

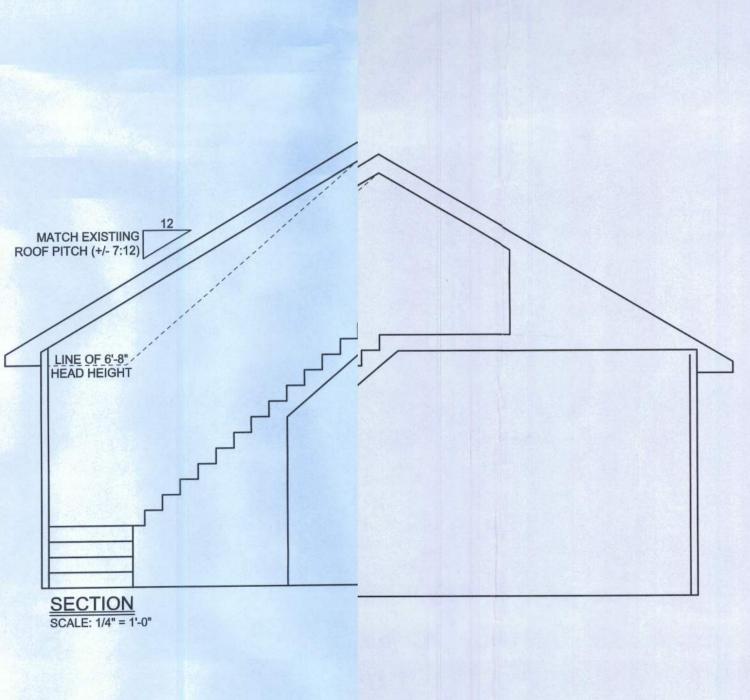
## **ELECTRICAL PLAN NOTES**

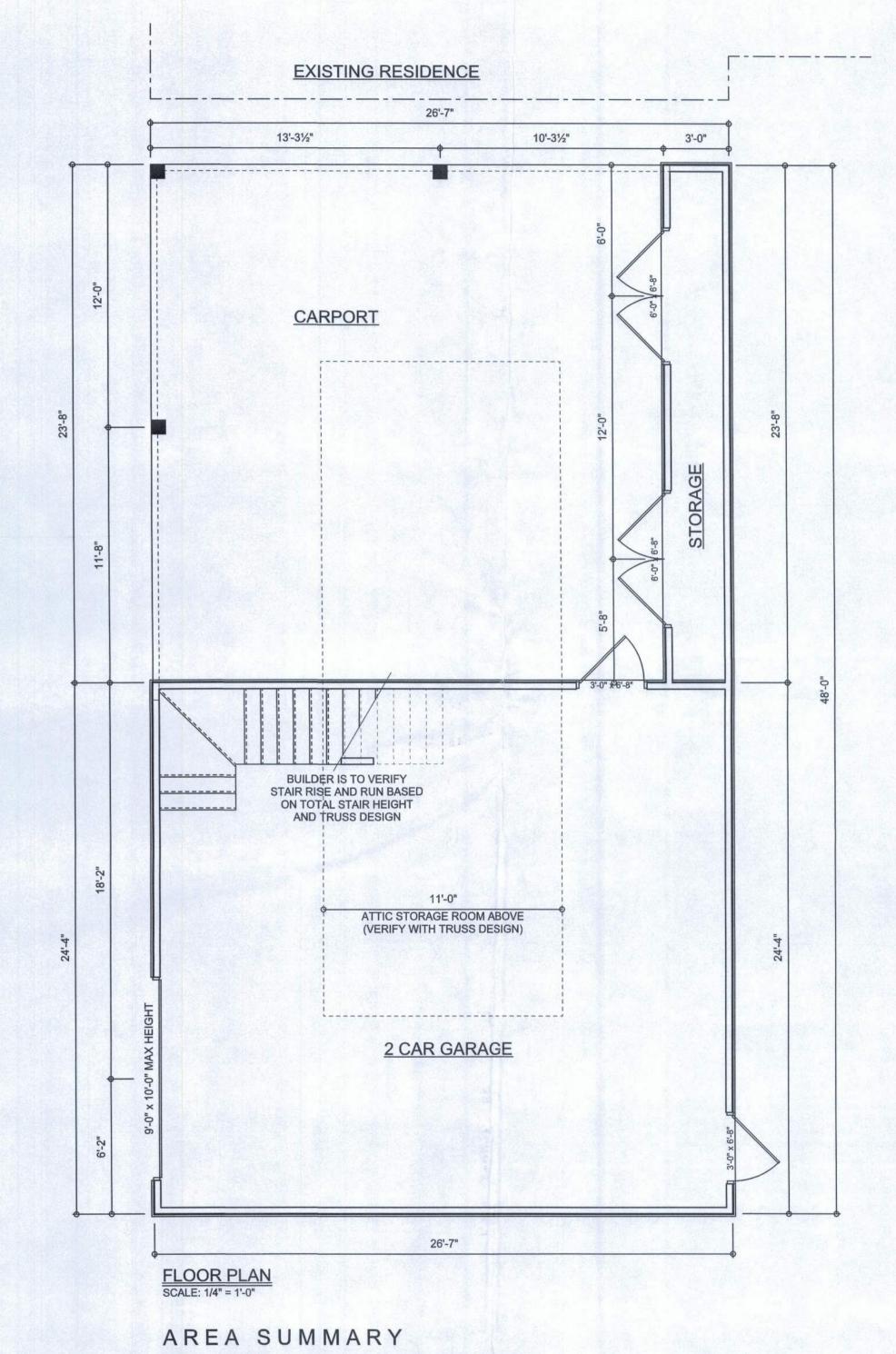
- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY E -4 BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5
  TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE
  DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
  DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE
  SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE ( UNDERGROUND OR OVERHEAD )
  TO BE DETERMINED BY POWER COMPANY.
- ALL 120-VOLT, SINGLE-PHASE, 15- AND 20-AMPERE BRANCH CIRCUITS SUPPLYING OUTLETS INSTALLED IN DWELLING UNIT FAMILY ROOMS, UITS DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DENS, BEDROWS, E -8 SUN ROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS, OR SIMILAROOMS ROOMS OR AREAS SHALL BE PROTECTED BY A LISTED ARC-FAULT CIMILAR INTERRUPTER, COMBINATION-TYPE INSTALLED TO PROVIDE PROTECT CIRCU. THE BRANCH CIRCUIT.
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC

- E -10 CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL
- CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10' E -11 OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.
- E -12 ALL OUTLETS LOCATED IN RESIDENTIAL TO BE TAMPER-RESISTANT PER NEC.

		<b>ELECTRICAL LEGEND</b>
		CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	00	DOUBLE SECURITY LIGHT
		2X4 FLUORESCENT LIGHT FIXTURE
	0	RECESSED CAN LIGHT
	<b>→</b>	BATH EXAUST FAN WITH LIGHT
	₩	BATH EXAUST FAN
	<b></b>	LIGHT FIXTURE
	Ф	DUPLEX OUTLET
	•	220v OUTLET
	Фан	GFI DUPLEX OUTLET
	•	SMOKE DETECTOR
	\$	WALL SWITCH
S,	\$3	3 WAY WALL SWITCH
UIT ON OF	\$4	4 WAY WALL SWITCH
	Ø wp/gFi	WATER PROOF GFI OUTLET
Paul	$\nabla$	PHONE JACK
	0	TELEVISION JACK
	•	GARAGE DOOR OPENER
	<b>С</b> М	CARBON MONOXIDE ALARM





1276

S.F.

