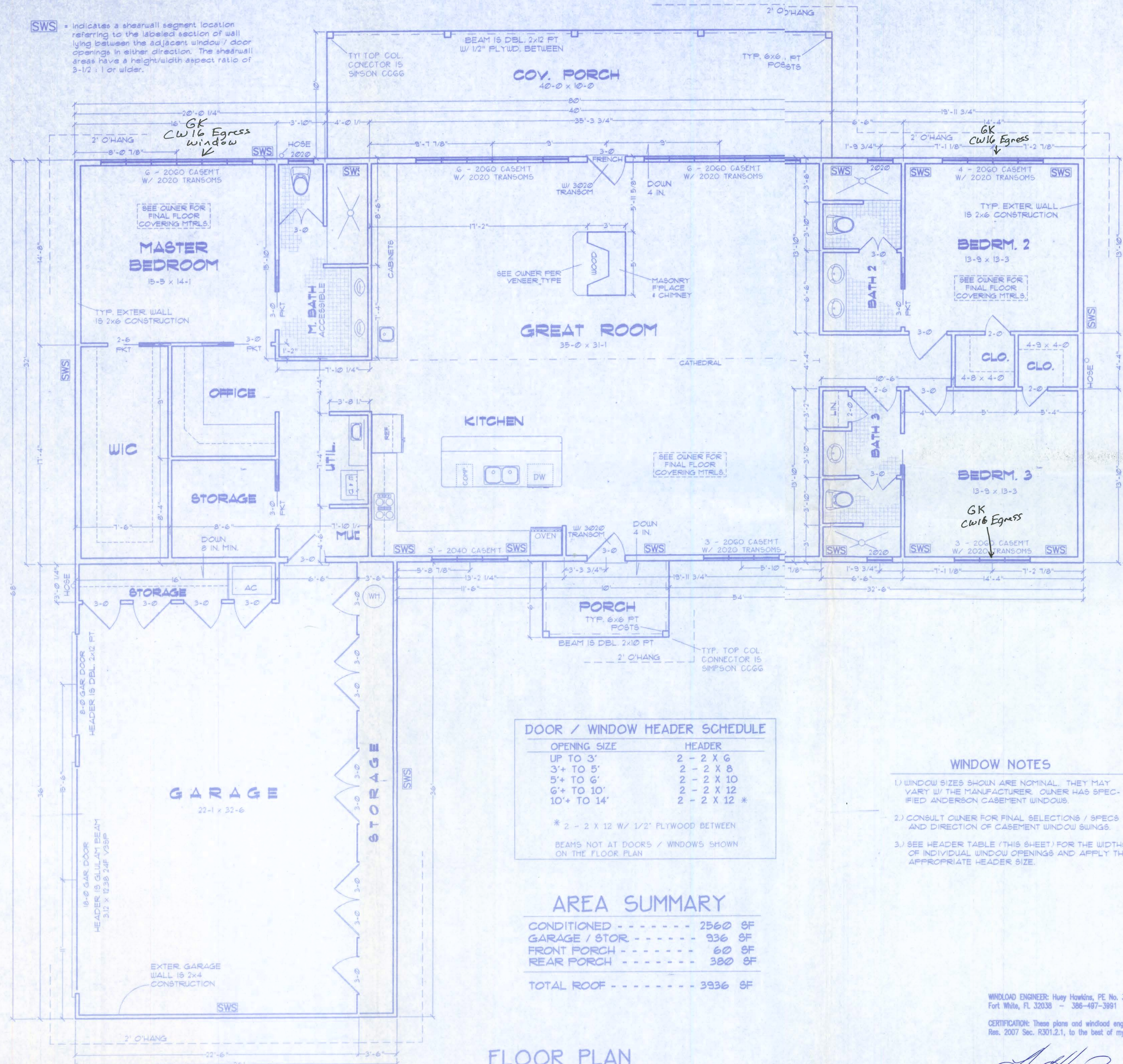
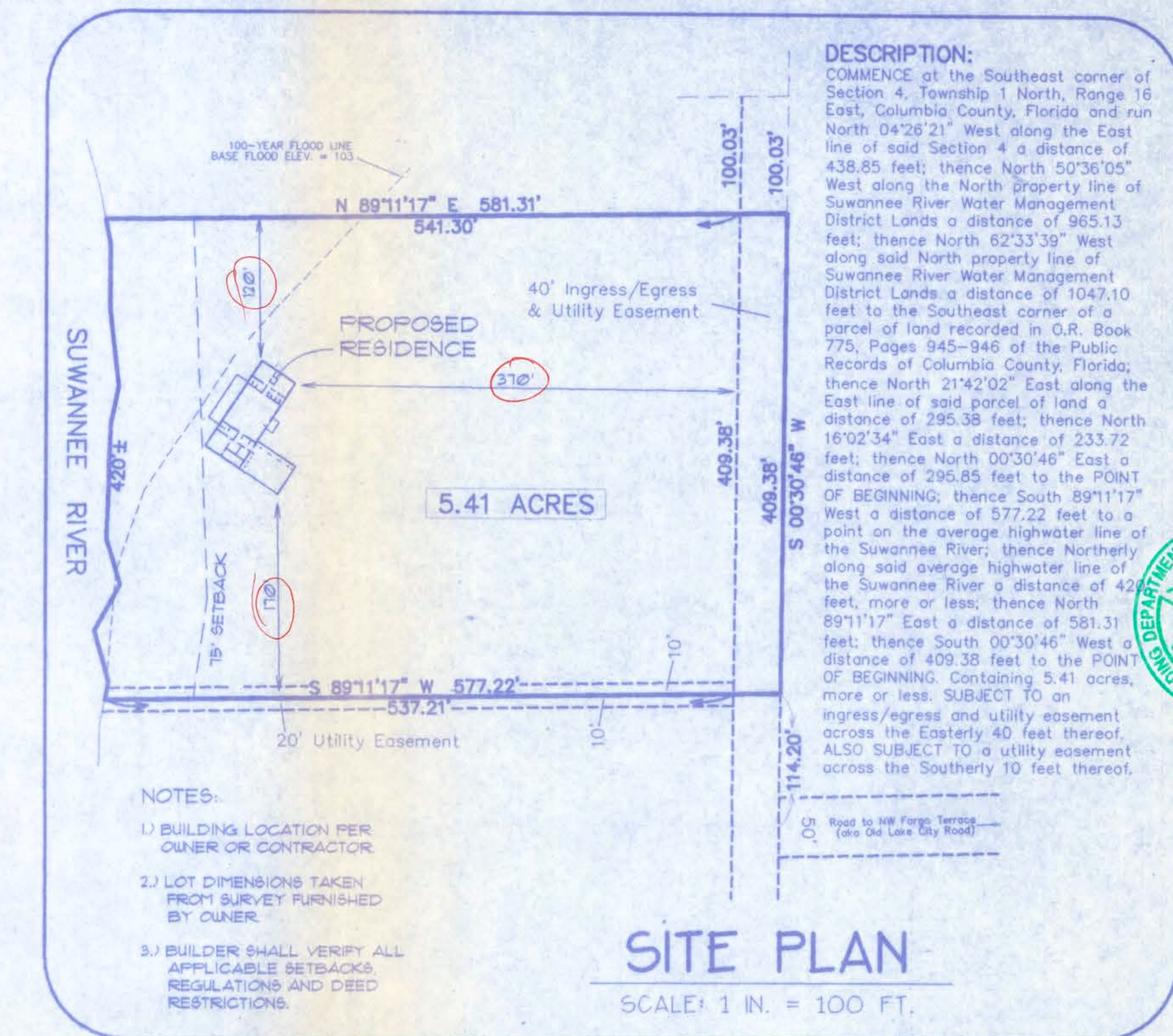


Greene Residence

SWS - Indicates a shearwall segment location referring to the labeled section of wall lying between the adjacent window / door openings in either direction. The shearwall areas have a height/width aspect ratio of 3-1/2 : 1 or wider.



FLOOR PLAN
SCALE: 1/4 IN. = 1 FT.



SITE PLAN
SCALE: 1 IN. = 100 FT.

GENERAL NOTES

- 1.) See 'Wind Load Detail Sheet 5-1' and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 4.) Windows to be double glazed. Sizes shown are nominal and may vary with manufacturer.
- 5.) Roof Truss design is the responsibility of the supplier.
- 6.) The Truss Manufacturer shall prepare Shop Drawings indicating Truss placement, Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- 7.) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- 8.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

Index to Sheets

SHEET A-1	-----	SITE PLAN + FLOOR PLAN + GEN. NOTES
SHEET A-2	-----	ELEVATIONS
SHEET A-3	-----	ELEVATIONS
SHEET A-4	-----	FOUNDATION + SECTIONS
SHEET A-5	-----	ELECTRICAL
SHEET 5-1	-----	ENGINEERING + WALL SEC.
SHEET ATS-1	-----	TE-DOWN SYSTEM MANUF. SPECS

WINDLOAD ENGINEER: Huey Hawkins, PE No. 33685 - 6855 SW Elm Church Rd., Fort White, FL 32038 - 386-497-3891

CERTIFICATION: These plans and windload engineering, comply with Florida Building Code Res. 2007 Sec. R301.2.1, to the best of my knowledge.

SIGNED: *Huey Hawkins* DATE: 2/12/2009

A-1

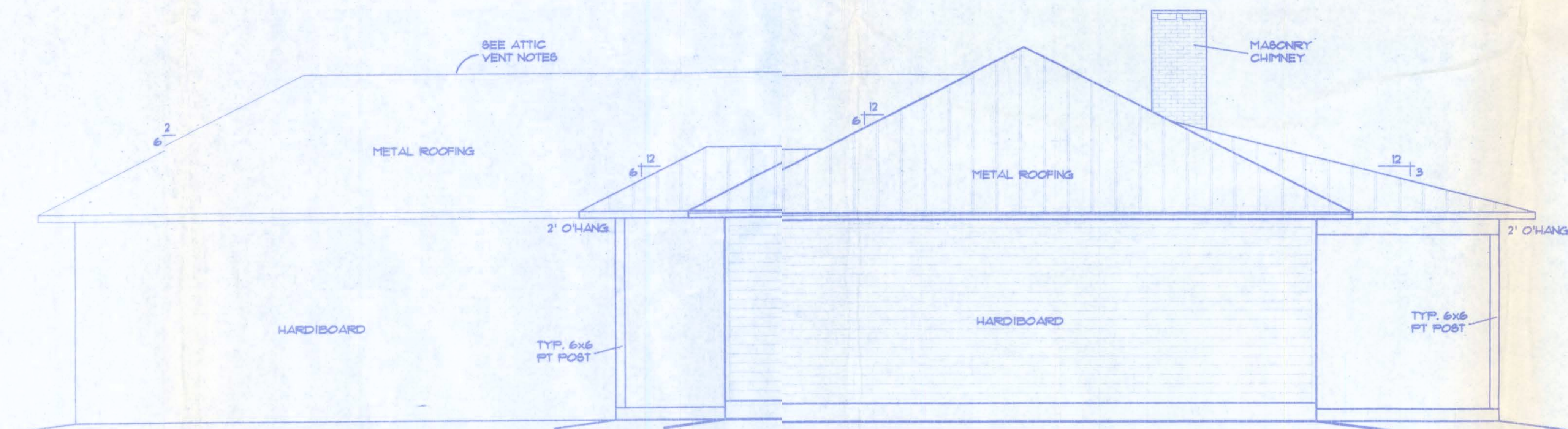
FILE: 09-003	GREENE RESIDENCE	SHEET: 1 OF 7
DATE: 6-8-09		CAD FILE: 09003
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	142 SW Sagewood Cir., Lake City, FL 32024 Phone (386) 755-5891	REV:





FRONT ELEVATION

SCALE: 1/4" IN. = 1 FT.



