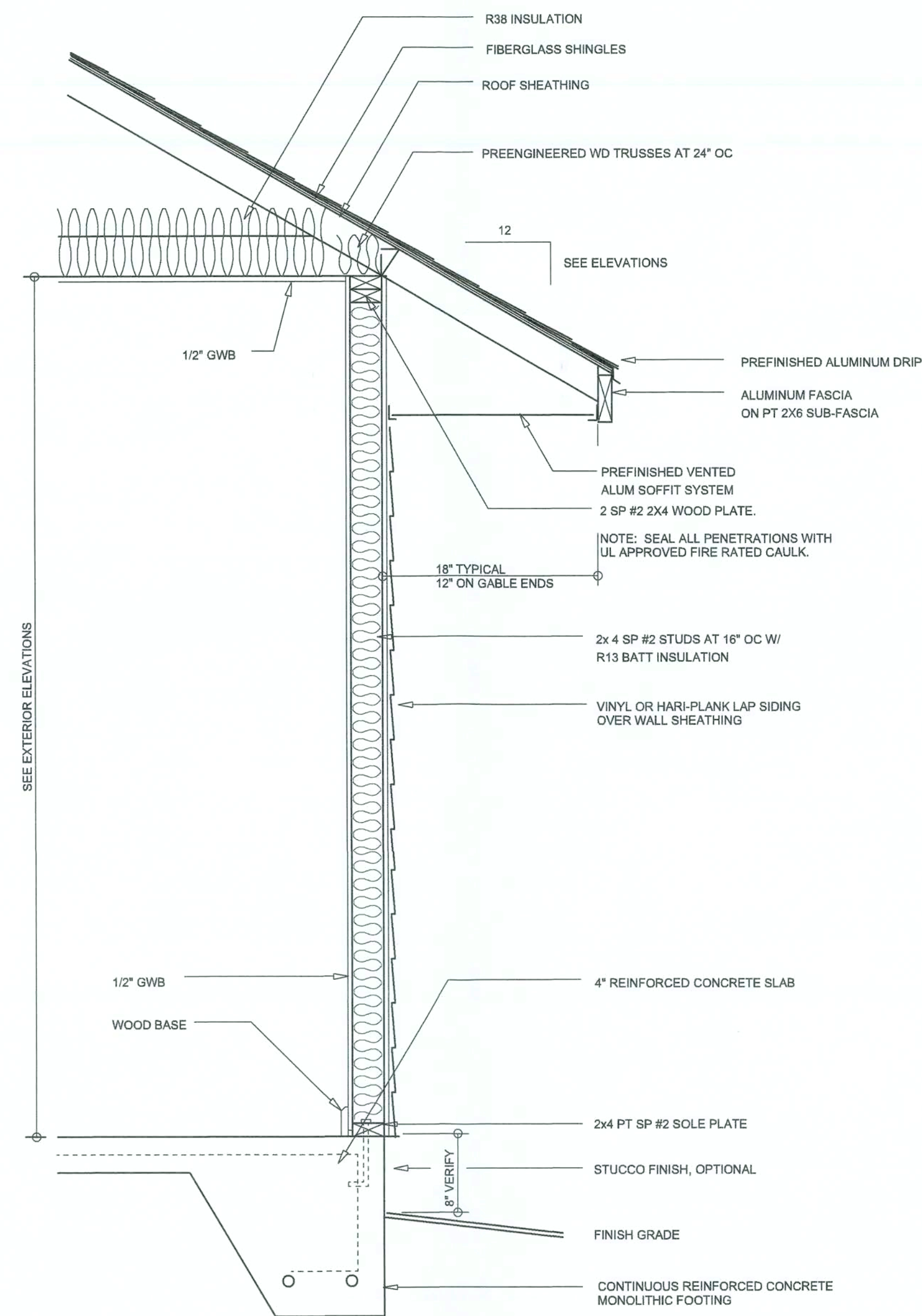




LEFT ELEVATION  
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION  
SCALE: 1/4" = 1'-0"



TYPICAL WALL SECTION  
SCALE: 1" = 1'-0"



REAR ELEVATION  
SCALE: 1/4" = 1'-0"



FRONT ELEVATION  
SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

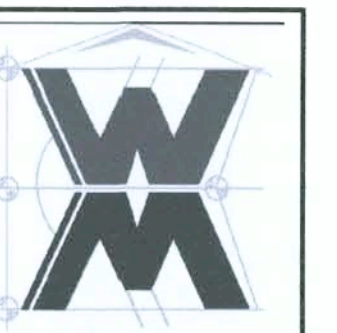
REVISIONS
July 02, 2019

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE

EXTERIOR ELEVATIONS  
SCALE: 1/4" = 1'-0"  
TYPICAL WALL SECTION  
SCALE: 1" = 1'-0"

A CUSTOM HOME FOR:  
**MASSEY**  
PROJECT ADDRESS: 169 SW Dove Way Lake City FL, 32024  
**GIBALTAR CONTRACTING, LLC.**  
LIC# 1269633 HIGH SPRINGS, FLORIDA

© WM DESIGN & ASSOCIATES, INC.  
426 SW COMMERCE DR, STE 130  
LAKE CITY, FL 32025  
(386) 758-8406  
will@willmyers.net

