

7/12 - 3/12 PITCH
24" O/H

BEARING HEIGHT SCHEDULE

8' 1-1/8"

NOTES:

- 1) REFER TO HB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BEARINGS) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BEARINGS REQUIRED.
- 2) ALL TRUSSES INCLUDING TRUSSES UNDER VALLEY FRAMES MUST BE COMPLETED DECIDED OR REFER TO DETAIL YDS FOR ALTERNATE BEARING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TRUSSES ARE DESIGNED FOR 2 o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
- 5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE ASSUMED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
- 6) S/Y42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) ALL ROOF TRUSSES HANGERS TO BE SHAPESON HOOKS UNLESS OTHERWISE NOTED. ALL FLOOR TRUSSES HANGERS TO BE SHAPESON THRU422 UNLESS OTHERWISE NOTED.
- 8) BEAM/HEADLINE/INTEL (RDX) TO BE FURNISHED BY BUILDER.

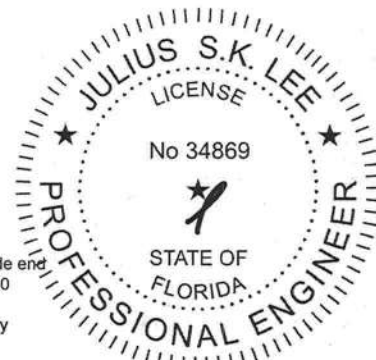
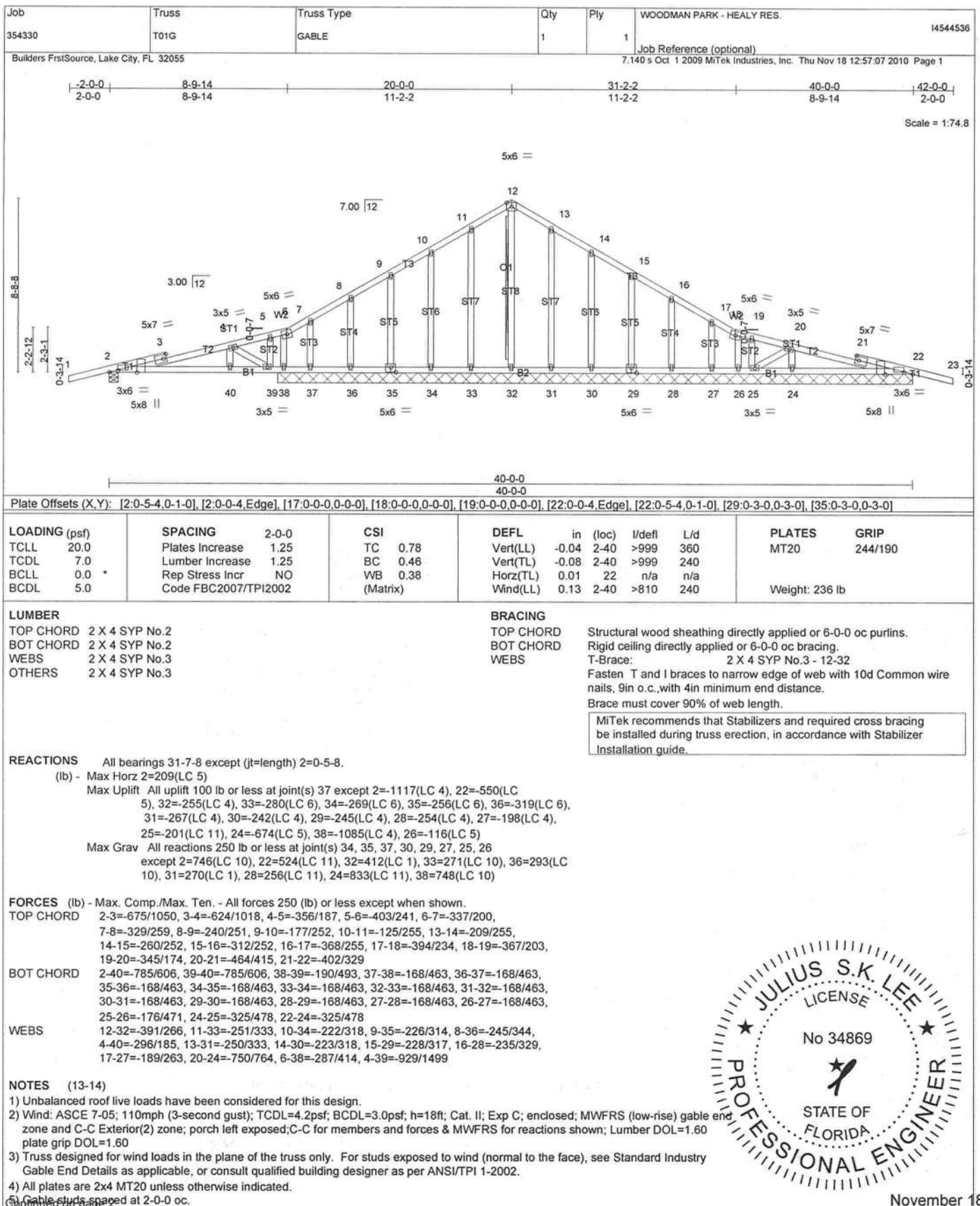
SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND WORKS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS, REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Approved by: _____ Date: _____



DESIGNER: WOODMAN PARK
CLIENT: HEALY RES.
DATE: 11-18-10
DRAWN BY: K.L.H.
SCALE: NTS
PROJECT: 354330



November 18, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - HEALY RES.	14544536
354330	T01G	GABLE	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

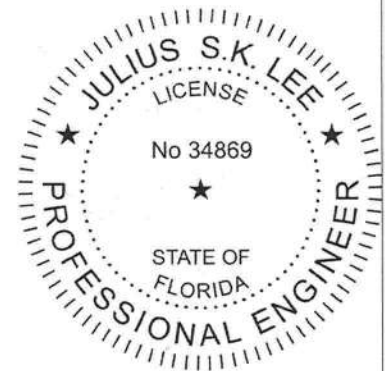
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NOTES (13-14)

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SYP No.2 .
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 37 except (jt=lb) 2=1117, 22=550, 32=255, 33=280, 34=269, 35=256, 36=319, 31=267, 30=242, 29=245, 28=254, 27=198, 25=201, 24=674, 38=1085, 26=116.
- 10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-114(F=-60), 6-12=-114(F=-60), 12-18=-114(F=-60), 18-23=-114(F=-60), 2-22=-10



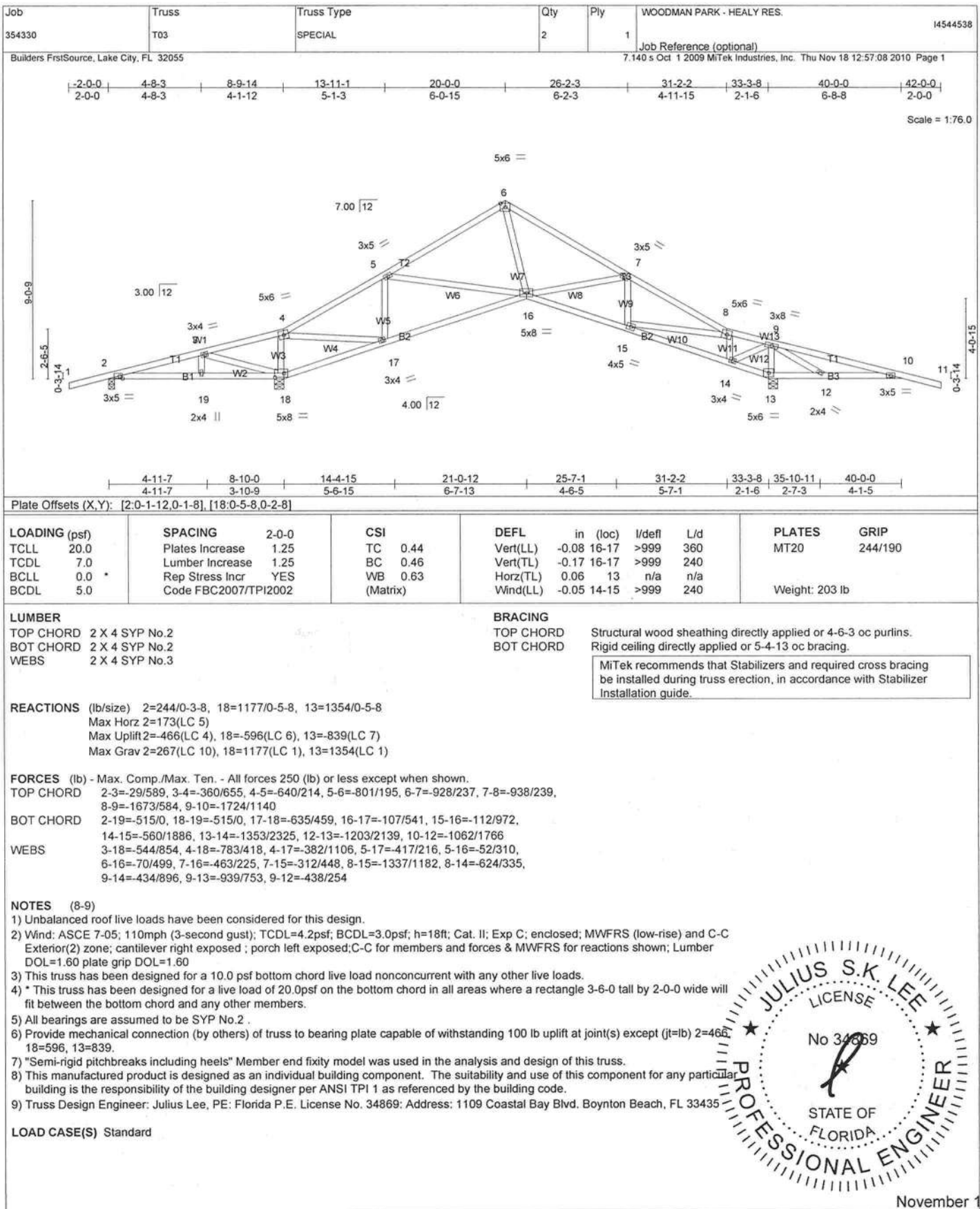
November 18, 2010



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Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - HEALY RES.
354330	T04G	GABLE	1	1	