RIGHT ELEVATION

SCALE: 1/4" IN. = 1 FT.

ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by venting openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with a 1/4 inch (3.2 mm) minimum to 1/2 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vent.

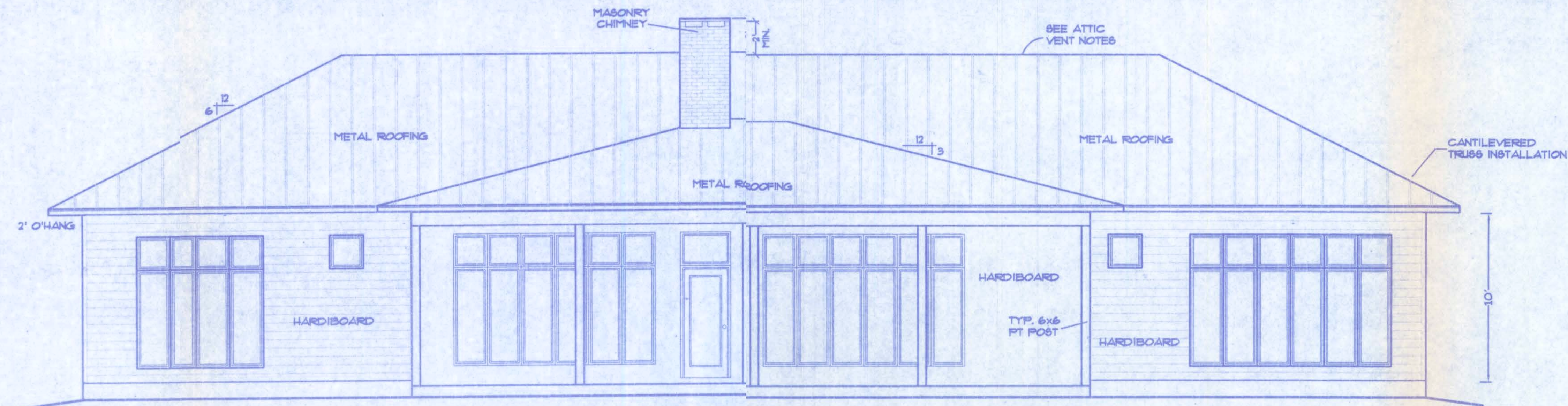
WINDLOAD ENGINEER: Huey Hawkins, PE No. 33665 - 6855 SW Elm Church Rd., Fort White, FL 32038 - 386-497-3991

CERTIFICATION: These plans and windload engineering, comply with Florida Building Code Res. 2007 Sec. R301.2.1, to the best of my knowledge.

SIGNED: *[Signature]* DATE: 2/13/2009

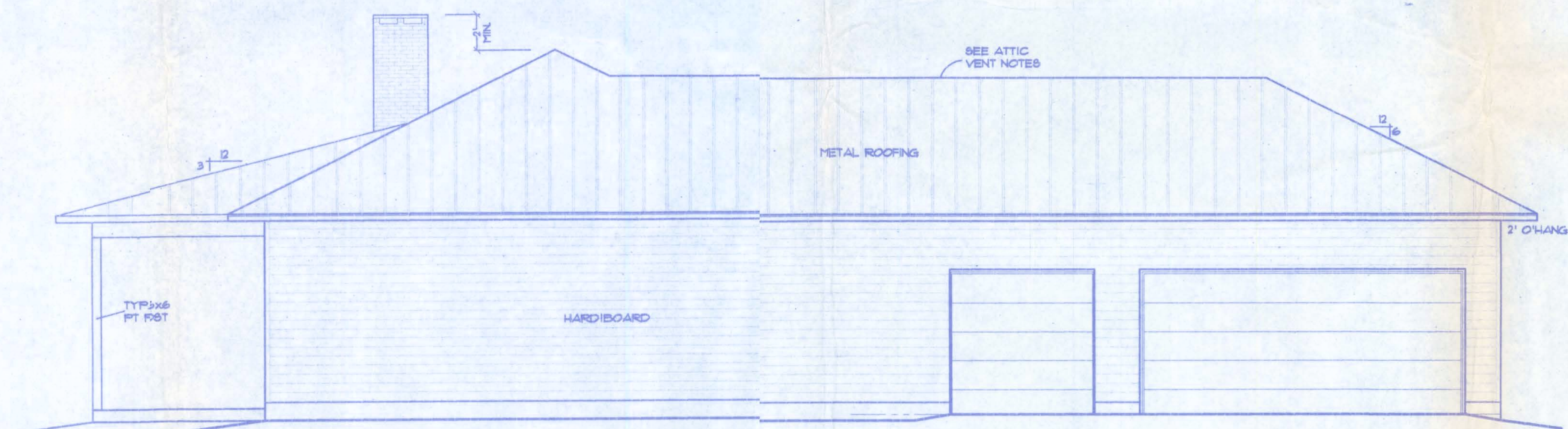
A-2

FILE: 09-003	GREENE RESIDENCE	SHEET: 2 OF 7
DATE: 6-8-09		CAD FILE: 09003
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV: 1-3-09
CHECK: T A D	192 SW Sagewood Cir., Lake City, FL 32024 Phone (386) 755-5891	REV:



REAR ELEVATION

SCALE: 1/4" IN. = 1 FT.



LEFT ELEVATION

SCALE: 1/4" IN. = 1 FT.

ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces shall have cross ventilation applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with 1/4 inch (3.2 mm) minimum to 1/2 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 5 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

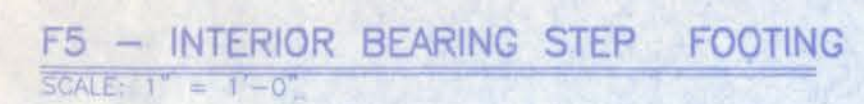
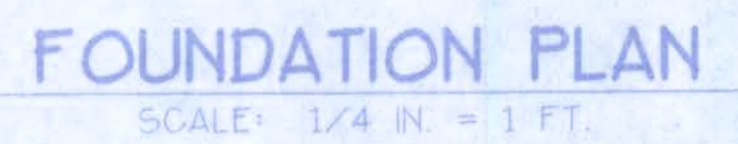
WINDLOAD ENGINEER: Huey Hawkins, PE No. 33665 - 6855 SW Elm Church Rd., Fort White, FL 32038 - 386-497-3991

CERTIFICATION: These plans and windload engineering, comply with Florida Building Code Res. 2007 Sec. R301.2.1, to the best of my knowledge.

SIGNATURE: *[Signature]* DATE: 2/13/2009

A-3

FILE: 09-003	GREENE RESIDENCE	SHEET: 3 OF 7
DATE: 6-8-09		CAD FILE: 09003
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services 192 SW Sagewood Cir., Lake City, FL 32024 Phone (386) 755-5891	REV: 1-3-09
CHECK: T A D		REV:

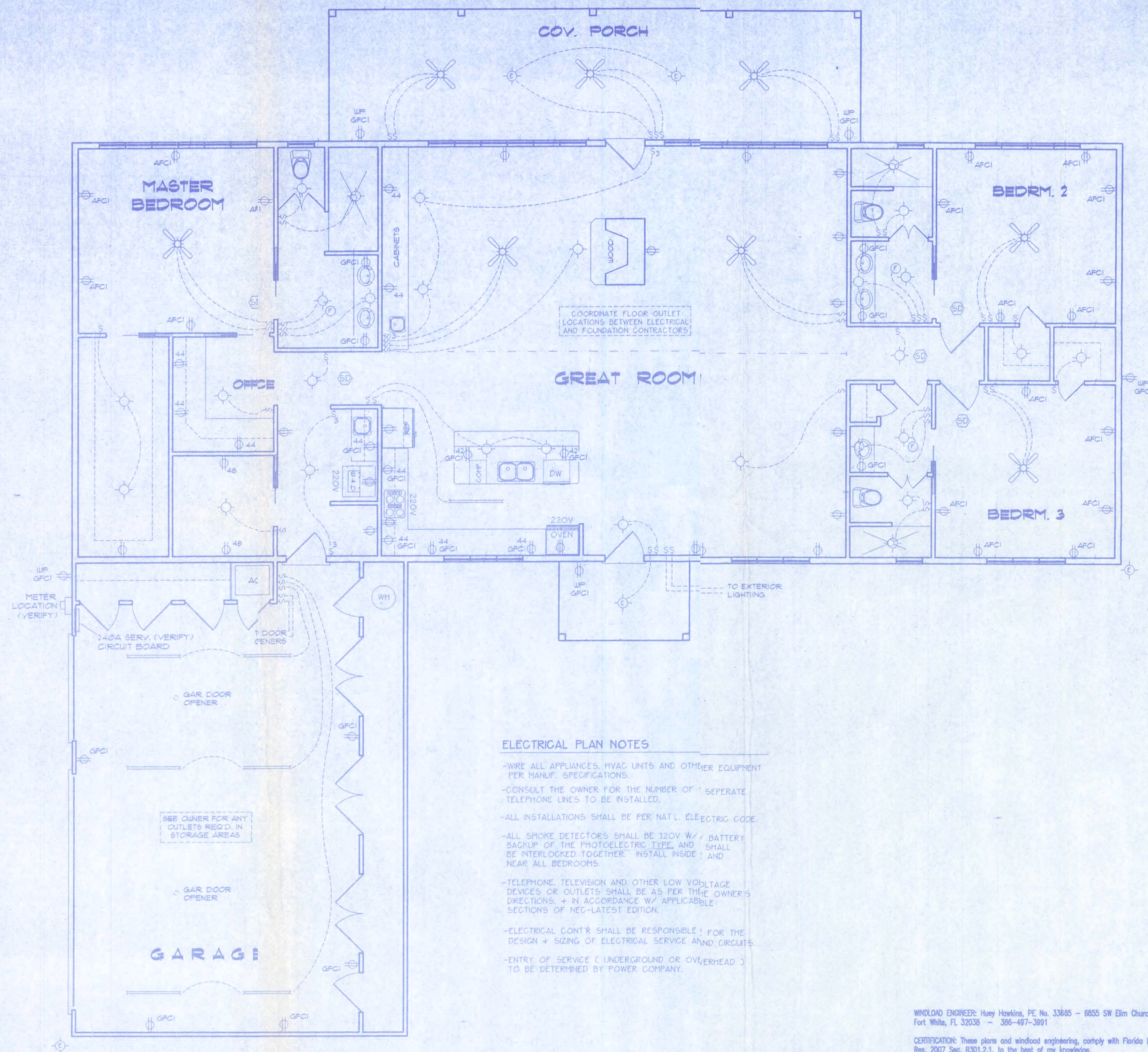


- CONTRACTOR SHALL EXAMINE ROOF TRUSS PLAN (BY SUPPLIER) TO DETERMINE ANY ADDITIONAL BEARING REQUIREMENTS BEFORE FINALIZING THE FOUNDATION PLAN.
- ALL CONCRETE IS 2500 PSI STRENGTH (MIN.)
- VERIFY DIMENSIONS WITH FLOOR PLAN
- SITE ANALYSIS AND PREPARATION DATA IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE CONTRACTOR / OWNER.

