

DATE 08/29/2007

# Columbia County Building Permit

PERMIT

This Permit Expires One Year From the Date of Issue

000026182

APPLICANT ANTHONY B. WILLIAMS, JR. PHONE 386.755.3343  
 ADDRESS 307 SW GERALD WITT GLN LAKE CITY FL  
 OWNER ANTHONY B. & REBECCA WILLIAMS, JR. PHONE 386.755.3343  
 ADDRESS 307 SW GERALD WITT GLN LAKE CITY FL 32024  
 CONTRACTOR ANTHONY B. WILLIAMS, JR. PHONE 386.755.3343  
 LOCATION OF PROPERTY 47-S TO C-240-W TO ICHETUCKNEE, TL GO 1/4 MILE TO GERALD WITT GLN @ END OF DRIVE.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 132000.00  
 HEATED FLOOR AREA 2640.00 TOTAL AREA 4060.00 HEIGHT 21.50 STORIES 1  
 FOUNDATION CONC WALLS FRAMED ROOF PITCH 8'12 FLOOR CONC  
 LAND USE & ZONING A-3 MAX. HEIGHT 35  
 Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00  
 NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO. \_\_\_\_\_

PARCEL ID 13-5S-15-00453-002 SUBDIVISION \_\_\_\_\_  
 LOT \_\_\_\_\_ BLOCK \_\_\_\_\_ PHASE \_\_\_\_\_ UNIT \_\_\_\_\_ TOTAL ACRES 2.00

OWNER J B Williams  
 Culvert Permit No. \_\_\_\_\_ Culvert Waiver \_\_\_\_\_ Contractor's License Number \_\_\_\_\_ Applicant/Owner/Contractor  
 EXISTING 07-0650-N BLK JTH N  
 Driveway Connection \_\_\_\_\_ Septic Tank Number \_\_\_\_\_ LU & Zoning checked by \_\_\_\_\_ Approved for Issuance \_\_\_\_\_ New Resident \_\_\_\_\_

COMMENTS: FLOOR ONE FOOT ABOVE THE ROAD. NOC ON FILE. M/H WILL BE REMOVED AFTER CONSTRUCTION. 45 DAYS OF CO ISSUANCE. SECTION 2.3.1.

Check # or Cash 1018

### FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power \_\_\_\_\_ Foundation \_\_\_\_\_ Monolithic \_\_\_\_\_  
 date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_  
 Under slab rough-in plumbing \_\_\_\_\_ Slab \_\_\_\_\_ Sheathing/Nailing \_\_\_\_\_  
 date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_  
 Framing \_\_\_\_\_ Rough-in plumbing above slab and below wood floor \_\_\_\_\_  
 date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_  
 Electrical rough-in \_\_\_\_\_ Heat & Air Duct \_\_\_\_\_ Peri. beam (Lintel) \_\_\_\_\_  
 date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_  
 Permanent power \_\_\_\_\_ C.O. Final \_\_\_\_\_ Culvert \_\_\_\_\_  
 date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_  
 M/H tie downs, blocking, electricity and plumbing \_\_\_\_\_ Pool \_\_\_\_\_  
 date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_  
 Reconnection \_\_\_\_\_ Pump pole \_\_\_\_\_ Utility Pole \_\_\_\_\_  
 date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_  
 M/H Pole \_\_\_\_\_ Travel Trailer \_\_\_\_\_ Re-roof \_\_\_\_\_  
 date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_ date/app. by \_\_\_\_\_

BUILDING PERMIT FEE \$ 660.00 CERTIFICATION FEE \$ 20.30 SURCHARGE FEE \$ 20.30  
 MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$ \_\_\_\_\_  
 FLOOD DEVELOPMENT FEE \$ \_\_\_\_\_ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ \_\_\_\_\_ **TOTAL FEE** 775.60  
 INSPECTORS OFFICE \_\_\_\_\_ CLERKS OFFICE \_\_\_\_\_

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

### This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

## Columbia County Building Permit Application

**For Office Use Only** Application # 0707-25 Date Received 7/10/07 By LH Permit # 26182

Application Approved by - Zoning Official BZK Date 16.07.07 Plans Examiner OK JTH Date 7-10-07

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments MH will be removed after Const. 45 days of issuing C.O. Section 2.3.1

NOC  EH  Deed or PA  Site Plan  State Road Info  Parent Parcel #  Development Permit  
Fax N/A

Name Authorized Person Signing Permit Anthony B Williams JR Phone (386) 755-3343

Address 307 SW Gerald Witt GLN Lake City, Fla 32024

Owners Name Anthony B Williams JR + Rebecca Williams Phone (386) 755-3343

911 Address 307 SW Gerald Witt GLN Lake City, Fla. 32024

Contractors Name Owner Builder Phone \_\_\_\_\_

Address \_\_\_\_\_

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Jason Ellison 4853 W SE 238 Lake Butler, FL 32054 / 386-622-1241

Mortgage Lenders Name & Address N/A

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 13-55-15-00453-002 Estimated Cost of Construction 240,000

Subdivision Name N/A Lot \_\_\_\_\_ Block \_\_\_\_\_ Unit \_\_\_\_\_ Phase \_\_\_\_\_

Driving Directions 47 S to 240 W, to Ichucknee RD turn LEFT. 1/4 mile turn RIGHT onto Gerald Witt GLN. End of DRIVE.

Type of Construction New Home Number of Existing Dwellings on Property 1 (MH)

Total Acreage 2 Lot Size \_\_\_\_\_ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 82' Side 145' Side 110' Rear 115'

Total Building Height 21'5" Number of Stories 1 Heated Floor Area 2640 Roof Pitch 8:12  
*TOTAL 4060*

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

**OWNERS AFFIDAVIT:** I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

**WARNING TO OWNER:** YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Anthony B Williams Jr  
Owner Builder or Authorized Person by Notarized Letter

Contractor Signature \_\_\_\_\_  
Contractors License Number \_\_\_\_\_  
Competency Card Number \_\_\_\_\_  
NOTARY STAMP/SEAL

STATE OF FLORIDA  
COUNTY OF COLUMBIA



Sworn to (or affirmed) and subscribed  
this 10 day of July 20 07.

Laurie Hodson  
Notary Signature

Personally known \_\_\_\_\_ or Produced Identification

**NOTORIZED DISCLOSURE STATEMENT**

**FOR OWNER/BUILDER WHEN ACTING AS THEIR OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).**

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

**TYPE OF CONSTRUCTION**

Single Family Dwelling  
 Farm Outbuilding

Two-Family Residence  
 Other \_\_\_\_\_

**NEW CONSTRUCTION OR IMPROVEMENT**

New Construction

Addition, Alteration, Modification or other Improvement

I Anthony B. Wilkin, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number \_\_\_\_\_

CB Wilkin / 7/9/07  
Owner Builder Signature / Date

The above signer is personally known to me or produced identification

Notary Signature Laurie Hodson Date 7-10-07



( Stamp / Seal )

**FOR BUILDING USE ONLY**

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7).

Date 7-10-07 Building Official/Representative J. H.

NOTICE OF COMMENCEMENT FORM  
COLUMBIA COUNTY, FLORIDA

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY  
CLERKS OFFICE BEFORE YOUR FIRST INSPECTION

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Tax Parcel ID Number 13-55-15-00453-002-HX Permit Number \_\_\_\_\_

307 SW Gerald with GLN Lake City, Fla 32624

1. Description of property: (legal description of the property and street address or 911 address)

Parcel of land in SW COR of SW 1/4 of NE 1/4 of NE 1/4 being 210ft E & W by 228ft N+S & Comm at SE Cor of NE 1/4 of NE 1/4 of Sec. Run W 40 ft to W 1/2 of CR 238, cont W 1020.26 ft for POB, cont W 54.13 ft N 228ft W 21 ft N 101.66 ft E 264.42 ft S 329.66 ft to POB ORB 338-468 ORB 643.237 738-234

2. General description of improvement: Construction of New Home

3. Owner Name & Address Anthony B. Williams & Rebecca Williams

307 SW Gerald with GLN Lake City 32624 Interest in Property \_\_\_\_\_

4. Name & Address of Fee Simple Owner (if other than owner): \_\_\_\_\_

5. Contractor Name Anthony B. Williams (owner) Phone Number 386-755-3343

Address 307 SW Gerald with GLN Lake City, FL 32624

6. Surety Holders Name NA Phone Number \_\_\_\_\_

Address \_\_\_\_\_

Amount of Bond \_\_\_\_\_

Inst:200712015188 Date:7/9/2007 Time:2:07 PM  
DC,P.DeWitt Cason, Columbia County Page 1 of 1

7. Lender Name NA

Address \_\_\_\_\_

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:

Name NA Phone Number \_\_\_\_\_

Address \_\_\_\_\_

9. In addition to himself/herself the owner designates \_\_\_\_\_ of

\_\_\_\_\_ to receive a copy of the Lien Notice as provided in Section 713.13 (1) -

(a) 7. Phone Number of the designee \_\_\_\_\_

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified) \_\_\_\_\_

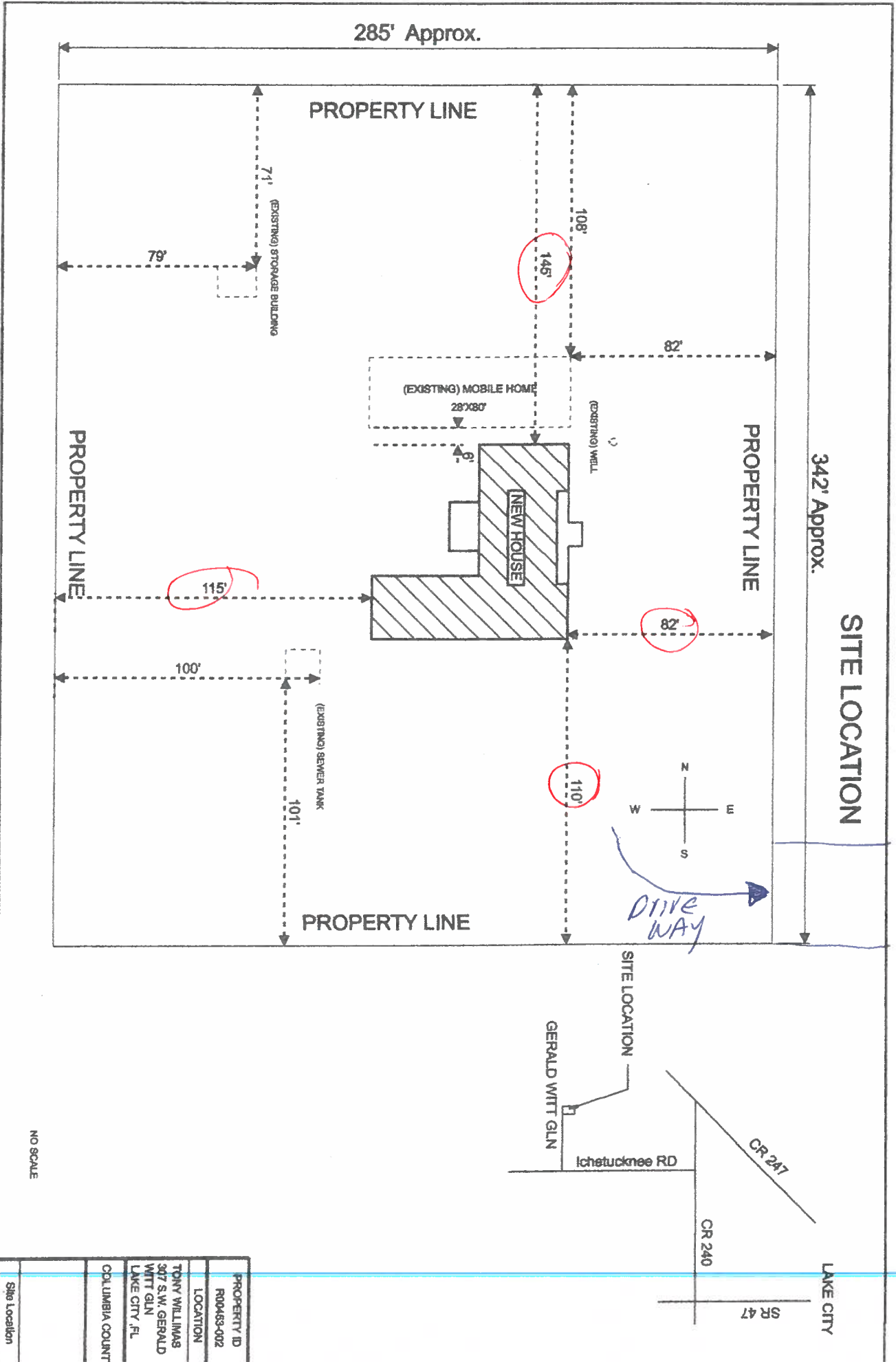
THE OWNER MUST SIGN THE NOTICE OF COMMENCEMENT AND NO ONE ELSE MAY BE PERMITTED TO SIGN IN HIS/HER STEAD.

AB Williams  
Signature of Owner

Sworn to (or affirmed) and subscribed before day of July 9, 2007.  
RoseAnn Aiello provided FLDL AS ID

Signature of Notary RoseAnn Aiello





PROPERTY ID	R00463-002
LOCATION	TONY WILLIAMS 347 S.W. GERALD WITT GLN LAKE CITY, FL
COLUMBIA COUNTY	
Site Location	

EX 0730 160234

Prepared by Martin M. Fugle

OFFICIAL RECORDS

WARRANTY DEED

Attorney at Law  
Post Office Box 1853  
Lake City, Florida 32055  
Phone 336-2000

THIS INSTRUMENT, made this 12<sup>th</sup> day of March, 1990,  
between **TERECIE WITT** and her husband, **GERALD WITT**, of the  
County of Columbia, State of Florida, Grantor, and **BERROCCA  
WILLIAMS**, whose post-office address is Route 2, Box 499, Lake  
City, Florida 32055, of the County of Columbia, State of  
Florida, Grantee,

**WITNESSETH:**

That said grantor, for and in consideration of the sum  
of **TEN AND NO/100 (\$10.00)** Dollars, and other good and  
valuable considerations to said grantor in hand paid by said  
grantee, the receipt whereof is hereby acknowledged, has  
granted, bargained and sold to the said grantee, and  
grantee's heirs, successors and assigns forever, the  
following described land, situate, lying and being in  
Columbia County, Florida, to-wit:

**TOWNSHIP 5 SOUTH - RANGE 15 EAST**

**Section 13:** A parcel of land in the Southwest  
corner of the Southwest 1/4 of Northeast 1/4 of  
Northeast 1/4 being 210 feet in length running East  
and West and 228 feet in length running North and  
South.

- ALSO -

A perpetual non-exclusive easement as to the South  
40 feet of the NE 1/4 of the NE 1/4 for ingress and  
egress from the property to Old Itchetucknee Road.  
Said easement shall run with the land.

**N.B.** This deed is made subject to the restriction that  
neither grantee nor his assigns may sell, mortgage,  
or otherwise encumber the property described herein  
during the lifetime of either grantor, without  
first obtaining the written consent of grantors or  
the survivor thereof.

and said grantor does hereby fully warrant the title to said  
land, and will defend the same against the lawful claims of  
all persons whatsoever.

**IN WITNESS WHEREOF**, Grantor has hereunto set grantor's  
hand and seal the day and year first above written.

**SIGNED, SEALED AND DELIVERED  
IN OUR PRESENCE:**

*Martin M. Fugle*  
*Dwight Cason*

*Terecie Witt* (SEAL)  
**TERECIE WITT**  
*Gerald Witt* (SEAL)  
**GERALD WITT**

DOCUMENTARY STAMP 55  
INTANGIBLE TAX 0  
P. DeWITT CASON, CLERK OF  
COURTS, COLUMBIA COUNTY  
BY *P. Rowley* D.C.

STATE OF FLORIDA  
CLERK OF COURTS  
COUNTY OF FLORIDA

DC 0738 PG 0235

OFFICIAL RECORDS

88-14951

RECORDED  
DEC 12 PM 12 '85  
RECORD VERIFIED  
CLERK OF COURTS  
FLORIDA COUNTY, FLORIDA  
D.C.

A PARCEL OF LAND IN SW COR OF  
 SW1/4 OF NE1/4 OF NE1/4, BEING  
 210 FT E & W BY 228 FT N & S.  
 & COMM AT SE COR OF NE1/4 OF

WILLIAMS REBECCA & ANTHONY JR 13-5S-15-00453-002  
 P O BX 3162  
 LAKE CITY, FL 32056

PRINTED 5/11/2007 13:44  
 APPR 5/12/2006 DFTW

Columbia County 2007 R  
 CARD 001 OF 001  
 BY JEFF

BUSE 000800 MOBILE HME AEP Y 1976 HTD AREA 113,900 INDEX 13515.00 DIST 3  
 MOD 2 MOBILE HME BATH 2.00 2026 EFF AREA 31,892 E-RATE 100,000 INDX  
 EXW 31 VINYL SID FIXT 64613 RCN 1993 AYB  
 RSTR 03 GABLE/HIP RMS BDRM 71.00 %GOOD 45,875 B BLDG VAL 1993 EYB  
 RCVR 03 COMP SHNGL UNITS C-W% #LOC: 307 GERALD WITT GLN SW LAKE CITY  
 INTW 05 DRYWALL HGTHT PMTR # I BAS1993  
 FLOR 14 CARPET STYS 1.0 # I BAS1993  
 10% 08 SHT VINYL ECON 2 # I  
 HTTP 04 AIR DUCTED FUNC SPCD DEPR 09 # I  
 A/C 03 CENTRAL SPCD DEPR 09 # I  
 QUAL 05 05 UD-1 N/A # I  
 FENDN N/A UD-2 N/A # I  
 SIZE N/A UD-3 N/A # I  
 CELL N/A UD-4 N/A # I  
 ARCH N/A UD-5 N/A # I  
 FRME 01 NONE UD-6 N/A # I  
 KITCH 01 01 UD-7 N/A # I  
 WINDO N/A UD-8 N/A # I  
 CLAS N/A UD-9 N/A # I  
 OCC N/A UD-9 N/A # I  
 COND 03 03 SUB VALUE # I  
 SUB A-AREA % E-AREA 1976 44743 # I  
 BAS93 1976 100 1976 44743 # I  
 POP93 144 35 50 1132 # I

FIELD CK: 44-12-32  
 BLDG TRAVERSE S12 E12 N12  
 W12S E32 N26S.  
 BAS1993=W76 S26 E44 FOP1993=  
 TXDT 003

GRANTOR GRANTEE

AE BN	CODE	DESC	LEN	WID	HEHT	QTY	OL	YR	ADJ	UNITS	UT	PRICE	ADJ UT PR	SPCD %	%GOOD	XFOB	VALUE	
Y	0297	SHED CONCRET				1		2005	1.00	1.000	UT	1000.000	1000.000		100.00		1,000	
Y	0252	LEAN-TO W/O				1		2005	1.00	1.000	UT	200.000	200.000		100.00		200	
TOTAL		2120	EXTRA FEATURES	2026	45875													

LAND DESC ZONE ROAD {UD1 {UD3 FRONT DEPTH FIELD CK:  
 UTIL {UD2 {UD4 BACK DT ADJUSTMENTS  
 Y 000102 SFR/MH A-1 0002 0003  
 Y 000102 SFR/MH A-1 0002 0003  
 Y 009945 WEILL/SEPT A-1 0002 0003  
 B001 - GLENN MH 0002 0003  
 2007 SALE - (DAUGHTER)



# Columbia County Property Appraiser

DB Last Updated: 5/11/2007

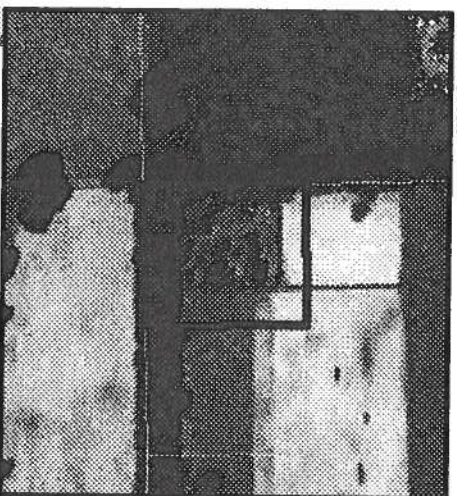
Parcel: 13-5S-15-00453-002 HX

## 2007 Proposed Values

### Owner & Property Info

Owner's Name	WILLIAMS REBECCA & ANTHONY JR		
Site Address	GERALD WITT		
Mailing Address	P O BX 3162 LAKE CITY, FL 32056		
Use Desc. (code)	MOBILE HOM (000200)	Tax District	3
Neighborhood	13515.00	Market Area	02
UD Codes	MKTA02		
Total Land Area	2.000 ACRES		
Description	A PARCEL OF LAND IN SW COR OF SW1/4 OF NE1/4 OF NE1/4, BEING 210 FT E & W BY 228 FT N & S. & COMM AT SE COR OF NE1/4 OF NE1/4 OF SEC. RUN W 40 FT TO W R/W OF CR 238, CONT W 1020.26 FT FOR POB, CONT W 54.13 FT, N 228 FT, W 210 FT N 101.66 FT, E 264.42 FT, S 329.66 FT TO POB. ORB 338-468, ORB 643-233, 738-234, 954-2281 PROB 972-1329, WD 1083-187.		

GIS Aerial



<< Prev Search Result: 4 of 4

Tax Record | Property Card | Interactive GIS Map | Print

### Property & Assessment Values

Mkt Land Value	cnt: (2)	\$28,400.00
Ag Land Value	cnt: (0)	\$0.00
Building Value	cnt: (1)	\$45,875.00
XFOB Value	cnt: (2)	\$1,200.00
Total Appraised Value		\$75,475.00

Just Value	\$75,475.00
Class Value	\$0.00
Assessed Value	\$44,843.00
Exempt Value	(code: HX) \$25,000.00
Total Taxable Value	\$19,843.00

### Sales History

Sale Date	Book/Page	Inst. Type	Sale Vimp	Sale Qual	Sale RCode	Sale Price
12/12/1990	738/234	WD	V	U	03	\$0.00

### Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	MOBILE HME (000800)	1993	Vinyl Side (31)	1976	2120	\$45,875.00

Note: All S.F. calculations are based on exterior building dimensions.

**Extra Features & Out Buildings**

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0297	SHED CONCR	2005	\$1,000.00	1.000	0 x 0 x 0	(.00)
0252	LEAN-TO W/	2005	\$200.00	1.000	0 x 0 x 0	(.00)

**Land Breakdown**

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000102	SFR/MH (MKT)	2.000 AC	1.00/1.00/1.00/1.00	\$13,200.00	\$26,400.00
009945	WELL/SEPT (MKT)	1.000 UT - (.000AC)	1.00/1.00/1.00/1.00	\$2,000.00	\$2,000.00

Columbia County Property Appraiser

DB Last Updated: 5/11/2007

<< Prev

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**Disclaimer**

This information was derived from data which was compiled by the Columbia County Property Appraiser's Office solely for the government purpose of property assessment. The information shown is a work in progress and should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's Office. The assessed values are NOT CERTIFIED values and therefore are subject to change before finalized for ad-valorem assessment purposes.

**Notice:**

Under Florida Law, e-mail addresses are public record. If you do not want your e-mail address released in response to a public-records request, do not send electronic mail to this entity. Instead contact this office by phone or in writing.