I4544540

Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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-2-0-0	4-8-3	8-9-14	13-11-1	20-0-0	26-2-3	31-2-2	33-3-8	40-0-0	42-0-0
2-0-0	4-8-3	4-1-12	5-1-3	6-0-15	6-2-3	4-11-15	2-1-6	6-8-8	2-0-0

Scale = 1:76.0

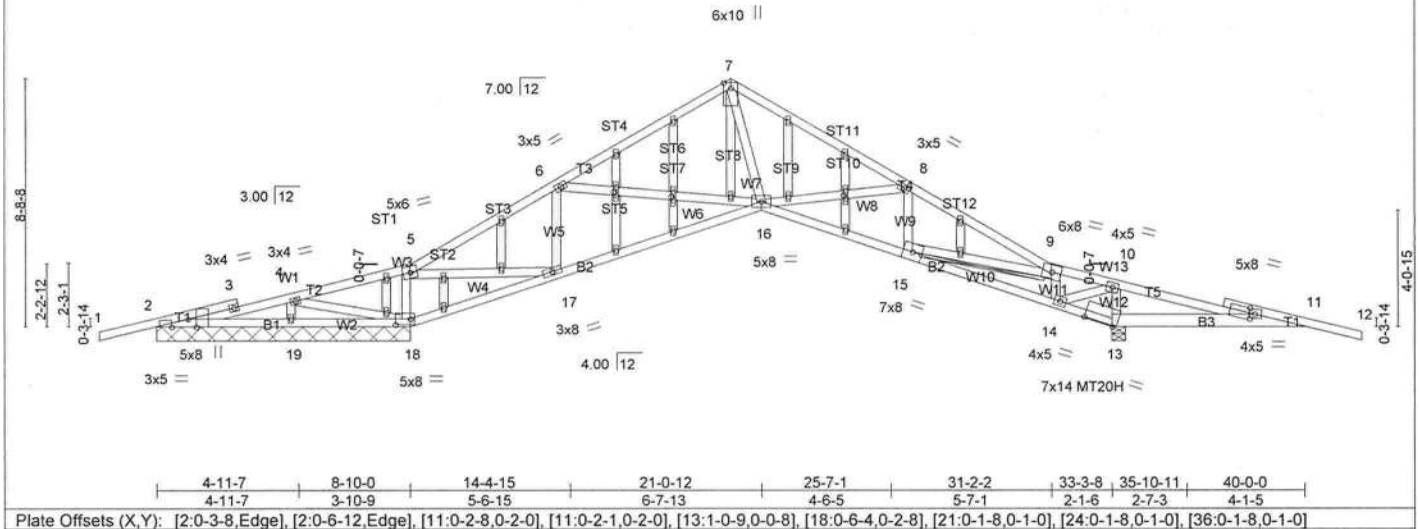


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-6-12,Edge], [11:0-2-8,0-2-0], [11:0-2-1,0-2-0], [13:1-0-9,0-0-8], [18:0-6-4,0-2-8], [21:0-1-8,0-1-0], [24:0-1-8,0-1-0], [36:0-1-8,0-1-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.89	Vert(LL)	-0.11 16-17	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.87	Vert(TL)	-0.25 16-17	>999	240	MT20H	187/143
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.90	Horz(TL)	0.13 13	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.63 11	>128	120		
								Weight: 236 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*
T5: 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B3: 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-3-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-2-12 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 9-15
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings 8-10-0 except (jt=length) 13=0-5-8.

- (lb) - Max Horz 2=208(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) except 2=560(LC 4), 18=1379(LC 6),
13=2122(LC 5), 19=378(LC 4)
Max Grav All reactions 250 lb or less at joint(s) 19 except 2=463(LC 10),
18=2134(LC 1), 13=2665(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-292/335, 3-4=-320/431, 4-5=-794/1259, 5-6=-1464/965, 6-7=-1698/960,
7-8=-1992/1143, 8-9=-2065/1192, 9-10=-3084/1396, 10-11=-3702/3059
BOT CHORD 2-19=-362/225, 18-19=-362/225, 17-18=-1161/698, 16-17=-746/1237, 15-16=-919/1790,
14-15=-1316/3332, 13-14=-3264/4027, 11-13=-2930/3705
WEBS 4-19=-164/412, 4-18=-824/530, 5-18=-1547/1061, 5-17=-1356/2259, 6-17=-805/571,
6-16=-314/60, 7-16=-559/1051, 8-16=-665/283, 8-15=-632/734, 9-15=-2853/2795,
9-14=-1222/781, 10-14=-1197/1964, 10-13=-1567/1088

NOTES (14-15)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- All plates are MT20 plates unless otherwise indicated.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 560 lb uplift at joint 2, 1379 lb uplift at joint 13, 2122 lb uplift at joint 13 and 378 lb uplift at joint 19.

Continued on Page 2



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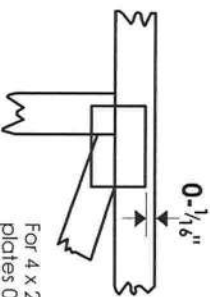
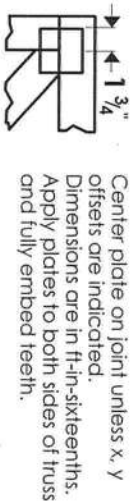
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Julius Lee
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Boynton, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



*Plate location details available in **Mitek 20/20** software or upon request.

PLATE SIZE

4 X 4

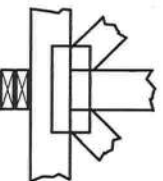
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

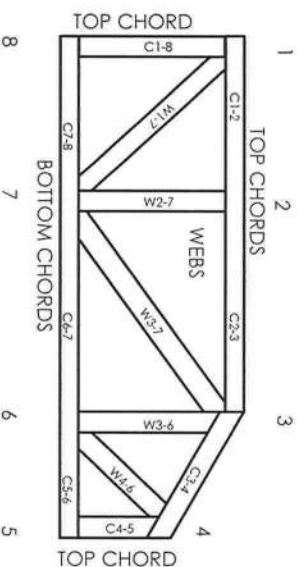


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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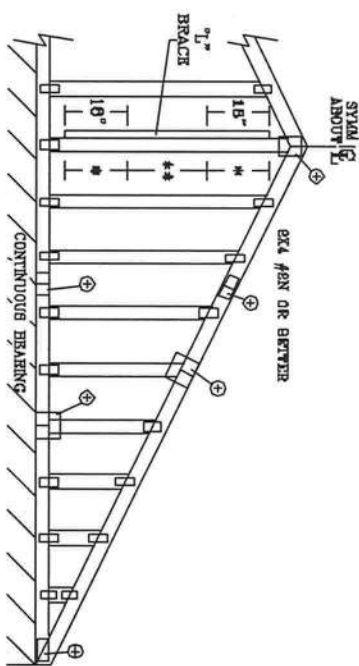
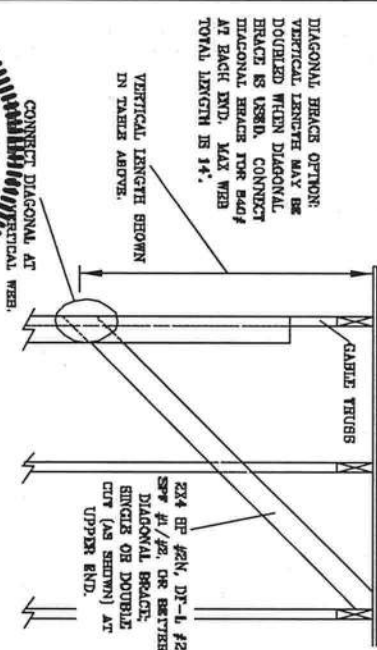
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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

MAX GABLE VERTICAL LENGTH														
GABLE VERTICAL SPACING	2x4 SPECIES	BRACE GRADE	NO BRACES	(1) 1x4 "L" BRACE *		(1) 2x4 "L" BRACE *		(2) 2x4 "L" BRACE **		(1) 2x6 "L" BRACE *		(2) 2x8 "L" BRACE *		
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	
24" O.C.	SPF HF	#1 / #2	3' 4"	5' 10"	6' 0"	6' 11"	7' 1"	6' 3"	6' 6"	10' 10"	11' 2"	12' 11"	13' 3"	
			#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	6' 3"	6' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	6' 3"	6' 3"	10' 0"	10' 0"	12' 11"	12' 11"	
			STANDARD	3' 3"	4' 2"	4' 2"	6' 6"	6' 6"	7' 5"	7' 5"	8' 3"	8' 3"	11' 6"	11' 6"
		#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"	
			#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
	SP DFL	#3	3' 6"	5' 0"	5' 0"	6' 7"	6' 8"	8' 3"	8' 6"	10' 3"	10' 3"	12' 11"	13' 7"	
			STUD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
		#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 6"	12' 8"	12' 8"	14' 0"	14' 0"	
			#3	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 6"	9' 6"	12' 4"	12' 4"	14' 0"	14' 0"	
			STANDARD	3' 8"	5' 2"	5' 2"	6' 10"	6' 10"	8' 2"	8' 2"	10' 7"	10' 7"	14' 0"	14' 0"
16" O.C.	SPF HF	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	
			#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 6"	10' 2"	12' 6"	13' 6"	14' 0"	14' 0"
		STUD	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"	
			STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
		SP DFL	#1 / #2	4' 3"	6' 11"	6' 11"	8' 8"	8' 11"	10' 6"	10' 6"	13' 8"	14' 0"	14' 0"	14' 0"
				#3	4' 2"	6' 11"	6' 11"	8' 8"	8' 8"	10' 5"	10' 5"	13' 6"	14' 0"	14' 0"
	STUD		4' 0"	6' 1"	6' 1"	7' 11"	8' 9"	10' 5"	10' 5"	13' 6"	14' 0"	14' 0"	14' 0"	
			STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
	SPF HF		#1	4' 3"	6' 11"	6' 11"	8' 8"	8' 11"	10' 6"	10' 6"	13' 8"	14' 0"	14' 0"	14' 0"
				#2	4' 2"	6' 11"	6' 11"	8' 8"	8' 8"	10' 5"	10' 5"	13' 6"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 9"	10' 5"	10' 5"	13' 6"	14' 0"	14' 0"	14' 0"	
			STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
SP DFL		#1	4' 3"	6' 11"	6' 11"	8' 8"	8' 11"	10' 6"	10' 6"	13' 8"	14' 0"	14' 0"	14' 0"	
			#2	4' 2"	6' 11"	6' 11"	8' 8"	8' 8"	10' 5"	10' 5"	13' 6"	14' 0"	14' 0"	14' 0"
	STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 9"	10' 5"	10' 5"	13' 6"	14' 0"	14' 0"	14' 0"		
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9'					