SIGNED: Amy O'Neil DATE: 7/13/2009

FILE: 09-003	GREENE RESIDENCE	SHEET: 4 OF 7
DATE: 6-8-09		CAD FILE: 09OC3
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting & Technical Services	REV:
CHECK: T A D	142 SW Oswegood, Galt, Lake City, FL 32024 Phone (C-386) 755-5891	REV:

A-4



ELECTRICAL SYMBOL LEGEND	
	= FLOURESCENT LIGHTING FIXTURE.
	= CEILING LIGHT FIXTURE
	= EXTERIOR LIGHTING FIXTURE
	= LIGHT SWITCH.
	= THREE-WAY SWITCH.
	= 110 V. DUPLEX OUTLET.
	= SPECIAL HEIGHT 110 V. DUPLEX OUTLET
	= GROUND FAULT CIRC. OUTLET
	= ARC FAULT CIRC. OUTLET
	= 110 V. SINGLE RECEPTACLE OUTLET.
	= 220 VOLT OUTLET (4 WIRE)
	= FAN LOCATION (CEILING)
	= FAN LOCATION (EXHAUST)
	= SMOKE DETECTOR

ELECTRICAL PLAN NOTES

- WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE, AND NEAR ALL BEDROOMS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, + IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.

ELECTRICAL PLAN

NOT TO SCALE

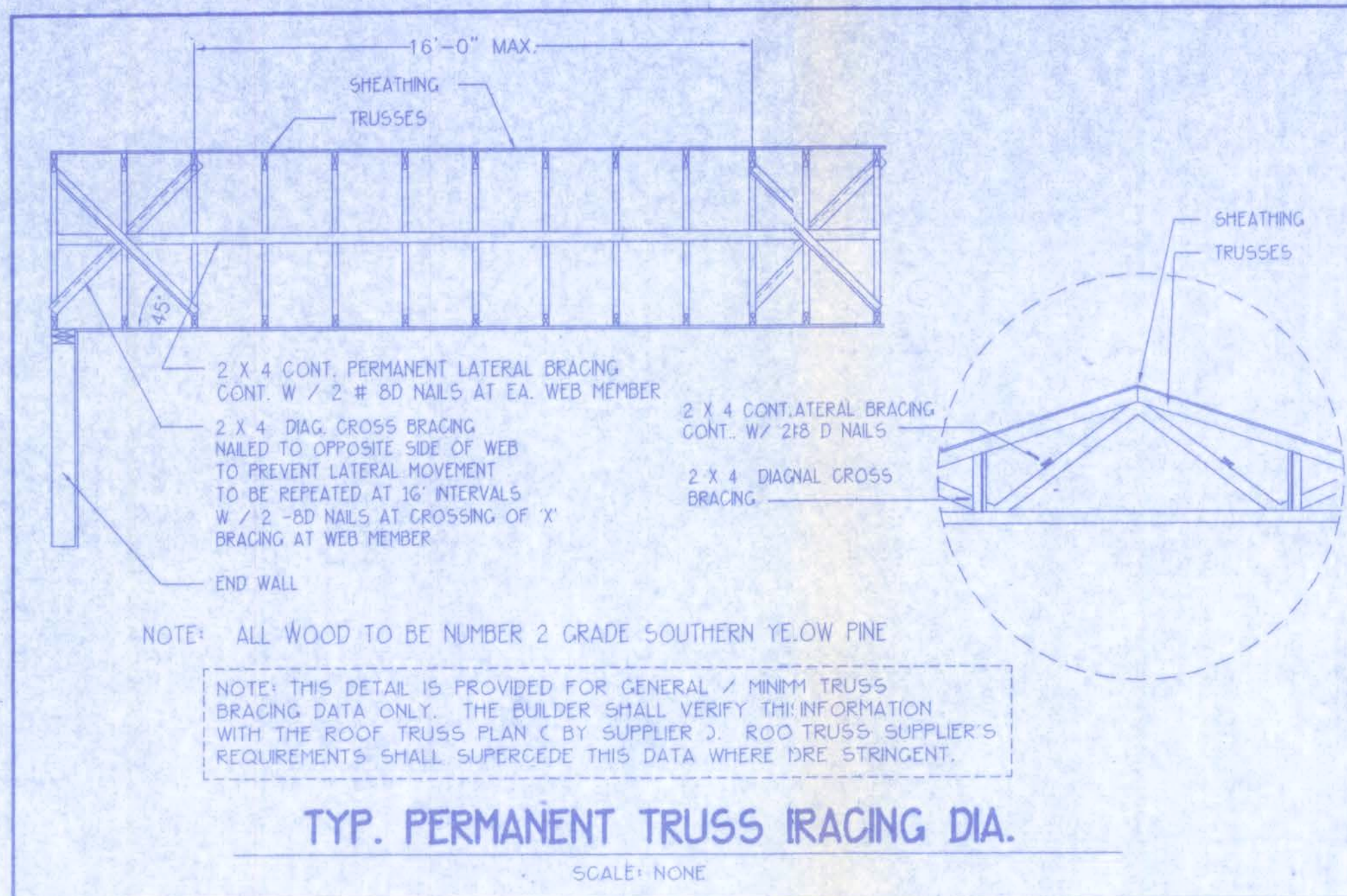
WINDLOAD ENGINEER: Huey Hawkins, PE No. 33685 - 6855 SW Elm Church Rd., Fort White, FL 32036 - 386-497-3991

CERTIFICATION: These plans and windload engineering, comply with Florida Building Code Res. 2007 Sec. R301.2.1, to the best of my knowledge.

SIGN: DATE: 7/13/2009

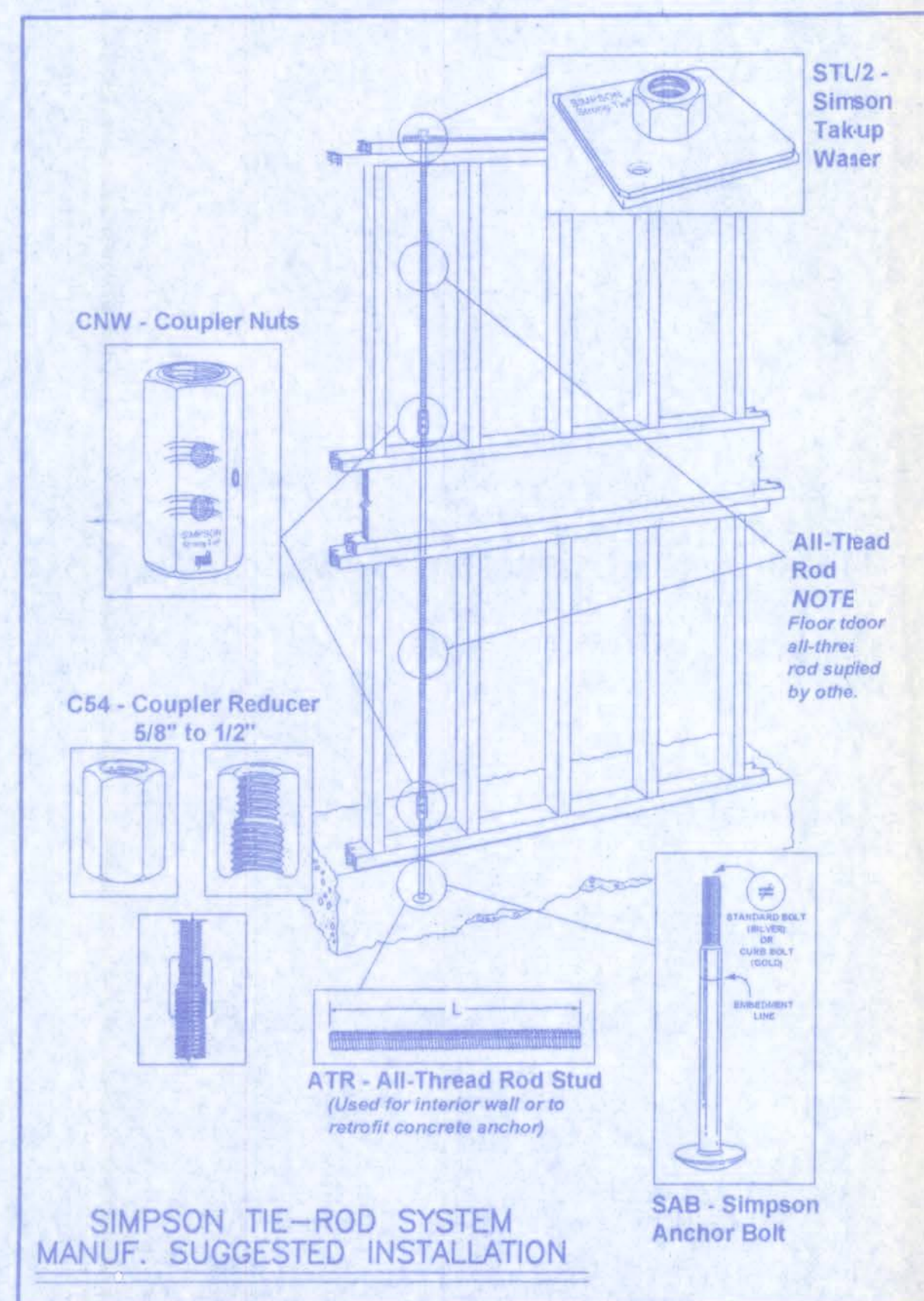
A-5

FILE: 09-003	GREENE RESIDENCE	SHEET: 5 OF 7
DATE: 6-8-09		CAD FILE: 09003
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services 192 SW Sagewood Cir., Lake City, FL 32024 Phone (386) 758-5891	REV:
CHECK: T A D		REV:



ENGINEER'S NOTES:

- The report establishes the minimum requirements for wind load stability. It is the owner/builder's responsibility to provide materials and construction techniques which comply with SBC requirements for the stated wind velocity. It is the builder's responsibility to provide a continuous load path from trusses to foundation.
- Since truss engineering was not complete at the time of this analysis, it is the builder's responsibility to select uplift connections based on truss engineering uplift and provide footings for interior bearing walls. Builder is to furnish truss engineering to wind load engineer for review of truss reactions on the building structure. Strap 2x6 rafters with min uplift connection 415lb each end, 2x8 rafters 700lb each end.
- Site analysis and preparation information is not part of this plan and is responsibility of the owner. All foundations and footings are designed for stable soil conditions with 1000 psf bearing capacity. It is the owner's / builder's responsibility to verify soil and clean fill are compacted to provide 1000 psf minimum bearing capacity or to request foundation design based on actual site conditions.
- Manufacturers and product number for connectors, anchors, and reinforcement listed for example not endorsement. An equivalent device of the same or other manufacturer can be substituted for any devices listed in the example tables as long as it meets the required load capacities. Manufacturer's installation instructions must be followed to achieve rated loads.
- Anchor bolts - A-307. Minimum embedment: 7" in concrete or reinforced concrete; 15" in grouted CMU.
- Concrete - Minimum compressive strength, $f_c = 2500$ psi.
- Rebar - Grade 40 deformed bars, $F_y = 36$ ksi. All laps 40Dx8 (25' for #5) less otherwise specified.
- Nails - All nails are common nails unless otherwise specified or accepted by test reports as having equal structural values.



