/FP 1 Quality Criteria.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

STEPPED DOWN CORNER SET

TOP CHORD 2X4 SO. PINE #2 or Better
 BOT CHORD 2X4 SO. PINE #2 or Better
 WEBS 2X4 SO. PINE #3 or Better

120 MPH MAX

Setback 7' or Less

2' TYP. MAX

#2 HIP OR COMMON TRUSS
 #1 HIP TRUSS

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

UPLIFT: 400# or Less

BRG LOC: *

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

CJ's
 2' TYP. MAX

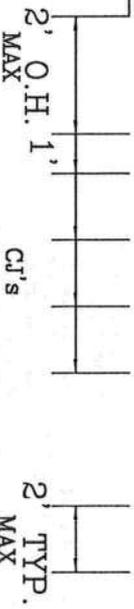
1'

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE

UPLIFT: 400# or Less

BRG LOC: *

UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)



PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

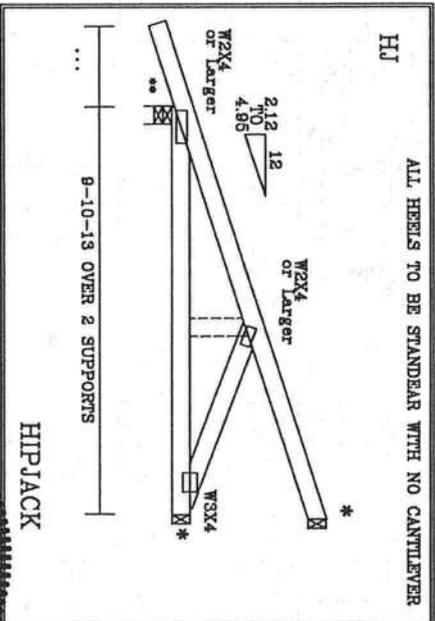
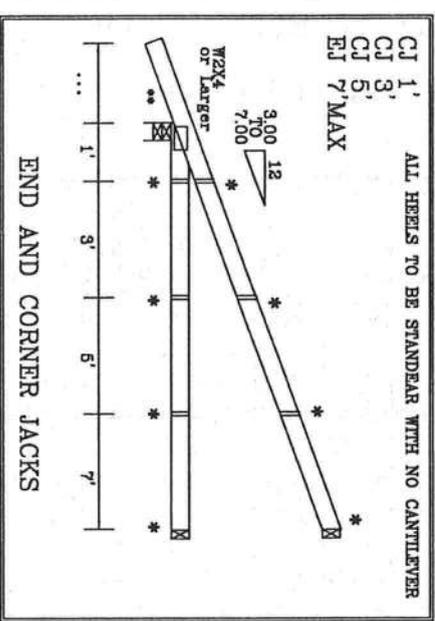
UPLIFT: 400# or Less

BRG LOC: *

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

* (3) 16d TOENAILS

** SEE EOR FOR THE DOWN

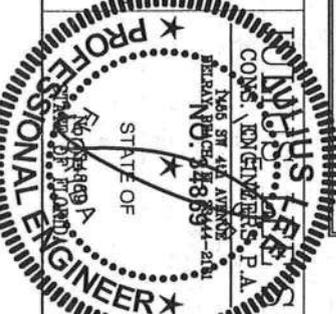


UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
 BC LIVE LOAD IS NON CONCURRENT 10*

CORNER SET
 SETBACK
 7'0" MAX

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS LATE INSTITUTE, 583 BIRCHMOUNT DR., SUITE 200, PALM BEACH, FL 33410 AND CHECK WITH TRUSS DESIGNER FOR THE TRUSS DESIGNER'S DESIGN. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN, INCLUDING HANDLING, SHIPPING, INSTALLING & BRACING, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. ALL TRUSS COMPONENTS SHALL BE SPECIFIED BY ALPINE AND THE ALPINE CONNECTOR PLATES ARE MADE OF 6061-T6 ALUMINUM ALLOY. ALL TRUSS COMPONENTS SHALL BE 40/60 (W/K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY CD SHALL BE PER ANNEK A3 OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF THE PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

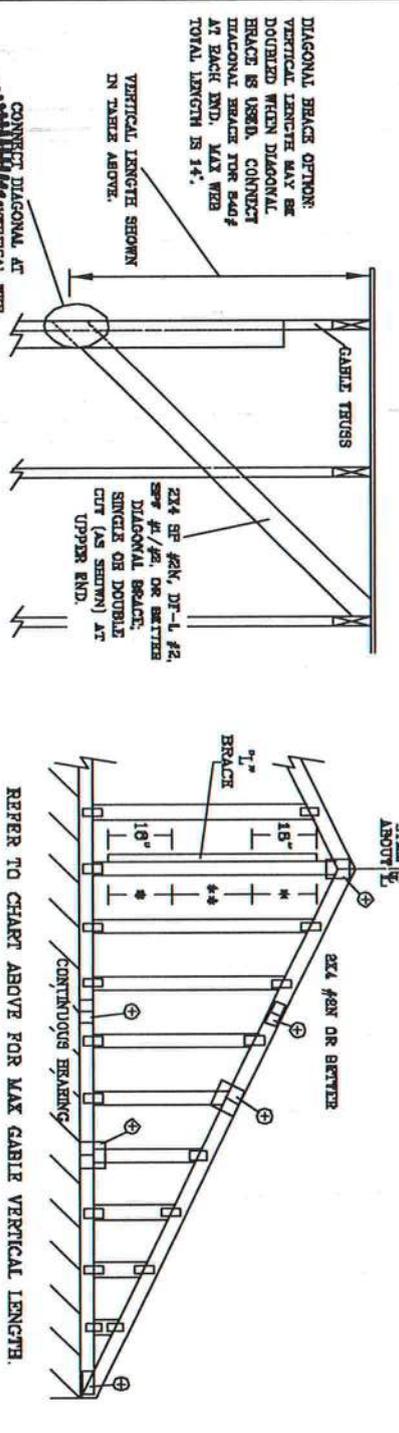


MEMBER	TYPE	PSF	CS
HC	TL	20	MAX PSF
HC	DL	20	MAX PSF
BC	TL	10*	MAX PSF
BC	DL	5	MAX PSF
EC	DL	5	MAX PSF
DRWG	DATE	7' MAX STBK	CS
REF	DATE	1.25	
ENG	DATE	2'	