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:																											
GROUP A:		GROUP B:																									
SPURTEE-PINE-YR <table border="1"> <tr> <td>#1</td> <td>#2</td> <td>STANDARD</td> <td></td> </tr> <tr> <td>#3</td> <td></td> <td>STD</td> <td></td> </tr> </table>		#1	#2	STANDARD		#3		STD		HDM-FIR <table border="1"> <tr> <td>#1</td> <td>#2</td> <td>#3</td> <td>#4</td> </tr> <tr> <td>#1</td> <td>#2</td> <td>#3</td> <td>#4</td> </tr> </table>		#1	#2	#3	#4	#1	#2	#3	#4								
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SOUTHERN PINE <table border="1"> <tr> <td>#1</td> <td></td> <td></td> <td></td> </tr> <tr> <td>#2</td> <td></td> <td></td> <td></td> </tr> </table>		#1				#2				DOUGLAS FIR-LARCH <table border="1"> <tr> <td>#1</td> <td></td> <td></td> <td></td> </tr> <tr> <td>#2</td> <td></td> <td></td> <td></td> </tr> </table>		#1				#2											
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#2																											
#1																											
#2																											

CABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS $L/240$.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BRACING (6 PSF WC DEAD LOAD).

CABLE END SUPPORTS LOAD FROM 4' 0"

PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

FOR 2 L BRACKETS; DRIVE NAILS AT 3 O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

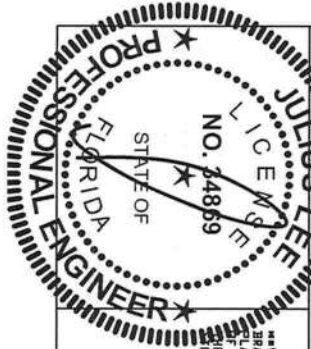
T. BRACING MUST BE A MINIMUM OF 80% OF WEB

EXPERIMENTAL PROCEDURE (see Table 1 for details)

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO BRIDGE
LESS THAN 4' 0"	1X1 OR B3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON THUS DESIGN FOR
PNAK, SPALUB, AND HEBEL PLATES.

PBAX, SPLICE, AND HIRL PLATES.



REVIEWED
By Julius Lee at 12:00 pm, Jun '11, 2008

****WARNING**** THESE ISSUES REQUIRING EXTENSIVE CARE FABRICATIONS, HANDLING, SHIPPING, INSTALLING AND BRACING, REFERS TO RESI-1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TP (TRUSS) PLATE INSTITUTE, 593 DUNCAN RD., SUITE 200, MARIETTA, VA 22767 AND VITA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, WADSWORTH, VT 56078) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE PRACTICES. UNLESS OTHERWISE INDICATED, TOP BOARD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.

1700 SW 7TH AVE/10B
DELRAY BEACH, FL 33444-2161

REF	ASCRT-02-CAB13015
DATE	11/26/03
DRWG	MTBK STD CABLE 15 E HT
-ENG	

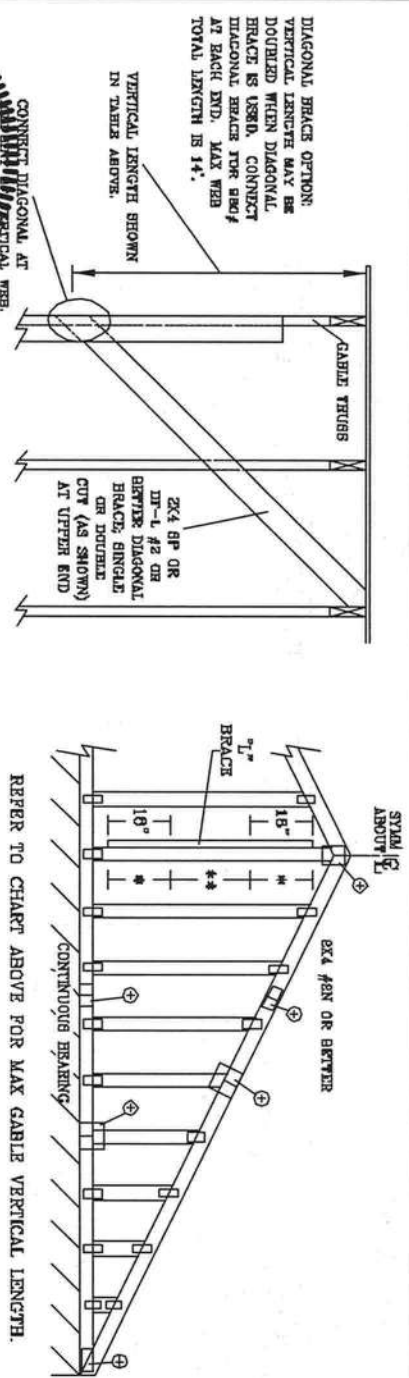
MAX. TOT. LD. 60 PSF

MAX SPACING 24.0"

No: 34869
STATE OF FLORIDA

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH																
GABLE VERTICAL SPACING SPECIES	BRACE GRADE	NO. BRACES	(1) 1X4 "L" BRACE * (1) 2X4 "L" BRACE * (2) 2X4 "L" BRACE ** (1) 2X6 "L" BRACE * (2) 2X6 "L" BRACE ** (2) 2X8 "L" BRACE **													
			GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B		
24" O.C.	SPF	#1 / #2	3' 2"	5' 6"	6' 8"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"			
			#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"		
	HF	STUD	3' 1"	4' 6"	4' 6"	5' 10"	6' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"			
			STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	5' 0"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"	12' 3"		
	SP	#1	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"			
			#2	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"		
	DFL	#3	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"			
			STUD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"		
	16" O.C.	SPF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 6"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"	14' 0"		
				#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"	
HF		STUD	3' 7"	5' 6"	5' 6"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"			
			STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"		
SP		#1	4' 0"	8' 4"	8' 4"	9' 8"	9' 8"	11' 9"	11' 9"	12' 8"	12' 8"	14' 0"	14' 0"			
			#2	3' 11"	8' 4"	8' 10"	7' 8"	7' 8"	8' 1"	8' 1"	12' 8"	12' 8"	14' 0"	14' 0"		
DFL		#3	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	8' 6"	11' 6"	11' 6"	14' 0"	14' 0"			
			STUD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	8' 6"	11' 6"	11' 6"	14' 0"	14' 0"		
12" O.C.		SPF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"		
				#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"	
	HF	STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"			
			STANDARD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"		
	SP	#1	4' 5"	8' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"			
			#2	4' 4"	8' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"		
	DFL	#3	4' 2"	6' 4"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"			
			STUD	4' 2"	6' 4"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"		
		STANDARD		4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	8' 9"	8' 9"	11' 4"	11' 4"	14' 0"	14' 0"		



CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DEFLECTION CRITERIA IS $L/240$.	
PROVIDE UPRAIT CONNECTIONS FOR 180 PSF OVER CONTINUOUS BRACING (6 PSF RC DEAD LOAD).	
CABLE END SUPPORTS LOAD FROM 4' 0" OUTLEAKERS WITH 8' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.	
ATTACH EACH "L" BRACE WITH 104 NAILS.	
* FOR (1) "L" BRACE, BRACE NAILS AT 8" O.C. IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.	
** FOR (2) "L" BRACES, BRACE NAILS AT 3" O.C. IN 16" END ZONES AND 8" O.C. BETWEEN ZONES.	
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.	

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO BRACE
LESS THAN 4' 0"	1X4 OR 2X4
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

BRACING GROUP SPECIES AND GRADES:	
GROUP A:	
SPRUCES-PINE-LARCH	HEM-FIR
#1 / #2 STUD	#2 STUD
#3 STUD	#3 STANDARD
GROUP B:	
DOUGLASS FIR-LARCH	SOUTHERN PINE
#1 STUD	#2 STUD
#3 STUD	STANDARD

MAX. TOT. LD. 60 PSF	
MAX. SPACING 24.0"	

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LIVE LOAD DEFLECTION CRITERIA IS $L/240$.	
PROVIDE UPRAIT CONNECTIONS FOR 180 PSF OVER CONTINUOUS BRACING (6 PSF RC DEAD LOAD).	
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DOUGLASS FIR-LARCH	SOUTHERN PINE
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MAX. TOT. LD. 60 PSF	
MAX. SPACING 24.0"	

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#3 STUD	#3 STANDARD
GROUP B:	
DOUGLASS FIR-LARCH	SOUTHERN PINE
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MAX. TOT. LD. 60 PSF	
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#3 STUD	#3 STANDARD
GROUP B:	
DOUGLASS FIR-LARCH	SOUTHERN PINE
#1 STUD	#2 STUD
#3 STUD	STANDARD

MAX. TOT. LD. 60 PSF	
MAX. SPACING 24.0"	

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CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO BRACE
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BRACING GROUP SPECIES AND GRADES:	
GROUP A:	
SPRUCES-PINE-LARCH	HEM-FIR
#1 / #2 STUD	#2 STUD
#3 STUD	#3 STANDARD
GROUP B:	
DOUGLASS FIR-LARCH	SOUTHERN PINE
#1 STUD	#2 STUD
#3 STUD	STANDARD

MAX. TOT. LD. 60 PSF	
MAX. SPACING 24.0"	