TOTAL AREA

REVISIONS

VIIOLOAD ENGINEER: Mark Disosway, PEIo.53915, POB 868, Lake City, FL 3206, 386-754-5419 DINENSIONS: Stard dimensions supercede scaled

Mat Disosway, P.E. for resolution.
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CE:TIFICATION: I hereby certify that I have exanined this plan, and that the applicable porons of the plan, relating to wind engineering corply with section R301.2.1, florida building cod residential 2007 & 2009 Supplements, to te best of my knowledge. LINTATION: This design is valid for one

builing, at specified location. P.E. 53915

Murphy McLean

DETACHED GARAGE

> ADDRESS: Lake City, FL

Mark Disosway P.E. P.O. Box 868 \_ake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

> PRINTED DATE: December 09, 2010

DRAWN BY: STRUCTURAL BY Evan Beamsley

INALS DATE: 2010-12-09

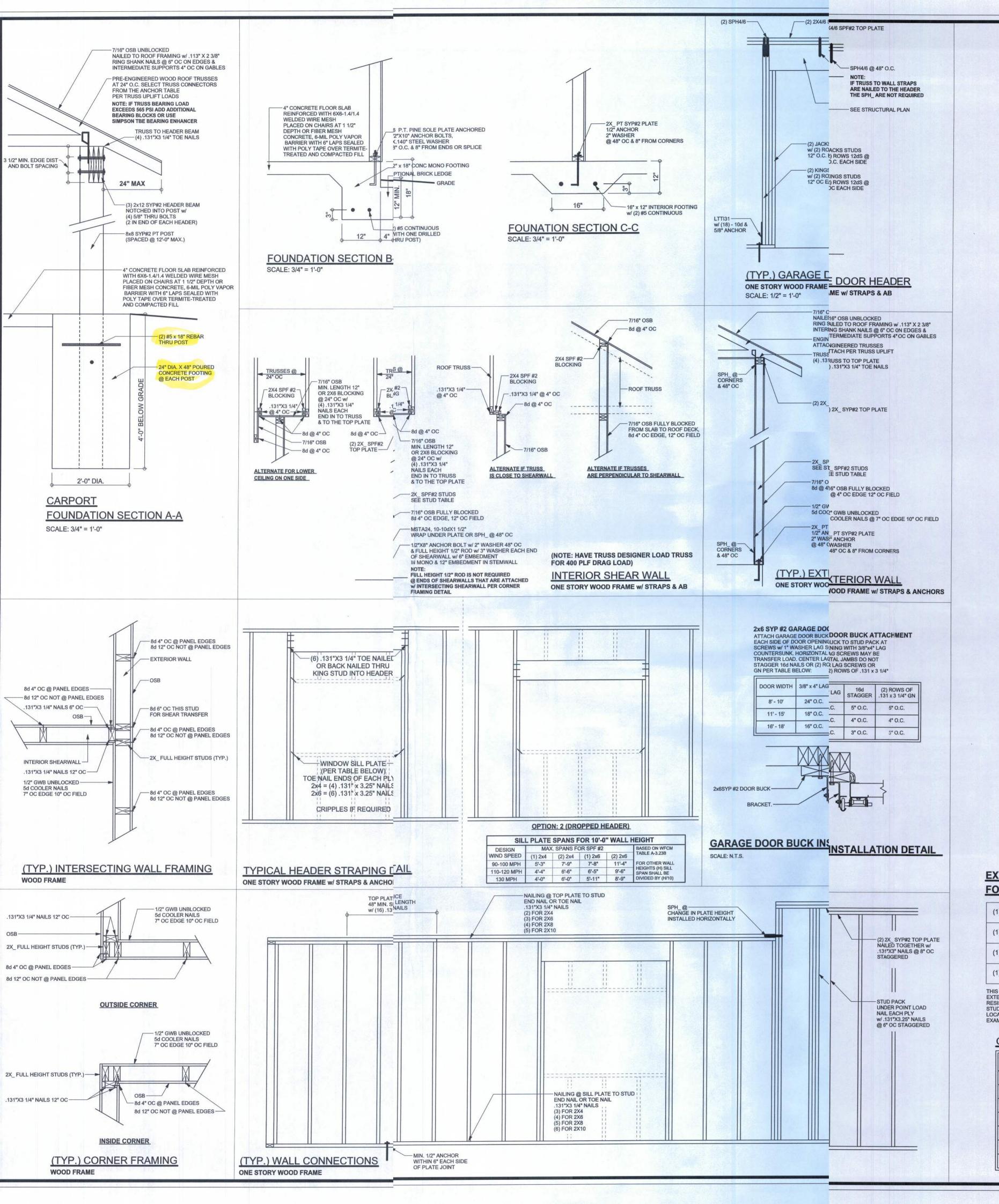
JOB NUMBER: 1011009 DRAWING NUMBER

OF 4 SHEETS

STAIR DESIGNOAD REQUIRMENTS: HANDRAIL PROFILE: - TYPE I: HANDRAILS WITH A CIRCULAR CROSS SECTION SHALL HAVE AN GUARDRAILS AND HANDRAI - 200 LB SINGLE CONCENTR LIVE LOAD APPLIED IN ANY DIRECTION AT ANY POINT 3 THE TOP OUTSIDE DIA. OF 1 1/4" - 2". IF HANDRAIL IS NOT CIRCULAR IT SHALL HAVE A PERIMETER DIMENSION OF 4" - 6 1/4" AND MAX. CROSS SECTION OF 2 1/4". TYPE II: HANDRAILS WITH A PERIMETER GREATER THAN 6 1/4" SHALL PROVIDE A GRASPABLE FINGER RECESS AREA ON BOTH SIDES OF THE