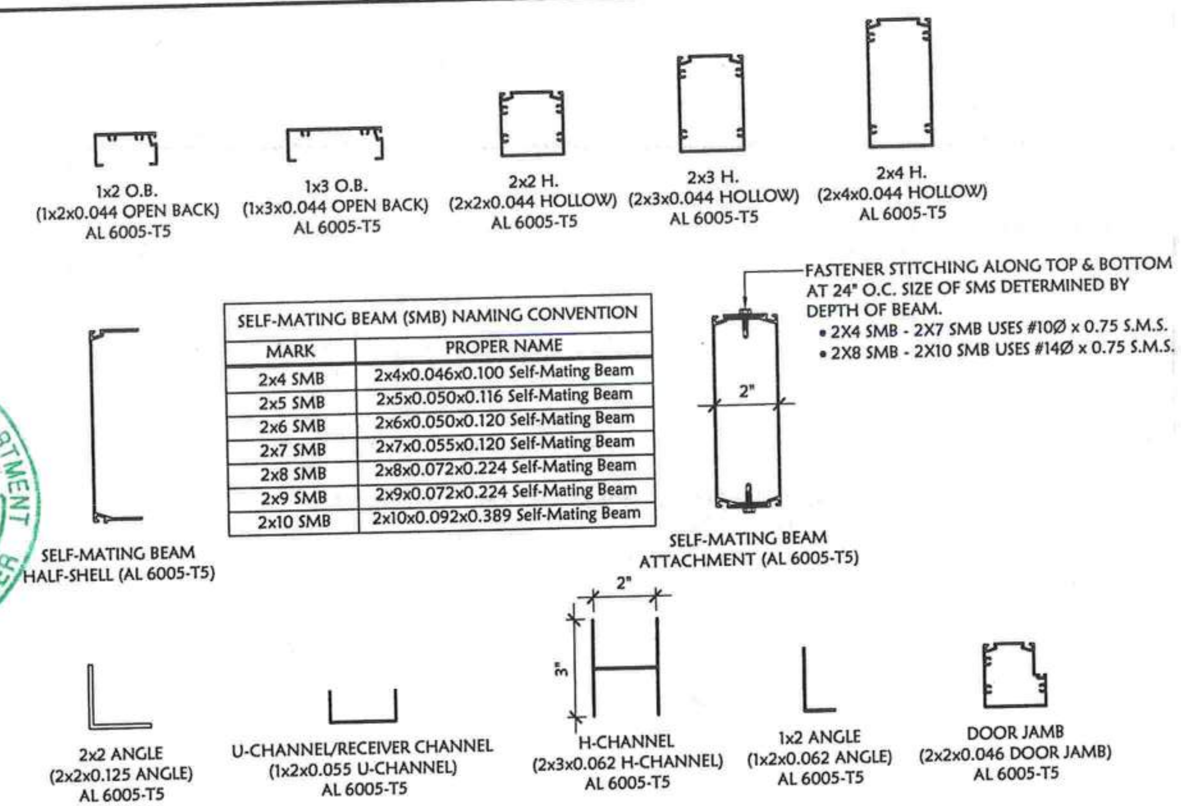
5



SELF-MATING BEAM (SMB) NAMING CONVENTION	
MARK	PROPER NAME
2x4 SMB	2x4x0.046x0.100 Self-Mating Beam
2x5 SMB	2x5x0.050x0.116 Self-Mating Beam
2x6 SMB	2x6x0.050x0.120 Self-Mating Beam
2x7 SMB	2x7x0.055x0.120 Self-Mating Beam
2x8 SMB	2x8x0.072x0.224 Self-Mating Beam
2x9 SMB	2x9x0.072x0.224 Self-Mating Beam
2x10 SMB	2x10x0.092x0.389 Self-Mating Beam

1 / S-1.0 Aluminum Self-Mating Beam (SMB) Naming Convention

SCALE: 3" = 1'-0"

COLUMN SCHEDULE (EXPOSURE "B")										
	TRIBUTARY WIDTH	4.0	4.5	5.0	5.5	6.0	6.5	7.0	7.5	8.0
COLUMN MEMBER TYPE		EFFECTIVE HEIGHT (FT.)								
	2x2 H	6.2	5.8	5.5	5.1	4.8	4.6	4.2	3.9	3.3
	2x3 H	8.1	7.8	7.3	6.9	6.7	6.4	6.1	5.9	5.5
	2x4 H	9.0	8.7	8.2	7.8	7.4	7.0	6.8	6.1	5.8
	2x4 SMB	13.9	13.1	12.4	11.8	11.3	10.9	10.5	10.1	9.8
	2x5 SMB	17.2	16.2	15.3	14.6	14.0	13.5	13.0	12.5	12.1
	2x6 SMB	19.6	18.5	17.6	16.7	16.0	15.4	14.8	14.3	13.9
	2x7 SMB	22.0	20.7	19.7	18.8	18.0	17.3	16.6	16.1	15.5
	2x8 SMB	30.3	28.9	27.4	26.1	25.0	24.0	23.2	22.4	21.7
	2x9 SMB	36.0	34.6	33.2	31.6	30.3	29.1	28.0	27.1	26.2
	2x10 SMB	41.5	39.9	38.5	37.0	35.4	34.0	32.6	31.7	30.7
	2x5 TFB	18.2	17.1	16.3	15.5	14.8	14.2	13.7	13.3	12.8
	2x7 TFB	24.2	22.8	21.6	20.6	19.7	18.9	18.2	17.6	17.1
	2x9 TFB	32.8	30.9	29.3	27.9	26.7	25.7	24.8	23.9	23.2

NOTES:
1. THIS TABLE APPLIES TO BEARING & NON-BEARING WALLS FOR HORIZONTAL WIND LOADS IN DESIGN.
2. HEIGHTS MAY BE INTERPOLATED BUT NOT EXTRAPOLATED.

PURLIN SCHEDULE (EXPOSURE "B")										
	TRIBUTARY WIDTH	4.0	4.5	5.0	5.5	6.0	6.5	7.0	7.5	8.0
MEMBER		EFFECTIVE SPAN (FT.)								
	2x2x0.044	8.0	8.0	8.0	8.0	8.0	8.0	7.9	7.7	7.5
	2x3x0.050	13.7	13.1	12.8	12.4	12.0	11.7	11.4	11.1	10.8
	2x3x0.060	15.3	14.7	14.3	13.8	13.4	12.9	12.4	11.9	11.4
	2x4x0.050	17.0	17.3	16.8	16.1	15.8	15.1	14.4	13.7	13.0

NOTES:
1. SPANS MAY BE INTERPOLATED BUT NOT EXTRAPOLATED.

CARRY BEAM SCHEDULE FOR SCREEN ENCLOSURE ROOFS (EXPOSURE "B")											
	TRIBUTARY WIDTH	8.0	12.0	20.0	24.0	30.0	34.0	40.0	44.0	48.0	52.0
COLUMN MEMBER TYPE		EFFECTIVE SPAN (FT.)									
	2x4 SMB	16.4	14.1	11.7	10.9	9.6	9.1	8.4	8.0	7.7	7.3
	2x5 SMB	20.1	17.4	14.7	13.7	12.3	11.5	10.7	10.2	9.8	9.3
	2x6 SMB	22.4	20.4	17.2	15.8	14.1	13.2	12.2	11.7	11.2	10.7
	2x7 SMB	25.4	23.4	19.5	18.3	16.4	15.3	14.2	13.5	12.9	12.4
	2x8 SMB	31.6	28.7	24.1	22.7	20.9	19.6	18.2	17.3	16.5	15.9
	2x9 SMB	34.3	31.6	26.6	25.0	22.7	21.3	19.7	18.8	18.1	17.3
	2x10 SMB	42.7	39.3	33.0	31.0	28.6	27.5	25.9	24.7	23.6	22.7
	2x5 TFB	24.1	22.2	18.5	17.4	15.6	14.5	13.5	12.8	12.3	11.8
	2x7 TFB	30.0	27.3	22.9	21.6	19.9	18.6	17.3	16.4	15.7	15.1
	2x9 TFB	36.3	33.4	28.1	26.4	24.3	23.4	22.0	21.0	20.1	19.3

NOTES:
1. SPANS MAY BE INTERPOLATED BUT NOT EXTRAPOLATED.

CHAIR RAIL SCHEDULE (EXPOSURE "B")						
	TRIBUTARY WIDTH	4.0	5.0	6.0	7.0	8.0
MEMBER		EFFECTIVE SPAN (FT.)				
	2x2x0.045	8.5	7.8	7.6	7.1	6.8
	2x3x0.050	11.0	10.4	9.7	9.1	8.7
	2x3x0.060	12.7	11.8	11.1	10.6	10.1
	2x4x0.050	13.6	12.7	11.7	11.2	10.5
	2x5x0.050	15.0	13.4	12.2	11.3	10.6

BEAM SCHEDULE (EXPOSURE "B")						
	TRIBUTARY WIDTH	4.0	5.0	6.0	7.0	8.0
BEAM MEMBER TYPE		EFFECTIVE SPAN (FT.)				
	2x4 SMB	20.4	18.9	17.3	16.0	14.9
	2x5 SMB	27.3	24.4	22.3	20.6	19.3
	2x6 SMB	31.8	28.9	26.4	24.4	22.8
	2x7 SMB	36.2	33.3	30.4	28.1	26.3
	2x8 SMB	47.1	43.7	41.2	39.1	37.3
	2x9 SMB	55.9	51.9	48.8	46.4	44.4
	2x10 SMB	60.0	59.9	56.4	53.5	51.2
	2x5 TFB	28.5	26.0	23.7	22.0	20.5
	2x7 TFB	38.5	35.7	33.4	30.9	28.9
	2x9 TFB	51.1	47.5	44.7	42.4	40.6

NOTES:
1. SPANS MAY BE INTERPOLATED BUT NOT EXTRAPOLATED.
2. FOR ANYTHING SMALL THAN A 2X4 (I.E. A 2X2 OF 2X3) USE THE PURLIN TABLE TO DETERMINE SPANS.

KNEE BRACE SCHEDULE (EXPOSURE "B")		
SIZE	LENGTH	QTY. PER FLANGE
2x2x0.044	UP TO 2'	2
2x3x0.050	2' TO 4'	3
2x4x0.050	4' TO 6'	4
2x4x0.048 x 0.100 SMB	6' TO 7'	4
2x6x0.050 x 0.120 SMB	7' TO 8'	6

NOTES:
1. BEAM MEMBER SIZE IS CHOSEN BY THE SPAN BETWEEN THE KNEE BRACES, TYPICALLY (U.N.O.)
2. IF THERE ARE NO KNEE BRACES THEN THE SPAN IS CHOSEN BY THE DISTANCE OF THE BEAM LENGTH MINUS HALF THE DEPTH OF EACH UPRIGHT. (i.e. - BEAM LENGTH - (HALF DEPTH OF UPRIGHT 1 + HALF DEPTH OF UPRIGHT 2).
3. KNEE BRACE LENGTH SHALL NOT EXCEED WHAT IS GIVEN IN THE KNEE BRACE SCHEDULE (i.e. 8' IN HORIZONTAL OR VERTICAL DISTANCE) UNLESS THE SITE-SPECIFIC LAYOUT HAS BEEN APPROVED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER.
4. UPRIGHT MEMBER SIZE IS CHOSEN BY THE SPAN FROM THE GROUND TO THE BOTTOM OF THE KNEE BRACE
5. CHAIR RAILS THAT ARE ATTACHED TO UPRIGHTS THROUGH THE WEB OF THE UPRIGHT INTO EVERY AVAILABLE SCREW BOSS OF THE CHAIR RAIL, MAY BE USED AS GUARD RAILS IN SCREEN ENCLOSURES PROVIDED THAT THEY ARE SET AT A HEIGHT OF BETWEEN 18" & 48" AND THAT THEY SPAN NO GREATER THAN WHAT THE CHAIR RAIL SCHEDULE' IDENTIFIES FOR EACH RESPECTIVE MEMBER.

- THE SPAN TABLES SHOWN ARE CALCULATED FOR WIND EXPOSURE "B" TYPE TERRAIN. IF EXPOSURE "C" IS REQUIRED, THEN MULTIPLY THE FIGURES IN THE TABLE BY *0.82*

General Structural Notes

GENERAL DRAWING NOTES:

- DO NOT SCALE DRAWINGS, USE DIMENSIONS PROVIDED, TYPICALLY. IN THE CASE OF DIMENSIONAL CONFLICT, ARCHITECTURAL DIMENSIONS GOVERN OVER STRUCTURAL DIMENSIONS, TYPICALLY.
- CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS w/ THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, & CIVIL DOCUMENTS. SC&D SHALL BE NOTIFIED IN WRITING OF ANY DISCREPANCY OR OMISSION.
- THE STRUCTURE IS STABLE ONLY IN IT'S COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, & INSTALLED BY THE CONTRACTOR, TYPICALLY.