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CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO BRACE
LESS THAN 4' 0"	1X4 OR 2X4
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPICES
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2X6X

+ REFER TO COLUMN THOSE DESIGN FOR
FRAX, SPULCR, AND BEAL DESIGN FOR

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OR SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

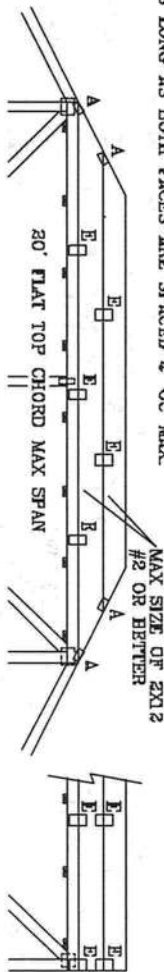
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, ENG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF

WIND TC DL=5 PSF, WIND BC DL=5 PSF

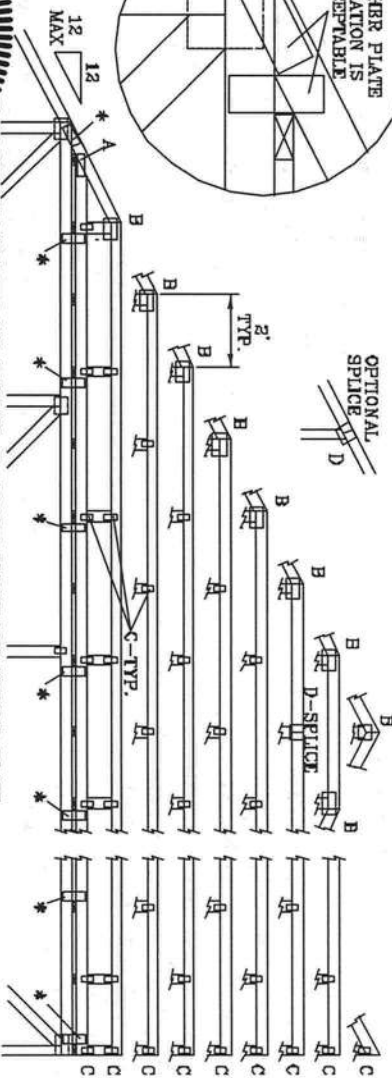
FRONT FACE (B,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF



EITHER PLATE LOCATION IS ACCEPTABLE

OPTIONAL SPLICE



THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045

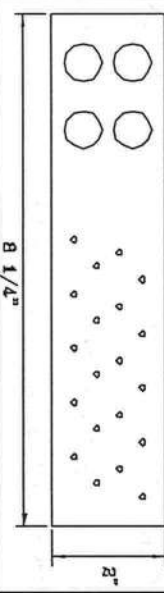
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.5X4	2.5X4	3X6
B	4X6	6X6	6X6	6X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	6X5	6X5	6X6
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY			

ATTACH TRUSS PLATES WITH (6) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLT. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB LENGTH	WEB BRACING CHART
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLT. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACED 4' OC OR LESS.



NOTES: TRUSSES BECOME EXPOSED DUE TO FABRICATING, HANDLING, SHIPPING, INSTALLING AND MAINTENANCE. REFER TO SEALING AND GROUTING COMPONENT SAFETY REPAIRATION, AND ISSUED BY THE TRUSS MANUFACTURER. SEE THE TRUSS MANUFACTURER'S INSTRUCTIONS FOR SAFETY PRACTICES PRIOR TO PERFORMING ANY MAINTENANCE. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 SW 4TH AVENUE
DIXIEWAY BEACH, FL 33444-2161

MAX LOADING

55 PSF AT

1.33 DUR. FAC.

50 PSF AT

1.25 DUR. FAC.

47 PSF AT

1.15 DUR. FAC.

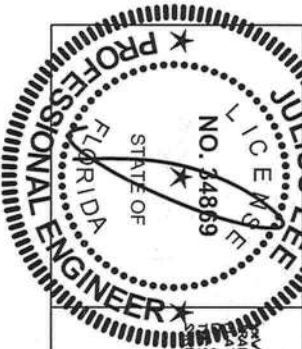
SPACING 24.0"

REF PIGGYBACK

DATE 09/12/07

DRWG/MIITEK STD PIGGY

-ENG JL



REVIEWED

By Julius Lee at 11:59 am, Jun 11, 2008

No. 34886
STATE OF FLORIDA

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

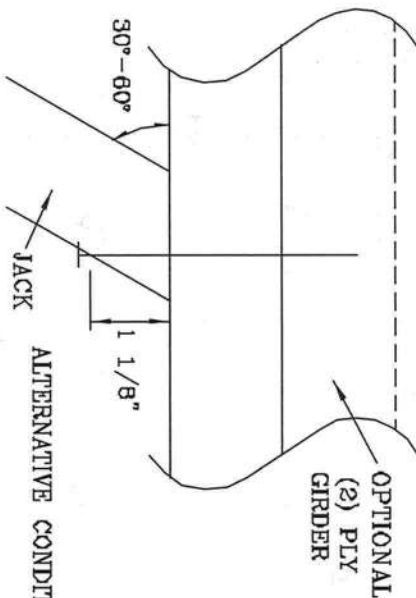
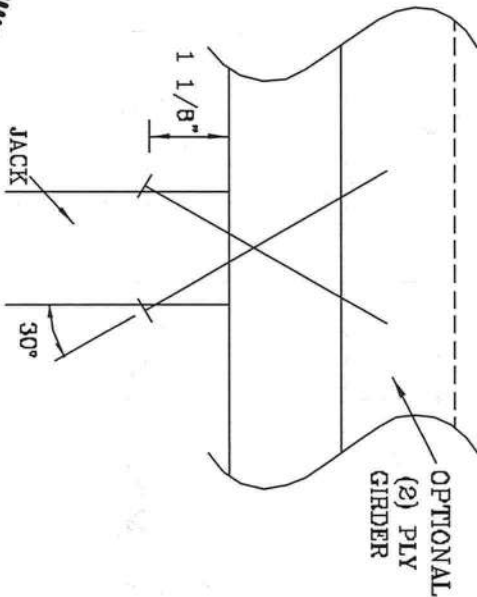
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

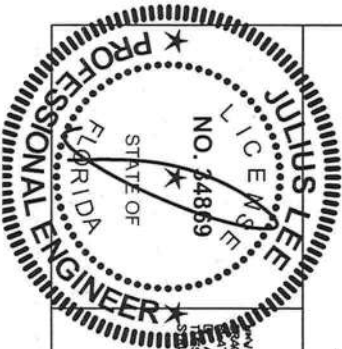
NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES
2	197#	256#	181#	234#	156#	203#	154#	189#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-43 GRADING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURER, 283 PONDVIEW DR., SUITE 200, HADLEY, MA 01039 AND/OR CONTACT THE TRUSS MANUFACTURER FOR MORE INFORMATION. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 374th AVE
DELMAR BEACH, FL 33441-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		

DUR. FAC. 1.00
SPACING

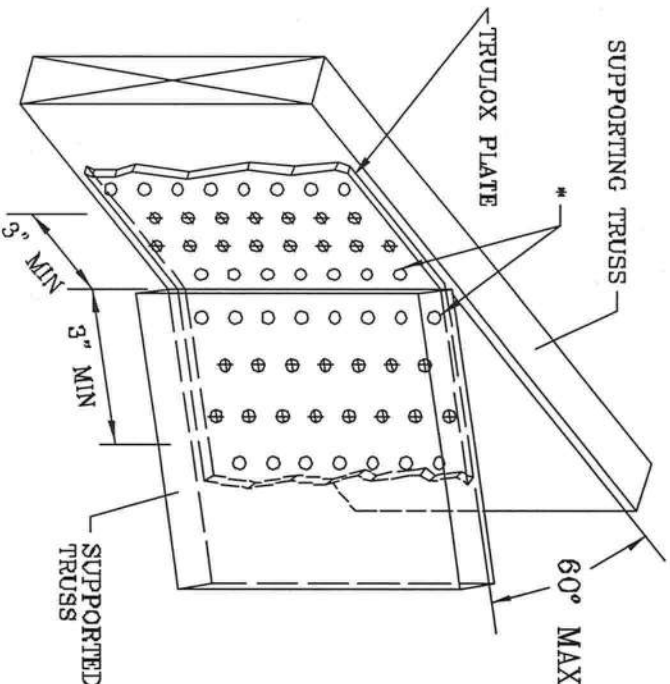
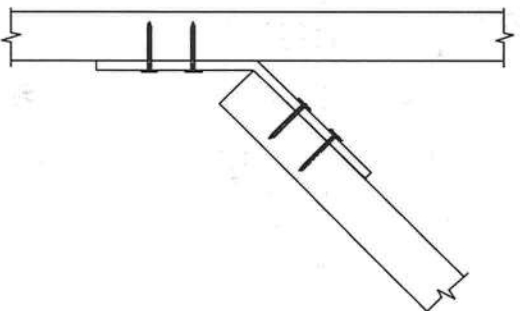
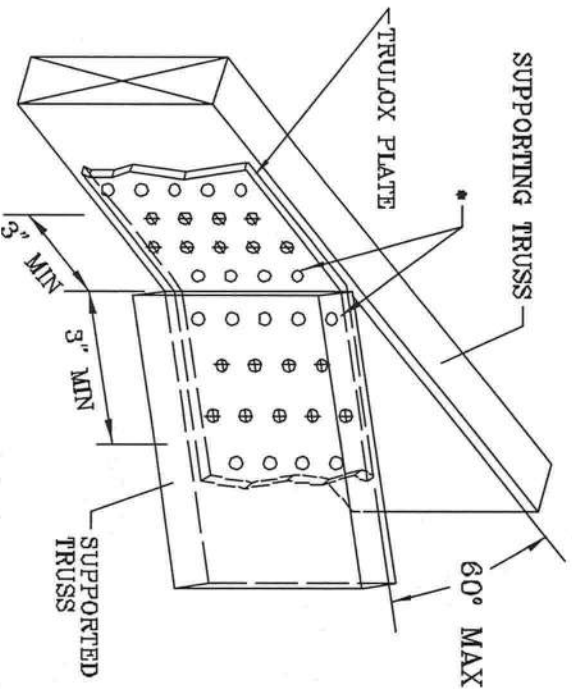
TRULOX CONNECTION DETAIL

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.
REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



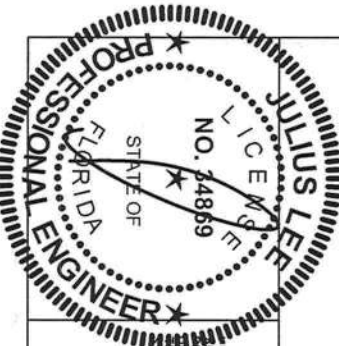
MINIMUM 3X6 TRULOX PLATE

MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350 #
6X6	16	990 #

REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008

THIS DRAWING REPLACES DRAWINGS 1,168,869 1,158,989/R
1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO AISC 1-43 (BUILDING EXPERTISE SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 300 PONDVIEW DR., SUITE 800, WOODBRIDGE, VA 22191) AND AISC 400 TRUSS CONSTRUCTION. TRUSSES SHOULD BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE AISC 400 TRUSS CONSTRUCTION. TRUSSES SHOULD BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE AISC 400 TRUSS CONSTRUCTION. TRUSSES SHOULD BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE AISC 400 TRUSS CONSTRUCTION.