PROFILE. THE FINGER RECESS SHALL BEGIN WITHIN A DISTANCE OF 3/4" GUARDRAILS IN-FILL COMP(S: - 50 LB LIVE LOAD APPLIED ONTALLY ON AN AREA EQUAL TO 1 FT2 (THIS LOAD NEED NOT BE A LED TO ACT CONCURRENTLY WITH ANY VERTICALLY FROM THE TALLEST PORTION OF THE PROFILE AND A DEPTH OF AT LEAST 5/8" WITHIN 7/8" BELOW THE WIDEST PORTION OF THE PROFILE. OTHER LIVE LOAD REQUIRI THIS REQUIRED DEPTH SHALL CONTINUE FOR AT LEAST 3/8" TO A LEVEL - GLAZING USED IN HANDHASEMBLIES AND GUARDS SHALL BE DESIGNED WITH A SAFETY OR OF 4. THE SAFETY FACTOR SHALL BE APPLIED TO EACH OF THE ENTRATED LOADS APPLIED TO THE TOP OF THE RAIL, AND TO THE ON THE IN-FILL COMPONENTS. THIES LOADS SHALL BE DETERM NOEPENDENT OF ONE ANOTHER, AND LOADS ARE ASSUMED NOT CCUR WITH ANY OTHER LIVE LOAD. THAT IS NOT LESS THAN 1 3/4" BELOW THE TALLEST PORTION OF THE PROFILE. THE WIDTH OF THE HANDRAIL ABOVE THE RECESS SHALL BE 1 3/4" - 2 3/4". EDGES SHALL HAVE A MIN. RADIUS OF 0.01". A 4 3/8" SPHERE CANNOT PASS THROUGH OPENING IN GUARDS ON SIDES OF STAIR - 40 PSF LIVE LOAD, OR 300 INCENTRATED LOAD OVER AN AREA OF 4 IN2 (WHICHEVER PRODURE GREATER STRESSES) 36" MIN. CLEAR WIDTH ABOVE HANDRAIL SO THAT A 4" SPHERE CANNOT PASS THROUGH HANDRAIL PROJECTION PENING IN GUARD U.N.O. 4 1/2" MAX. PROJECTION NOSINGS:
EVERY TREAD LESS THAT 10" SHALL HANOSING OR
EFFECTIVE PROJECTION OF APPROX. 112 THE LEVEL
BELOW. A NOSING IS NOT REQUIRED WITHE TREAD