REVIEWED
 By Julius Lee at 10:52 am, Jun 27, 2008

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		BRACE		NO BRACES		GROUP A		GROUP B		GROUP A		GROUP B		GROUP A		GROUP B	
GABLE VERTICAL SPACING	2X4 BRACE SPECIES	GRADE	BRACE	NO BRACES	(1) 1X4 T ¹ BRACE *		(1) 2X4 T ¹ BRACE *		(2) 2X4 T ¹ BRACE **		(1) 2X6 T ¹ BRACE *		(2) 2X6 T ¹ BRACE *		2X6 T ¹ BRACE **		
					GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B			
12" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"				
		#3	3' 3"	4' 11"	4' 11"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"					
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"					
	HF	STANDARD	3' 3"	4' 2"	4' 2"	5' 8"	7' 5"	7' 5"	8' 8"	8' 8"	11' 6"	11' 6"	13' 11"				
		#1	3' 8"	5' 10"	6' 3"	8' 11"	8' 5"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"					
		#2	3' 7"	6' 10"	6' 3"	7' 6"	8' 3"	8' 3"	10' 10"	11' 8"	12' 11"	13' 11"					
	SP	#3	3' 6"	5' 0"	6' 0"	6' 8"	8' 8"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"					
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	8' 7"	8' 7"	10' 3"	10' 3"	12' 11"	13' 7"					
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"					
	DFL	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 8"	12' 6"	12' 9"	14' 0"	14' 0"				
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"				
		STANDARD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"				
16" O.C.	SPF	#1 / #2	4' 3"	7' 4"	7' 4"	8' 9"	10' 6"	10' 6"	13' 8"	13' 8"	14' 0"	14' 0"					
		#3	4' 2"	6' 11"	6' 11"	8' 8"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"					
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"					
	HF	STANDARD	4' 2"	6' 11"	6' 11"	7' 10"	9' 10"	9' 10"	10' 5"	10' 5"	13' 8"	14' 0"					
		#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"					
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"					
	SP	#3	4' 4"	7' 2"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"					
		STUD	4' 4"	7' 2"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"					
		STANDARD	4' 3"	7' 1"	7' 1"	8' 9"	9' 2"	10' 5"	10' 5"	13' 8"	14' 0"	14' 0"					
	DFL	#1 / #2	4' 10"	7' 5"	7' 11"	8' 11"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"					
		#3	4' 8"	6' 11"	6' 11"	8' 11"	8' 11"	10' 6"	10' 6"	14' 0"	14' 0"	14' 0"					
		STANDARD	4' 8"	6' 11"	6' 11"	8' 11"	8' 11"	10' 6"	10' 6"	14' 0"	14' 0"	14' 0"					
24" O.C.	SPF	#1 / #2	5' 8"	9' 0"	9' 0"	10' 6"	13' 8"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"					
		#3	5' 7"	8' 0"	8' 0"	9' 6"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"					
		STANDARD	5' 7"	8' 0"	8' 0"	9' 6"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"					
	HF	STANDARD	5' 8"	9' 0"	9' 0"	10' 6"	13' 8"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"					
		#1	6' 4"	8' 8"	8' 8"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"					
		#2	6' 4"	8' 8"	8' 8"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"					
	SP	#3	6' 0"	8' 2"	8' 2"	9' 8"	12' 6"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"					
		STUD	6' 0"	8' 2"	8' 2"	9' 8"	12' 6"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"					
		STANDARD	6' 0"	8' 2"	8' 2"	9' 8"	12' 6"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"					
	DFL	#1 / #2	6' 10"	9' 5"	9' 11"	10' 11"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"					
		#3	6' 8"	8' 0"	8' 0"	9' 6"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"					
		STANDARD	6' 8"	8' 0"	8' 0"	9' 6"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"					



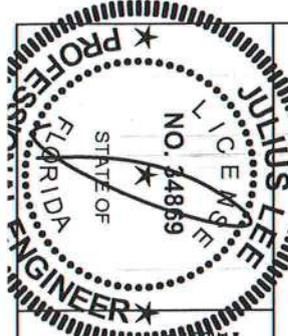
BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPRUCE-PINE-TYR	NOE-PTR	DOUGLAS FIR-LARCH	DOUGLAS FIR-LARCH
#1 / #2	#2	#1	#2
STUD	STUD	STUD	STUD
STANDARD	STANDARD	STANDARD	STANDARD

CABLE TRUSS DETAIL NOTES:
 LIVE LOAD DEFLECTION CRITERIA IS L/240.
 PROVIDE UPLIFT CONNECTIONS FOR 136 PSF OVER CONTINUOUS BRACING (6 PSF VC DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4" O" OUTLOOKERS WITH 3" O" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH T¹ BRACE WITH 10d NAILS.
 * FOR (1) T¹ BRACE: SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
 ** FOR (2) T¹ BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
 T¹ BRACING MUST BE A MINIMUM OF 80% OR MORE MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SLICES
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0" BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2X6

+ REFER TO COMMON TRUSS DESIGN FOR PEAK SPLICE AND BEEL PLATES.

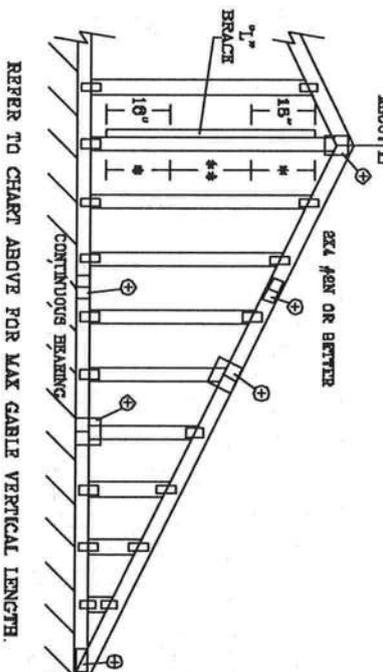
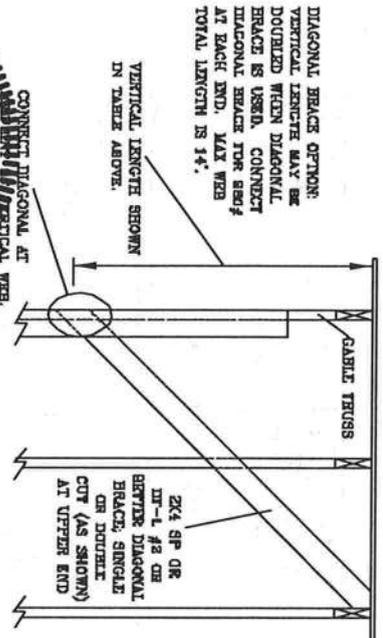


WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO ASCE 7-02 (LOADS) FOR DESIGN CONSIDERATIONS. VERIFY ALL TRUSS TRAYS CONNECTED TO ANCHORS. VERIFY ALL TRAYS ARE PROPERLY ATTACHED TO TRUSS TRAYS. VERIFY ALL TRAYS ARE PROPERLY ATTACHED TO TRUSS TRAYS. VERIFY ALL TRAYS ARE PROPERLY ATTACHED TO TRUSS TRAYS. VERIFY ALL TRAYS ARE PROPERLY ATTACHED TO TRUSS TRAYS.

REVIEWED
 By Julius Lee at 12:00 pm, Jun 11, 2008

Julius Lee's
 CONS. ENGINEERS P.A.
 1455 ST. 4th AVENUE
 MELBOURNE, FL 32941-8161
 No. 34869
 STATE OF FLORIDA
 REF: ASCE 7-02-GABR30N5
 DATE: 11/26/03
 DRWG: WITH STD. CABLE IS E ET
 -ENG

MAX GABLE VERTICAL LENGTH		BRACE		GROUP A		GROUP B		GROUP A		GROUP B		GROUP A		GROUP B	
GABLE SPACING	SPECIES	GRADE	NO BRACES	(1) 1x4 "L" BRACE		(1) 2x4 "L" BRACE		(2) 2x4 "L" BRACE		(1) 2x6 "L" BRACE		(2) 2x8 "L" BRACE		(2) 2x12 "L" BRACE	
				GROUP A	GROUP B										
12" O.C.	SPF	#1 / #2	3' 2"	5' 8"	6' 8"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"	12' 3"	12' 3"
		#3	3' 1"	4' 5"	4' 5"	6' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"
	HF	STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"	10' 7"	10' 7"
		#1	3' 8"	5' 8"	5' 8"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"	13' 2"
		#2	3' 6"	5' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"	13' 2"
	DFL	STUD	3' 3"	4' 6"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	12' 3"	12' 8"	12' 8"	12' 8"
		STANDARD	3' 0"	3' 10"	3' 10"	3' 10"	6' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"	10' 10"
		#1 / #2	3' 8"	5' 5"	5' 5"	6' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"
	SPF	#3	3' 7"	5' 5"	5' 5"	6' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	6' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"
		STANDARD	3' 7"	5' 5"	5' 5"	6' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"
SP	#1	4' 0"	6' 4"	6' 4"	6' 4"	8' 2"	8' 2"	9' 11"	9' 11"	12' 8"	12' 8"	14' 0"	14' 0"	14' 0"	
	#2	3' 11"	5' 4"	5' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"	14' 0"	
	STUD	3' 11"	5' 4"	5' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"	14' 0"	
DFL	#3	3' 8"	5' 6"	5' 6"	6' 7"	7' 4"	7' 4"	8' 11"	8' 11"	11' 5"	11' 6"	14' 0"	14' 0"	14' 0"	
	STUD	3' 8"	5' 6"	5' 6"	6' 7"	7' 4"	7' 4"	8' 11"	8' 11"	11' 5"	11' 6"	14' 0"	14' 0"	14' 0"	
	STANDARD	3' 8"	5' 6"	5' 6"	6' 7"	7' 4"	7' 4"	8' 11"	8' 11"	11' 5"	11' 6"	14' 0"	14' 0"	14' 0"	
SPF	#1 / #2	4' 0"	6' 11"	6' 11"	7' 2"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	13' 4"	14' 0"	14' 0"	14' 0"	
	#3	3' 11"	5' 3"	5' 3"	6' 3"	6' 3"	7' 1"	7' 10"	8' 6"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"	
	STUD	3' 11"	5' 3"	5' 3"	6' 3"	6' 3"	7' 1"	7' 10"	8' 6"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"	
HF	STANDARD	3' 11"	5' 4"	5' 4"	5' 4"	7' 1"	7' 1"	9' 10"	9' 10"	12' 11"	12' 10"	14' 0"	14' 0"	14' 0"	
	#1	4' 5"	6' 11"	6' 11"	7' 6"	8' 11"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"	14' 0"	
	#2	4' 4"	6' 11"	6' 11"	7' 6"	8' 11"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"	14' 0"	
SP	#3	4' 2"	6' 6"	6' 6"	6' 5"	8' 3"	8' 3"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"	14' 0"	
	STUD	4' 2"	6' 6"	6' 6"	6' 5"	8' 3"	8' 3"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"	14' 0"	
	STANDARD	4' 2"	6' 6"	6' 6"	6' 5"	8' 3"	8' 3"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"	14' 0"	
DFL	#1 / #2	4' 0"	5' 6"	5' 6"	5' 6"	7' 3"	7' 3"	8' 5"	8' 5"	10' 4"	10' 4"	14' 0"	14' 0"	14' 0"	
	#3	3' 8"	5' 6"	5' 6"	6' 5"	7' 3"	7' 3"	8' 5"	8' 5"	10' 4"	10' 4"	14' 0"	14' 0"	14' 0"	
	STUD	3' 8"	5' 6"	5' 6"	6' 5"	7' 3"	7' 3"	8' 5"	8' 5"	10' 4"	10' 4"	14' 0"	14' 0"	14' 0"	
STANDARD	#1 / #2	4' 0"	5' 6"	5' 6"	5' 6"	7' 3"	7' 3"	8' 5"	8' 5"	10' 4"	10' 4"	14' 0"	14' 0"	14' 0"	
	#3	3' 8"	5' 6"	5' 6"	6' 5"	7' 3"	7' 3"	8' 5"	8' 5"	10' 4"	10' 4"	14' 0"	14' 0"	14' 0"	
	STUD	3' 8"	5' 6"	5' 6"	6' 5"	7' 3"	7' 3"	8' 5"	8' 5"	10' 4"	10' 4"	14' 0"	14' 0"	14' 0"	