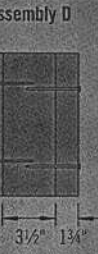

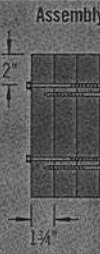
JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DEALY BEACH, FL 33444-2101

No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/28/03
DRWG	CNTRULOX1103
-ENG	JL

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

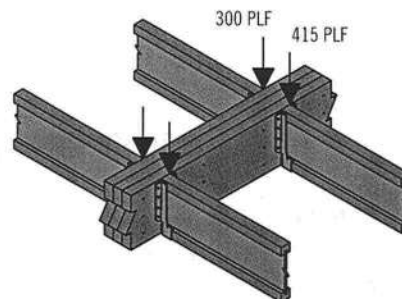
Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
								
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/8" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

- (1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.
- (2) Washers required. Bolt holes to be 1/16" maximum.
- (3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

Builders FrstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Nov 18 12:57:07 2010 Page 1

Scale = 1:74.8

The structural drawing shows a gabled roof truss system. The main truss members are labeled with numbers 1 through 23. The top chord consists of members 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, and 23. The bottom chord consists of members 31 through 40. Vertical web members connect the top and bottom chords. Plates are indicated by labels like 5x6, 3x5, 3x6, 5x8, etc. Bracing includes top and bottom chords, webs, and other members. Reactions are shown at the supports.

Plate Offsets (X,Y): [2:0-5-4,0-1-0], [2:0-0-4,Edge], [17:0-0-0,0-0-0], [18:0-0-0,0-0-0], [19:0-0-0,0-0-0], [22:0-0-4,Edge], [22:0-5-4,0-1-0], [29:0-3-0,0-3-0], [35:0-3-0,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0 1.25	TC 0.78	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.46	Vert(LL) -0.04 2-40 >999 360		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.38	Vert(TL) -0.08 2-40 >999 240		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) 0.01 22 n/a n/a		
			Wind(LL) 0.13 2-40 >810 240		
				Weight: 236 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 12-32
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTék recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings 31-7-8 except (jt=length) 2=0-5-8.
(lb) - Max Horz 2=209(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) 37 except 2=-1117(LC 4), 22=-550(LC 5), 32=-255(LC 4), 33=-280(LC 6), 34=-269(LC 6), 35=-256(LC 6), 36=-319(LC 6), 31=-267(LC 4), 30=-242(LC 4), 29=-245(LC 4), 28=-254(LC 4), 27=-198(LC 4), 25=-201(LC 11), 24=-674(LC 5), 38=-1085(LC 4), 26=-116(LC 5)
Max Grav All reactions 250 lb or less at joint(s) 34, 35, 37, 30, 29, 27, 25, 26 except 2=746(LC 10), 22=524(LC 11), 32=412(LC 1), 33=271(LC 10), 36=293(LC 10), 31=270(LC 1), 28=256(LC 11), 24=833(LC 11), 38=748(LC 10)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-675/1050, 3-4=-624/1018, 4-5=-356/187, 5-6=-403/241, 6-7=-337/200, 7-8=-329/259, 8-9=-240/251, 9-10=-177/252, 10-11=-125/255, 13-14=-209/255, 14-15=-260/252, 15-16=-312/252, 16-17=-368/255, 17-18=-394/234, 18-19=-367/203, 19-20=-345/174, 20-21=-464/415, 21-22=-402/329
2-40=-785/606, 39-40=-785/606, 38-39=-190/493, 37-38=-168/463, 36-37=-168/463, 35-36=-168/463, 34-35=-168/463, 33-34=-168/463, 32-33=-168/463, 31-32=-168/463, 30-31=-168/463, 29-30=-168/463, 28-29=-168/463, 27-28=-168/463, 26-27=-168/463, 25-26=-176/471, 24-25=-325/478, 22-24=-325/478
12-32=-391/266, 11-33=-251/333, 10-34=-222/318, 9-35=-226/314, 8-36=-245/344, 4-40=-296/185, 13-31=-250/333, 14-30=-223/318, 15-29=-228/317, 16-28=-235/329, 17-27=-189/263, 20-24=-750/764, 6-38=-287/414, 4-39=-929/1499

NOTES (13-14)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Cable studs spaced at 2-0-0 oc.

JULIUS S.K. LEE
LICENSE
No 34869
STATE OF FLORIDA
PROFESSIONAL ENGINEER

November



November 18, 2010

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Application of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - HEALY RES.	i4544536
354330	T01G	GABLE	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

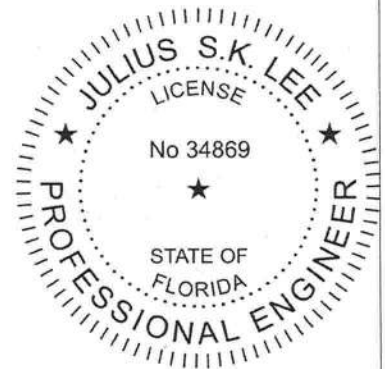
7.140 s Oct 1 2009 Mitek Industries, Inc. Thu Nov 18 12:57:07 2010 Page 2

NOTES (13-14)

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SYP No.2
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 37 except (jt=lb) 2=1117, 22=550, 32=255, 33=280, 34=269, 35=256, 36=319, 31=267, 30=242, 29=245, 28=254, 27=198, 25=201, 24=674, 38=1085, 26=116.
- 10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-114(F=-60), 6-12=-114(F=-60), 12-18=-114(F=-60), 18-23=-114(F=-60), 2-22=-10



November 18, 2010



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - HEALY RES.	14544538
354330	T03	SPECIAL	2	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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1-2-0-0	4-8-3	8-9-14	13-11-1	20-0-0	26-2-3	31-2-2	33-3-8	40-0-0	42-0-0
2-0-0	4-8-3	4-1-12	5-1-3	6-0-15	6-2-3	4-11-15	2-1-6	6-8-8	2-0-0

Scale = 1:76.0

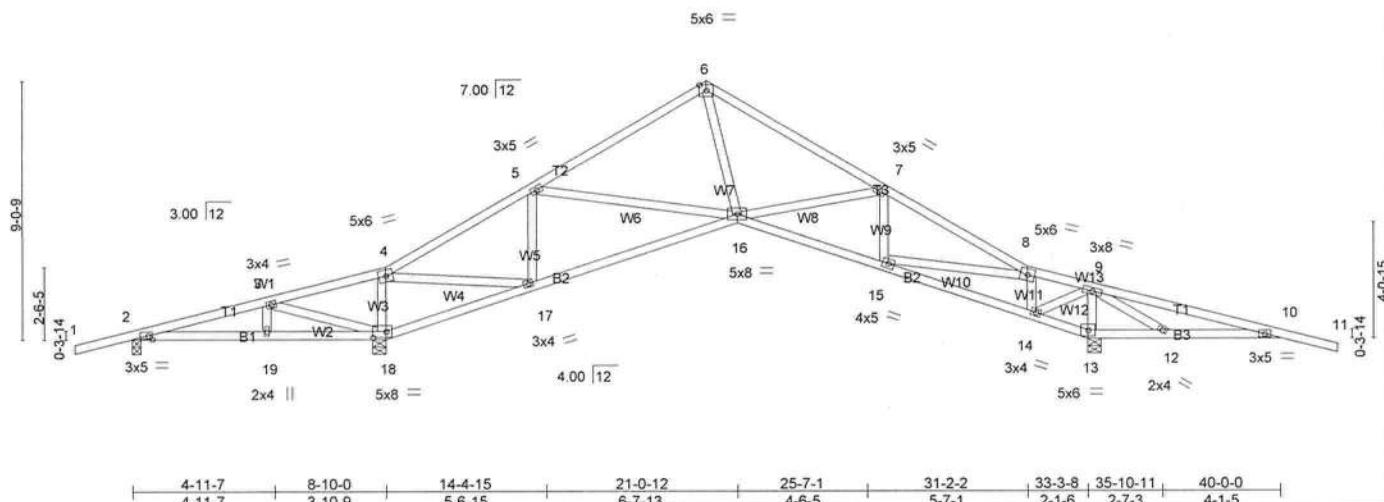


Plate Offsets (X,Y): [2:0-1-12,0-1-8], [18:0-5-8,0-2-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.44	Vert(LL)	-0.08 16-17	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.46	Vert(TL)	-0.17 16-17	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.63	Horz(TL)	0.06 13	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	-0.05 14-15	>999	240		
								Weight: 203 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 4-6-3 oc purlins.
Rigid ceiling directly applied or 5-4-13 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=244/0-3-8, 18=1177/0-5-8, 13=1354/0-5-8
Max Horz 2=173(LC 5)
Max Uplift 2=466(LC 4), 18=596(LC 6), 13=839(LC 7)
Max Grav 2=267(LC 10), 18=1177(LC 1), 13=1354(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-29/589, 3-4=-360/655, 4-5=-640/214, 5-6=-801/195, 6-7=-928/237, 7-8=-938/239, 8-9=-1673/584, 9-10=-1724/1140
BOT CHORD 2-19=-515/0, 18-19=-515/0, 17-18=-635/459, 16-17=-107/541, 15-16=-112/972, 14-15=-560/1886, 13-14=-1353/2325, 12-13=-1203/2139, 10-12=-1062/1766
WEBS 3-18=-544/854, 4-18=-783/418, 4-17=-382/1106, 5-17=-417/216, 5-16=-52/310, 6-16=-70/499, 7-16=-463/225, 7-15=-312/448, 8-15=-1337/1182, 8-14=-624/335, 9-14=-434/896, 9-13=-939/753, 9-12=-438/254