00200 BUILDING CODES:

- FLORIDA BUILDING CODE - 2010 EDITION
- ASCE/SEI 7-10: MINIMUM DESIGN LOADS FOR BUILDINGS & OTHER STRUCTURES

00500 STRUCTURAL DESIGN CRITERIA:

THE STRUCTURE HAS BEEN DESIGNED IN ACCORD WITH THE BUILDING CODE AND/OR MORE RESTRICTIVE REQUIREMENTS FOR LOADS AS GIVEN BELOW UNLESS SPECIFIC AREAS OF THE DRAWING SPECIFICALLY CALL FOR DIFFERENT LOADING CRITERIA.

GRAVITY LOADING IS TO BE UNIFORM (U.N.O.)
- ROOFS: 20 PSF (REDUCIBLE)

WIND LOADING PER FLORIDA BUILDING CODE & ASCE/SEI 7
- BASIC ULTIMATE WIND SPEED (V_{ULT}) = 160 MPH (3 SECOND GUST)
- BASIC NOMINAL WIND SPEED (V_{ASD}) = 124 MPH (3 SECOND GUST)
- EXPOSURE CATEGORY = "B"
- RISK CATEGORY = "II"
- INTERNAL PRESSURE COEFFICIENTS = N/A (OPEN STRUCTURE)
- COMPONENT & CLADDING PRESSURES = ±18.6 PSF (NOMINAL)

SCREEN FRAME WALL w/ GUARDRAIL:
- GUARDRAIL: 20 LB/FT UNIFORM LOAD IN ANY DIRECTION or 200 LB. CONCENTRATED LOAD (WHICHEVER IS GREATER)

01051 DRAWING DIMENSIONS AND COORDINATION:

DIMENSIONAL INFORMATION, PRICING, ALL DETAILS AND CONSTRUCTION SHALL BE BASED ON THE ENTIRE SET OF CONTRACT DOCUMENTS. COORDINATE THE REQUIREMENTS OF ALL PROFESSIONALS. USE INFORMATION FROM APPROVED SHOP DRAWINGS TO SUPPLEMENT CONTRACT DOCUMENTS WHERE NECESSARY. REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO PROCEEDING.

01100 SCOPE OF SERVICE:

STRUCTURAL CONCEPTS & DESIGN, LLC AS THE STRUCTURAL ENGINEER OF RECORD HAS DESIGNED AND IS RESPONSIBLE FOR ONLY THE SPECIFIC STRUCTURAL COMPONENTS SHOWN IN THIS SET OF STRUCTURAL CONSTRUCTION DOCUMENTS. IF A SPECIALTY ENGINEER, AS DEFINED BY THE DEPARTMENT OF PROFESSIONAL REGULATION, IS REQUIRED, HIS SERVICES MUST COMPLY WITH THE SCOPE OF SERVICES AS OUTLINED IN THE PROJECT CONSTRUCTION DOCUMENTS.

02000 FOUNDATIONS:

GEOTECHNICAL DATA AND RECOMMENDATIONS HAVE BEEN NOT BEEN PROVIDED AT THE TIME THESE DRAWINGS WERE ISSUED. BASED ON SIMILAR PROJECTS IN THE AREA, THE FOLLOWING FOUNDATION SYSTEM WILL BE USED:

- SHALLOW FOOTINGS w/ ALLOWABLE BEARING = 2,000 PSF* (* TO BE VERIFIED BY GEOTECHNICAL ENGINEER PRIOR TO CONSTRUCTION)

02200 EARTHWORK:

CONTRACTOR SHALL DEWATER SITE AS NECESSARY, SO THAT ALL CONCRETE CAN BE PLACED IN THE DRY. ALL BACKFILL SHALL BE ACCOMPLISHED USING MATERIAL CONSISTING OF CRUSHED STONE AND/OR MATERIAL APPROVED BY THE GEOTECHNICAL ENGINEER. THE BACKFILL SHALL BE COMPACTED TO 95% OF MAXIMUM DENSITY AS DETERMINED BY ASTM D-1557. NO BACKFILL MATERIAL SHALL BE PLACED AGAINST WALLS WHICH DO NOT HAVE PERMANENT FLOORS AT THE TOP AND BOTTOM WITHOUT PROVISIONS FOR ADEQUATE TEMPORARY BRACING OF THOSE WALLS. PROVIDE ADEQUATE EXCAVATION BRACING IN ACCORD WITH GEOTECHNICAL ENGINEER RECOMMENDATIONS TO MAINTAIN EXISTING FOOTINGS, UTILITIES, AND OTHER IMPROVEMENTS IN A SAFE CONDITION.

03100 FORMWORK:

CONTRACTOR SHALL DESIGN AND ERECT FORMWORK IN STRICT COMPLIANCE WITH ACI 347. SEE TYPICAL DETAILS FOR CAMBER REQUIREMENTS. CONTRACTOR SHALL COORDINATE ALL OPENINGS AS REQUIRED FOR OTHER TRADES. OPENINGS WHERE SHOWN ON THE STRUCTURAL DRAWINGS ARE TO IDENTIFY DESIGN INTENT ONLY. THE SPECIFIC DIMENSIONS AND LOCATIONS SHALL BE FURNISHED OR CONFIRMED BY THE TRADE REQUIRING THE OPENING. PROVIDE CHAMFERS AT ALL CORNERS IN CONCRETE MEMBERS EXPOSED TO VIEW. FORMWORK TO REMAIN IN PLACE UNTIL CONCRETE HAS ATTAINED ENOUGH STRENGTH TO SUPPORT ALL DEAD LOADS PLUS A MINIMUM OF 50 PSF OF ADDITIONAL CONSTRUCTION LOAD. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

03200 CONCRETE REINFORCEMENT:

WORK SHALL BE IN ACCORD WITH ACI 318-02, ACI 318R-02, ACI 315-99, ACI 318-02, CRSI "MANUAL OF STANDARD PRACTICE" 2001, CRSI "PLACING REINFORCING BARS" 1997, WIRE REINFORCEMENT INSTITUTE "MANUAL OF STANDARD PRACTICE-STRUCTURAL WELDED WIRE REINFORCEMENT", 2001. BARS SHALL CONFORM TO ASTM SPECIFICATION A615(S1), GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185. CONCRETE COVER REQUIRED AS FOLLOWS:

A. CAST AGAINST AND EXPOSED TO EARTH: 3"
B. FORMED, EXPOSED TO EARTH OR WEATHER: 2" (#6 & LARGER); 1½" (#5 & SMALLER)
C. SLABS AND WALLS:
   NO EARTH OR WEATHER EXPOSURE ¾" (#11 & SMALLER)
   3 HOUR FIRE RATING AND LESS ¾"

LAP SPLICE LENGTHS SHALL BE AS FOLLOWS:
1. ALL LAP SPLICES SHALL BE TENSION CLASS "B" UNLESS OTHER LAP CONDITIONS ARE SPECIFICALLY SHOWN ON THE DRAWINGS.
2. SPLICE LENGTHS SHALL BE SHOWN ON SHOP DRAWINGS.
3. USE GENERAL HOOK BAR DEVELOPMENT LENGTHS UNLESS SPECIAL CONFINEMENT CONDITIONS ARE SATISFIED IN ACCORDANCE WITH ACI 318.

03300 CAST-IN-PLACE CONCRETE:

TO BE MIXED AND PLACED IN ACCORDANCE WITH ACI 301-99. ALL REINFORCED CONCRETE TO HAVE 28 DAY COMPRESSIVE STRENGTHS AS FOLLOWS. (ALL STRUCTURAL ELEMENTS SHALL BE F'c = 4,000 PSI UNLESS NOTED OTHERWISE).

- SLABS: F'c = 3,000 PSI
- FOUNDATIONS: F'c = 3,000 PSI

03600 GROUT:

GROUTING IS CLASSIFIED AS "PRECISION GROUTING" FOR SUPPORT OF OPERATING MACHINE BASES, EQUIPMENT SUBJECT TO THERMAL MOVEMENT, AND BASE PLATES, BEARING PLATES, AND EXPANSION BEARINGS EXCEEDING 8" IN LEAST DIMENSION. ALL OTHER GROUTING MAY BE "ORDINARY GROUTING". METALLIC AGGREGATE GROUT MAY BE USED ONLY IN INTERIOR APPLICATIONS NOT EXPOSED TO VIEW IN FINISHED BUILDING AREAS. USE ORDINARY CEMENT GROUT ONLY WHERE SPECIFICALLY NOTED AS "CEMENT GROUT" ON DETAILS. USE NON-SHRINK GROUT FOR ALL OTHER LOCATIONS. PRECISION GROUT SHALL CONFORM TO CRD-C621-80 WHEN MIXED TO FLUID CONSISTENCY OF 22 TO 25 SECONDS (FLOW CONE METHOD, CRD-C611-80). REQUIRED 28 DAY STRENGTHS SHALL BE AS FOLLOWS:

- CEMENT GROUT: 1,800 PSI
- NON-SHRINK GROUT: 5,000 PSI
- PRECISION GROUT: 6,500 PSI

04220 CONCRETE UNIT MASONRY:

ALL MASONRY CONSTRUCTION TO BE IN ACCORDANCE WITH "SPECIFICATION FOR CONCRETE MASONRY CONSTRUCTION", ACI 530.1-02 AND ALL APPLICABLE LOCAL BUILDING CODE PROVISIONS. ALL MASONRY WALLS TO BE CONSTRUCTED ENTIRELY OF UNITS CONFORMING TO ASTM C 90, AND REINFORCED WITH #9 GAUGE LADDER TYPE HORIZONTAL MASONRY REINFORCING LOCATED AT 16" O.C. ALL MASONRY TO BE LAID IN TYPE "S" MORTAR (1,800 PSI ON THE JOB) WITH FULL HEAD AND BED JOINTS. ALL MASONRY CONSTRUCTION TO BE EITHER BOUND BY TIE BEAM, TIE COLUMN MEMBERS OR TIED TO FRAME WITH 16 GAUGE CONTINUOUS DOVETAIL SLOT AND 12 GAUGE DOVETAIL ANCHOR SPACED @ 16" O.C. (TOP AND TWO VERTICAL SIDES).

04230 REINFORCED UNIT MASONRY:

ALL REINFORCED MASONRY CONSTRUCTION SHALL BE IN ACCORD WITH APPLICABLE PROVISIONS OF CONCRETE REINFORCEMENT, CAST-IN-PLACE CONCRETE, AND CONCRETE MASONRY. VERTICAL REINFORCING SHALL ANCHOR INTO SUPPORTING CONCRETE MEMBERS A CLASS "B" LAP LENGTH PLUS 3" OR FULL DEPTH PLUS A STANDARD HOOK. LAPS WITHIN REINFORCED MASONRY SHALL BE 48 BAR DIAMETERS. CONTRACTOR SHALL COORDINATE PLACING OF DOWELS TO ACCOMMODATE MODULE OF MASONRY UNITS. ALL VERTICAL CELLS AND BEAMS WITH REINFORCING SHALL BE FILLED WITH COARSE GROUT CONSISTING OF 3,000 PSI CONCRETE WITH #8 COARSE AGGREGATE. USE HIGH-SLUMP (SUPERPLASTICIZED) WHERE HEIGHT OF LIFT EXCEEDS 5'-0". WHERE HEIGHT OF OPEN CELL EXCEEDS 5'-0", USE HIGH-LIFT GROUTING TECHNIQUE WHICH REQUIRES A CLEAN-OUT OPENING AT THE BOTTOM OF ALL CELLS. ALL WALLS TO BE REINFORCED WITH #5 BARS SPACED AT 48" O.C. MAX. VERTICAL, TYP. (U.N.O.)

STRUCTURAL CONCEPTS & DESIGN
2989 WEST S.R. 434, SUITE 300 • LONGWOOD, FL 32779
FLORIDA C.O.A. NO. 27191
06/07/13
Robert C. Scroggins, P.E.
FL Registration No. 56158

To the best of the Design Engineer's knowledge, the plans and specifications for this project comply with the applicable minimum building codes as determined by the local authority in accordance with the Florida Statutes.

