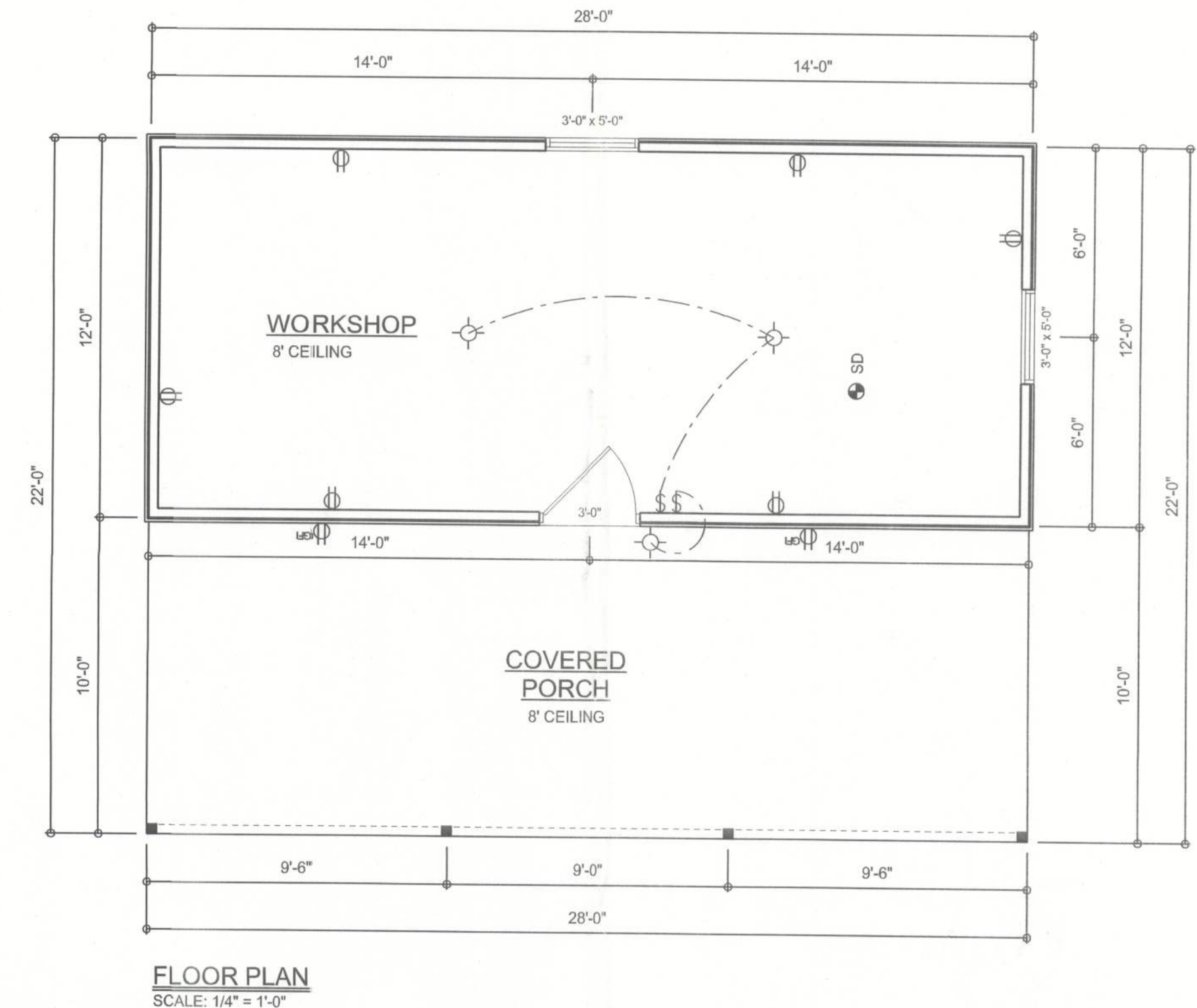
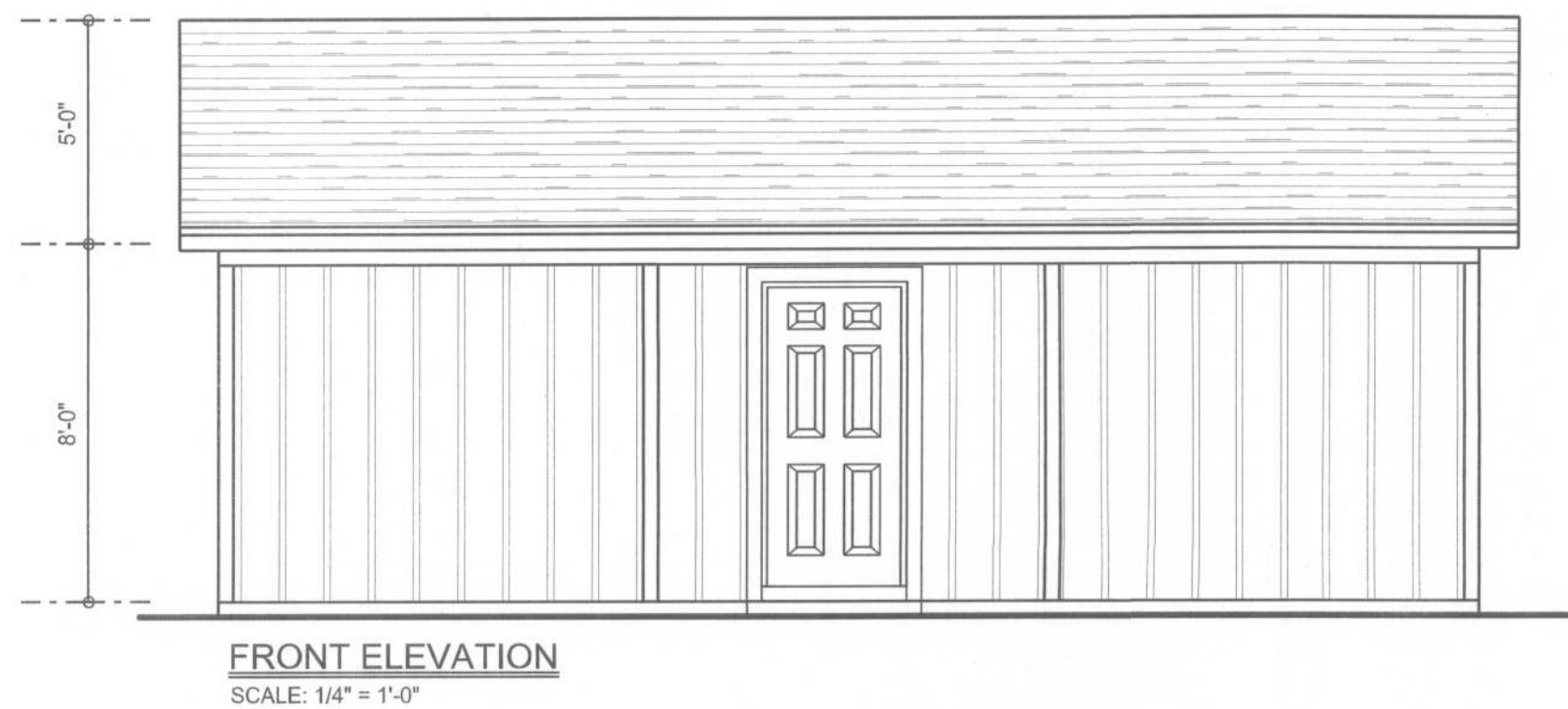