NOTES (8-9)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=466, 18=596, 13=839.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



November 18, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

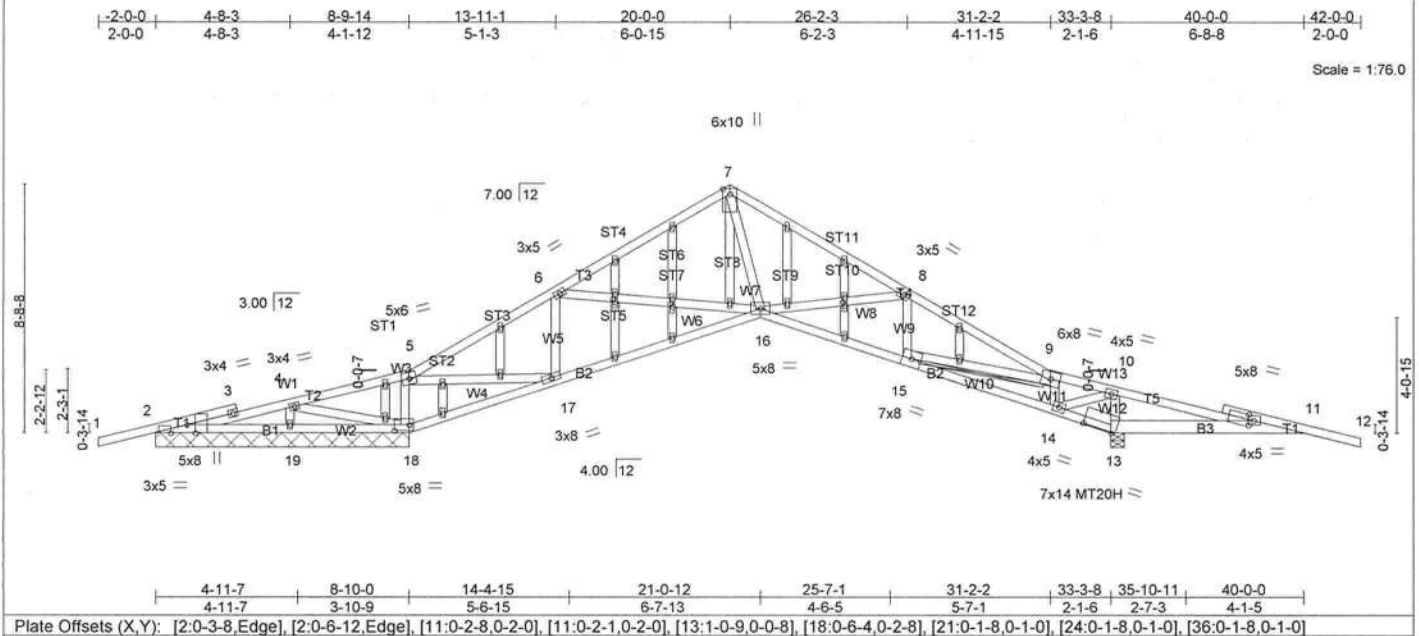
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and 8CSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - HEALY RES.	I4544540
354330	T04G	GABLE	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.89	Vert(LL) -0.11	16-17	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.87	Vert(TL) -0.25	16-17	>999	240	MT20H	187/143
BCLL 0.0 *	Rep Stress Incr NO	WB 0.90	Horz(TL) 0.13	13	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.63	11	>128	120		
							Weight: 236 lb	

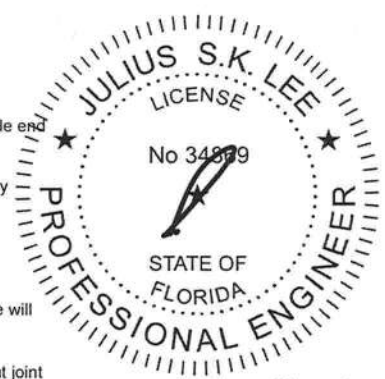
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2 *Except* T5: 2 X 4 SYP No.1D	TOP CHORD Structural wood sheathing directly applied or 3-3-7 oc purlins. BOT CHORD Rigid ceiling directly applied or 3-2-12 oc bracing.
BOT CHORD 2 X 4 SYP No.2 *Except* B3: 2 X 6 SYP No.1D	WEBS T-Brace: 2 X 4 SYP No.3 - 9-15
WEBS 2 X 4 SYP No.3	Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
OTHERS 2 X 4 SYP No.3	Brace must cover 90% of web length.
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS
All bearings 8-10-0 except (jt=length) 13=0-5-8.
(lb) - Max Horz 2=208(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) except 2=-560(LC 4), 18=-1379(LC 6), 13=-2122(LC 5), 19=-378(LC 4)
Max Grav All reactions 250 lb or less at joint(s) 19 except 2=463(LC 10), 18=2134(LC 1), 13=2665(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-292/335, 3-4=-320/431, 4-5=-794/1259, 5-6=-1464/965, 6-7=-1698/960, 7-8=-1992/1143, 8-9=-2065/1192, 9-10=-3084/1396, 10-11=-3702/3059
BOT CHORD 2-19=-362/225, 18-19=-362/225, 17-18=-1161/698, 16-17=-746/1237, 15-16=-919/1790, 14-15=-1316/3332, 13-14=-3264/4027, 11-13=-2930/3705
WEBS 4-19=-164/412, 4-18=-824/530, 5-18=-1547/1061, 5-17=-1356/2259, 6-17=-805/571, 6-16=-31/460, 7-16=-559/1051, 8-16=-665/283, 8-15=-632/734, 9-15=-2853/2795, 9-14=-1222/781, 10-14=-1197/1964, 10-13=-1567/1088

- NOTES (14-15)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
 - All plates are MT20 plates unless otherwise indicated.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 560 lb uplift at joint 2, 1379 lb uplift at joint 13, 2122 lb uplift at joint 13 and 378 lb uplift at joint 19.

Continued on Page 2



November 18, 2010



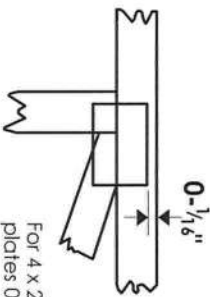
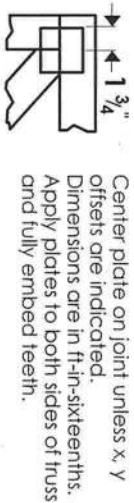
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in **Mitek 20/20** software or upon request.

PLATE SIZE

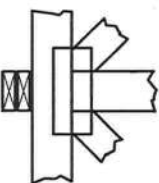
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



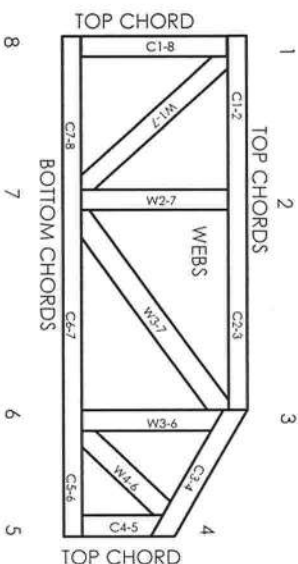
BEARING



Industry Standards:

ANSI/TP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B,
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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Julius Lee
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Boynton, FL 33435

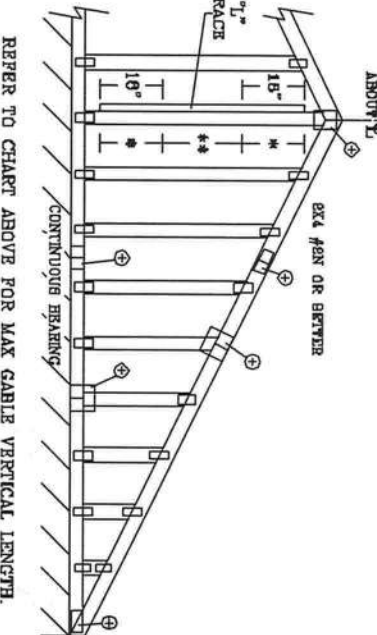
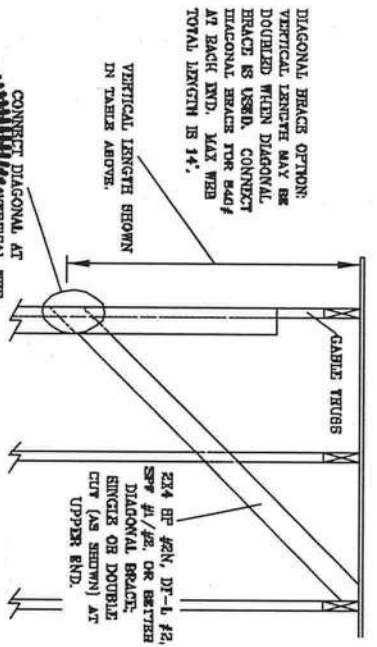
General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by ANSI/TP 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP 1 Quality Criteria.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