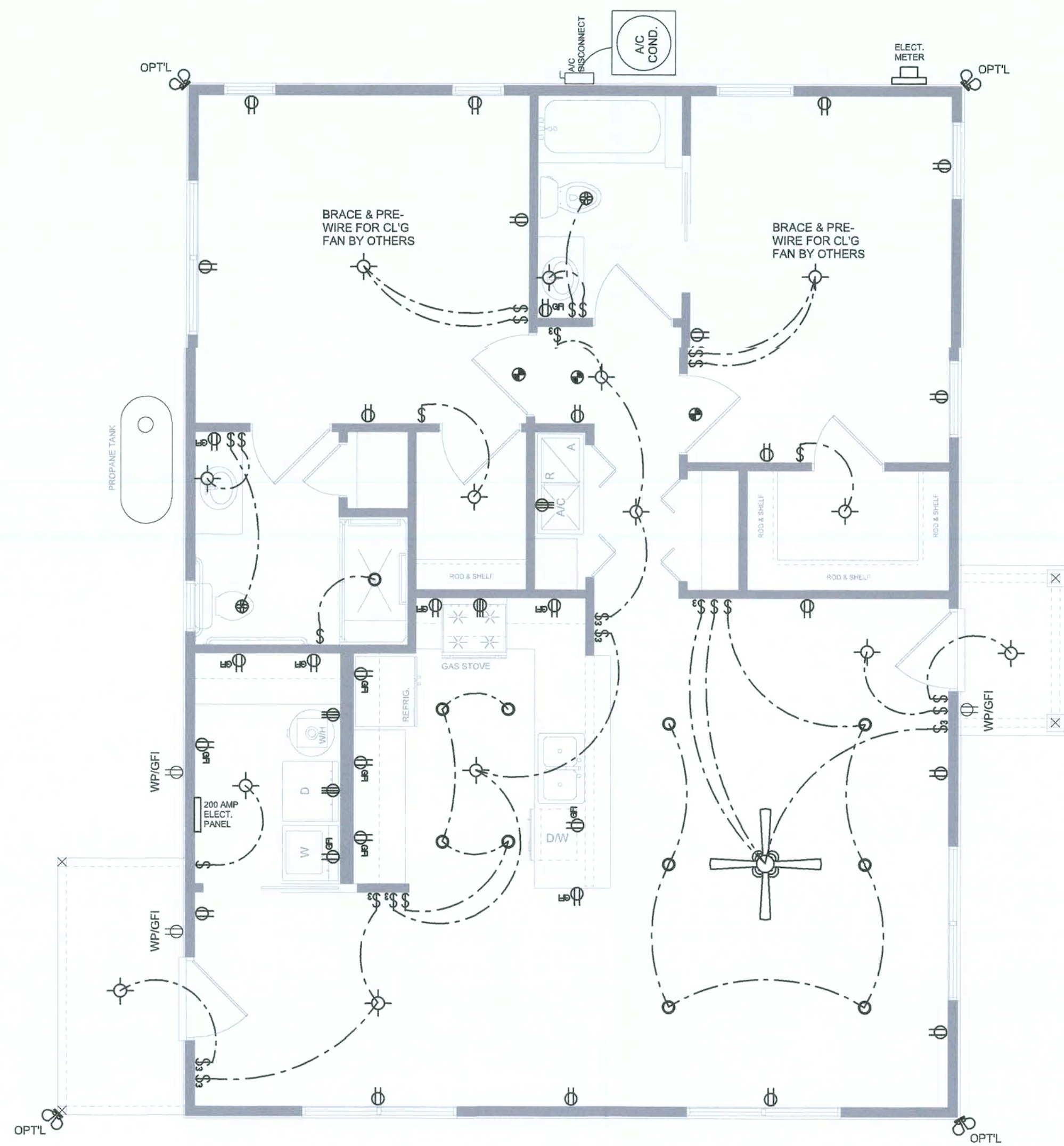


JOB NUMBER  
20190524

SHEET NUMBER  
**A.1**

*Will C. Myers*





**FLOOR PLAN**  
SCALE: 1/4" = 1'-0"

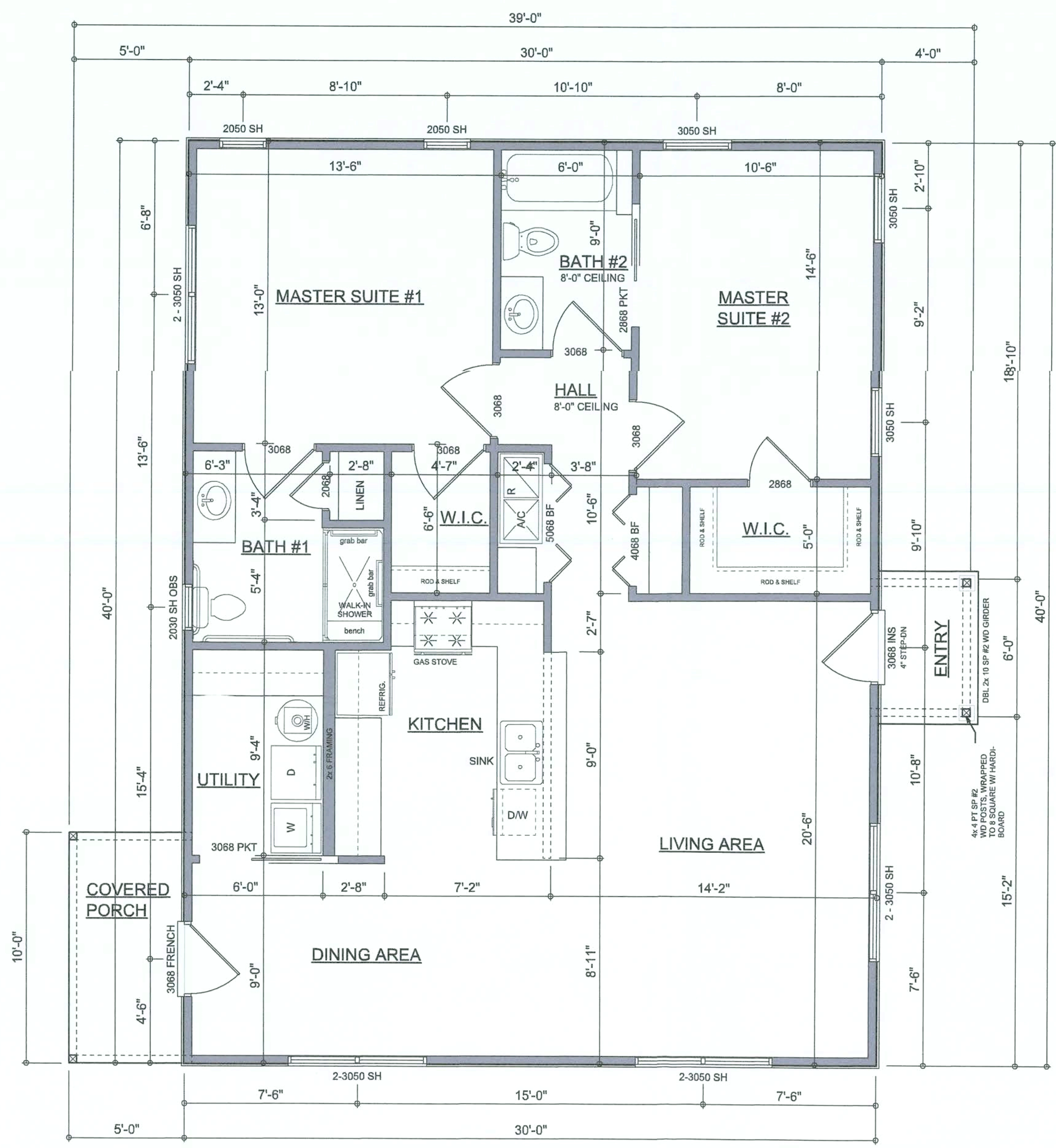
ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET (AFCI & TAMPER RESISTANT)
	220v OUTLET
	GFI DUPLEX OUTLET (PER NEC 406.8)
	TELEVISION JACK
	TELEPHONE JACK
	SMOKE / CARBON MONOXIDE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	2 OR 4 TUB FLUORESCENT FIXTURE

NOTE:  
ALL INTERIOR RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER NEC 406.11

ALL SMOKE DETECTORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR AND SHALL HAVE BATTERY BACKUP POWER AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS. CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR SHALL BE USED AS AN EQUIPMENT GROUND.

IT IS THE LICENSED ELECTRICAL CONTRACTORS RESPONSIBILITY TO INSURE THAT ALL WORK PERFORMED AND EQUIPMENT INSTALLED MEETS OR EXCEEDS THE NFPA70 2014 NATIONAL ELECTRIC CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.



**FLOOR PLAN**  
SCALE: 1/4" = 1'-0"

ALL CEILING HEIGHTS SHALL BE 8'-0" UNLESS OTHERWISE NOTED

AREA SUMMARY		
LIVING AREA	1,200	S.F.
ENTRY PORCH AREA	24	S.F.
COVERED PORCH AREA	50	S.F.
TOTAL AREA	1,274	S.F.

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

*Wm C Myers*

REVISIONS

July 02, 2019

DIMENSIONED FLOOR PLAN

SCALE: 1/4" = 1'-0"

ELECTRICAL PLAN

SCALE: 1/4" = 1'-0"

A CUSTOM HOME FOR:

**MASSEY**

PROJECT ADDRESS: 169 SW Dove Way Lake City FL, 32024

GIBALTAR CONTRACTING, LLC.

