Julius Lee

RE: 327272 - RICHARD KEEN - GLENN RES.

1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Site Information:

Project Customer: RICHARD KEEN Project Name: 327272 Model: GLENN RES.

Lot/Block:

Subdivision:

Address: 185 SW ARROWHEAD TER

City: COLUMBIA CTY

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: JAMES H. JOHNSTON

License #: CRC1328128

Address: 650 SW MAIN BLVD.

City: LAKE CITY.

State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002

Design Program: MiTek 20/20 7.1

Wind Code: ASCE 7-05 Wind Speed: 110 mph

Floor Load: N/A psf

Roof Load: 32.0 psf

This package includes 52 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules. This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No. Seal#		Truss Name	Date	No.	Seal#	Truss Name	Date
1	14241488	CJ1	3/4/010	18	14241505	HG13	3/4/010
2	14241489	CJ1A	3/4/010	19	14241506	HG17	3/4/010
3	14241490	CJ3	3/4/010	20	14241507	HG23	3/4/010
4	14241491	CJ3A	3/4/010	21	14241508	HJ2	3/4/010
5	14241492	CJ3B	3/4/010	22	14241509	HJ7	3/4/010
6	14241493	CJ5	3/4/010	23	14241510	HJ9	3/4/010
7	14241494	CJ5A	3/4/010	24	14241511	HJ9A	3/4/010
8	14241495	CJ5B	3/4/010	25	14241512	HJ9B	3/4/010
9	14241496	EJ5	3/4/010	26	14241513	HJ9C	3/4/010
10	14241497	EJ7	3/4/010	27	14241514	T01	3/4/010
11	14241498	EJ7A	3/4/010	28	14241515	T02	3/4/010
12	14241499	EJ7B	3/4/010	29	14241516	T03	3/4/010
13	14241500	EJ8	3/4/010	30	14241517	T04	3/4/010
14	14241501	EJ8A	3/4/010	31	14241518	T05	3/4/010
15	14241502	EJ8B	3/4/010	32	14241519	T06	3/4/010
16	14241503	EJ8C	3/4/010	33	14241520	T07	3/4/010
17	14241504	HG03	3/4/010	34	14241521	T08	3/4/010

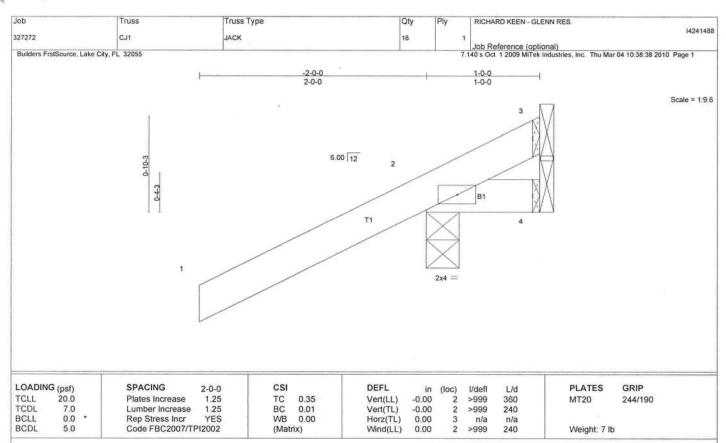
The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2011.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.





TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

U

N

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical

Max Horz 2=109(LC 6)

Max Uplift 2=-371(LC 6), 4=-12(LC 4), 3=-90(LC 1) Max Grav 2=256(LC 1), 4=14(LC 2), 3=163(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable eng zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 371 lb uplift at joint 2, 12 lb uplift at joint 4 and 90 lb uplift at joint 3.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

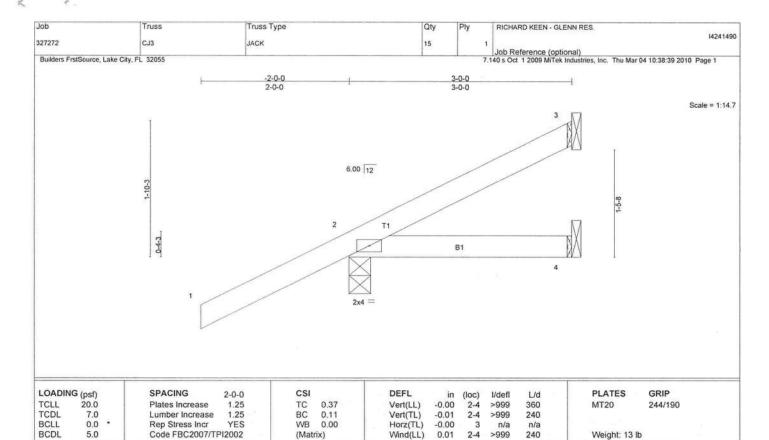
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

March 4,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters are proper incorporation of component is responsibility obliding designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flabrication, quality control, storage, delivery, erection and bracing, consult. ANSI/ITI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719,



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

U

D

FLORIDA

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical

Max Horz 2=166(LC 6)

Max Uplift3=-36(LC 7), 2=-311(LC 6), 4=-36(LC 4) Max Grav 3=31(LC 1), 2=250(LC 1), 4=42(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 3, 311 lb uplift at joint 2 and 36 lb uplift at joint 4.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

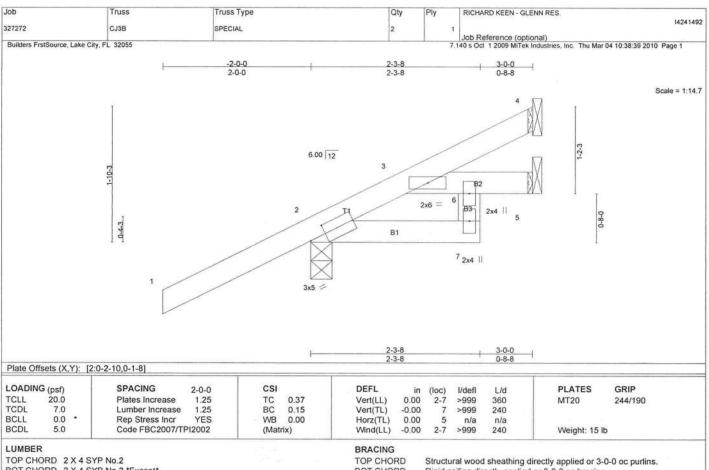
LOAD CASE(S) Standard

March 4,2010

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BOT CHORD 2 X 4 SYP No.2 *Except*

B3: 2 X 4 SYP No.3

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=41/Mechanical, 2=253/0-3-8, 5=9/Mechanical

Max Horz 2=166(LC 6)

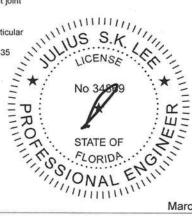
Max Uplift 4=-45(LC 6), 2=-265(LC 6)

Max Grav 4=41(LC 1), 2=253(LC 1), 5=48(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

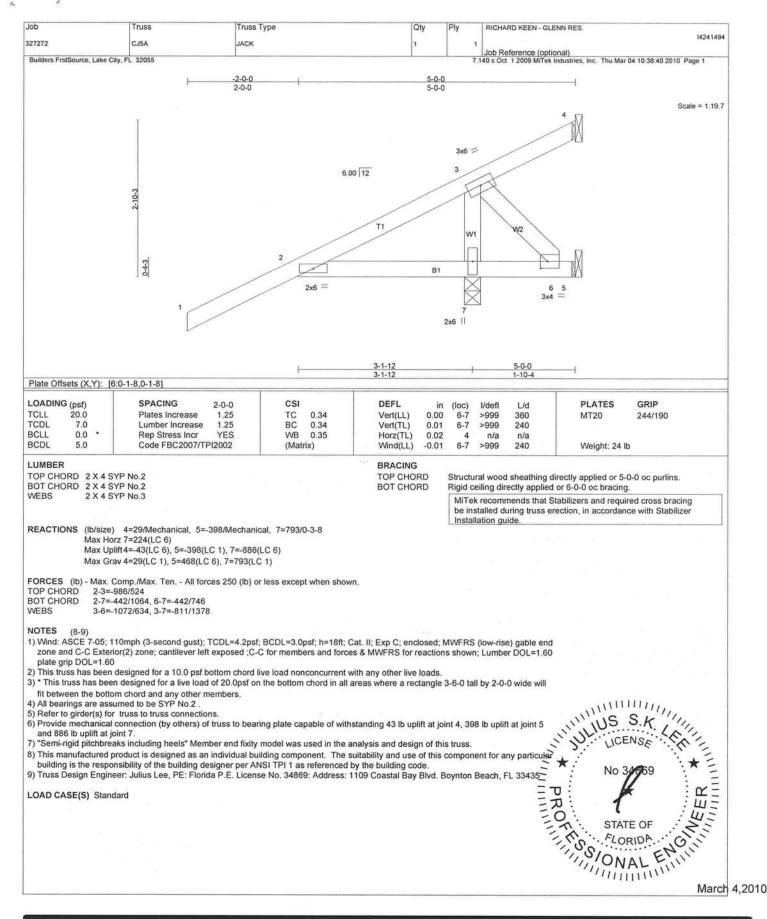
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 4 and 265 lb uplift at joint
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



March 4,2010

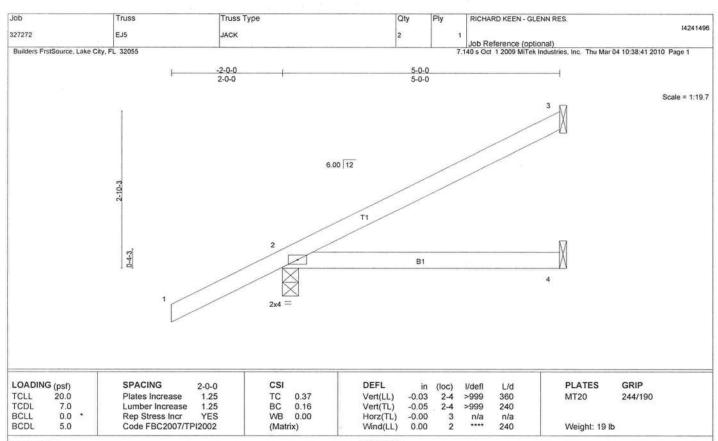
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Design valid for use only with Millex connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, defivery, erection and bracing, consult. ANSI/IPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719,



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE.

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ANSI/TPII Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=224(LC 6)

Max Uplift3=-114(LC 6), 2=-266(LC 6)

Max Grav 3=103(LC 1), 2=295(LC 1), 4=72(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, United

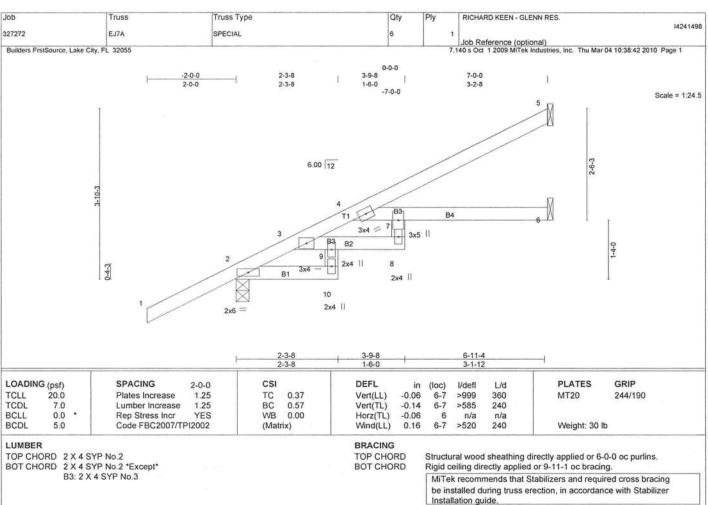
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

March 4,2010

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REACTIONS (lb/size) 5=121/Mechanical, 2=357/0-3-8, 6=82/Mechanical

Max Horz 2=203(LC 6)

Max Uplift5=-83(LC 6), 2=-190(LC 6), 6=-16(LC 6) Max Grav 5=121(LC 1), 2=357(LC 1), 6=106(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-4=-363/231

BOT CHORD 3-9=-307/192, 8-9=-394/299, 4-7=-299/394

NOTES

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C-Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SYP No.2.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 5, 190 lb uplift at joint 2 and 16 lb uplift at joint 6.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

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Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435

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Truss Type Job Truss Qty RICHARD KEEN - GLENN RES. 14241500 327272 F.18 MONO TRUSS Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:38:43 2010 Page 1 Builders FrstSource, Lake City, FL 32055 4-0-0 4-0-0 2-0-0 2-0-0 4-0-0 Scale = 1:26.9 6.00 12 2x4 > 0-4-3 6 3x5 = 3x4 =

Plate Offsets (X,Y): [2:0-2-10,0-1-8]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.14	2-6	>647	360	MT20	244/190
CDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.25	2-6	>367	240		
BCLL	0.0	Rep Stress Incr	YES	WB	0.10	Horz(TL)	-0.00	- 5	n/a	n/a		
BCDL	5.0	Code FBC2007/TF	212002	(Matr	rix)	Wind(LL)	-0.01	2-6	>999	240	Weight: 35 lb	

LUMBER

TOP CHORD 2 X 4 SYP No 2 BOT CHORD 2 X 4 SYP No.2 WEBS

2 X 4 SYP No.3

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=92/Mechanical, 2=381/0-3-8, 5=141/Mechanical

Max Horz 2=223(LC 6)

Max Uplift 4=-75(LC 6), 2=-198(LC 6), 5=-47(LC 6) Max Grav 4=92(LC 1), 2=381(LC 1), 5=147(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-299/143 **BOT CHORD** 2-6=-348/224 WEBS 3-6=-251/389

NOTES

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2

Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Rhd.

LOAD CASE(S) Standard STATE OF

March 4,2010

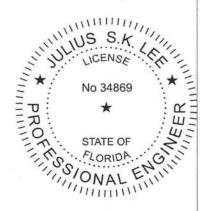
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1109 Coastal Bay Blvd. Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - GLENN RES.	
327272	EJ8A	MONO TRUSS	1	1	* 50	1424150
					Job Reference (optional)	
Buildon EastCourse Lake City	EL 220EE			7	140 c Oct 1 2000 MiTok Industrios Inc. Thu Mac 0	4.10-29-42.2010 Dogg 2

LOAD CASE(S) Standard Concentrated Loads (lb)

Vert: 1=-250 5=-826(B) 6=-285 7=-496(B)



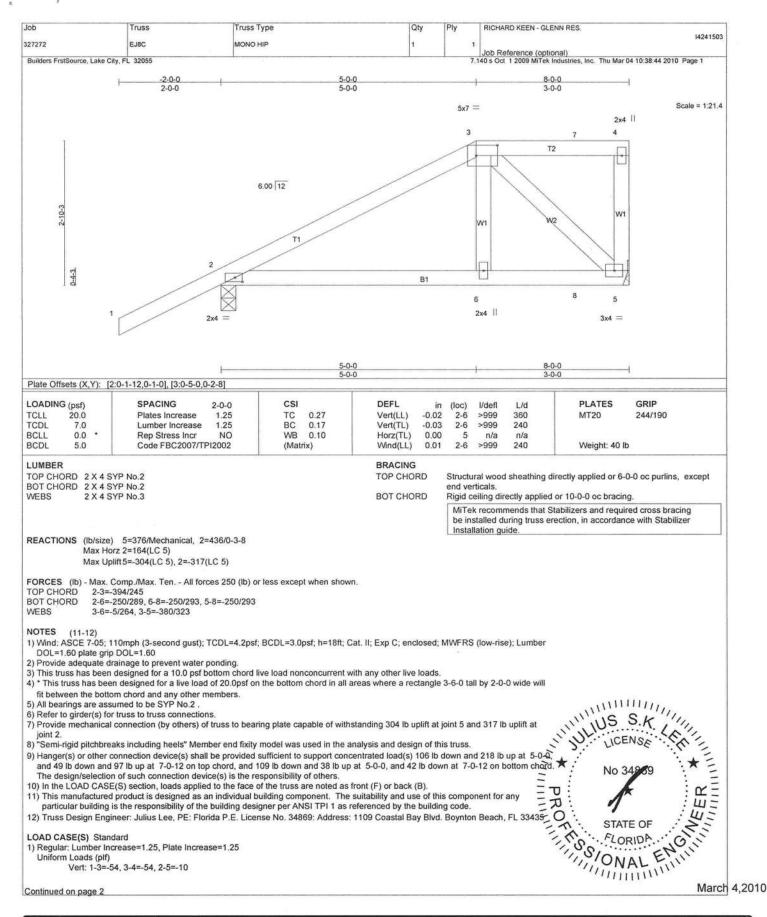


March 4,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

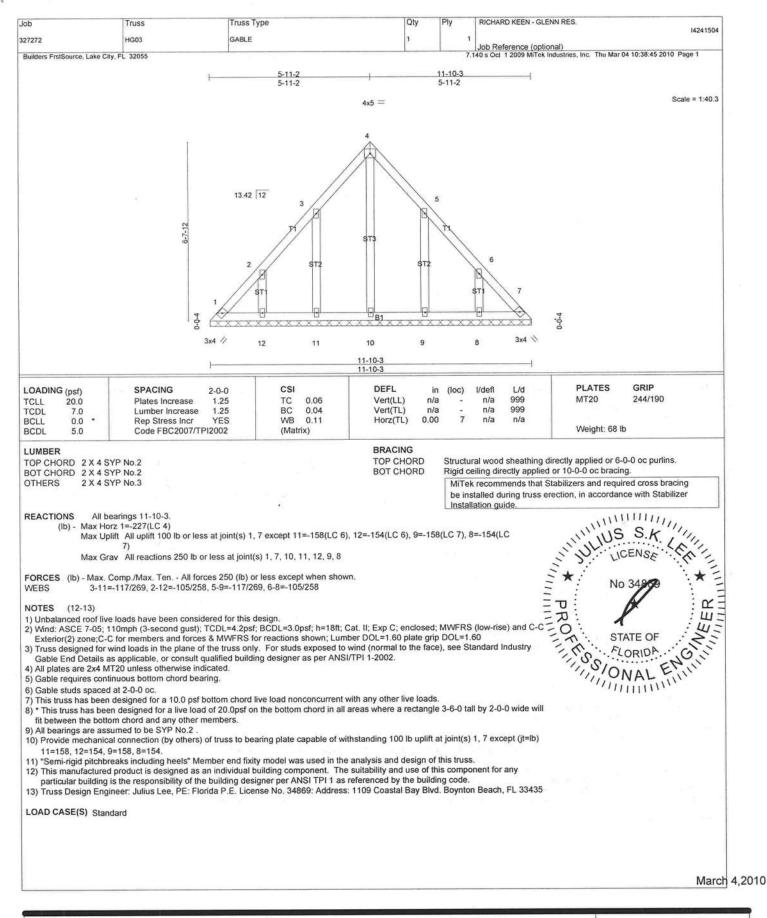
Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

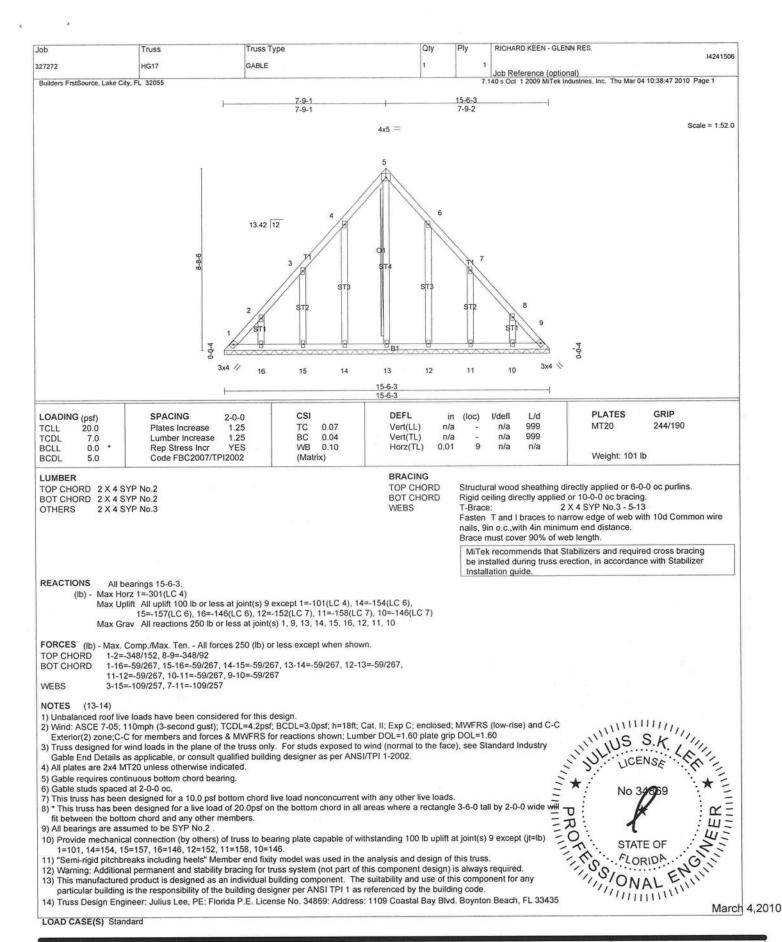
ANSI/TPI1 Quality Citteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.