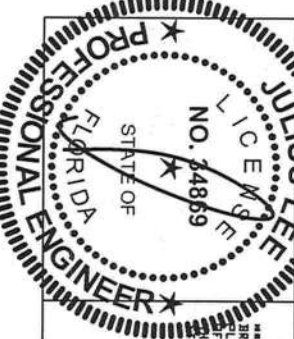
MAX GABLE VERTICAL LENGTH									
GABLE VERTICAL SPECIES	BRACE	NO BRACES	(1) 12" T ² BRACE *		(1) 24" T ² BRACE *		(2) 24" T ² BRACE **		(1) 24" T ² BRACE *
			GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	
24" O.C.	SPF	#1 / #2	3' 4"	5' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 8"
	STUD	#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	6' 3"	10' 1"
	HF	STANDARD	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	6' 3"	10' 1"
	SP	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	10' 10"
16" O.C.	SPF	#1 / #2	3' 6"	5' 0"	6' 0"	6' 11"	7' 5"	8' 3"	10' 4"
	STUD	#3	3' 6"	5' 0"	5' 0"	6' 6"	6' 6"	6' 3"	10' 4"
	HF	STANDARD	3' 6"	5' 0"	5' 0"	6' 6"	6' 6"	6' 3"	10' 4"
	SP	#1	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	10' 11"
12" O.C.	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	10' 11"
	STUD	#3	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	10' 11"
	HF	STANDARD	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	10' 11"
	SP	#1	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	10' 11"



BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPF	#1 / #2	SPF	#1 / #2
STUD	#3	STUD	#3
HF	STANDARD	HF	STANDARD
SP	#1	SP	#1

CABLE TRUSS DETAIL NOTES:

- 1. LIVE LOAD DEFLECTION CRITERIA IS $L/240$.
- 2. PROVIDE UPLIFT CONNECTIONS FOR 136 PSF OVER CONTINUOUS BEARING (6 PSF TO DEAD LOAD).
- 3. CABLE END SUPPORTS LOAD FROM 4' 0" OUTLEAKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
- 4. ATTACH EACH T² BRACE WITH 10d NAILS.
- 5. * FOR (1) T² BRACE, BRACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
- 6. ** FOR (2) T² BRACES, BRACE NAILS AT 8" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.
- 7. T² BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.



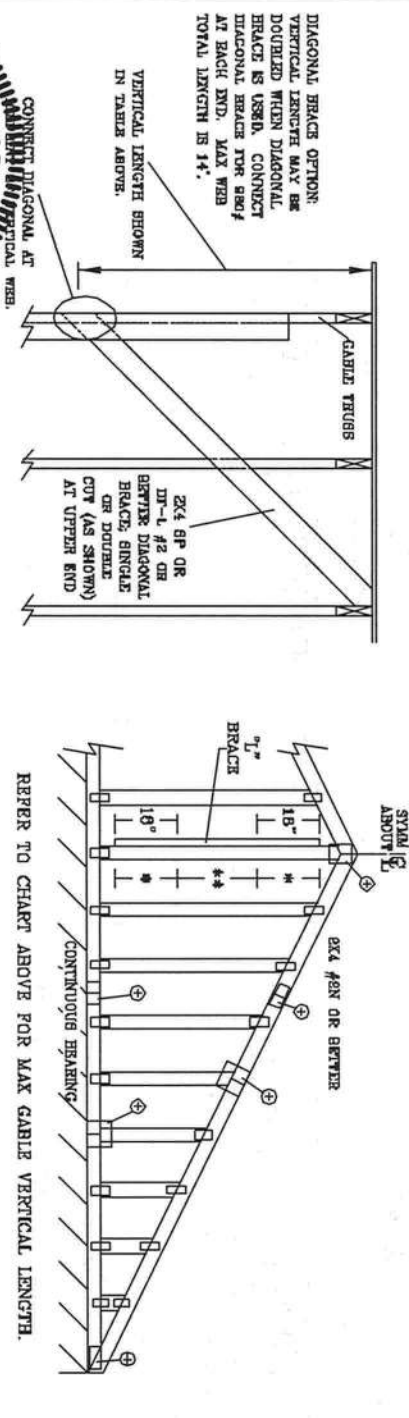
REVIEWED
By Julius Lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th Avenue
MIAMI BEACH, FL 33134-2161
No. 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF	MAX. SPACING 24.0"
REF ASCE 7-02-CAB13015	DATE 11/26/03
DRWG MTKR STD GABLE IS E ET	ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

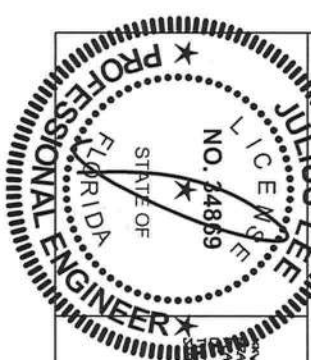
MAX GABLE VERTICAL LENGTH																		
GABLE VERTICAL SPACING SPECIES	BRACE	NO BRACES	(1) 1X4 "L" BRACE *				(1) 2X4 "L" BRACE *				(2) 2X4 "L" BRACE **				(1) 2X6 "L" BRACE *		(2) 2X8 "L" BRACE **	
			GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B		
24" O.C.	SPF	#1 / #2	3' 2"	5' 6"	6' 5"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"					
			#3	3' 1"	4' 5"	4' 5"	6' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"				
			STUD	3' 1"	4' 5"	4' 5"	6' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"				
		HF	STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	6' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"				
			#1	3' 6"	5' 8"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"				
			#2	3' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"				
	DFL	#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 8"					
			STUD	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 8"				
			STANDARD	3' 0"	3' 10"	3' 10"	6' 10"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"					
		SPF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 6"	7' 8"	8' 11"	11' 2"	14' 0"	14' 0"					
				#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	11' 2"	11' 9"	11' 9"	14' 0"	14' 0"			
				STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"			
16" O.C.	HF	STANDARD	3' 7"	5' 6"	6' 5"	7' 8"	7' 8"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"					
		#1	4' 0"	6' 4"	6' 5"	6' 2"	6' 2"	8' 3"	8' 3"	11' 9"	11' 9"	14' 0"	14' 0"					
		#2	3' 11"	6' 4"	6' 7"	7' 4"	7' 4"	8' 11"	8' 11"	11' 9"	11' 9"	14' 0"	14' 0"					
	SP	#3	3' 8"	5' 7"	6' 7"	7' 4"	7' 4"	7' 4"	8' 6"	11' 5"	11' 5"	14' 0"	14' 0"					
			STUD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	8' 5"	11' 4"	11' 4"	14' 0"	14' 0"				
			STANDARD	3' 6"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"				
12" O.C.	SPF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 3"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"					
			#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"				
			STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"				
	HF	STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"					
		#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 3"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"					
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 3"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"					
DFL	#3	4' 2"	6' 4"	6' 4"	8' 3"	8' 3"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"						
		STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"					
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"					



CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DEFLECTION CRITERIA IS $L/240$.	
PROVIDE UP/EAVE CONNECTIONS FOR 180 PSF OVER CONTINUOUS BRACING (6 PSF VC DEAD LOAD).	
CABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.	
ATTACH EACH "L" BRACE WITH 10d NAILS.	
* FOR (1) "L" BRACE, SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.	
** FOR (2) "L" BRACES, SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.	
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.	

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO BRACE
LESS THAN 4' 0"	1X6 OR BTR
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2X6

+ REFER TO COMMON TRUSS DESIGN FOR PLATE, BRACE, AND BEAM PLATES.



REVIEWED

By Julius Lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S

CONS. ENGINEERS P.A.

1466 SW 4th AVENUE

DELMAR BEACH, FL 33444-8161

REF ASCE7-02-CAB13030

DATE 11/26/03

DWG NAME STD GABLE 30' E 17

-ENG

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

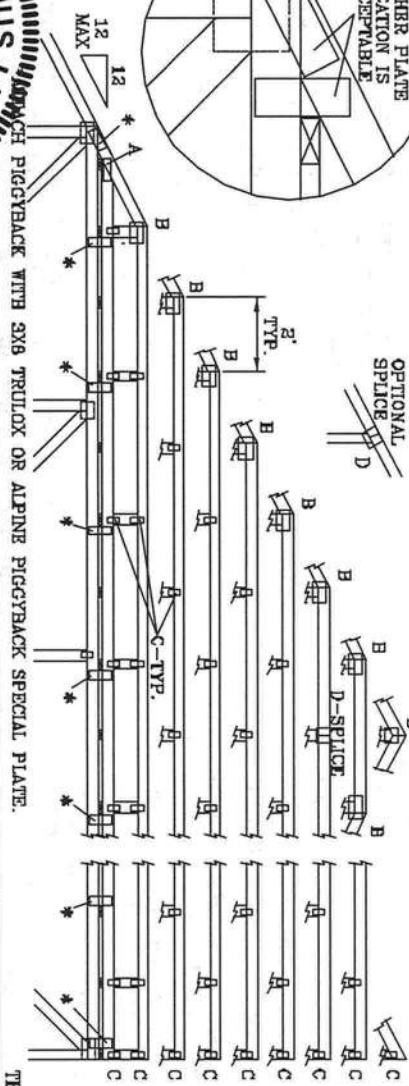
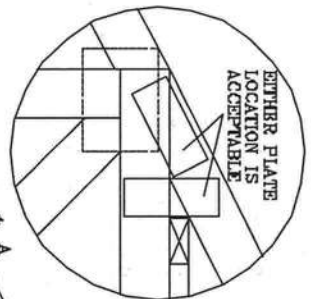
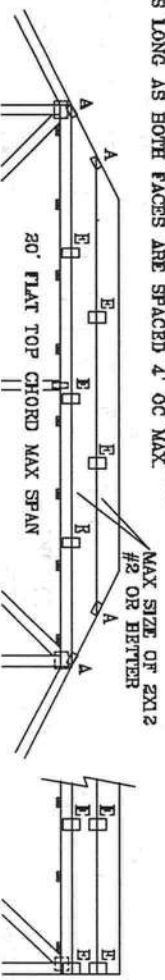
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C, WIND TC DL=6 PSF, WIND BC DL=6 PSF
110 MPH WIND, 30' MEAN HGT, PFCG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=6 PSF, WIND BC DL=6 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (E*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

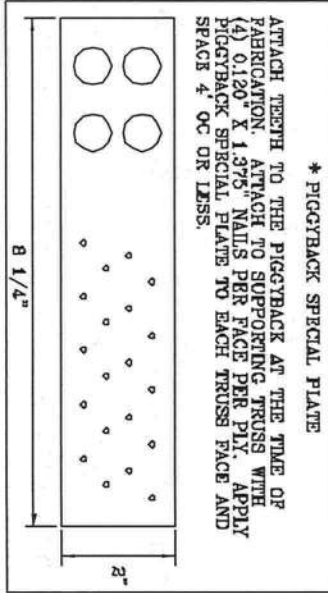


THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045

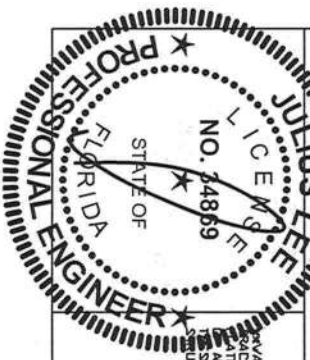
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.5X4	2.5X4	3X6
B	4X6	6X6	6X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	6X4	6X6	6X6	5X6
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY			

ATTACH TRUSS PLATES WITH (6) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB LENGTH	REQUIRED CHART
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80X LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4' OC.
10' TO 14'	2X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80X LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.