LIC# 1259633 HIGH SPRINGS, FLORIDA

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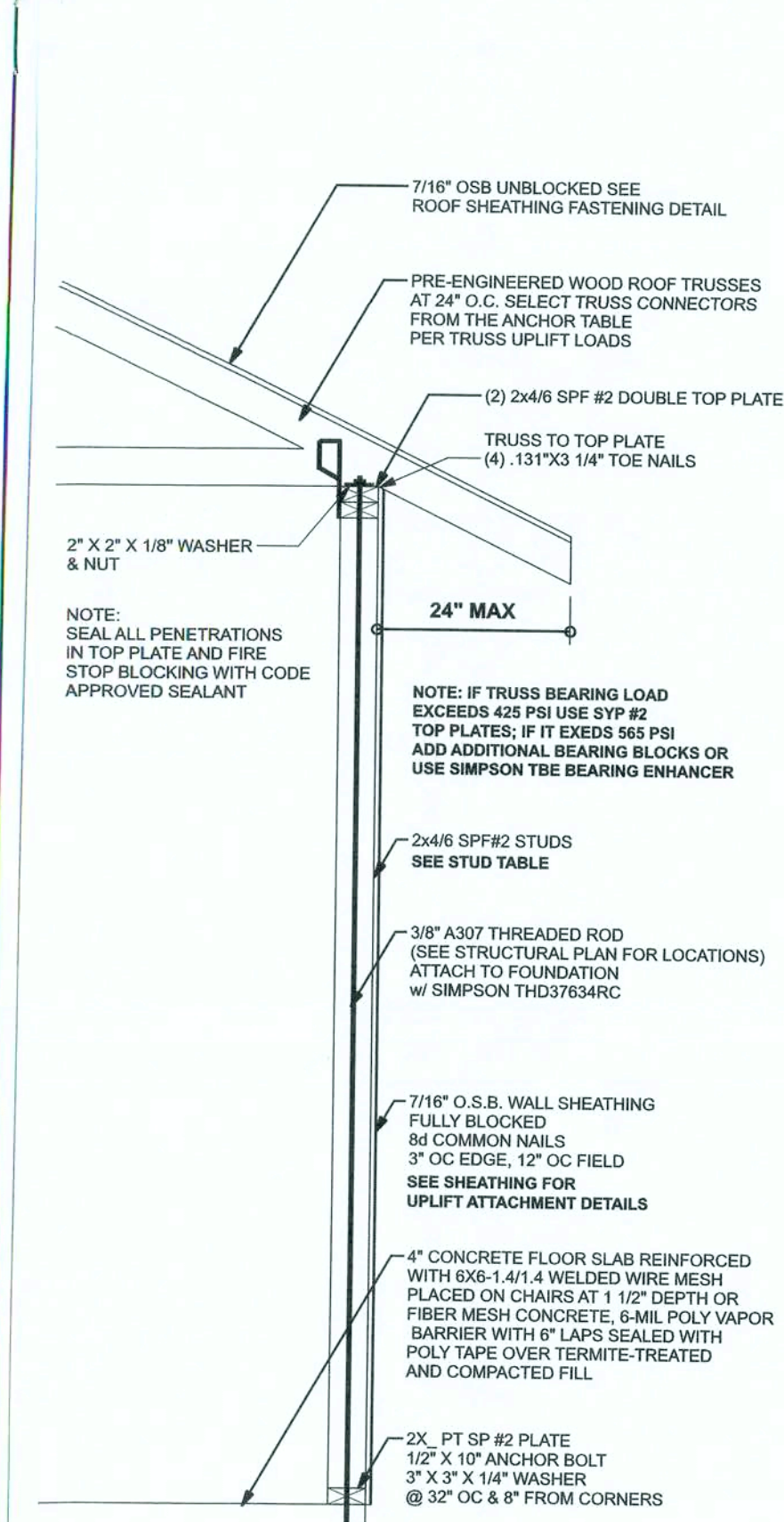
JOB NUMBER