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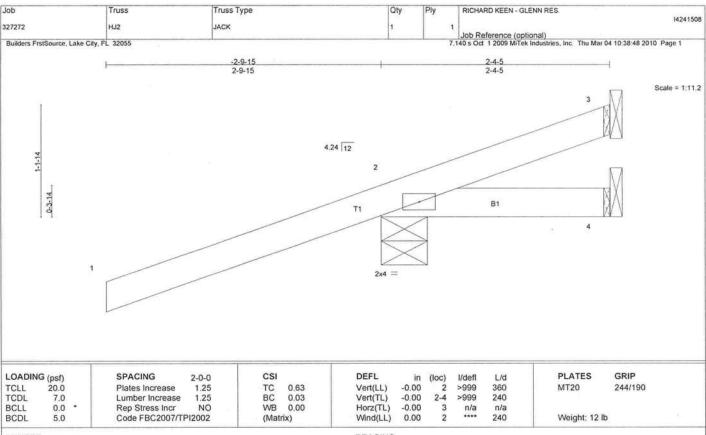


LOAD CASE(S) Standard

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Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435

March 4,2010



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 2-4-5 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. WIS SE

REACTIONS (lb/size) 3=-68/Mechanical, 2=355/0-5-11, 4=10/Mechanical

Max Horz 2=128(LC 3)

Max Uplift3=-68(LC 1), 2=-444(LC 3)

Max Grav 3=124(LC 3), 2=355(LC 1), 4=31(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18tt; Cat. II, Log.

zone; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

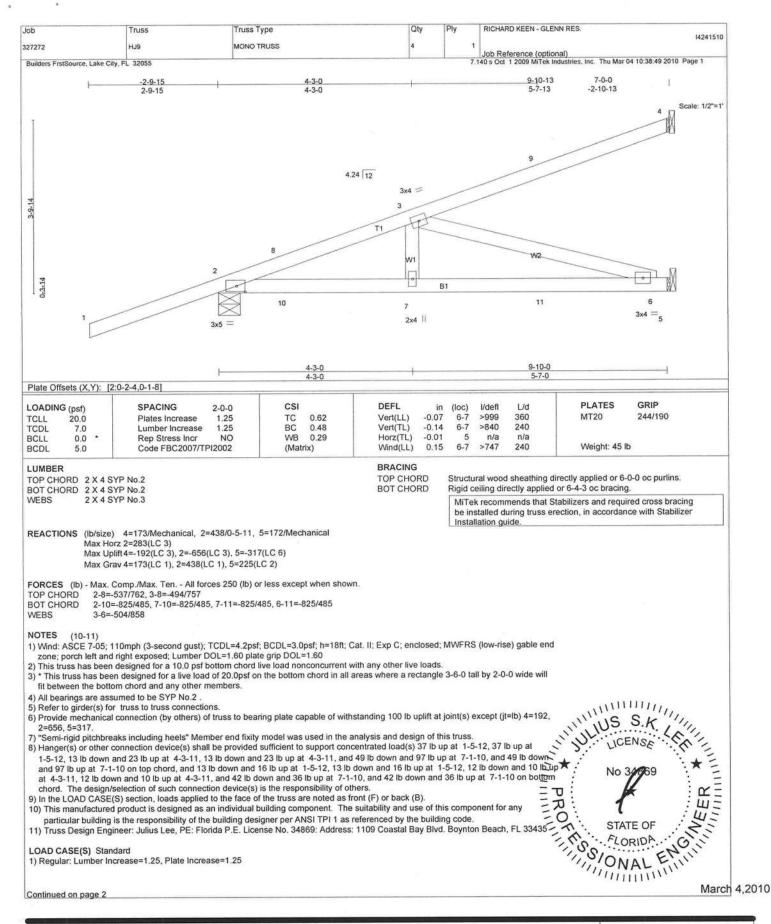
9) Truss Design Engineer: Julius Lee, PE: Flonda P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

No 34869 Ш

March 4,2010

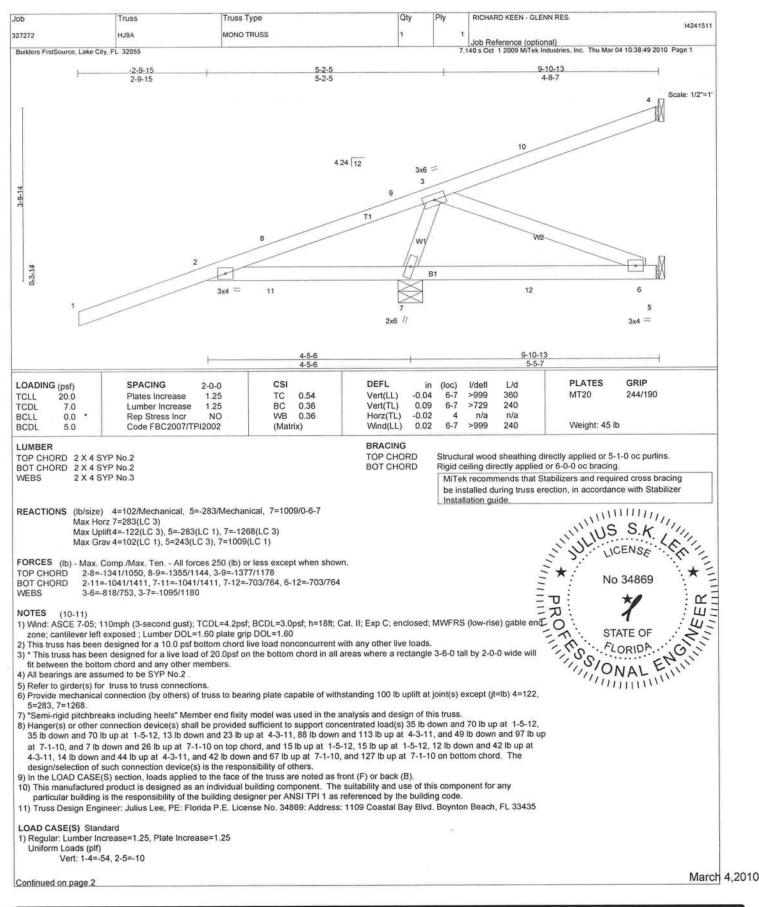
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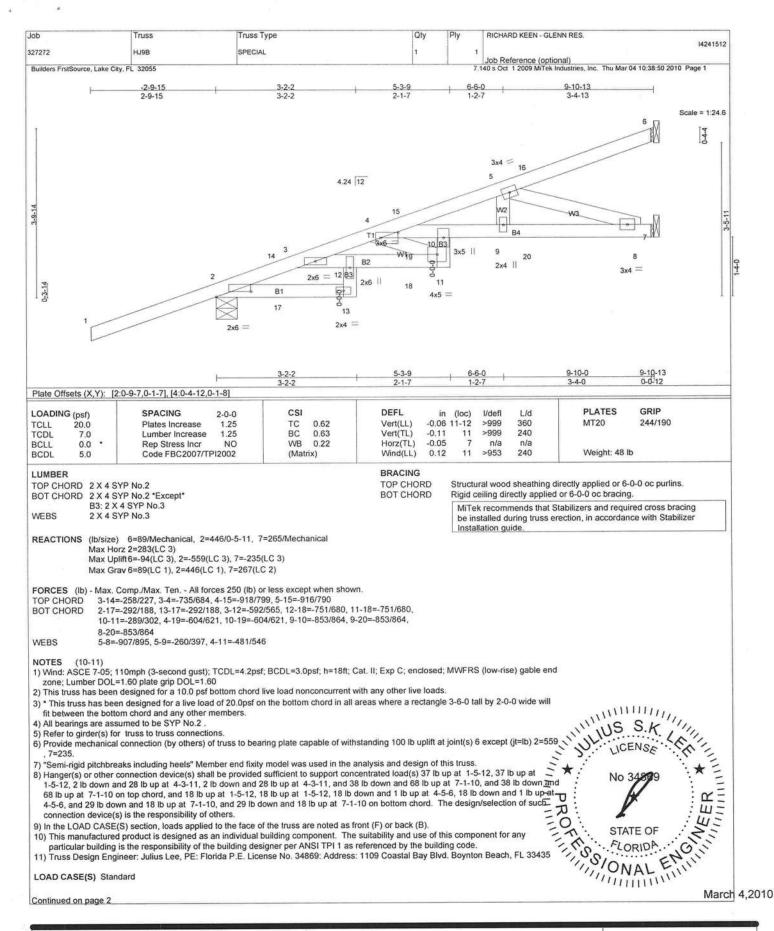
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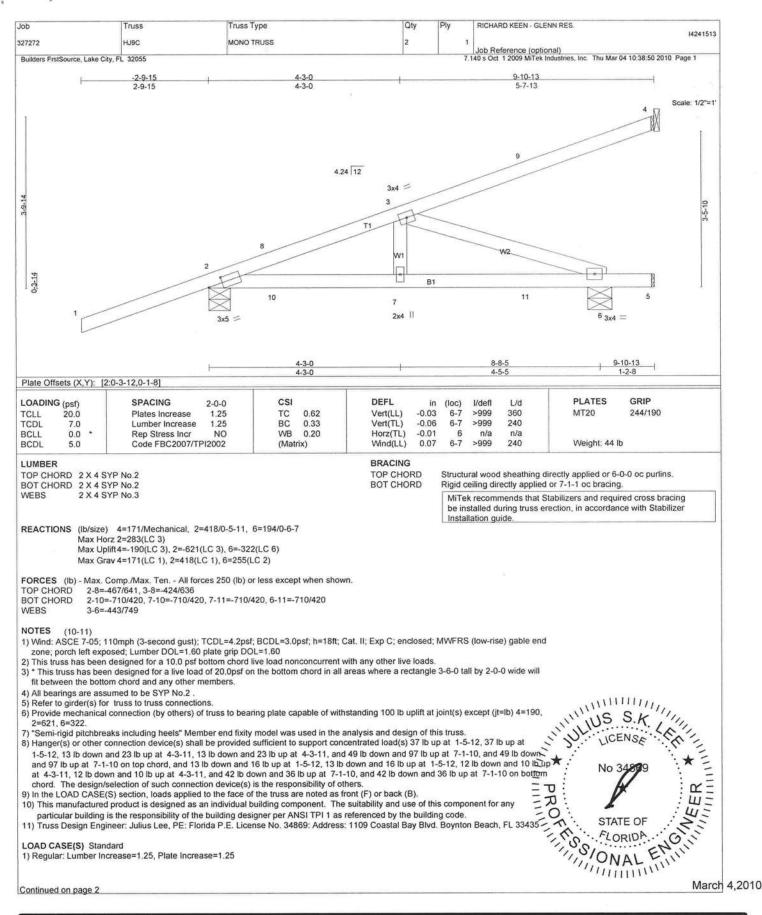
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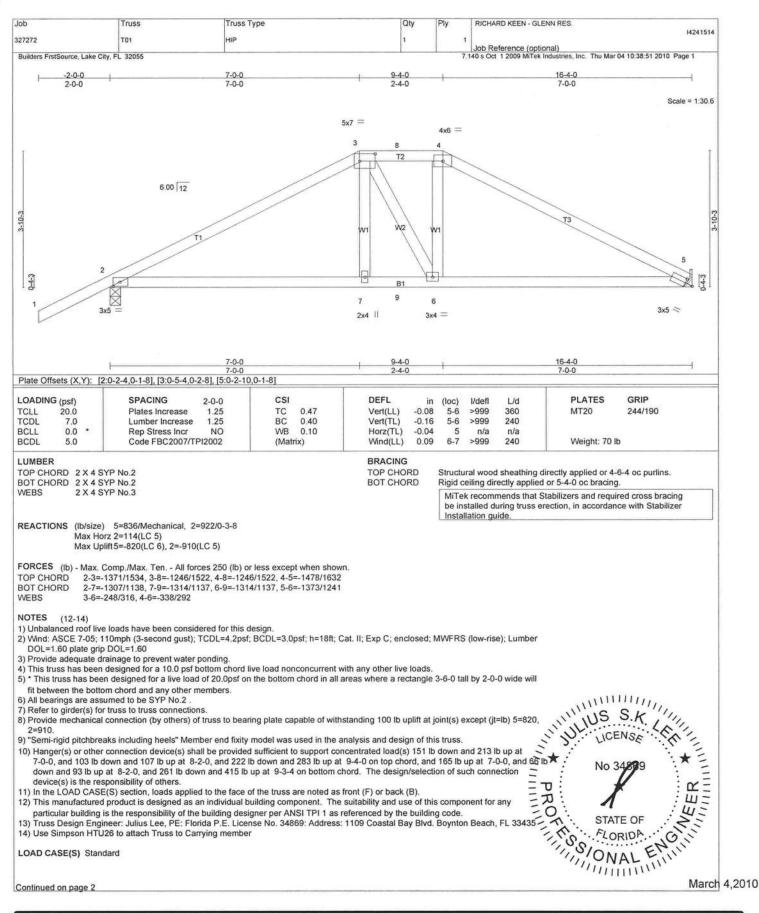
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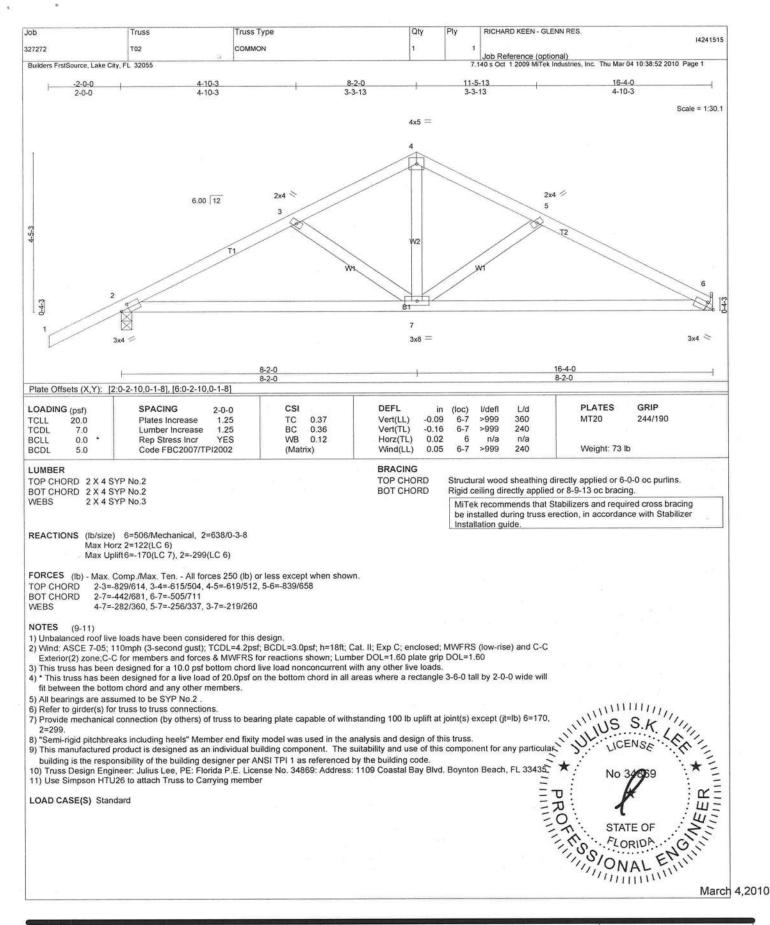
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-74-73 BEFORE USE.

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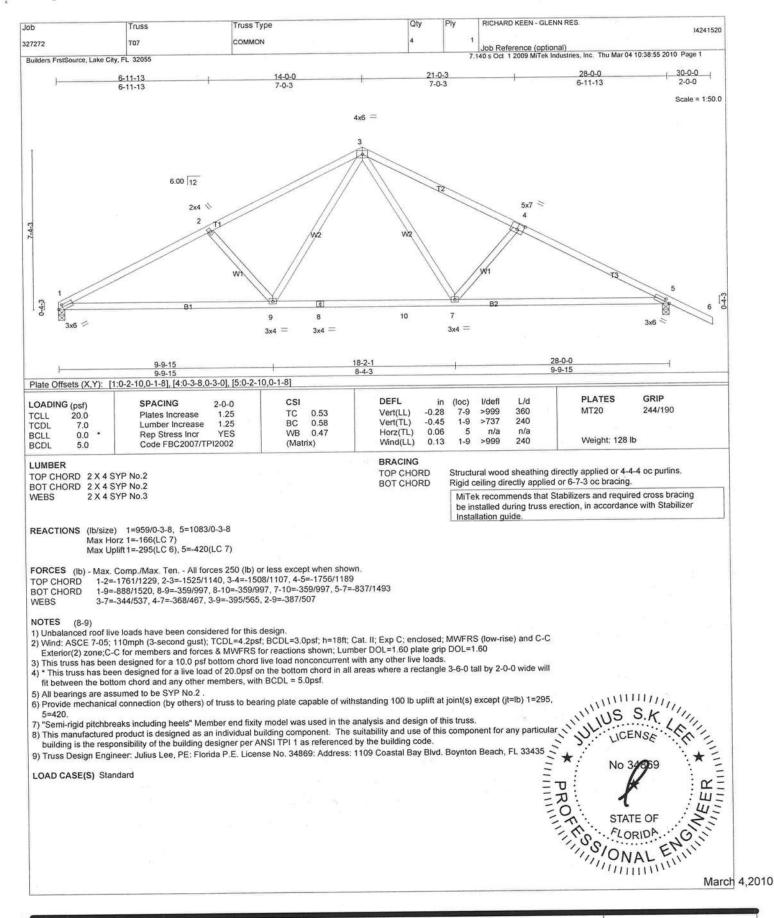
b	Truss	Truss Type	Qty	Ply	RICHARD KEEN - GLENN RES.	14241516
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building design 3) Truss Design DAD CASE(S) Regular: Lumbouniform Loads Vert: 1 Concentrated L	ner per ANSI TPI 1 as re Engineer: Julius Lee, Pl Standard er Increase=1.25, Plate (plf) .3=-54, 3-5=-54, 5-6=-54	eferenced by the building code. E: Florida P.E. License No. 34869: Ar	ddress: 1109 Coastal Bay Blv	d. Boynton	ent for any particular building is the resp	onsibility of the
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Qty RICHARD KEEN - GLENN RES Truss Type Job Touss 14241518 SPECIAL 327272 T05 Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10 38 54 2010 Page 1 Builders FrstSource, Lake City, FL 32055 26-4-0 5-4-12 28-4-0 | 30-0-0 | 32-0-0 | -2-0-0 2-0-0 12-0-0 1-8-0 5-4-14 4-0-0 4-11-4 Scale = 1:58.8 5x8 = 6x8 = 6.00 12 3x4 > 6 3x4 < 6-4-3 6-2-8 T4 12 13 15 3x5 < 16 3x4 = 3x6 = 2x4 || 3x8 = 3x5 = 3x4 5-4-14 Plate Offsets (X,Y): [2:0-2-10,0-1-8], [4:0-1-12,0-3-0], [5:0-4-10,Edge], [9:0-2-10,0-1-8] PLATES GRIP DEFL CSI LOADING (psf) SPACING in (loc) I/defl L/d MT20 244/190 Vert(LL) -0.14 12-13 >999 360 Plates Increase 1.25 TC 0.50 TCLL 20.0 Vert(TL) -0.31 12-13 >999 240 BC 0.65 TCDL 7.0 Lumber Increase 1.25 9 n/a YES WB 0.42 Horz(TL) 0.09 n/a BCLL 0.0 Rep Stress Incr 0.21 12-13 >999 240 Weight: 164 lb Code FBC2007/TPI2002 (Matrix) Wind(LL) BCDL 5.0 LUMBER BRACING Structural wood sheathing directly applied or 3-10-14 oc purlins, except TOP CHORD TOP CHORD 2 X 4 SYP No.2 2-0-0 oc purlins (5-2-3 max.): 4-5, 7-8. BOT CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 4-3-8 oc bracing. BOT CHORD 2 X 4 SYP No.3 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 2=1067/0-3-8, 9=1067/0-3-8 Max Horz 2=134(LC 6) Max Uplift 2=-421(LC 6), 9=-437(LC 7) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1779/1266, 3-4=-1340/1042, 4-5=-1167/1042, 5-6=-1357/1090, 6-7=-2179/1577, TOP CHORD 7-8=-1473/927, 8-9=-1605/1001 2-16=-930/1526, 15-16=-930/1526, 14-15=-543/1133, 13-14=-543/1133, BOT CHORD 12-13=-1037/1663, 11-12=-2123/3149, 9-11=-686/1350 3-15=-440/422, 4-15=-137/264, 5-13=-277/358, 6-13=-616/590, 6-12=-263/500, WEBS 7-12=-1279/984, 7-11=-1855/1496, 8-11=-527/752 NOTES (10-11) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C 6) All bearings are assumed to be SYP No.2.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=421.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
10) This manufactured product is designed as an individual building is the research. 7) Provide mechanical confidence of the second of the seco NAMER March 4,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