REVISIONS		

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#### AREA SUMMARY

WORKSHOP AREA	336	S . F .
PORCH AREA	280	S . F .
<b>TOTAL AREA</b>	<b>616</b>	<b>S . F .</b>

#### TOTAL SHEAR WALL SEGMENTS

	INDICATES SHEAR WALL SEGMENTS	
1st FLOOR	REQUIRED	ACTUAL
TRANSVERSE	16.7'	21.0'
LONGITUDINAL	10.8'	50.0'

#### HEADER LEGEND

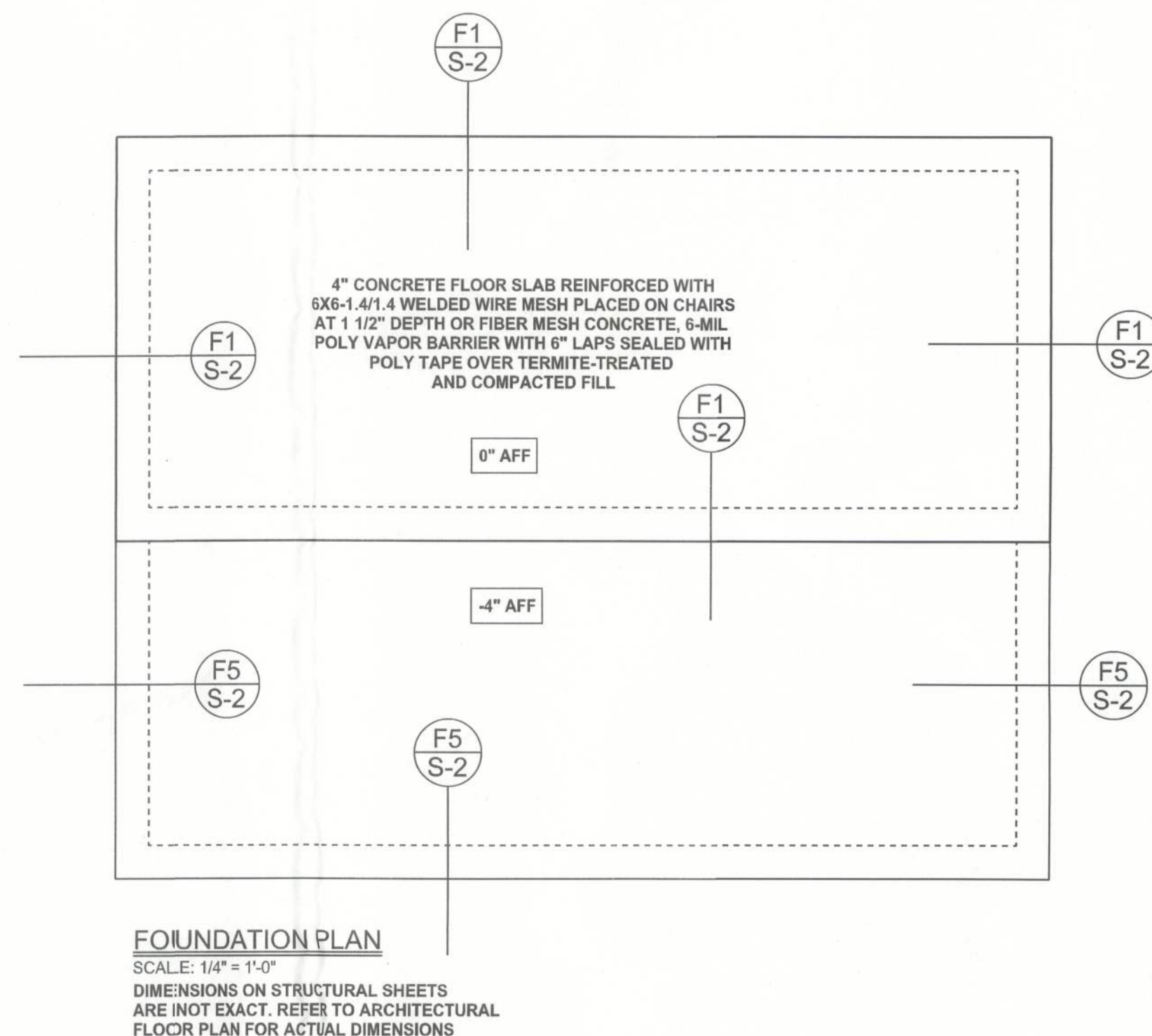
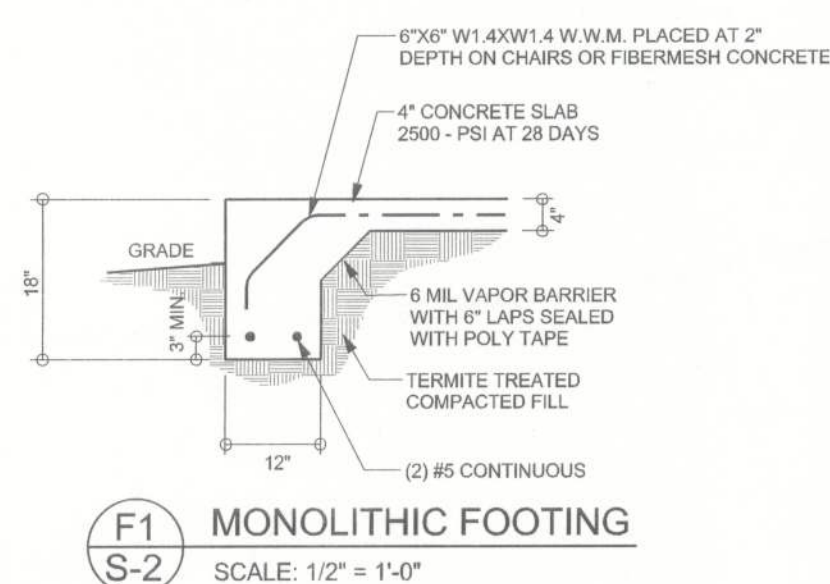
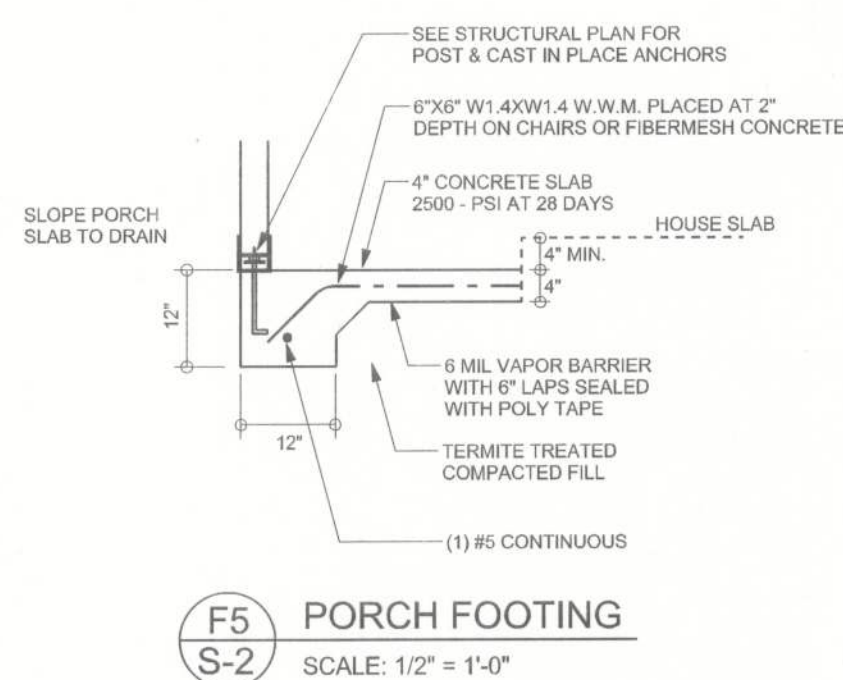
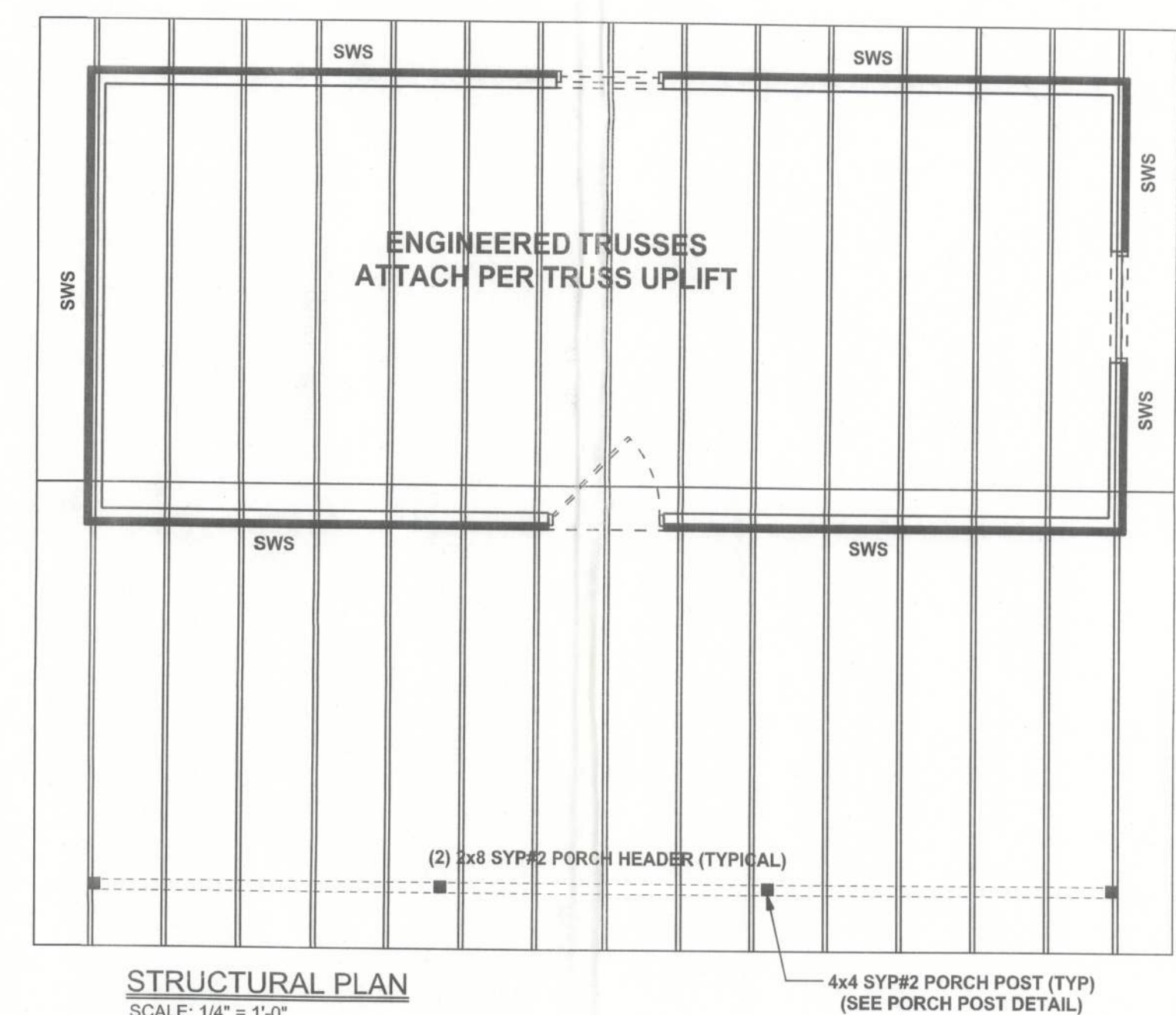
- (2) 2X12X0'.1J 1K ———— HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