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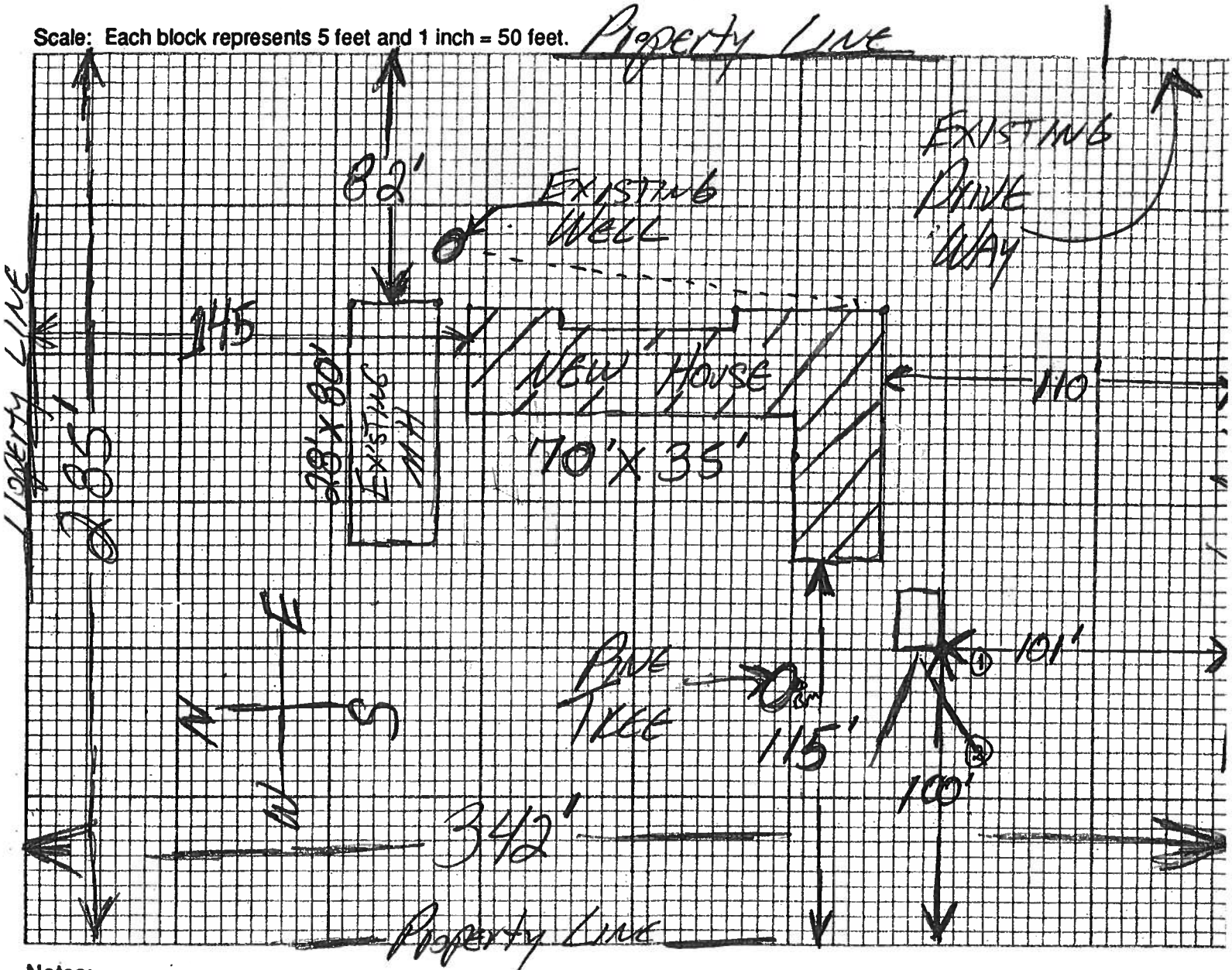
STATE OF FLORIDA  
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 07-0650-1

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes:  
EXISTING MOBILE HOME TO BE REMOVED AFTER NEW HOME COMPLETED. plan approved MSC

Site Plan submitted by: William Signature Title OWNER

Plan Approved APPROVED Not Approved \_\_\_\_\_ Date 8/28/17

By [Signature] Columbia CHD Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs  
Residential Whole Building Performance Method A

<b>Project Name:</b> WILLIAMS RESIDENCE <b>Address:</b> <b>City, State:</b> LAKE CITY, FL <b>Owner:</b> TONY WILLIAMS <b>Climate Zone:</b> North	<b>Builder:</b> OWNER <b>Permitting Office:</b> Columbia <b>Permit Number:</b> 26182 <b>Jurisdiction Number:</b> 221006
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<p>1. New construction or existing <span style="float: right;">New</span> <input type="checkbox"/></p> <p>2. Single family or multi-family <span style="float: right;">Single family</span> <input type="checkbox"/></p> <p>3. Number of units, if multi-family <span style="float: right;">1</span> <input type="checkbox"/></p> <p>4. Number of Bedrooms <span style="float: right;">3</span> <input type="checkbox"/></p> <p>5. Is this a worst case? <span style="float: right;">Yes</span> <input type="checkbox"/></p> <p>6. Conditioned floor area (ft<sup>2</sup>) <span style="float: right;">2640 ft<sup>2</sup></span> <input type="checkbox"/></p> <p>7. Glass type<sup>1</sup> and area: (Label reqd. by 13-104.4.5 if not default)</p> <p style="margin-left: 20px;">a. U-factor: <span style="margin-left: 100px;">Description</span> <span style="margin-left: 100px;">Area</span></p> <p style="margin-left: 40px;">(or Single or Double DEFAULT) 7a. (Dble Default) 320.5 ft<sup>2</sup> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. SHGC:</p> <p style="margin-left: 40px;">(or Clear or Tint DEFAULT) 7b. (Clear) 320.5 ft<sup>2</sup> <input type="checkbox"/></p> <p>8. Floor types</p> <p style="margin-left: 20px;">a. Stem Wall <span style="margin-left: 100px;">R=3.5, 2640.0ft<sup>2</sup></span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p>9. Wall types</p> <p style="margin-left: 20px;">a. Frame, Wood, Exterior <span style="margin-left: 100px;">R=13.0, 1717.5 ft<sup>2</sup></span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">d. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">e. N/A <input type="checkbox"/></p> <p>10. Ceiling types</p> <p style="margin-left: 20px;">a. Under Attic <span style="margin-left: 100px;">R=30.0, 2640.0 ft<sup>2</sup></span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p>11. Ducts</p> <p style="margin-left: 20px;">a. Sup: Con. Ret: Con. AH: Interior <span style="margin-left: 100px;">Sup. R=6.0, 170.0 ft</span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p>	<p>12. Cooling systems</p> <p style="margin-left: 20px;">a. Central Unit <span style="float: right;">Cap: 36.0 kBtu/hr</span> <input type="checkbox"/></p> <p style="margin-left: 40px;"><span style="float: right;">SEER: 13.00</span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p>13. Heating systems</p> <p style="margin-left: 20px;">a. Electric Heat Pump <span style="float: right;">Cap: 36.0 kBtu/hr</span> <input type="checkbox"/></p> <p style="margin-left: 40px;"><span style="float: right;">HSPF: 7.70</span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p>14. Hot water systems</p> <p style="margin-left: 20px;">a. Electric Resistance <span style="float: right;">Cap: 50.0 gallons</span> <input type="checkbox"/></p> <p style="margin-left: 40px;"><span style="float: right;">EF: 0.93</span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) <input type="checkbox"/></p> <p>15. HVAC credits <span style="float: right;">PT, CF,</span> <input type="checkbox"/></p> <p style="margin-left: 20px;">(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)</p>
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Glass/Floor Area: 0.12	Total as-built points: 27102	PASS
	Total base points: 32148	

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

**PREPARED BY:** Jason Elison


**DATE:** 6-15-07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

**OWNER/AGENT:** \_\_\_\_\_

**DATE:** \_\_\_\_\_

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



**BUILDING OFFICIAL:** \_\_\_\_\_

**DATE:** \_\_\_\_\_

<sup>1</sup> Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.  
EnergyGauge® (Version: FLRCPB v4.5.2)

# Code Compliance Checklist

## Residential Whole Building Performance Method A - Details

ADDRESS: , LAKE CITY, FL,

PERMIT #:

**6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST**

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

**6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)**

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

**ESTIMATED ENERGY PERFORMANCE SCORE\* = 88.1**

**The higher the score, the more efficient the home.**

TONY WILLIAMS, , LAKE CITY, FL,

<p>1. New construction or existing <span style="float: right;">New</span> <input type="checkbox"/></p> <p>2. Single family or multi-family <span style="float: right;">Single family</span> <input type="checkbox"/></p> <p>3. Number of units, if multi-family <span style="float: right;">1</span> <input type="checkbox"/></p> <p>4. Number of Bedrooms <span style="float: right;">3</span> <input type="checkbox"/></p> <p>5. Is this a worst case? <span style="float: right;">Yes</span> <input type="checkbox"/></p> <p>6. Conditioned floor area (ft<sup>2</sup>) <span style="float: right;">2640 ft<sup>2</sup></span> <input type="checkbox"/></p> <p>7. Glass type<sup>1</sup> and area: (Label reqd. by 13-104.4.5 if not default)</p> <p style="margin-left: 20px;">a. U-factor: <span style="float: right;">Description Area</span></p> <p style="margin-left: 40px;">(or Single or Double DEFAULT) <span style="float: right;">7a. (Dble Default) 320.5 ft<sup>2</sup></span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. SHGC:</p> <p style="margin-left: 40px;">(or Clear or Tint DEFAULT) <span style="float: right;">7b. (Clear) 320.5 ft<sup>2</sup></span> <input type="checkbox"/></p> <p>8. Floor types</p> <p style="margin-left: 20px;">a. Stem Wall <span style="float: right;">R=3.5, 2640.0ft<sup>2</sup></span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p>9. Wall types</p> <p style="margin-left: 20px;">a. Frame, Wood, Exterior <span style="float: right;">R=13.0, 1717.5 ft<sup>2</sup></span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">d. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">e. N/A <input type="checkbox"/></p> <p>10. Ceiling types</p> <p style="margin-left: 20px;">a. Under Attic <span style="float: right;">R=30.0, 2640.0 ft<sup>2</sup></span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p>11. Ducts</p> <p style="margin-left: 20px;">a. Sup: Con. Ret: Con. AH: Interior <span style="float: right;">Sup. R=6.0, 170.0 ft</span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p>	<p>12. Cooling systems</p> <p style="margin-left: 20px;">a. Central Unit <span style="float: right;">Cap: 36.0 kBtu/hr</span> <input type="checkbox"/></p> <p style="margin-left: 40px;"><span style="float: right;">SEER: 13.00</span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p>13. Heating systems</p> <p style="margin-left: 20px;">a. Electric Heat Pump <span style="float: right;">Cap: 36.0 kBtu/hr</span> <input type="checkbox"/></p> <p style="margin-left: 40px;"><span style="float: right;">HSPF: 7.70</span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p>14. Hot water systems</p> <p style="margin-left: 20px;">a. Electric Resistance <span style="float: right;">Cap: 50.0 gallons</span> <input type="checkbox"/></p> <p style="margin-left: 40px;"><span style="float: right;">EF: 0.93</span> <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) <input type="checkbox"/></p> <p>15. HVAC credits <span style="float: right;">PT, CF,</span> <input type="checkbox"/></p> <p style="margin-left: 20px;">(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating) <input type="checkbox"/></p>
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I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: \_\_\_\_\_ Date: \_\_\_\_\_

Address of New Home: \_\_\_\_\_ City/FL Zip: \_\_\_\_\_



*\*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.  
EnergyGauge® (Version: FLRCPB v4.5.2)

# Residential System Sizing Calculation

## Summary

TONY WILLIAMS  
LAKE CITY, FL

Project Title:  
WILLIAMS RESIDENCE

Code Only  
Professional Version  
Climate: North

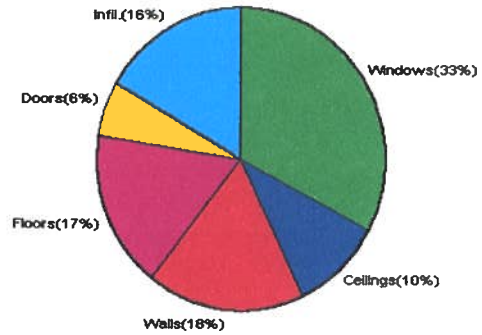
6/17/2007

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M)			
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)			
Winter design temperature	33 F	Summer design temperature	92 F
Winter setpoint	70 F	Summer setpoint	75 F
Winter temperature difference	37 F	Summer temperature difference	17 F
<b>Total heating load calculation</b>	<b>31339 Btuh</b>	<b>Total cooling load calculation</b>	<b>32682 Btuh</b>
Submitted heating capacity	% of calc Btuh	Submitted cooling capacity	% of calc Btuh
Total (Electric Heat Pump)	114.9 36000	Sensible (SHR = 0.75)	88.9 27000
Heat Pump + Auxiliary(0.0kW)	114.9 36000	Latent	388.7 9000
		Total (Electric Heat Pump)	110.2 36000

## WINTER CALCULATIONS

Winter Heating Load (for 2640 sqft)

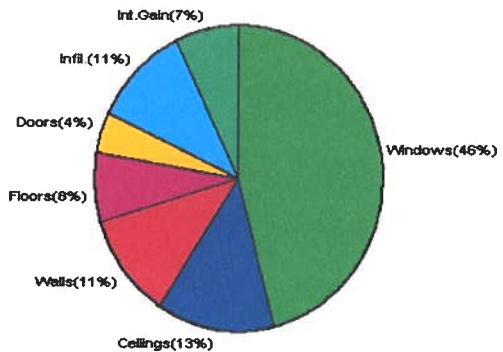
Load component	Load	
Window total	321 sqft	10317 Btuh
Wall total	1718 sqft	5640 Btuh
Door total	140 sqft	1813 Btuh
Ceiling total	2640 sqft	3111 Btuh
Floor total	2640 sqft	5325 Btuh
Infiltration	127 cfm	5133 Btuh
Duct loss		0 Btuh
<b>Subtotal</b>		<b>31339 Btuh</b>
Ventilation	0 cfm	0 Btuh
<b>TOTAL HEAT LOSS</b>		<b>31339 Btuh</b>



## SUMMER CALCULATIONS

Summer Cooling Load (for 2640 sqft)

Load component	Load	
Window total	321 sqft	14969 Btuh
Wall total	1718 sqft	3582 Btuh
Door total	140 sqft	1372 Btuh
Ceiling total	2640 sqft	4372 Btuh
Floor total		2492 Btuh
Infiltration	63 cfm	1179 Btuh
Internal gain		2400 Btuh
Duct gain		0 Btuh
Sens. Ventilation	0 cfm	0 Btuh
<b>Total sensible gain</b>		<b>30366 Btuh</b>
Latent gain(ducts)		0 Btuh
Latent gain(infiltration)		2316 Btuh
Latent gain(ventilation)		0 Btuh
Latent gain(internal/occupants/other)		0 Btuh
<b>Total latent gain</b>		<b>2316 Btuh</b>
<b>TOTAL HEAT GAIN</b>		<b>32682 Btuh</b>



Version 8  
For Florida residences only

EnergyGauge® System Sizing

PREPARED BY: Jason Elyson

DATE: 6-15-07

# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: , LAKE CITY, FL,	PERMIT #:
---------------------------	-----------

BASE	AS-BUILT
<b>GLASS TYPES</b> .18 X Conditioned X BSPM = Points Floor Area	Overhang Ornt Len Hgt Area X SPM X SOF = Points
.18      2640.0      18.59      8834.0	1.Double, Clear      N   0.0   0.0   80.0   19.20   1.00   1535.0 2.Double, Clear      N   0.0   0.0   32.0   19.20   1.00   614.0 3.Double, Clear      E   0.0   0.0   80.0   42.06   1.00   3365.0 4.Double, Clear      E   0.0   0.0   32.0   42.06   1.00   1346.0 5.Double, Clear      S   0.0   0.0   12.5   35.87   1.00   448.0 6.Double, Clear      W   0.0   0.0   60.0   38.52   1.00   2311.0 7.Double, Clear      W   0.0   0.0   24.0   38.52   1.00   924.0
	<b>As-Built Total:</b> 320.5      10543.0
<b>WALL TYPES</b> Area X BSPM = Points	Type      R-Value      Area X SPM = Points
Adjacent      0.0      0.00      0.0 Exterior      1717.5      1.70      2919.8	1. Frame, Wood, Exterior      13.0      1717.5      1.50      2576.3
<b>Base Total:</b> 1717.5      2919.8	<b>As-Built Total:</b> 1717.5      2576.3
<b>DOOR TYPES</b> Area X BSPM = Points	Type      Area X SPM = Points
Adjacent      0.0      0.00      0.0 Exterior      140.0      6.10      854.0	1.Exterior Insulated      60.0      4.10      246.0 2.Exterior Insulated      80.0      4.10      328.0
<b>Base Total:</b> 140.0      854.0	<b>As-Built Total:</b> 140.0      574.0
<b>CEILING TYPES</b> Area X BSPM = Points	Type      R-Value      Area X SPM X SCM = Points
Under Attic      2640.0      1.73      4567.2	1. Under Attic      30.0      2640.0      1.73 X 1.00      4567.2
<b>Base Total:</b> 2640.0      4567.2	<b>As-Built Total:</b> 2640.0      4567.2
<b>FLOOR TYPES</b> Area X BSPM = Points	Type      R-Value      Area X SPM = Points
Slab      0.0(p)      0.0      0.0 Raised      2640.0      -3.99      -10533.6	1. Stem Wall      3.5      2640.0      -4.70      -12408.0
<b>Base Total:</b> -10533.6	<b>As-Built Total:</b> 2640.0      -12408.0
<b>INFILTRATION</b> Area X BSPM = Points	Area X SPM = Points
2640.0      10.21      26954.4	2640.0      10.21      26954.4



# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: , LAKE CITY, FL,	PERMIT #:
---------------------------	-----------

BASE	AS-BUILT
<b>Summer Base Points: 33595.8</b>	<b>Summer As-Built Points: 32806.9</b>
Total Summer X System = Cooling Points Multiplier Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)
<b>33595.8 0.3250 10918.6</b>	<small>(sys 1: Central Unit 36000btuh , SEER/EFF(13.0) Ducts: Con(S), Con(R), Int(AH), R6.0(INS)</small> 32807 1.00 (1.00 x 1.147 x 0.91) 0.260 0.902 8035.1 <b>32806.9 1.00 1.044 0.260 0.902 8035.1</b>

# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: , LAKE CITY, FL,	PERMIT #:
---------------------------	-----------

BASE	AS-BUILT																																																
<b>GLASS TYPES</b> .18 X Conditioned X BWPM = Points Floor Area	<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2" style="width: 30%;">Type/SC</th> <th colspan="3" style="text-align: center;">Overhang</th> <th rowspan="2" style="width: 10%;">Area X WPM X WOF = Points</th> </tr> <tr> <th style="width: 5%;">Ornt</th> <th style="width: 5%;">Len</th> <th style="width: 5%;">Hgt</th> </tr> </thead> <tbody> <tr> <td>1.Double, Clear</td> <td>N</td> <td>0.0</td> <td>0.0</td> <td>80.0 24.58 1.00 1966.0</td> </tr> <tr> <td>2.Double, Clear</td> <td>N</td> <td>0.0</td> <td>0.0</td> <td>32.0 24.58 1.00 786.0</td> </tr> <tr> <td>3.Double, Clear</td> <td>E</td> <td>0.0</td> <td>0.0</td> <td>80.0 18.79 1.00 1503.0</td> </tr> <tr> <td>4.Double, Clear</td> <td>E</td> <td>0.0</td> <td>0.0</td> <td>32.0 18.79 1.00 601.0</td> </tr> <tr> <td>5.Double, Clear</td> <td>S</td> <td>0.0</td> <td>0.0</td> <td>12.5 13.30 1.00 166.0</td> </tr> <tr> <td>6.Double, Clear</td> <td>W</td> <td>0.0</td> <td>0.0</td> <td>60.0 20.73 1.00 1243.0</td> </tr> <tr> <td>7.Double, Clear</td> <td>W</td> <td>0.0</td> <td>0.0</td> <td>24.0 20.73 1.00 497.0</td> </tr> <tr> <td colspan="4"><b>As-Built Total:</b></td> <td style="text-align: right;"><b>320.5 6762.0</b></td> </tr> </tbody> </table>	Type/SC	Overhang			Area X WPM X WOF = Points	Ornt	Len	Hgt	1.Double, Clear	N	0.0	0.0	80.0 24.58 1.00 1966.0	2.Double, Clear	N	0.0	0.0	32.0 24.58 1.00 786.0	3.Double, Clear	E	0.0	0.0	80.0 18.79 1.00 1503.0	4.Double, Clear	E	0.0	0.0	32.0 18.79 1.00 601.0	5.Double, Clear	S	0.0	0.0	12.5 13.30 1.00 166.0	6.Double, Clear	W	0.0	0.0	60.0 20.73 1.00 1243.0	7.Double, Clear	W	0.0	0.0	24.0 20.73 1.00 497.0	<b>As-Built Total:</b>				<b>320.5 6762.0</b>
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7.Double, Clear	W	0.0	0.0	24.0 20.73 1.00 497.0																																													
<b>As-Built Total:</b>				<b>320.5 6762.0</b>																																													
<b>WALL TYPES</b> Area X BWPM = Points	<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="width: 40%;">Type</th> <th style="width: 10%;">R-Value</th> <th style="width: 10%;">Area X WPM = Points</th> </tr> </thead> <tbody> <tr> <td>1. Frame, Wood, Exterior</td> <td>13.0</td> <td>1717.5 3.40 5839.5</td> </tr> <tr> <td colspan="3"><b>As-Built Total:</b></td> </tr> <tr> <td colspan="2"></td> <td style="text-align: right;"><b>1717.5 5839.5</b></td> </tr> </tbody> </table>	Type	R-Value	Area X WPM = Points	1. Frame, Wood, Exterior	13.0	1717.5 3.40 5839.5	<b>As-Built Total:</b>					<b>1717.5 5839.5</b>																																				
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# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: , LAKE CITY, FL,	PERMIT #:
---------------------------	-----------

BASE			AS-BUILT					
<b>Winter Base Points: 24050.6</b>			<b>Winter As-Built Points: 24496.0</b>					
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
<b>24050.6</b>	<b>0.5540</b>	<b>13324.1</b>	(sys 1: Electric Heat Pump 36000 btuh ,EFF(7.7) Ducts:Con(S),Con(R),Int(AH),R6.0 24496.0 1.000 (1.000 x 1.169 x 0.93)0.443 0.950 11204.2					
<b>24050.6</b>	<b>0.5540</b>	<b>13324.1</b>	<b>24496.0</b>	<b>1.00</b>	<b>1.087</b>	<b>0.443</b>	<b>0.950</b>	<b>11204.2</b>

# WATER HEATING & CODE COMPLIANCE STATUS

## Residential Whole Building Performance Method A - Details

ADDRESS: , LAKE CITY, FL,	PERMIT #:
---------------------------	-----------

BASE				AS-BUILT							
<b>WATER HEATING</b>											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	X Multiplier	X Credit Multiplier	= Total
3		2635.00	7905.0	50.0	0.93	3		1.00	2620.76	1.00	7862.3
<b>As-Built Total:</b>											<b>7862.3</b>

<b>CODE COMPLIANCE STATUS</b>											
BASE					AS-BUILT						
Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points
<b>10919</b>		<b>13324</b>		<b>7905</b>	<b>32148</b>	<b>8035</b>		<b>11204</b>		<b>7862</b>	<b>27102</b>

PASS



**Schafer Engineering LLC**

**14952 Main St. Alachua FL 32615**



**E**

Prepared for:

**JASON ELIXSON CONSTRUCTION  
THE WILLIAMS RESIDENCE**

By:

**Schafer Engineering, LLC**

**386-462-1340 / 352-375-6329**

***NO COPIES ARE TO BE PERMITTED***

# SCHAFFER ENGINEERING LLC

**Trusses:** Pre-engineered with manufacturer's required bracing system installed.

**Roof sheathing:** Type OSB Size 7/16 Fastener type nails 8d/113 Ring Shank

Interior zone spacing: Interior 6 in. Periphery 3 in.

Edge and end zone spacing: Interior 6 in. Periphery 3 in.

**Top double pl:** Type Spruce Grade #1 #2 Size 2 x 4 Nail spacing 10 in.

**Studs:** Type Spruce Grade #1 #2 Size 2 x 4

Interior stud spacing 16 in. Composite (yes or no) Y

End stud spacing 16 in. Composite (yes or no) Y

**Shearwall siding:** Type OSB Thickness 7/16 in.

74' — Trans: Fastener 8d/131 Spacing: Int 8 in. Edge 3 in.  
62' — Long: Fastener 8d/131 Spacing: Int 8 in. Edge 3 in.

**Allowable unit shear on shearwalls:** 418 pounds per linear foot

**Unit shear transferred from diaphragm:** Trans: 246 Long: 58

**Wall tension transferred by:** Siding nails 8d/131 @ 3 O.C. edges

**Foundation anchor bolts:** Concrete strength 3000 psi Size 1/2 in. Shape L  
Washer 2" Embedment 7 in. Location of first anchor bolt from corner 8 in.

**Anchor Bolts @ 48" O.C.** Model A307 Loc. from corner 8 in.

**Type of foundation:** 1 #5 rebar continuous required in bond beam.

Floor slab 4 in. CMU: Size 8 x 16 in. Height 24 in. Reinf. #5 at 72 in.

Monolithic footing: Depth 20 in. Bottom width 12 in. Reinf. 2 # 5 bars

**Footing:** Width 20 in. Depth 10 in. Reinforcing 3 # 5 bars

Interior Footings: 16" W X 10" D

**Porch Columns:** 6x6x9'5" syp #2 @ 14" o.c. max **Column Fasteners:** Sipro - C1366/C166

**Special Comments:** Install a ceiling diaphragm on open porches using same nails,  
same nail patterns & same grade material as roof sheathing.

## NOTE:

1. Balloon frame ALL gable ends unless this summary is accompanied by Gable End Wall Brace detail.
2. All trusses must bear on exterior walls & porch beams.
3. All walls to be nailed with same nailing pattern as shear walls.
4. This is a wind load only, NOT a structural analysis.
5. This wind load is not valid without a raised, embossed seal.
6. It is assumed that ideal soil conditions and pad preparations are provided.
7. Fiber mesh or WWM may be used in concrete slab.
8. Trusses must be anchored and supported in accordance to the truss engineering.
9. Wind design and analysis valid for one use only, no copies permitted.
10. The foundation is for minimum design use and may be increased.
11. All headers over 12 feet to be pre-engineered.

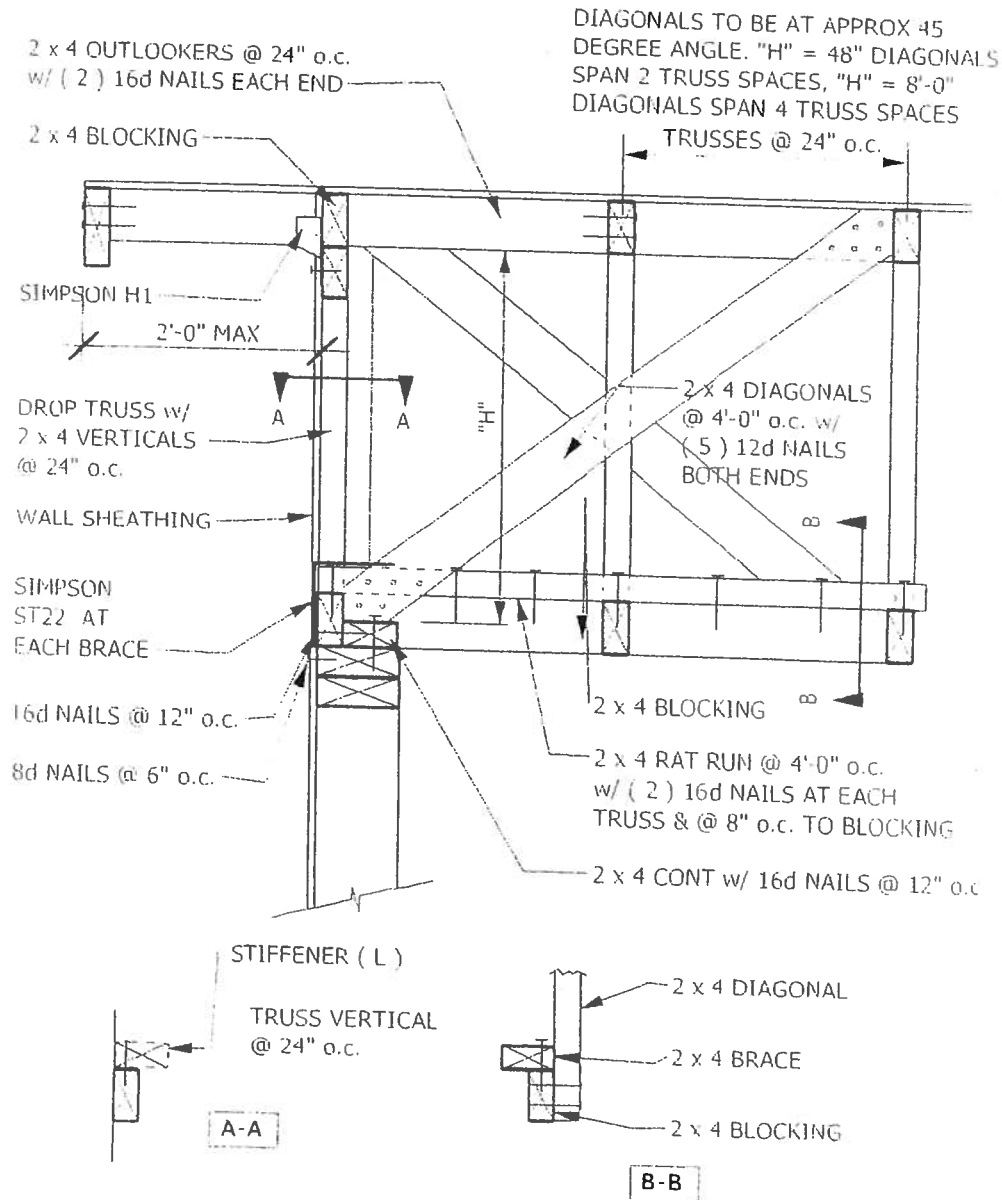
48984

7104 NW 42nd Ln

Gainesville, FL

# SCHAFFER ENGINEERING, LLC

7104 N. W. 42<sup>ND</sup> LANE  
GAINESVILLE, FLORIDA 32606

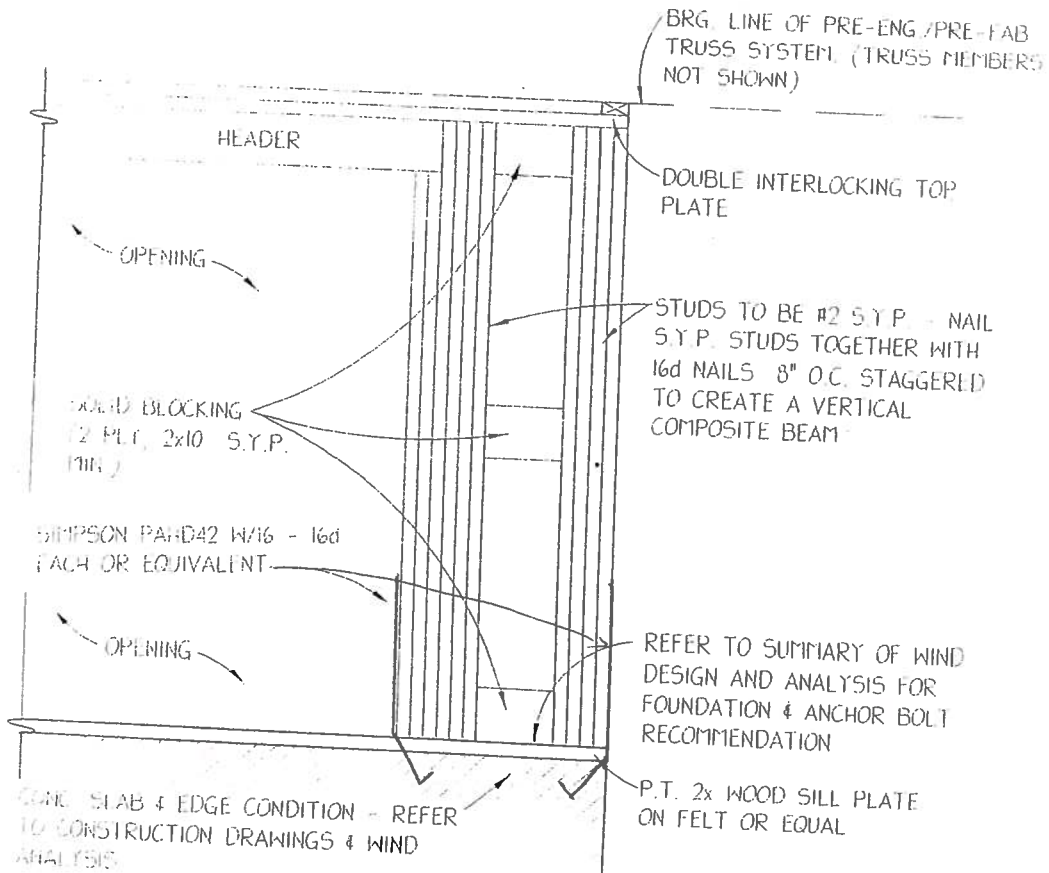


TYPICAL GABLE END BRACING

*[Handwritten Signature]*  
6-14-07

# SCHAFFER ENGINEERING, LLC

7104 N. W. 42<sup>ND</sup> LANE  
GAINESVILLE, FLORIDA 32606



## MINIMUM 2'-0" SHEAR WALL SEGMENT

ALLOWABLE EQUIVALENT SHEAR WALL EQUAL TO 1.5 TIMES THE ACTUAL WALL SEGMENT LENGTH. MINIMUM WALL SEGMENT LENGTH OF 2'-0" WHICH EQUATES TO A 3'-0" SHEAR WALL SEGMENT. SEE WINDLOAD ANALYSIS FOR NAIL SIZES AND SPACING ON SHEATHING.