20190524

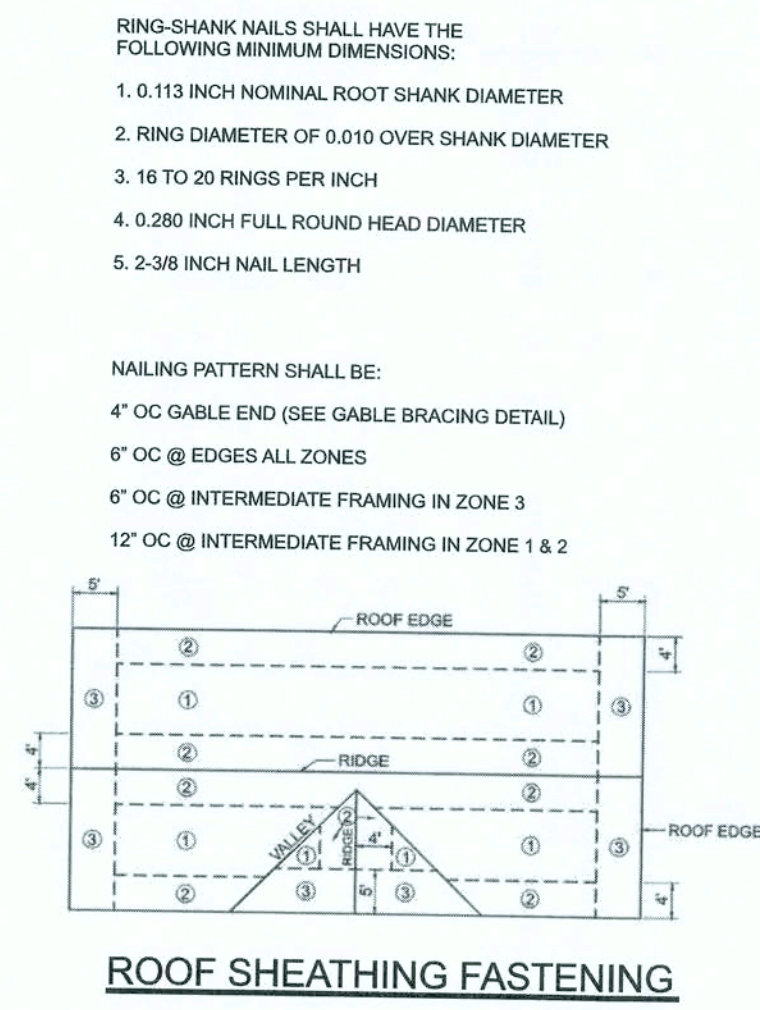
SHEET NUMBER

A.2

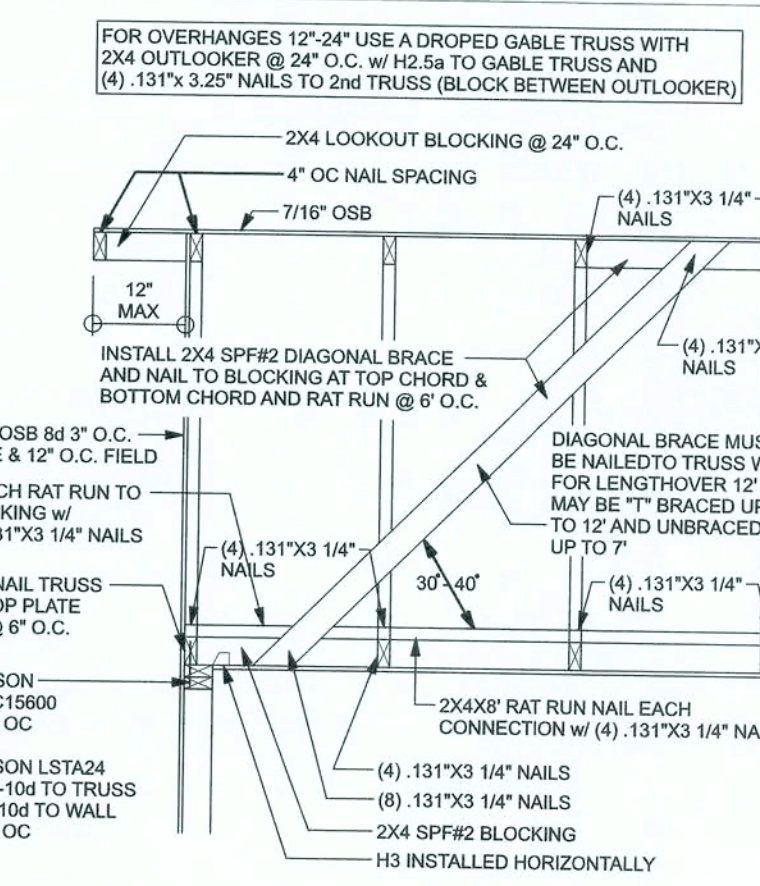




ONE STORY WALL SECTION  
SCALE: 3/4" = 1'-0"

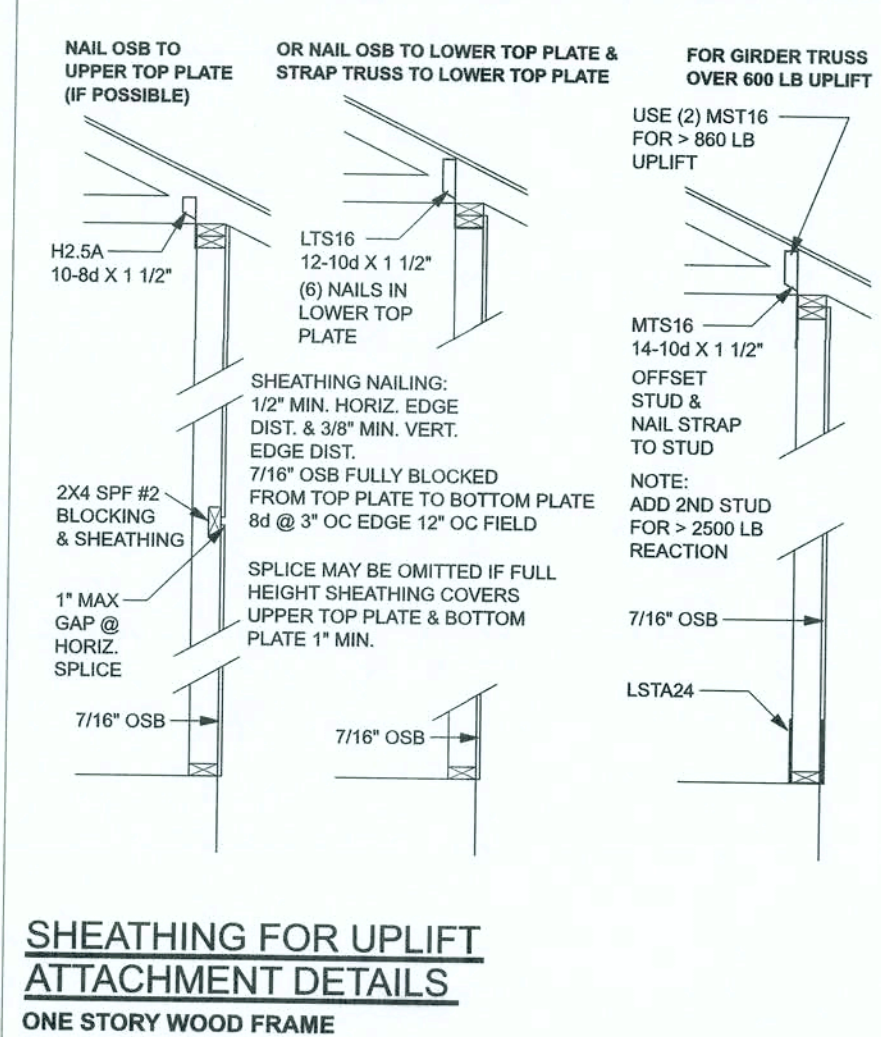


ROOF SHEATHING FASTENING

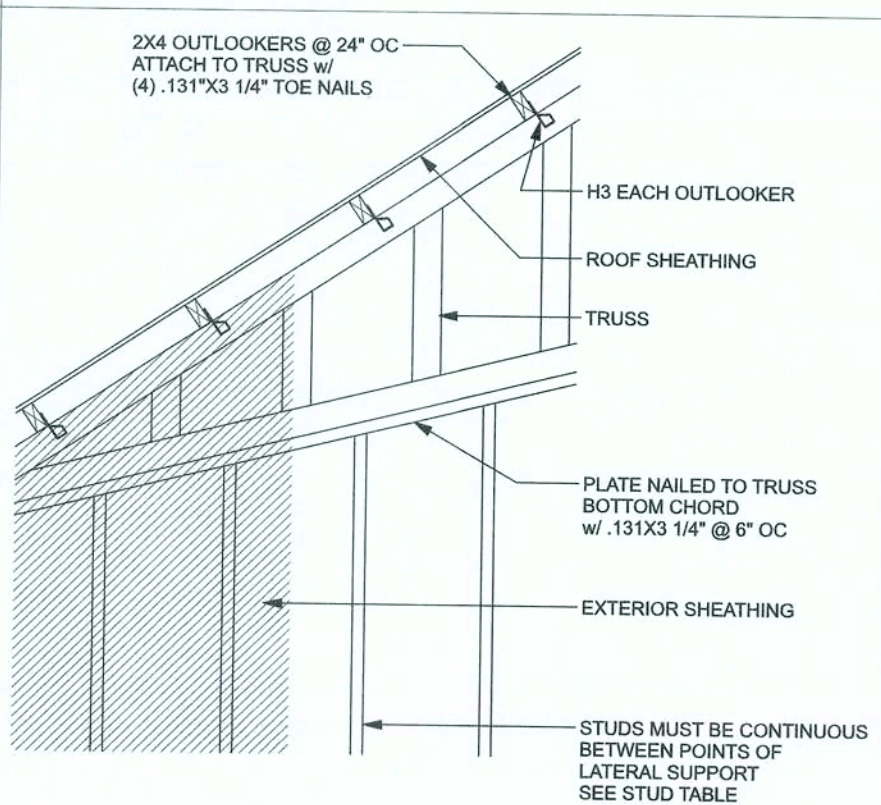


SPACE RAY RUN & DIAGONAL BRACE 6'-0" G.C.  
FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCLOSED

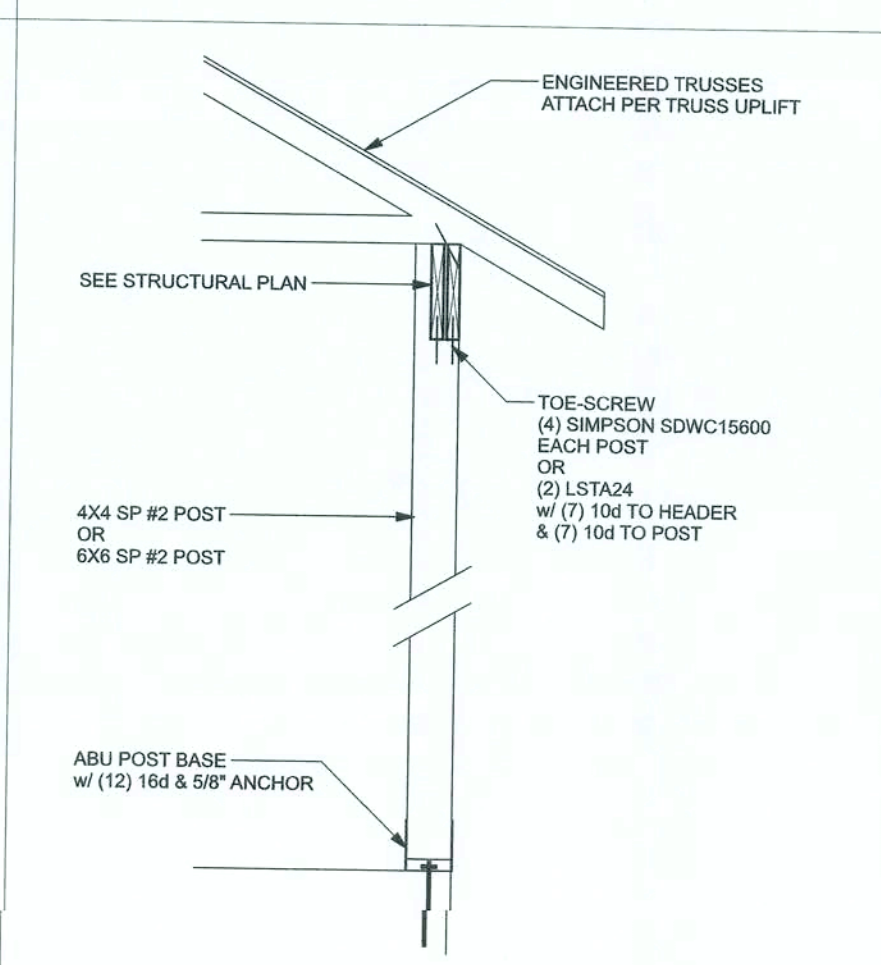
(TYP.) GABLE BRACING DETAIL  
WOOD FRAME



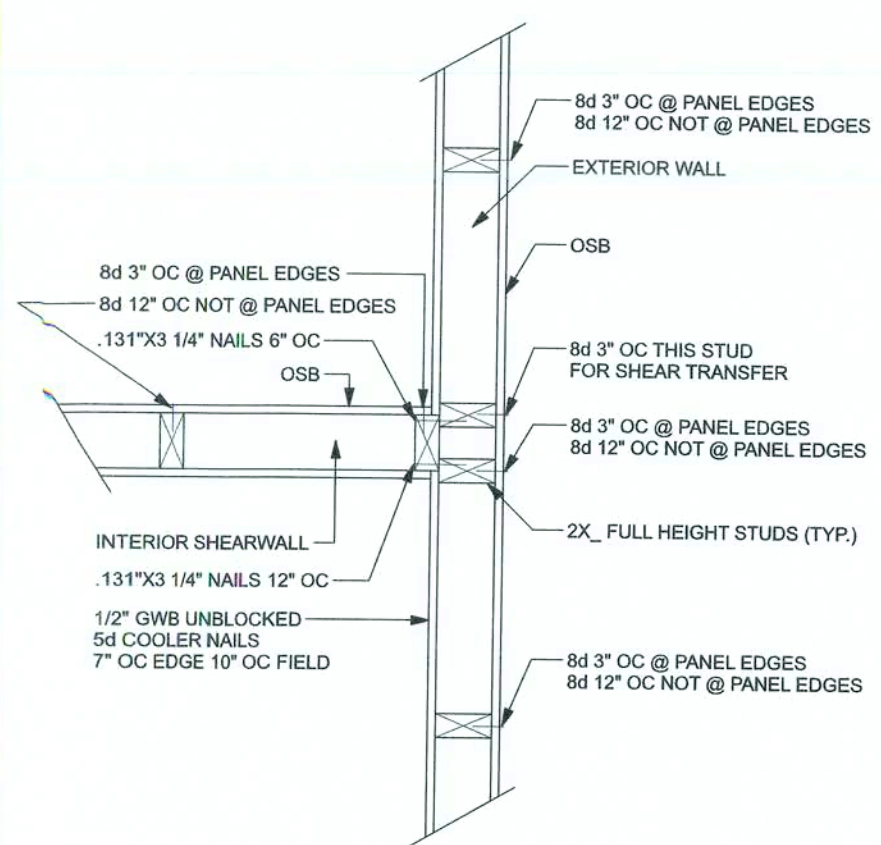
SHEATHING FOR UPLIFT ATTACHMENT DETAILS  
ONE STORY WOOD FRAME



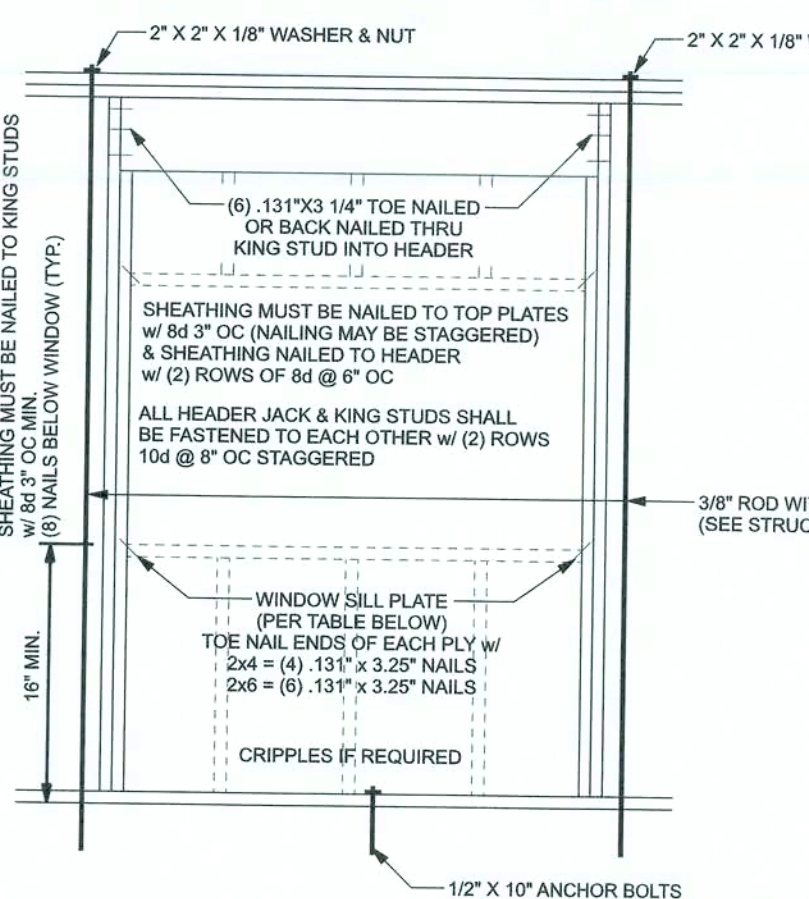
(TYP.) GABLE WALL w/ VAULTED CEILING  
WOOD FRAME