#### STRUCTURAL PLAN NOTES

- SN-1** ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SPF #2 U.N.O.
- SN-2** ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3** DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4** PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

#### WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT



WINDLOAD ENGINEER: Mark Disoway,  
PE No. 53915, PCB 868, Lake City, FL  
32056, 386-754-5419

**DIMENSIONS:**  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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Mark Disoway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disoway.

**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section F301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.



Cason Builders Inc

Bradley Workshop

ADDRESS:  
Alachua County, Florida

Mark Disoway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE:  
Thursday, December 12, 201

DRAWN BY: Evan Beamsley

STRUCTURAL BY: Evan Beamsley

FINALS DATE:

JOB NUMBER:

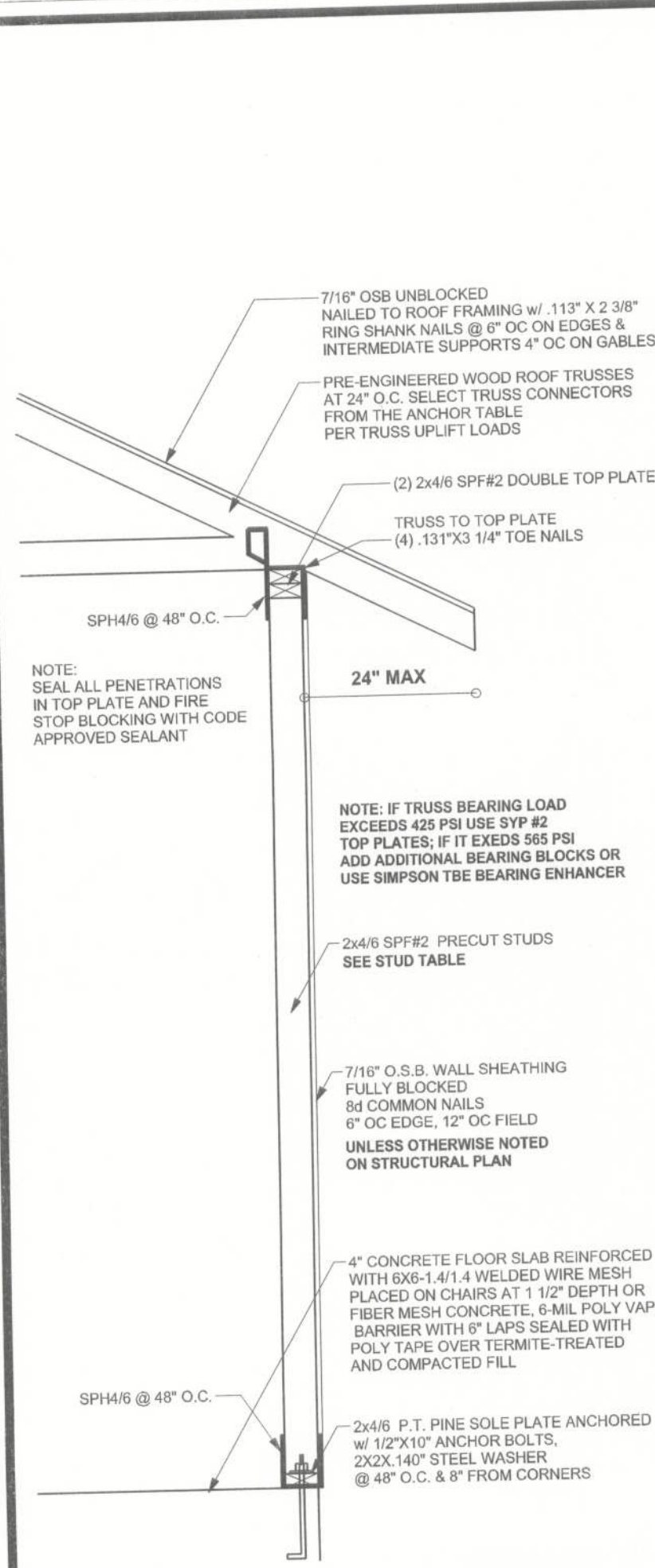
DRAWING NUMBER

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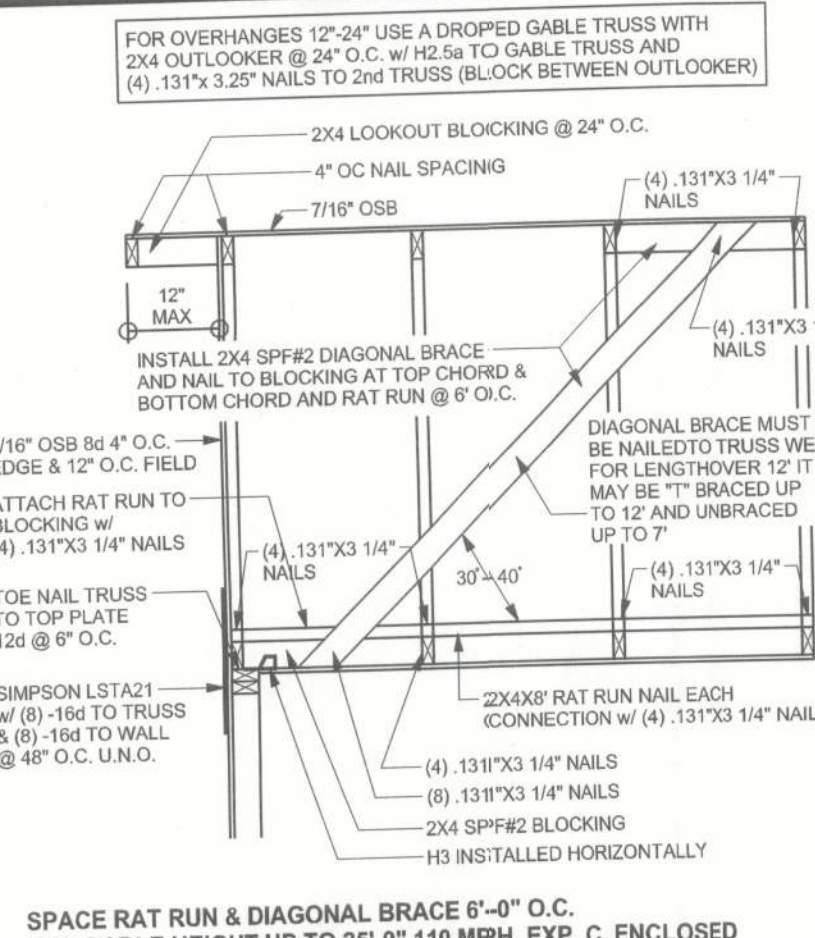
OF 2 SHEETS



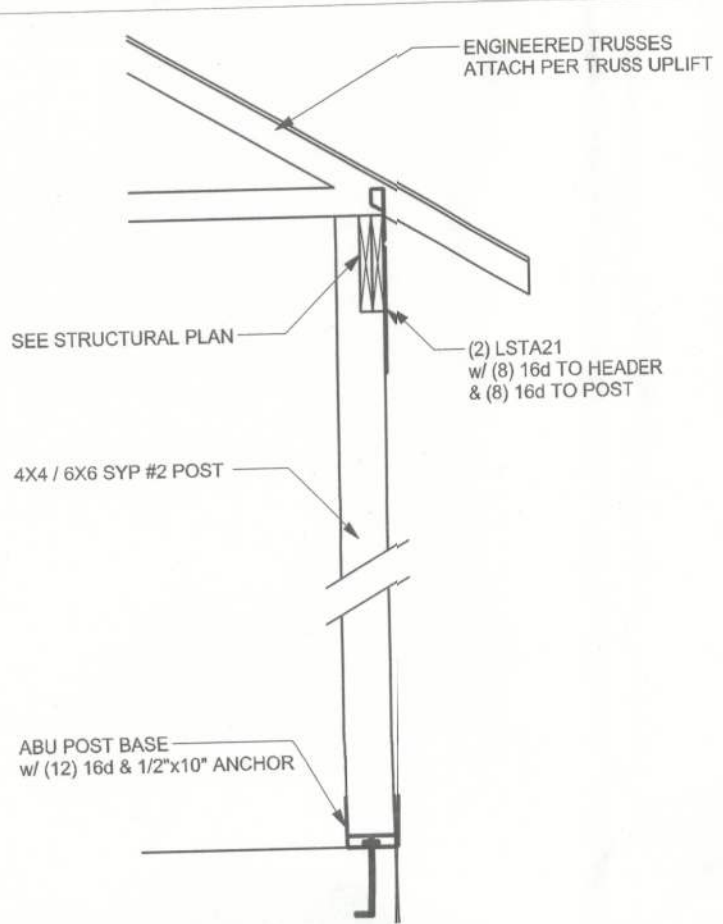




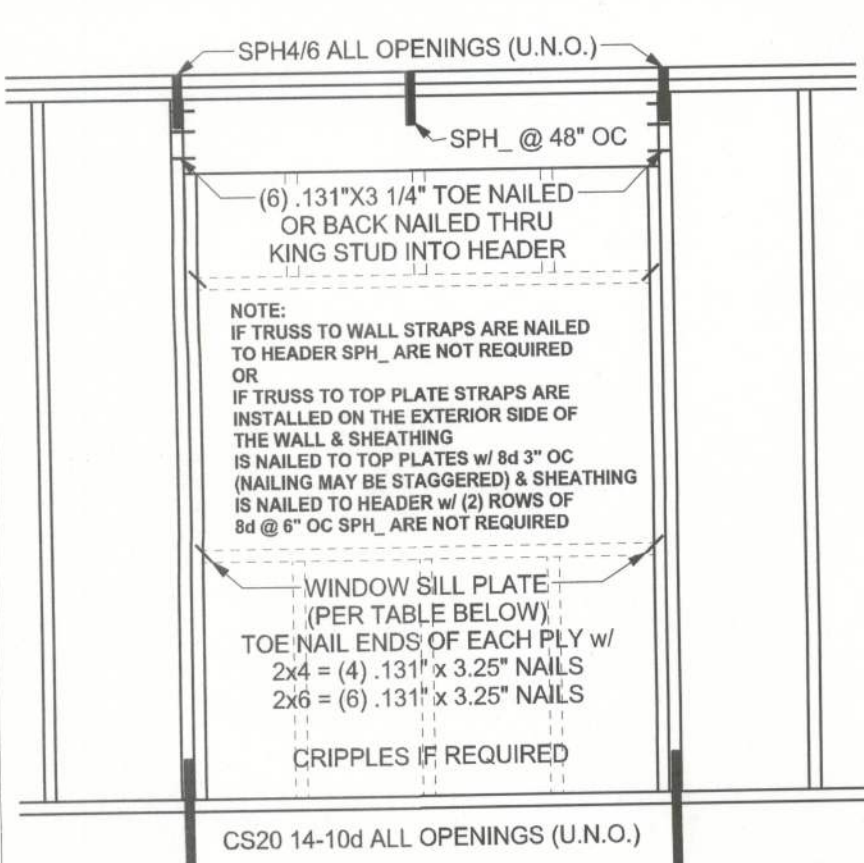
**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"