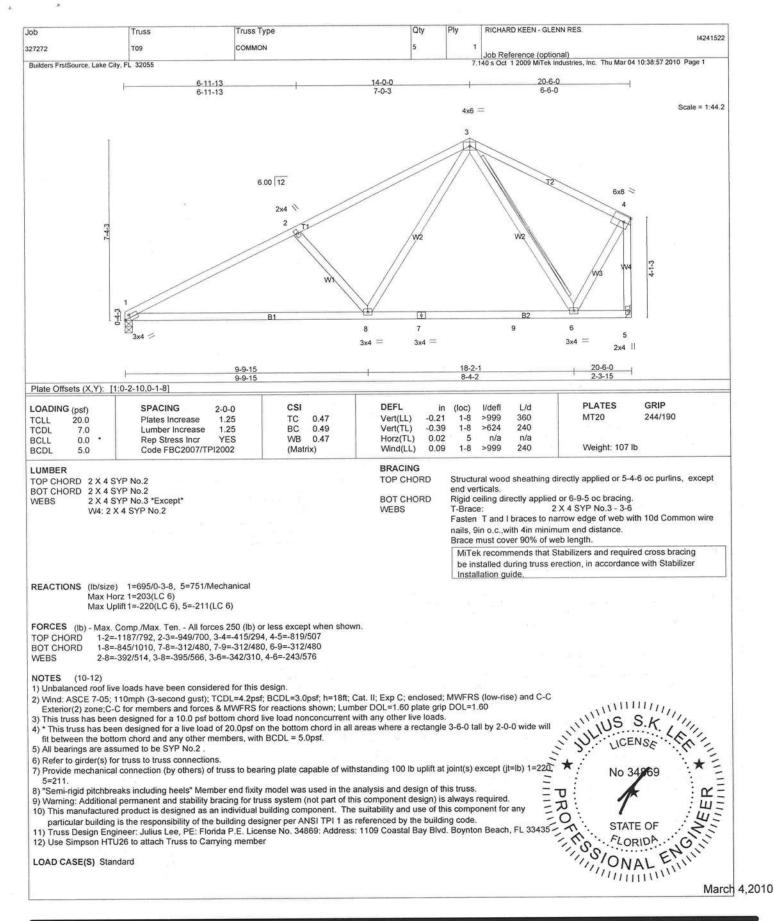
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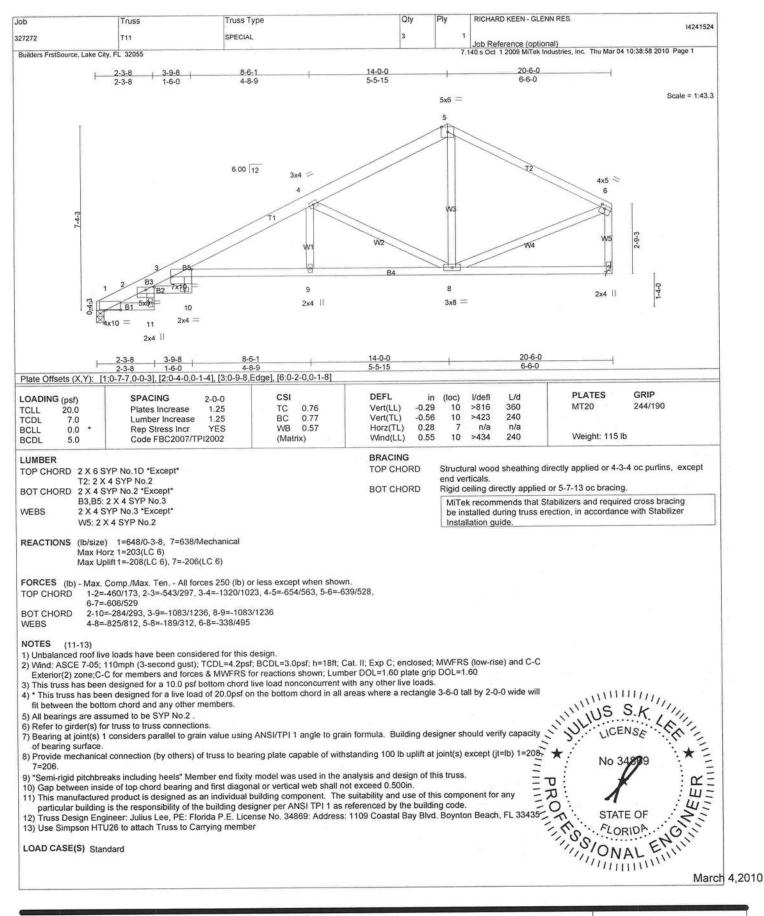
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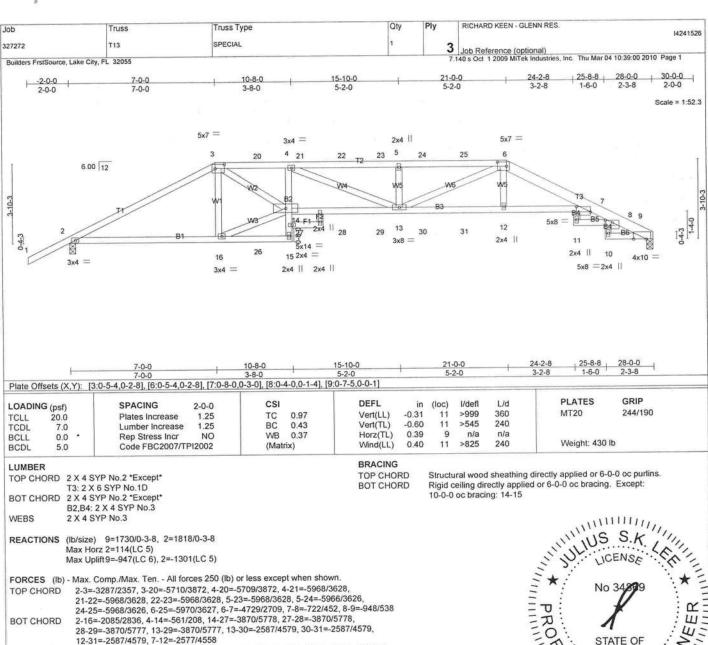
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6-13=-1077/1566, 6-12=-157/455

WEBS

NOTES (14-15)1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.

Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60

5) Provide adequate drainage to prevent water ponding.6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3-16=-828/404, 14-16=-2157/2911, 3-14=-2062/3416, 4-13=0/334, 5-13=-529/338,

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members

8) All bearings are assumed to be SYP No.2

9) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=947,

11) "Pin all pitchbreaks" Member end fixity model was used in the analysis and design of this truss

Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

No 34899

TROPING STATE OF FLORIDA. HOW

March 4,2010

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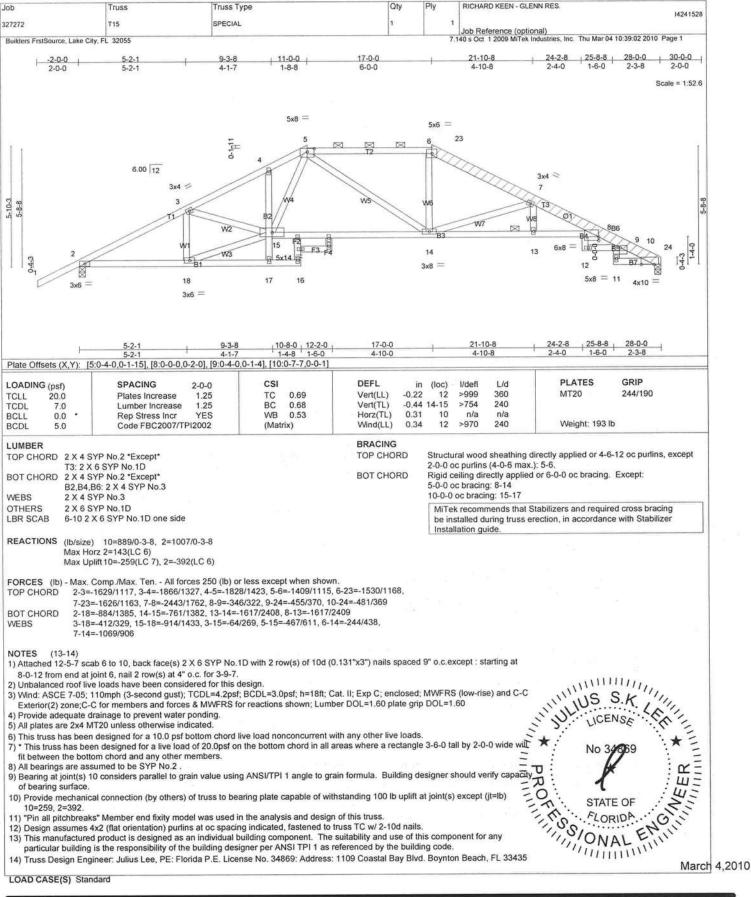
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Job Truss Type Qty Ply RICHARD KEEN - GLENN RES Truss 14241527 SPECIAL 327272 T14 Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:39:01 2010 Page 1 Builders FrstSource, Lake City, FL 32055 24-2-8 | 25-8-8 | 28-0-0 9-0-0 14-0-0 19-0-0 22-6-0 5-0-0 5-0-0 3-6-0 1-8-8 1-6-0 2-3-8 2-0-0 4-2-12 2-0-0 4-9-4 Scale = 1:53.2 4x6 || 3x8 = 5x6 = 23 6.00 12 3x4 = 3 4-10-3 W6 × W2 13 W3 14-3 3x8 = 12 18 17 16 4x10 = 3x6 = 3x6 = 10-8-0 12-2-0 14-0-0 1-4-8 1-6-0 1-10-0 Plate Offsets (X,Y): [8:0-0-0,0-2-0], [9:0-4-0,0-1-4], [10:0-7-8,0-0-5] GRIP LOADING (psf) DEFL PLATES SPACING (loc) L/d I/defl 2-0-0 Vert(LL) -0.22 13-14 360 MT20 244/190 1.25 TC 0.63 >999 TCII 20.0 Plates Increase BC 0.59 Vert(TL) -0.43 8-13 >762 240 1.25 TCDL 7.0 Lumber Increase WB 0.46 Horz(TL) 0.32 10 n/a n/a BCLL 0.0 Rep Stress Incr YES Code FBC2007/TPI2002 (Matrix) Wind(LL) 0.33 12 >996 240 Weight: 184 lb BCDL LUMBER BRACING Structural wood sheathing directly applied or 4-4-8 oc purlins, except TOP CHORD 2 X 4 SYP No.2 *Except* TOP CHORD 2-0-0 oc purlins (4-6-12 max.): 4-6. T3: 2 X 6 SYP No.1D Rigid ceiling directly applied or 5-9-2 oc bracing. Except: **BOT CHORD** BOT CHORD 2 X 4 SYP No.2 *Except B2.B4.B6: 2 X 4 SYP No.3 4-8-0 oc bracing: 8-13 2 X 4 SYP No.3 5-10-0 oc bracing: 14-15 WEBS 2 X 6 SYP No.1D 10-0-0 oc bracing: 15-17 OTHERS 6-10 2 X 6 SYP No.1D one side LBR SCAB MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide REACTIONS (lb/size) 10=889/0-3-8, 2=1007/0-3-8 Max Horz 2=128(LC 6) Max Uplift 10=-243(LC 7), 2=-375(LC 6) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1644/1110, 3-4=-1889/1297, 4-5=-1691/1243, 5-6=-1687/1261, 6-23=-1807/1316, TOP CHORD 7-23=-1867/1309, 7-8=-2753/1980, 8-9=-346/317, 9-24=-455/365, 10-24=-481/364 $2-18 = -883/1401,\ 4-15 = -382/621,\ 14-15 = -1206/2005,\ 13-14 = -1206/2005,\ 8-13 = -1856/2745$ **BOT CHORD** WEBS 3-18=-375/301, 15-18=-903/1433, 3-15=-96/331, 5-15=-469/295, 5-13=-473/270, 6-13=-361/572, 7-13=-1135/936 cc IIIIIUS S.K NOTES (13-14) 1) Attached 10-2-10 scab 6 to 10, back face(s) 2 X 6 SYP No.1D with 2 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except: starting at 5-9-14 from end at joint 6, nail 2 row(s) at 4" o.c. for 3-9-7. 2) Unbalanced roof live loads have been considered for this design 3) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 4) Provide adequate drainage to prevent water ponding. 5) All plates are 2x4 MT20 unless otherwise indicated. 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide with fit between the bottom chord and any other members. U 8) All bearings are assumed to be SYP No.2 9) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity 20 of bearing surface. 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 0 10=243, 2=375. 11) "Pin all pitchbreaks" Member end fixity model was used in the analysis and design of this truss. 12) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails. 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. March 4,2010 CONTINUES DESIGNE Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

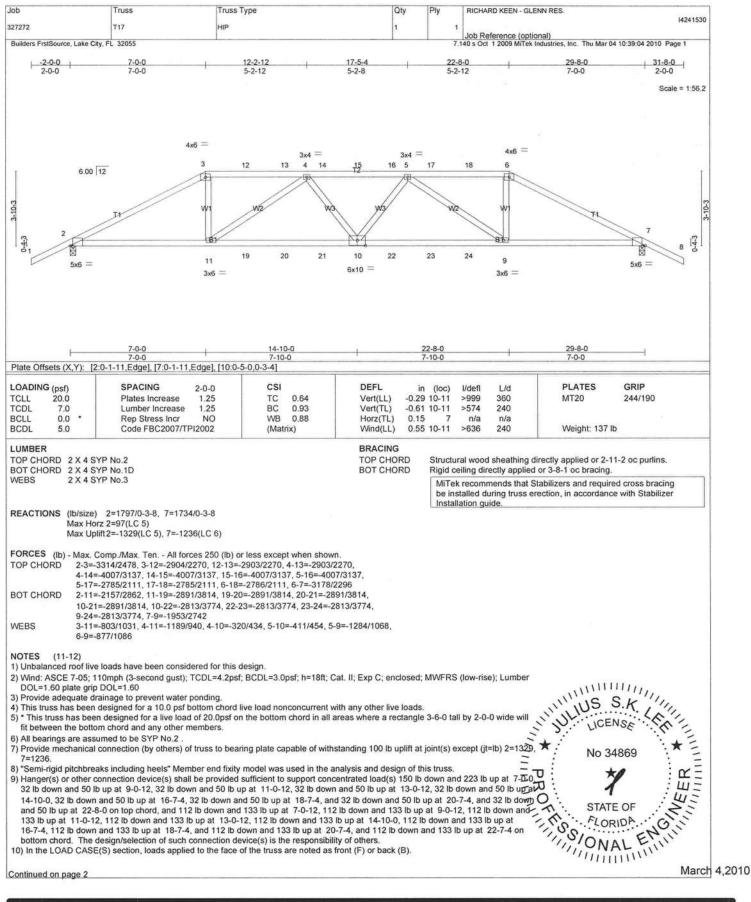
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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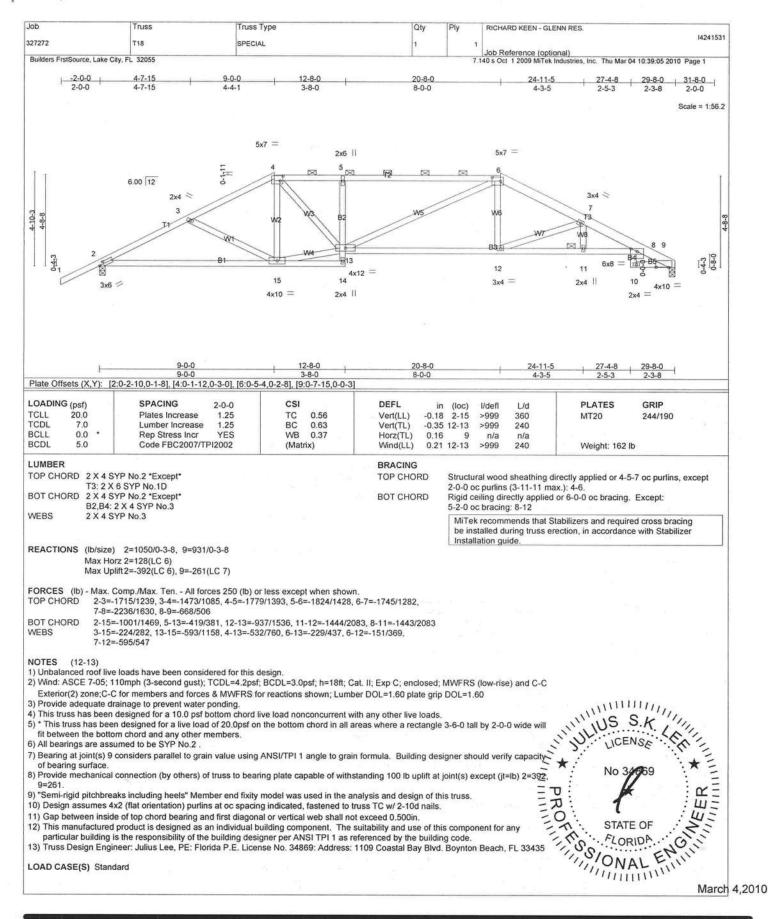
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Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - GLENN RES.	14241532
327272	T19	SPECIAL	1	1	Job Reference (optional)	10.20.00 2010 Page 2
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LOAD CASE(S) Standard

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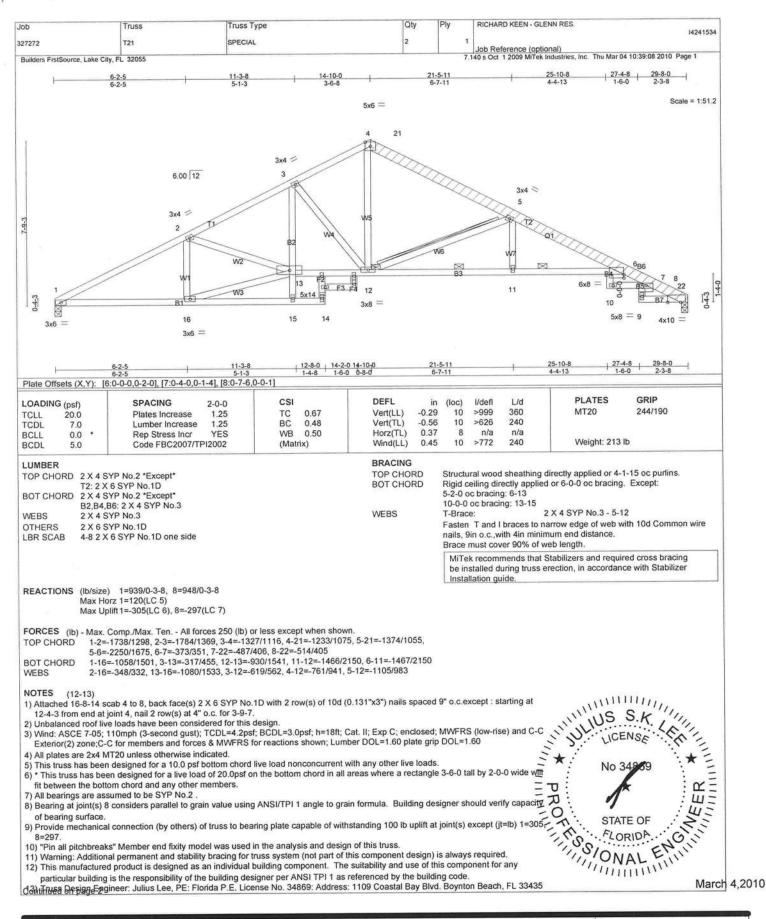
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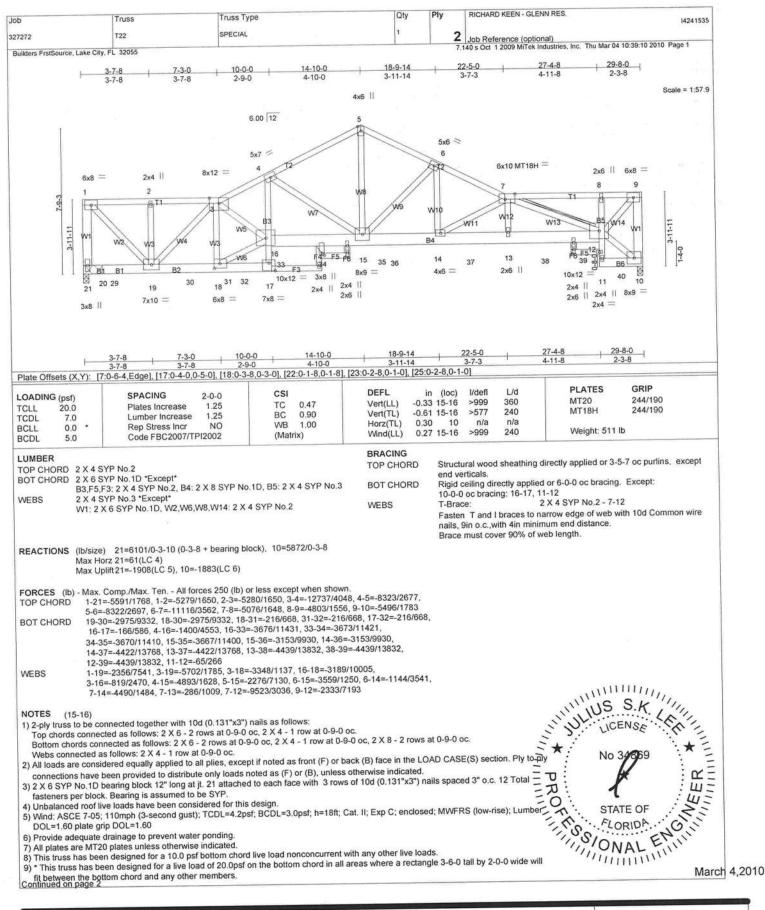