DESIGN DATA

WIND LOADS ARE PER FLORIDA BUILDING CODE 2001, SECTION 1606.2	
(FOR ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED AND GABLE SHAPED ROOFS HAVING A MEAN ROOF HEIGHT NOT EXCEEDING THE LEAST HORIZONTAL DIMENSION OF THE BUILDING OR 60 FT. NOT SITED ON THE UPPER HALF OF A HILL OR ESCARPMENT 60 FT IN EXPOSURE B, 30 FT IN EXPOSURE C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50% HEIGHT OR 1 MILE WHICHEVER IS LESS.)	
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE	
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION	
1.) BASIC WIND SPEED = 110 MPH	
2.) WIND IMPORTANCE FACTOR = 1	
3.) BUILDING CATEGORY = I	
4.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	
5.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES:	
(DOORS & WINDOWS) +21.8 / -29.1 PSF	
GARAGE DOOR (N/A)	
9x7 +19.3 / -22.2 PSF; 16x16, +18.5 / -20.9	
DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	18 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
NO SNOW LOAD	
WIND LOADS ARE PER FLORIDA BUILDING CODE 2001, SECTION 1606.2	

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2001, SECTION 1606 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT FOR NOTES ON THEIR SEALED TRUSS SHEETS.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES WHICH COMPLY WITH FBC 2001 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

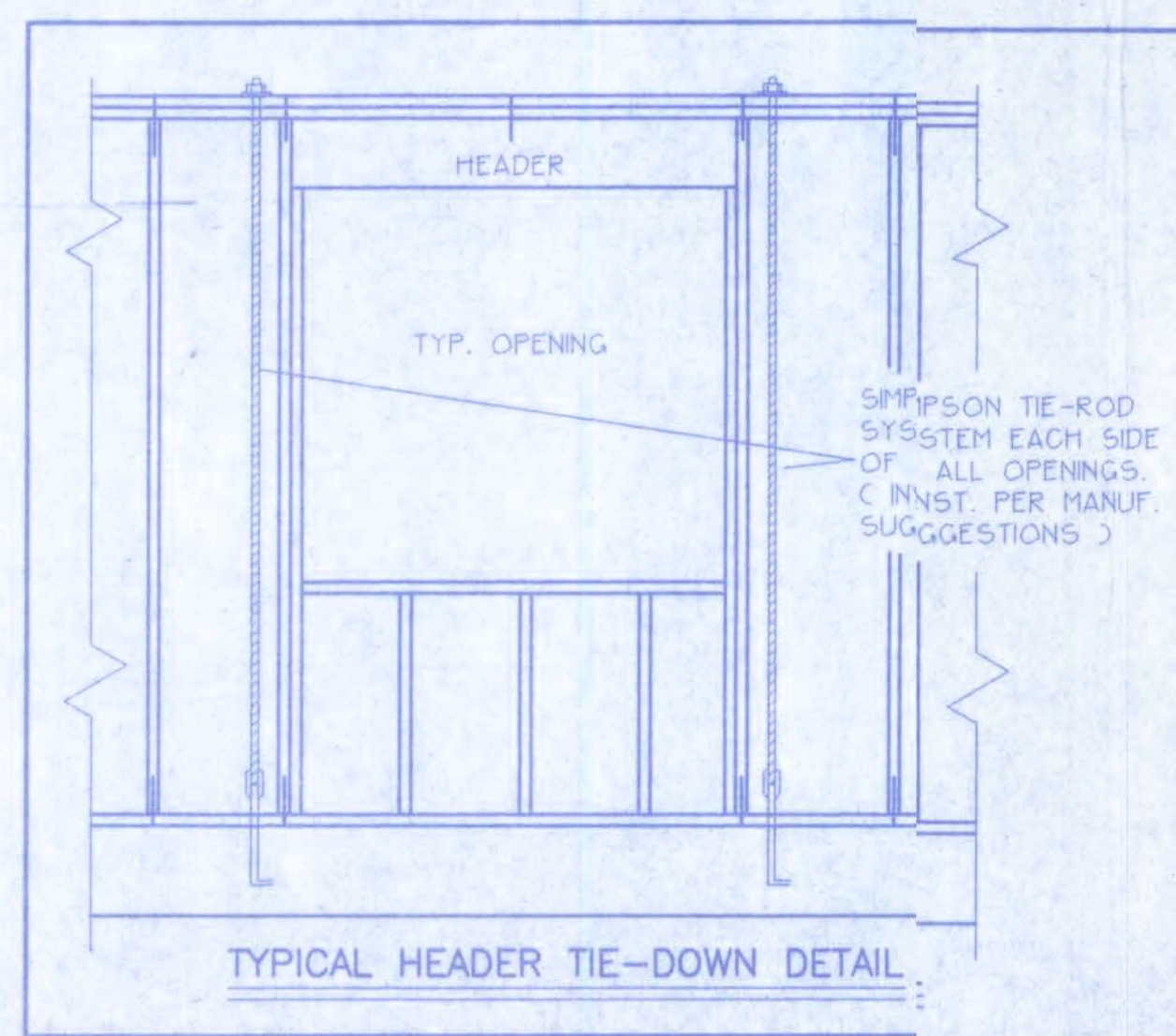
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. SELECT UPLIFT CONNECTIONS AND PROVIDE FOOTINGS BASED ON TRUSS ENGINEERING REACTIONS. FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS.

TRUSS ANCHOR TABLE

OBTAIN UPLIFT REQ. FROM TRUSS MANUFACTURER'S ENGINEERING					
UPLIFT LBS.	TOP CONNECTOR *		BOTTOM CONNECTOR *		
< 415	H2 5	10-8d	415	NO SPECIAL CONNECTOR REQ.	
< 750	H16	6-10d, 1-1/2"	750	NO SPECIAL CONNECTOR REQ.	
< 905	H10	16-8d, 1-1/2"	905	NO SPECIAL CONNECTOR REQ.	
< 1250	H16	10-10d, 1-1/2"	1250	SPH4 W/10-10d, 1-1/2" + 1/2" AB	970
< 1245	HTS20	24-10d, 1-1/2"	1245	LTT19 W/8-16d + 1/2" AB	1080
< 2490	2-HTS20	24-10d, 1-1/2"	2490	HD2A-2.5", 5/8" AB	2365

UPLIFT GREATER THAN 2500 LBS. REQUIRES ENGINEERING APPROVAL

* MODEL NUMBERS SHOWN ARE FOR SIMPSON CONNECTORS



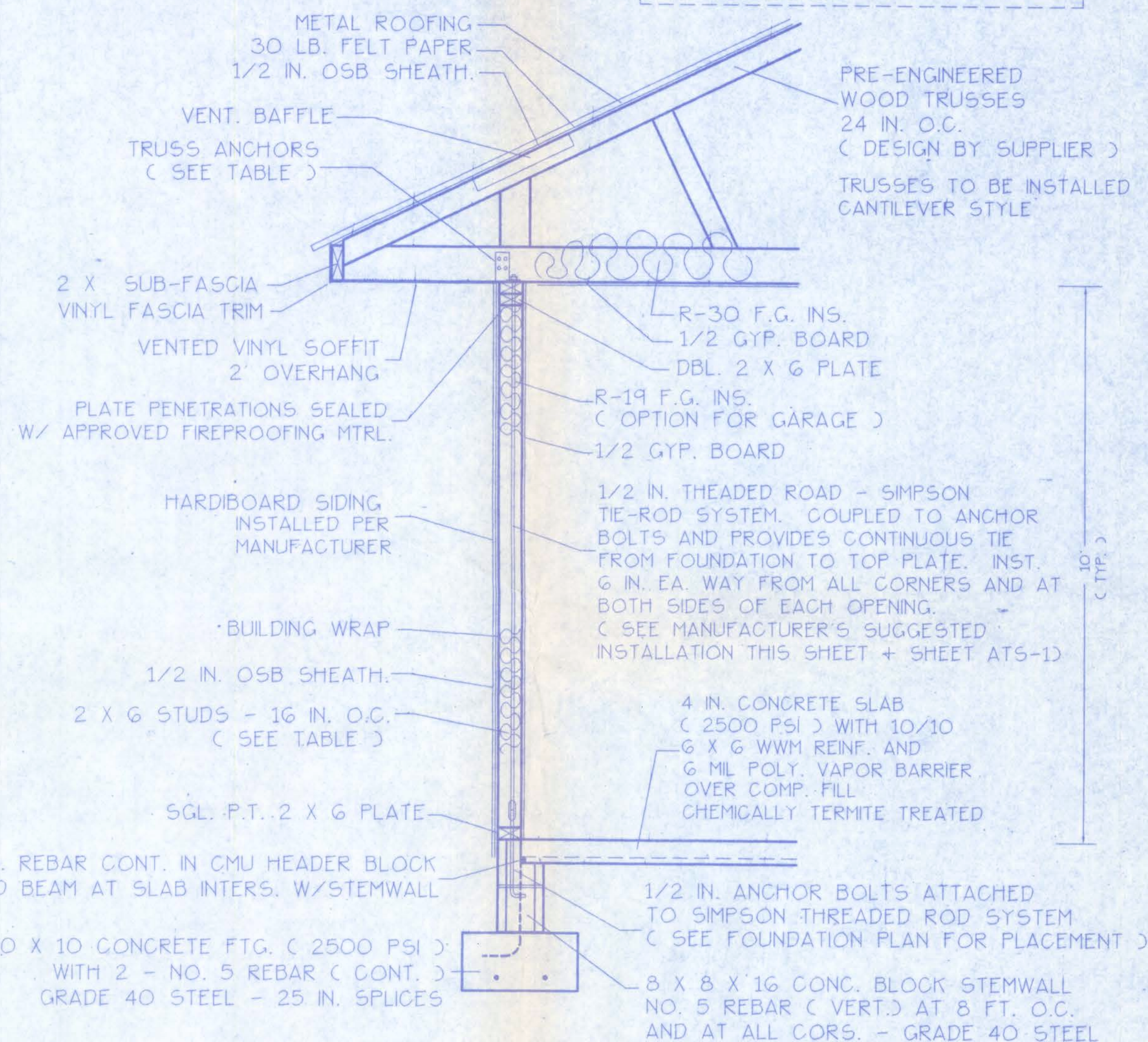
ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with 1/8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

GENERAL TRUSS NOTES:

- TRUSSES SHALL BE DESIGNED BY A LICENSED ENGINEER, AND IN ACCORDANCE WITH THE REQUIREMENTS OF THE "NATIONAL FOREST PRODUCTS ASSOCIATION" MANUAL FOR "STRESS RATED LUMBER AND ITS CONNECTIONS" LATEST E.D. ALONG W/ THE "TRUSS PLATE INSTITUTE" SUGGESTED GUIDELINES FOR TEMPORARY AND PERMANENT BRACING AND HANDLING OF TRUSSES. TRUSS SHOP DRAWINGS SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, DETS. + TRUSS TO TRUSS CONNECTIONS.
- TRUSS SHOP DRAWINGS SHALL BE SIGNED + SEALED BY THE DESIGNING ENGINEER.



