

REVISIONS 13May10 24May10 7Jur10 10Jin10

SOFTPIXN

Colimbia County, Florida

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PRINTED DATE:

JOB NUMBER:

1003085

CRAWING NUMBER

OF 4 SHEETS

STRUCTURAL B

David Disosway

une 10, 2010

DRAWN3Y:

FINALS IATE:

10Jun1

Lake City, Florida 32056

3'-0" x 5'-0"

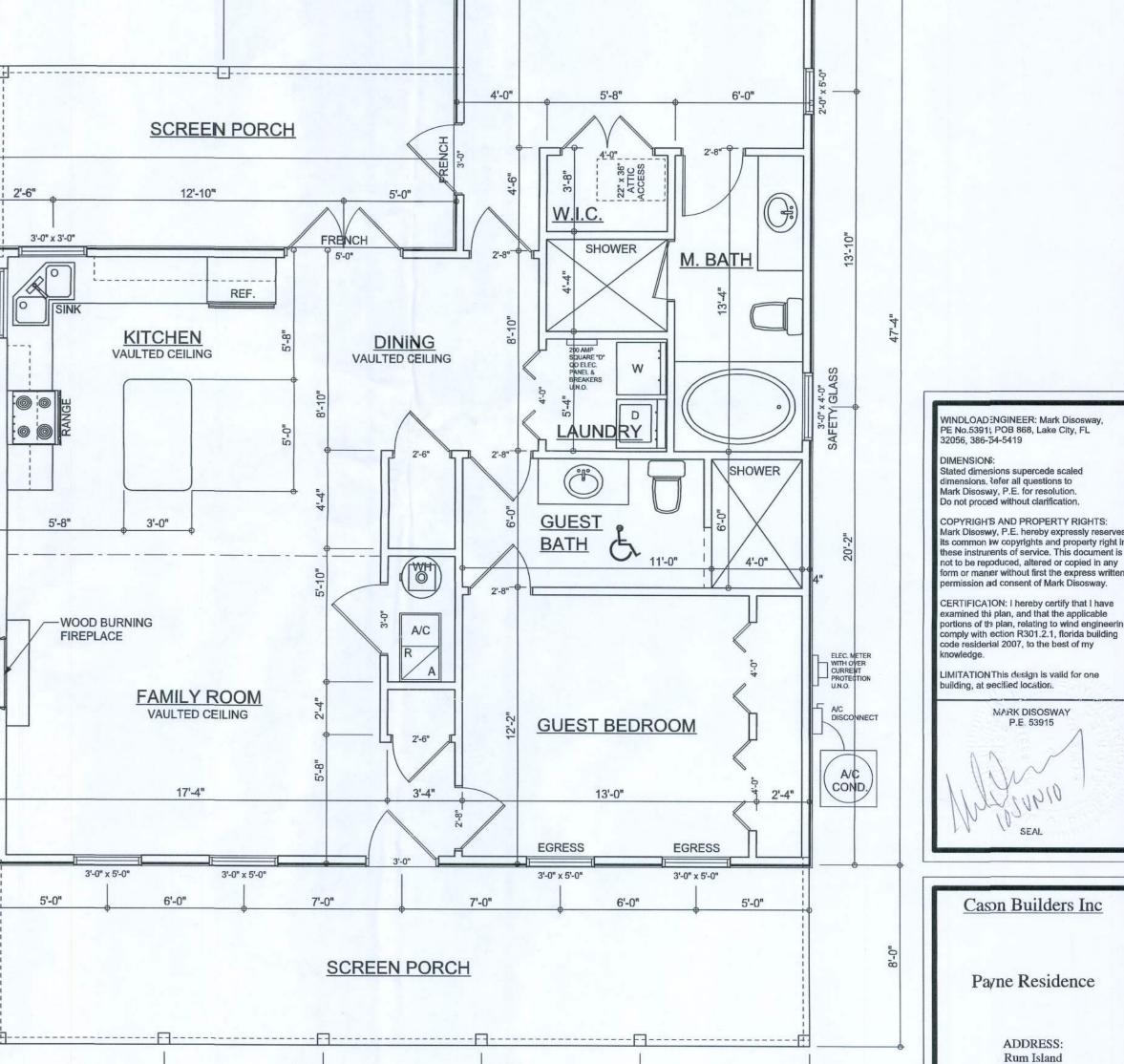
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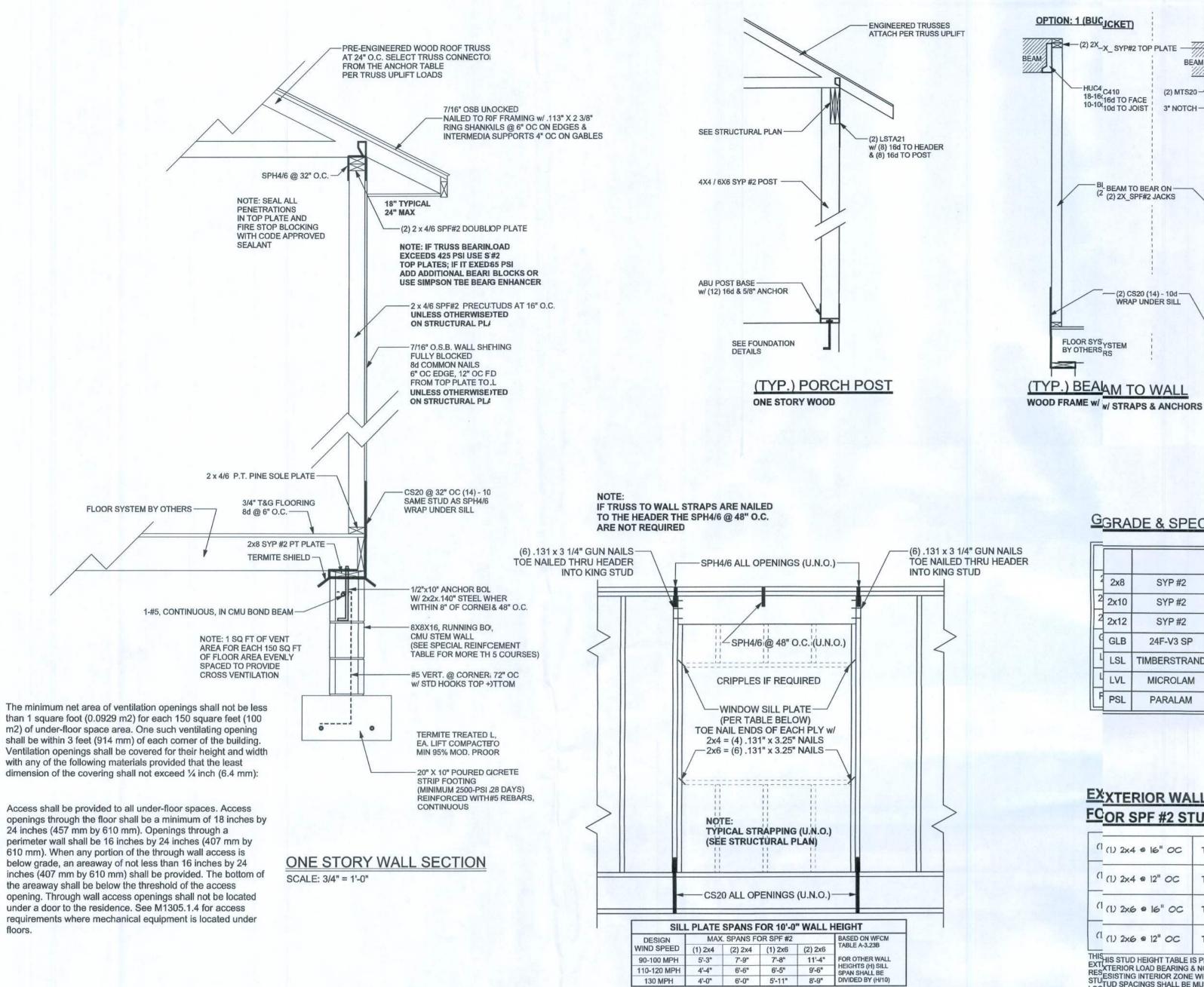
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1751

S.F.