(TYP.) PORCH POST  
ONE STORY WOOD



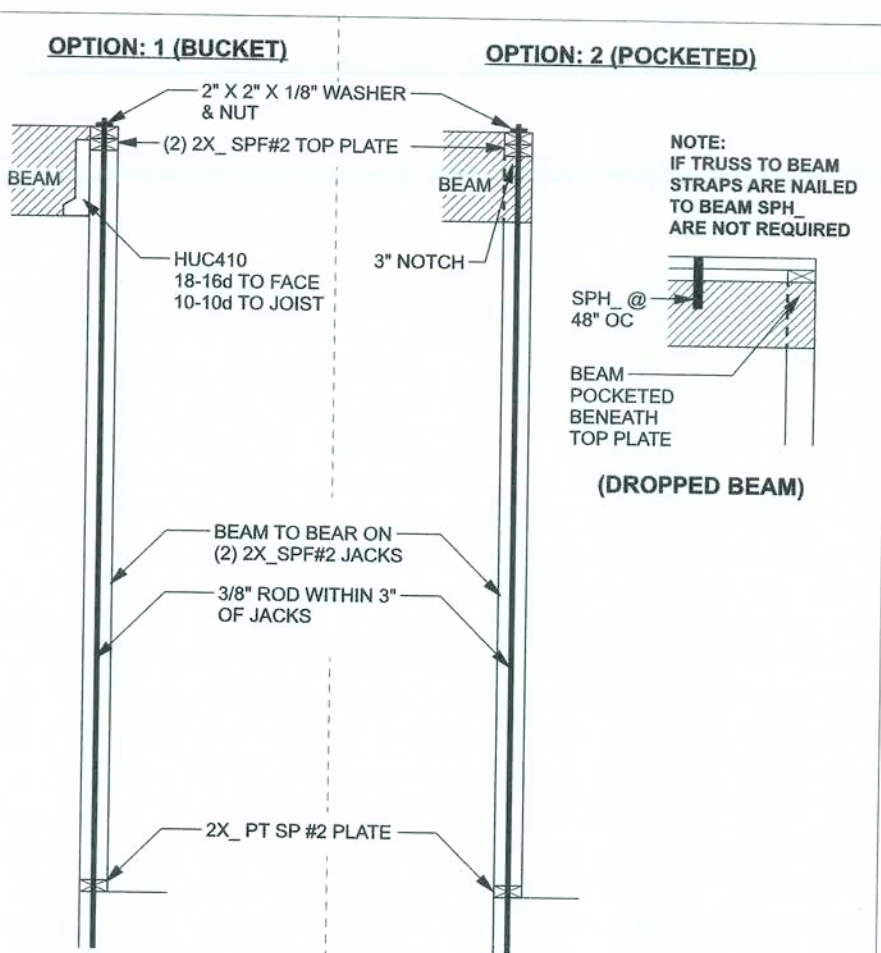
(TYP.) INTERSECTING WALL FRAMING  
WOOD FRAME



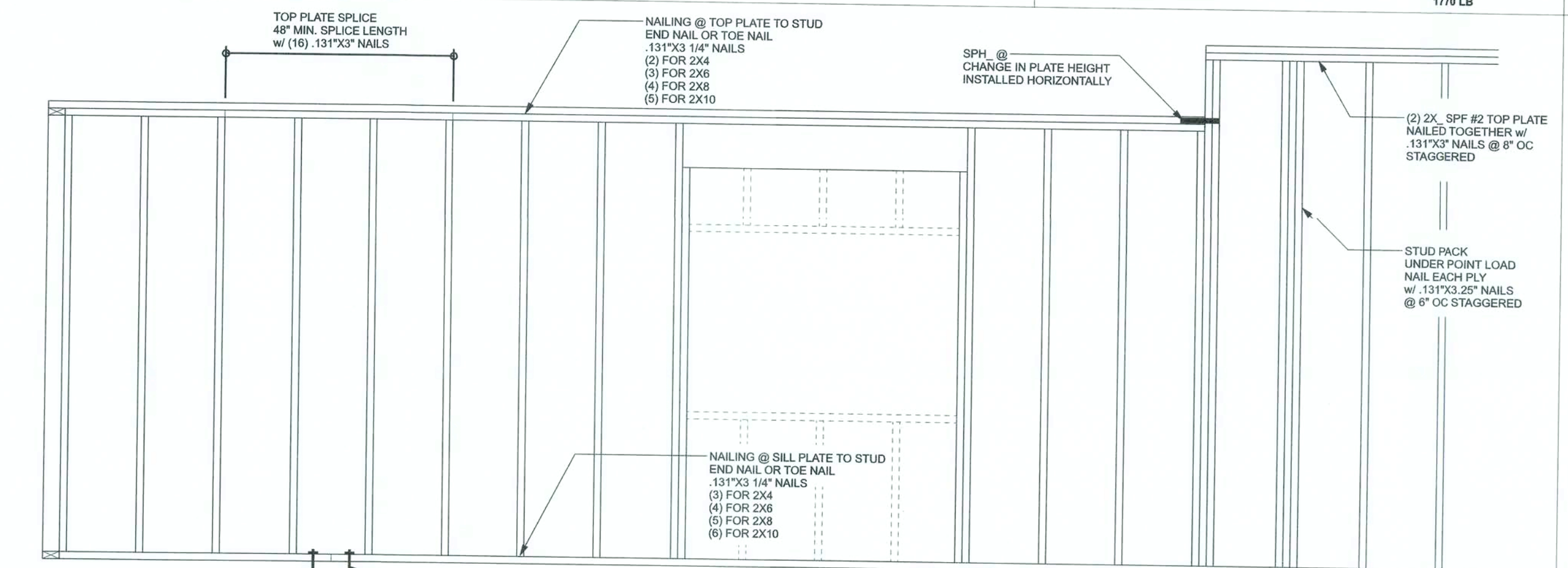
TYPICAL HEADER STRAPING DETAIL  
ONE STORY WOOD FRAME

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT				
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.2.8		
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6
130 MPH EXP. C	5'-2"	7'-6"	7'-3"	11'-3"