TYPICAL WALL SECTION

SCALE: 3/4 IN. = 1 FT.

WALL STUD TABLE

STUDS ARE SPRUCE/PINE/FIR AT 16 IN. O.C.	
1 - 2 X 4	TO 10 FT. WALL HEIGHT
2 - 2 X 4	TO 12.5 FT. WALL HEIGHT
3 - 2 X 4	TO 16.5 FT. WALL HEIGHT
1 - 2 X 6	TO 16 FT. WALL HEIGHT
2 - 2 X 6	TO 21 FT. WALL HEIGHT

TOP PLATE IS 2 - 2 X 4 (OR 2 X 6) SPRUCE/PINE/FIR.
BOTTOM PLATE IS 2 X 4 (OR 2 X 6) PRESS. TREATED BEARING ON FOUNDATION WALL.

WINDLOAD ENGINEER: Huey Hawkins, PE No. 33865 - 6855 SW Elm Church Rd., Fort White, FL 32038 - 386-487-3891

CERTIFICATION: These plans and windload engineering, comply with Florida Building Code Res. 2007 Sec. R301.2.1, to the best of my knowledge.

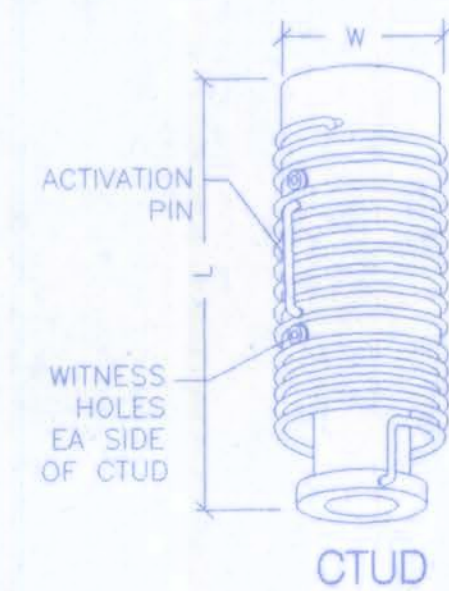
SIGNED: *[Signature]* DATE: 7/19/2009

S-1

FILE: 09-003	GREENE RESIDENCE	SHEET: 6 OF 7
DATE: 6-8-09		CAD FILE: 09003
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	192 SW Sagewood Cir., Lake City, FL 32084 Phone: (386) 755-5891	REV:

1. ATS IS AN ASSEMBLY OF STEEL COMPONENTS, WHICH INCLUDE RODS, PLATES, COUPLER NUTS, TAKE-UP DEVICES AND NUTS. STUDS, POSTS AND BLOCKING ARE NOT SHIPPED WITH THE ANCHOR TIEDOWN SYSTEM. COMPRESSION MEMBERS SHOWN ARE THE MINIMUM SIZES BASED ON FULL SECTION MEMBERS, WITH THE SPECIFICATIONS NOTED BELOW, AND ARE BASED ON LOADS MATCHING THE TENSION FORCE OF THE ROD OR COMPRESSION FORCES PROVIDED BY THE DESIGNER. THE DESIGNER IS RESPONSIBLE FOR COORDINATION OF MEMBER SPECIFICATIONS, STUD AND POST DESIGN DUE TO CODE LOAD COMBINATIONS AND MODIFICATIONS (HOLES, NOTCHES, ETC) TO THE STUDS AND POSTS.
2. SIMPSON STRONG-TIE IS PROVIDING THE ANCHOR TIEDOWN SYSTEMS TO MEET THE DESIGN FORCES PROVIDED AND DETERMINED BY THE DESIGNER AND THE GOVERNING JURISDICTION REQUIREMENTS. SIMPSON STRONG-TIE HAS NOT CONFIRMED AND IS NOT RESPONSIBLE FOR ANY OF THE DESIGN, ENGINEERING, CALCULATIONS OR DERIVATION OF STRUCTURAL FORCES RELAY TO THE BUILDING. SIMPSON STRONG-TIE HAS NOT CONFIRMED AND IS NOT RESPONSIBLE FOR VERIFYING THE ANCHOR TIEDOWN SYSTEM'S ADHERENCE TO BUILDING CODE DRIFT REQUIREMENTS OR ITS PERFORMANCE IN CONSIDERATION OF STRUCTURAL DEFORMATION COMPATIBILITY. BOTH THE DESIGNER AND THE GOVERNING JURISDICTION MUST APPROVE OTHER BRAND/EDOWN SYSTEMS, INCLUDING CALCULATIONS AND INSTALLATION DETAILS, PRIOR TO CONSTRUCTION.
3. ALL COMPRESSION LUMBER IS BASED ON THE FOLLOWING, UNO IN THE ELEVATION DRAWING: SPECIES GRADE ALL WOOD PLATES (TOP AND SOLE/SILL) ARE BASED ON THE FOLLOWING, UNO: PLESPECIES W/ F_{cperp} = F_{cperp}!
- LUMBER SPECIFICATIONS NOTED IN THE PROJECT STRUCTURAL DRAWINGS MAY VARY AND RESULT IN LOWER ALLOWABLE COMPRESSION CAPACITIES. NOTIFY SIMPSON FOR DIFFERENCES BETWEEN DRAWINGS.
4. PROVIDE SHEAR WALL EDGE NAILING, PER THE DESIGNER, EQUALLY DISTRIBUTED TO EACH STUD OR POST ON EITHER SIDE OF THE ATS ROD. SEE "SHEARWALL EDGE NAILING" FOR MORE DETAILS.
5. MILL CERTIFICATE OR LETTER OF COMPLIANCE PROVIDED FOR ALL 120 KSI & 150 KSI RODS. STRONG-ROD NUTS CONFORM TO ASTM A563 GRADE DH HEAVY HEX (HIGH STRENGTH) JO COUPLER NUTS CONFORM TO ASTM A563 GRADE A (STANDARD STRENGTH) OR ASTM A5 GRADE B OR DH (HIGH STRENGTH). ALL THREADS ARE UNC (UNIFIED NATIONAL COARSE).
6. CONTRACTOR OR INSTALLER OF ATS IS TO CUT RODS TO LENGTH AS REQUIRED.
7. SIMPSON STRONG-TIE HIGHLY RECOMMENDS THAT A PRE-CONSTRUCTION MEETING AND REVIEW OF THE ATS EXHIBIT SHEET BE HELD PRIOR TO PLACEMENT OF CONCRETE TO MITIGATE THE INSTALLATION PROCESS AND VERIFY QUANTITIES. TO COORDINATE THIS MEETING, CALL SIMPSON SALES AT 800-999-5099.
8. THE ENGINEER OF RECORD IS RESPONSIBLE FOR CHECKING ELONGATION AND THAT THE STEM MEETS CODE DRIFT REQUIREMENTS.

GENERAL NOTES AND CONDITIONS OF USE

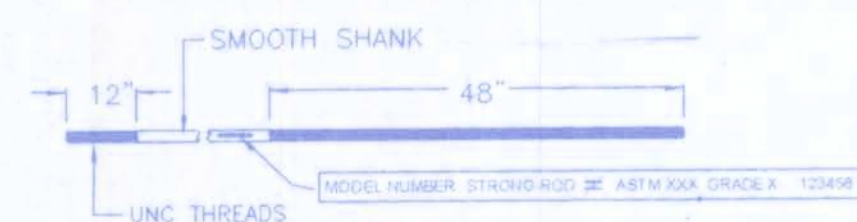


MODEL NO.	THREADED ROD SIZE (IN)	W (IN)	L (IN)	ALLG. TENSION CAPACITY (LBS)
ATS-CTUD55	5/8-5/8	1 7/8	5	5,520
ATS-CTUD77	7/8-7/8	2	5 1/2	11,795
ATS-CTUD75	7/8-5/8	2	5 1/2	11,795
ATS-CTUD99	1 1/8-1 1/8	2 1/2	6 1/8	5,955
ATS-CTUD97	1 1/8-7/8	2 1/2	6 1/8	5,955

- COUPLING TAKE-UP DEVICES COMPENSATE FOR UP TO 1" OF SHRINKAGE OR SETTLEMENT DUE TO DEAD LOAD AND CONSTRUCTION LOADING.
- ALLOWABLE CAPACITIES ARE BASED ON ULTIMATE LOADS DIVIDED BY A FACTOR OF SAFETY OF 3.
- ALLOWABLE CAPACITIES DO NOT INCLUDE A 33% SELF STRESS INCREASE. NO FURTHER INCREASE IS ALLOWED.