*[Signature]*  
6-18-07

48984  
7104 NW 42nd Ln  
Gainesville, FL

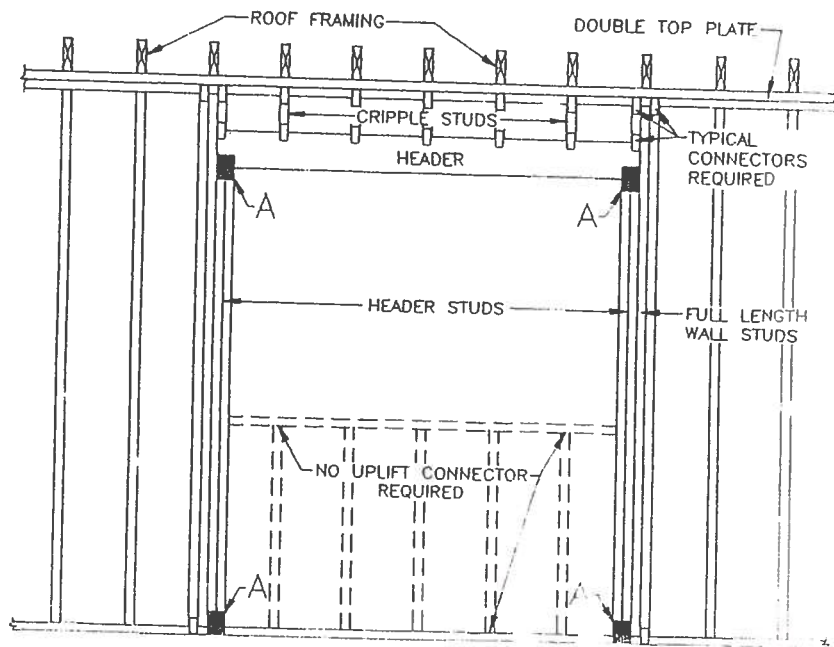


Unsupported Wall Height	Stud Spacing	Maximum Header Span (ft.)					
		3'	6'	9'	12'	15'	18'
		Number of Header Stud Supporting End of Header					
		1 <sup>1</sup>	1	2	2	2	2
		Number of Full-Length Studs at Each End of Header					
10' or less	12 in.	2	2	3	3	3	3
	16 in.	2	2	3	3	3	3
	24 in.	1	2	2	2	2	2
greater than 10'	12 in.	2	2	3	4	5	5
	16 in.	2	2	3	3	4	4
	24 in.	1	2	2	2	3	3

1. The header stud shall not be required if the header is supported by a suitable framing anchor.

Uplift connection requirement at points A (top and bottom of header studs) Uplift load per framing member above the header from Table 307F1 or 307A, as appropriate, multiplied by the number of framing members displaced divided by two.

NOTE Uplift connection is required at each end of header and at bottom of header studs in addition to connectors at wall studs and at top and bottom of cripples.



## TIE-DOWN TABLES

<b>HEADERS</b>				
Uplift Force Lbs	Top Connector **	Rating Lbs	Bottom Connector **	Rating Lbs
to 455	LSTA9	725	H3	455
to 910	LSTA12	905	2-H3	910
to 1265	LSTA18	1265	LTT19	1350
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2565
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3700

Total uplift for each truss resting on the header and divide by 2 to determine the uplift force. Use proper bolt anchors sufficient to support required load.

<b>TRUSSES/GIRDERS</b>		
Uplift Force Lbs	Top Connector **	Bottom Connector **
to 500	H2.5	N/A
501-1049	H10	N/A
1050-1350	TS22	LTT19
1351-1750	2-TS22	LTT20
1751-2570	2-TS22	HD2A
2571-3665	3-TS22	HD5A
3666-5260	2-MST148	HTT22
5261-8300	2-MST48	HD10A

Two 12d common toenails are required per truss/rafter per bearing point into plate. Use proper bolt anchors.  
 Strap rafters to truss or at each end with minimum uplift resistance of 450# each end.  
 Strap ridge beam at each end with minimum uplift resistance of 1000#.  
 It is the contractors' responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation.

	Top Connector **	Rating Lbs	Bottom Connector **	Rating
<b>BEAM SEATS</b>	LSTA18*	1200	LTT19*	1250
<b>POSTS</b> (max 17' spacing)	2-LSTA18	2400	ABU44	2300

\*or per truss engineering  
 Use proper bolt anchors  
 All beams to be sheathed or strapped to Double Top Plate when applicable.

**CRIPPLES** | Sheathing nailing alone adequate w/8d nails @ 3" O.C.

<b>STUDS</b>
Wall sheathing nailing Adequate exterior walls bottom w/8d nails.
Use SP1 & SP2 @32" O.C. on all interior non-sheathed bearing walls.
Interior anchor bolts to be 1/2" x 8" A307 or 1/2" x 7" wedge anchor or equivalent.

\*\* Equivalent Simpson hardware, or other manufacturer, may be substituted for any of the hardware specified on this page as long as it meets the required load capacities/uplift resistance.

NOTE:

1. For nailing into SPF members, multiply table values by .86
2. See truss engineering for anchor tie-down values.

# ASCE 7-02

6/13/07

## Wind Load Design per ASCE 7-02

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	33.7	Deg
Type of Roof	Hipped	
Eave Height (Eht)	9.00	ft
Ridge Height (RHt)	21.43	ft
Mean Roof Height (Ht)	15.28	ft
Width Perp. to Wind (B)	79.84	ft
Width Parallel to Wind (L)	77.58	ft
Damping Ratio (beta)	0.01	

*Red values should be changed only through "Main Menu"*

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.20
Flexible Structure	No

Calculated Parameters	
Importance Factor	1
<i>Hurricane Prone Region (V&gt;100 mph)</i>	
<b>Table C6-4 Values</b>	
Alpha =	7.000
Zg =	1200.000
At =	0.143
Bt =	0.840
Am =	0.250
Bm =	0.450
Cc =	0.300
l =	320.00 ft
Epsilon =	0.333
Zmin =	30.00 ft

Gust Factor Category I: Rigid Structures - Simplified Method		
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85
Gust Factor Category II: Rigid Structures - Complete Analysis		
Zm	Zmin	30.00 ft
lzm	$Cc * (33/z)^{0.167}$	0.3048
Lzm	$l * (zm/33)^{Epsilon}$	309.99 ft
Q	$(1/(1+0.63*((B+Ht)/Lzm)^{0.63}))^{0.5}$	0.8773
Gust2	$0.925 * ((1+1.7 * lzm * 3.4 * Q)/(1+1.7 * 3.4 * lzm))$	0.8526
Gust Factor Category III: Flexible or Dynamically Sensitive Structures		
Vhref	$V * (5280/3600)$	161.33 ft/s
Vzm	$bm * (zm/33)^{Am} * Vhref$	70.89 ft/s
NF1	$NatFreq * Lzm / Vzm$	4.37 Hz
Rn	$(7.47 * NF1) / (1 + 10.302 * NF1)^{1.667}$	0.0552
Nh	$4.6 * NatFreq * Ht / Vzm$	0.99
Nb	$4.6 * NatFreq * B / Vzm$	5.18
Nd	$15.4 * NatFreq * Depth / Vzm$	16.85
Rh	$1 / (Nh - (1 / (2 * Nh^2) * (1 - Exp(-2 * Nh))))$	0.5700
Rb	$1 / (Nb - (1 / (2 * Nb^2) * (1 - Exp(-2 * Nb))))$	0.1744
Rd	$1 / (Nd - (1 / (2 * Nd^2) * (1 - Exp(-2 * Nd))))$	0.0576
RR	$((1/Beta) * Rn * Rh * Rb * (0.53 + 0.47 * Rd))^{0.5}$	0.5527
gg	$(2 * LN(3600 * n1))^{0.5} + 0.577 / (2 * LN(3600 * n1))^{0.5}$	4.19
Gust3	$0.925 * ((1 + 1.7 * lzm * (3.4^2 * Q^2 + GG^2 * RR^2)^{0.5}) / (1 + 1.7 * 3.4 * lzm))$	0.99

Gust Factor Summary			
Main Wind-force resisting system:		Components and Cladding:	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.85	Gust Factor (G)	0.85

# ASCE 7-02

6/13/07

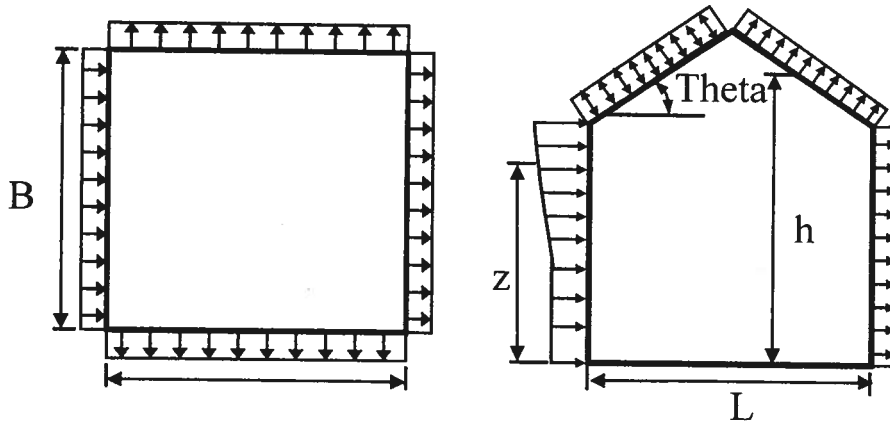
Wind Load Design per ASCE 7-02

## 6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev. ft	Kz	Kzt	Kd	qz lb/ft <sup>2</sup>	Pressure (lb/ft <sup>2</sup> )	
					Windward Wall*	
			1.00		+GCpi	-GCpi
21.43	0.70	1.00	1.00	21.70	11.58	18.02
20	0.70	1.00	1.00	21.70	11.58	18.02
15.28	0.70	1.00	1.00	21.70	11.58	18.02
15	0.70	1.00	1.00	21.70	11.58	18.02

**Figure 6-3 - External Pressure Coefficients, Cp**

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	$2.01 \cdot (Ht/zg)^{2/\alpha}$	0.58	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	$.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot Kh \cdot Kht \cdot Kd$	17.90	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 79.84 ft wall)	-0.50	-10.85	-4.41
Leeward Walls (Wind Dir Parallel to 77.58 ft wall)	-0.49	-10.76	-4.32
Side Walls	-0.70	-13.90	-7.46
Roof - Normal to Ridge (Theta >= 10)			
Windward - Max Negative	-0.05	-4.01	2.43
Windward - Max Positive	0.37	2.49	8.93
Leeward Normal to Ridge	-0.60	-12.38	-5.93
Overhang Top	-0.05	-0.79	-0.79
Overhang Bottom	0.80	0.68	0.68
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 7.64 ft	-0.90	-16.95	-10.51

# ASCE 7-02

6/13/07

## Wind Load Design per ASCE 7-02

Dist from Windward Edge: 7.64 ft to 15.28 ft	-0.90	-16.95	-10.51
Dist from Windward Edge: 15.28 ft to 30.56 ft	-0.50	-10.85	-4.41
Dist from Windward Edge: > 30.56 ft	-0.30	-7.80	-1.36

\* Horizontal distance from windward edge

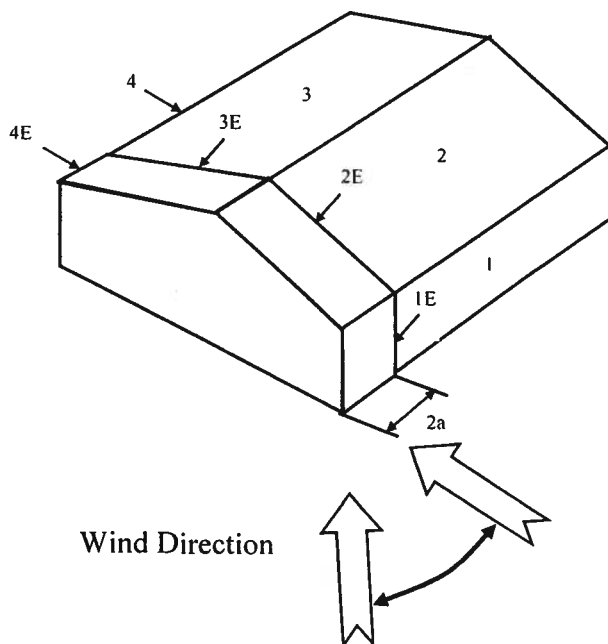
### Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.58
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.90

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.56	0.18	-0.18	21.70	8.25	16.06
2	0.21	0.18	-0.18	21.70	0.65	8.46
3	-0.43	0.18	-0.18	21.70	-13.24	-5.43
4	-0.37	0.18	-0.18	21.70	-11.94	-4.12
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.69	0.18	-0.18	21.70	11.07	18.88
2E	0.27	0.18	-0.18	21.70	1.95	9.77
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

\* p = qh \* (GCpf - GCpi)



# ASCE 7-02

6/13/07

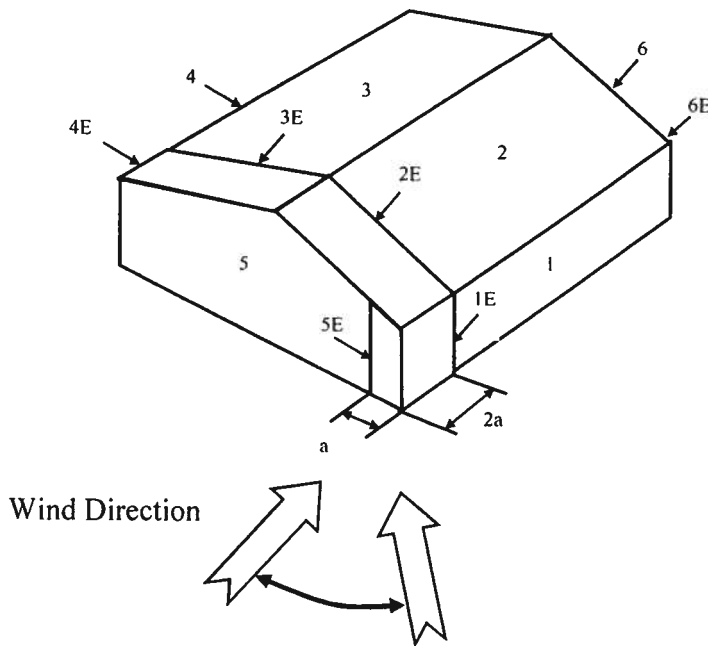
## Wind Load Design per ASCE 7-02

**Figure 6-4 - External Pressure Coefficients, GCpf**  
 Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.58
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.90

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

\* p = qh \* (GCpf - GCpi)





## ASCE 7-02

6/13/07

### Wind Load Design per ASCE 7-02

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
<b>Enclosed Buildings</b>	<b>0.18</b>	<b>-0.18</b>

**Table 6-8 External Pressure Coefficients for Arched Roofs, Cp**

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.31	-18.48	-10.85
	P (-GCpi) - psf	5.13	-12.04	-4.41
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.19	-18.48	-10.85
	P (-GCpi) - psf	3.19	-18.48	-10.85

**Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf**

Variable	Description	Value	
L	Roof dimension normal to wind direction	77.58	ft
B	Roof dimension parallel to wind direction	79.84	ft
L/B	Ratio of L to B	0.972	
Theta	Slope of Roof	33.7	Deg
Cf	Force Coefficient	0.00	
X	Distance to center of pressure from windward edge	0.00	ft



# Residential Window Diversity

## MidSummer

TONY WILLIAMS  
LAKE CITY, FL

Project Title:  
WILLIAMS RESIDENCE

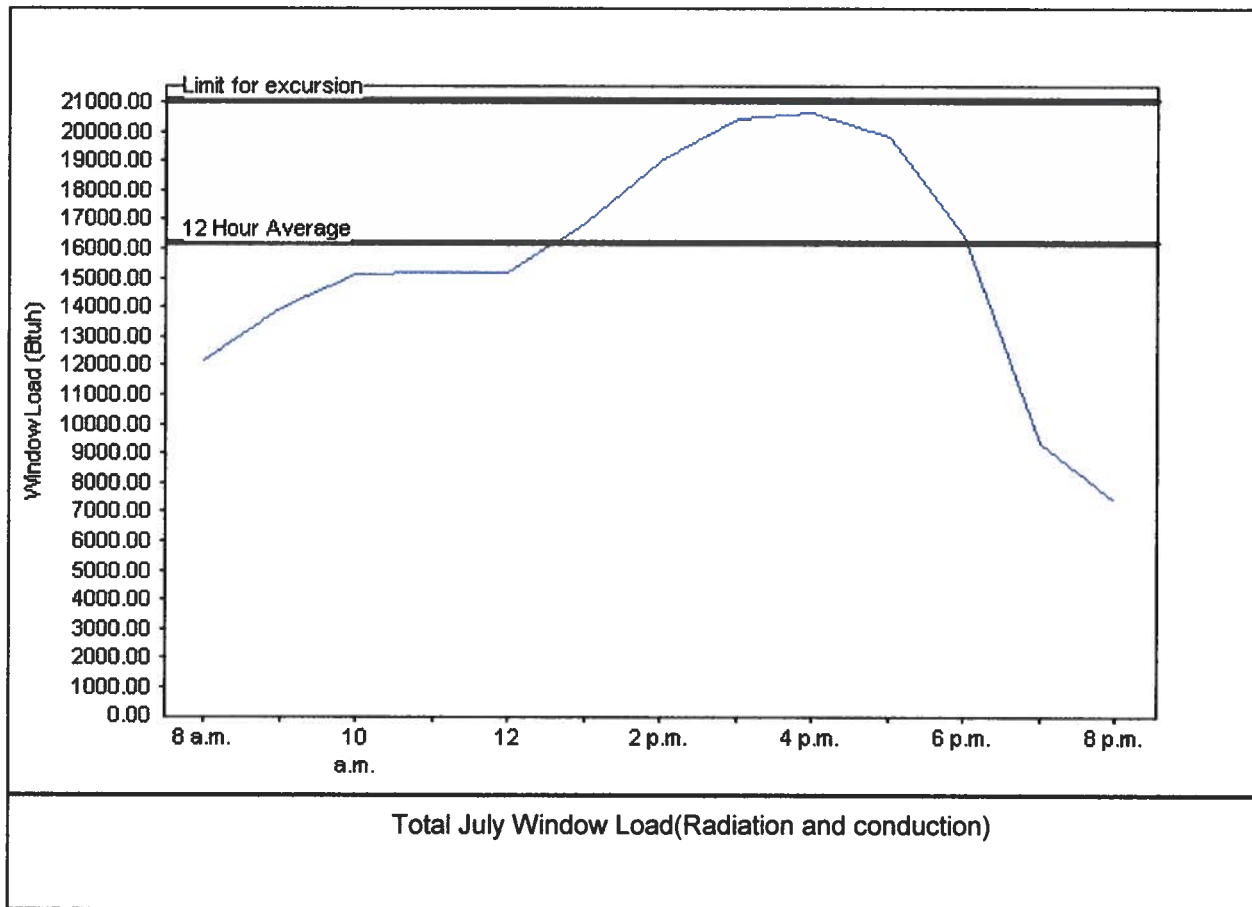
Code Only  
Professional Version  
Climate: North

6/17/2007

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	16189 Btu
Summer setpoint	75 F	Peak window load for July	20686 Btu
Summer temperature difference	17 F	Excursion limit(130% of Ave.)	21045 Btu
Latitude	29 North	Window excursion (July)	None

### WINDOW Average and Peak Loads



The midsummer window load for this house does not exceed the window load excursion limit.  
This house has adequate midsummer window diversity.

EnergyGauge® System Sizing for Florida residences only  
PREPARED BY: Jason Elixson  
DATE: 6-15-07



# System Sizing Calculations - Summer

## Residential Load - Room by Room Component Details

TONY WILLIAMS

Project Title:  
WILLIAMS RESIDENCE

Code Only  
Professional Version  
Climate: North

LAKE CITY, FL

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F  
This calculation is for Worst Case. The house has been rotated 180 degrees.

6/17/2007

### Component Loads for Zone #1: Main

Window	Type*	Ornt	Overhang		Window Area(sqft)			HTM		Load
			Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded	
1	2, Clear, 0.87, B-M, N,H	S	0ft.	0ft.	80.0	0.0	80.0	19	23	1874 Btuh
2	2, Clear, 0.87, None,N,N	S	0ft.	0ft.	32.0	0.0	32.0	29	34	1076 Btuh
3	2, Clear, 0.87, B-M, N,H	W	0ft.	0ft.	80.0	0.0	80.0	19	52	4185 Btuh
4	2, Clear, 0.87, None,N,N	W	0ft.	0ft.	32.0	0.0	32.0	29	80	2545 Btuh
5	2, Clear, 0.87, B-M, N,H	N	0ft.	0ft.	12.5	0.0	12.5	19	19	243 Btuh
6	2, Clear, 0.87, B-M, N,H	E	0ft.	0ft.	60.0	0.0	60.0	19	52	3139 Btuh
7	2, Clear, 0.87, None,N,N	E	0ft.	0ft.	24.0	0.0	24.0	29	80	1908 Btuh
Window Total					321 (sqft)					14969 Btuh
<b>Walls</b>	Type		R-Value/U-Value		Area(sqft)		HTM		Load	
1	Frame - Wood - Ext		13.0/0.09		1717.5		2.1		3582 Btuh	
Wall Total					1718 (sqft)				3582 Btuh	
<b>Doors</b>	Type				Area (sqft)		HTM		Load	
1	Insulated - Exterior				60.0		9.8		588 Btuh	
2	Insulated - Exterior				80.0		9.8		784 Btuh	
Door Total					140 (sqft)				1372 Btuh	
<b>Ceilings</b>	Type/Color/Surface		R-Value		Area(sqft)		HTM		Load	
1	Vented Attic/DarkShingle		30.0		2640.0		1.7		4372 Btuh	
Ceiling Total					2640 (sqft)				4372 Btuh	
<b>Floors</b>	Type		R-Value		Size		HTM		Load	
1	Stem Wall with Stem Wall Insul		3.5		2640 (sqft)		0.9		2492 Btuh	
Floor Total					2640.0 (sqft)				2492 Btuh	
Zone Envelope Subtotal:									26787 Btuh	
<b>Infiltration</b>	Type		ACH		Volume(cuft) wall area(sqft)		CFM=		Load	
	SensibleNatural		0.16		23760 1718		0.0		0 Btuh	
<b>Internal gain</b>			Occupants		Btuh/occupant		Appliance		Load	
			0		X 230 +		2400		2400 Btuh	
Sensible Envelope Load:									29187 Btuh	
<b>Duct load</b>	Average sealed, Supply(R6.0-Cond.), Return(R6.0-Cond.) (DGM of 0.000)									0 Btuh
<b>Sensible Zone Load</b>									<b>29187 Btuh</b>	

# Manual J Summer Calculations

## Residential Load - Component Details (continued)

TONY WILLIAMS  
LAKE CITY, FL

Project Title:  
WILLIAMS RESIDENCE

Code Only  
Professional Version  
Climate: North

6/17/2007

### WHOLE HOUSE TOTALS

<b>Whole House Totals for Cooling</b>	<b>Sensible Envelope Load All Zones</b>	<b>30366 Btuh</b>
	Sensible Duct Load	0 Btuh
	<b>Total Sensible Zone Loads</b>	<b>30366 Btuh</b>
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	<b>Total sensible gain</b>	<b>30366 Btuh</b>
	Latent infiltration gain (for 54 gr. humidity difference)	2316 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (0 people @ 200 Btuh per person)	0 Btuh
	Latent other gain	0 Btuh
	<b>Latent total gain</b>	<b>2316 Btuh</b>
	<b>TOTAL GAIN</b>	<b>32682 Btuh</b>

### EQUIPMENT

1. Central Unit	#	36000 Btuh
-----------------	---	------------

\*Key: Window types (Pn - Number of panes of glass)  
 (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)  
 (U - Window U-Factor or 'DEF' for default)  
 (InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))  
 (ExSh - Exterior shading device: none(N) or numerical value)  
 (BS - Insect screen: none(N), Full(F) or Half(H))  
 (Ornt - compass orientation)



Version 8  
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# System Sizing Calculations - Summer

## Residential Load - Whole House Component Details

TONY WILLIAMS

Project Title:  
WILLIAMS RESIDENCE

Code Only  
Professional Version  
Climate: North

LAKE CITY, FL

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F  
This calculation is for Worst Case. The house has been rotated 180 degrees.