REV.	DATE	DESCRIPTION

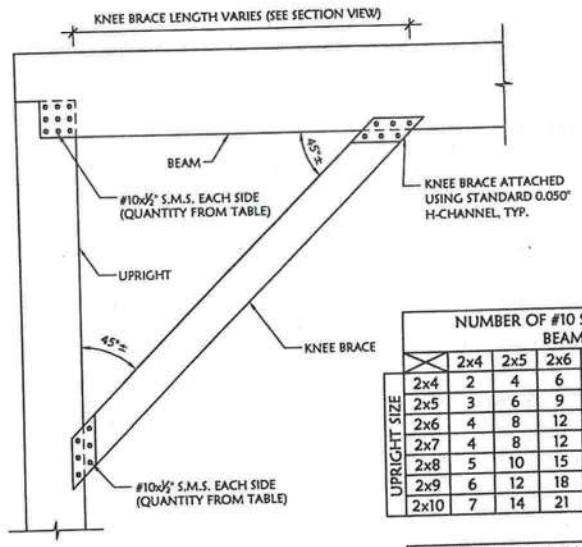
Client: Florida Pool Enclosures, Inc

Project: Residential Pool Screen Enclosure
________________, Florida

Structural Notes & Connection Detail

13405

Project No.:	#13-002.7
Drawn By:	TLW
Checked By:	RCS
Approved By:	RCS
Date:	06/07/13
Sheet	S-1.0

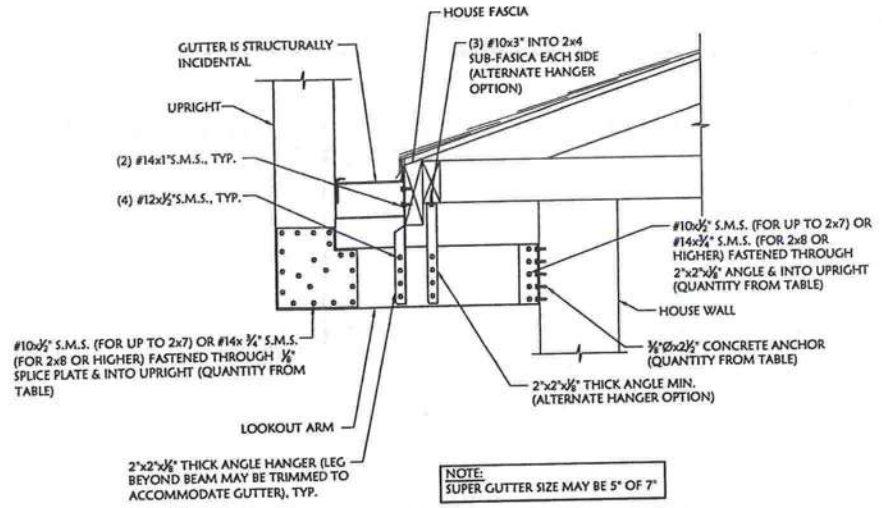


NUMBER OF #10 S.M.S. REQUIRED BEAM SIZE							
UPRIGHT SIZE	2x4	2x5	2x6	2x7	2x8	2x9	2x10
2x4	2	4	6	6	8	10	12
2x5	3	6	9	9	12	15	18
2x6	4	8	12	12	16	20	24
2x7	4	8	12	12	16	20	24
2x8	5	10	15	15	20	25	30
2x9	6	12	18	18	24	30	36
2x10	7	14	21	21	28	35	42

NUMBER OF #14 S.M.S. REQUIRED BEAM SIZE							
UPRIGHT SIZE	2x4	2x5	2x6	2x7	2x8	2x9	2x10
2x4	N/A	2	2	3	4	5	6
2x5	2	2	4	4	6	6	8
2x6	2	4	6	8	10	12	14
2x7	3	4	8	8	10	12	14
2x8	4	6	10	10	12	14	16
2x9	5	6	12	12	14	16	18
2x10	6	8	14	14	16	18	20

1 / S-2.1 Typical Beam to Column Connection w/ Knee Brace

SCALE: 1½" = 1'-0"



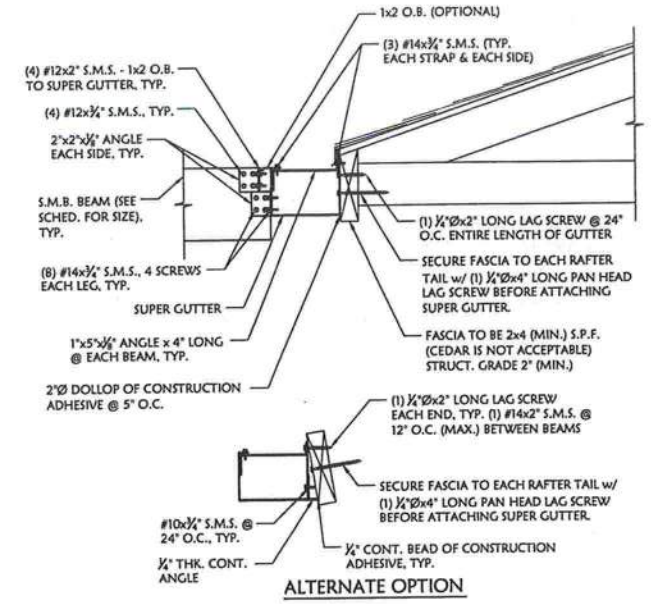
NOTE:
SUPER GUTTER SIZE MAY BE 5" OF 7"

NUMBER OF GUSSET PLATE SCREWS							
UPRIGHT SIZE	2x4	2x5	2x6	2x7	2x8	2x9	2x10
QTY. OF SCREWS	16	18	20	24	26	30	30

NUMBER OF FASTENERS TO ANGLE CLIP							
UPRIGHT SIZE	2x4	2x5	2x6	2x7	2x8	2x9	2x10
QTY. OF SCREWS	3	4	5	6	7	8	9

2 / S-2.1 Lookout Arm Connection Detail

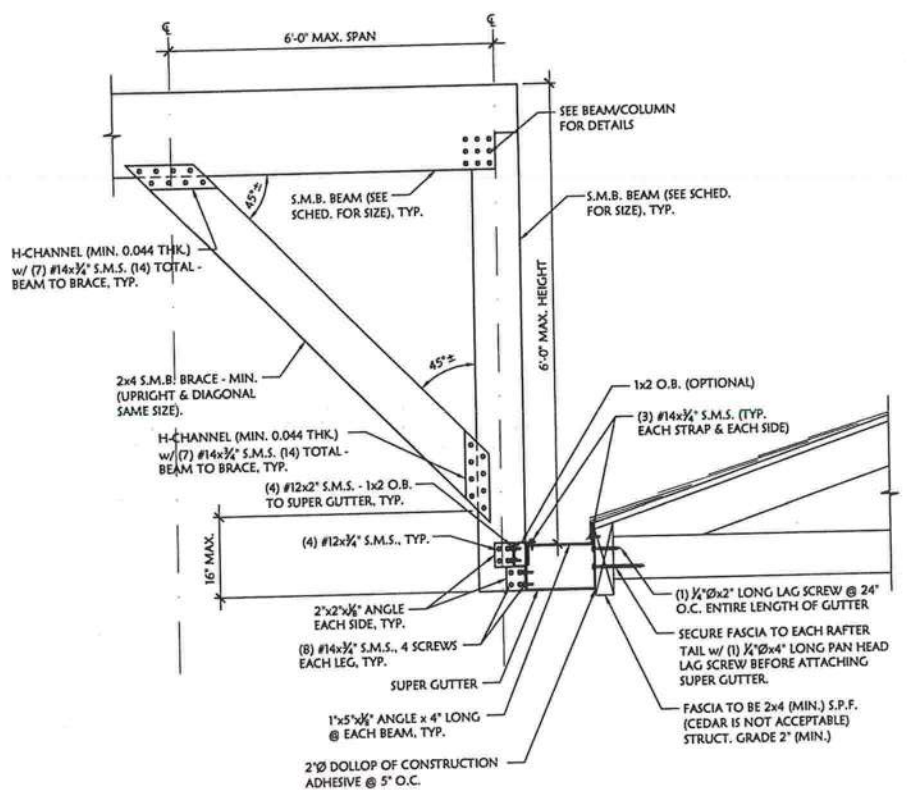
SCALE: 1½" = 1'-0"



ALTERNATE OPTION

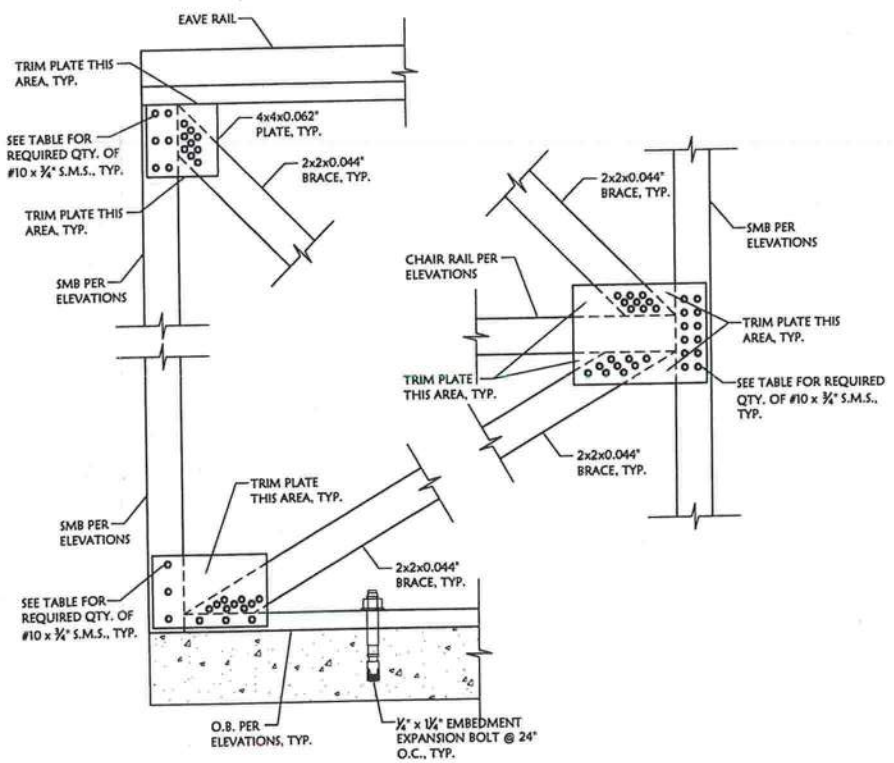
3 / S-2.1 Roof Beam to Super Gutter Connection

SCALE: 1½" = 1'-0"



4 / S-2.1 Transom Upright to Super Gutter Connection Detail

SCALE: 1½" = 1'-0"



5 / S-2.1 Typical K-Brace Connection Detail

SCALE: 1½" = 1'-0"

STRUCTURAL CONCEPTS & DESIGN
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FLORIDA C.O.A. NO. 27191

06/07/13
Robert C. Scroggins, P.E.
FL Registration No. 56158

To the best of the Design Engineer's knowledge, the plans and specifications for this project comply with the applicable minimum building codes as determined by the local authority in accordance with the Florida Statutes.

Structural Framing Sections & Details

Project: Residential Pool Screen Enclosure
, Florida

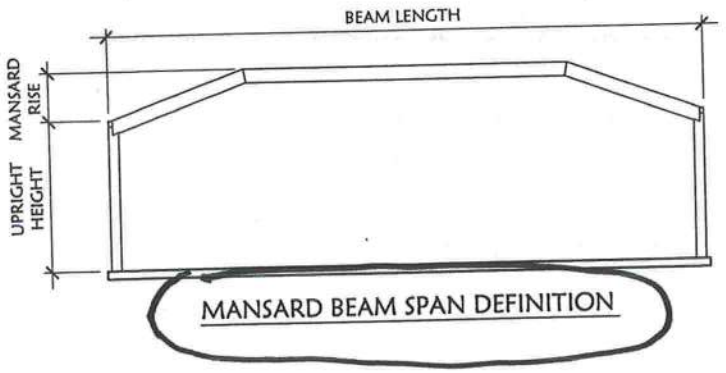
Client: Florida Pool Enclosures, Inc

REV.	DATE	DESCRIPTION

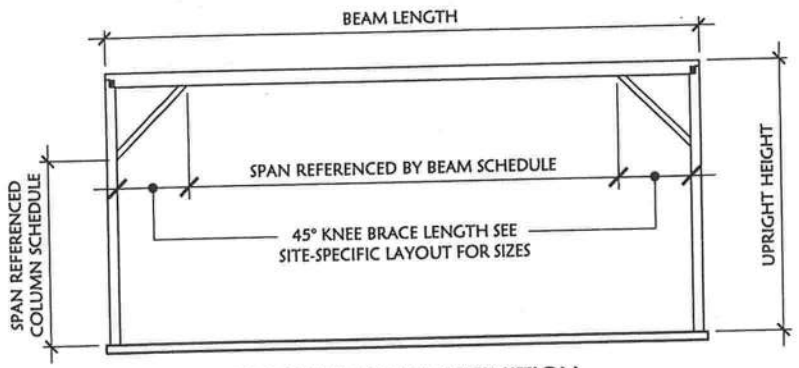
Project No.: #13-002.7
Drawn By: TLW
Checked By: RCS
Approved By: RCS
Date: 06/07/13