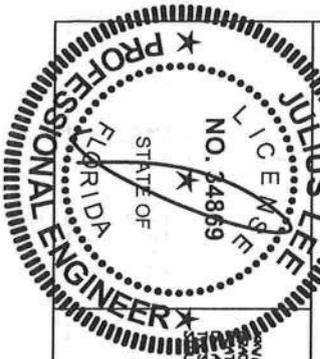


BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPRUCE-PINE-LARCHE	DOUGLAS FIR-LARCHE	SPRUCE-PINE-LARCHE	DOUGLAS FIR-LARCHE
#1 / #2	#1	#1 / #2	#1
STUD	STUD	STUD	STUD
STANDARD	STANDARD	STANDARD	STANDARD

CABLE TRUSS DETAIL NOTES:
 LIVE LOAD DEFLECTION CRITERIA IS L/240.
 PROVIDE UPLIFT CONNECTIONS FOR 160 PSF OVER CONTINUOUS BEARING (6 PSF VC DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4' 0" OUTLEAKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
 ATTACH EACH "L" BRACE WITH 10# NAILS.
 * FOR (1) "L" BRACE, SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
 ** FOR (2) "L" BRACES, SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
 "L" BRACING MUST BE A MINIMUM OF 80% OR WEB MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES			
VERTICAL LENGTH	NO STICK	1x4 OR BTR	2x4
LESS THAN 4' 0"			
GREATER THAN 4' 0" BUT LESS THAN 11' 8"			
GREATER THAN 11' 8"			2.5x4

+ REFER TO COLUMN TRUSS DESIGN FOR PEAK, SPICE, AND BEEL PLATES



STRUCTURAL TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTING. REFER TO ECOT 1-43 QUALITY CONTROL SAFETY INFORMATION, PUBLISHED BY THE TRUSS INSTITUTE, 382 BRUNNEN DR., SUITE 200, MERRISON, VA 22079 AND VITA (WOOD TRUSS COLLECTIVE) FROM BROW ENTERPRISES, L.L.C., 1000 W. 10TH AVE., SUITE 100, MIAMI, FL 33135 FOR SHEET PRACTICES PRIOR TO PERFORMING STRUCTURAL PANELS AND BOLTS ORIGIN SHALL HAVE A PERMANENT ATTACHED ROAD TAGS

REVIEWED
 By Julius Lee at 12:00 pm, Jun 11, 2008

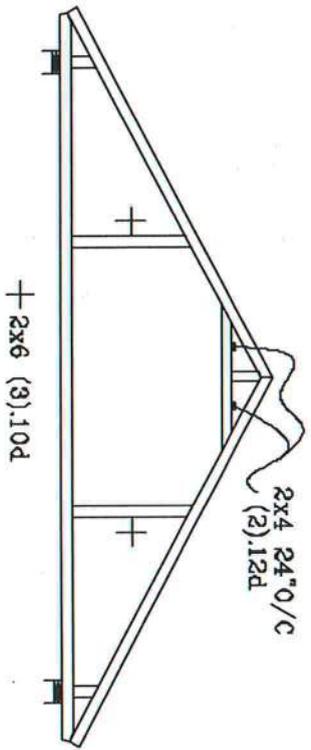
JULIUS LEE'S
 CONS. ENGINEERS P.A.
 1456 SW 4th AVENUE
 DELRAY BEACH, FL 33444-6161

No. 34698
 STATE OF FLORIDA

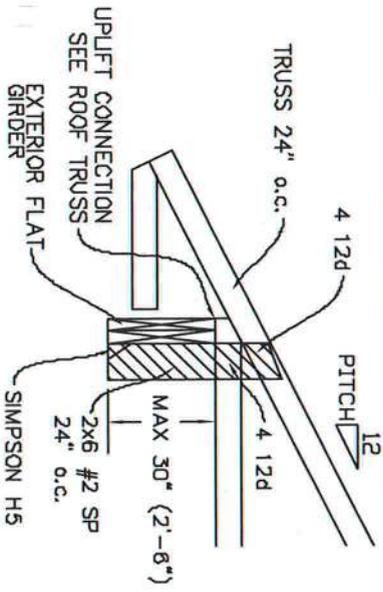
MAX. TOT. LD. 60 PSF
 MAX. SPACING 24.0"

REF ASCT-02-CAB13030
 DATE 11/26/03
 DWG MAKE STD GABLE 50' E WF
 -ENG

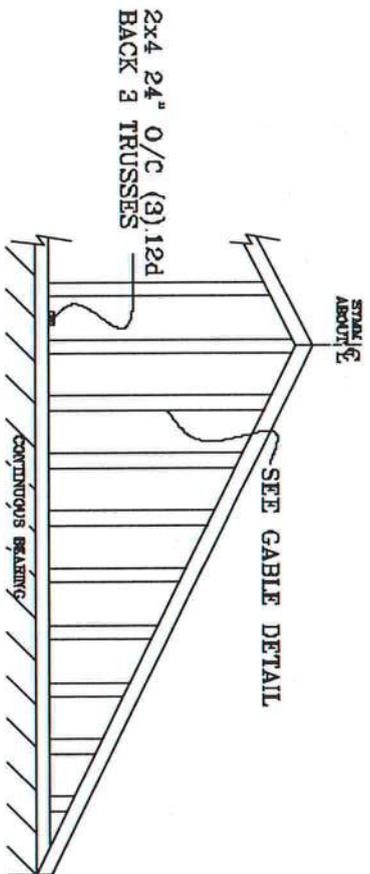
TYPICAL ATTIC TRUSS BRACING



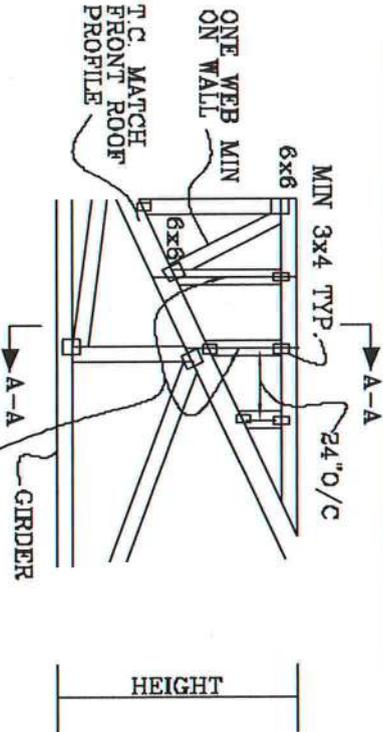
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