IS A MIN. 11". THE RADIUS OF CURVATUITHE
LEADING EDGE OF TREAD SHALL BE NOTER THAT 9/16" 1 1/2" MIN. -HANDRAIL A 6" SPHERE CANNOT PASS -AND WALL THROUGH THE TRIANGULAR AREA FORMED BY RISER, BEVELING OF NOSING SHALL NOT EXCE." TREAD AND GUARD RAIL HANDRAILS SHALL BE CONT. ON AT RISERS SHALL BE VERTICAL OR ED AT AN ANGLE NOT MORE THAN 30 DEGFROM VERTICAL. OPEN RISERS ARE PERMITTED, PED THAT THE OPENING BETWEEN TREADS DOIT PERMIT THE PASSAGE OF A 4" DIA. SPHERE. (NG BETWEEN TREADS IS NOT LIMITED IF TOTAL IS 30" OR LESS LEAST ONE SIDE OF ALL STAIRS WITH 4 OR MORE RISERS A LANDING SHALL BE PROVIDED AT THE TOP AND BOTTOM. LANDINGS (FIRECODE) SHALL HAVE A MIN WIDTH OF NOT GYP. BOARD LESS THAT STAIR WIDTH AND HAVE UNDER ALL A MIN 36" MEASURED IN DIRECTION OF TRAVEL. A DOOR AT THE TOP OF THE GREARISER IN ANY
FLIGHT OIRS SHALL NOT
EXCEED TALLEST STAIR IS PERMITTED, PROVIDED IT DOES NOT SWING OVER STAIR. THE SUM OF TWO RISERS AND A TREAD, EXCLUDING NOSING. SHALL BE BETWEEN 24" AND 25" WINDER TREADS: (2 RISERS + 1 TREAD = 24" - 25") WINDER TREADS SHALL HAVE A MIN. TREAD DEPTH OF 10" MEASURED AT A POINT 12" FROM NARROWEST END. 9" MIN. TREAD DEPTH THE GREATEST DEPTH IN ANY WINDER TREADS SHALL HAVE A MIN. DEPTH FLIGHT OF STAIRS SHALL NOT OF 6" AT ANY POINT. WITHIN A FLIGHT OF STAIRS THE **EXCEED THE SMALLEST** BY MORE THAN 3/8" MAX. TREAD SLOPE OF 2 PERCENT (1" VERT UNIT IN 48" HORZ. UNITS) SECTION VIEW