Sheet
S-2.1

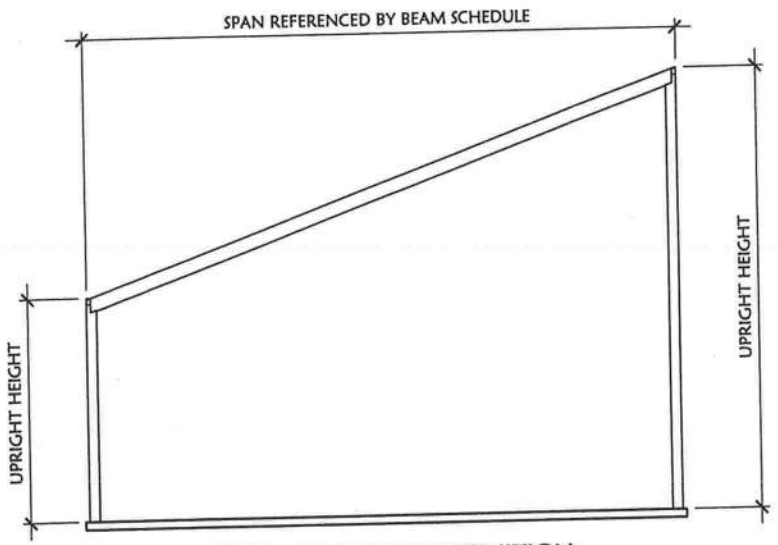
F:\Structural Server\Florida Pool Enclosures\2013 Projects\13-002 Residential Pool Screen Enclosures (FBC 2010) 06/07/13 TLW



MANSARD BEAM SPAN DEFINITION



FLAT BEAM SPAN DEFINITION



SLOPED BEAM SPAN DEFINITION

1 S-2.2 **Typical Beam Span Definitions**
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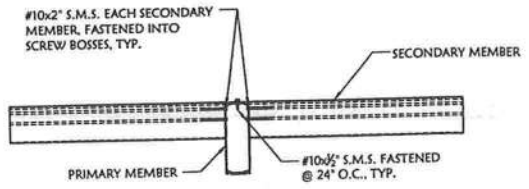
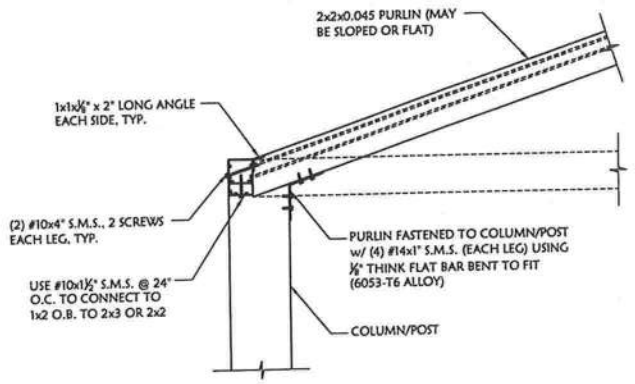


ELEVATION

2 S-2.2 **Typical Secondary Member to Primary Member Connection**
SCALE: 1½" = 1'-0"



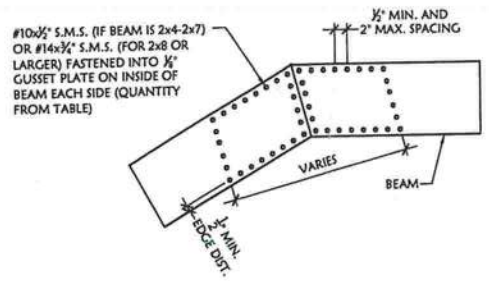
3 S-2.2 **Typical Purlin to Sidewall Post/Column Connection Detail**
SCALE: 1½" = 1'-0"

QUANTITY OF SCREWS REQUIRED

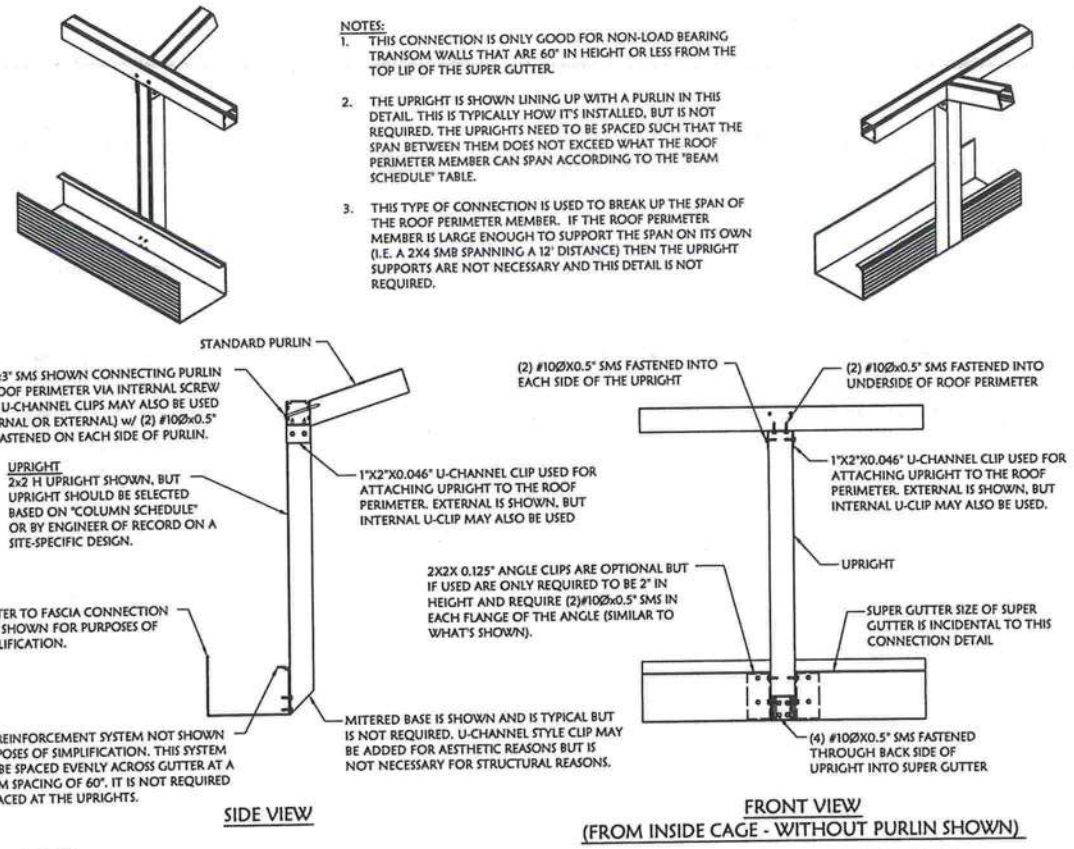
BEAM SIZE	# SCREWS
2x4-2x6	12
2x7	16
2x8	18
2x9	20
2x10	22

LENGTH OF SPLICE PLATE

BEAM SIZE	LENGTH
2x4	8 IN.
2x5	10 IN.
2x6	12 IN.
2x7	14 IN.
2x8	16 IN.
2x9	18 IN.
2x10	18 IN.



4 S-2.2 **Mansard Beam Splice Detail**
SCALE: 1½" = 1'-0"



5 S-2.2 **NON-LOAD BEARING TRANSOM WALL CONNECTION**
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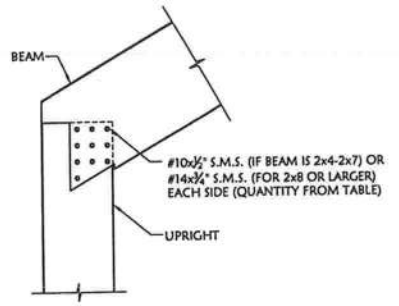

6 S-2.2 **Mansard Beam to Upright Connection Detail**
SCALE: 1½" = 1'-0"

QUANTITY OF #10 FASTENERS REQUIRED

BEAM SIZE \ UPRIGHT SIZE	2x4	2x5	2x6	2x7	2x8	2x9	2x10
2x4	N/A	N/A	N/A	N/A	N/A	N/A	N/A
2x5	3	4	4	4	4	4	4
2x6	5	7	8	9	9	9	9
2x7	7	10	12	14	14	14	14
2x8	9	13	16	18	18	19	20
2x9	11	16	20	22	22	25	27
2x10	13	19	24	27	27	31	34

QUANTITY OF #14 FASTENERS REQUIRED

BEAM SIZE \ UPRIGHT SIZE	2x4	2x5	2x6	2x7	2x8	2x9	2x10
2x4	N/A	N/A	N/A	N/A	N/A	N/A	N/A
2x5	3	3	3	3	3	3	3
2x6	5	6	6	7	6	6	7
2x7	7	9	9	11	10	12	12
2x8	7	10	9	12	11	13	14
2x9	9	13	12	15	15	17	19
2x10	11	16	16	19	19	22	25

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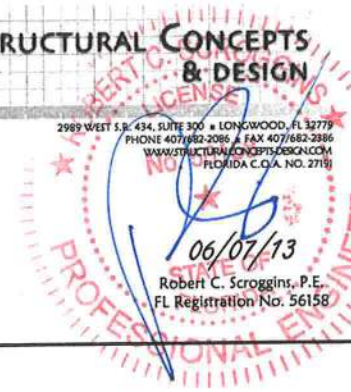

06/07/13
Robert C. Scroggins, P.E.
FL Registration No. 56158

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Structural Framing Sections & Details

Project: Residential Pool Screen Enclosure
, Florida

Client:

Florida Pool Enclosures, Inc
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REV.	DATE	DESCRIPTION

Project No.: #13-002.7
Drawn By: TLW
Checked By: RCS
Approved By: RCS
Date: 06/07/13

Sheet
S-2.2

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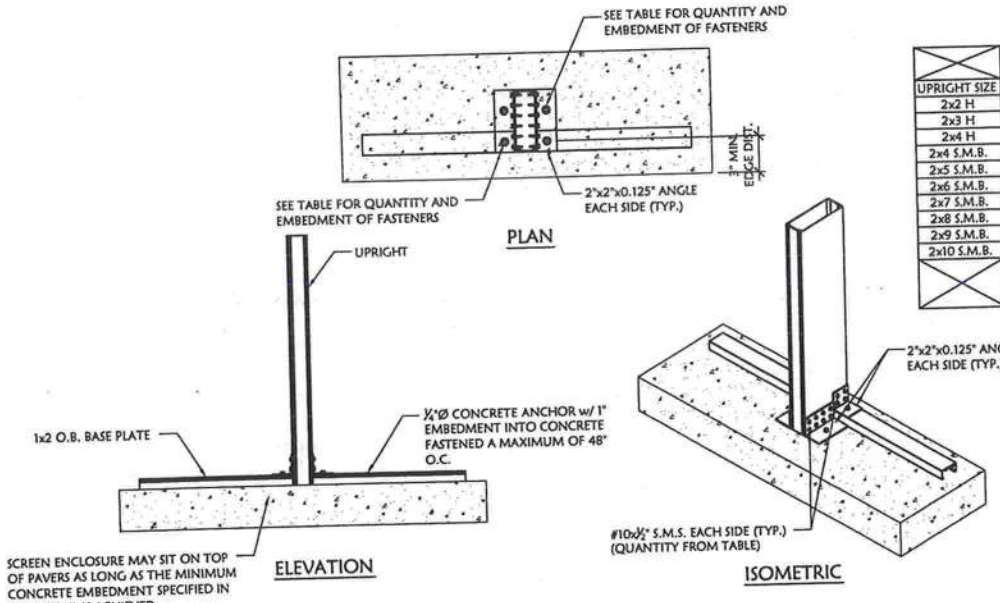
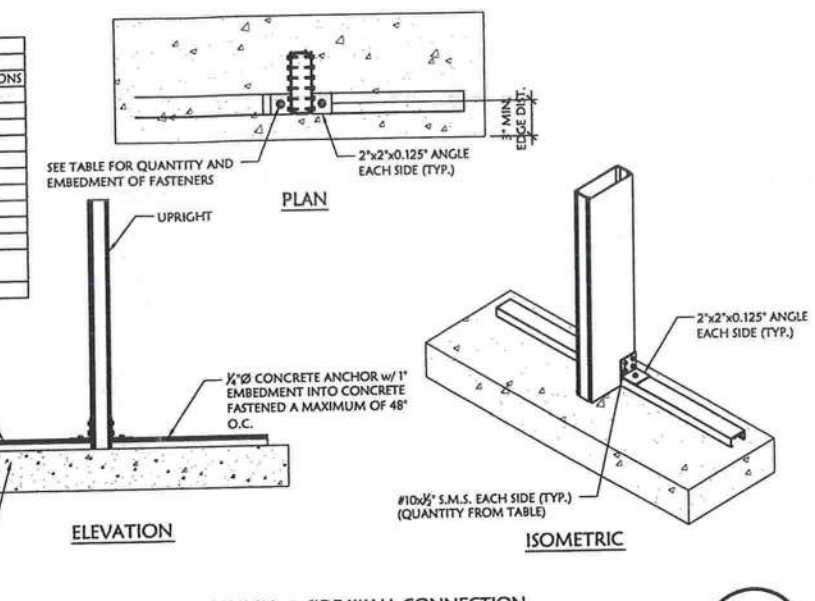