GABLE END TRUSS DETAIL

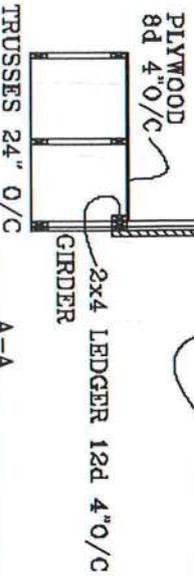


TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL

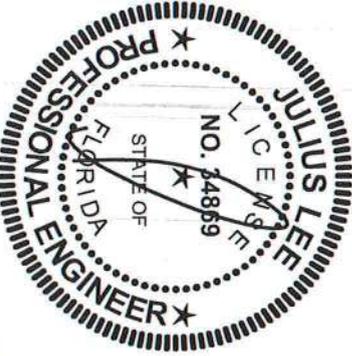


SEE ROOF TRUSSES FOR UPLIFT

SEE CABL. END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008



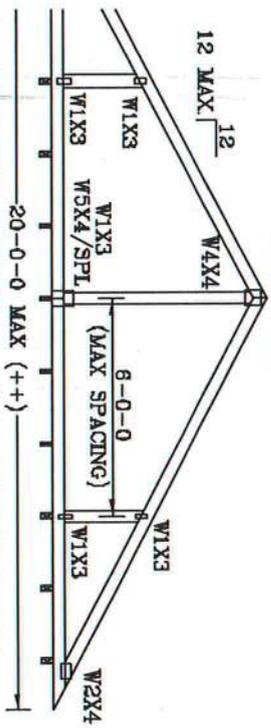
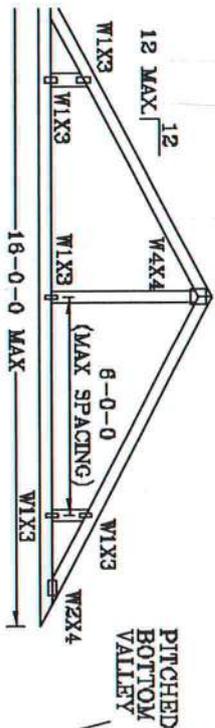
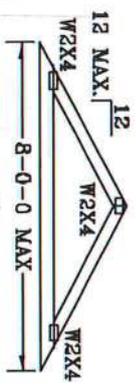
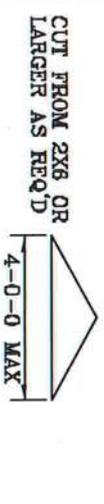
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1465 SW 4th AVENUE
DIKRAY BEACH, FL 33444-2161

No. 34869
STATE OF FLORIDA

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
 BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
 WEBS 2X4 SP #3 OR BETTER.

- * 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- ** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
 - (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND, 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

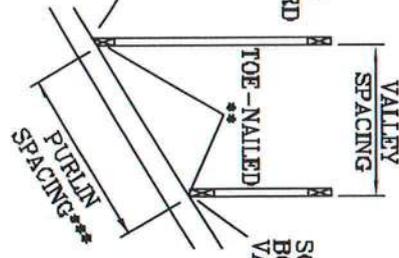


UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".
 MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
 OR PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
 OR BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
 ** LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".
 BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.

NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
 ** LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".
 BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.

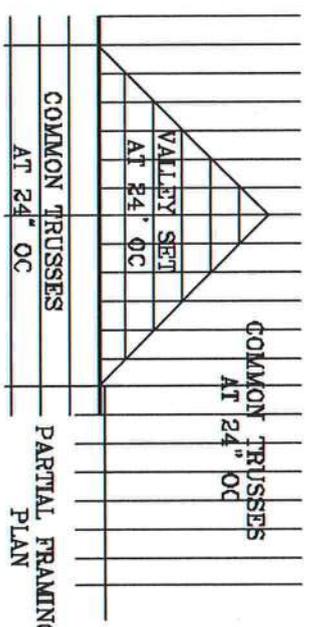


PITCHED CUT BOTTOM CHORD VALLEY

SQUARE CUT BOTTOM CHORD VALLEY

OPTIONAL STUB END DETAIL

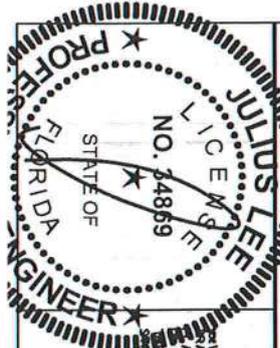
OPTIONAL HIP JOINT DETAIL



COMMON TRUSSES AT 24" OC

PARTIAL FRAMING PLAN

THIS DRAWING REPLACES DRAWING A105



JULIUS LEE
 LICENSE NO. 34869
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER

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 1455 SW 4th AVENUE
 DEALY BLDG. IV, SUITE 810A
 MIAMI BEACH, FL 3344-810A

TC LL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC LL	0	0	PSF	-ENG	JL
TOT. ID.	32	40	PSF		
DUR.FAC	1.25	1.25			
SPACING	24"				

REVIEWED
 By Julius Lee at 11:59 am, Jun 11, 2008

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

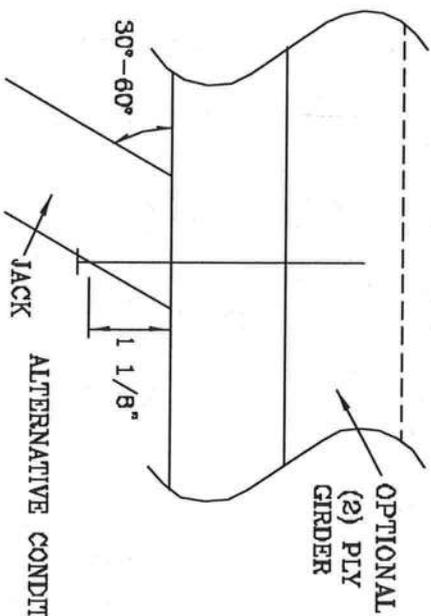
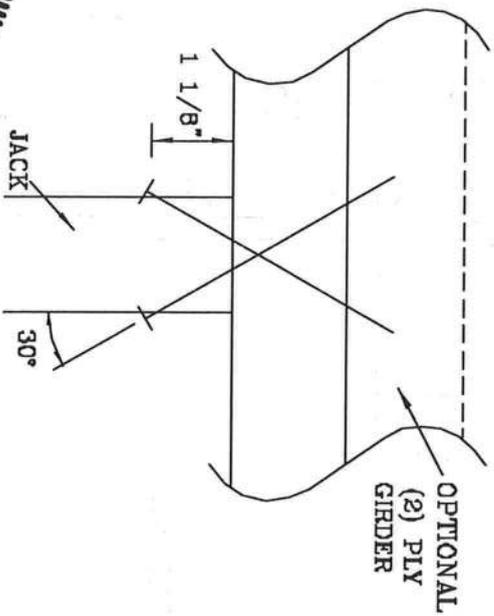
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