6/17/2007

### Component Loads for Whole House

Window	Type*		Overhang		Window Area(sqft)			HTM		Load	
	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, B-M, N,H	S	0ft.	0ft.	80.0	0.0	80.0	19	23	1874	Btuh
2	2, Clear, 0.87, None,N,N	S	0ft.	0ft.	32.0	0.0	32.0	29	34	1076	Btuh
3	2, Clear, 0.87, B-M, N,H	W	0ft.	0ft.	80.0	0.0	80.0	19	52	4185	Btuh
4	2, Clear, 0.87, None,N,N	W	0ft.	0ft.	32.0	0.0	32.0	29	80	2545	Btuh
5	2, Clear, 0.87, B-M, N,H	N	0ft.	0ft.	12.5	0.0	12.5	19	19	243	Btuh
6	2, Clear, 0.87, B-M, N,H	E	0ft.	0ft.	60.0	0.0	60.0	19	52	3139	Btuh
7	2, Clear, 0.87, None,N,N	E	0ft.	0ft.	24.0	0.0	24.0	29	80	1908	Btuh
<b>Window Total</b>					<b>321 (sqft)</b>					<b>14969 Btuh</b>	
<b>Walls</b>	<b>Type</b>		<b>R-Value/U-Value</b>		<b>Area(sqft)</b>		<b>HTM</b>		<b>Load</b>		
1	Frame - Wood - Ext		13.0/0.09		1717.5		2.1		3582 Btuh		
<b>Wall Total</b>					<b>1718 (sqft)</b>				<b>3582 Btuh</b>		
<b>Doors</b>	<b>Type</b>				<b>Area (sqft)</b>		<b>HTM</b>		<b>Load</b>		
1	Insulated - Exterior				60.0		9.8		588 Btuh		
2	Insulated - Exterior				80.0		9.8		784 Btuh		
<b>Door Total</b>					<b>140 (sqft)</b>				<b>1372 Btuh</b>		
<b>Ceilings</b>	<b>Type/Color/Surface</b>		<b>R-Value</b>		<b>Area(sqft)</b>		<b>HTM</b>		<b>Load</b>		
1	Vented Attic/DarkShingle		30.0		2640.0		1.7		4372 Btuh		
<b>Ceiling Total</b>					<b>2640 (sqft)</b>				<b>4372 Btuh</b>		
<b>Floors</b>	<b>Type</b>		<b>R-Value</b>		<b>Size</b>		<b>HTM</b>		<b>Load</b>		
1	Stem Wall with Stem Wall Insul		3.5		2640 (sqft)		0.9		2492 Btuh		
<b>Floor Total</b>					<b>2640.0 (sqft)</b>				<b>2492 Btuh</b>		
<b>Envelope Subtotal:</b>									<b>26787 Btuh</b>		
<b>Infiltration</b>	<b>Type</b>		<b>ACH</b>	<b>Volume(cuft)</b>	<b>wall area(sqft)</b>	<b>CFM=</b>		<b>Load</b>			
	SensibleNatural		0.16	23760	1718	0.0		1179 Btuh			
<b>Internal gain</b>			<b>Occupants</b>	<b>Btuh/occupant</b>	<b>Appliance</b>		<b>Load</b>				
			0	X 230	+ 2400		2400 Btuh				
<b>Sensible Envelope Load:</b>									<b>30366 Btuh</b>		
<b>Duct load</b>	(DGM of 0.000)									0 Btuh	
<b>Sensible Load All Zones</b>									<b>30366 Btuh</b>		

# Manual J Summer Calculations

## Residential Load - Component Details (continued)

TONY WILLIAMS  
LAKE CITY, FL

Project Title:  
WILLIAMS RESIDENCE

Code Only  
Professional Version  
Climate: North

6/17/2007

### WHOLE HOUSE TOTALS

<b>Whole House Totals for Cooling</b>	<b>Sensible Envelope Load All Zones</b>	<b>30366 Btuh</b>
	Sensible Duct Load	0 Btuh
	<b>Total Sensible Zone Loads</b>	<b>30366 Btuh</b>
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	<b>Total sensible gain</b>	<b>30366 Btuh</b>
	Latent infiltration gain (for 54 gr. humidity difference)	2316 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (0 people @ 200 Btuh per person)	0 Btuh
	Latent other gain	0 Btuh
	<b>Latent total gain</b>	<b>2316 Btuh</b>
	<b>TOTAL GAIN</b>	<b>32682 Btuh</b>

### EQUIPMENT

1. Central Unit	#	36000 Btuh
-----------------	---	------------

\*Key: Window types (Pn - Number of panes of glass)  
 (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)  
 (U - Window U-Factor or 'DEF' for default)  
 (InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))  
 (ExSh - Exterior shading device: none(N) or numerical value)  
 (BS - Insect screen: none(N), Full(F) or Half(H))  
 (Omt - compass orientation)



Version 8  
For Florida residences only

# System Sizing Calculations - Winter

## Residential Load - Room by Room Component Details

TONY WILLIAMS

Project Title:  
WILLIAMS RESIDENCE

Code Only  
Professional Version  
Climate: North

LAKE CITY, FL

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F  
This calculation is for Worst Case. The house has been rotated 180 degrees.

6/17/2007

### Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft)	X	HTM=	Load
1	2, Clear, Metal, 0.87	S	80.0		32.2	2575 Btuh
2	2, Clear, Metal, 0.87	S	32.0		32.2	1030 Btuh
3	2, Clear, Metal, 0.87	W	80.0		32.2	2575 Btuh
4	2, Clear, Metal, 0.87	W	32.0		32.2	1030 Btuh
5	2, Clear, Metal, 0.87	N	12.5		32.2	402 Btuh
6	2, Clear, Metal, 0.87	E	60.0		32.2	1931 Btuh
7	2, Clear, Metal, 0.87	E	24.0		32.2	773 Btuh
	Window Total		321(sqft)			10317 Btuh
Walls	Type	R-Value	Area	X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1718		3.3	5640 Btuh
	Wall Total		1718			5640 Btuh
Doors	Type	R-Value	Area	X	HTM=	Load
1	Insulated - Exterior		60		12.9	777 Btuh
2	Insulated - Exterior		80		12.9	1036 Btuh
	Door Total		140			1813Btuh
Ceilings	Type/Color/Surface	R-Value	Area	X	HTM=	Load
1	Vented Attic/D/Shin	30.0	2640		1.2	3111 Btuh
	Ceiling Total		2640			3111Btuh
Floors	Type	R-Value	Size	X	HTM=	Load
1	Stem Wall with Stem Wall I	3.5	2640.0	sqft	2.0	5325 Btuh
	Floor Total		2640			5325 Btuh
Zone Envelope Subtotal:						26206 Btuh
Infiltration	Type	ACH	X	Volume(cuft)	walls(sqft)	CFM=
	Natural	0.32		23760	1718	0.0
						0 Btuh
Ductload	Average sealed, Supply(R6.0-Cond.), Return(R6.0-Cond)\DLM of 0.000)					0 Btuh
Zone #1	Sensible Zone Subtotal					26206 Btuh

# Manual J Winter Calculations

## Residential Load - Component Details (continued)

TONY WILLIAMS  
LAKE CITY, FL

Project Title:  
WILLIAMS RESIDENCE

Code Only  
Professional Version  
Climate: North

6/17/2007

### WHOLE HOUSE TOTALS

	Subtotal Sensible	31339 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	31339 Btuh

### EQUIPMENT

1. Electric Heat Pump	#	36000 Btuh
-----------------------	---	------------

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)  
(Frame types - metal, wood or insulated metal)  
(U - Window U-Factor or 'DEF' for default)  
(HTM - ManualJ Heat Transfer Multiplier)  
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types )



Version 8  
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# System Sizing Calculations - Winter

## Residential Load - Whole House Component Details

TONY WILLIAMS  
LAKE CITY, FL

Project Title:  
WILLIAMS RESIDENCE

Code Only  
Professional Version  
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F  
This calculation is for Worst Case. The house has been rotated 180 degrees.

6/17/2007

### Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	S	80.0	32.2	2575 Btuh
2	2, Clear, Metal, 0.87	S	32.0	32.2	1030 Btuh
3	2, Clear, Metal, 0.87	W	80.0	32.2	2575 Btuh
4	2, Clear, Metal, 0.87	W	32.0	32.2	1030 Btuh
5	2, Clear, Metal, 0.87	N	12.5	32.2	402 Btuh
6	2, Clear, Metal, 0.87	E	60.0	32.2	1931 Btuh
7	2, Clear, Metal, 0.87	E	24.0	32.2	773 Btuh
Window Total			321(sqft)		10317 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1718	3.3	5640 Btuh
Wall Total			1718		5640 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exterior		60	12.9	777 Btuh
2	Insulated - Exterior		80	12.9	1036 Btuh
Door Total			140		1813Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	2640	1.2	3111 Btuh
Ceiling Total			2640		3111Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Stem Wall with Stem Wall I	3.5	2640.0 sqft	2.0	5325 Btuh
Floor Total			2640		5325 Btuh
Envelope Subtotal:					26206 Btuh
Infiltration	Type	ACH X Volume(cuft)	walls(sqft)	CFM=	Load
	Natural	0.32 23760	1718	0.0	5133 Btuh
Ductload	(DLM of 0.000)				0 Btuh
All Zones	Sensible Subtotal All Zones				31339 Btuh

### WHOLE HOUSE TOTALS

	Subtotal Sensible	31339 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	31339 Btuh



# Manual J Winter Calculations

## Residential Load - Component Details (continued)

TONY WILLIAMS  
LAKE CITY, FL

Project Title:  
WILLIAMS RESIDENCE

Code Only  
Professional Version  
Climate: North

6/17/2007

### EQUIPMENT

1. Electric Heat Pump	#	36000 Btuh
-----------------------	---	------------

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)  
(Frame types - metal, wood or insulated metal)  
(U - Window U-Factor or 'DEF' for default)  
(HTM - ManualJ Heat Transfer Multiplier)  
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types )



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**Test Results: (Continued)**


<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
<u>Optional Performance</u>			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 35.3 psf (positive)	0.46"*	0.41" max
	@ 47.2 psf (negative)	0.67"*	0.41" max

*\*Exceeds L/175 for deflection, but meets all other test requirements.*


	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 53.0 psf (positive)	0.03"	0.29" max
	@ 52.5 psf (negative)	0.02"	0.29" max

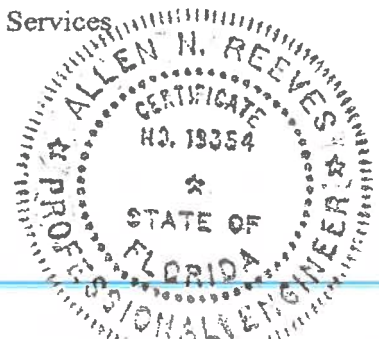
Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC

  
Mark A. Hess  
Technician

MAH:nlb  
01-41641.01

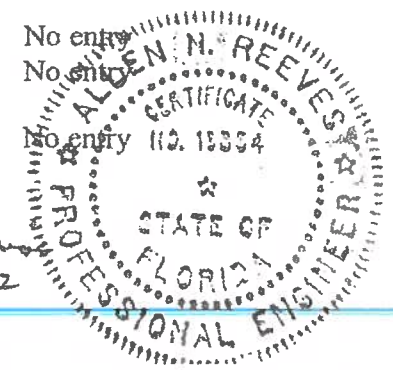
  
Allen N. Reeves, P.E.  
Director - Engineering Services  
7 JUNE 2002



Test Results: (Continued)

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds) @ 15.0 psf (positive) @ 15.0 psf (negative)	0.15" 0.29"	0.41" max. 0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds) @ 22.5 psf (positive) @ 22.5 psf (negative)	0.01" 0.01"	0.29" max. 0.29" max.
2.2 .6.2	Deglazing Test (ASTM E 987-88) In operating direction at 70 lbs Right sash, meeting rail Right sash, bottom rail Middle sash, meeting rail Middle sash, bottom rail Left sash, meeting rail Left sash, bottom rail  In remaining direction at 50 lbs Right sash, right stile Right sash, left stile Middle sash, right stile Middle sash, left stile Left sash, right stile Left sash, left stile	0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25%  0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12%	0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100%  0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100%
2 .8	Forced Entry Resistance (ASTM F 588-97)  Type: A Grade: 10  Lock Manipulation Test  Test A1 through A5 Test A7  Lock Manipulation Test	No entry  No entry No entry  No entry	No entry  No entry No entry  No entry

*Allen N. Reeves*  
7 JUNE 2002



**Test Specimen Description: (Continued)**

**Hardware:**

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Metal cam lock with keeper	1	Midspan of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

**Drainage:** Sloped sill

**Reinforcement:** No reinforcement was utilized

**Installation:** The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

**Test Results:**

The results are tabulated as follows

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft <sup>2</sup>	0.3 cfm/ft <sup>2</sup> max

*Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

Water Resistance (ASTM E 547-00)  
(with and without screen)  
WTP = 2.86 psf

No leakage

No leakage

*Allen N. Reeves*  
7 JUNE 2002



**Test Specimen Description: (Continued)**

**Glazing Details:** The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

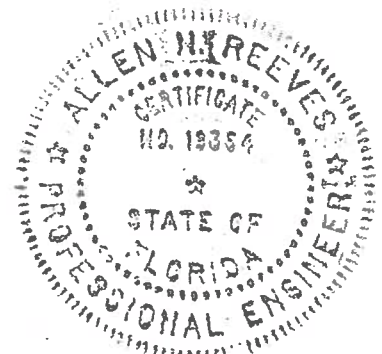
**Weatherstripping:**

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.230" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

**Frame Construction:** The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

**Sash Construction:** The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

**Screen Construction:** The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.



*Allen H. Reeves*  
7 JUNE 2002

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.  
P.O. Box 370  
650 West Market Street  
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01  
Test Date: 05/13/02  
And: 05/16/02  
Report Date: 06/05/02  
Expiration Date: 05/16/06

**Project Summary:** Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

**Test Specification:** The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

**Test Specimen Description:**

**Series/Model:** 650

**Type:** Aluminum Triple Single Hung Window

**Overall Size:** 9' 3-1/2" wide by 5' 11-11/16" high

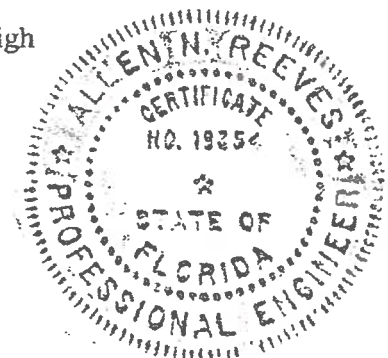
**Active Sash Size (3):** 3' 0-1/4" wide by 2' 10-3/4" high

**Fixed Daylight Opening Size (3):** 2' 8-1/4" wide by 2' 9-1/8" high

**Screen Size (3):** 2' 9-1/8" wide by 2' 11" high

**Finish:** All aluminum was painted white.

130 Derry Court  
York, PA 17402-9405  
phone: 717.764.7700  
fax: 717.764.4129  
www.archtest.com



Allen N. Reeves  
7 JUNE 2002



**AAMA/NWWDA 101/I.S.2-97  
TEST REPORT**

**Rendered to:**

**MI HOME PRODUCTS, INC.**

**SERIES/MODEL: 650**

**TYPE: Aluminum Triple Single Hung Window**

<b>Title of Test</b>	<b>Summary of Results</b>
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft <sup>2</sup>
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-41641.01 for complete description and data.



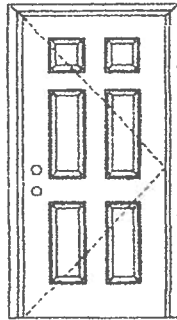
X

Opaque Inswing Unit

COP-WL-JH4101-02

# WOOD-EDGE STEEL DOORS

## APPROVED ARRANGEMENT:



Test Data Review Certificate #3026447A and COP/TEST Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.itswh.com](http://www.itswh.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**Note:**  
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door  
Maximum unit size - 3'0" x 6'8"

**Design Pressure**  
**+66.0/-66.0**  
limited water unless special threshold design is used.

**Large Missile Impact Resistance**  
**Hurricane protective system (shutters) is NOT REQUIRED.**

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national state or local building codes specify the edition required.

## MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0001-02.

## MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02.

## APPROVED DOOR STYLES:



Flush



Arch Top 3-panel



3-panel



6-panel



New England 4-panel



Eyebrow 4-panel



8-panel



9-panel



15-panel



5-panel



5-panel with scroll



Eyebrow 5-panel



Eyebrow 5-panel with scroll



X

Opaque Inswing Unit

COP-WL-JH4101-02

# WOOD-EDGE STEEL DOORS

## CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

## PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH  
 MIAMI-DADE BCCO  
 PA201, PA202 & PA203

**COMPANY NAME**  
 CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

*Kurt L Balthaz*

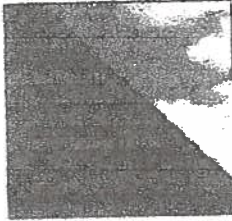
State of Florida, Professional Engineer  
 Kurt Balthazor, P.E. - License Number 56533



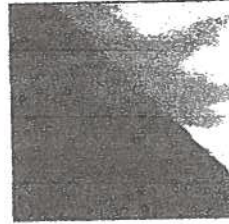
Test Data Review Certificate #3026447A and COP/test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.elsemko.com](http://www.elsemko.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.



# ELK



**PRESTIQUE®  
HIGH DEFINITION®**



**RAISED PROFILE®**

### Prestique Plus High Definition and Prestique Gallery Collection\*

Product size \_\_\_\_\_ 13 1/4" x 39 3/4"  
Exposure \_\_\_\_\_ 5 1/2"  
Pieces/Bundle \_\_\_\_\_ 16  
Bundles/Square \_\_\_\_\_ 4/98.5 sq.ft.  
Squares/Pallet \_\_\_\_\_ 11

50-year limited warranty period:  
5-7\*\* years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability\*. 5-year limited wind warranty\*. Wind Coverage: standard 80 mph, extended 90 mph\*\*\*

### Raised Profile

Product size \_\_\_\_\_ 13 1/4" x 38 1/2"  
Exposure \_\_\_\_\_ 5 1/2"  
Pieces/Bundle \_\_\_\_\_ 22  
Bundles/Square \_\_\_\_\_ 3/100 sq.ft.  
Squares/Pallet \_\_\_\_\_ 16

30-year limited warranty period:  
5-7\*\* years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability\*. 5-year limited wind warranty\*. Wind Coverage: standard 70 mph.

### Prestique High Definition

Product size \_\_\_\_\_ 13 1/4" x 39 3/4"  
Exposure \_\_\_\_\_ 5 1/2"  
Pieces/Bundle \_\_\_\_\_ 16  
Bundles/Square \_\_\_\_\_ 4/98.5 sq.ft.  
Squares/Pallet \_\_\_\_\_ 14

40-year limited warranty period:  
5-7\*\* years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability\*. 5-year limited wind warranty\*. Wind Coverage: standard 80 mph, extended 90 mph\*\*\*

### HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/FLX®  
Size: 12" x 12"  
Exposure: 6 1/2"  
Pieces/Bundle: 45  
Coverage: 4 Bundles =  
100 linear feet

Vented RidgeCrest® w/FLX®  
Size: 13" x 13"  
Exposure: 9 1/2"  
Pieces/Box: 26  
Coverage: 5 boxes =  
100 linear feet

### Prestique High Definition

Product size \_\_\_\_\_ 13 1/4" x 38 1/2"  
Exposure \_\_\_\_\_ 5 1/2"  
Pieces/Bundle \_\_\_\_\_ 22  
Bundles/Square \_\_\_\_\_ 3/100 sq.ft.  
Squares/Pallet \_\_\_\_\_ 16

30-year limited warranty period:  
5-7\*\* years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability\*. 5-year limited wind warranty\*. Wind Coverage: standard 80 mph.

### Elk Starter Strip

52 Bundles/Pallet  
18 Pallets/Truck  
936 Bundles/Truck  
19 Pieces/Bundle  
1 Bundle = 120.33 linear feet

Available Colors (Check Availability): Antique Stone, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood, Forest Green, Wedgewood, Birchwood, Sandalwood  
Gallery Collection: Balsam Forest®, Weathered Sage®, Sierra Sunset®

All Prestique, Raised Profile and Seal-A-Ridge, and Prestique Starter Strip roofing products contain sealant which activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard™ treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae.

All Prestique and Raised Profile shingles meet UL Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790) and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 109 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles have approval from the Florida Building Code Commission, Metro-Dade County, ICBD, and Texas Department of Insurance.

\*See actual limited warranty for conditions and limitations.

\*\*Effective January 1, 2004, the seven year non-prorated Underlayment Coverage Period applies only when a full Elk Roof System is installed with the original installation of the Elk shingles, all in accordance with Elk's application instructions for such products. A full Elk roof system includes Elk Hip and Ridge shingles on all hips and ridges, Elk Starter Strip along all rake and eave edges, an Elk vented cap system, and Elk All-Climate Self-Adhering Underlayment to all valleys. Additionally, Elk All-Climate Self-Adhering Underlayment is required along the rake and eave edges of the roof in and north of the states of VA, KY, MD, MS, CO, UT, NV, & OR.

\*\*\*For a limited Wind Warranty up to 110 mph for Prestique Gallery Collection, Prestique Plus, or 90 mph for Prestique I or Grand, at least six (6) properly placed NAILS and Elk Starter Strip shingles are required. See application instructions printed on the shingle wrapper for additional requirements.

## SPECIFICATIONS

**SCOPE:** Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula FLX.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

**PREPARATION OF ROOF DECK:** Roof deck to be dry, well-seasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/8" (9.525mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard; or chipboard. Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other classes.

**Materials:** Underlayment for standard roof slopes, 4 per foot (101.6/304.8mm) or greater, apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For low slopes (4 per foot (101.6/304.8mm) to a minimum of 2 per foot (50.8/304.8mm)), use two plies of underlayment overlapped a minimum of 19". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

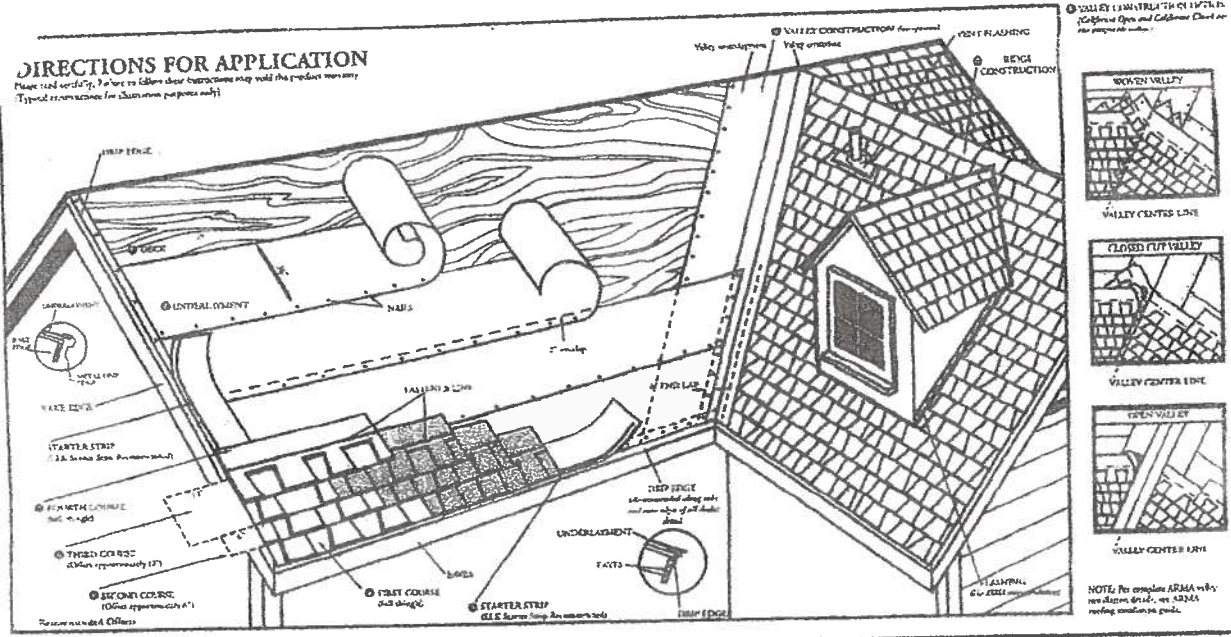
For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula FLX with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements. In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

For specifications in CSI format, call 800.354.SPEC (7737) or e-mail [specinfo@elkcorp.com](mailto:specinfo@elkcorp.com).

## DIRECTIONS FOR APPLICATION

Please read carefully. Failure to follow these instructions may void the product warranty.  
Typical recommendations for illustration purposes only.



### DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All staves should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

#### 1 DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

#### 2 UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt, Elk Vetsashield® or self-adhesive underlayment is also acceptable. Cover drip edge at eaves only.

For low slope (2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 18". Begin by fastening a 18" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

#### EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Technical Services Department for application specifications over other decks and other slopes.

#### 3 STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR THE HEADLAP OF A STRIP SHINGLE WITH THE ADHESIVE STRIP POSITIONED AT THE EAVE EDGE. With at least 3" trimmed from the end of the first shingle, start at the rake edge overhanging the eave and rake edges 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

#### 4 FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof.

#### 5 SECOND COURSE

Offset the second course of shingles with respect to the first by approximately 6". Other offsets are approved if greater than 4".

#### 6 THIRD COURSE

Offset the next course by 6" with respect to the second course, or consistent with the original offset.

#### 7 FOURTH COURSE

Start at the rake and continue with full shingles across roof.

#### FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rock shingles straight up the roof. Offsets may be adjusted around valleys and penetrations.

#### 8 VALLEY CONSTRUCTION

Open, wove and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

#### 9 RIDGE CONSTRUCTION

For ridge construction Elk recommends Class "A" Z-Ridge or Seal-A-Ridge® with formula FLX™ or RidgeCrest™ with FLX (See ridge package for installation instructions). Vented RidgeCrest or 3-tab shingles are also approved.

#### FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Using the fastener line as a reference, nail or staple the shingle in the double thickness common head area. For shingles without a fastener line, nails or staples must be placed between and/or in the sealant dots.

**NAILS:** Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for re-roofs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

**STAPLES:** Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less. This product meets the requirements of the IRC 2003 code when fastened with 4 nails.

#### MANSARD APPLICATIONS

Correct fastening is critical to the performance of the roof. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

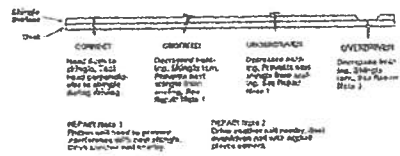
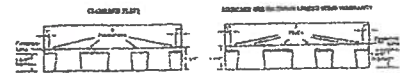
#### LIMITED WIND WARRANTY

For a Limited Wind Warranty, all Prestique and Raised Profile™ shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.