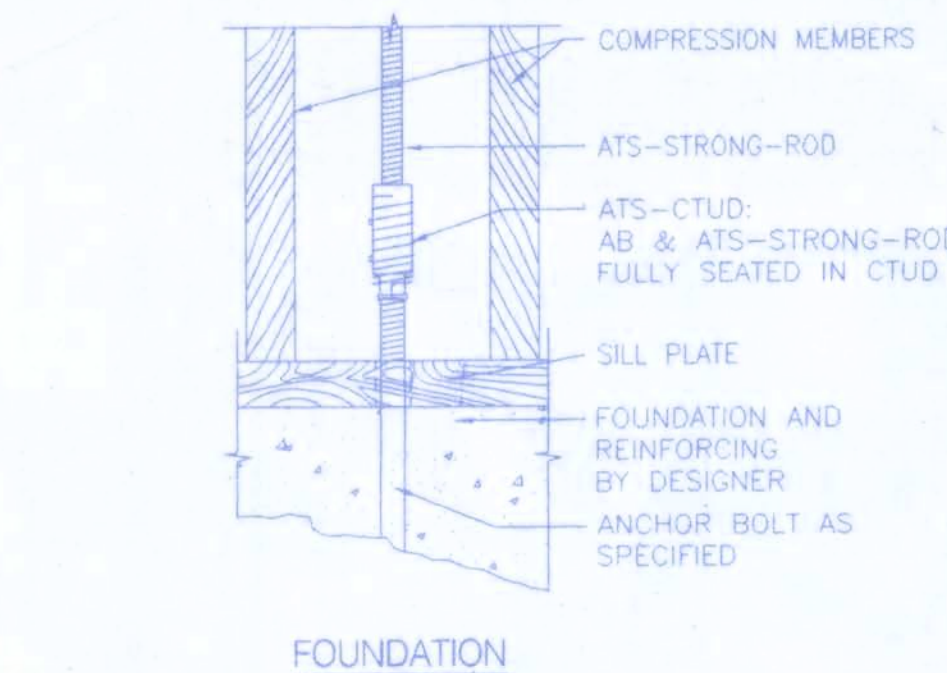
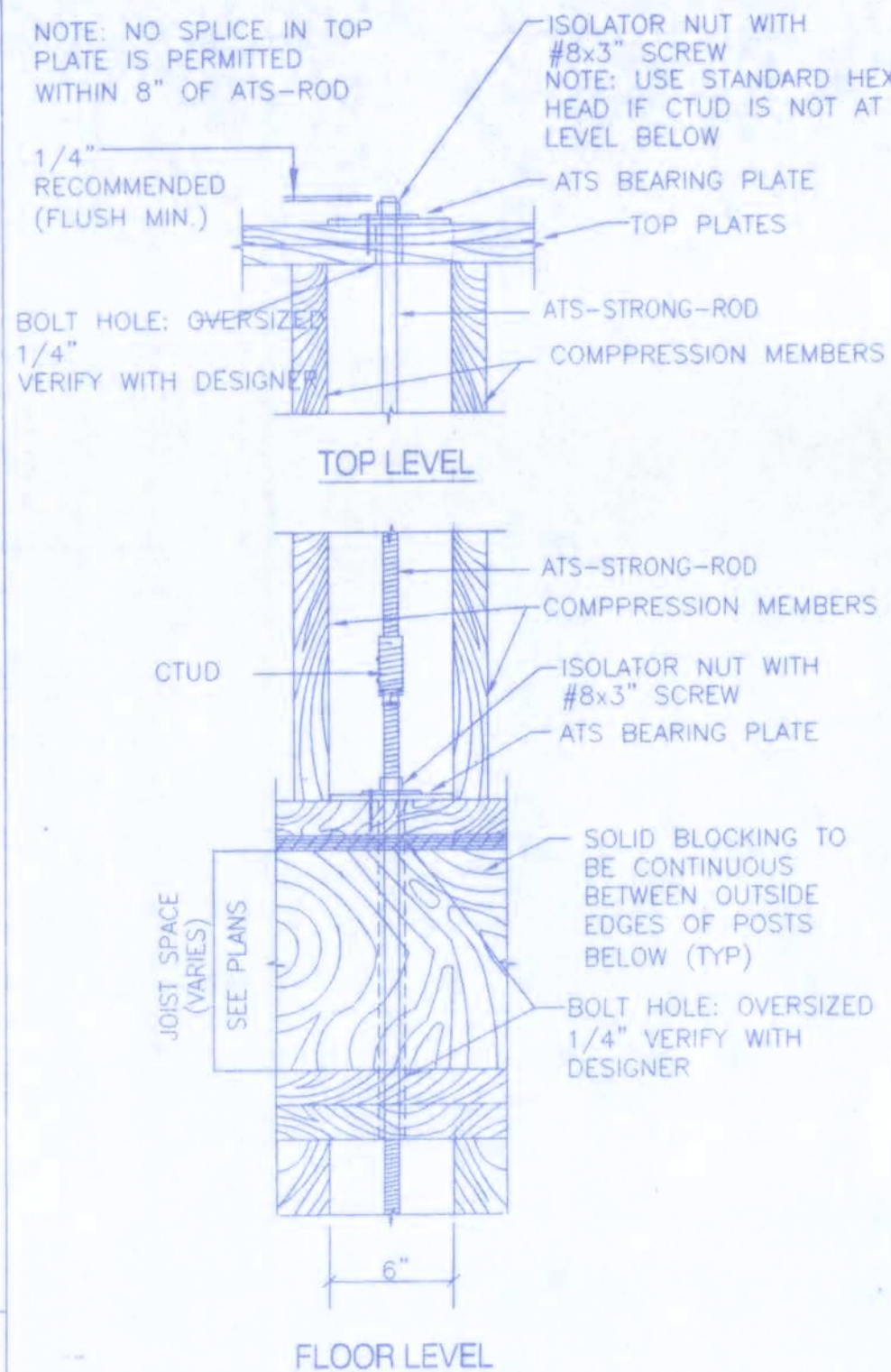
COUPLING TAKE UP DEVICE (CTUD)

STRONG-ROD MODEL NO.	ROD DIA. IN.	ALLOWABLE TENSILE CAPACITY			COLOR CODE
		AISC 9TH ED.		AISC 13TH ED.	
		P100	P133	P150	
SR5	5/8	6,075 LBS.	8,100 LBS.	6,340 LBS.	BLUE
SR7	7/8	11,905 LBS.	15,875 LBS.	12,815 LBS.	GREEN
SR9	1 1/8	19,660 LBS.	26,240 LBS.	21,205 LBS.	ORANGE
SR5H	5/8	12,150 LBS.	16,200 LBS.	13,570 LBS.	BLUE
SR7H	7/8	23,810 LBS.	31,745 LBS.	27,060 LBS.	GREEN
SR9H	1 1/8	39,365 LBS.	52,485 LBS.	44,730 LBS.	ORANGE
SR9H150	1 1/8	49,205 LBS.	65,605 LBS.	55,915 LBS.	ORANGE
SR10H150	1 1/4	60,745 LBS.	80,995 LBS.	69,030 LBS.	PURPLE

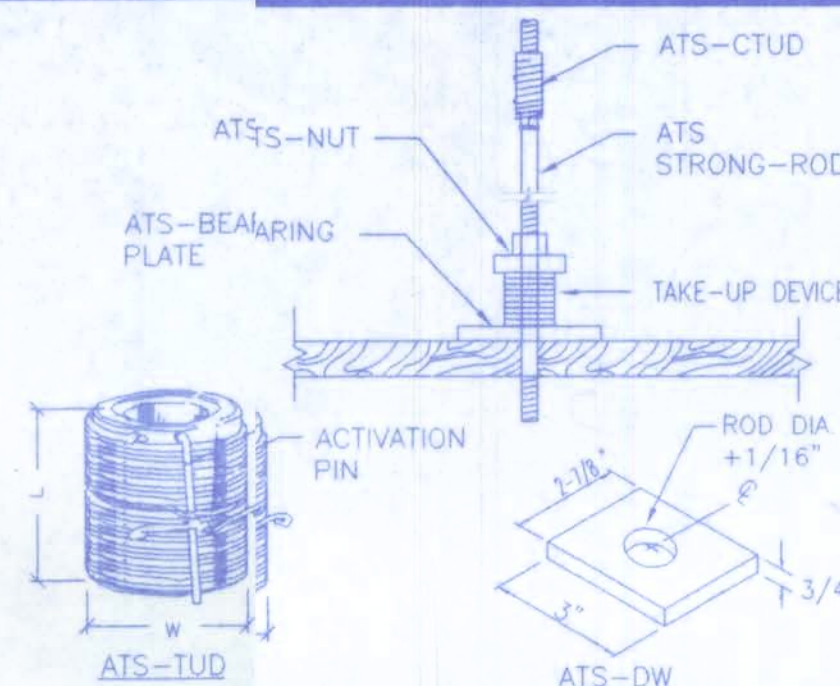
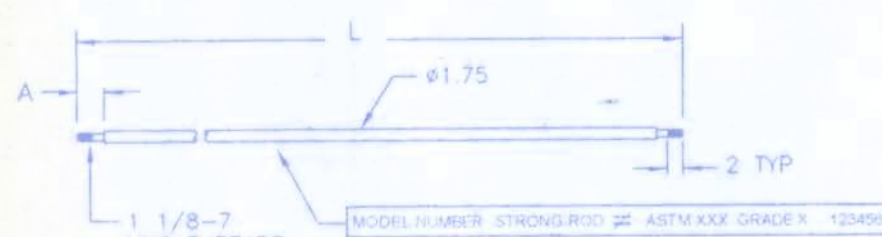


- STANDARD STRONG-ROD IS FABRICATED FROM ASTM A36 WITH F_y MIN=43ksi, F_u MIN=58ksi. HIGH STRENGTH STRONG-ROD IS FABRICATED FROM ASTM A193 GRADE B7 OR A449 WITH F_y MIN=92ksi, F_u MIN=120ksi. H150 STRONG-ROD IS FABRICATED FROM ASTM A354 CLASS B8 WITH F_y MIN=130ksi, F_u MIN=150ksi.
- STRONG-ROD IS LASER ETCHED WITH THE MODEL, ASTM STANDARD, AND LOT NUMBER.
- STRONG-ROD IS UNCOATED OR PLAIN BLACK STEEL.
- STRONG-ROD MODEL DESIGNATION: THE NUMBER FOLLOWING ATS-SR() REPRESENTS THE ROD DIAMETER IN 1/8" (i.e. ATS-SR5 IS A 5/8" DIAMETER ROD).
- STRONG-ROD XL ARE 1 3/4" DIA WITH 1 1/8" THREADED ENDS AND ARE USED TO REDUCE ELONGATION.

STRONG-ROD



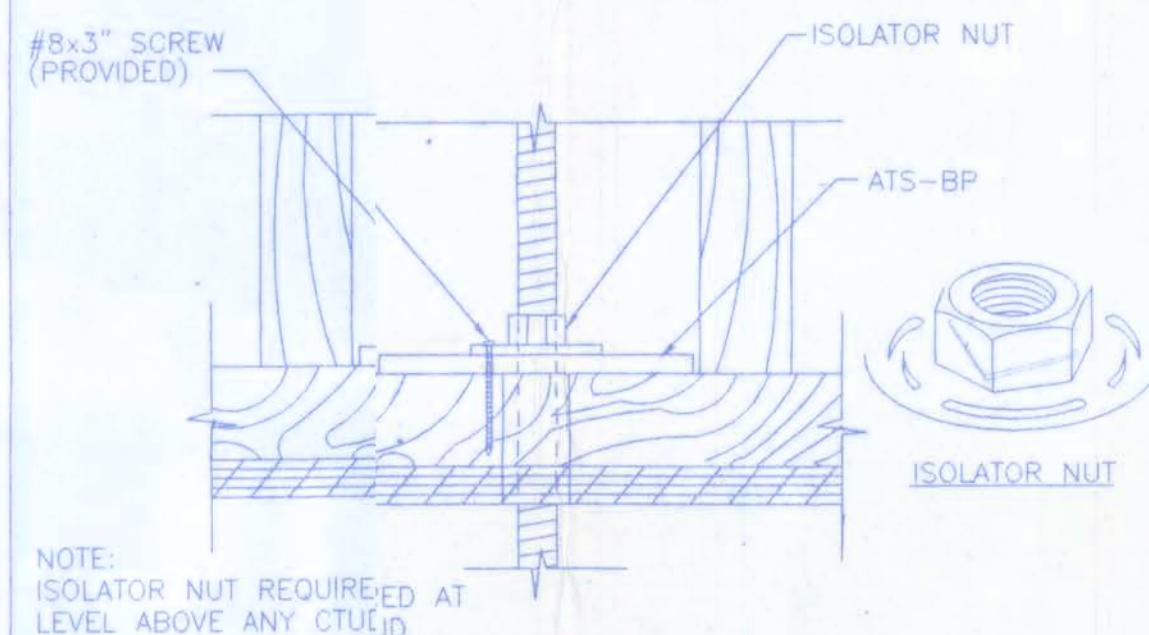
ATS INSTALLATION DETAILS



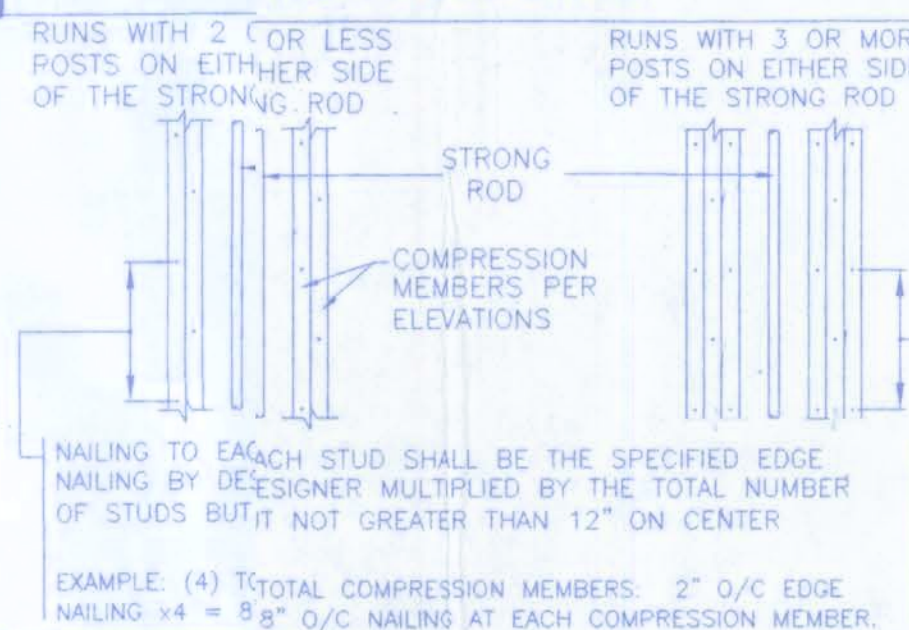
MODEL NO.	MAXIMUM THREADED ROD SIZE (IN)	W (IN)	L (IN)	ALLOWABLE CAPACITY (LBS)
TUD9	1 1/8	2 1/8	2 1/4	34,855
TUD10	1 1/4	2 3/8	2 1/4	45,400