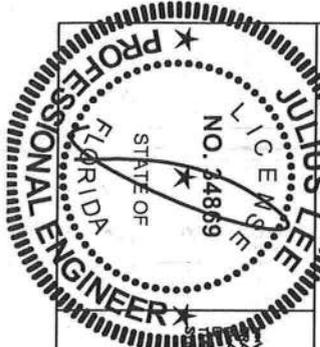
MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



THIS DRAWING REPLACES DRAWING 784040



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTING. REFER TO BEST PRACTICES FOR QUALITY COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION OF AMERICA, 1400 BY 4TH AVENUE, SUITE 200, NORTON, VA 23719 AND VITA (WOOD TRUSS DESIGN) TRUSS MANUFACTURERS ASSOCIATION, 1400 BY 4TH AVENUE, SUITE 200, NORTON, VA 23719. THESE FOUNDATIONS, STRUCTURAL PANELS AND BRITISH CORD SHALL HAVE A PERMANENTLY ATTACHED IDENTIFICATION LABEL.

REVIEWED
By Julius Lee at 11:39 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 BY 4TH AVENUE
SUITE 200, NORTON, VA 23719

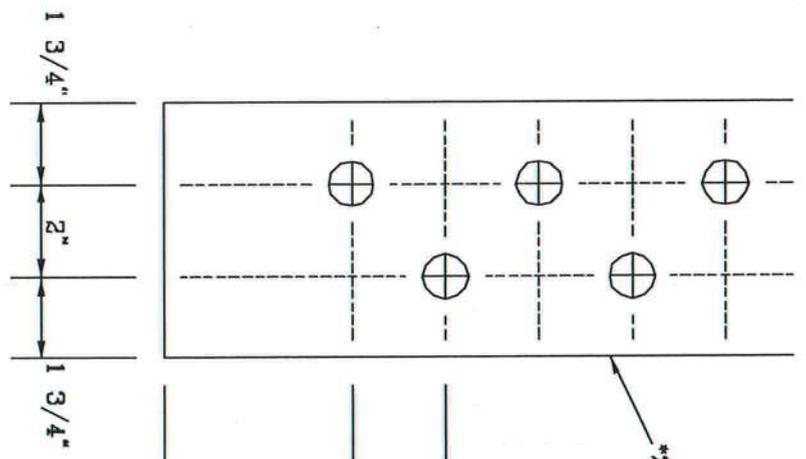
TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

No. 34889
STATE OF FLORIDA

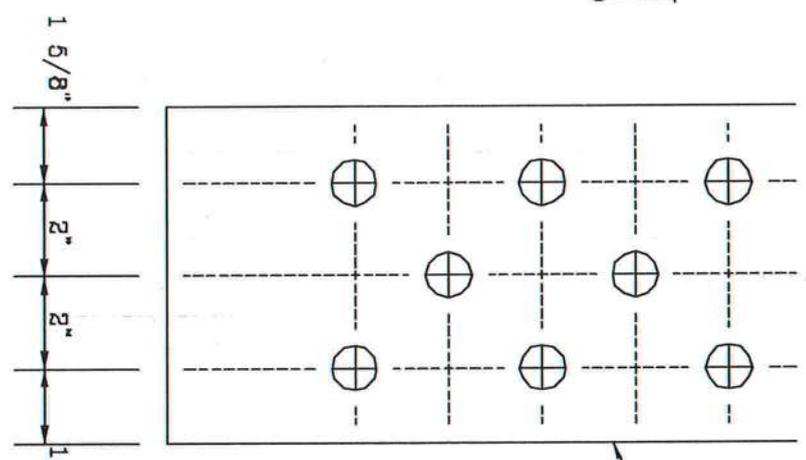
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
 BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
 WASHERS REQUIRED UNDER BOLT HEAD AND NUT

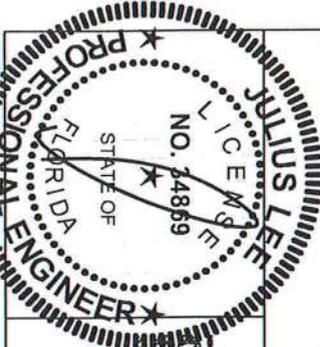


2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTING. REFER TO POST-CORE BUILDING INDEPENDENT SAFETY DEPARTMENT, ESTABLISHED BY THE CROSS-LAMINATED TIMBER INSTITUTE, 2001 CONCORD DR., SUITE 200, WASHINGTON, STATE, 98090. SAFETY PRACTICES PRIOR TO PERFORMANCE OF THESE FUNCTIONS, UNLESS OTHERWISE INDICATED, THE OWNER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
 By Julius Lee at 11:59 am, Jun 11, 2008

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 1400 57th AVE. SUITE 200
 DEERBEEK, FL 33441-2161
 No. 34869
 STATE OF FLORIDA

TC IL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSPI103
BC IL	PSF		-ENG JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