For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 90 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4" of an inch.

#### HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the fastener line or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.



Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified. All Prestique and Raised Profile shingles have a U.L.® Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

**CAUTION TO WHOLESALE:** Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.



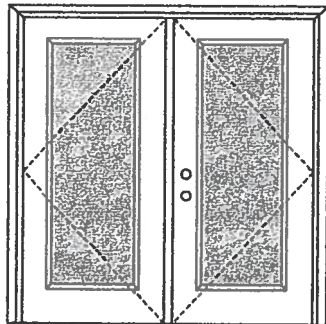
**XX**

Glazed Outswing Unit

COP-WL-JH4162-02

# WOOD-EDGE STEEL DOORS

## APPROVED ARRANGEMENT:



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.itssemko.com](http://www.itssemko.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**Note:**  
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

**Double Door**  
Maximum unit size = 6'0" x 6'8"

**Design Pressure**  
**+40.5/-40.5**  
Limited water unless special threshold design is used.

**Large Missile Impact Resistance**  
**Hurricane protective system (shutters) is REQUIRED.**

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

## MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

## MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

## APPROVED DOOR STYLES:

### 1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

### 1/2 GLASS:



105 Series\*



106, 160 Series\*



129 Series\*



200 Series\*



12 R/L, 23 R/L, 24 R/L Series\*



107 Series\*



108 Series



304 Series

\*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

**XX**

Glazed Outswing Unit

COP-WL-JH4162-02

# WOOD-EDGE STEEL DOORS

**APPROVED DOOR STYLES:**

**3/4 GLASS:**



404 Series

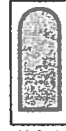


410 Series



450 Series

**FULL GLASS:**



109 Series



114, 120, 122 Series



152 Series



149 Series



300 Series

**CERTIFIED TEST REPORTS:**

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1864-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

**PRODUCT COMPLIANCE LABELING:**

TESTED IN  
ACCORDANCE WITH  
MIAMI-DADE BCCO PA202  
  
COMPANY NAME  
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer  
Kurt Balthazor, P.E. – License Number 56533



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/AWH website (www.etsmko.com), the Masonite website (www.masonite.com) or the Masonite technical center.



June 17, 2002  
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



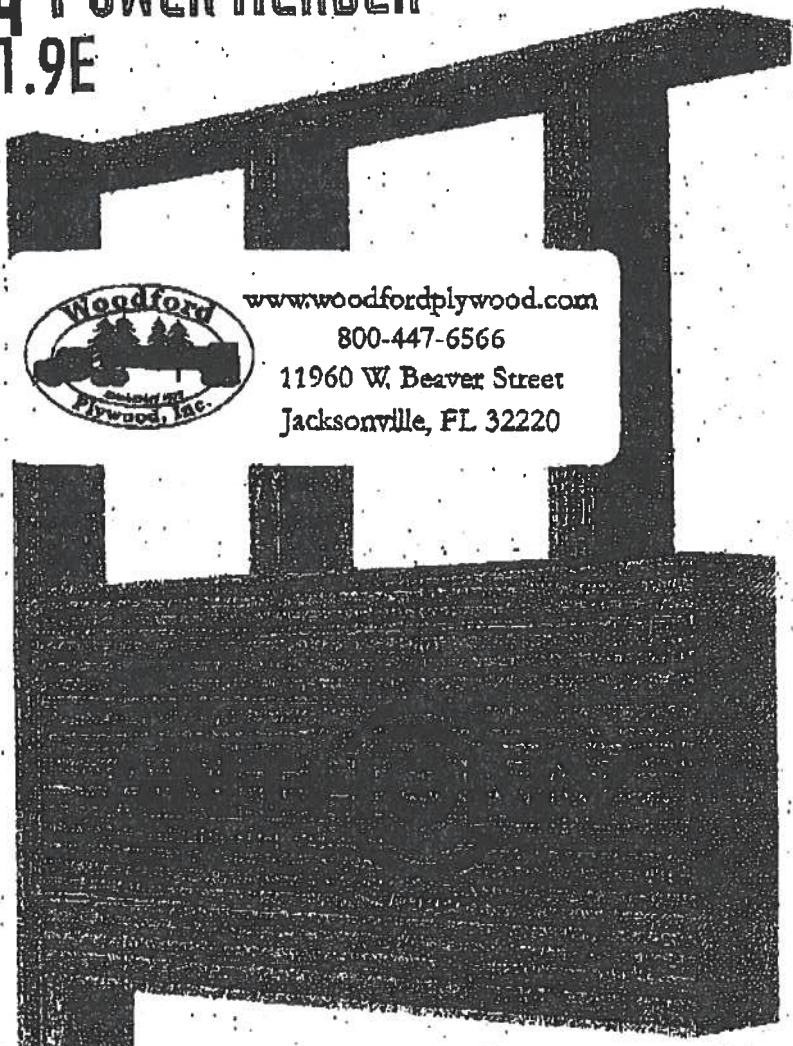
Exclusively from

Masonite International Corporation



# Anthony POWER HEADER®

## 2600F<sub>b</sub> - 1.9E



www.woodfordplywood.com  
800-447-6566  
11960 W. Beaver Street  
Jacksonville, FL 32220

### Anthony POWER HEADER® Advantages

- ◆ Less Expensive than LVL or PSL
- ◆ Lighter than Steel, LVL or PSL
- ◆ Pre-Cut Lengths
- ◆ Renewable Resource
- ◆ Cambered or Non-cambered
- ◆ 3-1/2" Width to Match Framing
- ◆ One Piece - No Nail Laminating
- ◆ Lifetime Warranty

## Garage Header Sizing Tables

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# Anthony POWER HEADER®

# 26Fb - 1.9E

## ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS ( $F_b$ )= 2600  
 COMPRESSION PERP. TO GRAIN ( $F_{c\perp}$ )= 740  
 HORIZONTAL SHEAR ( $F_v$ )= 225  
 MODULUS OF ELASTICITY (MOE)=  $1.9 \times 10^6$

Span (ft)	7.7	9.0	10.4	11.7	12.9	14.2	15.5
Weight (lb/ft)	326	514	789	1115	1521	2014	2504
Moment Capacity (ft-k)	8865	12015	15996	20145	24772	29877	34660
Reaction (lb)	3908	4550	5250	5892	6533	7175	7317

### NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress,  $F_b$ , shall be modified by the Volume Factor,  $C_v$ , as outlined in AITC 117 - Design 2001 and the NDS for Wood Construction 2002.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for POWER HEADER®.

### GARAGE HEADER COMPARISONS

Weight (lb/ft)	810 / 540	990 / 720	640 / 400	765 / 510	750 / 480	900 / 600
Section	3-1/2" x 8-3/8"	3-1/2" x 9-3/4"	3-1/2" x 12-5/8"	3-1/2" x 14"	3-1/2" x 15-3/8"	3-1/2" x 16-3/4"
Section	3-1/2" x 9-5/8"	3-1/2" x 9-5/8"	3-1/2" x 13-3/4"	3-1/2" x 15-1/8"	3-1/2" x 16-1/2"	3-1/2" x 17-7/8"
Section	3-1/2" x 9"	3-1/2" x 10-1/2"	3-1/2" x 13-1/2"	3-1/2" x 15"	3-1/2" x 16-1/2"	3-1/2" x 18"
Section	3-1/2" x 9-1/4"	3-1/2" x 9-1/4"	3-1/2" x 14"	3-1/2" x 14"	3-1/2" x 16"	3-1/2" x 16"
Section	3-1/2" x 11-1/4"	3-1/2" x 11-1/4"	3-1/2" x 14"	3-1/2" x 16"	3-1/2" x 18"	---

For more information on POWER HEADER®,  
 or other laminated structural products from  
 Anthony Forest Products Company please call  
 1-800-221-2326 or FAX at 870-862-6502.

Distributed by:

Power Header® is a trademark of  
**Anthony Forest Products Company**  
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 Internet address: <http://www.anthonystore.com>  
 e-mail: [info@anthonystore.com](mailto:info@anthonystore.com)  
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# Anthony POWER HEADER®

25Fb - 1.9E

## 3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

Header Size	Roof Only										Deflection Limit	
	125% Non-Snow	115% Snow	100% Snow		100% Snow		100% Snow		100% Snow			
8x8	844	896	1216		1573		1930		2287		1/8"	
8x10	161	207	254	330	390	510	552	669	752	824	1/8"	
8x12	114	145	180	231	277	359	391	510	534	653	707	789

Header Size	Roof, Wall, and Floor										Deflection Limit
	125% Non-Snow	115% Snow	100% Snow		100% Snow		100% Snow		100% Snow		
8x8	844	975	1322		1679		2036		2393		1/8"
8x10	161	207	254	330	390	510	552	724	752	897	1/8"
8x12	114	145	180	231	277	359	391	510	534	699	693

Header Size	Roof, Wall, and Floor										Deflection Limit			
	125% Non-Snow	115% Snow	100% Snow		100% Snow		100% Snow		100% Snow					
8x8	562	778	888	1056	1363	1367	1582		1797		1/8"			
8x10	107	153	169	245	260	380	368	540	501	715	664	864	840	
8x12	76	107	120	171	185	267	261	380	356	521	471	664	609	813

### NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded and outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

### GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load and live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

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# Anthony POWER HEADER®

## 26Fb - 1.9E

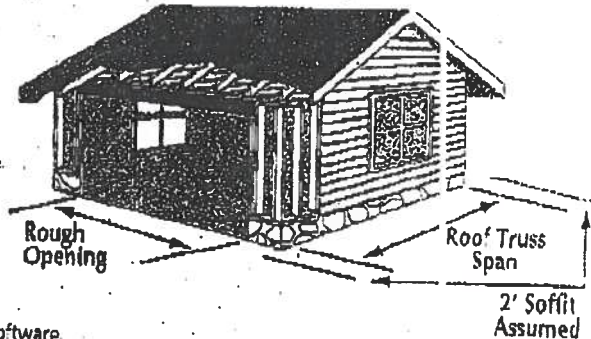
**3-1/2" WIDTH GARAGE HEADER APPLICATION - SINGLE STORY**  
**HEADER SUPPORTING: 1/2 ROOF SPAN**

SPAN	9'-3"			16'-3"			18'-3"			9'-3"			16'-3"			18'-3"											
	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	7-3/8	14	16-3/4
SHADING	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8				7-3/4	15-3/8		
	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	16-3/4	9-3/4	15-3/8						7-3/4				
	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4										7-3/4				
	8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4										7-3/4				
	8-3/8	14	15-3/8	8-3/8	15-3/8		8-3/8	15-3/8		9-3/4			9-3/4										7-3/4				
	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4										7-3/4				
	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4										7-3/4				
	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4										7-3/4				

SPAN	9'-3"			16'-3"			18'-3"			9'-3"			16'-3"			18'-3"											
	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8
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	8-3/8	12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td>	14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td>	8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td>	12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td>	14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td>	8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td>	12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td>	14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td>	8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td></td></td></td></td></td>	12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td></td></td></td></td>	14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td></td></td></td>	8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td></td></td>	12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td></td>	14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td></td>	8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td></td>	12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td></td>	14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td></td>	8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td></td>	12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td></td>	14 <td>8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td></td>	8-3/8 <td>12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td></td>	12-5/8 <td>14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td></td>	14 <td>8-3/8 <td>12-5/8 <td>15-3/8</td> </td></td>	8-3/8 <td>12-5/8 <td>15-3/8</td> </td>	12-5/8 <td>15-3/8</td>	15-3/8

**NOTES:**

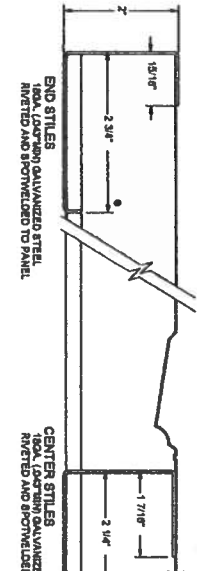
1. Table assumes a simple span header supporting a uniform load transferred from 1/2 the roof span plus a 2' soffit.
2. Roof live and dead loads shown are applied vertically to the horizontal projection. No reductions in roof live loads or snow loads were considered. The header weight is accounted for in the table.
3. Deflection is limited to L/240 for live load and L/180 for total load.
4. Headers are assumed to have continuous lateral support along top edge.
5. Bearing length based on full width bearing is indicated as follows:  
 Non-shaded sizes require two trimmers (3" bearing).  
 Shaded sizes require three trimmers (4.5" bearing).  
 Shaded and outlined sizes require four trimmers (6" bearing).
6. Applications where load carrying capacity of 16-3/4" depth has been exceeded. See AFP 30F<sub>b</sub> POWER BEAM® literature or AFP's Power-Sizer Software.



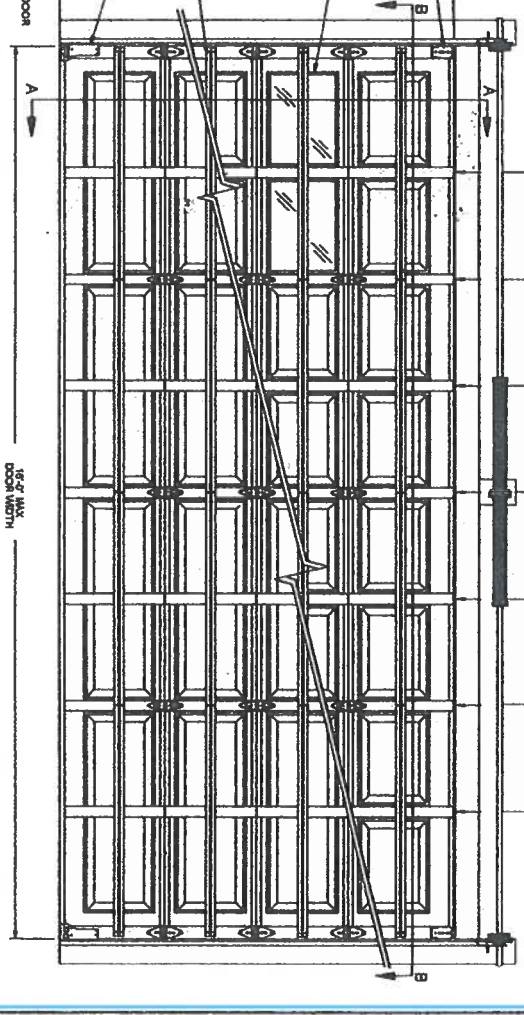
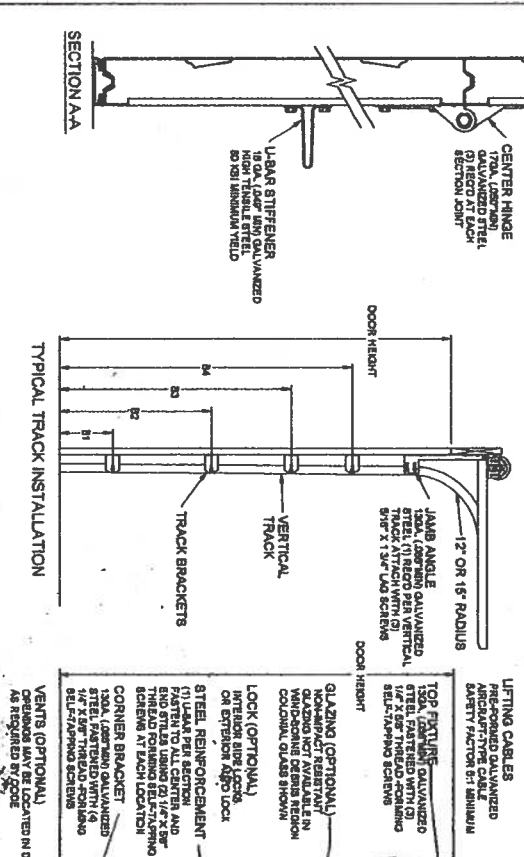
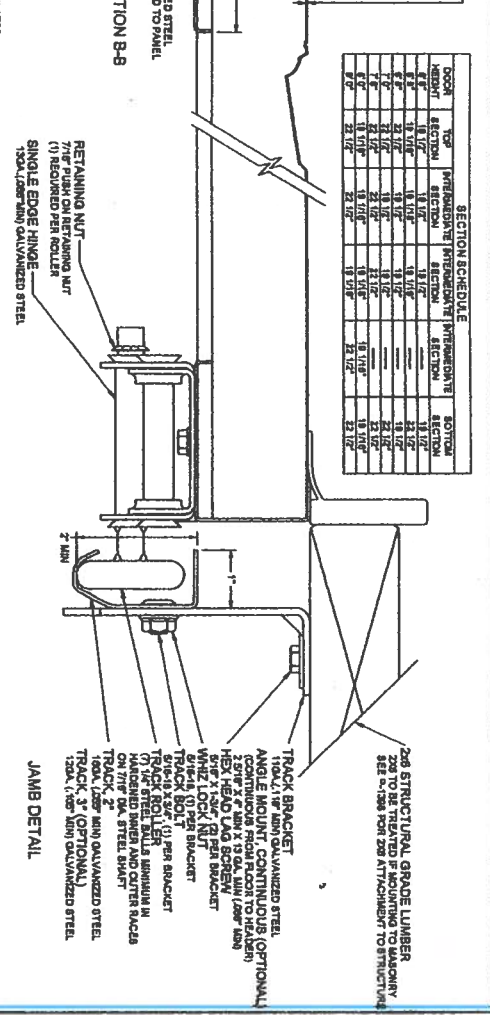
Received Time Jun. 8. 6:58AM



DOOR HEIGHT	B1	B2	B3	B4
6'-0"	12"	20"	28"	36"
6'-6"	12"	20"	28"	36"
7'-0"	12"	20"	28"	36"
7'-6"	12"	20"	28"	36"
8'-0"	12"	20"	28"	36"
8'-6"	12"	20"	28"	36"
9'-0"	12"	20"	28"	36"
9'-6"	12"	20"	28"	36"
10'-0"	12"	20"	28"	36"
10'-6"	12"	20"	28"	36"
11'-0"	12"	20"	28"	36"
11'-6"	12"	20"	28"	36"
12'-0"	12"	20"	28"	36"
12'-6"	12"	20"	28"	36"



DOOR HEIGHT	SECTION	SECTION	SECTION	SECTION	SECTION	SECTION
6'-0"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"
6'-6"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"
7'-0"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"
7'-6"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"
8'-0"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"
8'-6"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"
9'-0"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"
9'-6"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"
10'-0"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"
10'-6"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"
11'-0"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"
11'-6"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"
12'-0"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"
12'-6"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"	18 1/2"



**DOORS TESTED PER ASTM E-330**

NO GLASS		COLONIAL GLASS		RANCH GLASS	
QTY OF CENTER STILES	DESIGN TEST	DESIGN TEST	DESIGN TEST	DESIGN TEST	DESIGN TEST
7	+18.3	+27.5	+18.3	+27.5	+18.3
	-20.4	-30.6	-20.4	-30.6	-20.4
			N/A		N/A

REV.	DESCRIPTION	DATE	BY	CHKD
A	RELEASED FOR PRODUCTION	07/18/04	DL	DL

SCALE: FULL

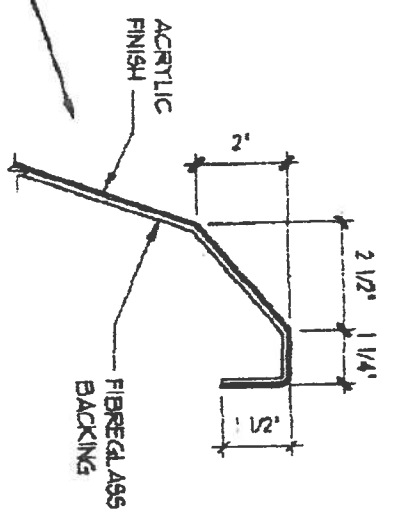
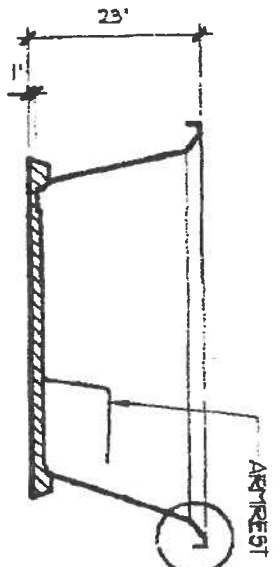
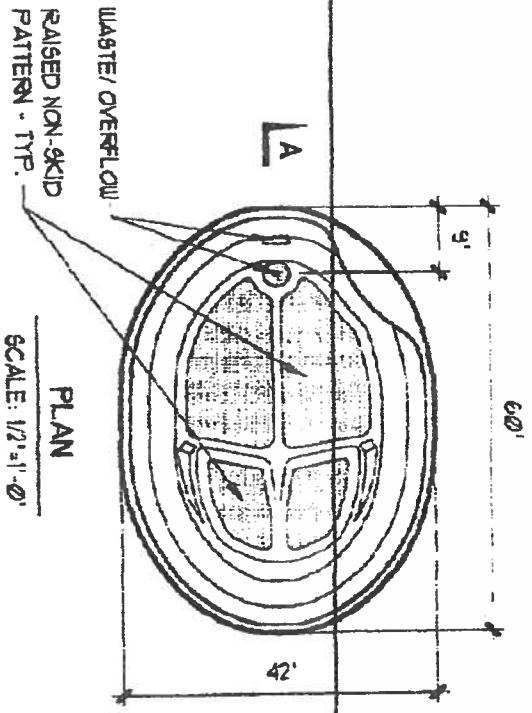
DRAWN BY: M. McDonald  
 CHECKED BY: Q. Whipple  
 DATE: 07/18/04

100# LOSTEM GALVANIZED STEEL

NO. P-2342

1 of 1

TYPE: SPEC. WINDOW LOAD  
 CHARACTERISTIC: CLASSIC  
 DECOR: 18' WIDE



MODEL NO. NB-410  
42" W X 60" L X 22" D



DATE	10-1-96	<b>BAKER MFG. CORP.</b> Manufacturers of: Spillway Spas • Natural Spas Natural Whirlpool Baths 7460 Chancellor Drive Orlando, FL 32809	SPECIFICATION DRAWING FOR <b>NATURAL WHIRLPOOL BATH</b> MODEL NO. NB-410	REVISIONS	BY
SCALE	AS NOTED				
DESIGNED BY	ESN				
CHECKED BY	RKN				
APPROVED BY					
DRAWING NUMBER	8-410				

# Plans

Site location - Map

Property ID Information .

Wind Load :- SPEC

Garbage Door - SPEC

Whirlpool - SPEC

Garbage Headers SPEC

Shingles SPEC

Doors SPEC

Deed Door - Specs

Window testing Report

Wind Load Report

Form 600A-2004R Energy Report

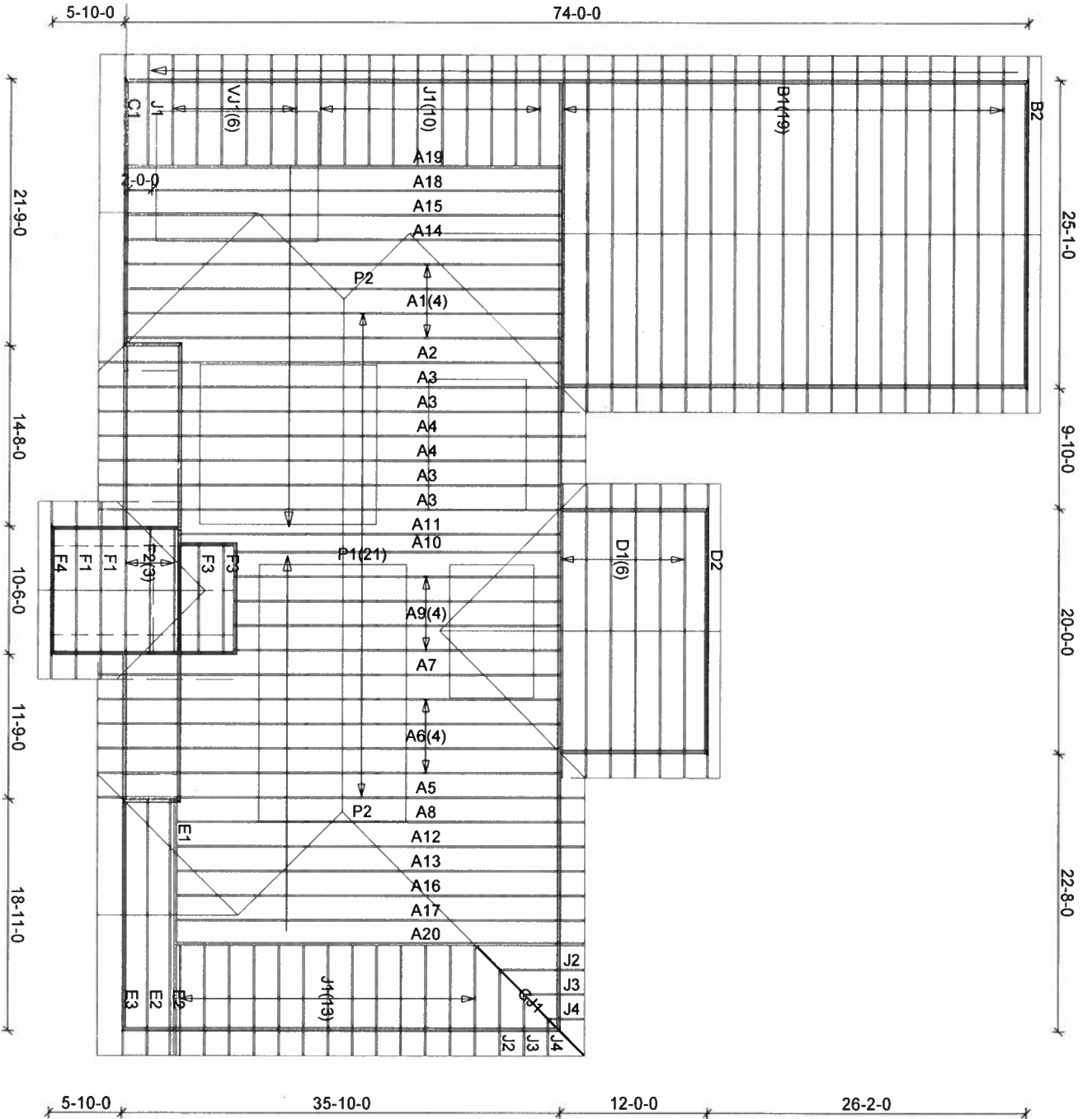
Truss Report

Mayo Truss Co. Inc.  
 845 East US 27  
 MAYO, FL 32066  
 (386)294-3988  
 (877)-558-6262

**JASON ELIXSON CONSTRUCTION**  
 TONY WILLIAMS RESIDENCE  
 110 MPH ASCE WIND LOAD

Roof Loading  
 TC Live: 20.00 psf  
 TC Dead: 10.00 psf  
 BC Live: 0.00 psf  
 BC Dead: 10.00 psf  
 TC Stress Inc: 25.00  
 BC Stress Inc: 25.00  
 Spacing: 2'-0" o.c.