TYPICAL STAIR AND GUARDRAIL REQUIRMENTS

**GREATEST DEPTH AT 12" SHALL NOT EXCEED THE SMALLEST BY MORE THAN 3/8"** 



#### **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN
FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W 1.4 × W 1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

#### BUILDER'S RESPONSIBILITY

	ER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE LLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SIT BACKFILL HE	CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND GHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	ERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 TS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE	ONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL AD ENGINEER IMMEDIATELY.
DESIGN, PLAC	RUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS EMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, USS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL EATIONS.

#### **MASONRY NOTES:**

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI530 1-02 Section Specific Requirements

ACI530.1-02 Section	Specific Requirements		
Compressive strength	8" block bearing walls F'm = 1500 psi		
1         Mortar         ASTM C 270, Type N, Ul           2         Grout         ASTM C 476, admixtures			
Grout	ASTM C 476, admixtures require approv		
CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block		
Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"		
Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)		
Coating for corrosion protection  Anchors, sheet metal ties compenseded in mortar or grout, A A525, Class G60, 0.60 oz/ft2 or			
F Coating for corrosion protection  Joint reinforcement in walls expended of moisture or wire ties, anchors, sties not completely embedded in grout, ASTM A153, Class B2, 1. or 304SS			
Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.		
Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.		
	Compressive strength Mortar Grout CMU standard  Clay brick standard  Reinforcing bars, #3 - #11  Coating for corrosion protection  Coating for corrosion protection		

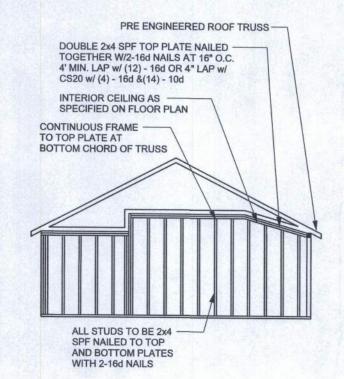
# EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-6" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-7" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-7" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3,20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