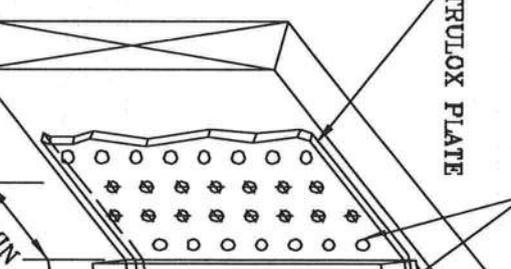
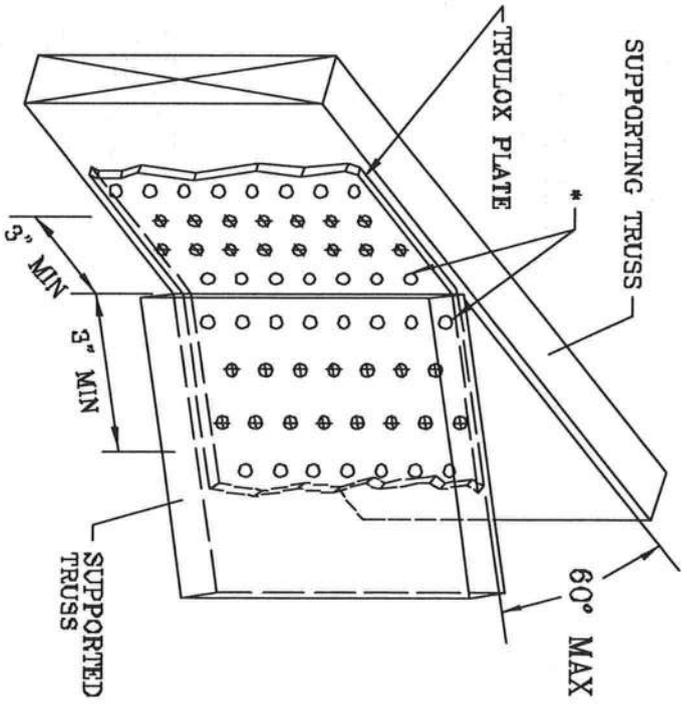
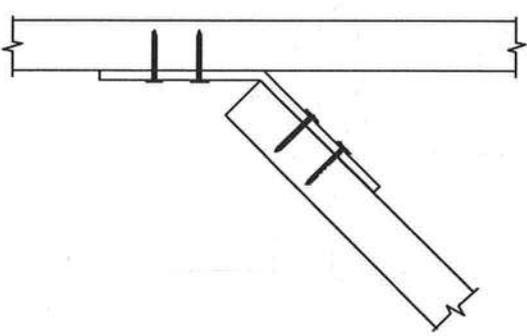
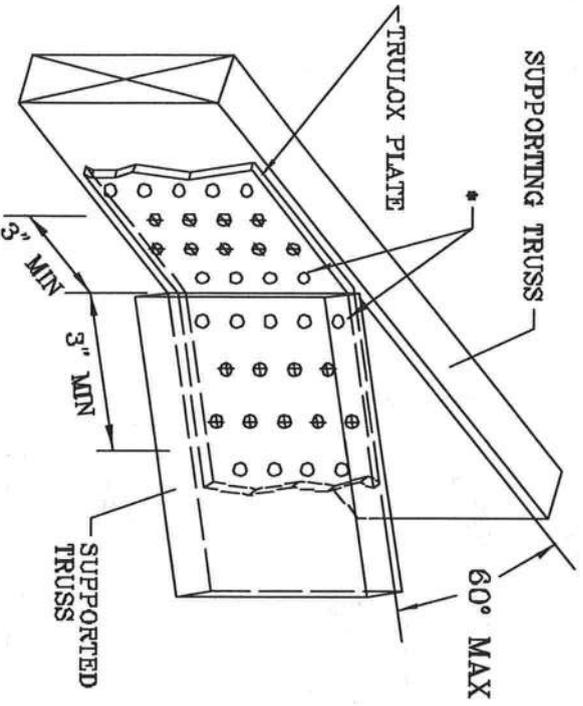
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



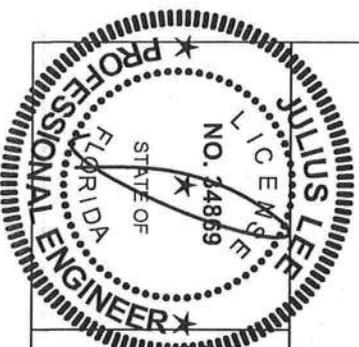
MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
6X6	15	990#

MINIMUM 5X6 TRULOX PLATE

REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008

THIS DRAWING REPLACES DRAWINGS 1.158.989 1.158.988/R
1.154.944 1.152.217 1.152.017 1.159.194 & 1.151.524



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO 3031-1-03 BUILDING DEPARTMENT SAFETY REGULATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 384 JENNIFER DR., SUITE 800, NAUWATON, VA 22715 AND VITA CYCLO TRUSS COUNCIL, 1000 W. 10TH ST., SUITE 100, WASHINGTON, VA 22715 FOR SAFETY PRACTICES PRIOR TO PERFORMING STRUCTURAL PANELS AND SECTION CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

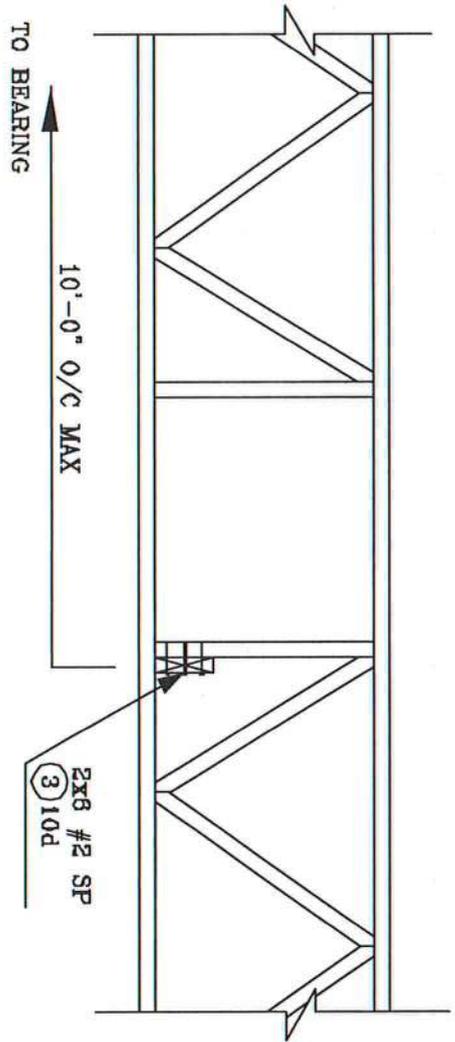
JULIUS LEE'S
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1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-3818