S.F





1/2" GWB UNBLOCKED

---- 1/2" GWB UNBLOCKED

5d COOLER NAILS

-8d 6" OC @ PANEL EDGES

8d 12" OC NOT @ PANEL EDGES -

7" OC EDGE 10" OC FIELD

OGIDE CORNER

INSE CORNER

(TYP.) CORER FRAMING

7" OC EDGE 10" OC FIELD

.131"X3 1/4" NAILS 12" OC-

8d 6" OC @ PANEL EDGES -

2X_FULL HEIGHT STUDS (TYP.) -

8d 12" OC NOT @ PANEL EDGES ---

2X_FULL HEIGHT STUDS (TYP.) -

.131"X3 1/4" NAILS 12" OC -

-8d 6" OC @ PANEL EDGES

-EXTERIOR WALL

-8d 6" OC THIS STUD

FOR SHEAR TRANSFER

-8d 6" OC @ PANEL EDGES

8d 12" OC NOT @ PANEL EDGES

-2X_FULL HEIGHT STUDS (TYP.)

-8d 6" OC @ PANEL EDGES

8d 12" OC NOT @ PANEL EDGES

8d 6" OC @ PANEL EDGES-

.131"X3 1/4" NAILS 6" OC -

INTERIOR SHEARWALL -

1/2" GWB UNBLOCKED -

7" OC EDGE 10" OC FIELD

5d COOLER NAILS

.131"X3 1/4" NAILS 12" OC -

OSB -

(TYP.) INTERSECTING WALL FRAMING

8d 12" OC NOT @ PANEL EDGES

TYPICAL HEADER STRAPING DETAIL

H3 EACH-

7/16" OSB 8d 4" O.C. --

EDGE & 12" O.C. FIELD

ATTACH RAT RUN TO-

BLOCKING w/ (4) .131"X3 1/4" NAILS

TOE NAIL TRUSS -

TO TOP PLATE

SIMPSON LSTA2

@ 48" O.C. U.N.O.

w/ (8) -16d TO TRUSS

12d @ 6" O.C.

INSTALL 2X4 SPF#2 DIAGONAL BRACE -

SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.

FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED

(TYP.) GABLE BRACING DETAIL

AND NAIL TO BLOCKING AT TOP CHORD & BOTTOM CHORD AND RAT RUN @ 6' O.C.

-2X4 OUTRIGGER @ 24" O.C.

-BLOCKING REQUIRED BETWEEN OUT RIGGERS

(4) 131"X3 1/4"

DIAGONAL BRACE MUS

MAY BE "T" BRACED UP

TO 12' AND UNBRACED

-2X4X8' RAT RUN NAIL EACH

-(4) .131"X3 1/4" NAILS

H3 INSTALLED HORIZONTALLY

-(8) .131"X3 1/4" NAILS

- 2X4 SPF#2 BLOCKING

CONNECTION w/ (4) .131"X3 1/4" NAILS

FOR LENGTHOVER 12' IT

(4) .131"X3 1/4"-

BE NAILEDTO TRUSS WEBS

OPTION: 1 (BUCJCKET) **OPTION: 2 (POCKETED)** (2) 2X_X_SYP#2 TOP PLATE -IF TRUSS TO BEAM TO BEAM SPH_ ARE NOT REQUIRED (2) MTS20 18-16(16d TO FACE 10-10(10d TO JOIST 3" NOTCH-**POCKETED** TOP PLATE (DROPPED BEAM) BI-BEAM TO BEAR ON -(2) 2X_SPF#2 JACKS WRAP UNDER SILL FLOOR SYS YSTEM FLOOR SYSTEM

GRADE & SPECIES TABLE

| | | | Fb (psi) | E (10 ⁶ psi) |
|---|------|--------------|----------|-------------------------|
| 4 | 2x8 | SYP#2 | 1200 | 1.6 |
| 2 | 2x10 | SYP#2 | 1050 | 1.6 |
| 2 | 2x12 | SYP#2 | 975 | 1.6 |
| C | GLB | 24F-V3 SP | 2400 | 1.8 |
| L | LSL | TIMBERSTRAND | 1700 | 1.7 |
| L | LVL | MICROLAM | 1600 | 1.9 |
| F | PSL | PARALAM | 2900 | 2.0 |