Account: CONTRACTORS  
 Job: ELIXSON-WILL  
 Designer: MMURRAY  
 Checker: MMURRAY  
 Date: 06-06-07





RE: ELIXSON-WILL -

**Site Information:**

Project Customer: JASON ELIXSON CONSTRUCTION Project Name: TONY WILLIAMS  
 Lot/Block: - Subdivision: -  
 Address: -  
 City: COLUMBIA COUNTY State: FL

**Name Address and License # of Structural Engineer of Record, if there is one, for the building.**

Name: License #:  
 Address: State:  
 City:

**General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):**

Design Code: FBC2004/TPI2002 Design Program: Robbins OnLine Plus 21.0.032□  
 Wind Code: ASCE 7-02 Wind Speed: 110 mph Floor Load: N/A psf  
 Roof Load: 40.0 psf

This package includes 40 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T2566251	A1	6/5/07	18	T2566268	A18	6/5/07
2	T2566252	A2	6/5/07	19	T2566269	A19	6/5/07
3	T2566253	A3	6/5/07	20	T2566270	A20	6/5/07
4	T2566254	A4	6/5/07	21	T2566271	B1	6/5/07
5	T2566255	A5	6/5/07	22	T2566272	B2	6/5/07
6	T2566256	A6	6/5/07	23	T2566273	C1	6/5/07
7	T2566257	A7	6/5/07	24	T2566274	CJ1	6/5/07
8	T2566258	A8	6/5/07	25	T2566275	D1	6/5/07
9	T2566259	A9	6/5/07	26	T2566276	D2	6/5/07
10	T2566260	A10	6/5/07	27	T2566277	E1	6/5/07
11	T2566261	A11	6/5/07	28	T2566278	E2	6/5/07
12	T2566262	A12	6/5/07	29	T2566279	E3	6/5/07
13	T2566263	A13	6/5/07	30	T2566280	F1	6/5/07
14	T2566264	A14	6/5/07	31	T2566281	F2	6/5/07
15	T2566265	A15	6/5/07	32	T2566282	F3	6/5/07
16	T2566266	A16	6/5/07	33	T2566283	F4	6/5/07
17	T2566267	A17	6/5/07	34	T2566284	J1	6/5/07

The truss drawing(s) referenced above have been prepared by Robbins Engineering, Inc. under my direct supervision based on the parameters provided by Mayo Truss Company, Inc..

Truss Design Engineer's Name: Magid, Michael  
 My license renewal date for the state of Florida is February 28, 2009.

**NOTE:** The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

Michael S. Magid, FL Lic. #53681  
 Robbins Engineering  
 6904 Parke East Blvd  
 Tampa, FL, 33610  
 FL Cert.#5555

June 5, 2007

6904 Parke East Boulevard  
 Tampa, FL 33610-4115 Magid, Michael  
 Phone: 813-972-1135 • Fax: 813-971-6117  
 www.robbseng.com

RE: ELIXSON-WILL -

**Site Information:**

Project Customer: JASON ELIXSON CONSTRUCTION Project Name: TONY WILLIAMS

Lot/Block: - Subdivision: -

Address: -

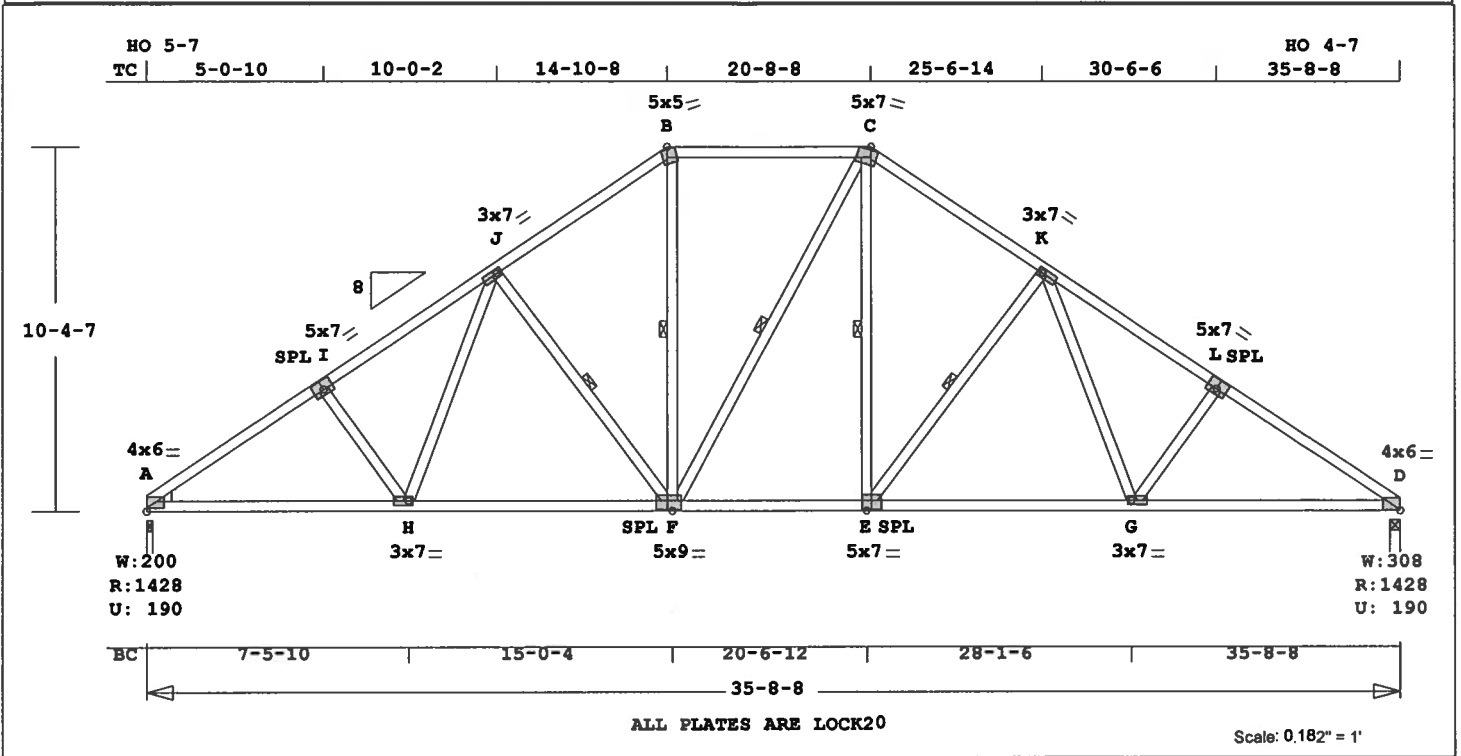
City: COLUMBIA COUNTY

State: FL

No.	Seal#	Truss Name	Date
35	T2566285	J2	6/5/07
36	T2566286	J3	6/5/07
37	T2566287	J4	6/5/07
38	T2566288	P1	6/5/07
39	T2566289	P2	6/5/07
40	T2566290	VJ1	6/5/07

Job <b>ELIXSON-WILL</b>	Mark <b>AI</b>	Quan 4	Type HIPP	Span 350808'	Pl-H1 8	Left OH 0	Right OH 0	Engineering <b>T2566251</b>
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TONY WILLIAMS



ALL PLATES ARE LOCK20

Scale: 0.182" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 277.3 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI	-Size-	---Lumber---
TC	0.35	2x 4 SP-#2
BC	0.52	2x 4 SP-#2
WB	0.11	2x 4 SP-#2
PB	---	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC	Cont.	0-0-0 14-10-8
TC	24.0"	14-10-8 20-8-8
TC	Cont.	20-8-8 35-8-8
BC	Cont.	0-0-0 35-8-8

WB 1 rows CLB on J -F  
WB 1 rows CLB on F -B  
WB 1 rows CLB on F -C  
WB 1 rows CLB on E -C  
WB 1 rows CLB on E -K  
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber	Duration Factor	1.25
Plate	Duration Factor	1.25
TC	Fb=1.15	Fc=1.10 Ft=1.10
BC	Fb=1.10	Fc=1.10 Ft=1.10

Jt	Down	Uplift	Horiz-
A	1428	190 U	230 R
D	1428	190 U	230 R

Jt	Brg Size	Required
A	2.0"	1.7"
D	3.5"	1.7"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A	I	0.33	2093	C	0.03 0.30
I	J	0.25	1925	C	0.09 0.16
J	B	0.24	1476	C	0.08 0.16
B	C	0.35	1228	C	0.01 0.34

Bottom Chords	Bottom Chords	Bottom Chords	Bottom Chords
A-H	0.52	1728	T 0.29 0.23
H-F	0.42	1493	T 0.15 0.27
F-E	0.35	1227	T 0.20 0.15
E-G	0.45	1504	T 0.15 0.30
G-D	0.48	1770	T 0.18 0.30
-----Webs-----			
I-H	0.05	234	C
H-J	0.05	374	T
J-F	0.10	441	C
F-B	0.09	495	T
F-C	0.05	124	T
E-C	0.09	499	T
E-K	0.11	454	C
K-G	0.06	407	T
G-L	0.05	253	C

TL Defl -0.23" in E -G L/999  
LL Defl -0.09" in E -G L/999  
Shear // Grain in B -C 0.21

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	Type	Plt Size	X	Y	JSI
Plate - LOCK	LOCK	4.0x 6.0	1.3	0.7	0.92
Plate - RHS	LOCK	5.0x 7.0	0.3	0.5	0.73
Jt Type	LOCK	3.0x 7.0	Ctr	Ctr	0.42
A	LOCK	5.0x 5.0	0.9-3.1	0.64	
B	LOCK	5.0x 7.0	1.6-3.4	0.70	
C	LOCK	3.0x 7.0	Ctr	Ctr	0.42
K	LOCK	5.0x 7.0	0.3	0.5	0.73
L	LOCK	4.0x 6.0	0.5	0.4	0.70
D	LOCK	3.0x 7.0	Ctr	Ctr	0.41
H	LOCK	5.0x 9.0	0.5-0.5	0.75	
F	LOCK	5.0x 7.0	Ctr	0.5	0.75
E	LOCK	3.0x 7.0	Ctr	Ctr	0.41
G	LOCK				

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.  
NOTES:  
Trusses Manufactured by: Mayo Truss Co. Inc.  
Analysis Conforms To: FBC2004

Design checked for 10 psf non-concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as Components and Claddings\* for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor: 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 2130 Lbs  
Max tens. force 1770 Lbs  
Quality Control Factor 1.25

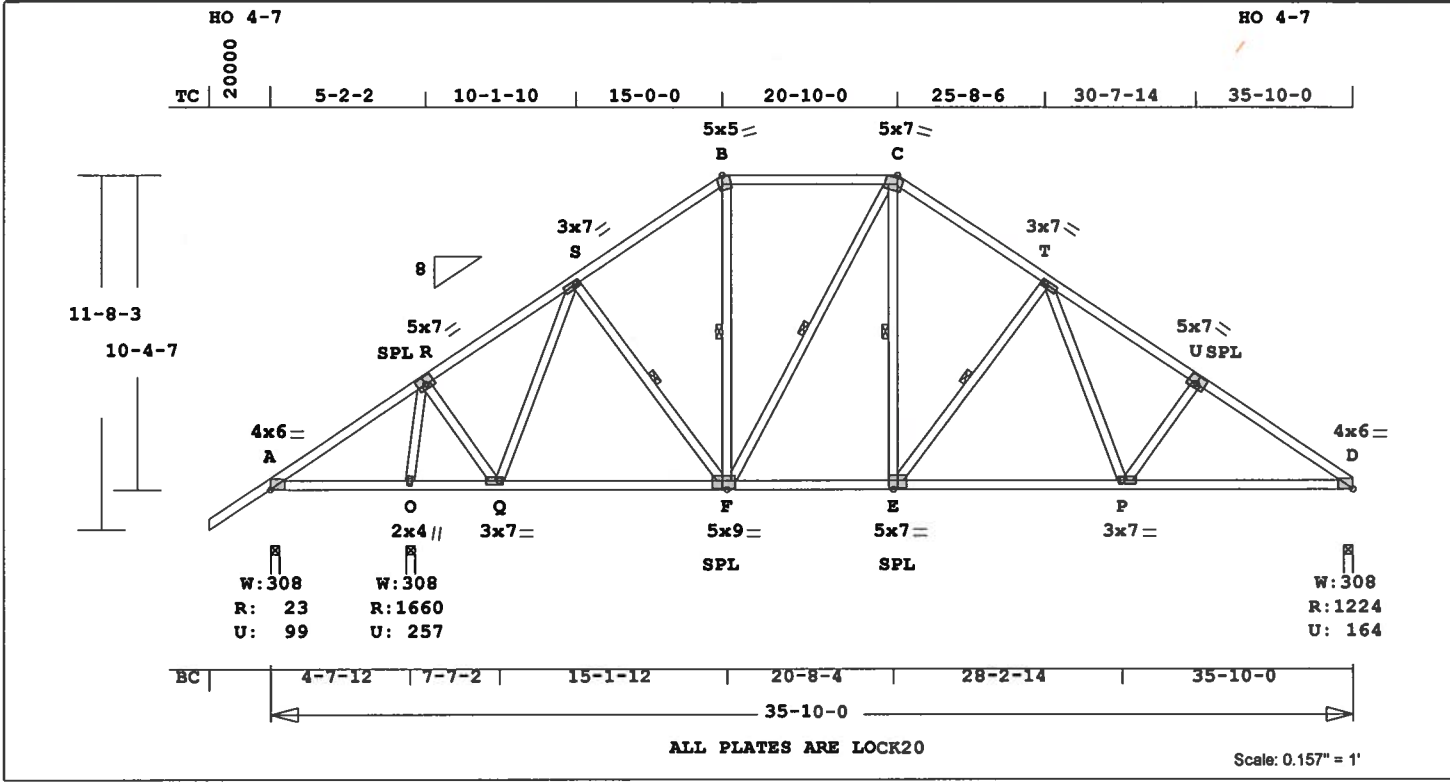
Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610



Job <b>ELIXSON-WILL</b>	Mark <b>A2</b>	Quan 1	Type HIPP	Span '351000'	Pl-H1 8	Left OH 2- 0- 0	Right OH 0	Engineering T2566252
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 287.5 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

TC	0.33	2x 4	SP-#2
BC	0.46	2x 4	SP-#2
WB	0.43	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	15- 0- 0
TC	24.0"	15- 0- 0 20-10- 0
TC Cont.	20-10- 0	35-10- 0
BC Cont.	0- 0- 0	35-10- 0

WB 1 rows CLB on S -F  
WB 1 rows CLB on F -B  
WB 1 rows CLB on F -C  
WB 1 rows CLB on E -C  
WB 1 rows CLB on E -T

Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15 Fc=1.10 Ft=1.10		
BC Fb=1.10 Fc=1.10 Ft=1.10		

Jt	Down	Uplift	Horiz-
A	23	99 U	230 R
O	1660	258 U	
D	1224	164 U	231 R

Jt	Brg Size	Required
A	3.5"	1.5"
O	3.5"	1.8"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -R	0.33	317 T	0.06	0.27
R -S	0.27	583 C	0.00	0.27
S -B	0.21	978 C	0.06	0.15
B -C	0.31	812 C	0.00	0.31
C -T	0.22	1114 C	0.00	0.22
T -U	0.23	1607 C	0.08	0.15

U -D	0.21	1776 C	0.07	0.14
-----Bottom Chords-----				
A -O	0.16	251 C	0.00	0.16
O -Q	0.23	122 T	0.00	0.23
Q -F	0.32	718 T	0.07	0.25
F -E	0.34	923 T	0.09	0.25
E -P	0.43	1205 T	0.12	0.31
P -D	0.46	1478 T	0.15	0.31
-----Webs-----				
O -R	0.25	1566 C		
R -Q	0.16	904 T		
Q -S	0.43	676 C		
S -F	0.02	162 T	1 Br	
F -B	0.04	287 T	1 Br	
F -C	0.09	229 C	1 Br	
E -C	0.09	501 T	1 Br	
E -T	0.11	463 C	1 Br	
T -P	0.06	415 T		
P -U	0.05	257 C		

TL Defl -0.03" in A -O L/999  
LL Defl -0.01" in A -O L/999  
Shear // Grain in B -C 0.21

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	0.5	0.4	0.70
R	LOCK	5.0x 7.0	-0.3	0.5	0.73
S	LOCK	3.0x 7.0	Ctr	Ctr	0.42
B	LOCK	5.0x 5.0	0.9-3.1		0.64
C	LOCK	5.0x 7.0	-1.6-3.4		0.70
T	LOCK	3.0x 7.0	Ctr	Ctr	0.42
U	LOCK	5.0x 7.0	0.3	0.5	0.73
D	LOCK	4.0x 6.0	-0.5	0.4	0.70
O	LOCK	2.0x 4.0	Ctr	Ctr	0.45
Q	LOCK	3.0x 7.0	Ctr	Ctr	0.41
F	LOCK	5.0x 9.0	0.5-0.5		0.75
E	LOCK	5.0x 7.0	Ctr	-0.5	0.75
P	LOCK	3.0x 7.0	Ctr	Ctr	0.41

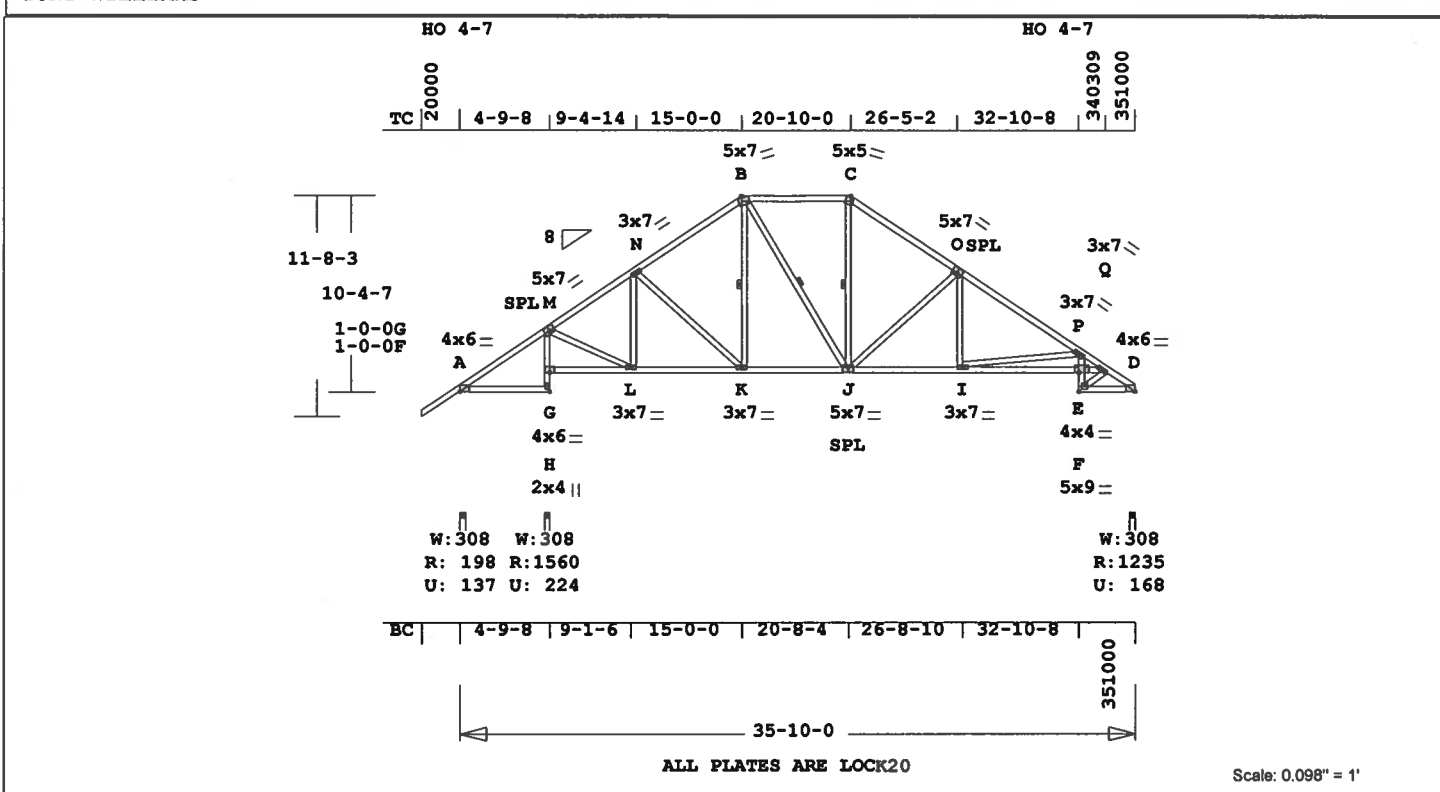
REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.  
NOTES:  
Trusses Manufactured by: Mayo Truss Co. Inc.  
Analysis Conforms To: FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as Components and Claddings\* for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
User-defined wind-exposed BC regions --From-- --To--  
0- 0- 0 4- 7-12  
Max comp. force 1776 Lbs  
Max tens. force 1478 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

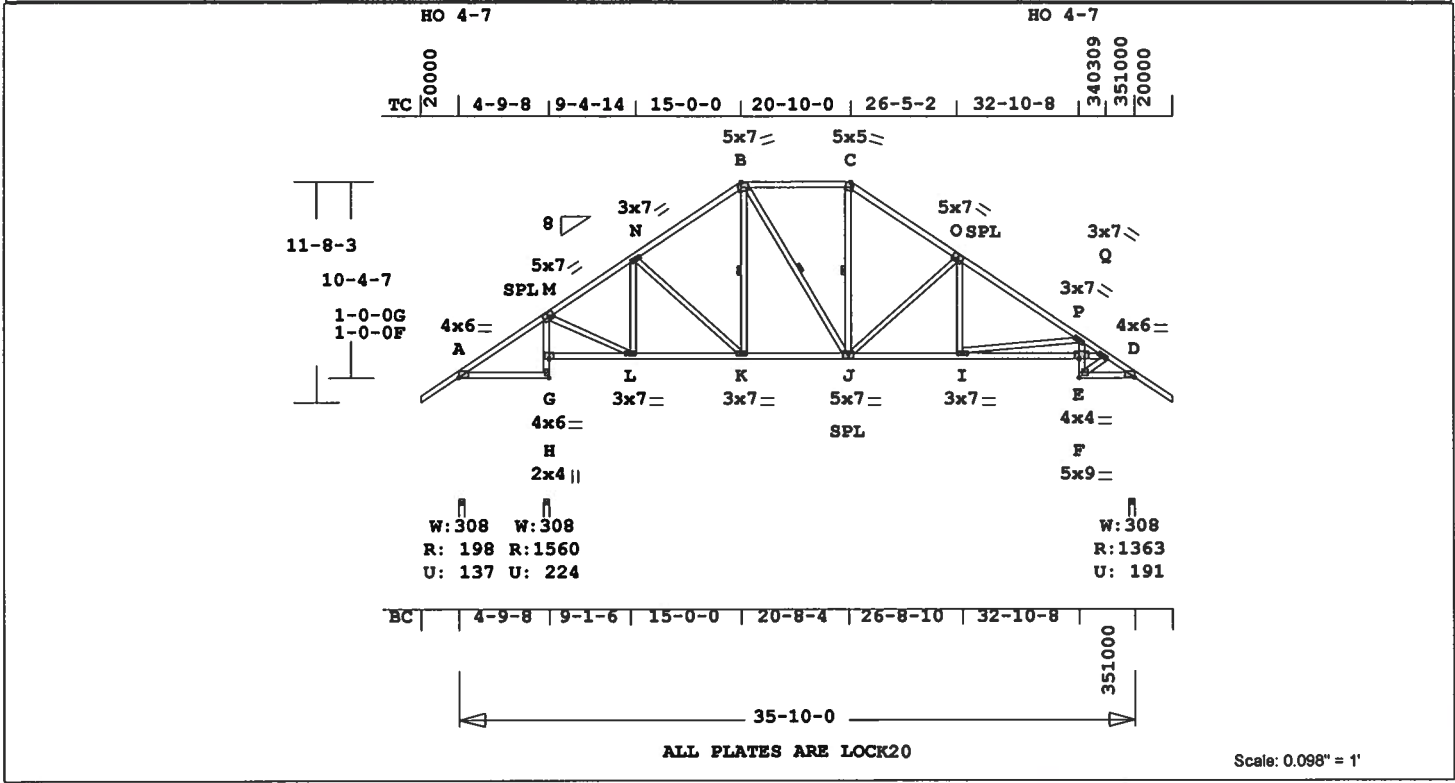
Job <b>ELIXSON-WILL</b>	Mark <b>A3</b>	Quan <b>4</b>	Type <b>SP</b>	Span <b>351000</b>	Pl-H1 <b>8</b>	Left OH <b>2- 0- 0</b>	Right OH <b>0</b>	Engineering <b>T2566253</b>
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<p>Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 285.8 LBS</p> <p>Online Plus -- Version 21.0.032 RUN DATE: 05-JUN-07</p> <p>CSI -Size- ---Lumber---</p> <p>TC 0.46 2x 4 SP-#2 BC 0.74 2x 4 SP-#2 CW 0.56 2x 4 SP-#2 WB 0.87 2x 4 SP-#2</p> <p>Brace truss as follows:</p> <p>O.C. From To TC Cont. 0- 0- 0 15- 0- 0 TC 24.0" 15- 0- 0 20-10- 0 TC Cont. 20-10- 0 35-10- 0 BC Cont. 0- 0- 0 35-10- 0</p> <p>WB 1 rows CLB on K -B WB 1 rows CLB on B -J WB 1 rows CLB on J -C Attach CLB with (2)-10d nails at each web.</p> <p>psf-Ld Dead Live TC 10.0 20.0 BC 10.0 0.0 TC+BC 20.0 20.0 Total 40.0 Spacing 24.0"</p> <p>Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10</p> <p>Total Load Reactions (Lbs) Jt Down Uplift Horiz A 198 137 U 230 R H 1561 225 U D 1236 169 U 231 R</p> <p>Jt Brg Size Required A 3.5" 1.5" H 3.5" 1.7" D 3.5" 1.5"</p> <p>Plus 9 Wind Load Case(s) Plus 1 UBC LL Load Case(s)</p> <p>Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords----- A -M 0.28 275 T 0.04 0.24 M -N 0.24 1053 C 0.00 0.24 N -B 0.24 1117 C 0.06 0.18 B -C 0.29 1044 C 0.01 0.28 C -O 0.41 1266 C 0.01 0.40 O -P 0.46 1863 C 0.07 0.39 P -Q 0.31 3638 C 0.15 0.16 Q -D 0.19 1794 C 0.01 0.18 -----Bottom Chords----- A -H 0.14 131 C 0.00 0.14 G -L 0.15 154 C 0.00 0.15 L -K 0.27 876 T 0.09 0.18</p>	<p>K -J 0.27 927 T 0.09 0.18 J -I 0.44 1561 T 0.26 0.18 I -F 0.74 3211 T 0.53 0.21 E -D 0.27 1397 T 0.23 0.04 -----Chord-Webs----- H -G 0.28 1507 C 0.01 0.27 G -M 0.13 1450 C 0.02 0.11 E -F 0.56 850 T 0.15 0.41 F -P 0.40 917 T 0.16 0.24 -----Webs----- M -L 0.20 1136 T L -N 0.13 390 C N -K 0.07 119 T K -B 0.03 195 T 1 Br B -J 0.04 223 T 1 Br J -C 0.06 409 T 1 Br J -O 0.52 691 C I -O 0.07 457 T I -P 0.87 1666 C E -Q 0.53 2884 T E -Q 0.14 1463 C</p> <p>TL Defl -0.29" in I -F L/999 LL Defl -0.13" in I -F L/999 Shear // Grain in E -F 0.26</p> <p>Plates for each ply each face. PLATING CONFORMS TO TPI. REPORTS: SBCCI 9761 ROBBINS ENGINEERING, INC. BASED ON SP LUMBER USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI A LOCK 4.0x 6.0 0.5 0.4 0.70 M LOCK 5.0x 7.0-0.3 0.5 0.77 N LOCK 3.0x 7.0 Ctr Ctr 0.44 B LOCK 5.0x 7.0 1.6-3.4 0.70 C LOCK 5.0x 5.0-0.9-3.1 0.64 O LOCK 5.0x 7.0 0.3 0.5 0.73 P LOCK 3.0x 7.0 Ctr Ctr 0.48 Q LOCK 3.0x 7.0-0.4 0.3 0.97 D LOCK 4.0x 6.0-0.5 0.4 0.70 H LOCK 2.0x 4.0 Ctr Ctr 0.60 G LOCK 4.0x 6.0 Ctr 0.2 0.75 L LOCK 3.0x 7.0 Ctr Ctr 0.46 K LOCK 3.0x 7.0 Ctr Ctr 0.44 J LOCK 5.0x 7.0 Ctr-0.5 0.75 I LOCK 3.0x 7.0 Ctr Ctr 0.85 F LOCK 5.0x 9.0 Ctr 0.8 0.64 E LOCK 4.0x 4.0 Ctr Ctr 0.84</p> <p>REVIEWED BY: Robbins Engineering, Inc. 6904 Parke East Blvd. Tampa, FL 33610</p>	<p>REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS. NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf non- concurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as Components and Claddings* for Exterior zone location. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: B Occupancy Factor : 1.00 Building Type: Enclosed TC Dead Load: 5.0 psf BC Dead Load: 5.0 psf User-defined wind-exposed BC regions --From-- --To-- 0- 0- 0 4- 9- 8 Max comp. force 3638 Lbs Max tens. force 3211 Lbs Quality Control Factor 1.25</p> <p>Michael S. Magid, FL Lic. #53681 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555</p>
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Job <b>ELIXSON-WILL</b>	Mark <b>A4</b>	Quan <b>2</b>	Type <b>SP</b>	Span <b>351000'</b>	P1-H1 <b>8</b>	Left OH <b>2- 0- 0</b>	Right OH <b>2- 0- 0</b>	Engineering <b>T2566254</b>
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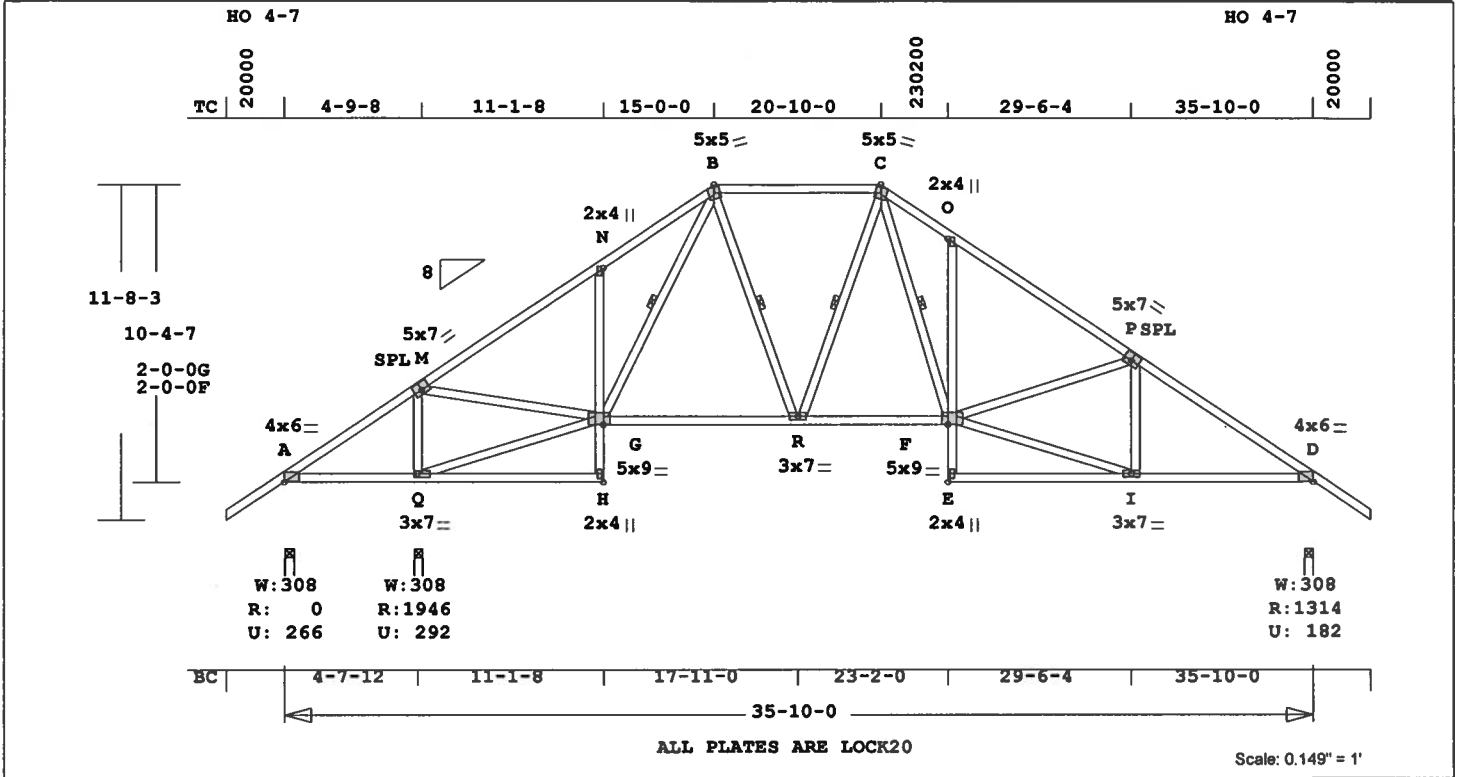
TONY WILLIAMS