- TAKE-UP DEVICES COMPENSATE FOR UP TO 1" OF SHRINKAGE OR SETTLEMENT DUE TO DEAD LOAD AND CONSTRUCTION LOADING.
 - ALLOWABLE CAPACITIES ARE BASED ON ULTIMATE LOADS DIVIDED BY A F.S. OF 3.
 - CAPACITIES DO NOT INCLUDE A 33% STEEL STRESS INCREASE. NO FURTHER INCREASE IS ALLOWED.
- ORDER OF INSTALLATION
- VERIFY WITH DESIGNER THAT CUMULATIVE SHRINKAGE IS LESS THAN 1".
 - PLACE TUD OVER ROD FROM LEVEL BELOW ONTO APPROPRIATE BEARING PLATE.
 - INSTALL PLATE WASHER, AND SECURE THE NUT ON TOP. FINGER TIGHTEN NUT PLUS AN ADDITIONAL 1/3 TO 1/2 TURN WITH WRENCH.
 - REMOVE WIRES AND ACTIVATION PIN.

INCREMENTAL SHRINKAGE DETAIL



ISOLATOR NUT DETAIL

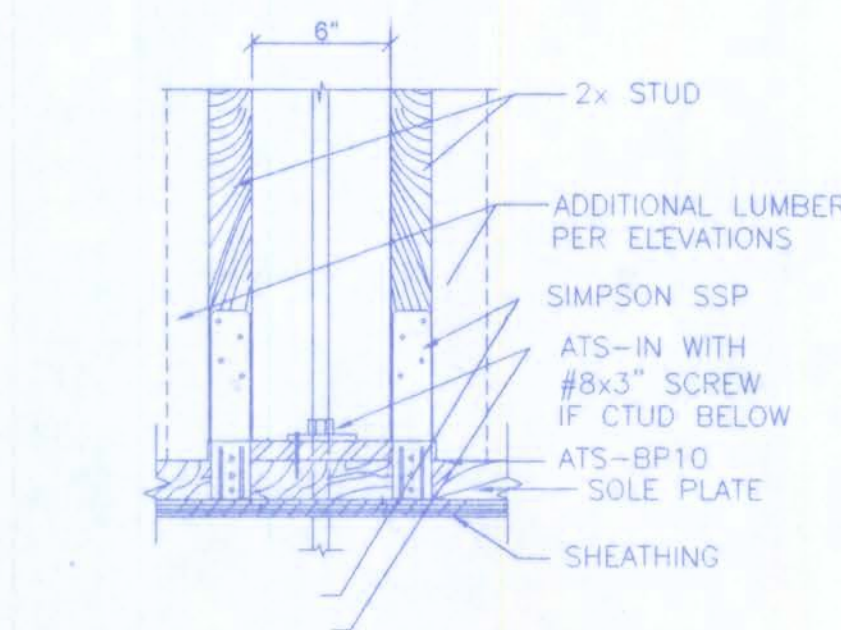


SHEARWALL EDGE NAILING

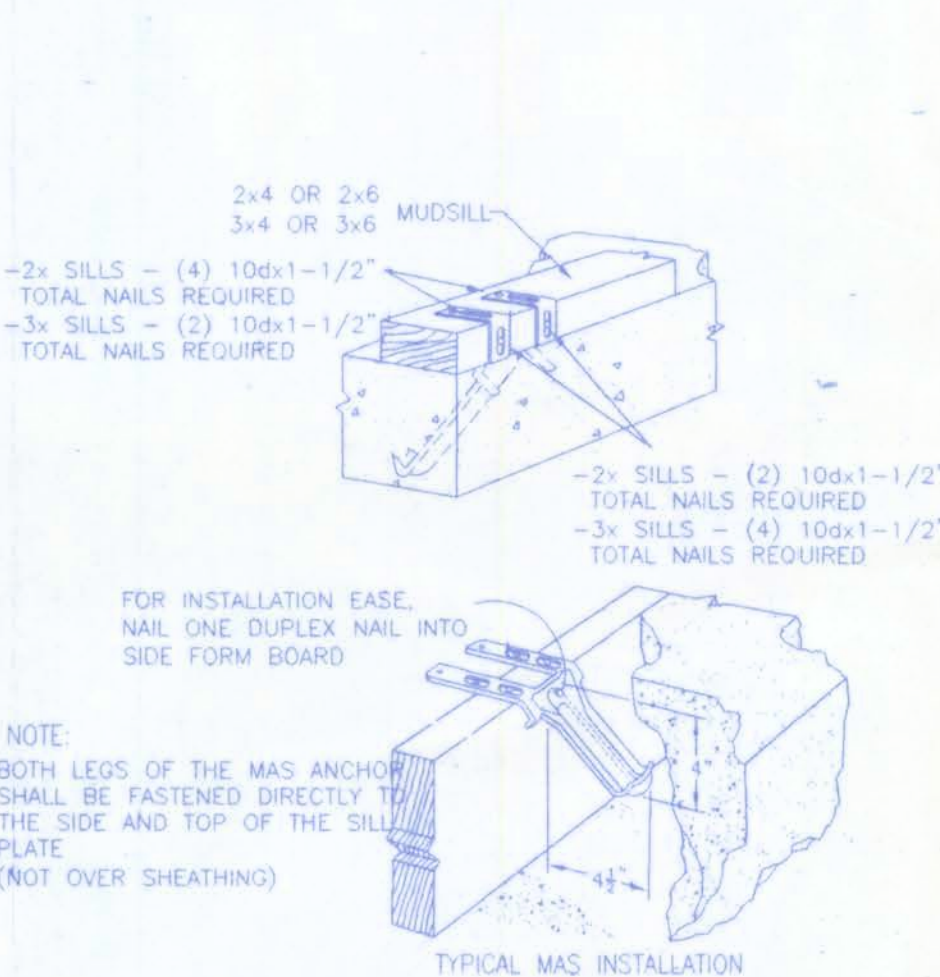
MODEL	ATS PLATE DIMENSIONS					ALLOWABLE BEARING CAPACITY				COLOR CODE	
	LENGTH (IN)	WIDTH (IN)	t (IN)	Dhole (IN)	Drod (IN)	DF (lb)	SP (lb)	HF (lb)	SPF (lb)		
						P100/P133	P100/P133	P100/P133	P100/P133		
ATS-BP5-3x3.5	3	1/2	3	3/8	11/16	5/8	6,775	6,125	4,390	4,605	BLUE
ATS-BP5-3x5.5	5.5	3	5/8	11/16	5/8	10,540	9,530	6,830	7,165	BLUE	
ATS-BP7-3x5.5	5.5	3	5/8	15/16	7/8	10,280	9,290	6,660	6,990	GREEN	
ATS-BP9-3x5.5	5.5	3	5/8	1 3/16	1 1/8	9,950	8,995	6,450	6,765	ORANGE	
ATS-BP5-5x5.5	5.5	5	3/4	11/16	5/8	17,885	16,165	11,590	12,160	BLUE	
ATS-BP7-5x5.5	5.5	5	5/8	15/16	7/8	17,620	15,930	11,420	11,985	GREEN	
ATS-BP9-5x5.5	5.5	5	5/8	1 3/16	1 1/8	17,295	15,635	11,205	11,760	ORANGE	
ATS-BP10-3x9	9	3	1	1 5/16	1 1/4	15,700	14,195	10,175	10,675	PURPLE	
ATS-BP10-3x12	12	3	1 1/2	1 5/16	1 1/4	21,325	19,280	13,820	14,500	PURPLE	
ATS-BP10-3x15	15	3	1 3/4	1 5/16	1 1/4	26,950	24,365	17,465	18,325	PURPLE	
ATS-BP10-5x9	9	5	1	1 5/16	1 1/4	26,950	24,365	17,465	18,325	PURPLE	
ATS-BP10-5x12	12	5	1 1/2	1 5/16	1 1/4	36,325	32,840	23,540	24,700	PURPLE	