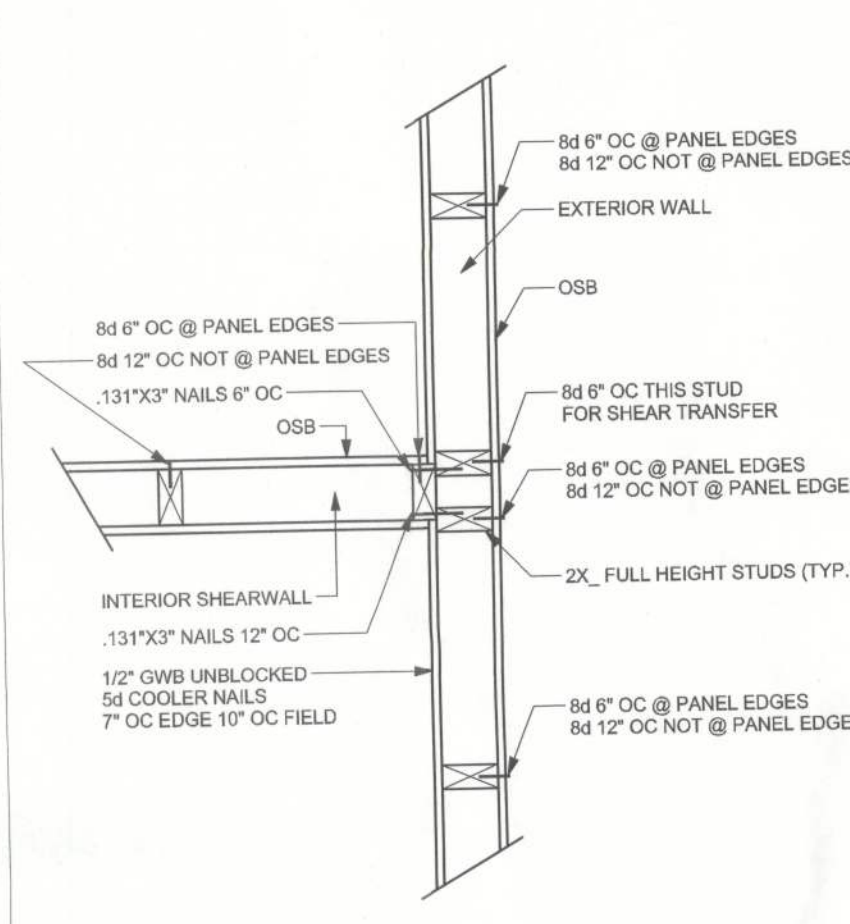
**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