EXXTERIOR WALL STUD TABLE FCOR SPF #2 STUDS

| (1 (1) 2x4 @ 16" OC | TO 10'-6" STUD HEIGHT |
|---------------------|------------------------|
| (1 (1) 2x4 @ 12" OC | TO 11'-7" STUD HEIGHT |
| (1 (1) 2x6 9 16" OC | TO 16'-10" STUD HEIGHT |
| 1 (1) 2x6 @ 12" OC | TO 18'-7" STUD HEIGHT |

TUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

COCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING AXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOIR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.
ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED. APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/6" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

| | OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE F PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. |
|---------------------|---|
| | IONS, FOUNDATION BEARING CAPACITY, GRADE AND D SPEED AND DEBRIS ZONE, AND FLOOD ZONE. |
| | IND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 THE STATED WIND VELOCITY AND DESIGN PRESSURES. |
| | US LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU ITS A CONTINUOUS LOAD PATH CONNECTION, CALL EER IMMEDIATELY. |
| DESIGN, PLACEMENT P | NUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, INECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL |

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

| PLIFT LBS. SYP | UPLIFT LBS. SPF | TRUSS CONNECTOR* | TO PLATES | TO RAFTER/TRUSS | TO STUDS |
|----------------|-----------------|------------------------|----------------|-----------------|--|
| < 420 | < 245 | H5A | 3-8d | 3-8d | |
| < 455 | < 265 | H5 | 4-8d | 4-8d | |
| < 360 | < 235 | H4 | 4-8d | 4-8d | |
| < 455 | < 320 | H3 | 4-8d | 4-8d | |
| < 415 | < 365 | H2.5 | 5-8d | 5-8d | |
| < 600 | < 535 | H2.5A | 5-8d | 5-8d | |
| < 950 | < 820 | H6 | 8-8d | 8-8d | The same of the sa |
| < 745 | < 565 | Н8 | 5-10d, 1 1/2" | 5-10d, 1 1/2" | |
| < 1465 | < 1050 | H14-1 | 13-8d | 12-8d, 1 1/2" | |
| < 1465 | < 1050 | H14-2 | 15-8d | 12-8d, 1 1/2" | |
| < 990 | < 850 | H10-1 | 8-8d, 1 1/2" | 8-8d, 1 1/2" | |
| < 760 | < 655 | H10-2 | 6-10d | 6-10d | |
| < 1470 | < 1265 | H16-1 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1470 | < 1265 | H16-2 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1000 | < 860 | MTS24C | 7-10d 1 1/2" | 7-10d 1 1/2" | |
| < 1450 | < 1245 | HTS24 | 12-10d 1 1/2" | 12-10d 1 1/2" | |
| < 2900 | < 2490 | 2 - HTS24 | | | |
| < 2050 | < 1785 | LGT2 | 14 -16d | 14 -16d | |
| | | | 11.11. | ., | |
| | | HEAVY GIRDER TIEDOWNS* | | | TO FOUNDATION |
| < 3965 | < 3330 | MGT | | 22 -10d | 1-5/8" THREADED ROD 12" EMBEDMENT |
| < 10980 | < 6485 | HGT-2 | | 16 -10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 10530 | < 9035 | HGT-3 | | 16 -10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 9250 | < 9250 | HGT-4 | | 16 -10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| | | STUD STRAP CONNECTOR* | | | TO STUDS |
| < 435 | < 435 | SSP DOUBLE TOP PLATE | 3 -10d | | 4 -10d |
| < 455 | < 420 | SSP SINGLE SILL PLATE | 1 -10d | | 4 -10d |
| < 825 | < 825 | DSP DOUBLE TOP PLATE | 6 -10d | | 8 -10d |
| < 825 | < 600 | DSP SINGLE SILL PLATE | 2 -10d | A3 | 8 -10d |
| < 885 | < 760 | SP4 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH4 | 8 | | 10-10d, 1 1/2" |
| < 885 | < 760 | SP6 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH6 | | | 10-10d, 1 1/2" |
| < 1235 | < 1165 | LSTA18 | 14-10d | | |
| < 1235 | < 1235 | LSTA21 | 16-10d | | |
| < 1030 | < 1030 | CS20 | 18-8d | | |
| < 1705 | < 1705 | CS16 | 28-8d | | |
| | | STUD ANCHORS* | TO STUDS | | TO FOUNDATION |
| < 1350 | < 1305 | LTT19 | 8-16d | | 1/2" AB |
| < 2310 | < 2310 | LTTI31 | 18-10d, 1 1/2" | | 1/2" AB |
| < 2775 | < 2570 | HD2A | 2-5/8" BOLTS | | 5/8" AB |
| < 4175 | < 3695 | HTT16 | 18 - 16d | | 5/8" AB |
| < 1400 | < 1400 | PAHD42 | 16-16d | | |
| < 3335 | < 3335 | HPAHD22 | 16-16d | | |
| < 2200 | < 2200 | ABU44 | 12-16d | | 1/2" AB |
| | | | 12-16d | | 1/2" AB |
| < 2300 | < 2300 | ABU66 | 12"100 | | |