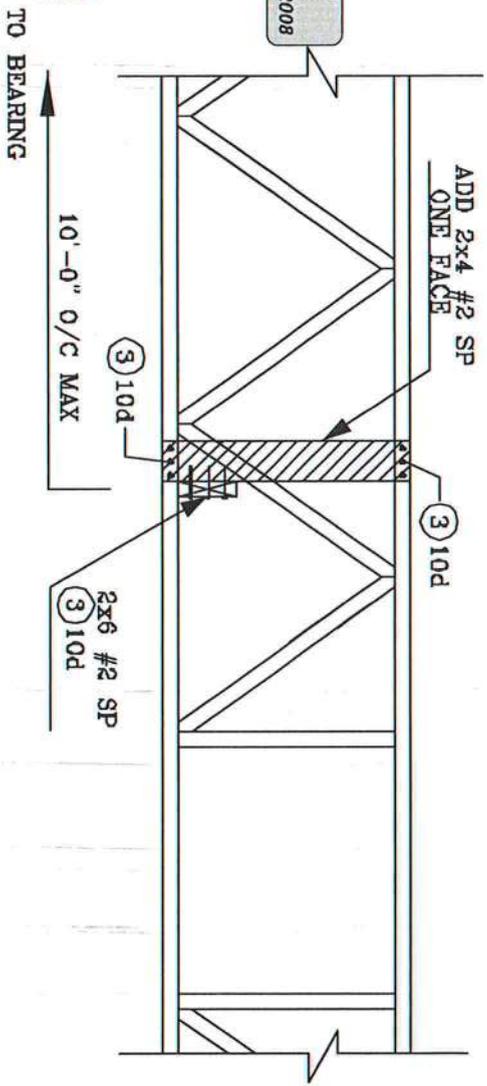
No: 34889
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
	-ENG JL

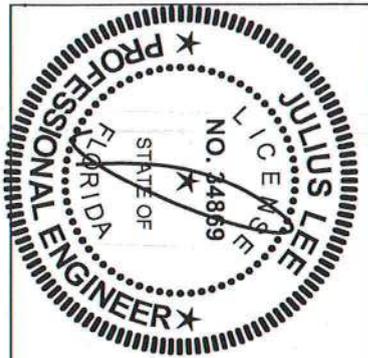
**STRONG BACK DETAIL
SYSTEM-42 OR FLAT TRUSS**



**ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP**



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008

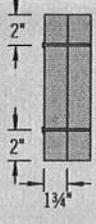
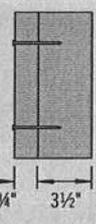
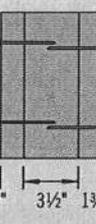
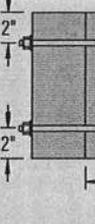


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No: 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

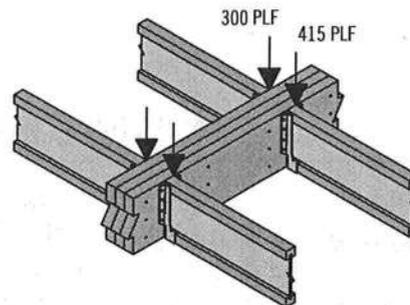
Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
								
3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply			
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	16"	1,020	765	765	680		
		24"				455	465	455
USP WS35 ⁽⁴⁾	2	19.2"				565	580	565
		16"				680	695	680
		24"	480	360	360	320		
USP WS6 ⁽³⁾⁽⁴⁾	2	19.2"	600	450	450	400		
		16"	715	540	540	480		
		24"				350	525	350
3 3/4" TrussLok ⁽⁴⁾	2	19.2"				440	660	440
		16"				525	790	525
		24"	635	475	475	425		
5" TrussLok ⁽⁴⁾	2	19.2"	795	595	595	530		
		16"	955	715	715	635		
		24"		500	500	445	480	445
6 3/4" TrussLok ⁽⁴⁾	2	19.2"		625	625	555	600	555
		16"		750	750	665	725	665
		24"				445	620	445
	2	19.2"				555	770	555
		16"				665	925	665

- (1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.
- (2) Washers required. Bolt holes to be 1/16" maximum.
- (3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- **Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6 ⁽¹⁾	4	1,915	1,435 ⁽⁴⁾	1,435	1,275	1,860 ⁽²⁾	1,405 ⁽²⁾
	6	2,870	2,150 ⁽⁴⁾	2,150	1,915	2,785 ⁽²⁾	2,110 ⁽²⁾
	8	3,825	2,870 ⁽⁴⁾	2,870	2,550	3,715 ⁽²⁾	2,810 ⁽²⁾
3 3/8" or 5" TrussLok™	4	2,545	1,910 ⁽⁴⁾	1,910	1,695	1,925 ⁽³⁾	1,775 ⁽³⁾
	6	3,815	2,860 ⁽⁴⁾	2,860	2,545	2,890 ⁽³⁾	2,665 ⁽³⁾
	8	5,090	3,815 ⁽⁴⁾	3,815	3,390	3,855 ⁽³⁾	3,550 ⁽³⁾

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

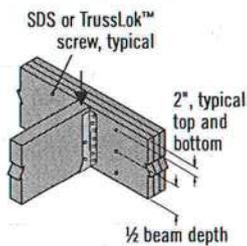
(2) 6" long screws required.

(3) 5" long screws required.

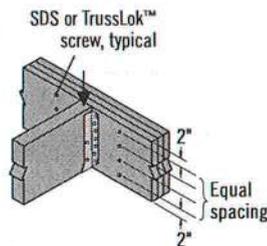
(4) 3 1/2" and 3 3/8" long screws must be installed on both sides.

Connections

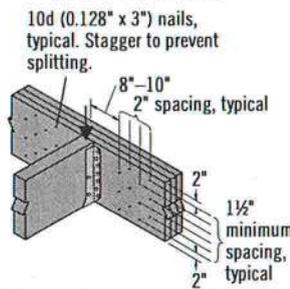
4 or 6 or Screw Connection



8 Screw Connection

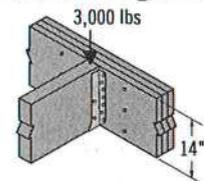


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

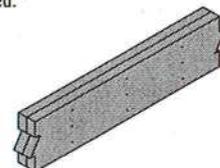
on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

L6