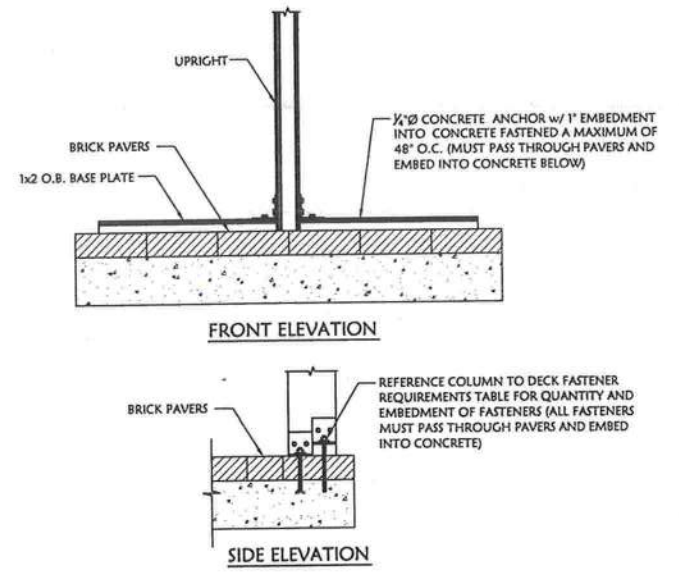
MAIN COLUMN UNDER BEAM CONNECTION

COLUMN TO DECK FASTENERS REQUIRED

	MIN. QUANTITY OF SPECIFIED FASTENERS			
	FASTENERS TO UPRIGHT		FASTENERS TO DECK	
UPRIGHT SIZE	#10 S.M.S.	#14 S.M.S.	1/4"Ø TAPSONS	3/8"Ø TAPCONS
2x2 H	2	2	1	1
2x3 H	3	2	1	1
2x4 H	4	4	2	1
2x4 S.M.B.	4	4	2	1
2x5 S.M.B.	5	4	2	1
2x6 S.M.B.	6	4	3	2
2x7 S.M.B.	7	6	3	2
2x8 S.M.B.	N/A	8	4	3
2x9 S.M.B.	N/A	9	4	3
2x10 S.M.B.	N/A	10	N/A	4
	N/A	N/A	1.25	1.25
	MIN. EMBEDMENT DEPTH (INCHES)			



COLUMN @ SIDE WALL CONNECTION

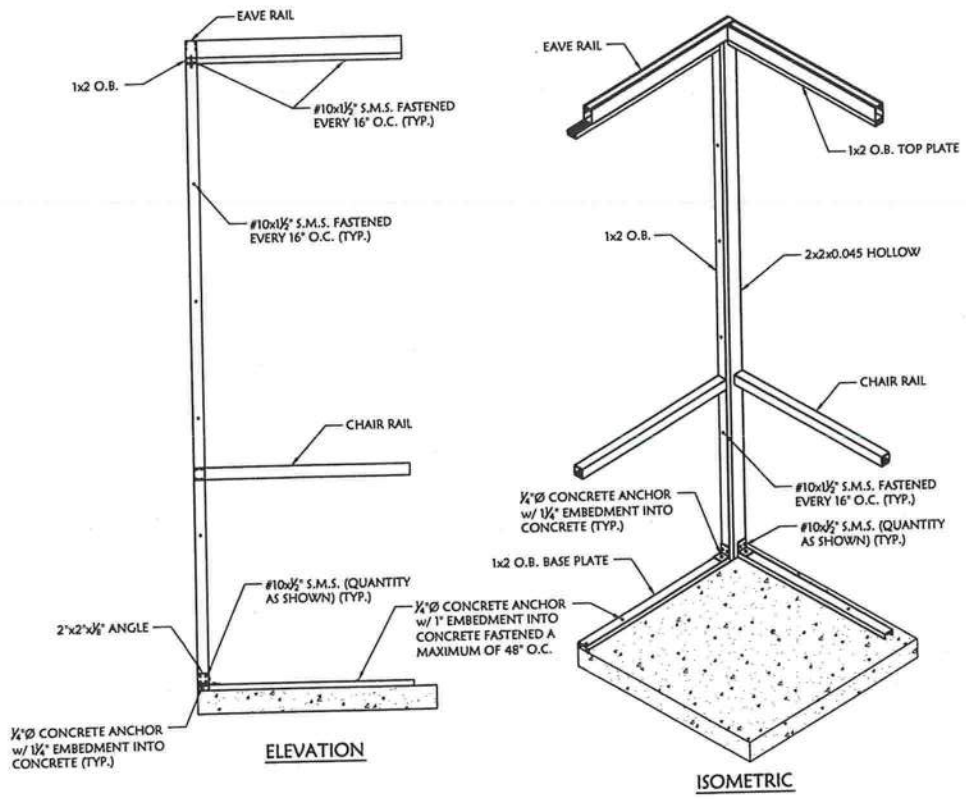


2 S-2.3 Column Connection to Concrete Deck w/ Pavers

SCALE: 1½" = 1'-0"

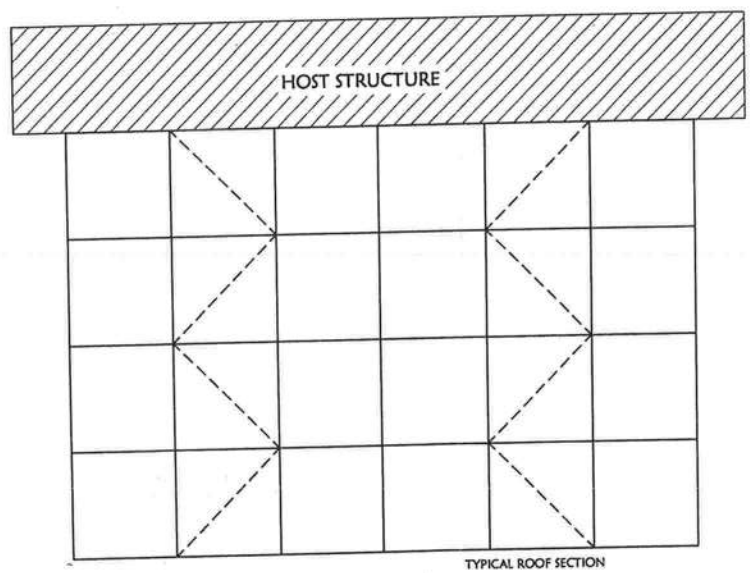
1 S-2.3 Typical Column to Concrete Connection Details

SCALE: 1½" = 1'-0"



3 S-2.3 Typical Front Wall to Side Wall Connections

N.T.S.

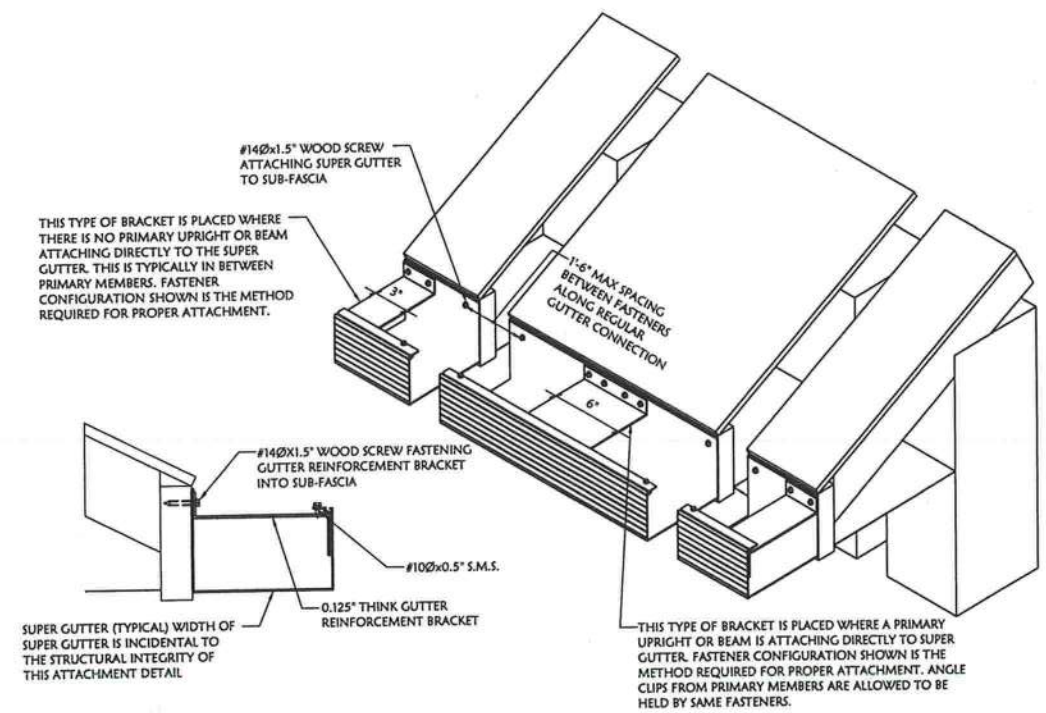


4 S-2.3 Diagonal Roof Bracing Plan

N.T.S.

NOTES:

1. ROOF BRACING MAY BE PLACED IN EITHER THE FIRST OR SECOND ROOF SECTION ON EACH SIDE OF THE ENCLOSURE
2. THE ORIENTATION (i.e. DIRECTION) OF THE ROOF BRACING IS INCIDENTAL TO STRUCTURAL INTEGRITY
3. IF THERE ARE AN ODD NUMBER OF PANELS, A BRACE IN THE CENTER MAY BE OMITTED
4. ANY ENCLOSURE THAT IS BRACED ON MORE THAN ONE SIDE BY THE HOST STRUCTURE DOES NOT REQUIRE ROOF BRACING



5 S-2.3 Typical Super Gutter Connection to Host Structure

SCALE: 1½" = 1'-0"

NOTES:

1. FASCIA SHOWN IS A PLUMB CUT FASCIA. IF THE FASCIA IS SQUARE CUT (PERPENDICULAR TO THE ROOF PITCH) THEN THE SUPER GUTTER MAY HAVE SOMETHING BEHIND IT (I.E. "L-ANGLE" OR WOOD WEDGE) TO KEEP IT PLUMB. THIS COMPONENT IS INCIDENTAL TO THE STRUCTURAL INTENTION OF THIS DESIGN.
2. FOR A RUN OF SUPER GUTTER THAT NON-LOAD BEARING, THE MAX SPACING OF THE 3" REINFORCEMENT BRACKETS IS 60".
3. FOR A RUN OF SUPER GUTTER THAT HAS A STRUCTURAL OR LOAD BEARING COMPONENT ATTACHING TO IT AND THERE IS A CONTINUOUS LOAD ACROSS THE SUPER GUTTER WITHOUT PRIMARY LOAD POINTS (I.E. BEAM OR UPRIGHT) THEN THE MAXIMUM SPACING BETWEEN THE 3" REINFORCEMENT BRACKETS IS 48".
4. GUTTER MAY BE ATTACHED TO A CONVENTIONAL WOOD FRAMED WALL IN THE SAME MANNER THAT IS SHOWN IN THIS DETAIL. IF THE WALL IS A C.M.U. THEN REPLACE THE #14Ø WOOD SCREWS WITH 0.25"ØX1.25" TAPCONS (OR EQUIVALENT). THIS CONNECTION SHALL ALSO REMAIN THE SAME FOR A SUPER GUTTER ATTACHING TO A CARRY BEAM WHERE THE WOOD LAG SCREWS ARE X 0.75" S.M.S. REPLACED BY #10Ø.
5. WHEN THE SUPER GUTTER IS BEING USED FOR STRUCTURAL PURPOSES (I.E. STRUCTURAL SUPPORT MEMBERS ATTACH TO IT) THEN THE FASTENERS THAT ATTACH TO THE FASCIA MUST PENETRATE INTO THE SUB-FASCIA AT A MINIMUM OF 1 IN. EMBEDMENT.