<p>Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 290.3 LBS</p> <p>Online Plus -- Version 21.0.032 RUN DATE: 05-JUN-07</p> <p>CSI -Size- ----Lumber-----          TC 0.46 2x 4 SP-#2          BC 0.74 2x 4 SP-#2          CW 0.56 2x 4 SP-#2          WB 0.87 2x 4 SP-#2</p> <p>Brace truss as follows:          O.C. From To          TC Cont. 0- 0- 0 15- 0- 0          TC 24.0" 15- 0- 0 20-10- 0          TC Cont. 20-10- 0 35-10- 0          BC Cont. 0- 0- 0 35-10- 0          WB 1 rows CLB on K -B          WB 1 rows CLB on B -J          WB 1 rows CLB on J -C          Attach CLB with (2)-10d nails          at each web.</p> <p>psf-Ld Dead Live          TC 10.0 20.0          BC 10.0 0.0          TC+BC 20.0 20.0          Total 40.0 Spacing 24.0"          Lumber Duration Factor 1.25          Plate Duration Factor 1.25          TC Fb=1.15 Fc=1.10 Ft=1.10          BC Fb=1.10 Fc=1.10 Ft=1.10</p> <p>Total Load Reactions (Lbs)          Jt Down Uplift Horiz-          A 198 137 U 230 R          H 1560 225 U          D 1364 191 U 231 R</p> <p>Jt Bryg Size Required          A 3.5" 1.5"          H 3.5" 1.7"          D 3.5" 1.6"</p> <p>Plus 9 Wind Load Case(s)          Plus 1 UBC LL Load Case(s)</p> <p>Membr CSI P Lbs Ax1-CSI-Bnd          -----Top Chords-----          A -M 0.28 275 T 0.04 0.24          M -N 0.24 1053 C 0.00 0.24          N -B 0.24 1117 C 0.06 0.18          B -C 0.29 1044 C 0.01 0.28          C -O 0.41 1266 C 0.01 0.40          O -P 0.46 1863 C 0.07 0.39          P -Q 0.31 3638 C 0.15 0.16          Q -D 0.19 1794 C 0.01 0.18          -----Bottom Chords-----          A -H 0.14 131 C 0.00 0.14          G -L 0.15 154 C 0.00 0.15          L -K 0.27 876 T 0.09 0.18</p>	<p>K -J 0.27 927 T 0.09 0.18          J -I 0.44 1561 T 0.26 0.18          I -F 0.74 3211 T 0.53 0.21          E -D 0.27 1397 T 0.23 0.04          -----Chord-Webs-----          H -G 0.28 1506 C 0.01 0.27          G -M 0.13 1450 C 0.02 0.11          E -F 0.56 850 T 0.15 0.41          F -P 0.40 917 T 0.16 0.24          -----Webs-----          M -L 0.20 1136 T          L -N 0.13 389 C          N -K 0.07 119 T          K -B 0.03 195 T          B -J 0.04 223 T          J -C 0.06 409 T          J -O 0.52 691 C          I -O 0.07 457 T          I -P 0.87 1666 C          F -Q 0.53 2884 T          E -Q 0.14 1462 C</p> <p>TL Defl -0.29" in I -F L/999          LL Defl -0.13" in I -F L/999          Shear // Grain in E -F 0.26</p> <p>Plates for each ply each face.          PLATING CONFORMS TO TPI.          REPORTS: SBCCI 9761          ROBBINS ENGINEERING, INC.          BASED ON SP LUMBER          USING GROSS AREA TEST.          Plate - LOCK 20 Ga, Gross Area          Plate - RHS 20 Ga, Gross Area          Jt Type Plt Size X Y JSI          A LOCK 4.0x 6.0 0.5 0.4 0.70          M LOCK 5.0x 7.0-0.3 0.5 0.77          N LOCK 3.0x 7.0 Ctr Ctr 0.44          B LOCK 5.0x 7.0 1.6-3.4 0.70          C LOCK 5.0x 5.0-0.9-3.1 0.64          O LOCK 5.0x 7.0 0.3 0.5 0.73          P LOCK 3.0x 7.0 Ctr Ctr 0.48          Q LOCK 3.0x 7.0-0.4 0.3 0.97          D LOCK 4.0x 6.0-0.5 0.4 0.70          H LOCK 2.0x 4.0 Ctr Ctr 0.60          G LOCK 4.0x 6.0 Ctr Ctr 0.2 0.75          L LOCK 3.0x 7.0 Ctr Ctr 0.46          K LOCK 3.0x 7.0 Ctr Ctr 0.44          J LOCK 5.0x 7.0 Ctr-0.5 0.75          I LOCK 3.0x 7.0 Ctr Ctr 0.85          F LOCK 5.0x 9.0 Ctr 0.8 0.64          E LOCK 4.0x 4.0 Ctr Ctr 0.84</p> <p>REVIEWED BY:          Robbins Engineering, Inc.          6904 Parke East Blvd.          Tampa, FL 33610</p>	<p>REFER TO ROBBINS ENG. GENERAL          NOTES AND SYMBOLS SHEET FOR          ADDITIONAL SPECIFICATIONS.          NOTES:          Trusses Manufactured by:          Mayo Truss Co. Inc.          Analysis Conforms To:          FBC2004          OH Loading          Soffit psf 2.0          Design checked for 10 psf non-          concurrent LL on BC.          Wind Loads - ANSI / ASCE 7-02          Truss is designed as          Components and Claddings*          for Exterior zone location.          Wind Speed: 110 mph          Mean Roof Height: 15-0          Exposure Category: B          Occupancy Factor : 1.00          Building Type: Enclosed          TC Dead Load: 5.0 psf          BC Dead Load: 5.0 psf          User-defined wind-exposed BC          regions --From-- --To--          0- 0- 0 4- 9- 8          Max comp. force 3638 Lbs          Max tens. force 3211 Lbs          Quality Control Factor 1.25</p> <p>Michael S. Magid, FL Lic. #53681          Robbins Engineering          6904 Parke East Blvd          Tampa, FL, 33610          FL Cert.#5555</p>
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Job <b>ELIXSON-WILL</b>	Mark <b>A5</b>	Quan 1	Type SP	Span 351000	Pl-H1 8	Left OH 2- 0- 0	Right OH 2- 0- 0	Engineering T2566255
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TONY WILLIAMS



ALL PLATES ARE LOCK20 Scale: 0.149" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 309.1 LBS

Online Plus -- Version 21.0.032	G -R 0.33 895 T 0.09 0.24	NOTES:
RUN DATE: 05-JUN-07	R -F 0.35 1073 T 0.11 0.24	Trusses Manufactured by:
	E -I 0.24 12 C 0.00 0.24	Mayo Truss Co. Inc.
	I -D 0.38 1418 T 0.14 0.24	Analysis Conforms To:
		FBC2004
		OH Loading
		Soffit psf 2.0
		Design checked for 10 psf non-
		concurrent LL on BC.
		Wind Loads - ANSI / ASCE 7-02
		Truss is designed as
		Components and Claddings*
		for Exterior zone location.
		Wind Speed: 110 mph
		Mean Roof Height: 15-0
		Exposure Category: B
		Occupancy Factor : 1.00
		Building Type: Enclosed
		TC Dead Load: 5.0 psf
		BC Dead Load: 5.0 psf
		User-defined wind-exposed BC
		regions --From-- ---To---
		0- 0- 0 4- 7-12
		Max comp. force 1697 Lbs
		Max tens. force 1474 Lbs
		Quality Control Factor 1.25

CS1 -Size- ---Lumber---	
TC 0.49 2x 4 SP-#2	
BC 0.38 2x 4 SP-#2	
CW 0.15 2x 4 SP-#2	
WB 0.31 2x 4 SP-#2	
Brace truss as follows:	
O.C. From To	
TC Cont. 0- 0- 0 15- 0- 0	
TC 24.0" 15- 0- 0 20-10- 0	
TC Cont. 20-10- 0 35-10- 0	
BC Cont. 0- 0- 0 35-10- 0	
WB 1 rows CLB on G -B	
WB 1 rows CLB on B -R	
WB 1 rows CLB on R -C	
WB 1 rows CLB on C -F	
Attach CLB with (2)-10d nails	
at each web.	
psf-Ld Dead Live	
TC 10.0 20.0	
BC 10.0 0.0	
TC+BC 20.0 20.0	
Total 40.0 Spacing 24.0"	
Lumber Duration Factor 1.25	
Plate Duration Factor 1.25	
TC Fb=1.15 Fc=1.10 Ft=1.10	
BC Fb=1.10 Fc=1.10 Ft=1.10	

Chord-Webs	
H -G 0.15 98 T 0.01 0.14	
G -N 0.07 349 C 0.00 0.07	
E -F 0.05 102 T 0.00 0.05	
F -O 0.04 291 C 0.01 0.03	
Webs	
Q -M 0.23 1608 C	
Q -G 0.31 566 C	
M -G 0.27 1472 T	
G -B 0.03 145 T 1 Br	
B -R 0.06 343 T 1 Br	
R -C 0.04 174 C 1 Br	
C -F 0.15 856 T 1 Br	
F -P 0.12 232 C	
F -I 0.27 1474 T	
I -P 0.07 302 C	

TL Defl -0.18" in E -I L/999	
LL Defl -0.08" in E -I L/999	
Shear // Grain in B -C 0.23	
Plates for each ply each face.	
PLATING CONFORMS TO TPI.	
REPORTS: SBCCI 9761	
ROBBINS ENGINEERING, INC.	
BASED ON SP LUMBER	
USING GROSS AREA TEST.	
Plate - LOCK 20 Ga, Gross Area	
Plate - RHS 20 Ga, Gross Area	
Jt Type Plt Size X Y JSI	
A LOCK 4.0x 6.0 0.5 0.4 0.70	
M LOCK 5.0x 7.0-0.3 0.5 0.73	
N LOCK 2.0x 4.0 Ctr Ctr 0.23	
B LOCK 5.0x 5.0 0.1-3.7 0.79	
C LOCK 5.0x 5.0-0.1-3.7 0.79	
O LOCK 2.0x 4.0 Ctr Ctr 0.23	
P LOCK 5.0x 7.0 0.3 0.5 0.73	
D LOCK 4.0x 6.0-0.5 0.4 0.70	
Q LOCK 3.0x 7.0 Ctr Ctr 0.52	
H LOCK 2.0x 4.0 Ctr Ctr 0.58	
G LOCK 5.0x 9.0 Ctr 0.8 0.70	
R LOCK 3.0x 7.0 Ctr Ctr 0.39	
F LOCK 5.0x 9.0 Ctr 0.8 0.77	
E LOCK 2.0x 4.0 Ctr Ctr 0.58	
I LOCK 3.0x 7.0 Ctr Ctr 0.52	

Jt Down Uplift Horiz	
A 267 G 231 R	
Q 1947 292 U	
D 1315 182 U 232 R	
G = Gravity Uplift	
Jt Brg Size Required	
A 3.5" 1.5"	
Q 3.5" 2.1"	
D 3.5" 1.6"	

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd	
-----Top Chords-----	
A -M 0.49 696 T 0.12 0.37	
M -N 0.37 1078 C 0.00 0.37	
N -B 0.26 1091 C 0.00 0.26	
B -C 0.34 1011 C 0.01 0.33	
C -O 0.25 1605 C 0.09 0.16	
O -P 0.38 1606 C 0.01 0.37	
P -D 0.38 1697 C 0.01 0.37	
-----Bottom Chords-----	
A -Q 0.27 556 C 0.00 0.27	
Q -H 0.27 42 C 0.00 0.27	

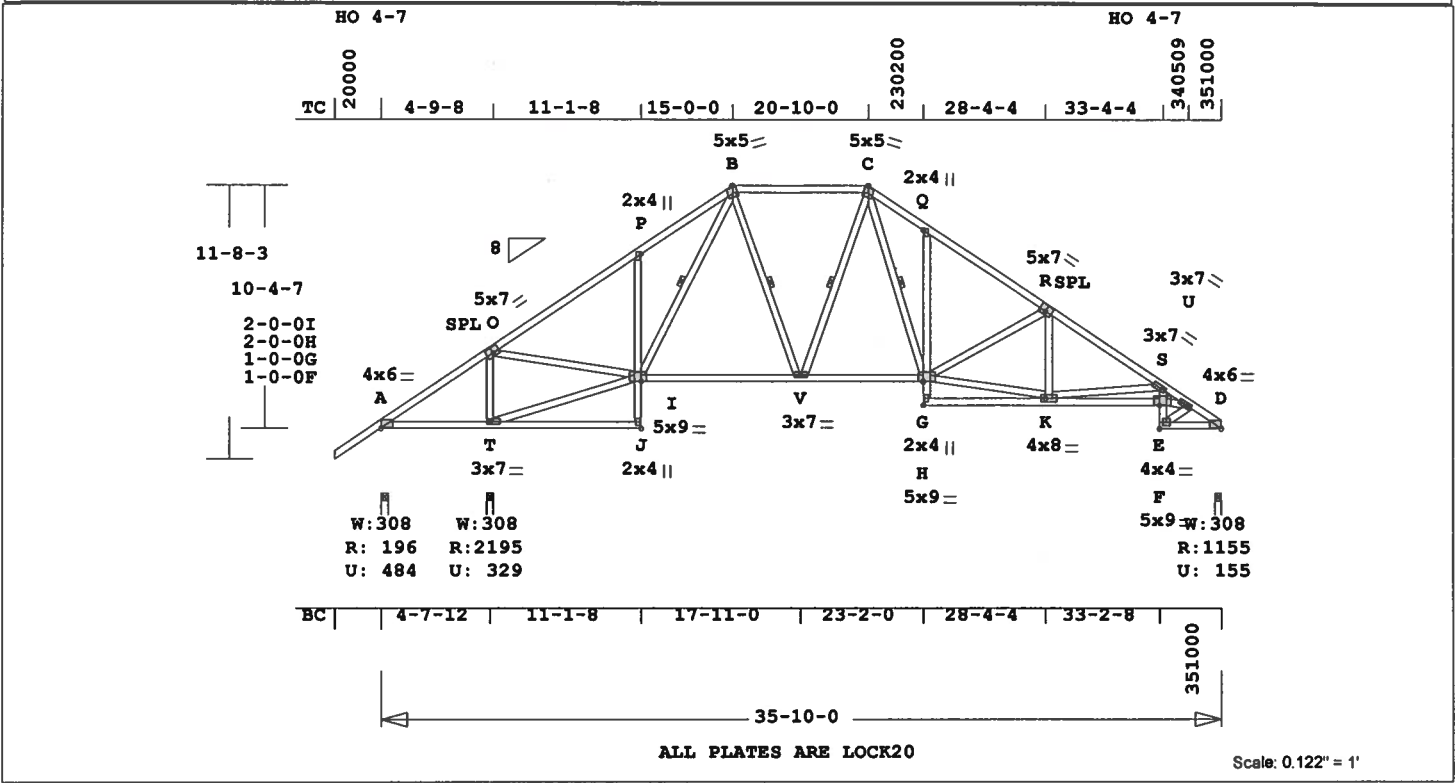
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555



Job <b>ELIXSON-WILL</b>	Mark <b>A7</b>	Quan 1	Type SP	Span 351000	Pl-H1 8	Left OH 2-0-0	Right OH 0	Engineering T2566257
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TONY WILLIAMS



ALL PLATES ARE LOCK20

Scale: 0.122" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 313.6 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

TC	CSI -Size-	---Lumber---
0.57	2x 4	SP-#2
0.81	2x 4	SP-#2
0.63	2x 4	SP-#2
0.52	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0-0-0	15-0-0
TC	24.0"	15-0-0 to 20-10-0
TC Cont.	20-10-0	35-10-0
BC Cont.	0-0-0	35-10-0

WB 1 rows CLB on I -B  
WB 1 rows CLB on B -V  
WB 1 rows CLB on V -C  
WB 1 rows CLB on C -H

Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"

Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (lbs)

Jt	Down	Uplift	Horiz-
A	196	484	231
T	2196	329	U
D	1155	155	U 232

G = Gravity Uplift

Jt	Brg Size	Required
A	3.5"	1.5"
T	3.5"	2.3"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Mnbr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A-O	0.57	1067	T 0.19	0.38
O-P	0.38	906	C 0.00	0.38
P-B	0.26	920	C 0.00	0.26
B-C	0.32	943	C 0.01	0.31
C-Q	0.20	1502	C 0.01	0.19
Q-R	0.33	1524	C 0.01	0.32
R-S	0.34	1932	C 0.02	0.32
S-U	0.24	3674	C 0.09	0.15
U-D	0.16	1660	C 0.01	0.15
-----Bottom Chords-----				
A-T	0.29	861	C 0.00	0.29
T-J	0.29	45	C 0.00	0.29

I-V	0.32	815	T 0.08	0.24
V-H	0.34	1015	T 0.10	0.24
G-K	0.21	39	T 0.00	0.21
K-F	0.81	3197	T 0.53	0.28
E-D	0.25	1284	T 0.21	0.04
-----Chord-Webs-----				
J-I	0.16	97	T 0.01	0.15
I-P	0.07	351	C 0.00	0.07
G-H	0.08	77	T 0.00	0.08
H-Q	0.06	223	C 0.01	0.05
E-F	0.63	741	T 0.13	0.50
F-S	0.37	1027	T 0.18	0.19
-----Webs-----				
T-O	0.26	1752	C	
T-I	0.49	883	C	
O-I	0.30	1636	T	
I-B	0.04	177	C	1 Br
B-V	0.06	367	T	1 Br
V-C	0.05	206	C	1 Br
C-H	0.14	806	T	1 Br
H-R	0.18	429	C	
H-K	0.29	1620	T	
K-R	0.03	204	T	
K-S	0.47	1583	C	
F-U	0.52	2839	T	
E-U	0.12	1273	C	

TL Defl	-0.25"	in K -F	L/999
LL Defl	-0.12"	in K -F	L/999
Shear // Grain		in E -F	0.31

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	LOCK 20 Ga, Gross Area				
Plate - RHS	20 Ga, Gross Area				
Jt Type	Plt Size	X	Y	JSI	
A LOCK	4.0x	6.0	0.5	0.4	0.70
O LOCK	5.0x	7.0	0.3	0.5	0.73
P LOCK	2.0x	4.0	Ctr	Ctr	0.23
B LOCK	5.0x	5.0	0.1-3.7	0.79	
C LOCK	5.0x	5.0	0.1-3.7	0.79	
Q LOCK	2.0x	4.0	Ctr	Ctr	0.23
R LOCK	5.0x	7.0	0.3	0.5	0.73
S LOCK	3.0x	7.0	Ctr	Ctr	0.45
U LOCK	3.0x	7.0	0.4	0.3	0.89
D LOCK	4.0x	6.0	0.5	0.4	0.70
T LOCK	3.0x	7.0	Ctr	Ctr	0.52
J LOCK	2.0x	4.0	Ctr	Ctr	0.58
I LOCK	5.0x	9.0	Ctr	0.8	0.70
V LOCK	3.0x	7.0	Ctr	0.39	
H LOCK	5.0x	9.0	Ctr	0.8	0.77
G LOCK	2.0x	4.0	Ctr	0.58	
K LOCK	4.0x	8.0	Ctr	0.64	
F LOCK	5.0x	9.0	Ctr	0.8	0.64
E LOCK	4.0x	4.0	Ctr	0.84	