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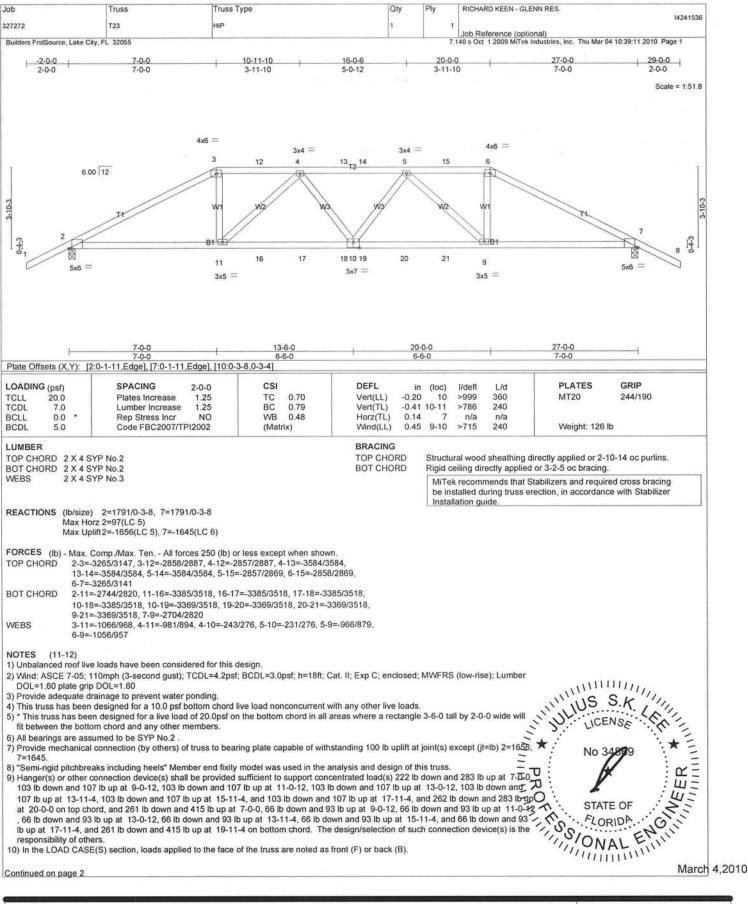
ANSI/TP1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information

available from Truss Plate Institute, \$83 D'Onofrio Drive, Modison, WI 53719.



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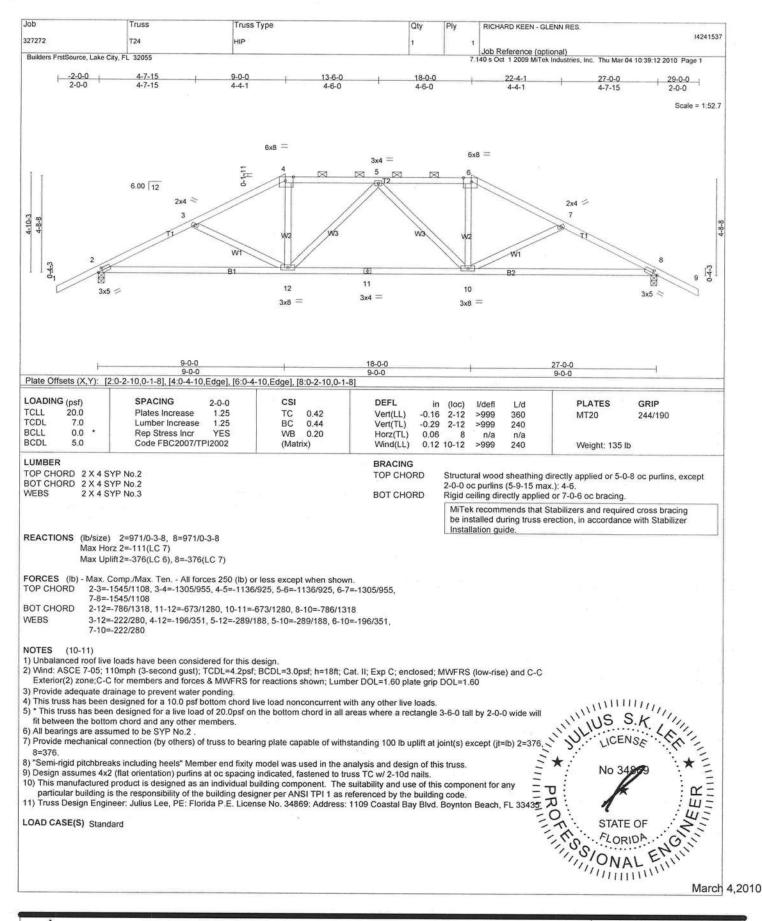
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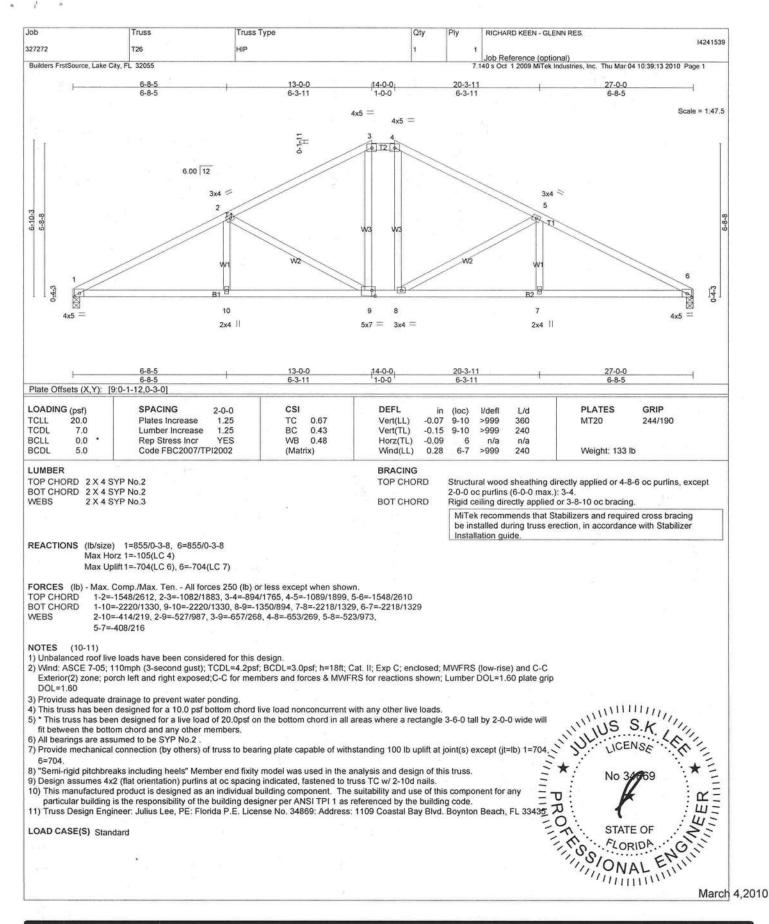
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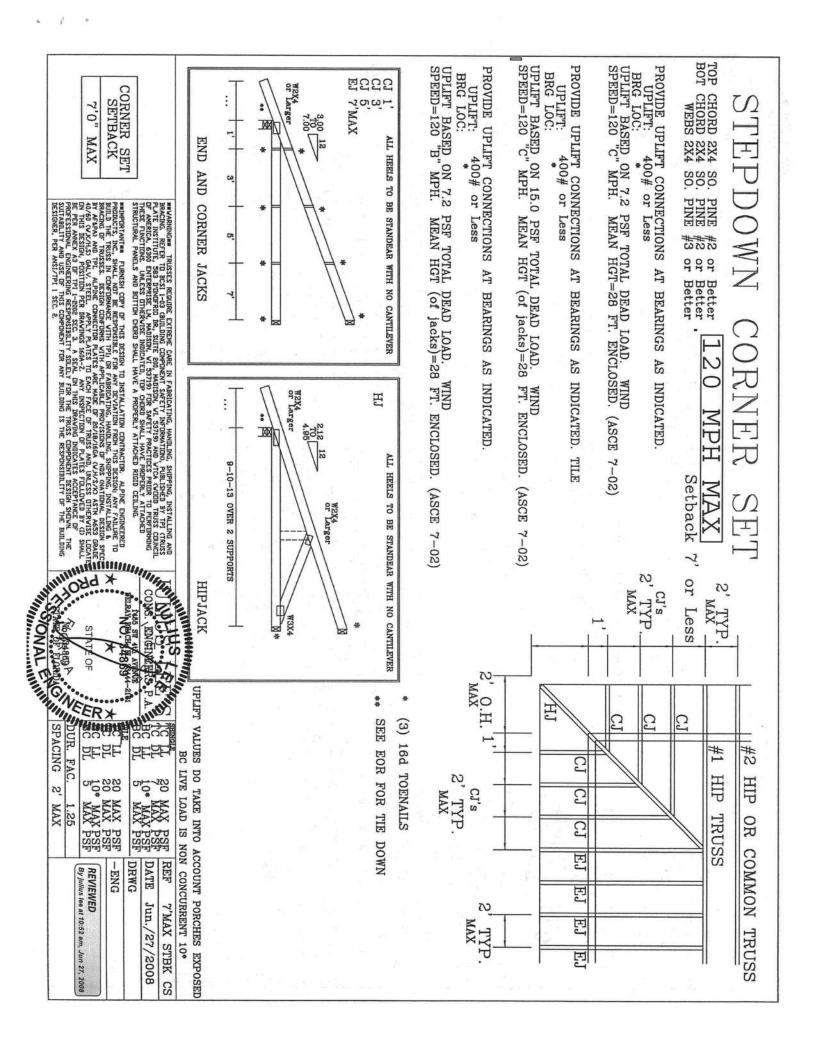
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Salety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.



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NG GROUP SPECIES AND GRADES:

GROUP

A:

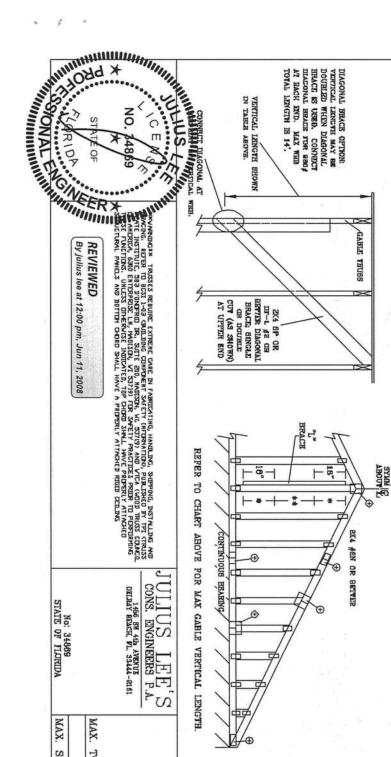
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100	4' 0"	4.	4' 2"	4' 4"	4' 5"	3' 11"	3' 11"	3' 11"	4' 0"	3' 8"	3 8	3' 9"	8' 11"	4 0"	8. 3.	8' 7"	3' 7"	3' 8"	3' 0"	3' 3*	3, 3,	3' 6"	3' 6"	2, 11,	3' 1"	3' 1"	3. 33.	BRACES	Š
	5' 6"	6' 4'	6' 6"	6' 11"	B' 11°	5' 4"	8' 3"	8' 3"	6' 11"	4' 9"	5' 6"	5' ?"	8' 4"	B' 4"	4' 8"	5' 6"	5' 5'	6' 4"	3' 10"		4' 6"	5' 6"	5' 8"	3' 9"	4' 6"	4' 5"	5, 6,	GROUP A	(1) 1X4 °
	5' 8"	8' 4"	6' 5"	7' 8"	7' 8"	6' 4"	6. 3.	8 3	7. 5.		5' 8"	6. 7.	6' 10"	B' 10"	4. B.	6' 6"	5' 5"	6. 6.	3' 10"	4' 8"	4' 6"	6' 11."	5' 11"	з′ ө.	4' 5°	4' 5"	6, 8,	GROUP B	"L" BRACE .
	7' 3"	8, 3,	e' 3°	8' 3"	8, 3,	7' 1"	A' 3"	8' 3"	8° 3°	6" 3"	7' 3"	7. 4.	7' 8"	7' 8"	6. 8.	7, 22,	7 2*	7. 6.	6' 1"	5' 11"	6. 0,	6' 6"	6, 8,	6, 0,	b' 10*	6, 10,	6' 6"	GROUP	(1) 2X4 "L" BRACE
	7' 9"	8' 6"	B' 6"	8' 11"	B' 11°	7' 1"	B' 3°	8' 3"	8. 6.	6' 3"	7' 3°	7' 4"	8' 1"	B' 1°	6' 2"	2 2*	7' 2"	7' 8"	6' 1"	5' 11"	8. 0.	7' 0"	7' 0"	5' 0"	5' 10°	5' 10"	6. 8.	A GROUP B	L" BRACE .
	8, 8,	9, 10,	9' 10"	9' 10"	B, 10°	9' 6"	9' 10"	8, 10,	9. 10.	8 ² 5"	8' 11"	8' 11"	8' 11"	8' 11"	8' 3"	8' 11"	8' 11"	8. 11	8' 11"	7' 10"	7' 10"	7' 10"	7' 10"	6, 9,	7' 10"	7' 10"	7' 10"	GROUP A	(2) 2X4 "L"
	9, 9,	10' 4"	10' 4"	10' 7"	10' 7"	9' 6"	10.00	8' 10"	10, 1,	8' 5"	8, 9,		8, 3,	8' 7"		8' 11"	8' 11"	9, 5,	e, 11,	8° 0"	8′ 1″	e' 5°	8' 5"	6' 9"	7' 10"	7' 10"	8′0°	GROUP B	BRACE **
	11' 4"	12' 11"	12' 11"	12 11	12' 11"	11' 1"	18. 10.	12' 11"	12' 11"		11. 4"	11. 6.	11, 8,	11, 9,	9. 7.	11, 1,	11' 2"	11' 9'	8' 0"	B' 3°		10' 3"	10' 3"	7' 10"	9' 1°	9' 1"	10' 3"	GROUP A	(1) 2X6 °L°
	11' 4"	13. 1.	18' 3"	13' 11°	11	11' 1"	12' 10"	12' 11"	15' 4"	8	11' 4"	11' 6"	12' B"	12' B"		11' 1"	11' 2"	12' 1"	8' 0"	8 3	9' 4"	11' 1"	11' 1"	7' 10"	9' 1"	9' 1"	10' 7"	GROUP	BRACE .
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14 O°	14. 0.	13' 3"	14' 0"	14. 0	14' 0"	14, 0,	18. 11.	14' 0"	14' 0"	14' 0"	10' 10"	12' 3"	12' 3'	12' 3"	12' 3"	10' 7"	12' 3"	12' 3"	12' 3"	B GROUP A GROUP B	(2) ZXB 'L
	14' 0"	14. 0	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14 0	14. 0	13' 3"	14 0	14 0	14' 0"	14' 0"	18, 11,	14' 0°	14' 0"	14. 0.	10' 10"	12' 8"	12' 8"	13' 2°	13' 2"	10' 7"	12' 3°	12' 3"	12' 7"	GROUP B	ZXB "L" BRACE **
OND DATE OF THE PORT OF THE PO	CARL END SUBSORTS 104	CONTINUOUS BEARING (6	PROVIDE UPLAT CONNECTE	NOT NOTECTIVE DAYS TAKE		CAHLE TRUSS I			8	100	BOUTHING PINE		71.00	1 14	The state of the s	IIORO			STADDARD	Office Code	DOUGLAS FIR-LARCH		100	41 / 42 STANDARD	ADDILICA-DINE-MB	HORD	BRACING GROUP SPE		

GROUP B: HEM-FIR

SOUTHERN PORE
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9TUD
STANDARD

STANDARD CIVIE

DOUGLAS FIR-LARCH



DIAGONAL BRACE OPTION:
VERTICAL LENUTH MAY BE
DOUBLED WHEN DIAGONAL
HRACE IS USED, CONDET
HIACONAL BRACE TOR SEG
AT EACH END, MAX WEB

SBUET TEUBS

BLE TRUSS DETAIL NOTES:

DUTINDWARDS WITH E' O" DVERBANG, OR 12" PLYWOOD OVERHANG. ID DEPLECTION CHATERIA IS L/240. UPLAIT CONNECTIONS FOR 180 FLF OVER (1001S BEARING (6 PSF TC DYAD LOAD).

ATTACE EACH 'L' BRACE WITH 104 MAILS.

\$ FOR (1) 'L' BRACE; SPACE WALLS AF 8' O.C.

\$ FOR (2) 'L' BRACE; AND 4" O.C. BETWEEN ZONES

\$ FOR (2) 'L' BRACES; SPACE WALLS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES T. BRACING MUST BE A MINIMUM OF BOX OF WEB

5	PLATES	PRAY SPIJOR AND HERE
1	2.5X4	GREATER THAN 11' 6"
	ZX4	CREATER THAN 4' D', BUT
肾	1X4 DR E	IESS THAN 4' 0°
Q	ND SPL	VEHINCAL LENGTH

			NA P		
No: 34869 STATE OF TLURIDA			DELBAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	N. HH J. N. I.
MAX.	MAX.				
MAX. SPACING 24.0"	MAX. TOT. LD. 60 PSF				
CING	ED.				
24	60				
o __	PSF	N. S.			
		-ENG	DWG x	DATE	REF
			DWG MITEK STD GABLE SO' E HI	11/26/09	ASCE7-02-GAB13030

BOT CHORD 2X4 2X4 はなは BETTER BETTER

PIGGYBACK DETAIL

TYPE

SPANS

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52

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:
110 MPH WIND, 30 MEAN HGT, ASCE 7-02, CLOSED BLDG,
LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT L EXP C. WIND TO DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 50' MBAN HGT, FBG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, BLDG, LOCATED ANYWHERE IN ROOF, CAT II, WIND TC DL=6 PSF, WIND BC DL=6 PSF EXP. C.

D

6X6 1.5X4

5X5

C

1.5X4

1.6X4 5X6

AXB **5**X4 BX8.