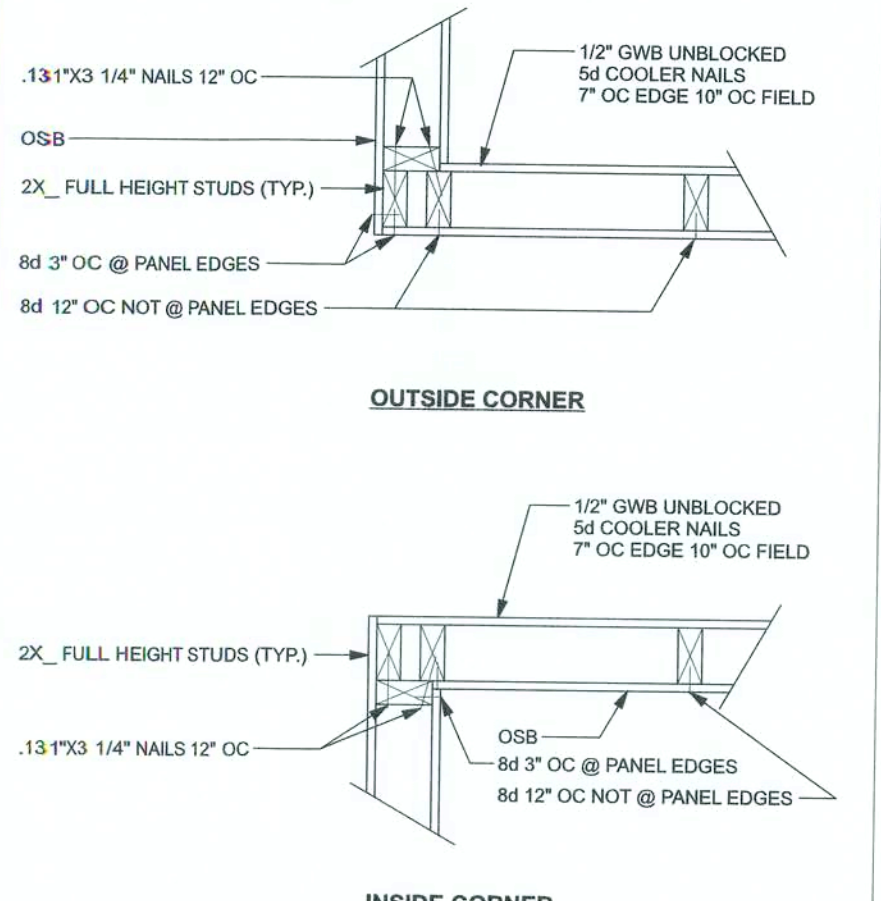
FOR OTHER WALL HEIGHTS (IN SILL SPAN SHALL BE DIVIDED BY (4/10))



(TYP.) BEAM TO WALL  
WOOD FRAME w/ RODS



(TYP.) WALL CONNECTIONS  
ONE STORY WOOD FRAME



(TYP.) CORNER FRAMING  
WOOD FRAME

CONNECTOR TABLE				
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
415	290	H3	4-6x1 1/2"	4-6x1 1/2"
575	485	H2.5A	5-6x1 1/2"	5-6x1 1/2"
1340	1015	H10A	9-10x1 1/2"	9-10x1 1/2"
720	620	LTS12-30	6-10x1 1/2"	6-10x1 1/2"
1000	960	MTS12-30	7-10x1 1/2"	7-10x1 1/2"
1450	1240	MTS25-30	12-10x1 1/2"	12-10x1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LST241	8-10x1	8-10x1
1640	1455	MTS24	9-10x1	9-10x1
1030	1030	CSS2	7-10x1	7-10x1
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
985	535	SP1	6-10x1	4-10x1
1065	605	SP2	6-10x1	6-10x1
771	771	LST244	16-16x1	wrap under or over plate
1235	1235	LST244	16-16x1	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stairwell	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	12"x12" Titan HD
4235	3940	HTT4	16-16x2 1/2"	12"x12" Titan HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	12"x6" Titan HD
4235	3940	HTT4	16-16x2 1/2"	12"x12" Titan HD
Uplift SP	Uplift SPF	Post Bases @ Stairwell	To Post	Anchor
2200	ABU44	12-16x1	5/8"x12" Dbl & Epoxy	
2300	ABU66	12-16x1	5/8"x12" Dbl & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200	ABU44	12-16x1	5/8"x12" Dbl & Epoxy	
2300	ABU66	12-16x1	5/8"x12" Dbl & Epoxy	

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.2.8B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/404 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)	
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE

	SP	#2	Fb	E
2x8	SP #2	925	1.4	
2x10	SP #2	800	1.4	
2x12	SP #2	750	1.4	
GLB	24F-V3 SP	2600	1.9	
LSL	TIMBERSTRAND	1700	1.7	
LVL	MICROLAM	2950	2.0	
PSL	PARALAM	2900	2.0	

## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE TRUSS DESIGNER BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2x8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $F_c = 2500$  PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 65KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C-1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTORS APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS,  $F_y = 40$  KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNCL. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.A.G.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3' FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

## BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DERRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FCRI REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