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ROBERT C. SCROGGINS LICENSE STATE OF FLORIDA PROFESSIONAL ENGINEER

06/07/13
Robert C. Scroggins, P.E.
FL Registration No. 56158

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, Florida

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REV.	DATE	DESCRIPTION

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Sheet
S-2.3

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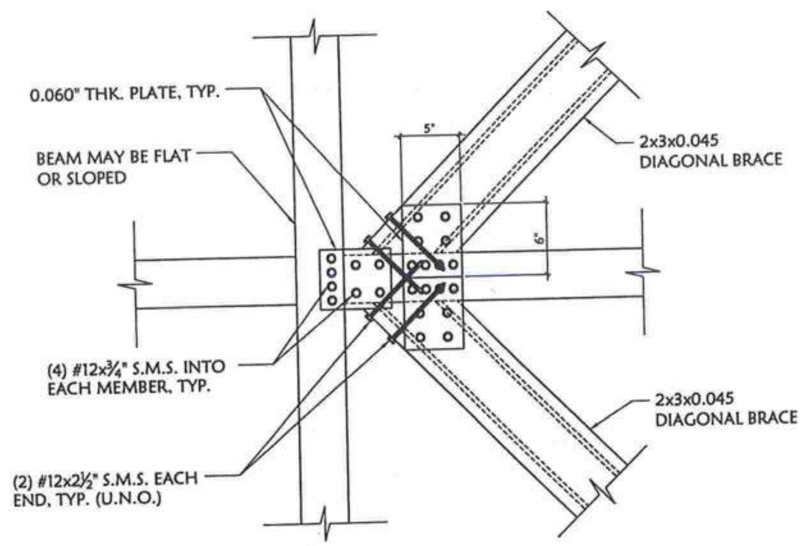
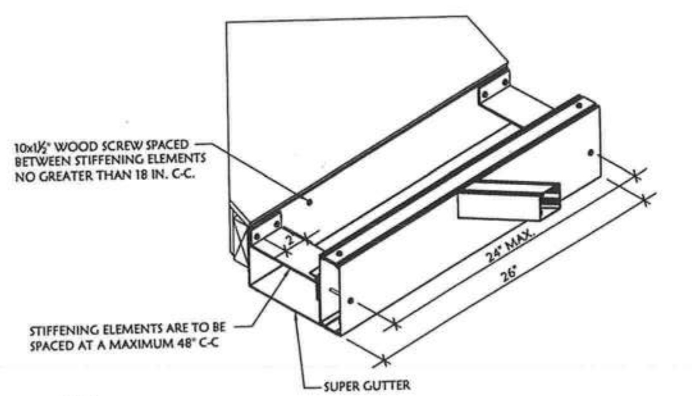
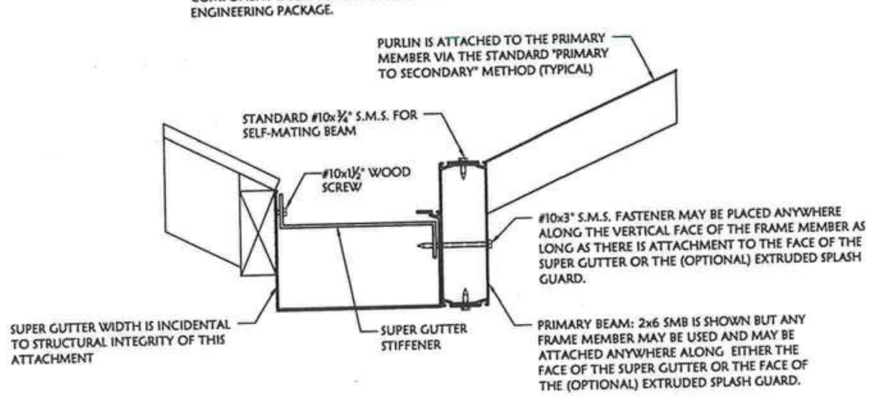


1 / S-2.4 Typical Roof Brace to Beam Connection Detail



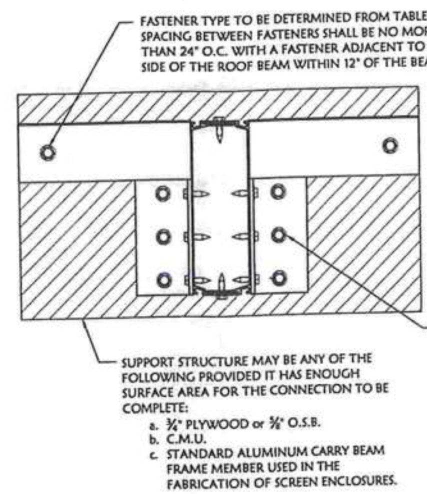
NOTES:

1. 2"x2"x0.125" ANGLE MAY BE USED IN LIEU OF (OR IN ADDITION TO) THE #10x3" S.M.S. ATTACHMENT AND MAY BE ON TOP OR BOTTOM OF THE ATTACHING FRAME MEMBER. THE SPACING AND TYPE OF FASTENERS WILL REMAIN THE SAME AS STATED IN THIS DETAIL. EACH FASTENER WILL BE ON THE SAME PLANE FOR EACH OPPOSING FACE (FLANGE) OF THE 2x2 ANGLE.
2. SUPER GUTTER IS SHOWN AS THE COMPONENT BEING FASTENED TO, BUT THIS DETAIL CAN BE USED IN THE CASE WHERE SUPER GUTTER IS SUBSTITUTED FOR A DIFFERENT ALUMINUM STRUCTURAL COMPONENT (I.E. SELF-MATING BEAM ETC.) PROVIDED THE PRIMARY STRUCTURAL COMPONENT IS SUPPORTED PROPERLY IN ACCORDANCE WITH THE DETAILS IN THE SEALED ENGINEERING PACKAGE.



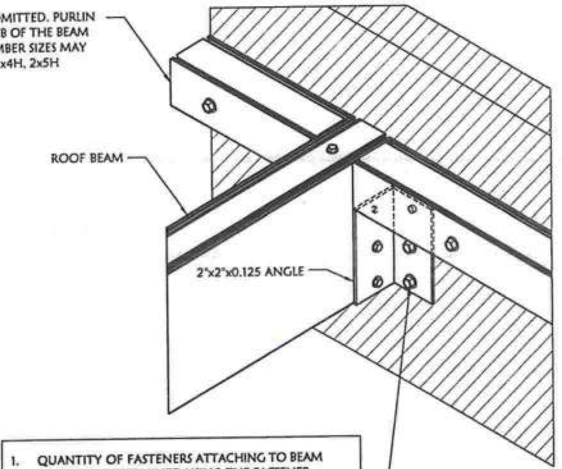
2 / S-2.4 Parallel Frame Member to Super Gutter Connection

N.T.S.



1. QUANTITY OF FASTENERS ATTACHING TO BEAM ARE TO BE DETERMINED USING THE FASTENER TABLE. TYPE OF FASTENERS WILL BE #10 S.M.S. FOR BEAM SIZES BETWEEN 2X4 - 2X7 & #14 S.M.S. FOR BEAM SIZES 2X8 - 2X10.
2. FASTENERS SHALL NOT BE LESS THAN 0.75" CENTER-TO-CENTER FROM EACH OTHER NOR SHALL THEY BE GREATER THAN 2" CENTER-TO-CENTER FROM EACH OTHER.
3. FASTENERS MAY BE ARRANGED IN ANY PATTERN PROVIDED THE MINIMUM QTY FROM THE TABLE IS MET ALONG WITH THE ABOVE MENTIONED SPACING REQUIREMENTS.

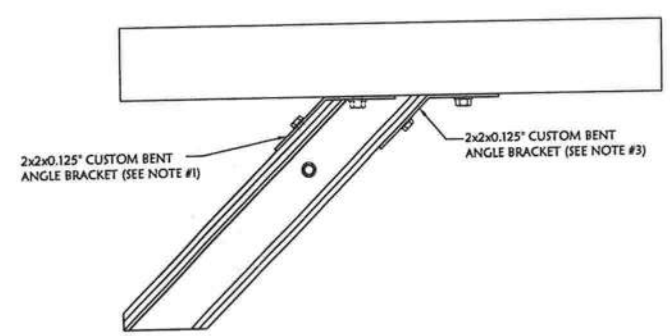
NOTE:
U-CHANNEL MAY BE USED IN LIEU OF 2x2 ANGLE CLIPS ON EITHER SIDE OF THE BEAM AS LONG AS THE THICKNESS OF THE U-CHANNEL BEING USED IS GREATER THAN OR EQUAL TO THE THICKNESS OF THE BEAM TO WHICH IT IS ATTACHING.



1. QUANTITY OF FASTENERS ATTACHING TO BEAM ARE TO BE DETERMINED USING THE FASTENER TABLE. TYPE OF FASTENERS WILL BE #10 S.M.S. FOR BEAM SIZES BETWEEN 2X4 - 2X7 & #14 S.M.S. FOR BEAM SIZES 2X8 - 2X10.
2. FASTENERS SHALL NOT BE LESS THAN 0.75" CENTER-TO-CENTER FROM EACH OTHER NOR SHALL THEY BE GREATER THAN 2" CENTER-TO-CENTER FROM EACH OTHER.
3. FASTENERS MAY BE ARRANGED IN ANY PATTERN PROVIDED THE MINIMUM QTY FROM THE TABLE IS MET ALONG WITH THE ABOVE MENTIONED SPACING REQUIREMENTS.

3 / S-2.4 Straight Beam to Support Structure Connection Detail

N.T.S.



4 / S-2.4 Angled Beam to Support Structure Connection Detail

N.T.S.

NOTES:

1. ACUTE SIDE OF BEAM MAY BE NOTCHED TO ALLOW FOR THE 2X2X0.125" CUSTOM BENT ANGLE BRACKET TO SLIDE IN BETWEEN THE BEAM AND SUPPORT STRUCTURE. THE FABRICATION METHOD TO ACHIEVE THIS WOULD BE TO SIMPLY ATTACH THE ANGLE BRACKET TO THE BEAM PRIOR TO INSTALLING THE BEAM HALF-SHELL, PROVIDED THE ACUTE SIDE OF THE BEAM IS INSTALLED FIRST. THIS WOULD ALLOW FOR THE FASTENERS TO BE INSTALLED INTO THE SUPPORT STRUCTURE PRIOR TO THE INSTALLATION OF THE OBTUSE SIDE OF THE BEAM.
2. IF THE FABRICATION METHOD SHOWN HERE CANNOT BE ACHIEVED, THEN A SECONDARY 2x2x0.125" ANGLE MUST BE INSTALLED BENEATH THE BEAM. TYPE AND QTY OF FASTENERS SHOULD REFERENCE THE ROW LABLED 2x2 H IN THE "FASTENER REQUIREMENTS" TABLE.
3. A CUSTOM BENT 2x2x0.125" ANGLE MUST ALWAYS BE INSTALLED ON THE OBTUSE SIDE OF THE BEAM. 4) ALL OTHER NOTES AND SPECIFICATIONS FROM THE "STRAIGHT BEAM TO SUPPORT STRUCTURE" DETAIL APPLY TO THIS DETAIL.

BEAM TO HOIST STRUCTURE FASTENER REQUIREMENTS

	MIN. QUANTITY OF SPECIFIED FASTENERS					
	FASTENERS TO BEAM		FASTENERS TO CMU		FASTENERS TO WOOD	
UPRIGHT SIZE	#10 S.M.S.	#14 S.M.S.	1/4"Ø TAPCONS	3/8"Ø TAPCONS	#10 WOOD SCREW	#14 WOOD SCREW
2x2 H	2	2	1	1	2	1
2x3 H	3	3	1	1	3	1
2x4 H	4	4	1	1	4	2
2x4 S.M.B.	4	4	1	1	4	2
2x5 S.M.B.	5	5	2	1	5	3
2x6 S.M.B.	6	6	3	2	6	4
2x7 S.M.B.	7	7	4	2	7	5
2x8 S.M.B.	8	8	4	3	8	6
2x9 S.M.B.	9	9	5	4	9	7
2x10 S.M.B.	10	10	6	5	10	8
	N/A	N/A	1.25	1.25	1.0	1.0
	MIN. EMBEDMENT DEPTH (INCHES)					

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________________, Florida

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Approved By:	RCS
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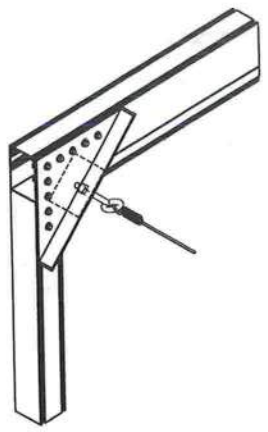
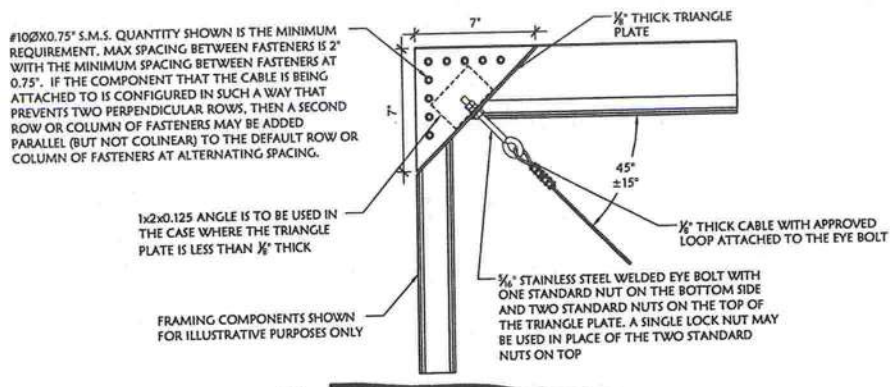
Sheet S-2.4