* PIGGYBACK SPECIAL PLATE
ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1406 NW 4th Avenue
Doral, FL 33141-2161

No: 34866
STATE OF FLORIDA

MAX LOADING	55 PSF AT 1.33 DUR. FAC.	DATE 09/12/07
	50 PSF AT 1.25 DUR. FAC.	DRWG/ITEK STD PIGGY
SPACING	47 PSF AT 1.15 DUR. FAC.	-ENG JL
	24.0"	

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

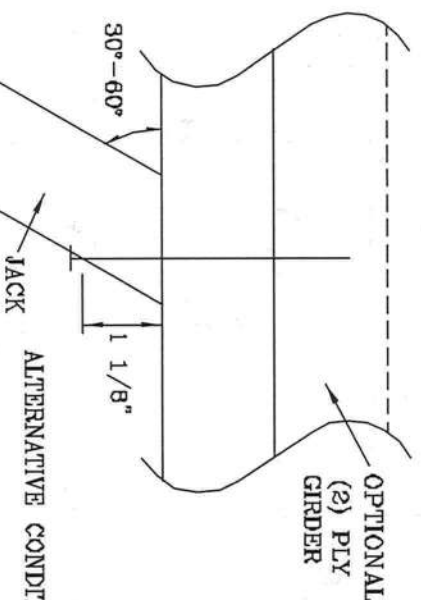
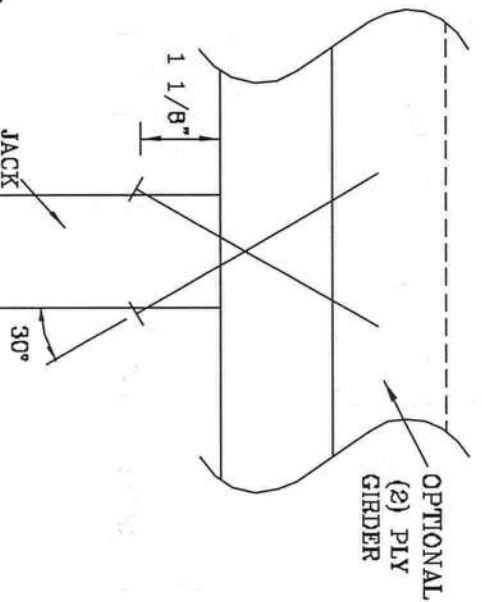
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

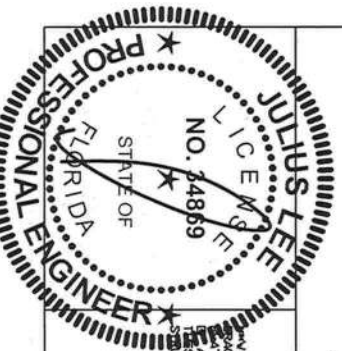
MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



THIS DRAWING REPLACES DRAWING 784040



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTING. TO BE USED IN CONJUNCTION WITH THE COMPANY'S TRUSS CONSTRUCTION MANUAL. TRUSSES ARE DESIGNED FOR SPECIFIC LOADS AND CONDITIONS. ANY DEVIATION FROM THE MANUFACTURER'S INSTRUCTIONS MAY VOID THE WARRANTY. TRUSSES ARE NOT TO BE USED FOR ANY OTHER PURPOSES. TRUSSES ARE NOT TO BE USED FOR ANY OTHER PURPOSES. TRUSSES ARE NOT TO BE USED FOR ANY OTHER PURPOSES.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.

1400 SW 4th Avenue
Delray Beach, FL 33444-2161

No. 34869
STATE OF FLORIDA

TC LL PSF REF TOE-NAIL

TC DL PSF DATE 09/12/07

BC DL PSF DRWG C/TONAIL1103

BC LL PSF -ENG JL

TOT. LD. PSF

DUR. FAC. 1.00

SPACING

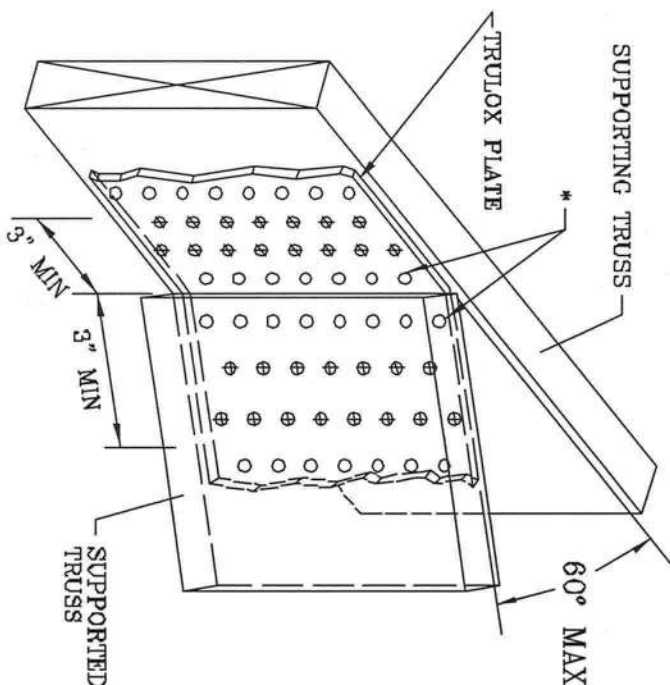
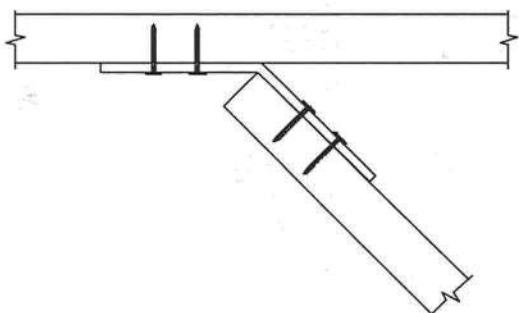
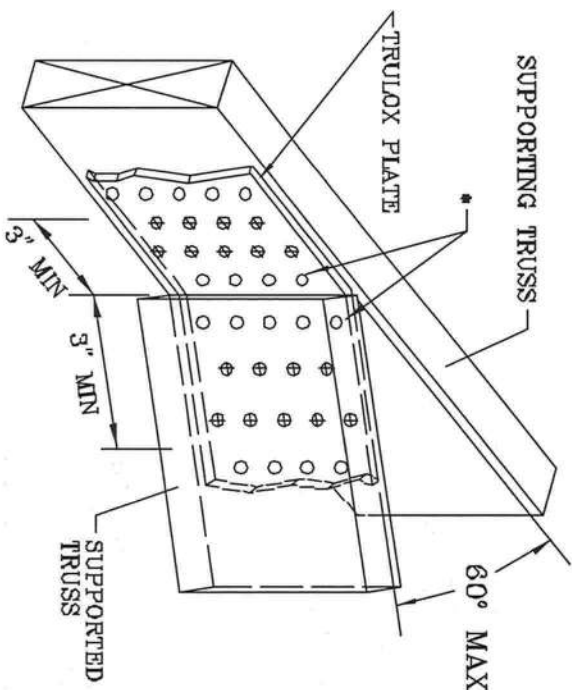
TRULOX CONNECTION DETAIL

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.
REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

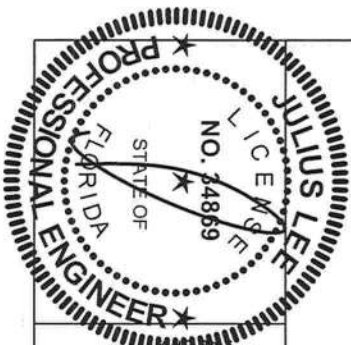


MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
6X6	16	990#

MINIMUM 6X6 TRULOX PLATE

REVIEWED
By Julius Lee at 11:56 am, Jun 11, 2008



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND MAINTAINING. REFER TO SECT 1-103 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 384 DOWNSIDE DR., SUITE 800, WATSON, VA 22792 AND VITA CYCLO TRUSS COUNCIL, 6300 ENTERPRISE LN, WATSON, VA 22792 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

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DELRAY BEACH, FL 33444-2101

No. 34869
STATE OF FLORIDA

THIS DRAWING REPLACES DRAWINGS 1.168.988 1.168.988/B
1.164.844 1.162.217 1.162.017 1.159.154 & 1.161.524

REF TRULOX

DATE 11/26/03

DRWG CNTRULOX1103

—ENG JL

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/4" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 1/16" maximum.

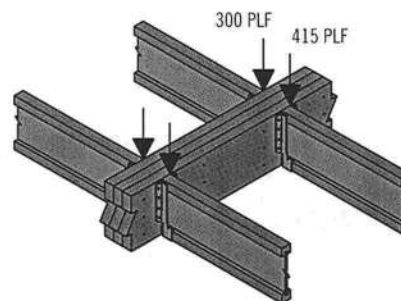
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.