## ROOF SYSTEM DESIGN:

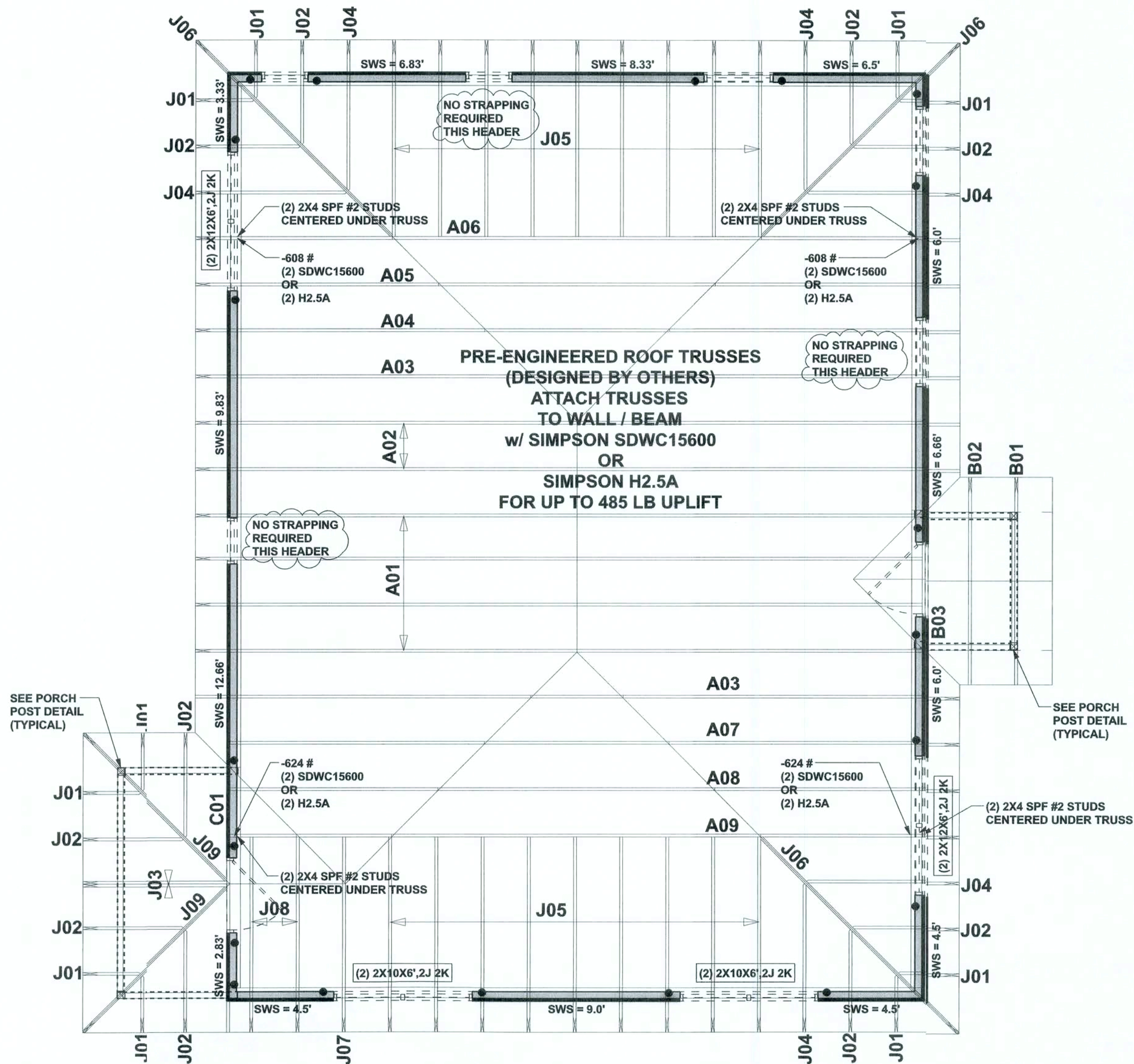
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FCRI, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FCRI REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO BONES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN CRITERIA & LOADS:	
BUILDING CODE	6TH EDITION, FLORIDA BUILDING CODE RESIDENTIAL (2017)
CODE FOR DESIGN LOADS	ASCE 7-10
<b>WINDLOADS</b>	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	1
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION (BUILDER MUST FIELD VERIFY)	ENCLOSED
INTERNAL PRESSURE (BUILDER MUST FIELD VERIFY)	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
<b>FLOOR LOADING</b>	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
<b>ROOF LOADING</b>	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C) (VUL)	
EFFECTIVE WIND AREA (F <sub>TZ</sub> )	ZONE 4 INTERIOR
0 - 20	42.8 - 48.2
20 - 40	42.8 - 57
40 - 60	42.8 - 65.5
60 - 80	42.8 - 74.1
80 - 100	42.8 - 82.7
100 - 120	42.8 - 91.3
120 - 140	42.8 - 100.0
140 - 160	42.8 - 108.7
160 - 180	42.8 - 117.3
180 - 200	42.8 - 126.0
200 - 220	42.8 - 134.7
220 - 240	42.8 - 143.3
240 - 260	42.8 - 152.0
260 - 280	42.8 - 160.7
280 - 300	42.8 - 169.3
300 - 320	42.8 - 178.0
320 - 340	42.8 - 186.7
340 - 360	42.8 - 195.3
360 - 380	42.8 - 204.0
380 - 400	42.8 - 212.7
400 - 420	42.8 - 221.3
420 - 440	42.8 - 230.0
440 - 460	42.8 - 238.7
460 - 480	42.8 - 247.3
480 - 500	42.8 - 256.0
500 - 520	42.8 - 264.7
520 - 540	42.8 - 273.3
540 - 560	42.8 - 282.0
560 - 580	42.8 - 290.7
580 - 600	42.8 - 299.3
600 - 620	42.8 - 308.0
620 - 640	42.8 - 316.7
640 - 660	42.8 - 325.3
660 - 680	42.8 - 334.0
680 - 700	42.8 - 342.7
700 - 720	42.8 - 351.3
720 - 740	42.8 - 360.0
740 - 760	42.8 - 368.7
760 - 780	42.8 - 377.3
780 - 800	42.8 - 386.0
800 - 820	42.8 - 394.7
820 - 840	42.8 - 403.3
840 - 860	42.8 - 412.0
860 - 880	42.8 - 420.7
880 - 900	42.8 - 429.3
900 - 920	42.8 - 438.0
920 - 940	42.8 - 446.7
940 - 960	42.8 - 455.3
960 - 980	42.8 - 464.0
980 - 1000	42.8 - 472.7

</





**TALL STEM WALL TABLE:**  
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with #5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

**MASONRY NOTE:**  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI/ASCE 1-02 Section	Specific Requirements
1.4A	Compressive strength
2.1	Mortar
2.2	Grout
2.3	CMU standard
2.3	Clay brick standard
2.4	Reinforcing bars, #3 - #11
2.4F	Coating for corrosion protection
2.4F	Coating for corrosion protection
3.3.E.2	Pipes, conduits, and accessories
3.3.E.7	Movement joints

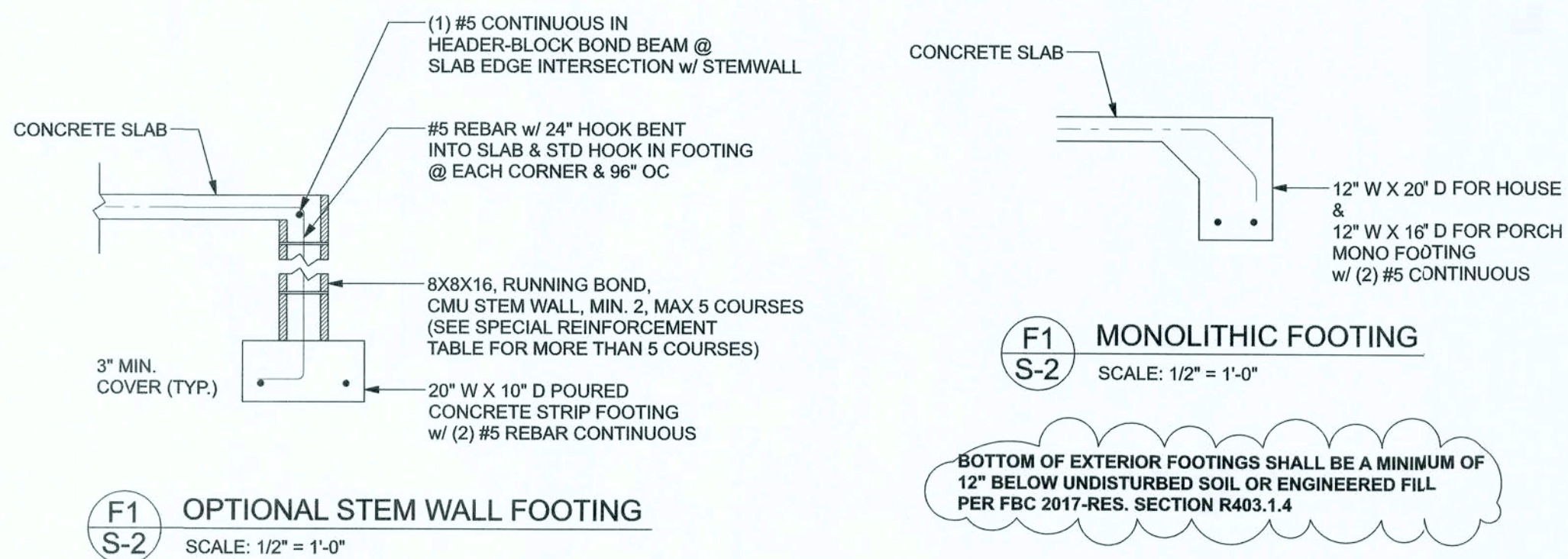
**FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"

**FOUNDATION NOTES**

FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

FN - 2 WALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN

FN - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/ 6X6-1/4" 4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL



CONNECTIONS, WALL & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, W.B. HOWLAND TRUSS CO. JOB #19-3159

Gibraltar Contracting, LLC

Massey

PROJECT ADDRESS: Columbia County, FL

**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to and engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915

PROFESSIONAL SEAL OF THE STATE OF FLORIDA

Monday, July 1, 2019

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**JOB NUMBER:**  
190739  
**S-2**  
OF 2 SHEETS