BEARING PLATES

ALT. BRIDGE DOWN OPTION

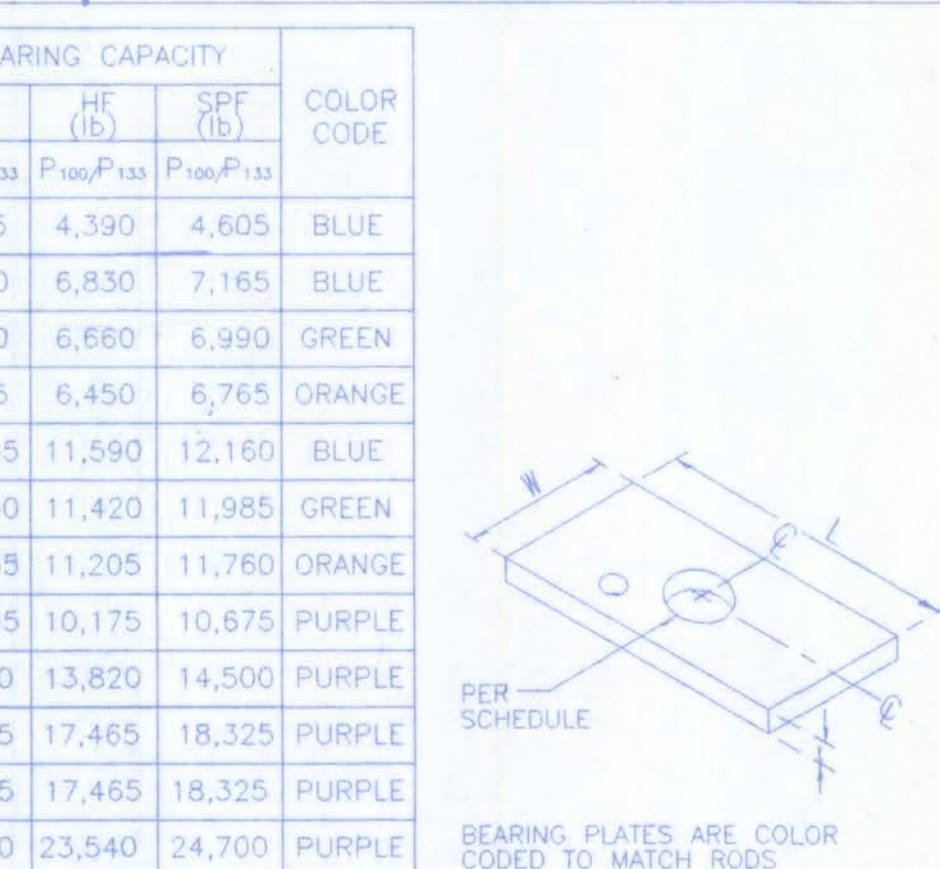


STUDS OVER BP10 PLATES

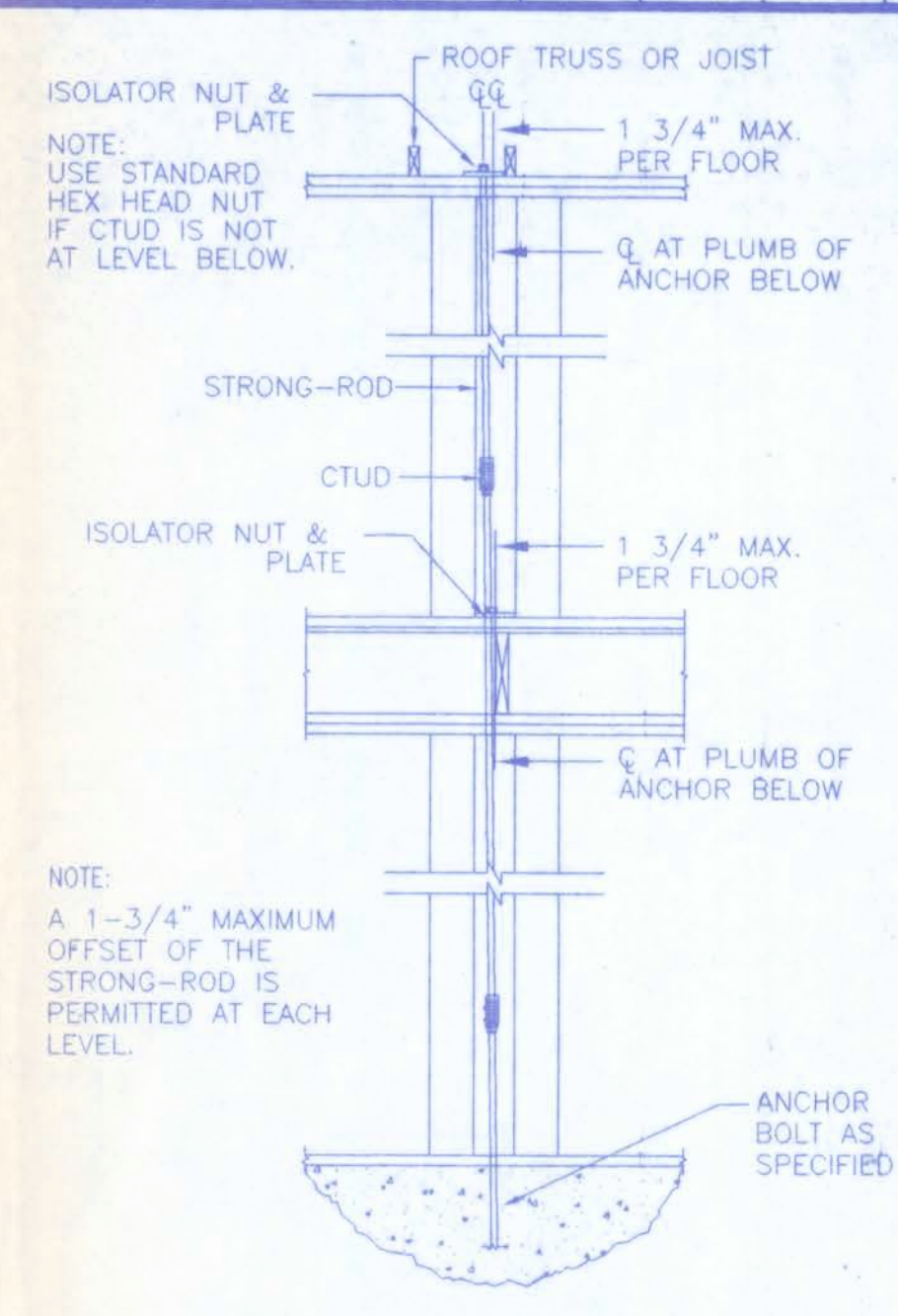


- NOTES:
- DESIGNER TO DETERMINE QUANTITY AND SPACING OF MAS CONNECTORS.
 - INSTALL SIMPSON MAS ANCHORS ON SHEATHING SIDE OF SILL PLATE ONLY.
 - 2,000 PSI MINIMUM CONCRETE STRENGTH REQUIRED.
 - 4" MINIMUM CONCRETE END DISTANCE.
 - 6" MINIMUM CENTER TO CENTER SPACING FOR SHEAR LOADING ONLY.
 - ALL OTHER MAS SPECIFICATIONS NOTED IN MOST CURRENT SIMPSON CATALOG SHALL BE FOLLOWED.

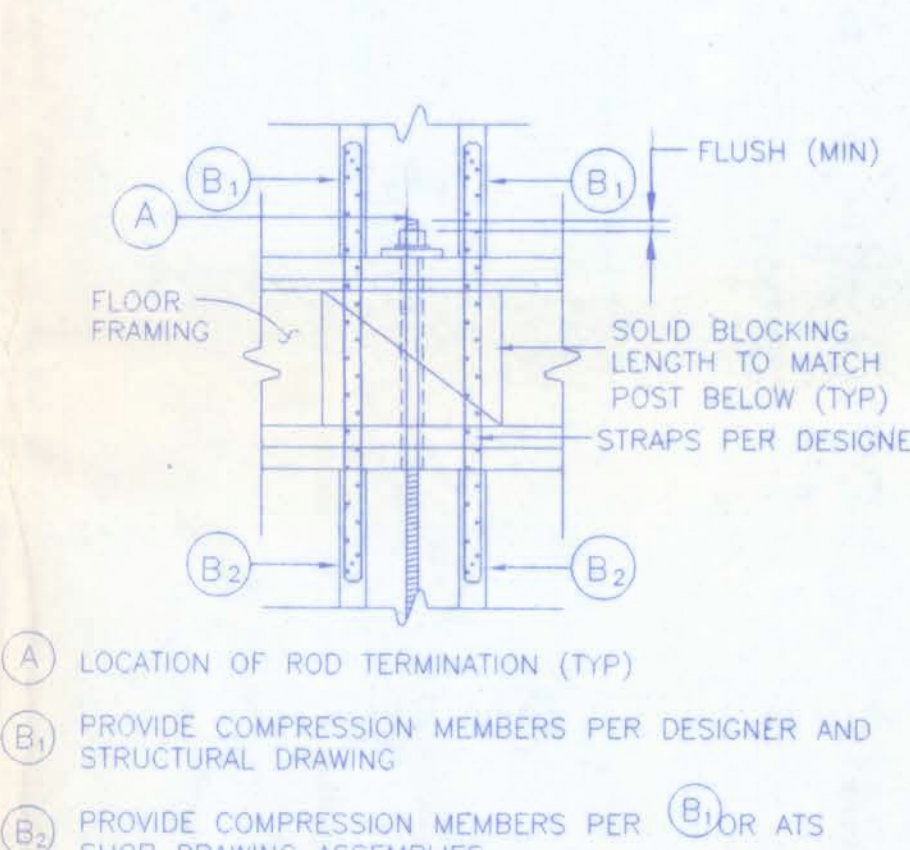
ALT. SHEAR ANCHOR BOLT



ANCHOR BOLT LAYOUT



ALLOWABLE ROD OFFSET

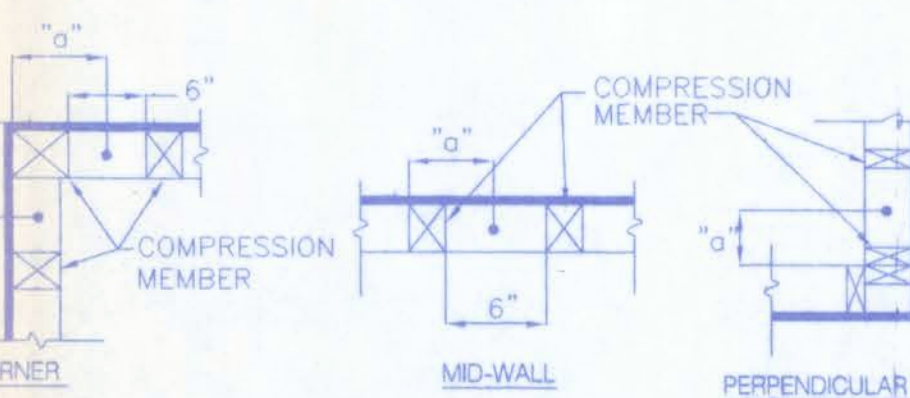


- NOTES:
- VERTICAL NAIL-ON STRAPS ARE SOLD SEPARATELY.
 - WHEN NAIL-ON STRAPS OCCUR IN COMBINATION WITH ATS, REFER TO THE STRUCTURAL DRAWINGS AND THE ATS INSTALLATION DETAILS.

ALT. STRAP TRANSITION

POST SIZE	"a" DISTANCE FROM BOLT TO OUTSIDE OF POST
2x4 or 2x6	4 1/2"
3x4 or 3x6	5 1/2"
2-2x4 or 2-2x6	6"
2-3x4 or 2-3x6	8"
4x6 or 6x6	8 1/2"
4x8 or 6x8	10 1/4"
4x10 or 6x10	12 1/4"

- NOTES:
- COMPRESSION MEMBERS DO NOT INCLUDE TRIMMERS.
 - WHEN WALL WIDTHS VARY FROM FLOOR TO FLOOR, COORDINATE ANCHOR BOLT LOCATION WITH NARROWEST WALL.



ANCHOR BOLT LAYOUT

REVISIONS	NO.	DATE

DESIGNER'S NOTE: THIS DATA TAKEN FROM PRODUCT SPECIFICATIONS BY SIMPSON STRONGTIE CO.

SIMPSON STRONG-TIE Co., Inc.
1515 South Main Street
Brea, CA 92621
800 International Street
Columbus, OH 43260
McKinney, TX 75069
PHONE: 800-999-5099
WEB SITE: www.strongtie.com

STRONG-TIE
ANCHOR TIEDOWN SYSTEM

GREENE RESIDENCE

THIS SHEET PROVIDES MANUF. INFO ONLY

NAME: N/A
DATE: N/A
SCALE: N/A
CHECKED BY: N/A
SHEET: N/A

ATS-1

7 OF 7 SHEETS
JOB NO. N/A