TRIANGLE PLATE TOP ATTACHMENT

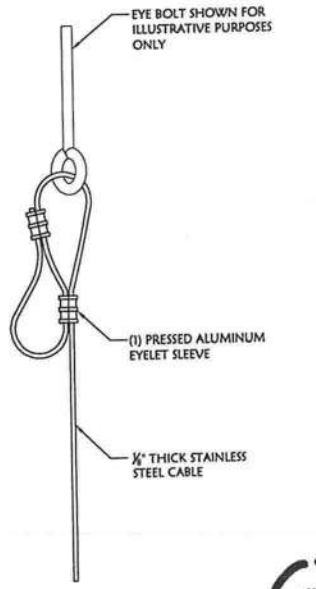
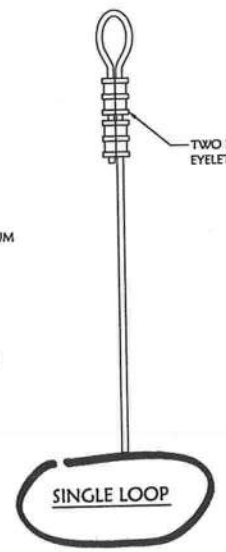
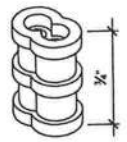


FIGURE 8 LOOP



SINGLE LOOP



PRESSED EYELET SLEEVE
(SHOWN AFTER BEING MECHANICALLY PRESSED

CABLE CONNECTION NOTES:

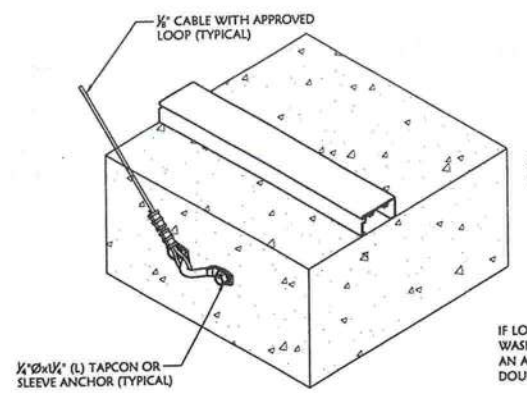
1. TO DETERMINE THE QUANTITY OF CABLES NEEDED FOR A SCREEN WALL BRACED ON ONE SIDE BY A HOST STRUCTURE TAKE THE TOTAL SQUARE FOOTAGE OF THE BRACED WALL AND DIVIDE BY 250. ROUND THE CALCULATED VALUE TO THE CLOSEST WHOLE NUMBER AND SUBTRACT ONE. IT IS INTENDED TO NOT HAVE A CABLE ON A WALL THAT IS LESS THAN 250 S.F.

   EXAMPLE: 430 S.F. / 250 = 1.72 --> ROUNDS TO 2 --> 2-1 = 1 CABLE
   EXAMPLE: 230 S.F. / 250 = 0.92 --> ROUNDS TO 1 --> 1-1 = 0 CABLES

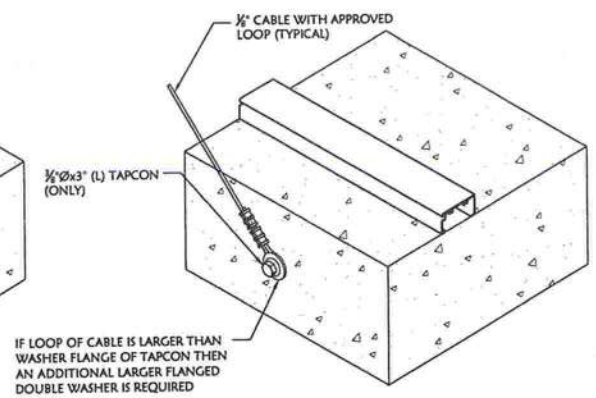
2. TO DETERMINE THE QUANTITY OF CABLES NEEDED FOR AN UNBRACED SCREEN WALL, TAKE THE TOTAL SQUARE FOOTAGE OF THE UNBRACED WALL AND DIVIDE BY 250. ROUND THE CALCULATED VALUE TO THE CLOSEST WHOLE NUMBER AND MULTIPLY BY 2. IT IS REQUIRED THAT AN UNBRACED SCREEN WALL HAVE AN EQUAL AMOUNT OF CABLES OPPOSING EACH OTHER.

   EXAMPLE: 535 S.F. / 250 = 2.14 --> ROUNDS TO 2 --> 2*2 = 4 CABLES (2 PAIRS OF OPPOSING CABLES)
   EXAMPLE: 780 S.F. / 250 = 3.12 --> ROUNDS TO 3 --> 3*2 = 6 CABLES (3 PAIRS OF OPPOSING CABLES)

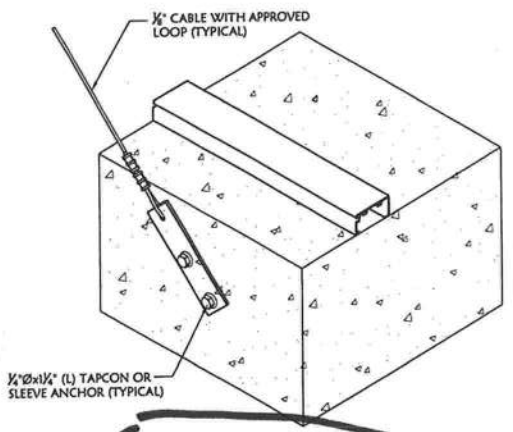
3. ANY ONE OF THE APPROVED CABLE-TO-DECK ATTACHMENTS SHOWN MAY BE USED. FIELD CONDITIONS WILL DICTATE THE TYPE OF ATTACHMENT USED AND MAY CHANGE DYNAMICALLY FROM THE ORIGINAL DESIGN INTENT. THIS HAS NO BEARING ON THE DESIGN OR THE STRUCTURAL INTEGRITY OF THE ENCLOSURE AND THEREFORE IS ALLOWED TO BE DONE, AS LONG AS ONE OF THE APPROVED ATTACHMENT DETAILS IS UTILIZED.
4. CABLES MAY ATTACH THROUGH PAVERS AS LONG AS THE MINIMUM CONCRETE EMBEDMENT OF 1 ¼" IS ACHIEVED BY USING A LONGER TAPCON
5. MINIMUM CONCRETE EDGE DISTANCE FOR ALL CONCRETE FASTENERS IS 2 ½".
6. CABLES SHOULD BE AT A 45° ANGLE TO THE VERTICAL UPRIGHTS (±15°).



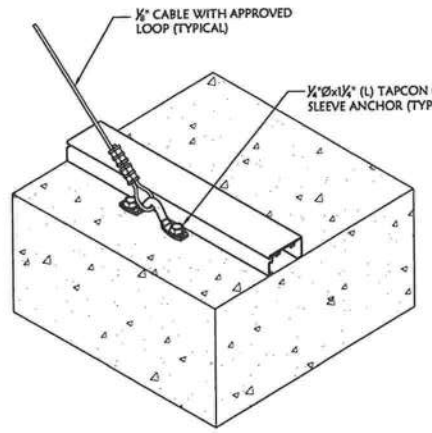
CAMELBACK TO SIDE DECK



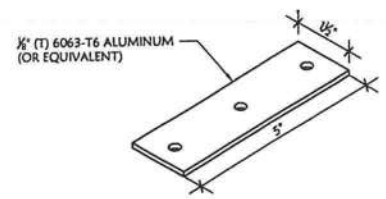
SINGLE CONCRETE FASTENER TO SIDE DECK



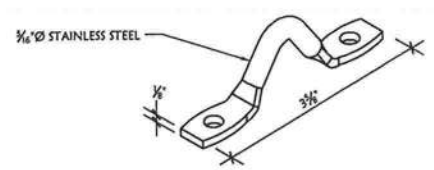
FLAT BAR TO SIDE OF DECK



CAMELBACK TO TOP OF DECK



FLAT BAR PLATE



CAMELBACK CLIP BRACKET

1 S-2.5 Typical Cable Attachment Details
N.T.S.

STRUCTURAL CONCEPTS & DESIGN
2989 WEST S.R. 434, SUITE 300 • LONGWOOD, FL 32779
PHONE 407/622-2046 • FAX 407/682-2386
WWW.STRUCTURALCONCEPTS-DESIGN.COM
FLORIDA C.O.A. NO. 27191

ROBERT C. SCROGGINS LICENSE NO. 56158 STATE OF FLORIDA PROFESSIONAL ENGINEER

06/07/13
Robert C. Scroggins, P.E.
Fl. Registration No. 56158

To the best of the Design Engineer's knowledge, the plans and specifications for this project comply with the applicable minimum building codes as determined by the local authority in accordance with the Florida Statutes.

Structural Framing
Sections & Details

Project:
Residential Pool Screen Enclosure
, Florida

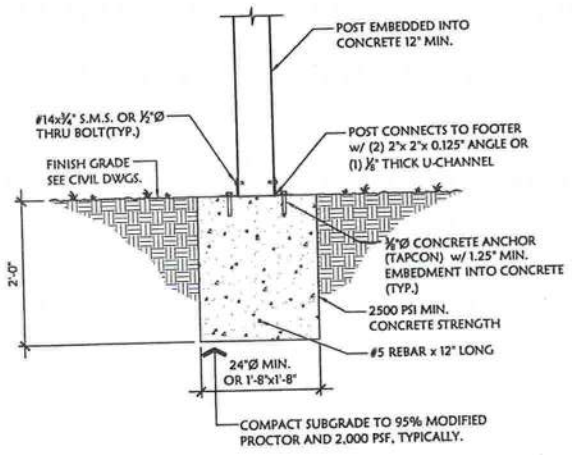
Client:
Florida Pool Enclosures, Inc
Screened In Your Lifestyle
922 HICKORY STREET • ALTAMONTE SPRINGS, FL 32701
TEL 407/260-2800 • FAX 407/260-6481
www.FLORIDAPOOLENCLOSURES.com

REV.	DATE	DESCRIPTION

Project No.: #13-002.7
Drawn By: TLW
Checked By: RCS
Approved By: RCS
Date: 06/07/13

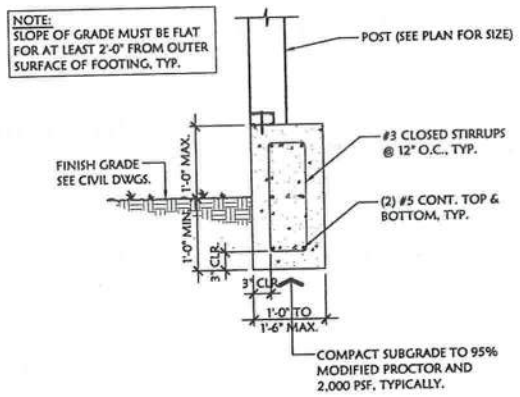
Sheet
S-2.5

F:\Structural Server\Florida Pool Enclosures\2013 Projects\13-002 Residential Pool Screen Enclosures (FBC 2010) 06/07/13 TLW



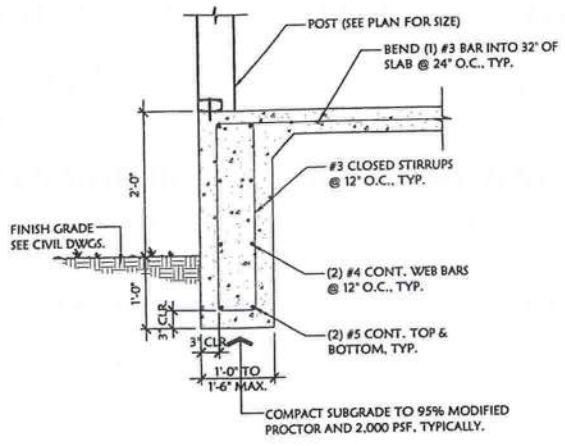
1 S-2.6 **Typical Isolated Footing Detail**

SCALE ¾" = 1'-0"