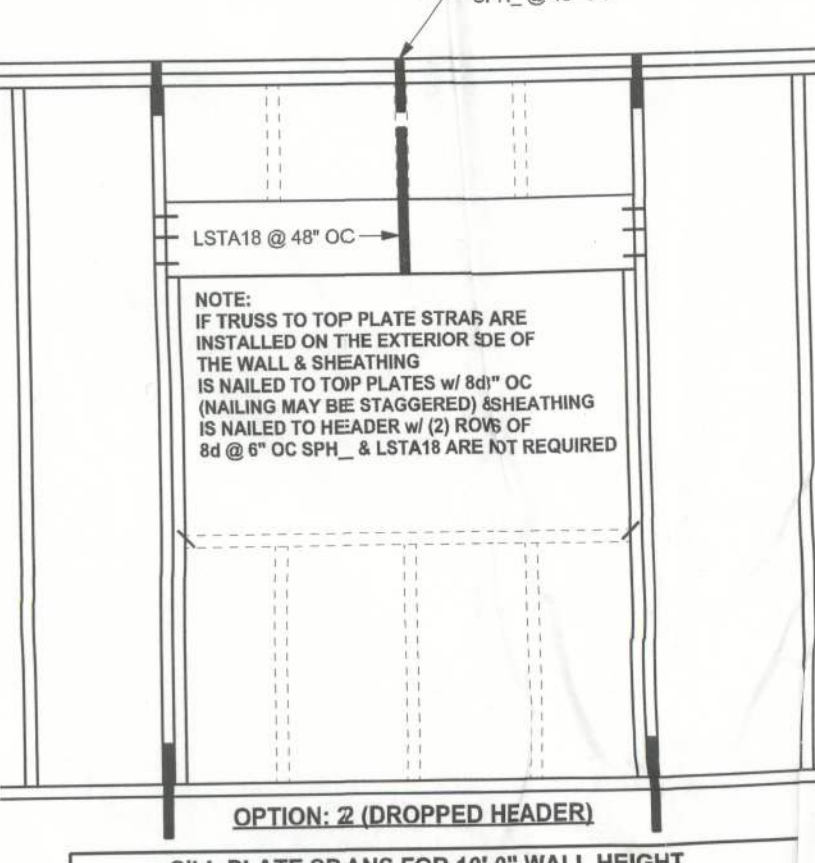
**(TYP.) PORCH POST**  
ONE STORY WOOD



**TYPICAL HEADER STRAPING DETAIL**  
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

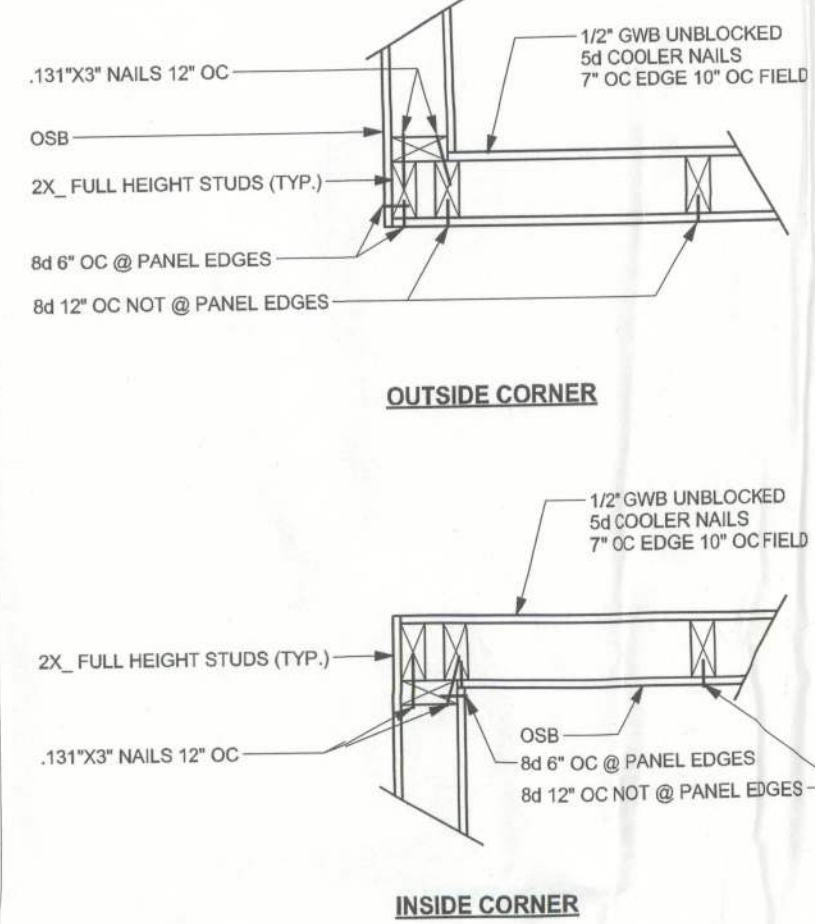


**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME



**ANCHOR TABLE**  
OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES
H5	455	265	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H3	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5	415	365	150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H6	950	820					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	15-8d
H10	990	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	760	655					6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
LT512 - LT520	1000	620					7-10d x 1 1/2"	7-10d x 1 1/2"
MT512 - MT530	1000	660					7-10d x 1 1/2"	7-10d x 1 1/2"
HT516 - HT530	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"



**(TYP.) CORNER FRAMING**  
WOOD FRAME

## ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES
H5	455	265	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H3	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5	415	365	150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H6	950	820					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	15-8d
H10	990	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	760	655					6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
LT512 - LT520	1000	620					7-10d x 1 1/2"	7-10d x 1 1/2"
MT512 - MT530	1000	660					7-10d x 1 1/2"	7-10d x 1 1/2"
HT516 - HT530	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"

**HEAVY GIRDER TIEDOWNS**

LG2	2050	1765	700	170	700	170	14-16d	14-16d
LG13-SDS2.5	3685	2655	795	410	795	410	12-SDS 1/4" x 2 1/2"	26-16dS
LG14-SDS3	4060	3660	2000	675	2000	675	12-SDS 1/4" x 3"	36-16dS
M3T	3965	3330					22-10d	5/8" ANCHOR
HGT-2	10980	6485					16-10d	2-5/8" ANCHOR
HGT-3	10530	9035					16-10d	2-5/8" ANCHOR
HGT-4	9250	9250					16-10d	2-5/8" ANCHOR

**STUD STRAP CONNECTOR**

SSP DOUBLE TOP PLATE	435	435					3-10d	4-10d
SSP SINGLE TOP PLATE	455	420					1-10d	4-10d
DSP DOUBLE TOP PLATE	825	825					6-10d	8-10d
DSP SINGLE SILL PLATE	825	600					2-10d	5-10d
SP1	585	535					4-10d	6-10d
SP2	1065	605					6-10d	6-10d
SP4	885	760					6-10d x 1 1/2"	
SPH4	1240	1065					6-10d x 1 1/2"	
SP6	885	760					6-10d x 1 1/2"	
SPH6	1240	1065					10-10d x 1 1/2"	
LSTA18	1235	1110					14-10d	
LSTA21	1235	1235					14-10d	
CS20	1030	1030					14-10d	
CS16	1705	1705					22-10d	

**STUD ANCHORS**

LTT19	1350	1305					8-16d	1/2" ANCHOR
LTT31	2310	2310					18-10d x 1 1/2"	5/8" ANCHOR
HD2A	2775	2570					2-5/8" BOLTS	5/8" ANCHOR
HTT16	4175	3695					18-16d	5/8" ANCHOR
HTT22	5260	5250					32-16d	5/8" ANCHOR
ABU44	2200	2200					12-16d	5/8" ANCHOR
ABU66	2300	2300					12-16d	5/8" ANCHOR
ABU88	2320	2320					18-16d	2-5/8" ANCHOR

(1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL HOLES  
(2) FOR SYP GIRDER & SPF STUDS

## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2010 FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS ASSUMING 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $f'_c = 3000$  PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x 4 W1 4,  $F_y = 80$  KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 2'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD FOR THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHEN SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12'-0". DO NOT CUT W/M OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A OVER LINE.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS,  $F_y = 80$  KSI, ALL LAP SPICES 40" DB (25" FOR #5 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB 24F-V3SP,  $F_b = 2.4$  KSI,  $E = 1800$  KSI; UNO, SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (13d), 6"OC PANEL EDGES; 1/2"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/16"; WITH 5/8" BOLTS TO BE 3" x 3" x 5/16"; WITH 3/4" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

## BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH 2010 FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

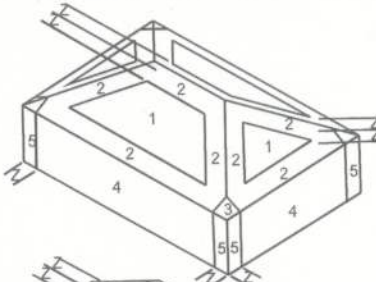
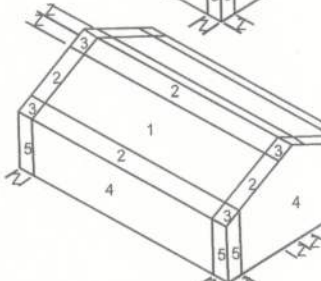
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