OR 3X6 TRULOX AT 4'

DC,

H

4XB

5X6

9X9

БХВ 3X6

>

2X4

2.5X4

2.6X4

NO. 64869

NO. 64869 FRONT FACE (B,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. LOCATION IS
ACCEPTABLE 12 X 23 20' FLAT TOP CHORD MAX SPAN A A TYP. H A B #2 OR BETTER 要 Ш J-TYP. D-SPLICE THE STATE OF THE S C C

10' TO 14' N	7'9" TO 10' N	0' TO 7'9" N	WEB LENGTH		ATTACH THULO EQUAL PER F. BE CONNECTED INFORMATION.
2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER. OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 184 NAILS AT 4° OC.	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.	NO BRACING	REQUIRED BRACING	WEB BRACING CHART	ATTACH TRULOX PLATES WITH (6) 0.120" X 1.375" NAILS, OR SQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

)	ATTACH TEETH TO THE PABRICATION. ATTACH (4) 0.120" X 1.375" NA PICGYBACK SPECIAL PI SPACE 4" OC OR LESS.
2 / / 2	0	۵	٥	•	THE PIGGYBACK ACH TO SUPPOR NAILS PER FA L PLATE TO EAC ESS.
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	۰	9	۵	0	THE TI G TRUSS PER PLY TRUSS E
	٥	a	٥	٥	T THE TIME OF (G TRUSS WITH PER PLY. APPLY TRUSS FACE AND

SIHI DRAWING REPLACES DRAWINGS 634,016 834.017 8 847,045

CONS.

STATE OF FLORIDA			DINEAN BEACH, FL. STAAA 2161	CONS. ENGINEERS P.A.	2,441 SIII III
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	50 PSF AT 1.25 DUR. FAC.	1.33	55 PSF AT	MAX LOADING
	ч	-ENG JL	DRWGMITEK STD PIGGY	DATE 09/12/07	REF PIGGYBACK

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

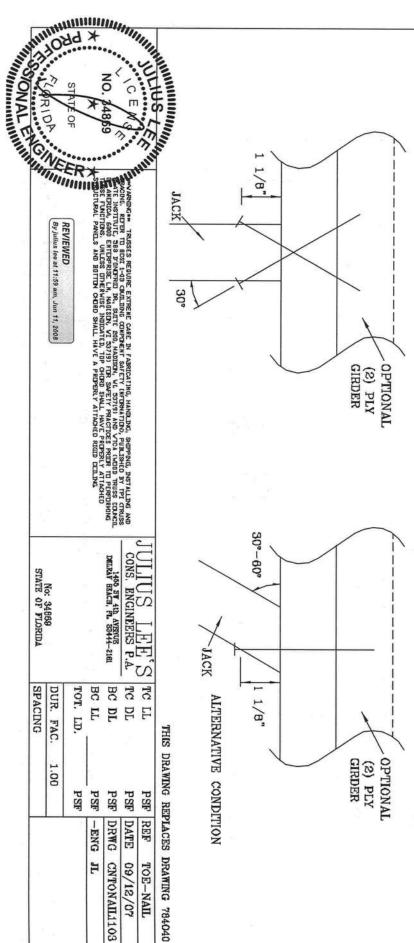
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

ALL VALUE	6	4	3	N	TOE-NAILS	NUMBER OF
YAW S	493#	394#	296#	187#	1 PLY	SOUTH
ALL VALUES MAY BE MILLTIDLIED BY APPROPRIATE DURATION OF LOAD FACTOR	639#	511#	383#	256#	2 PLIES	SOUTHERN PINE
ID AV API	452#	361#	271#	181#	1 PLY	DOUGLAS
HAINDEG	585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH
NOTTARTION	390#	312#	234#	156#	1 PLY	
A LVVI BO	507#	406#	304#	203#	2 PLIES	HEM-FIR
STOP A	384#	307#	230#	154#	1 PLY	SPRUCE
	496#	397#	298#	199#	2 PLIES	SPRUCE PINE FIR

VOLVE OF MAI DE MODILIFUED DI AFFINOFNIAIE DONALION 5 LOAD PACTOR.



TRULOX CONNECTION

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (+).

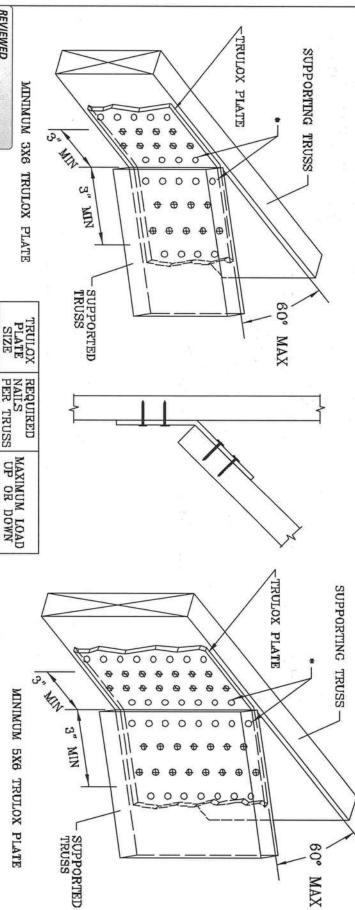
NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH NUST

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

INFORMATION NOT SHOWN THIS DETAIL FOR LUMBER, PLATES, AND OTHER REFER TO ENGINEER'S SEALED DESIGN REFERENCING

MAX



NO. 64869

OF TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANGLING, SHIPPING, INSTALLING AND REFER TO BOSI 1-03 (BUILLING COPPINCH SAFETY PROPARION, PUBLISED BY THY (TRUSS STITUTE, 593 DYNORTHI BIO, SATTE BOD, MORISHA, V.E. 3975) AND VICA CAUTH TRUSS COUNCILL, A. 6300 DYTEORRISE LM, MADISSIN, V.E. 3975) FOR SAFETY PRACTICES PRICE TO PERFORMIG TO THE CONTROL OF SAFETY PRACTICES. PRICE TO PERFORMIG LANGUAGE CONTROL OF SAFETY PRACTICES AND SOTTON CHECK SMALL HAVE A PROPERLY ATTACHED RIGID CELLING.

CONS. ENGINEERS P.A. DETRYA BEYON' 11" 30444-2191

LEE'S 1,154,844

No: 34869 STATE OF FLORIDA

REVIEWED

3X6 **6X6**

15 9

#088 350#

THIS DRAWING REPLACES DRAWINGS 1,168,989 1,158,989/R

MINIMUM 5X6 TRULOX PLATE

1,152,217 1,152,017 1,159,154 & 1,151,524

DATE REF

DRWG -ENG

> CNTRULOX1103 11/26/03 TRULOX

Ħ

PER TRUSS

MAXIMUM LOAD UP OR DOWN

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

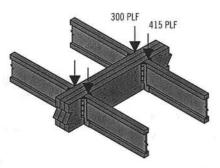
			是表现的情報。	A STORE OF WAR	Co	onnector Pattern		
	Number of	Connector	Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
Connector Type	Rows	On-Center Spacing	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	134"	11/4" 31/2"	134" 31/2" 134"	2"	134"
			3½" 2-ply	51/4" 3-ply	51/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3")	2	12"	370	280	280	245		
Nail ⁽¹⁾	3	12"	555	415	415	370	DAMES OF THE STATE OF	THE REPORT OF THE PARTY OF THE
1/2" A307		24"	505	380	520	465	860	340
hrough Bolts(2)(4)	2	19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	510	455	TOTAL CONTRACTOR	
SDS 1/4" x 31/2"(4)	2	19.2"	850	640	640	565		
		16"	1,020	765	765	680	EXECUTE AND PARTY OF THE PARTY	THE REAL PROPERTY.
		24"				455	465	455
SDS 1/4" x 6"(3)(4)	2	19.2"	CERTIFICATION OF THE PARTY OF T			565	580	565
	Maria Maria	16"	100	000	200	680	695	680
		24"	480	360	360	320	MALENTALISMENT	
USP WS35 (4)	2	19.2"	600	450	450	400		TOTAL SALES AND ADDRESS OF THE PARTY OF THE
		16"	715	540	540	480	525	350
Hen wee (2)(A)		24" 19.2"	590/05019019015015			350 440	660	440
USP WS6 (3)(4)	2	16"	Delin British British	MEGUINICES NO.		525	790	525
		24"	635	475	475	425	730	323
33/8"	2	19.2"	795	595	595	530	Ben and the second of the seco	
TrussLok(4)		16"	955	715	715	635		
		24"	300	500	500	445	480	445
5"	2	19.2"	ATOMICS SHOWING	625	625	555	600	555
TrussLok(4)		16"	PERSONAL PROPERTY.	750	750	665	725	665
		24"		EURANGA DIR	SERVICE SANS	445	620	445
63/4"	2	19.2"			-	555	770	555
TrussLok(4)		16"		THE RESERVE OF THE PARTY OF THE		665	925	665

- Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.
- (2) Washers required. Bolt holes to be %6" maximum.
- (3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic cells indicate Connector Pattern must be installed on both sides.
 Stagger fasteners on opposite side of beam by ½ the required Connector Spacing.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides
 of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16—33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 134" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 31/2" screws at 19.2" on-center.

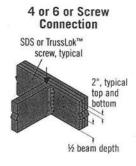
MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

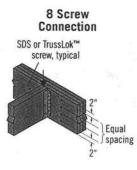
Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

				C.	onnector Pattern	A STATE OF THE PARTY OF	
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
Connector Type	Number of Connectors	13A	-11-	134" 31/2"	13' 3' 14'	1 2"	-11-
		3½" 2-ply	51/4" 3-ply	51/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-piy
	6	1,110	835	835	740		
Od (0.128" x 3")	12	2,225	1,670	1,670	1,485	ALEGE STREET, TO SEE	RESIDENCE CONTROL
Nail	18	3,335	2,505	2,505	2,225	ele in	
	24	4,450	3,335	3,335	2,965	THE PARTY OF THE P	400000000000000000000000000000000000000
SDS Screws	4	1,915	1,435(4)	1,435	1,275	1,860(2)	1,405(2)
x 31/2" or WS35	6	2,870	2,150 (4)	2,150	1,915	2,785(2)	2,110(2)
" x 6" or WS6(1)	8	3,825	2,870 (4)	2,870	2,550	3,715(2)	2,810(2)
33/8" or 5"	4	2,545	1,910 (4)	1,910	1,695	1,925(3)	1,775(3)
TrussLok™	6	3,815	2,860 (4)	2,860	2,545	2,890(3)	2,665(3)
	8	5,090	3,815 (4)	3,815	3,390	3,855(3)	3,550(3)

- (1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (2) 6" long screws required.
- (3) 5" long screws required.
- (4) 3½" and 35%" long screws must be installed on both sides.

Connections





Nail Connection 10d (0.128" x 3") nails, typical. Stagger to prevent splitting. -10" 2" spacing, typical 11/2" minimum spacing, typical There must be an equal number of

Point Load Design Example

See General Notes on page 38



First, verify that a 3-ply 1¾" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 13/4" assembly, eight 3¾" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

nails on each side of the connection

134" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d-16d (0.148"-0.162" diameter) nails. the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 33/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed
- on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.
- . Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded

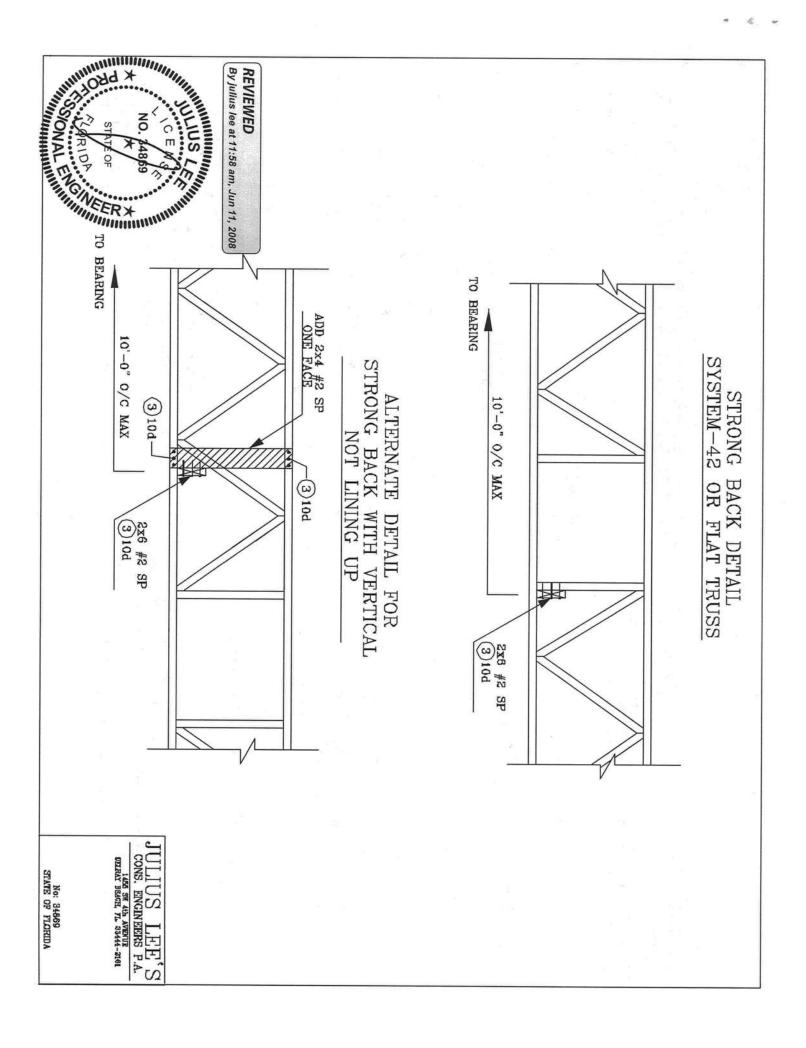
31/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"





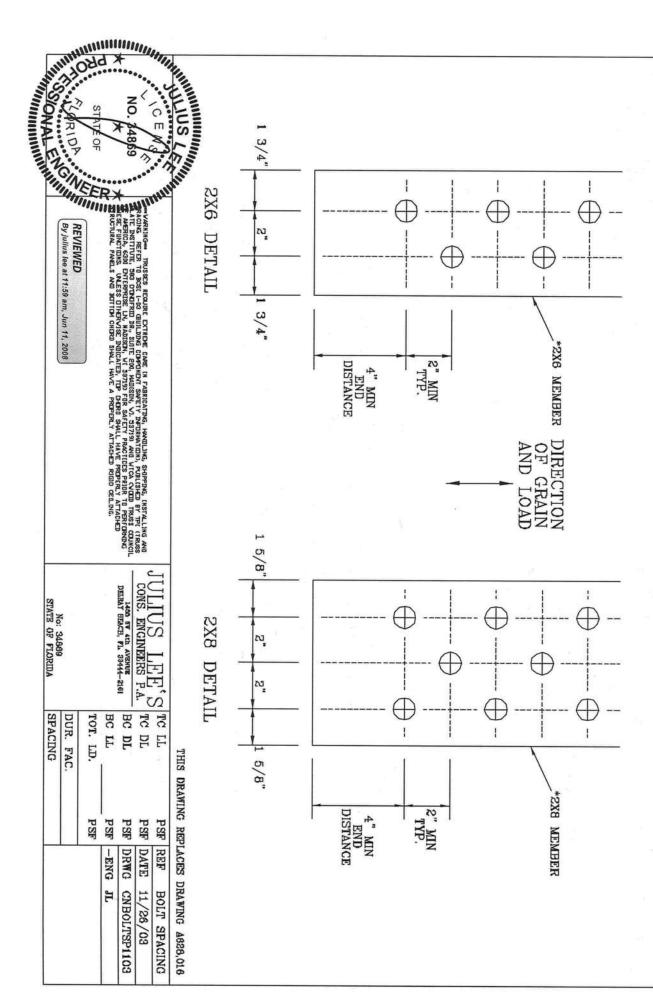
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/52" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLIS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PAITERNS SHOWN BELOW. APPLIED

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



No: 34869 STATE OF FLORIDA

SPACING

TOT. LD. DUR. FAC.

PSF

VALLEY TRUSS DETAIL

TOP WEHS CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR 2X4 SP #3 OR BETTER. BETTER

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: FHC 2004 110 MPH, ASCE 7-02 110 MPH WIND OH (3) 16d FOR ASCE 7-02 130 MPH WIND. 16' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. (2) 18d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR

EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9". UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEH, VALLEY WEH, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING.

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

ENGINEERS' SEALED DESIGN. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

: ++ LARGER SPANS NAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS HENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