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone Effective Wind Area (ft2)

1 27.8 -30.5 25.3 -25.3

2 | 27.8 | -35.7 | 25.3 | -30.5

2 O'hg -56.8 -56.8

3 27.8 -35.7 25.3 -30.5

3 O'hg -95.6 -59.3

4 | 30.5 | -33.0 | 25.9 | -28.5

Doors & Windows | 30.5 | -40.7

(Zone 5, 10 ft2)

x7 Garage Door

16x7 Garage Door

10 100

.) BASIC WIND SPEED = 110 MPH

) WIND IMPORTANCE FACTOR = 1.0

) WIND EXPOSURE = C

4.) BUILDING CATEGORY = II i.) ROOF ANGLE = 10-45 DEGREES) MEAN ROOF HEIGHT = <30 FT

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

30 PSF (SLEEPING ROOMS)

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS:

MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT

ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS **REVISIONS** 13May10 24May10 7Jun'0 10Jun10

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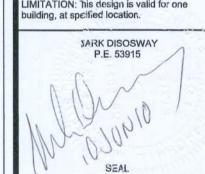
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DIMENSIONS: limensions. Refer all questions to Mark Disosway P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this lan, and that the applicable portions of the lan, relating to wind enginee comply with setion R301.2.1, florida building code residentia 2007, to the best of my

LIMITATION: his design is valid for one building, at speified location.



Casca Builders Inc

Payne Residence

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David Disosway

PRINTED DATE: Jule 10, 2010 DRAWN B': STRUCTURAL BY

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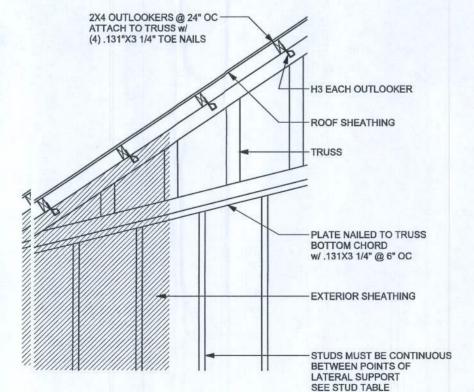
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1003085

OF 4 SHEETS

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



(TYP.) GABLE WALL w/ VAULTED CEILING WOOD FRAME

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

| | ACI530.1-02 Section | Specific Requirements | | |
|---------|----------------------------------|---|--|--|
| 1.4A | Compressive strength | 8" block bearing walls F'm = 1500 psi | | |
| 2.1 | Mortar | ASTM C 270, Type N, UNO | | |
| 2.2 | Grout | ASTM C 476, admixtures require approva | | |
| 2.3 | CMU standard | ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block | | |
| 2.3 | Clay brick standard | ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5" | | |
| 2.4 | Reinforcing bars, #3 - #11 | ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5) | | |
| 2.4F | Coating for corrosion protection | Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS | | |
| 2.4F | Coating for corrosion protection | Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet meta ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS | | |
| 3.3.E.2 | Pipes, conduits, and accessories | Any not shown on the project drawings require engineering approval. | | |
| 3.3.E.7 | Movement joints | Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings. | | |