## ROOF SYSTEM DESIGN

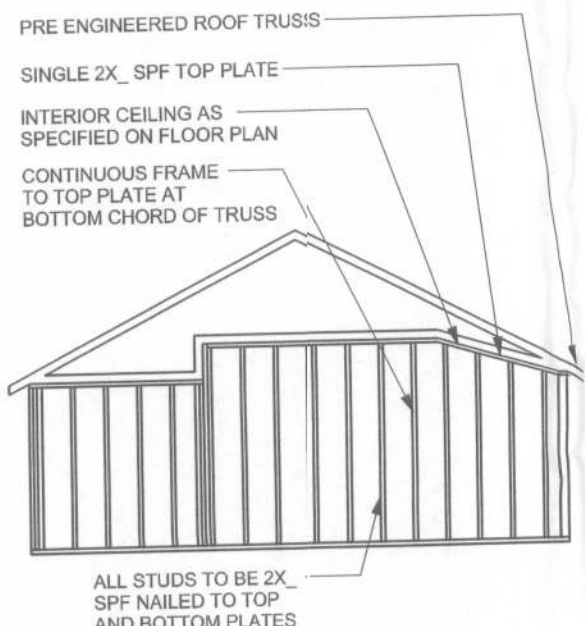
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH 2010 FBCR, SECTION R301.2 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF 2010 FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

## DESIGN DATA

WIND LOADS PER 2010 FLORIDA BUILDING CODE RESIDENTIAL, SECTION R301.2.1																
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT)																
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE																
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION																
1) BASIC WIND SPEED = 130 MPH, (3 SEC GUST, 33 FT, EXP. C)																
2) WIND EXPOSURE = C, BUILDER MUST FIELD VERIFY																
3) TOPOGRAPHIC FACTOR = 1.0, BUILDER MUST FIELD VERIFY																
4) BUILDING CATEGORY = II, (MRI = 700 YR)																
5) ROOF ANGLE = 7-45 DEGREES																
6) MEAN ROOF HEIGHT = <30 FT																
7) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)																
8) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))																
		<table><tr><th>Zone</th><th>Effective Wind Area (ft<sup>2</sup>)</th></tr><tr><td>1</td><td>10</td></tr><tr><td>1</td><td>39 - 43</td></tr><tr><td>2</td><td>39 - 68</td></tr><tr><td>3</td><td>39 - 100</td></tr><tr><td>4</td><td>43 - 48</td></tr><tr><td>5</td><td>43 - 57</td></tr></table>	Zone	Effective Wind Area (ft <sup>2</sup> )	1	10	1	39 - 43	2	39 - 68	3	39 - 100	4	43 - 48	5	43 - 57
Zone	Effective Wind Area (ft <sup>2</sup> )															
1	10															
1	39 - 43															
2	39 - 68															
3	39 - 100															
4	43 - 48															
5	43 - 57															
		<table><tr><td>Garage Door</td><td>2010 FBCR, Table R301.2.4</td><td></td><td></td></tr><tr><td>8'x7' Garage Door</td><td></td><td>37</td><td>-42</td></tr><tr><td>16'x7' Garage Door</td><td></td><td>36</td><td>-40</td></tr></table>	Garage Door	2010 FBCR, Table R301.2.4			8'x7' Garage Door		37	-42	16'x7' Garage Door		36	-40		
Garage Door	2010 FBCR, Table R301.2.4															
8'x7' Garage Door		37	-42													
16'x7' Garage Door		36	-40													
DESIGN LOADS																
FLOOR: 40 PSF (ALL OTHER DWELLING ROOMS)																
30 PSF (SLEEPING ROOMS)																
30 PSF (ATTICS WITH STORAGE)																
10 PSF (ATTICS WITHOUT STORAGE, <3:12)																
ROOF: 20 PSF (FLAT OR <4:12)																
16 PSF (4:12 TO <12:12)																
12 PSF (12:12 AND GREATER)																
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)																
SOIL BEARING CAPACITY 1000PSF																
NOT IN FLOOD ZONE (BUILDER TO VERIFY)																

## GRADE & SPECIES TABLE

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0



**CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL**  
WOOD FRAME

**MAXIMUM EXTERIOR STUD LENGTHS (EXPOSURE C)**

WIND SPEED	STUD SPACING	SPECIES & GRADE OF STUD					
		SPRUCE-PINE-FIR #2			SOUTHERN PINE #2		
		2x4	2x6	2x8	2x4	2x6	2x8
130 MPH	12" OC	11'-7"	18'-7"	20'-0"	12'-2"	19'-6"	20'-0"
	14" OC	10'-8"	16'-10"	20'-0"	11'-0"	17'-7"	20'-0"
	16" OC	10'-8"	16'-10"	20'-0"	11'-0"	17'-7"	20'-0"
	24" OC	9'-1"	14'-2"	17'-10"	9'-6"	15'-2"	19'-2"

FOR STUD LENGTH OVER 20'-0" SELECT LENGTH FROM APPROVED STUD SPAN TABLE OR PROVIDE LOAD CALCULATION SHEET.  
MAX STUD LENGTHS IN THIS TABLE ARE BASED ON WIND LOAD ZONE 4 AND ASSUME THAT ALL STUDS ARE SHEATHED ON THE EXTERIOR WITH A MINIMUM OF 3/8" WOOD SHEATHING AND ON THE INTERIOR WITH GYPSUM WALL BOARD.  
STUD SPACING SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. X 0.85 = 13.6" O.C.  
STUDS SHALL BE CONTINUOUS BETWEEN HORIZONTAL SUPPORTS

**EXTERIOR WALL STUD TABLE**  
BASED ON WFCM TABLE A-3.20B

## REVISIONS

NO.	DESCRIPTION
1	ISSUED FOR PERMIT

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway, PE No. 53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Cason Builders Inc