# GRADE & SPECIES TABLE

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO
CEILING DIAPHRAGM DETAIL

#### **ANCHOR TABLE**

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

		The state of the s	. O. LAILO	10 KALIENTKUSS	10 21002
< 420	< 245	H5A	3-8d	3-8d	4
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	77 L R L 1077
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	190
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	2 4 4 C C C
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	28 H - C - C - C - C - C - C - C - C - C -
< 760	< 655	H10-2	6-10d	6-10d	TOTAL COLUMN TO
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*	( P. 15 m - 5 )		TO FOUNDATION
1564					TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d	Jan Line Con	4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		WE ENGLISH TEL
	New Hard	STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d	LEGITA W.	5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d	120/0701	2-5/8" AB

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR\* TO PLATES TO RAFTER/TRUSS

## ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERA BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

## DESIGN DATA

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

30 PSF (SLEEPING ROOMS

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

_	ID LOADS PER FLORIDA BUILDING CODE 2007 I					
ON	ICLOSED SIMPLE DIAPHRAGM BUILDINGS WI AN ROOF HEIGHT NOT EXCEEDING LEAST HO UPPER HALF OF HILL OR ESCARPMENT 60FT DPE AND UNOBSTRUCTED UPWIND FOR 50x H	ORIZONTAL	DIME	NSION	OR 60 I	FT; NO
_	LDING IS NOT IN THE HIGH VELOCITY HURRI			40.74		
BU	LDING IS NOT IN THE WIND-BORNE DEBRIS F	REGION				
1.)	BASIC WIND SPEED = 110 MPH	5 14 19 19	18/14	De la	7.27	211
2.)	WIND EXPOSURE = C					
3.)	WIND IMPORTANCE FACTOR = 1.0	0,10110	100	Name of the last	Thirty.	Bell S
4.)	BUILDING CATEGORY = II		No.	Bhs		
5.)	ROOF ANGLE = 10-45 DEGREES					
6.)	MEAN ROOF HEIGHT = <30 FT	arity Table			dicess.	
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (E	ENCLOSED	BUILD	ING)	1990	500
8.)						
0.1	COMPONENTS AND CLADDING DESIGN WIN	D PRESSU	RES (T	ABLE F	R301.2(	2))
0.,	COMPONENTS AND CLADDING DESIGN WIN				10, 1157	
0.,	COMPONENTS AND CLADDING DESIGN WIN	Zone	Effec	tive W	ind Ar	ea (ft
0.,	COMPONENTS AND CLADDING DESIGN WIN	Zone	Effec	tive W	ind Ar	ea (ft
.,	COMPONENTS AND CLADDING DESIGN WIN	Zone 1	Effec 27.8	tive W 10 -30.5	25.3	ea (ft 100 -25
0.,	COMPONENTS AND CLADDING DESIGN WIN	Zone 1 2	Effec 27.8	tive W 10 -30.5 -35.7	25.3	ea (ft 100 -25 -30
0.,	COMPONENTS AND CLADDING DESIGN WIN	Zone 1	27.8 27.8	10 -30.5 -35.7 -56.8	25.3 25.3	ea (ft 100 -25 -30 -56
5.7		2 2 O'hg	Effec 27.8	10 -30.5 -35.7 -56.8	25.3	ea (ft 100 -25 -30 -56 -30
0.7		1 2 2 O'hg 3	27.8 27.8 27.8	-30.5 -35.7 -56.8 -35.7	25.3 25.3	ea (ft 100 -25 -30 -56 -30 -59
		2 O'hg 3 O'hg	27.8 27.8 27.8	-30.5 -35.7 -56.8 -35.7 -95.6	25.3 25.3 25.3	ea (ft 100 -25 -30 -56 -30 -59 -28
0.7		2 2 O'hg 3 3 O'hg 4 5	27.8 27.8 27.8 27.8 30.5 30.5 8 Wine	-30.5 -35.7 -56.8 -35.7 -95.6 -33.0 -40.7	25.3 25.3 25.3 25.3	ea (ft 100 -25 -30 -56 -30 -59 -28 -31
0.7		2 O'hg 3 O'hg 4 5	27.8 27.8 27.8 30.5 30.5	-30.5 -35.7 -56.8 -35.7 -95.6 -33.0 -40.7	25.3 25.3 25.3 25.3 25.9 25.9	ea (ft 100 -25 -30 -56 -30 -59 -28 -31
		2 O'hg 3 O'hg 4 5	27.8 27.8 27.8 30.5 30.5 8 Winest Case 5, 10	-30.5 -35.7 -56.8 -35.7 -95.6 -33.0 -40.7 dows eft2)	25.3 25.3 25.3 25.3 25.9 25.9	ea (fit 100 -25 -30 -56 -30 -59 -28 -31
		Zone  1 2 2 O'hg 3 3 O'hg 4 5  Doors Wors (Zone	27.8 27.8 27.8 30.5 30.5 8 Windst Case 5, 10	tive W 10 -30.5 -35.7 -56.8 -35.7 -95.6 -33.0 -40.7 dows e ft2)	25.3 25.3 25.3 25.9 25.9 30.5	ea (fit 100 -25 -30 -56 -30 -59 -28 -31 -40
		Zone  1 2 2 O'hg 3 3 O'hg 4 5  Doors Word (Zone) 8x7 Gar	27.8 27.8 27.8 30.5 30.5 8 Windst Case 5, 10	tive W 10 -30.5 -35.7 -56.8 -35.7 -95.6 -33.0 -40.7 dows e ft2)	25.3 25.3 25.3 25.9 25.9 25.9 30.5	ea (ft2