CUT FROM 2X6 OR LARGER AS REQ'D

12 NAX.

W2X4

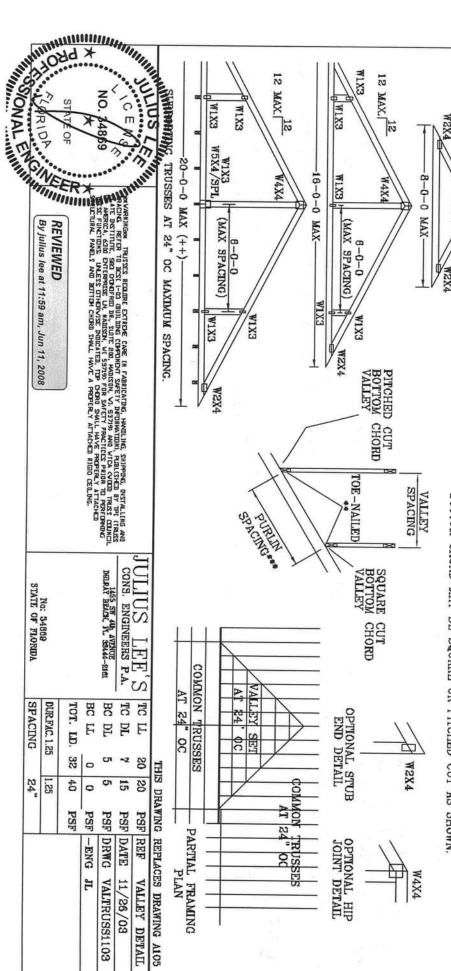
12

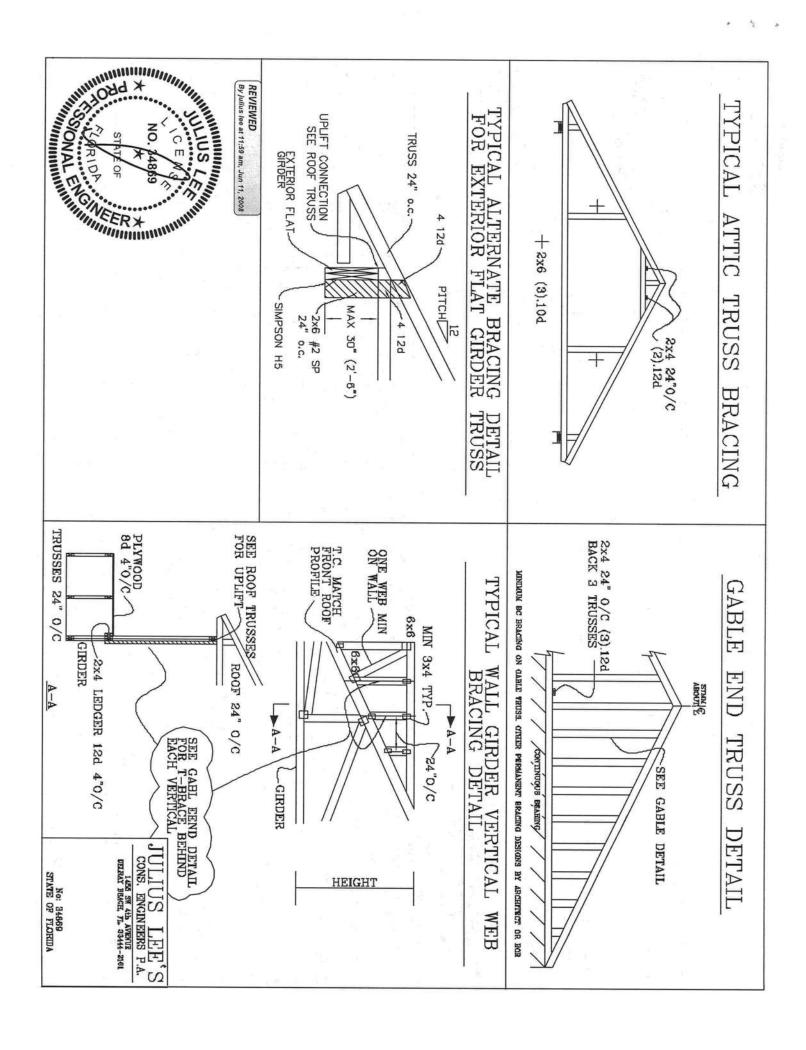
4-0-0

MAX

NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN





NO. 4869 NO. 4869 NO. 4869 NO. 4869 REPER TO CHART ABOVE FOR 1 ***MARCHAS*** TRUSSES REQUIRE EXTREME CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND PLANE (INSTITUTE, TOR EDUCATION, MADISM, VI. 237195 AND VICA VACIOUR TRUSS CHARMS (INSTITUTE, TOR EDUCATION, MADISM, VI. 237195 AND VICA VACIOUR TRUSS CHARMS (INSTITUTE, TOR EDUCATION, MADISM, VI. 237195 AND VICA VACIOUR TRUSS CHARMS (INSTITUTE, TOR EDUCATION, MADISM, VI. 237195 AND VICA VACIOUR TRUSS CHARMS (INSTITUTE, TOR EDUCATION, MADISM, VI. 237195 AND VICA VACIOUR TRUSS CHARMS (INSTITUTE, TOR EDUCATION, MADISM, VI. 237195 AND VICA VACIOUR TRUSS CHARMS (INSTITUTE HARDER) AND VICA VACIOUR TRUSS CHARMS (INSTITUTE HARDEN) AND VACIOUR TRUSS CHARMS (INSTI DIAGONAL BHACE OPTION: VERTICAL LENUTH MAX BE DOUBLED WINN DIAGONAL BHACE IS USED. CONNECT BHACE IS USED. CONNECT BHACE IS USED. WAS THE BHACE IN THE BHACE TORAL LENGTH IE 14*. LENGTH MAX GABLE VERTICAL VERTICAL LENGTH IN TABLE ABOVE. SPACING SPECIES 12" O.C. 16 O.C. O.C. GABLE VERTICAL SPF DFL SPF SPF DFI DFL SP SP H SP H ASCE NAOHB STANDARD STANDARD #1 / #8 STANDARD GRADE STANDARD STANDARD STANDARD STUD STUD STUD #2 STUD STUD B# 古話也 BRACE 7-02: 2# GABLE TRUBS BRACES 3' 9" 130 ZX4 BF #2N, DF-L #2, SPF #1/#2, OR BETTEH DIAGONAL BRACE; BINGLE OR DOUBLE CUY (AS SELWY) AT GROUP A (1) 1X4 "L" BRACE * MPH UPPER END. GROUP B WIND GROUP A (1) 2X4 "L" BRACE . SPEED, GROUP B 15 ABOUT GROUP A (2) 2X4 "L" BRACE ** 5 5 CHART ABOVE FOR MAX GABLE VERTICAL LENGTH MEAN CONLINGOR BEVERING BX4 #EN OR BETTER GROUP B 9. 10. 2° 10. 2° 60 E0 HEIGHT, 0 0 CONS. GROUP A (1) 2X0 "L" BRACE * DELRAY HEACH, FL 33444-2161 2 2 2 2 2 2 2 2 a 10 ō. No: 34869 STATE OF FLORIDA ENGINEERS P.A. ENCLOSED, GROUP B 12' 4" 18 (2) ZXB "L" HRACE ** GROUP A 12 11 2 Ñ 2 12, מז Н MAX. MAX. GROUP II 11' 8" 13' 11" 13' 7" 12' 0" 13' 3" 12' 11" TOT. SPACING 1.00, ₩ E ATTACE EACH "L" ERACE WITH 10d NAILS AF 2" O.C. \$ FOR (1) "L" BRACES SPACE NAILS AF 2" O.C. \$ FOR (2) "L" BRACES AND A" O.C. BETWEEN ZONES \$ FUR (2) "L" BRACES; BEACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. T. BRACING MUST BE A MINIMUM OF BOX OF WEB CABLE END EUPPORTS LOAD FROM 4' 0" PROVIDE UPLAT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). LIVE LOAD DEPLECTION CRITERIA IS L/240. MEMBER LENGTH. DOUGLAS FIR-LARCH #3 STUID STANDARD SPRUCE-PINE-IIR 41 / 4/2 STANDARD 43 STUD PLYWOOD OVERHANG. OUTLOOKERS WIN E' O' OVERBANG, OR 12" BRACING GROUP SPECIES 60 CABLE TRUSS DETAIL NOTES: SOUTH NORTH PUBLISHED EXPOSURE GREATER THAN 11' 6" BUT 24.0" VERTICAL LENGTH PEAK, SPLICE, AND HEEL PLATES. CABLE VERTICAL PLATE SIZES PSF DATE REF DRWG MIEK SID GABLE 16 E HI HI & BIR GROUP B: GROUP C DOUGLAS FIR-LARCH 11/26/03 ASCEY-02-CAB13015 A: SOUTHERN PINE IX4 OR EXS AND STANDARD 2.5X4 8110 XXX STANDARD GRADES: GIVIS

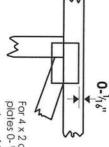
Symbols

PLATE LOCATION AND ORIENTATION



and fully embed teeth Dimensions are in ft-in-sixteenths.

Apply plates to both sides of truss offsets are indicated Center plate on joint unless x, y



For 4×2 orientation, locate plates 0- $\frac{1}{108}$ " from outside edge of truss.

connector plates required direction of slots in This symbol indicates the

*Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

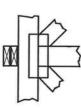
the length parallel to slots width measured perpendicular to slots. Second dimension is The first dimension is the plate

LATERAL BRACING LOCATION



if indicated. output. Use T, I or Eliminator bracing Indicated by symbol shown and/or by text in the bracing section of the

BEARING



number where bearings occur. reaction section indicates joint Indicates location where bearings (supports) occur. Icons vary but

Industry Standards:

ANSI/TPII:

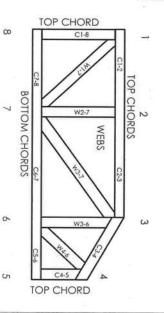
DSB-89: BCSII:

Plate Connected Wood Truss Construction. National Design Specification for Metal Design Standard for Bracing.

Installing & Bracing of Metal Plate Connected Wood Trusses. Guide to Good Practice for Handling. Building Component Safety Information

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

9730, 95-43, 96-31, 9667A NER-487, NER-561 95110, 84-32, 96-67, ER-3907, 9432A ESR-1311, ESR-1352, ER-5243, 9604B

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Boynton, FL 33435 Julius Lee l 109 Coastal Bay Blvd.



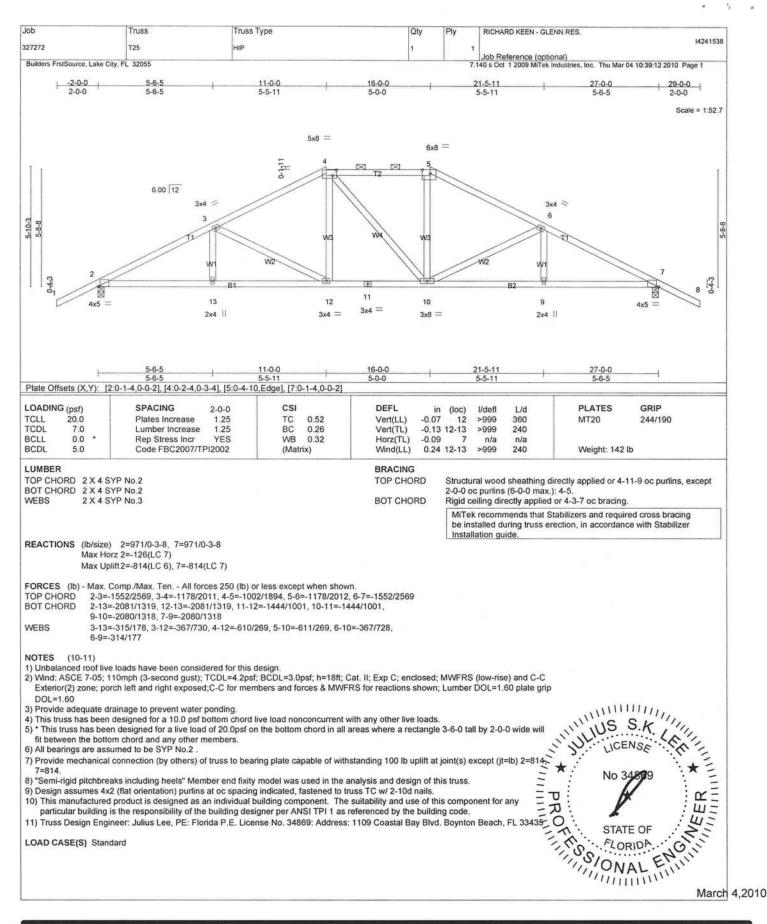
General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSII.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.

2

- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, properly owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- 10 responsibility of truss fabricator. General practice is to Camber is a non-structural consideration and is the camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted
- 15. Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flabrication, quality control, storage, delivery, erection and bracing, consult MSIV/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, S83 Dronofs Drive, Madison, WI S3719.

	Truss	Truss Type	Qty	Ply	RICHARD KEEN - GLENN RES	
7272	T23	HIP	1	EX. 33	1	1424153
uilders FrstSource, La	ike City, FL 32055				Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 1	10.20-11.2010 D
2) Truss Design E DAD CASE(S) S Regular: Lumbe	Engineer: Julius Lee, PE: Flo Standard r Increase=1.25, Plate Incre	orida P.E. License No. 34869: A	ent. The suitability and use of Address: 1109 Coastal Bay B	this compo	nent for any particular building is the respor	nsibility of the
Vert: 1-3 Concentrated Lo	plf) 3=-54, 3-6=-54, 6-8=-54, 2-7 pads (lb)	'=-10	I4(B) 12=-103(B) 13=-103(B)	14=-103(B)	15=-103(B) 16=-32(B) 17=-32(B) 18=-32(B)) 19=-32(B)
					No 348	K LEN 1
					ROSTATE	OF W

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - GLENN RES.	
327272	T22	SPECIAL	1			14241535
Builders FrstSource, Lake City, I	FL 32055				Job Reference (optional)	9:10:2010 Page 2

NOTES (15-16)

10) All bearings are assumed to be SYP No.2.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 21=1908, 10=1883.

12) "Semi-rigid pitchbreaks including heets" Member end fixity model was used in the analysis and design of this truss.

12) Sethi-rigid prichibreaks including heets including heets when he had seen in the alrays and design of this buss.

13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 741 lb down and 217 lb up at 1-7-12, 741 lb down and 217 lb up at 5-7-12, 741 lb down and 217 lb up at 7-7-12, 741 lb down and 217 lb up at 8-6-4, 628 lb down and 212 lb up at 14-6-4, 626 lb down and 212 lb up at 18-6-4, 636 lb down and 212 lb up at 18-6-4, 636 lb down and 216 lb up at 22-6-4, 636 lb down and 217 lb up at 18-6-4, 636 lb down and 218 lb up at 24-6-4, 636 lb down and 218 lb up at 24-6-4, 636 lb down and 219 lb up at 24-6-4, 636 lb down down and 206 lb up at 26-6-4, 735 lb down and 209 lb up at 28-6-4, and 619 lb down and 204 lb up at 10-6-4, and 619 lb down and 204 lb up at 12-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

14) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

15) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.16) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

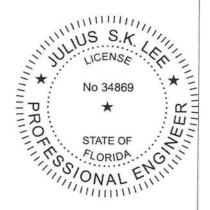
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 5-7=-54, 7-9=-54, 17-21=-10, 12-16=-10, 10-11=-10

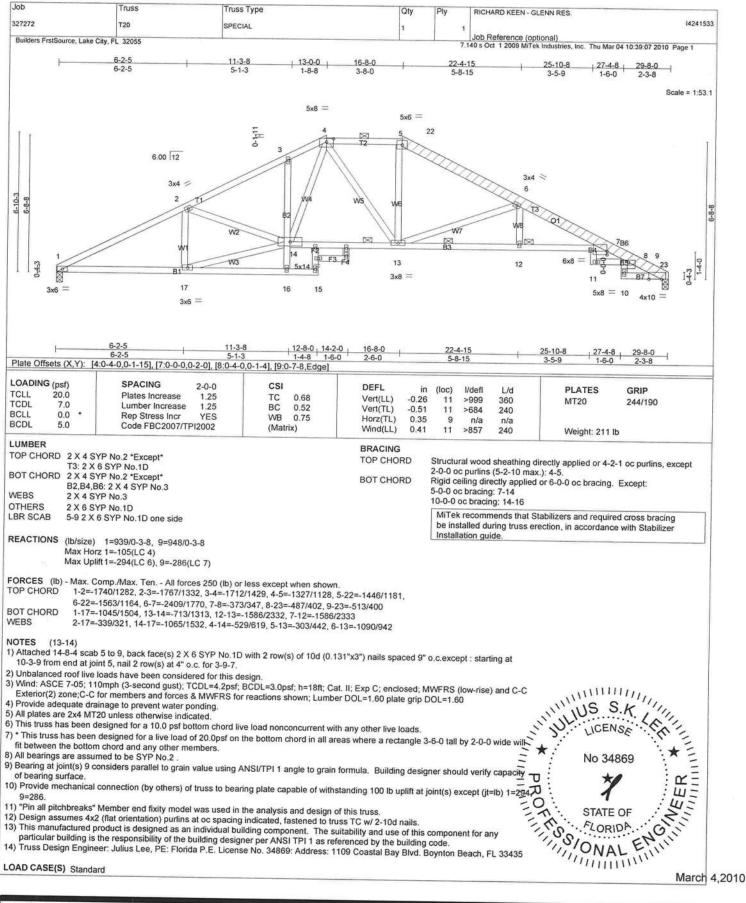
Concentrated Loads (lb)

Vert: 19=-741(B) 14=-628(B) 13=-636(B) 29=-741(B) 30=-741(B) 31=-741(B) 32=-741(B) 33=-619(F) 34=-619(F) 35=-628(B) 36=-628(B) 37=-636(B) 38=-636(B) 39=-636(B) 40=-735(B)





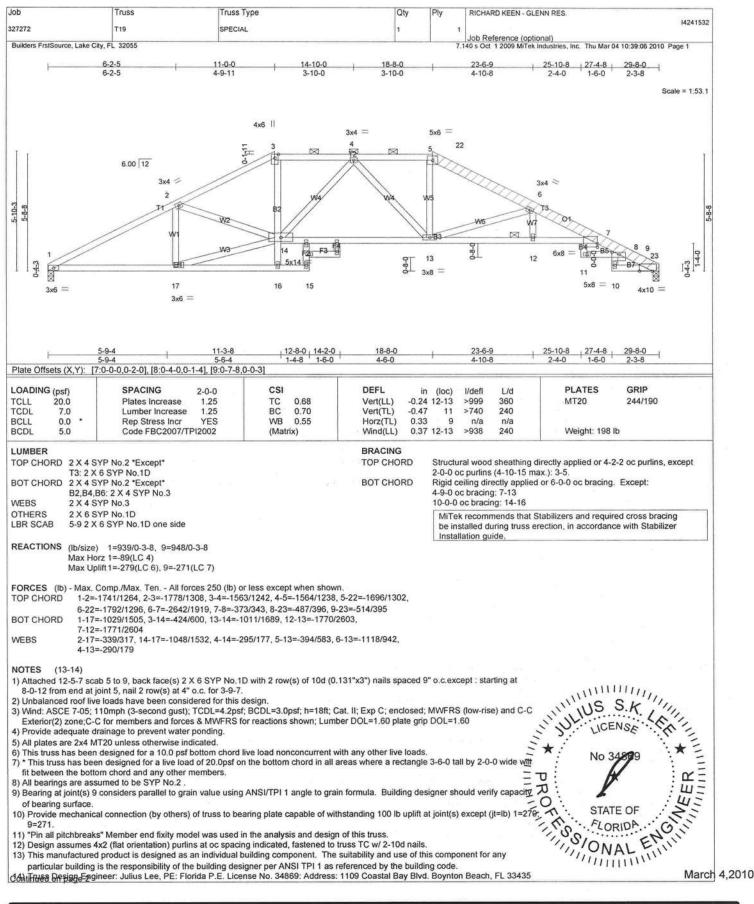
Job	Truss	Truss Type	 Qty	Ply	RICHARD KEEN - GLENN RES.	
327272	T21	SPECIAL	2	1		14241534
Builders FrstSource, Lake City,	600000	and extension		7.1	Job Reference (optional) 40 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 1	0:39:08 2010 Page 2
LOAD CASE(S) Standar	d					
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

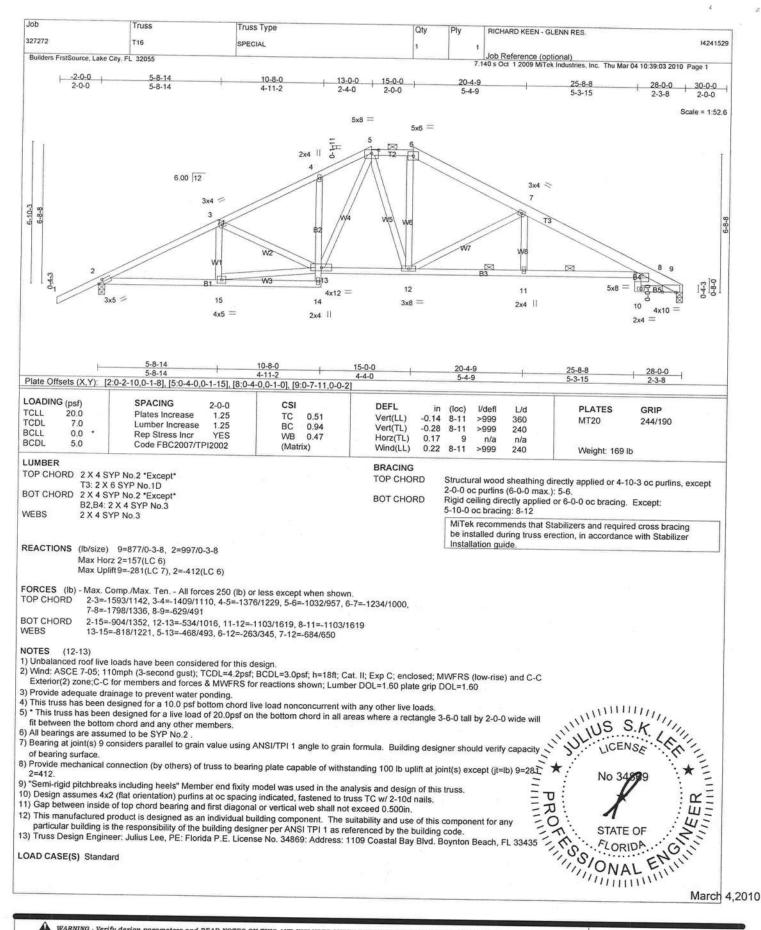
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the responsibility of the parameters of the control of the control of the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult AMS/ITPI Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Iruss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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27272	Truss	Truss Type		Qty	Ply	RICHARD KEE	N - GLENN RES.	
	T17	HIP		1		1		1424153
Builders FrstSource, Lake City, F	1 32055					Job Reference	e (optional)	
1) This manufactured probuilding designer per visual probusing the probusing the probusing the probusing designs and probusing the probusing	oduct is designed a ANSI TPI 1 as refe or: Julius Lee, PE: I d isse=1.25, Plate Inc 8-6=-54, 6-8=-54, 2) 6=-32(F) 10=-112	renced by the building of Florida P.E. License No crease=1.25	ode. . 34869: Address: 1109	Coastal Bay Blv	his compo	nent for any pa	MiTek industries, inc. Thu Mar 0 riticular building is the resp 435 (F) 19=-112 20=-112 21=-	onsibility of the
							No. STA	S.K. S.K. SA869 * TE OF ORIDA ORIDA ORIDA AL ENSE
							- 7111	11111



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component, Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insures stability during construction is the responsibility of the eractor. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/IPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply RICHARD KEE	N - GLENN RES.	
327272	T14	SPECIAL	1	1		14241527
Builders FrstSource, Lake City, F	FL 32055			7.140 s Oct 1 2009	e (optional) MiTek Industries, Inc. Thu Mar 04 10:39:01 2010	Page 2
LOAD CASE(S) Stondar						1.0
LOAD CASE(S) Standard	1					
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					WILLS S.K	11,
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						March 4

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSS-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ob	Truss	Truss Type	Qty	Ply	RICHARD KEEN - GLENN RES.
27272 Builders FrstSource, La	T13	SPECIAL	1	3	Job Reference (optional)
NOTES (14-15)				lh dawa and	d 283 lb up at 7-0-0, 103 lb down and 107 lb up at 9-0-12,
13) Hanger(s) or o	ther connection device(s)	Shall be brovided sufficient to support co			

building designer per ANSI TPI 1 as referenced by the building code.

15) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

down and 213 lb up at 20-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

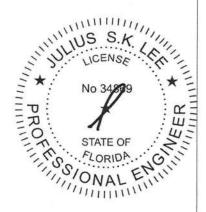
Uniform Loads (plf)

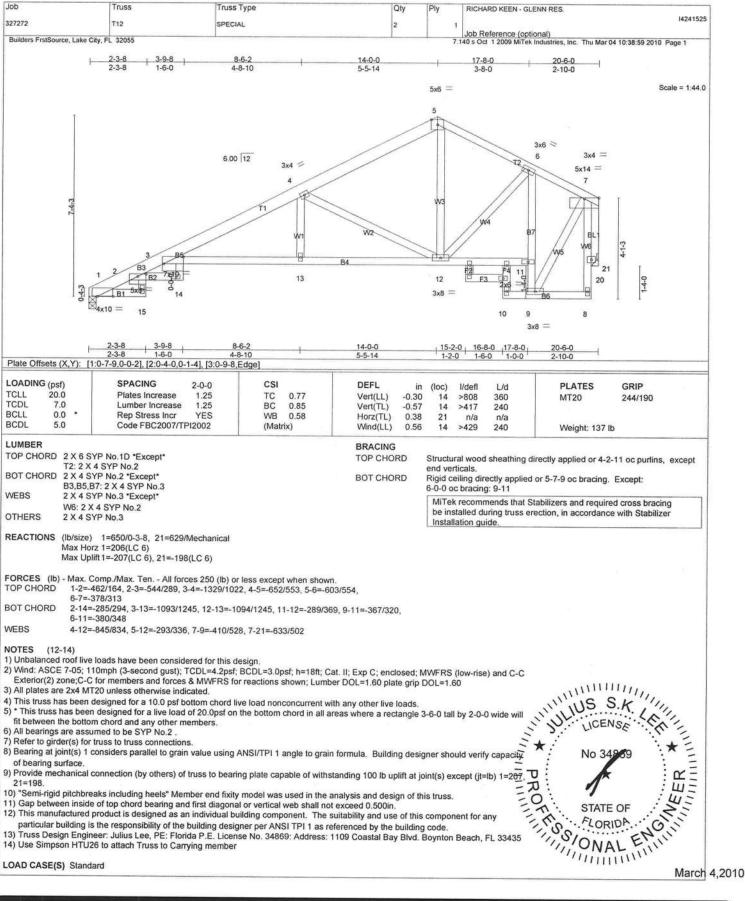
Vert: 1-3=-54, 3-6=-54, 6-7=-54, 7-8=-54, 8-9=-54, 2-15=-10, 7-14=-10, 8-11=-10, 9-10=-10

Concentrated Loads (lb)

Vert: 3=-222(B) 6=-107(B) 16=-194(B) 12=-322(B) 20=-103(B) 21=-92(B) 22=-92(B) 23=-92(B) 24=-92(B) 25=-92(B) 26=-32(B) 27=-46(B) 28=-46(B) 29=-46(B) 30=-46(B) 31=-46(B)

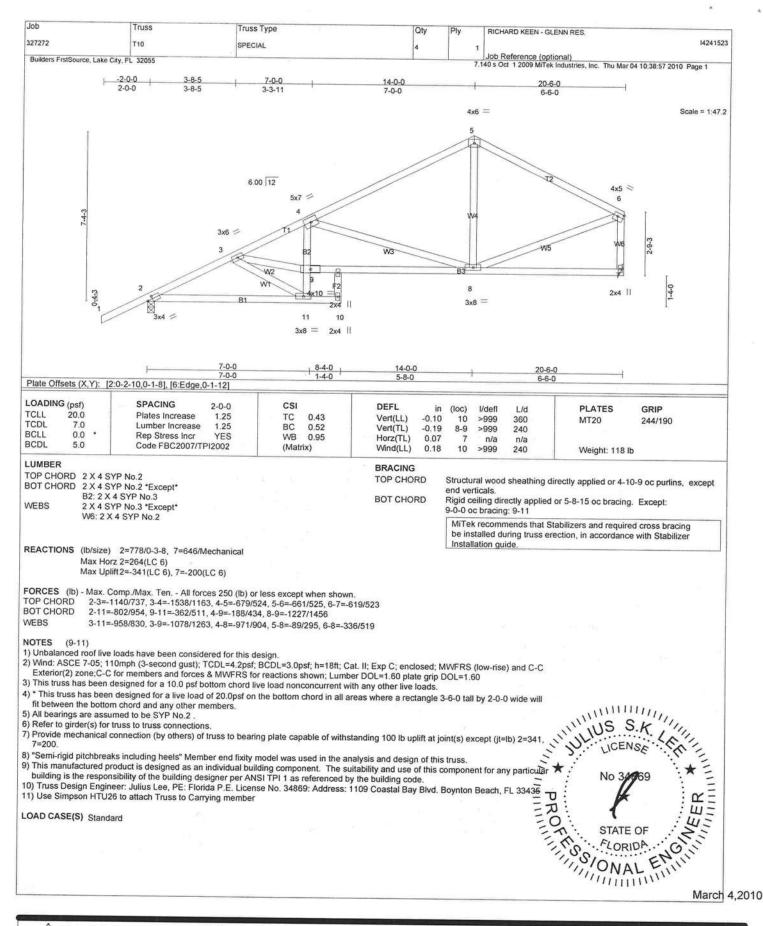
14) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the





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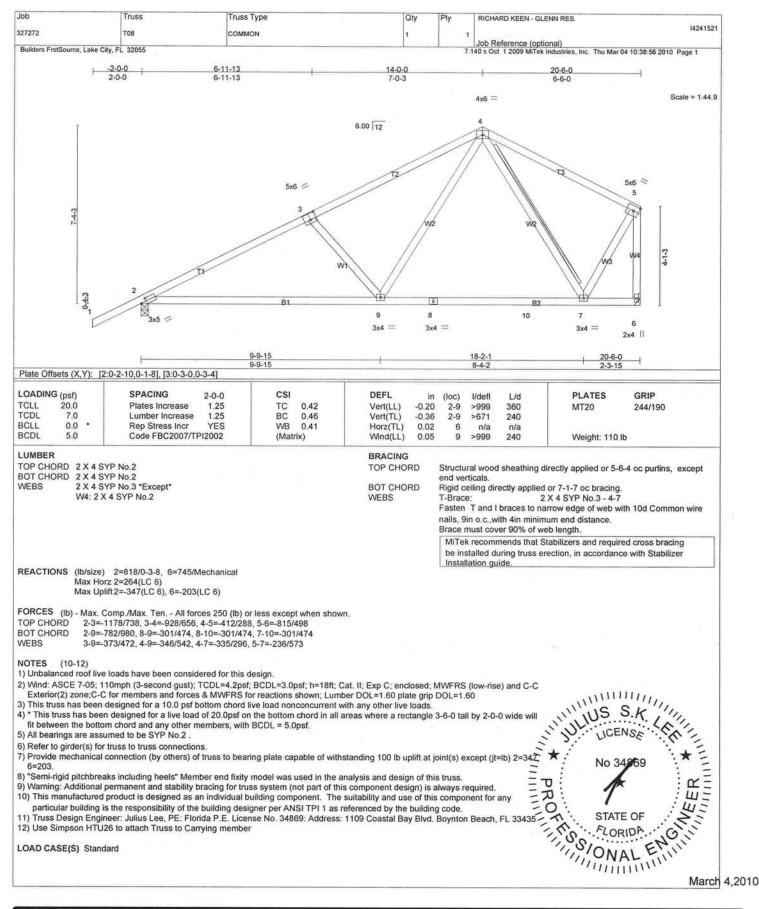


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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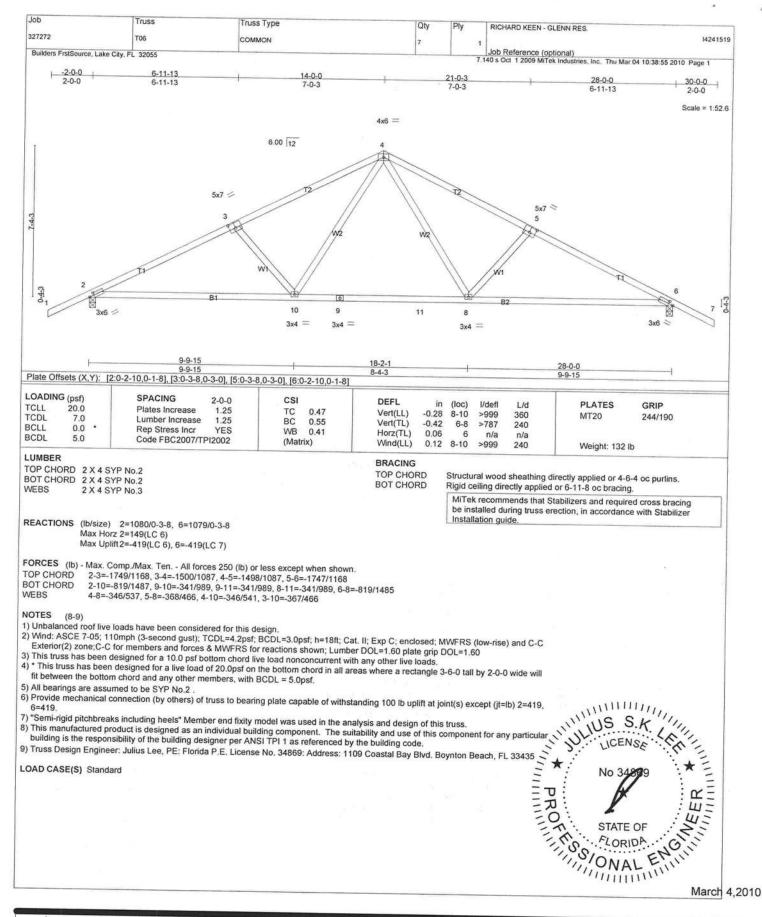
Applicability of design parameters and proper incorporation of component is responsibility of building designer- not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the insure stability during construction is the responsibility of the building designer. For general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult.

AMSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Salety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



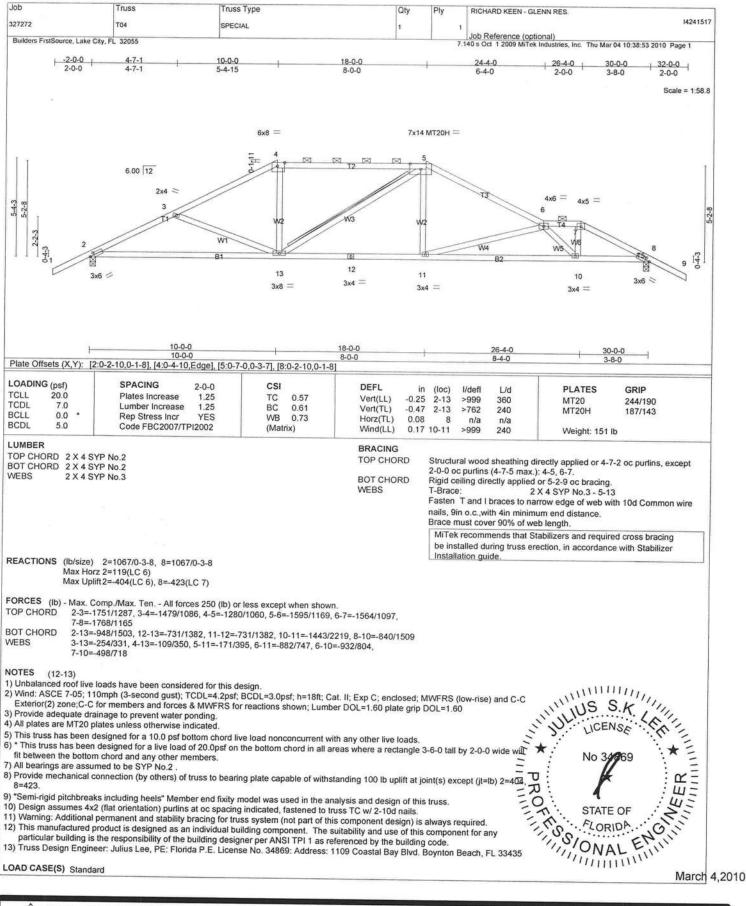
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Job Truss Truss Type Qty Ply RICHARD KEEN - GLENN RES 14241516 327272 тоз SPECIAL 2 Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:38:53 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -2-0-0 30-0-0 5-6-8 8-0-0 14-0-0 20-0-0 22-5-8 24-5-8 2-0-0 8-0-0 6-0-0 6-0-0 2-5-8 2-0-0 2-0-0 Scale = 1:57.8 5x7 = 2x4 || 5x7 = 3 15 14 4x5 = 6.00 12 4x5 = 3-1-7 16 17 19 10 21 12 13 11 3x6 = 4x5 > 2x4 || 7x8 = 3x5 = 3x5 = 6-0-0 Plate Offsets (X,Y): [3:0-5-4,0-2-8], [5:0-5-4,0-2-8], [12:0-4-0,0-4-8] LOADING (psf) SPACING CSI DEFL 2-0-0 **PLATES** GRIP in (loc) I/defl L/d TCLL 20.0 Plates Increase 1.25 TC Vert(LL) 0.37 -0.17 11-12 >999 360 MT20 244/190 1.25 TCDL 70 Lumber Increase BC 0.56 Vert(TL) -0.32 11-12 240 >999 BCLL 0.0 Rep Stress Incr NO WB 0.35 Horz(TL) 0.06 n/a n/a BCDL Code FBC2007/TPI2002 5.0 (Matrix) Wind(LL) 0.21 11-12 >999 240 Weight: 347 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 5-11-1 oc purlins. TO STATE OF FLORIDA. BOT CHORD 2 X 6 SYP No.1D **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS 2 X 4 SYP No.3 REACTIONS (lb/size) 2=1880/0-3-8, 8=2502/0-3-8 Max Horz 2=107(LC 5) Max Uplift 2=-934(LC 5), 8=-1373(LC 6) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown 2-3=-3446/1663, 3-4=-4736/2560, 4-14=-4736/2560, 14-15=-4736/2560, 5-15=-4736/2560, TOP CHORD 5-6=-5141/2731, 6-7=-4321/2299, 7-8=-4803/2475 **BOT CHORD** 2-13=-1436/2983, 12-13=-1432/2983, 12-16=-2384/4668, 16-17=-2384/4668, 17-18=-2384/4668, 11-18=-2384/4668, 11-19=-2724/5348, 19-20=-2724/5348, 10-20=-2724/5348, 10-21=-2134/4220, 8-21=-2134/4220 WEBS 3-12=-1349/2211, 4-12=-378/244, 5-11=-1036/1922, 6-11=-1088/611, 6-10=-1830/996, TO WEF 7-10=-1026/1902 NOTES (12-13)1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2 X 6 - 2 rows at 0-7-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc. 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated. 3) Unbalanced roof live loads have been considered for this design. 4) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60 Provide adequate drainage to prevent water ponding. 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 8) All bearings are assumed to be SYP No.2 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=934, 10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 38 lb down and 58 lb up at

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

others

Continued on page 2

ARRITING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MII-7473 BEFORE USS. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TRI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

16-11-4, and 38 lb down and 58 lb up at 18-11-4 on top chord, and 1103 lb down and 782 lb up at 16-4-12, 131 lb down and 53 lb up at 16-11-4, 131 lb down and 53 lb up at 18-11-4, 221 lb down and 99 lb up at 20-11-4, and 221 lb down and 99 lb up at 22-11-4, and 366 lb down and 310 lb up at 24-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of

Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - GLENN RES.	
327272	T01	HIP	3	1	Programme and the second secon	14241514
Builders FrstSource, Lake City, Fr	L 32055			7.1	Job Reference (optional) 140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:38:51 2010 Page	e 2
Uniform Loads (plf) Vert: 1-3=-54, 3 Concentrated Loads (lb)	se=1.25, Plate Increase=1. -4=-54, 4-5=-54, 2-5=-10					
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					WILLIUS S.K	
					No 34869	=
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)	Truss	Truss Type		Qty	Ply	RICHARD KEEN	N - GLENN RES.	
272	нлас	MONO TRUSS		2	-			142415
ilders FrstSource, Lake City					7	Job Reference	e (optional) MiTek Industries, Inc. Thu Mar (14 10 20 50 2010 D
						.140 5 Oct 1 2009	will ex industries, inc. Thu Mar i	14 10:38:50 2010 Page 2
AD CASE(S) Standa Jniform Loads (plf) Vert: 1-4=-54 Concentrated Loads (Vert: 3=46(F=	, 2-5=-10 lb)	B=-4) 8=75(F=37, B=37) 9=	=-99(F=-49, B=-49) 10	=10(F=5, B=	5) 11=-28	(F=-14, B=-14)		
							1111111	SKIII
							ALL MANAGER	S.K.
							i ★ No	34869
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MIT-7473 BEFORE USE.

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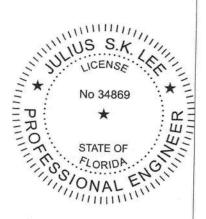
ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, S83 D'Onofrio Drive, Madison, WI S3719.

Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435

2	нлав	1 0000000000000000000000000000000000000		Qty	Ply	RICHARD KEEN - GLE		
	1.000	SPECIAL		1	1			142415
ers FrstSource, Lake City	r. FL 32055				7.1	Job Reference (option	nal)	04 10:38:50 2010 Page 2
niform Loads (plf) Vert: 1-6=-54 oncentrated Loads (ease=1.25, Plate Increa , 2-13=-10, 11-12=-10, lb)		B=-38) 17=10(F=5, B=5	i) 18=3(F=1, B				
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							77111	1111116
								1

b	Truss	Truss Type	Qty	Ply Ric	CHARD KEEN - GLENN RES.	142415
7272	HJ9A	MONO TRUSS	1	1 Joh	Reference (optional)	
ilders FrstSource, Lake City, Fl	32055			7.140 s	Oct 1 2009 MiTek Industries, Inc	. Thu Mar 04 10:38:49 2010 Page 2
AB 0405/0\ 0111						
OAD CASE(S) Standard Concentrated Loads (lb)						
Vert: 7=-9(F=-5,	B=-4) 8=-71(F=-35, B=-	35) 9=-65(F=-88, B=23) 10=-24(F=	25, B=-49) 11=10(F=5	, B=5) 12=113	(F=127, B=-14)	
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ob	Truss	Truss Type	Qty	Ply	RICHARD KEEN - GLENN RES.
27272	HJ9	MONO TRUSS	4		1 1424
uilders FrstSource	Lake City, FL 32055				Job Reference (optional)
					7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:38:49 2010 Page 2
DAD CASE(S)	Standard				
Uniform Load					
	1-4=-54, 2-5=-10				
Concentrated					
Vert:	3=46(F=23, B=23) 7=-8(F=-	4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49,	D- 40) 40 4045 5 5	41 KS SE	
	,,,,	4, b 4) 0-75(1-57, b-37) 9-99(F=-49,	B=-49) 10=10(F=5, B=	5) 11=-28	B(F=-14, B=-14)





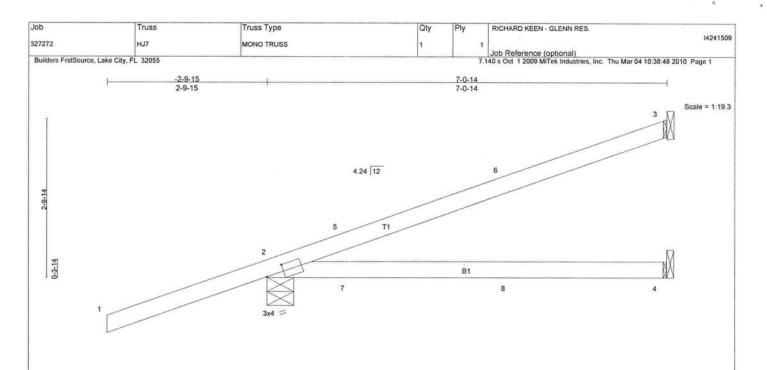


Plate Offsets (X,Y): [2:0-3-12,0-1-8]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.09	2-4	>876	360	MT20	244/190
TCDL	7.0	Lumber Increase	e 1.25	BC	0.32	Vert(TL)	-0.14	2-4	>578	240		
BCLL	0.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2007/	TPI2002	(Mati	rix)	Wind(LL)	0.13	2-4	>636	240	Weight: 26 lb)

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=110/Mechanical, 2=335/0-5-11, 4=30/Mechanical

Max Horz 2=225(LC 3)

Max Uplift3=-138(LC 3), 2=-490(LC 3), 4=-40(LC 8) Max Grav 3=110(LC 1), 2=335(LC 1), 4=97(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (10-11)

- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 3=138, 2=490.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 37 lb up at 1-5-12, 37 lb up at 1-5-12, and 13 lb down and 23 lb up at 4-3-11, and 13 lb down and 23 lb up at 4-3-11 on top chord, and 18 lb up at 1-5-12, 18 lb up at 1-5-12, and 12 lb down and 42 lb up at 4-3-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33436

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 2-4=-10

Concentrated Loads (lb)

Vert: 5=75(F=37, B=37) 6=46(F=23, B=23) 7=10(F=5, B=5) 8=-8(F=-4, B=-4)

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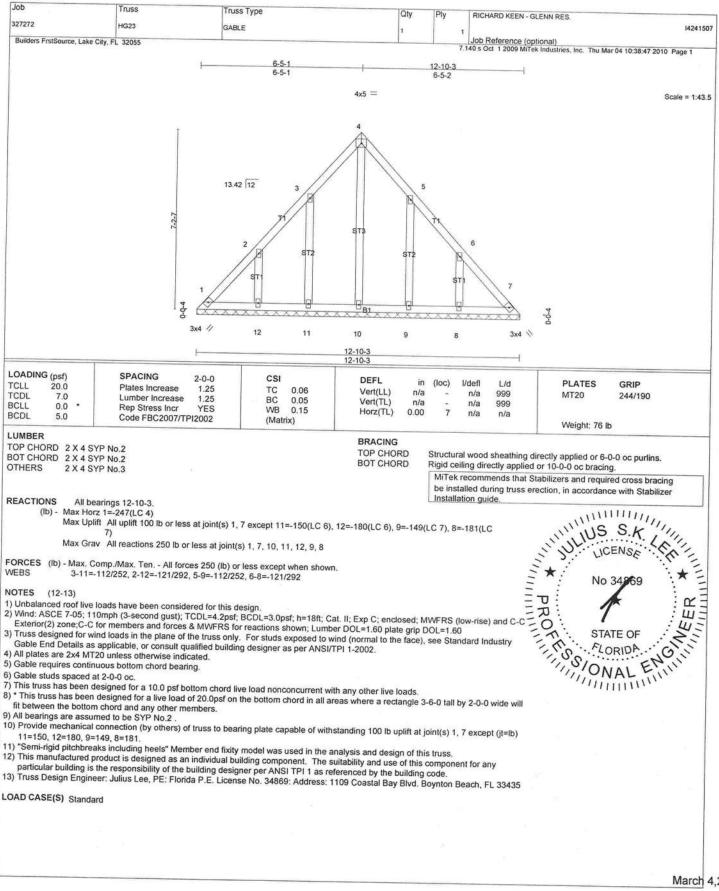
March 4,2010

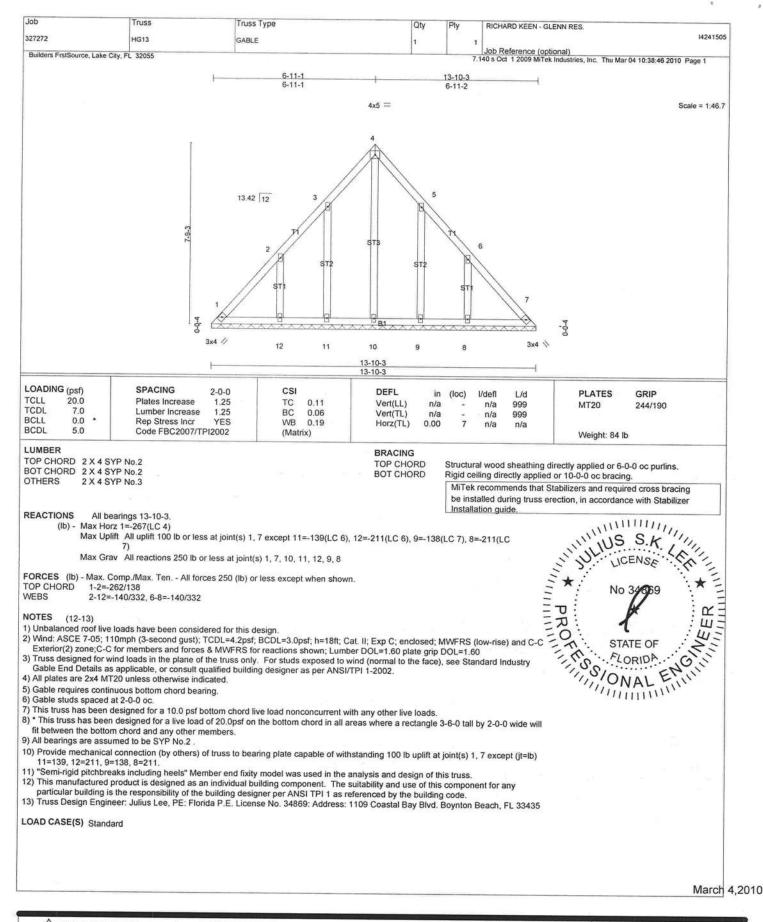
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitlek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult.

AMSI/TPI Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





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AMSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

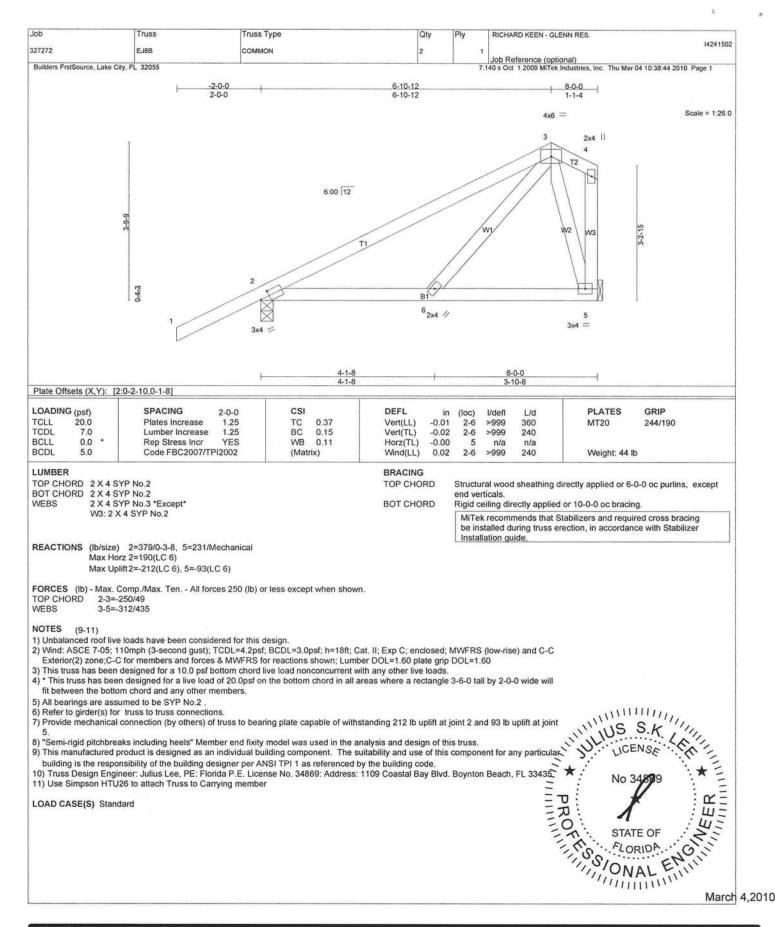
	Truss	Truss Type	Qty	Ply	DICHARD WEEK	
272	EJ8C	MONO HIP	15.0		RICHARD KEEN - GLENN RES.	142415
ilders FrstSource, Lake City, F	L 32055		1		Job Reference (optional)	
				1	7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:38:44 2010 Pag	ge 2
AD CASE(S) Standard	ř.					
Concentrated Loads (lb) 3=-106(F) 7=-49(F) 8=-	A 47000				
Veit. 034(F)	5100(F) /=-49(F) 8=-	14(F)				
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					"11S S.K"11	
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March 4,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MII-7473 BEFORE USE.

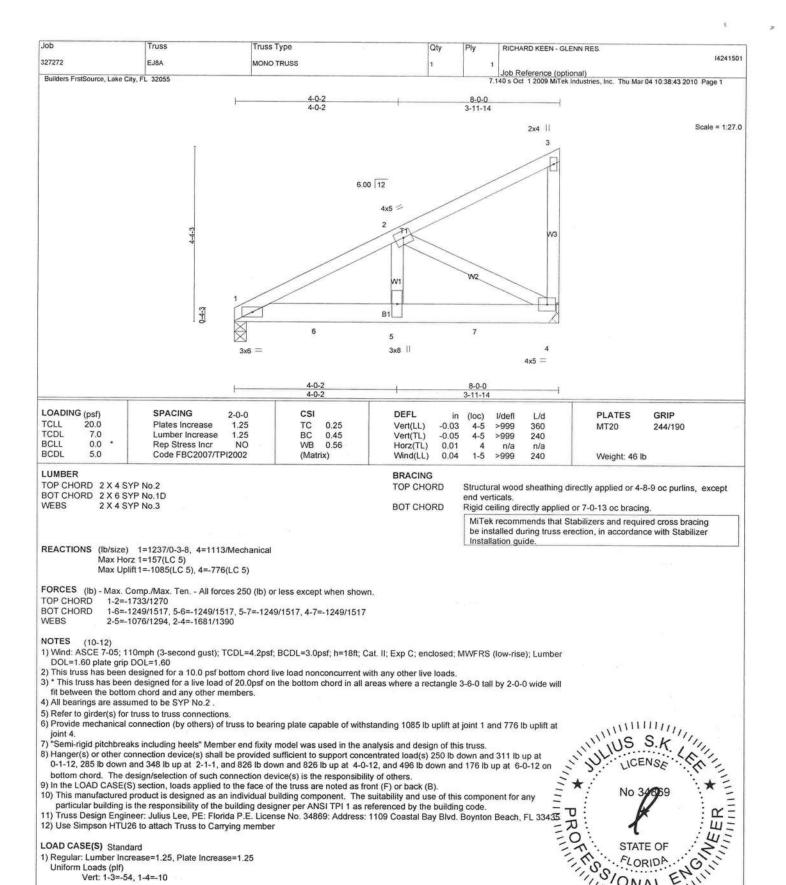
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer on thus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of building designer, for general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

ANSI/TPI Quality Criteria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, qualify control, storage, delivery, erection and bracing, consult. AMSI/ITI ADSI/ITI DISABLE OF ADSI/ITI Building Component Safety Information available from Truss Plate Institute, 583 D'Onatrio Drive, Madison, WI 53719.

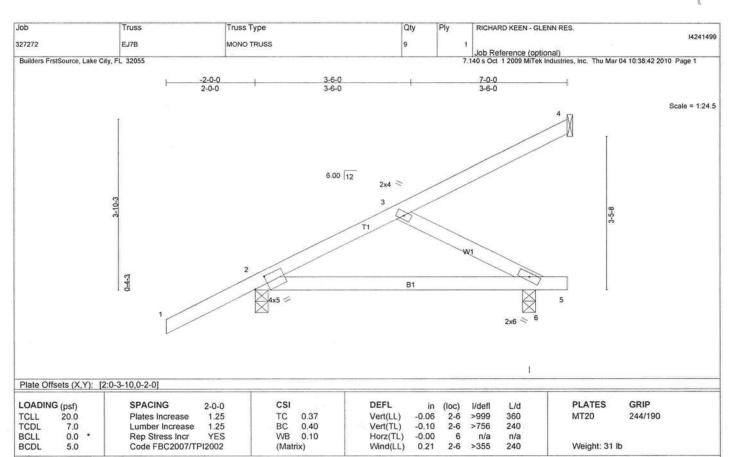


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to turning construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TRI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute. S83 D'Onofrio Drive, Madison, WI 53719.

Continued on page 2

Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 8-7-7 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=86/Mechanical, 2=343/0-3-8, 6=122/0-3-8

Max Horz 2=203(LC 6)

Max Uplift 4=-67(LC 6), 2=-290(LC 6), 6=-136(LC 6) Max Grav 4=86(LC 1), 2=343(LC 1), 6=138(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-251/166 BOT CHORD 2-6=-372/184

WEBS 3-6=-206/415

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Extenor(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Rountee. PROTEIN STANFLORIL E.

March 4,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TRI Quality Criteria, DSB-89 and BCS11 Building Component Salety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, Wi 53719.

1109 Coastal Bay Blvd. Boynton, FL 33435

Qty RICHARD KEEN - GLENN RES. Job Truss Truss Type Ply 13 MONO TRUSS 327272 F.17 Job Reference (optional) 7.140 s Jun 24 2009 MiTek Industries, Inc. Thu Mar 04 12:43:19 2010 Page 1 Builders FrstSource, Lake City, FL 32055 7-0-0 -2-0-0 2-0-0 7-0-0 Scale: 1/2"=1" 04-3 3x4 =

LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	- 1	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.09	2-4	>921	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.17	2-4	>476	240	VIDOURONS	
BCLL	0.0		Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a	000000000000000000000000000000000000000	
BCDL	5.0		Code FBC2007/TF		(Matr	ix)	Wind(LL)	0.42	2-4	>195	240	Weight: 26 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=157/Mechanical, 2=352/0-3-8, 4=42/Mechanical

Max Horz 2=203(LC 6)

Max Uplift 3=-125(LC 6), 2=-301(LC 6), 4=-87(LC 5) Max Grav 3=157(LC 1), 2=352(LC 1), 4=96(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

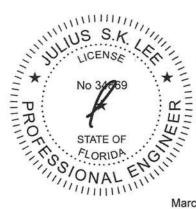
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 125 lb uplift at joint 3, 301 lb uplift at joint 2 and 87 lb uplift at joint 4.

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



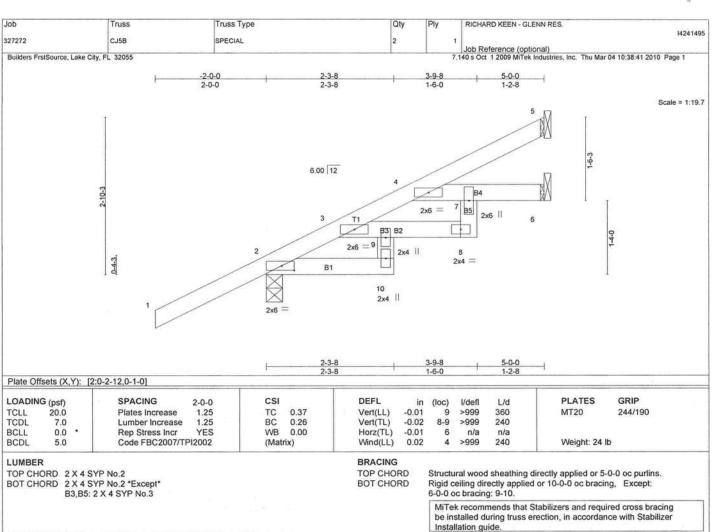
March 4,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flabication, quality control, storage, delivery, erection and bracing, consult.

AMSI/TRI Quality Citleria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 537 19.



REACTIONS (lb/size) 5=65/Mechanical, 2=299/0-3-8, 6=69/Mechanical

Max Horz 2=224(LC 6)

Max Uplift5=-69(LC 6), 2=-264(LC 6), 6=-27(LC 6)

Max Grav 5=65(LC 1), 2=299(LC 1), 6=90(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 5, 264 lb uplift at joint 2

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

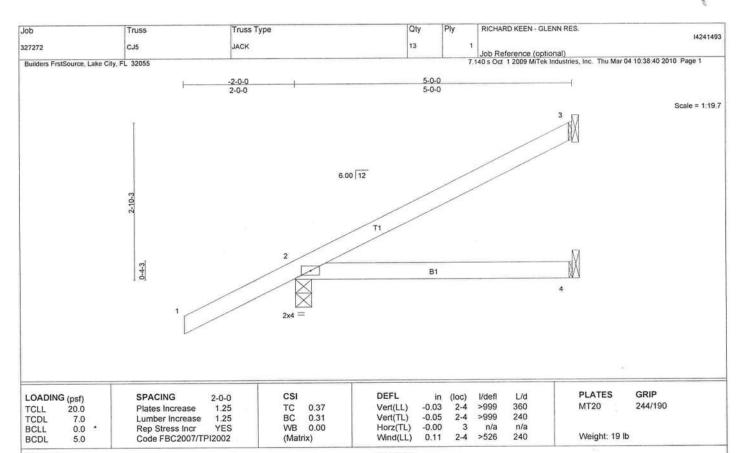
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

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March 4,2010

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown for lateral support of individual web members only. Additional temporary bracing to inverse stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/TRIO. Quality Criteria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute. 583 D'Onofrio Drive, Madison, WI 53719.



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

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REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=224(LC 6)

Max Uplift3=-114(LC 6), 2=-341(LC 6), 4=-61(LC 4) Max Grav 3=103(LC 1), 2=295(LC 1), 4=72(LC 2)

FORCES (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable eng zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 114 lb uplift at joint 3, 341 lb uplift at joint 2 and 61 lb uplift at joint 4.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

March 4,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. AN MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER KEPSCHECUS.

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Job Truss Truss Type Qty Ply RICHARD KEEN - GLENN RES. 14241491 327272 JACK CJ3A Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:38:39 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -2-0-0 3-0-0 2-0-0 Scale = 1:17.3 6.00 12 2x4 =

OADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.06	2	>928	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.12	2	>502	240		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.11	4	n/a	n/a		
BCDL	5.0	Code FBC2007/TI	212002	(Matr	rix)	Wind(LL)	0.23	2	>257	240	Weight: 13 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=158/Mechanical, 3=142/Mechanical, 4=15/Mechanical Max Horz 1=171(LC 6)

Max Uplift 1=-144(LC 6), 3=-184(LC 6), 4=-38(LC 4) Max Grav 1=158(LC 1), 3=142(LC 1), 4=44(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 1, 184 lb uplift at joint 3 and 38 lb uplift at joint 4.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

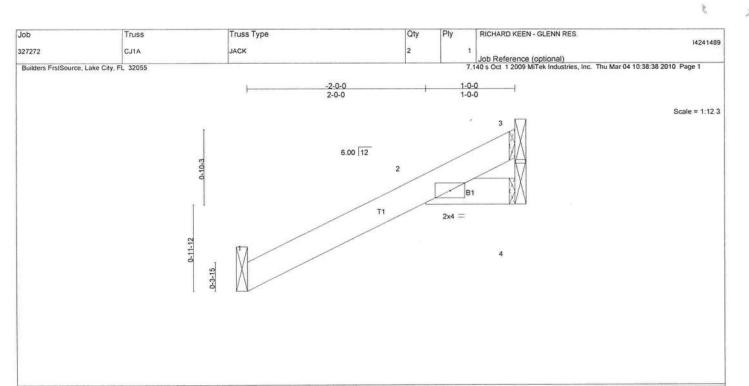
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March 4,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibly of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracings to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/TPII Quality Criteria, DSB-89 and BCSII Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	-0.01	1-2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.02	1-2	>999	240		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2007/T	PI2002	(Mati	rix)	Wind(LL)	0.02	1-2	>999	240	Weight: 7 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING TOP CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing with Stabil be installed during truss erection, in accordance with Stabilizer Installation guide.

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REACTIONS (lb/size) 1=95/Mechanical, 4=5/Mechanical, 3=89/Mechanical Max Horz 1=85(LC 6)

Max Uplift 1=-42(LC 6), 3=-88(LC 6) Max Grav 1=95(LC 1), 4=15(LC 2), 3=89(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber-DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 1 and 88 lb uplift at joint

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

March 4,2010

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RE: 327272 - RICHARD KEEN - GLENN RES.

Site Information:

Project Customer: RICHARD KEEN Project Name: 327272 Model: GLENN RES.

Lot/Block: Address: 185 SW ARROWHEAD TER Subdivision:

State: FL City: COLUMBIA CTY

No.	Seal#	Truss Name	Date
35	14241522	T09	3/4/010
36	14241523	T10	3/4/010
37	14241524	T11	3/4/010
38	14241525	T12	3/4/010
39	14241526	T13	3/4/010
40	14241527	T14	3/4/010
41	14241528	T15	3/4/010
42	14241529	T16	3/4/010
43	14241530	T17	3/4/010
44	14241531	T18	3/4/010
45	14241532	T19	3/4/010
46	14241533	T20	3/4/010
47	14241534	T21	3/4/010
48	14241535	T22	3/4/010
49	14241536	T23	3/4/010
50	14241537	T24	3/4/010
51	14241538	T25	3/4/010
52	14241539	T26	3/4/010