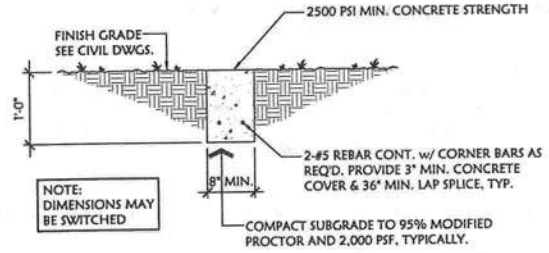
2 S-2.6 **Retaining Wall Footing**

SCALE ¾" = 1'-0"



3 S-2.6 **Retaining Wall Footing**

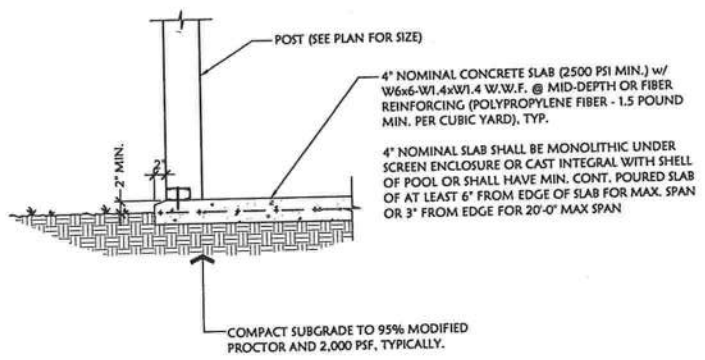
SCALE ¾" = 1'-0"



4 S-2.6 **Typical Continuous Perimeter Footing Detail**

SCALE ¾" = 1'-0"

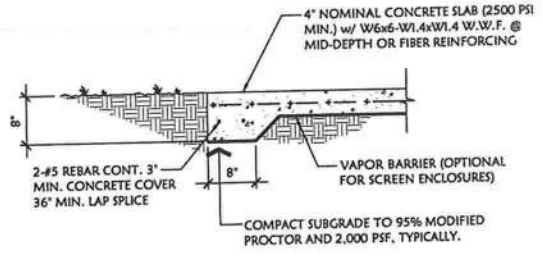
(MAX. POOL ENCLOSURE ROOF SPAN OF 50'-0"



5 S-2.6 **Post to Concrete Slab Connection**

SCALE ¾" = 1'-0"

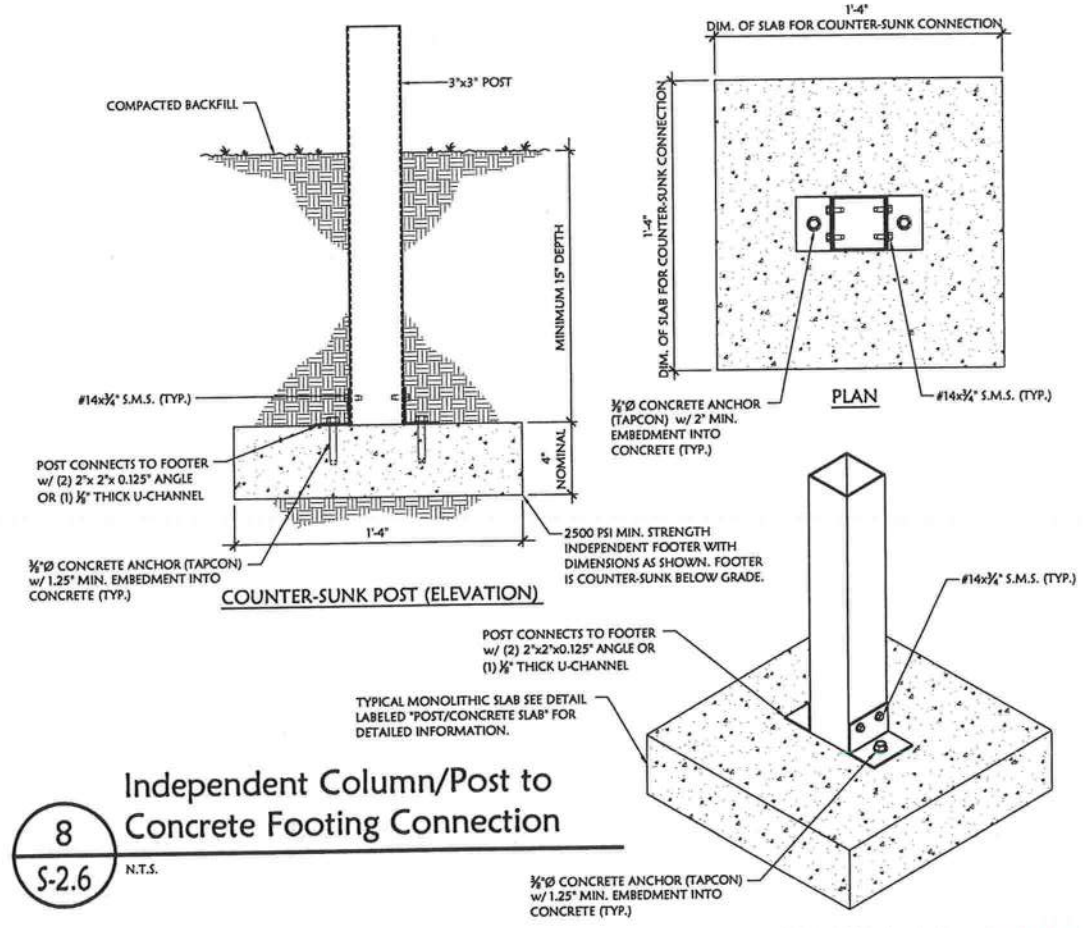
(MAX. POOL ENCLOSURE ROOF SPAN OF 40'-0"



7 S-2.6 **Typical Thickened Slab Footing**

SCALE ¾" = 1'-0"

(MAX. POOL ENCLOSURE ROOF SPAN OF 50'-0"



8 S-2.6 **Independent Column/Post to Concrete Footing Connection**

N.T.S.

KNEE WALL TABLE				
H	W	N		X
		#3	#4	
32"	12"	3	2	10'-0"
40"	12"	3	2	8'-0"
48"	18"	N/A	3	6'-0"
56"	18"	N/A	3	4'-0"
64"	24"	N/A	3	2'-8"
72"	30"	N/A	4	1'-8"

6 S-2.6 **Knee Wall Footing For Screen Enclosures**

SCALE ¾" = 1'-0"

To the best of the Design Engineer's knowledge, the plans and specifications for this project comply with the applicable minimum building codes as determined by the local authority in accordance with the Florida Statutes.

Structural Framing Sections & Details

Project: **Residential Pool Screen Enclosure**, Florida

Client: Florida Pool Enclosures, Inc. — 922 HICKORY STREET • ALTAMONTE SPRINGS, FL 32701 — TEL 407.260.2800 • FAX 407.260.4481 — www.FLORIDAPOOLENCLOSURES.com

REV.	DATE	DESCRIPTION

Project No.: #13-002.7
Drawn By: TLW
Checked By: RCS
Approved By: RCS
Date: 06/07/13

Sheet **S-2.6**

STRUCTURAL CONCEPTS & DESIGN
2989 WEST S.R. 434, SUITE 300 • LONGWOOD, FL 32779
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06/07/13
Robert C. Scroggins, P.E.
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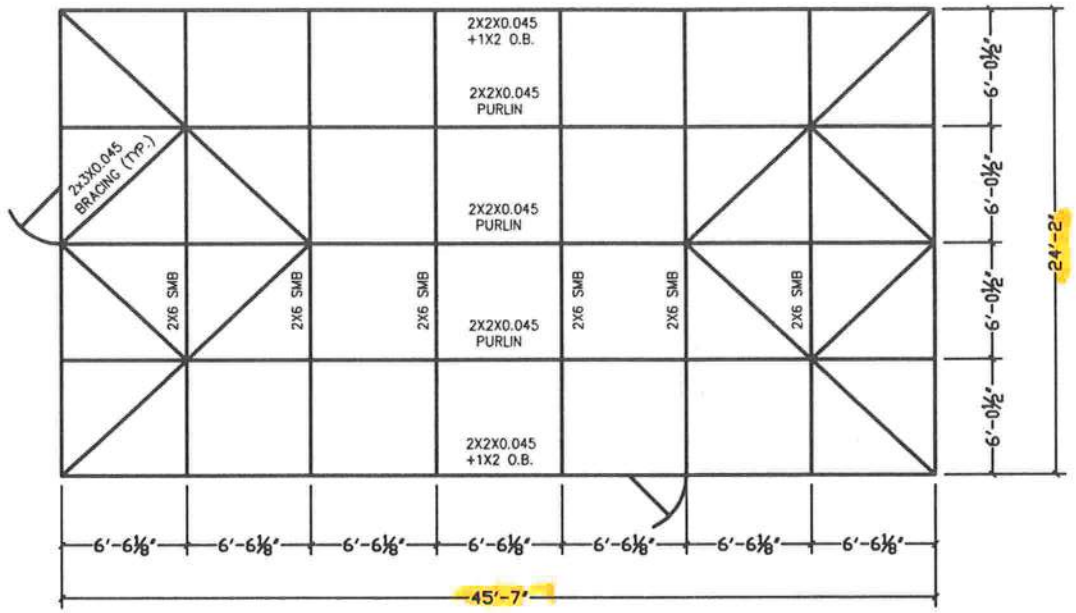
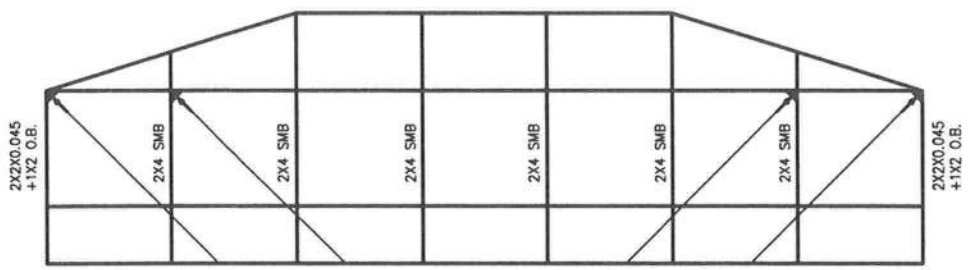
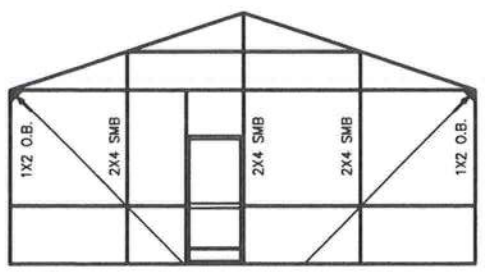


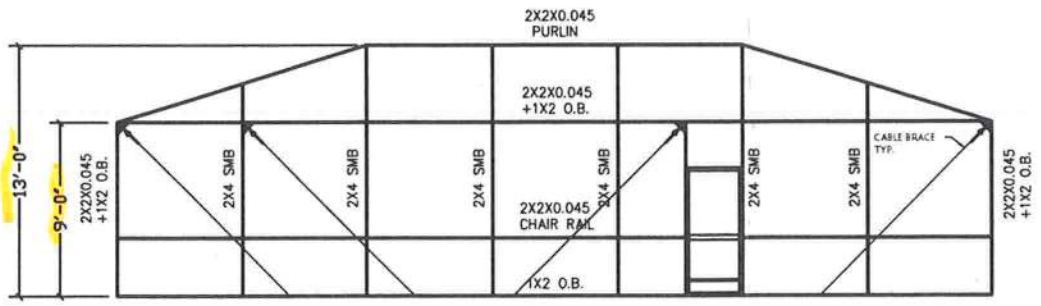
POOL ENCLOSURE PLAN



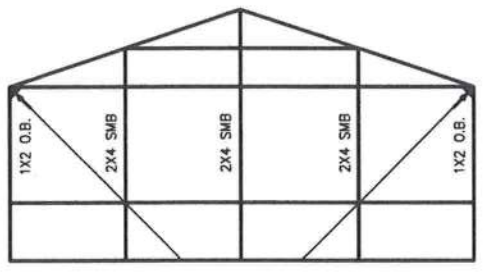
BACK FRAMING ELEVATION



LEFT FRAMING ELEVATION



FRONT FRAMING ELEVATION



RIGHT FRAMING ELEVATION

Florida Pool Enclosures
922 Hickory St.
Altamonte Springs, FL
407-260-2800
fax 407-260-6411

Job # 13405

Project Address:
Eric Preston
247 S.E. Mogave
Lake City, FL 32025

tax district: Columbia County

date: 06/29/13

scale: NTS

prepaired by: Allen Thompson

Structural Concepts & Design, LLC

The locations of doors are incidental to the design.
Girts remain the same throughout the design unless otherwise noted.
Dimensions are to center of walls and members.
Length given for knee braces represents horizontal and vertical displacements.