WINDLOA ENGINEER: Mark Disosway, PE No.5395, POB 868, Lake City, FL 32056, 38(754-5419

**REVISIONS** 

DIMENSICIS: Stated dimnsions supercede scaled dimensions Refer all questions to Mark Disoray, P.E. for resolution. Do not proed without clarification.

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CERTIFIC/TION: I hereby certify that I have

examined tis plan, and that the applicable portions of ne plan, relating to wind engineeri comply witsection R301.2.1, florida building code residatial 2007 & 2009 Supplements, to the best of my knowledge.

LIMITATIO: This design is valid for one



Murphy McLean

DETACHED GARAGE

> ADDRESS: Lake City, FL

Mak Disosway P.E. P.O. Box 868 LakeCity, Florida 32056 Phore: (386) 754 - 5419 Fax (386) 269 - 4871

PRINTED DATE: December 09, 2010

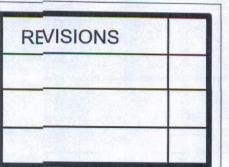
DRAWNBY: STRUCTURAL BY:
Evan Beamsley

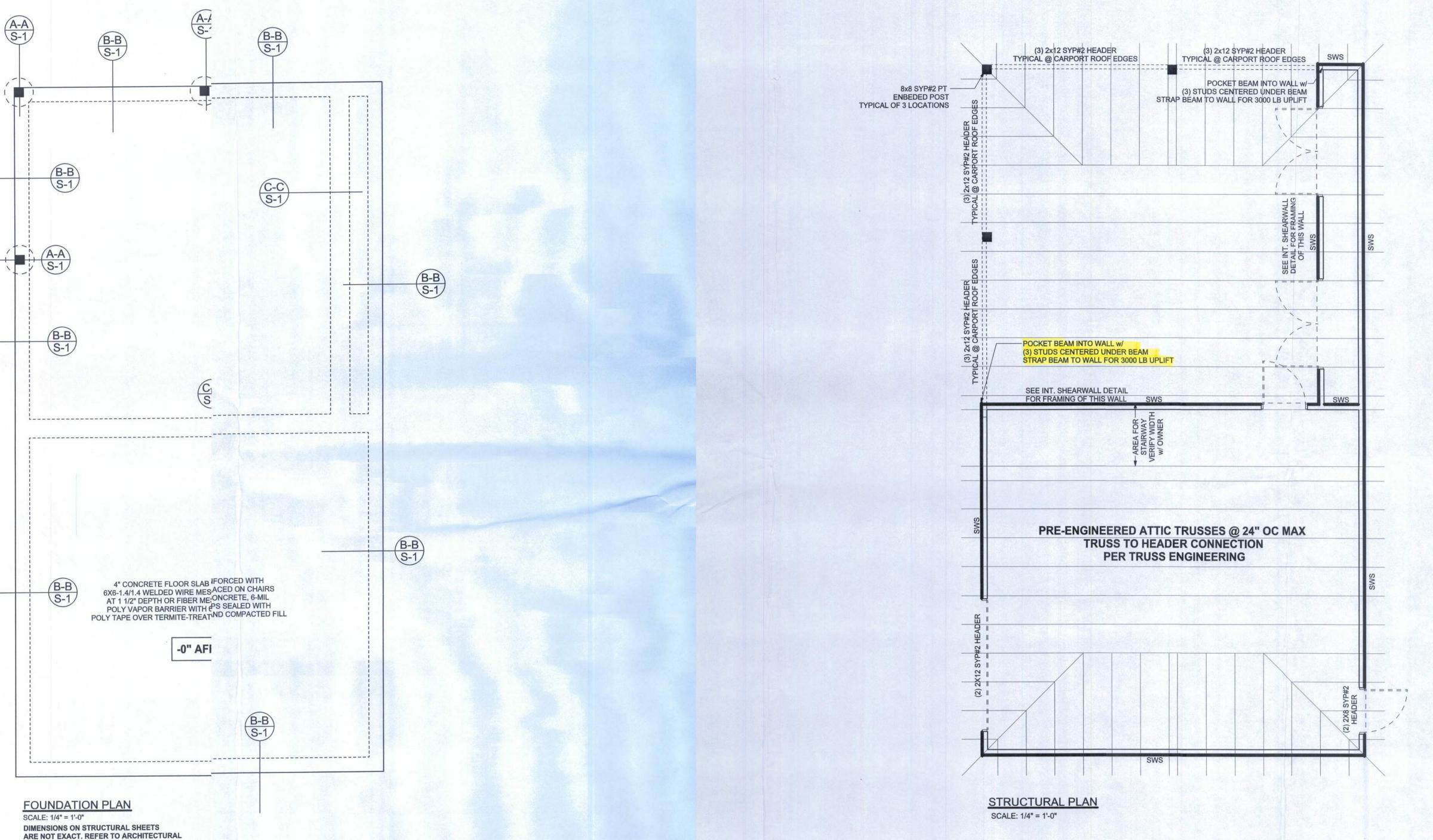
FINALS)ATE:

J0B NUMBER: 1011009

RAWING NUMBER

OF 4 SHEETS





FLOOR PLAN FOR ACTUAL DIMENSIONS

## WALL LEGEND

	EXTERIOR WALL
=====	INTERIOR NON-LOAD BEARING WALL
Z777773 = = = = 17777723	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

## STRUCTURAL PLAN NOTES

SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT SN-2 LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WINDLOD ENGINEER: Mark Disosway, PE No.5915, POB 868, Lake City, FL 32056, 36-754-5419

DIMENSONS:
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permissin and consent of Mark Disosway. CERTIFFATION: I hereby certify that I have

examine this plan, and that the applicable portions f the plan, relating to wind engineering comply vth section R301.2.1, florida building code resential 2007 & 2009 Supplements, to the bet of my knowledge. LIMITATON: This design is valid for one building, it specified location.



Murphy McLean

DETACHED GARAGE

ADDRESS: Lake City, FL

Nark Disosway P.E. P.O. Box 868 Lale City, Florida 32056 Phine: (386) 754 - 5419 Fix: (386) 269 - 4871

> PRINTED DATE: December 09, 2010 DRAVN BY:

STRUCTURAL BY: Evan Beamsley FINAS DATE: 201+12-09

JOB NUMBER: 1011009 DRAWING NUMBER

> **S-2** OF 4 SHEETS