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

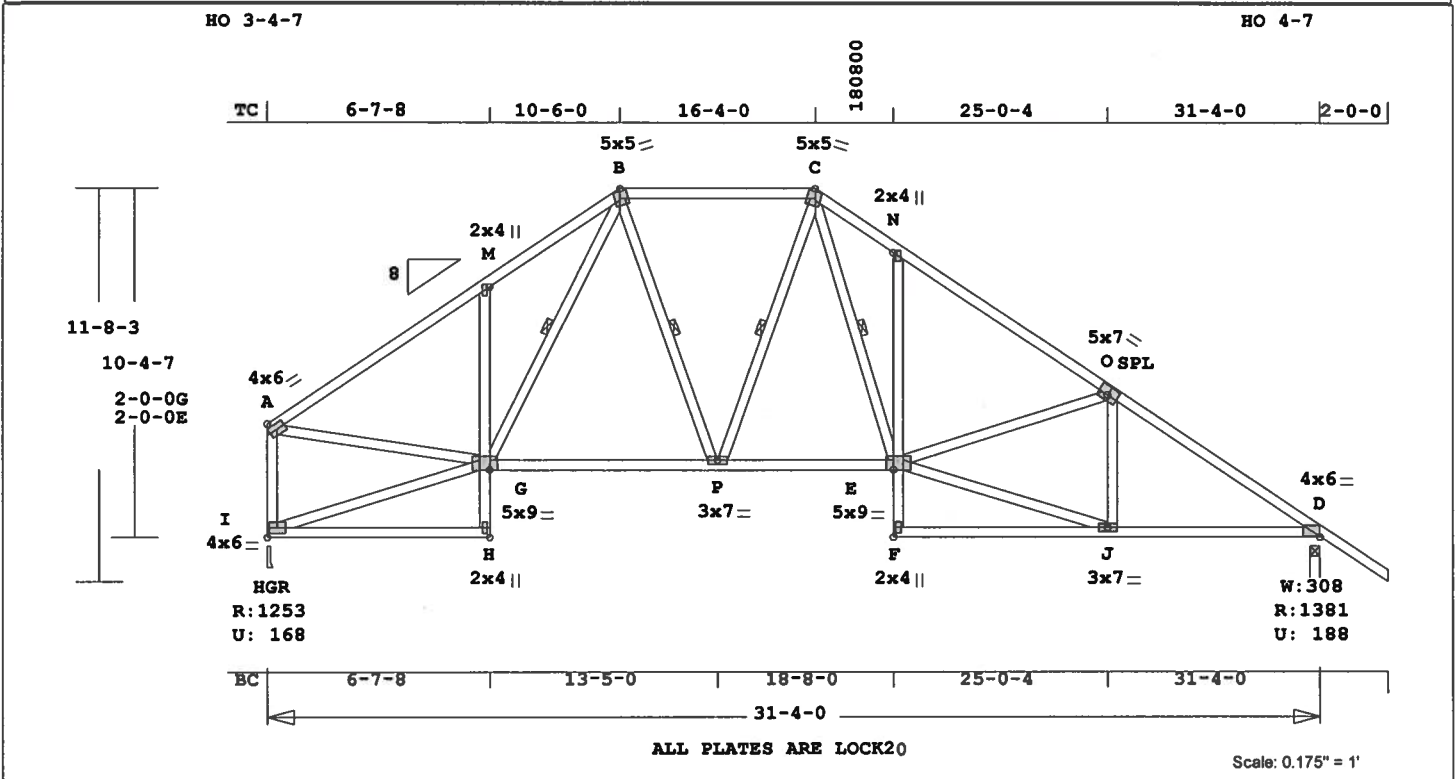
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
User-defined wind-exposed BC regions --From-- ---To---  
0-0-0 4-7-12  
Max comp. force 3674 Lbs  
Max tens. force 3197 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>A8</b>	Quan 1	Type SP	Span 31'04"00	P1-H1 8	Left OH 0	Right OH 2'-0"-0	Engineering <b>T2566258</b>
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TONY WILLIAMS



ALL PLATES ARE LOCK20

Scale: 0.175" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 285.9 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI	Size	Lumber
TC	0.49	2x 4 SP-#2
BC	0.39	2x 4 SP-#2
CW	0.14	2x 4 SP-#2
WB	0.29	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0-0-0	10-6-0
TC	24.0"	10-6-0 16-4-0
TC Cont.	16-4-0	31-4-0
BC Cont.	0-0-0	31-4-0

WB 1 rows CLB on G - B  
WB 1 rows CLB on B - P  
WB 1 rows CLB on P - C  
WB 1 rows CLB on C - E  
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
I	1253	168 U	266 R
D	1381	188 U	214 R

Jt	Brg Size	Required
I	3.5"	1.5"
D	3.5"	1.6"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
-----Top Chords-----					
A - M	0.49	1432	C	0.04	0.45
M - B	0.37	1474	C	0.01	0.36
B - C	0.35	1157	C	0.01	0.34
C - N	0.25	1759	G	0.09	0.16
N - O	0.38	1760	C	0.02	0.36
O - D	0.37	1812	C	0.01	0.36
-----Bottom Chords-----					

Member	From	To	Chord	Web	
I - H	0.31	37	C	0.00	0.31
G - P	0.35	1063	T	0.10	0.25
P - E	0.37	1195	T	0.20	0.17
F - J	0.24	14	T	0.00	0.24
J - D	0.39	1513	T	0.15	0.24
-----Chord-Webs-----					
H - G	0.14	126	T	0.01	0.13
G - M	0.08	400	C	0.02	0.06
F - E	0.05	102	T	0.00	0.05
E - N	0.05	291	C	0.01	0.04
-----Webs-----					
I - A	0.17	1190	C	WindLd	
I - G	0.03	241	T		
A - G	0.22	1226	T		
G - B	0.05	368	T		1 Br
B - P	0.04	293	T		1 Br
P - C	0.02	110	C		1 Br
C - E	0.16	875	T		1 Br
E - O	0.11	208	C		
E - J	0.29	1571	T		
J - O	0.07	334	C		

Defl	Value	Member
TL Defl	-0.20"	I - H L/999
LL Defl	-0.10"	I - H L/999
Shear // Grain		in A - M 0.24

Plates for each ply each face.  
PLAYING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	Lock	20 Ga	Gross Area		
Plate - LOCK 20 Ga <td></td> <td></td> <td>Gross Area</td>			Gross Area		
Plate - RHS 20 Ga <td></td> <td></td> <td>Gross Area</td>			Gross Area		
Jt Type	Plt Size	X	Y	JSI	
A LOCK	4.0x	6.0	0.1	0.1	0.66
M LOCK	2.0x	4.0	Ctr	Ctr	0.23
B LOCK	5.0x	5.0	0.5-3.4	0.88	
C LOCK	5.0x	5.0	0.5-3.4	0.90	
N LOCK	2.0x	4.0	Ctr	Ctr	0.23
O LOCK	5.0x	7.0	0.3	0.5	0.68
D LOCK	4.0x	6.0	-0.5	0.4	0.66
I LOCK	4.0x	6.0	Ctr	Ctr	0.66
H LOCK	2.0x	4.0	Ctr	Ctr	0.58
G LOCK	5.0x	9.0	Ctr	0.8	0.65
P LOCK	3.0x	7.0	Ctr	Ctr	0.37
E LOCK	5.0x	9.0	Ctr	0.7	0.72
F LOCK	2.0x	4.0	Ctr	Ctr	0.58
J LOCK	3.0x	7.0	Ctr	Ctr	0.50

REVIEWED BY:  
Robbins Engineering, Inc.

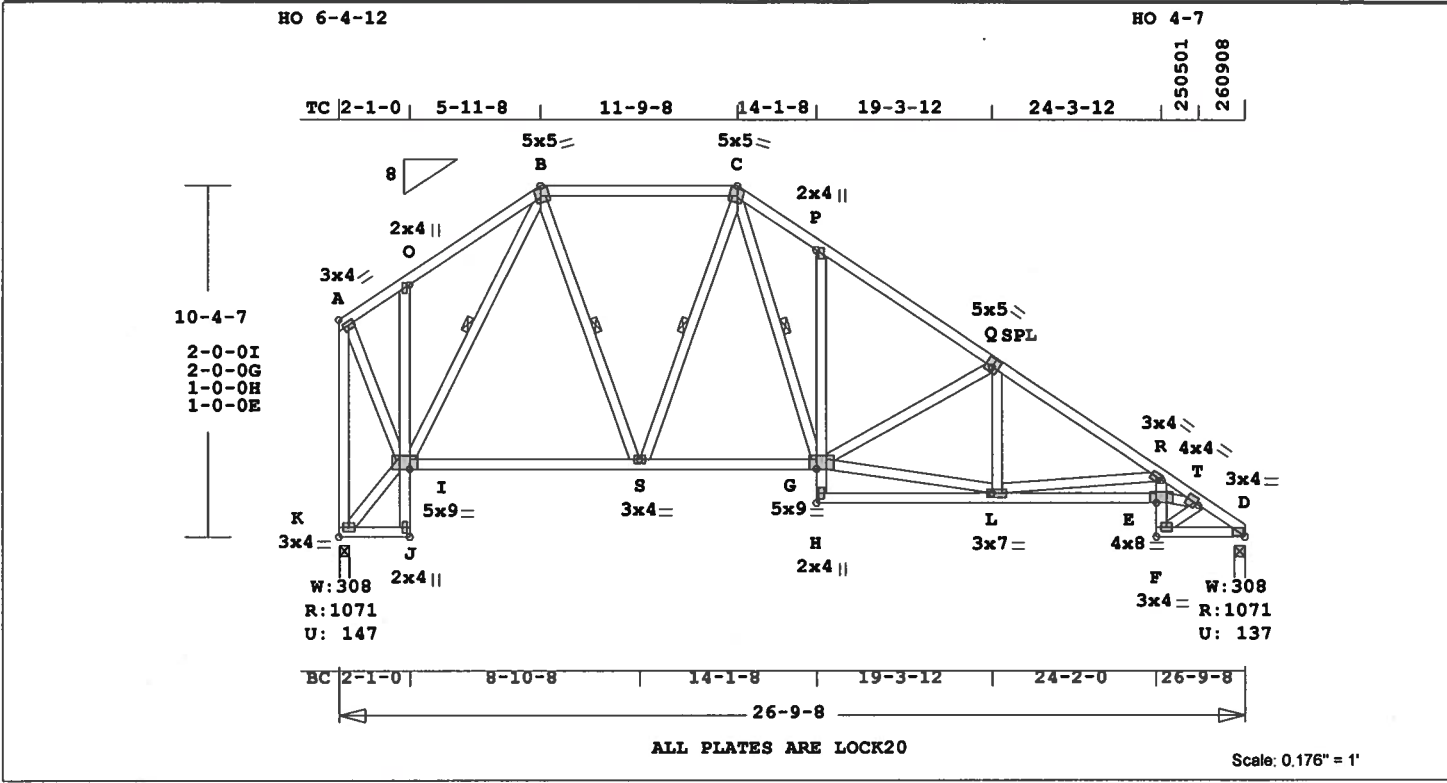
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 1812 Lbs  
Max tens. force 1571 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 266.5 LBS

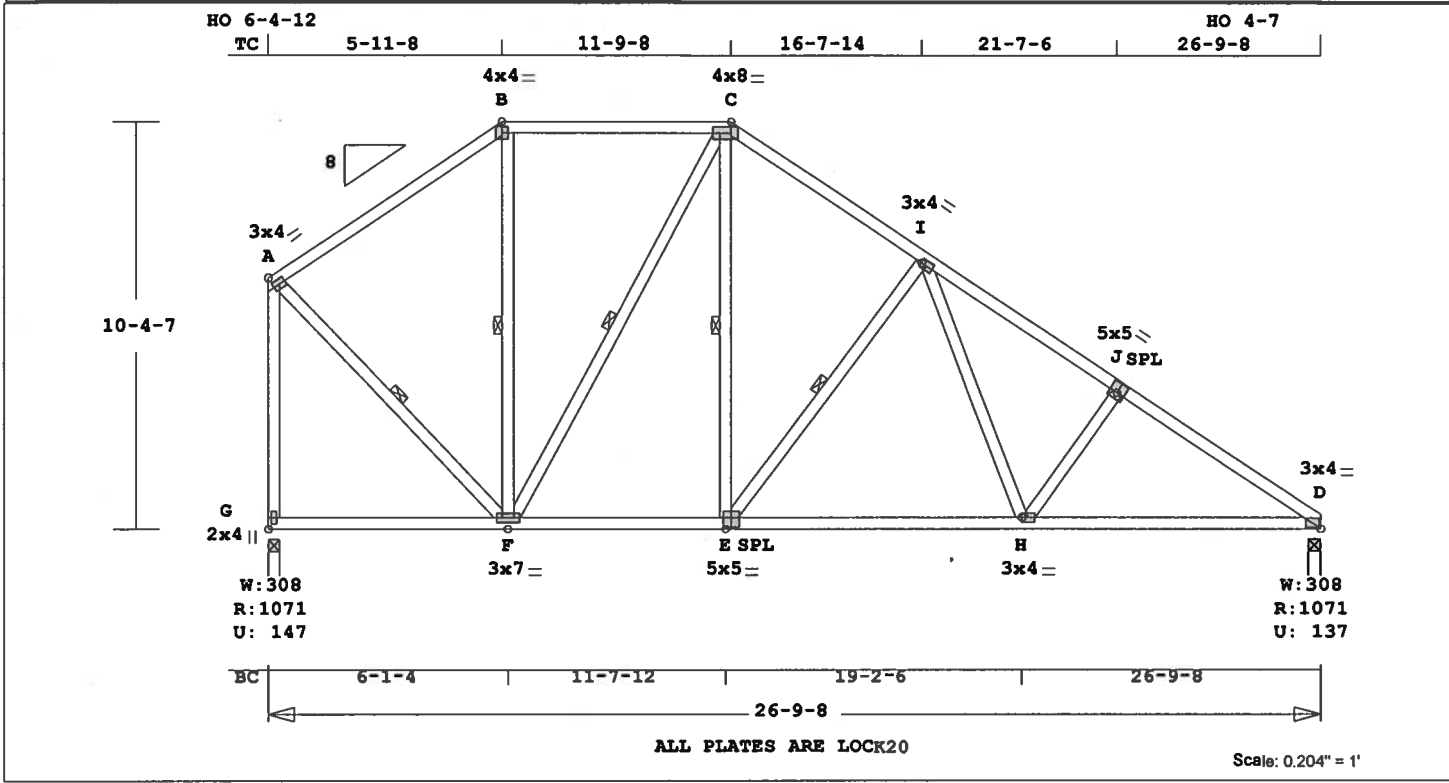
Online Plus -- Version 21.0.032  
 RUN DATE: 05-JUN-07

<p>CSI -Size- ---Lumber---</p> <p>TC 0.33 2x 4 SP-#2          BC 0.75 2x 4 SP-#2          CW 0.58 2x 4 SP-#2          WB 0.51 2x 4 SP-#2</p> <p>Brace truss as follows:</p> <table border="0"> <tr><td>O.C.</td><td>From</td><td>To</td></tr> <tr><td>TC Cont.</td><td>0-0-0</td><td>5-11-8</td></tr> <tr><td>TC</td><td>24.0"</td><td>5-11-8 11-9-8</td></tr> <tr><td>TC Cont.</td><td>11-9-8</td><td>26-9-8</td></tr> <tr><td>BC Cont.</td><td>0-0-0</td><td>26-9-8</td></tr> </table> <p>WB 1 rows CLB on I -B          WB 1 rows CLB on B -S          WB 1 rows CLB on S -C          WB 1 rows CLB on C -G</p> <p>Attach CLB with (2)-10d nails at each web.</p> <table border="0"> <tr><td>psf-Ld</td><td>Dead</td><td>Live</td></tr> <tr><td>TC</td><td>10.0</td><td>20.0</td></tr> <tr><td>BC</td><td>10.0</td><td>0.0</td></tr> <tr><td>TC+BC</td><td>20.0</td><td>20.0</td></tr> <tr><td>Total</td><td>40.0</td><td>Spacing 24.0"</td></tr> </table> <p>Lumber Duration Factor 1.25          Plate Duration Factor 1.25          TC Fb=1.15 Fc=1.10 Ft=1.10          BC Fb=1.10 Fc=1.10 Ft=1.10</p> <p>Total Load Reactions (Lbs)</p> <table border="0"> <tr><td>Jt</td><td>Down</td><td>Uplift</td><td>Horiz</td></tr> <tr><td>K</td><td>1072</td><td>148</td><td>305 R</td></tr> <tr><td>D</td><td>1072</td><td>138</td><td>204 R</td></tr> </table> <table border="0"> <tr><td>Jt</td><td>Brg</td><td>Size</td><td>Required</td></tr> <tr><td>K</td><td></td><td>3.5"</td><td>1.5"</td></tr> <tr><td>D</td><td></td><td>3.5"</td><td>1.5"</td></tr> </table> <p>Plus 9 Wind Load Case(s)          Plus 1 UBC LL Load Case(s)</p> <table border="0"> <tr><td>Membr</td><td>CSI</td><td>P</td><td>Lbs</td><td>Axl</td><td>CSI</td><td>Bnd</td></tr> </table> <p>-----Top Chords-----</p> <table border="0"> <tr><td>A</td><td>O</td><td>0.13</td><td>395</td><td>C</td><td>0.00</td><td>0.13</td></tr> <tr><td>O</td><td>B</td><td>0.13</td><td>433</td><td>C</td><td>0.00</td><td>0.13</td></tr> <tr><td>B</td><td>C</td><td>0.29</td><td>760</td><td>C</td><td>0.00</td><td>0.29</td></tr> <tr><td>C</td><td>P</td><td>0.20</td><td>1308</td><td>C</td><td>0.01</td><td>0.19</td></tr> <tr><td>P</td><td>Q</td><td>0.33</td><td>1330</td><td>C</td><td>0.01</td><td>0.32</td></tr> <tr><td>Q</td><td>R</td><td>0.33</td><td>1749</td><td>C</td><td>0.01</td><td>0.32</td></tr> <tr><td>R</td><td>T</td><td>0.21</td><td>3388</td><td>C</td><td>0.07</td><td>0.14</td></tr> <tr><td>T</td><td>D</td><td>0.14</td><td>1532</td><td>C</td><td>0.01</td><td>0.13</td></tr> </table> <p>-----Bottom Chords-----</p> <table border="0"> <tr><td>K</td><td>J</td><td>0.02</td><td>39</td><td>C</td><td>0.00</td><td>0.02</td></tr> <tr><td>I</td><td>S</td><td>0.30</td><td>603</td><td>T</td><td>0.06</td><td>0.24</td></tr> <tr><td>S</td><td>G</td><td>0.32</td><td>862</td><td>T</td><td>0.08</td><td>0.24</td></tr> <tr><td>H</td><td>L</td><td>0.21</td><td>33</td><td>T</td><td>0.00</td><td>0.21</td></tr> </table>	O.C.	From	To	TC Cont.	0-0-0	5-11-8	TC	24.0"	5-11-8 11-9-8	TC Cont.	11-9-8	26-9-8	BC Cont.	0-0-0	26-9-8	psf-Ld	Dead	Live	TC	10.0	20.0	BC	10.0	0.0	TC+BC	20.0	20.0	Total	40.0	Spacing 24.0"	Jt	Down	Uplift	Horiz	K	1072	148	305 R	D	1072	138	204 R	Jt	Brg	Size	Required	K		3.5"	1.5"	D		3.5"	1.5"	Membr	CSI	P	Lbs	Axl	CSI	Bnd	A	O	0.13	395	C	0.00	0.13	O	B	0.13	433	C	0.00	0.13	B	C	0.29	760	C	0.00	0.29	C	P	0.20	1308	C	0.01	0.19	P	Q	0.33	1330	C	0.01	0.32	Q	R	0.33	1749	C	0.01	0.32	R	T	0.21	3388	C	0.07	0.14	T	D	0.14	1532	C	0.01	0.13	K	J	0.02	39	C	0.00	0.02	I	S	0.30	603	T	0.06	0.24	S	G	0.32	862	T	0.08	0.24	H	L	0.21	33	T	0.00	0.21	<p>6904 Parke East Blvd.          Tampa, FL 33610</p> <p>REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.</p> <p>NOTES:          Trusses Manufactured by: Mayo Truss Co. Inc.          Analysis Conforms To: FBC2004          Design checked for 10 psf non-concurrent LL on BC.          Wind Loads - ANSI / ASCE 7-02          Truss is designed as Components and Claddings* for Exterior zone location.          Wind Speed: 110 mph          Mean Roof Height: 15-0          Exposure Category: B          Occupancy Factor : 1.00          Building Type: Enclosed          TC Dead Load: 5.0 psf          BC Dead Load: 5.0 psf          Max comp. force 3388 Lbs          Max tens. force 2952 Lbs          Quality Control Factor 1.25</p> <p>Plates for each ply each face.          PLATING CONFORMS TO TPI.          REPORTS: SBCCI 9761          ROBBINS ENGINEERING, INC.          BASED ON SP LUMBER          USING GROSS AREA TEST.</p> <table border="0"> <tr><td>Plate</td><td>-</td><td>LOCK 20</td><td>Ga,</td><td>Gross Area</td></tr> <tr><td>Plate</td><td>-</td><td>RHS</td><td>20</td><td>Ga,</td><td>Gross Area</td></tr> </table> <table border="0"> <tr><td>Jt</td><td>Type</td><td>Plt</td><td>Size</td><td>X</td><td>Y</td><td>JSI</td></tr> <tr><td>A</td><td>LOCK</td><td>3.0x</td><td>4.0</td><td>Ctr</td><td>Ctr</td><td>0.78</td></tr> <tr><td>O</td><td>LOCK</td><td>2.0x</td><td>4.0</td><td>Ctr</td><td>Ctr</td><td>0.23</td></tr> <tr><td>B</td><td>LOCK</td><td>5.0x</td><td>5.0</td><td>0.5-3.4</td><td>0.82</td><td></td></tr> <tr><td>C</td><td>LOCK</td><td>5.0x</td><td>5.0</td><td>0.5-3.4</td><td>0.84</td><td></td></tr> <tr><td>P</td><td>LOCK</td><td>2.0x</td><td>4.0</td><td>Ctr</td><td>Ctr</td><td>0.23</td></tr> <tr><td>Q</td><td>LOCK</td><td>5.0x</td><td>5.0</td><td>0.3</td><td>0.5</td><td>0.63</td></tr> <tr><td>R</td><td>LOCK</td><td>3.0x</td><td>4.0</td><td>Ctr</td><td>Ctr</td><td>0.84</td></tr> <tr><td>T</td><td>LOCK</td><td>4.0x</td><td>4.0</td><td>0-0.8</td><td>0.6</td><td>0.88</td></tr> <tr><td>D</td><td>LOCK</td><td>3.0x</td><td>4.0</td><td>Ctr</td><td>Ctr</td><td>0.83</td></tr> <tr><td>K</td><td>LOCK</td><td>3.0x</td><td>4.0</td><td>Ctr</td><td>Ctr</td><td>0.78</td></tr> <tr><td>J</td><td>LOCK</td><td>2.0x</td><td>4.0</td><td>Ctr</td><td>Ctr</td><td>0.58</td></tr> <tr><td>I</td><td>LOCK</td><td>5.0x</td><td>9.0</td><td>Ctr</td><td>0.8</td><td>0.78</td></tr> <tr><td>S</td><td>LOCK</td><td>3.0x</td><td>4.0</td><td>Ctr</td><td>Ctr</td><td>0.62</td></tr> <tr><td>G</td><td>LOCK</td><td>5.0x</td><td>9.0</td><td>Ctr</td><td>0.8</td><td>0.67</td></tr> <tr><td>H</td><td>LOCK</td><td>2.0x</td><td>4.0</td><td>Ctr</td><td>Ctr</td><td>0.58</td></tr> <tr><td>L</td><td>LOCK</td><td>3.0x</td><td>7.0</td><td>Ctr</td><td>Ctr</td><td>1.00</td></tr> <tr><td>E</td><td>LOCK</td><td>4.0x</td><td>8.0</td><td>Ctr</td><td>0.2</td><td>0.59</td></tr> <tr><td>F</td><td>LOCK</td><td>3.0x</td><td>4.0</td><td>Ctr</td><td>Ctr</td><td>0.98</td></tr> </table> <p>TL Defl -0.22" in L -E L/999          LL Defl -0.11" in L -E L/999          Shear // Grain in F -E 0.29</p>	Plate	-	LOCK 20	Ga,	Gross Area	Plate	-	RHS	20	Ga,	Gross Area	Jt	Type	Plt	Size	X	Y	JSI	A	LOCK	3.0x	4.0	Ctr	Ctr	0.78	O	LOCK	2.0x	4.0	Ctr	Ctr	0.23	B	LOCK	5.0x	5.0	0.5-3.4	0.82		C	LOCK	5.0x	5.0	0.5-3.4	0.84		P	LOCK	2.0x	4.0	Ctr	Ctr	0.23	Q	LOCK	5.0x	5.0	0.3	0.5	0.63	R	LOCK	3.0x	4.0	Ctr	Ctr	0.84	T	LOCK	4.0x	4.0	0-0.8	0.6	0.88	D	LOCK	3.0x	4.0	Ctr	Ctr	0.83	K	LOCK	3.0x	4.0	Ctr	Ctr	0.78	J	LOCK	2.0x	4.0	Ctr	Ctr	0.58	I	LOCK	5.0x	9.0	Ctr	0.8	0.78	S	LOCK	3.0x	4.0	Ctr	Ctr	0.62	G	LOCK	5.0x	9.0	Ctr	0.8	0.67	H	LOCK	2.0x	4.0	Ctr	Ctr	0.58	L	LOCK	3.0x	7.0	Ctr	Ctr	1.00	E	LOCK	4.0x	8.0	Ctr	0.2	0.59	F	LOCK	3.0x	4.0	Ctr	Ctr	0.98
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Job <b>ELIXSON-WILL</b>	Mark <b>A10</b>	Quan 1	Type HIPP	Span '260908'	P1-H1 8	Left OH 0	Right OH 0	Engineering <b>T2566260</b>
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 230.4 LBS  
 Online Plus -- Version 21.0.032  
 RUN DATE: 05-JUN-07

CSI	-Size-	-----Lumber-----
TC	0.36	2x 4 SP-#2
BC	0.43	2x 4 SP-#2
WB	0.49	2x 4 SP-#2

Brace truss as follows:  
 O.C. From To  
 TC Cont. 0-0-0 5-11-8  
 TC 24.0" 5-11-8 11-9-8  
 TC Cont. 11-9-8 26-9-8  
 BC Cont. 0-0-0 26-9-8  
 WB 1 rows CLB on A -F  
 WB 1 rows CLB on F -B  
 WB 1 rows CLB on F -C  
 WB 1 rows CLB on E -C  
 WB 1 rows CLB on E -I  
 Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15 Fc=1.10 Ft=1.10		
BC Fb=1.10 Fc=1.10 Ft=1.10		

Total Load Reactions (Lbs)			
Jt	Down	Uplift	Horiz-
G	1072	148 U	304 R
D	1072	138 U	203 R

Jt	Brg Size	Required
G	3.5"	1.5"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)  
 Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -B	0.36	596 C	0.00	0.36
B -C	0.27	505 C	0.00	0.27

Bottom Chords					
G -F	0.20	224 T	0.00	0.20	
F -E	0.29	696 T	0.07	0.22	
E -H	0.41	983 T	0.10	0.31	
H -D	0.43	1261 T	0.12	0.31	
Webs					
G -A	0.49	1020 C	WindLd		
A -F	0.13	727 T		1 Br	
F -B	0.01	118 T		1 Br	
F -C	0.16	396 C		1 Br	
E -C	0.09	510 T		1 Br	
E -I	0.11	470 C		1 Br	
I -H	0.07	420 T			
H -J	0.05	259 C			
TL Defl	-0.16"	in E -H		L/999	
LL Defl	-0.07"	in E -H		L/999	
Shear // Grain		in B -C		0.22	

Plates for each ply each face.  
 PLATING CONFORMS TO TPI.  
 REPORTS: SBCCI 9761  
 ROBBINS ENGINEERING, INC.  
 BASED ON SP LUMBER  
 USING GROSS AREA TEST.  
 Plate - LOCK 20 Ga, Gross Area  
 Plate - RHS 20 Ga, Gross Area  

Jt Type	Plt Size	X	Y	JSI
A	LOCK 3.0x 4.0	Ctr	Ctr	0.78
B	LOCK 4.0x 4.0	Ctr	Ctr	0.98
C	LOCK 4.0x 8.0	Ctr	Ctr	0.98
I	LOCK 3.0x 4.0	Ctr	Ctr	0.59
J	LOCK 5.0x 5.0	0.3	0.5	0.63
D	LOCK 3.0x 4.0	Ctr	Ctr	0.83
G	LOCK 2.0x 4.0	Ctr	Ctr	0.40
F	LOCK 3.0x 7.0	Ctr	Ctr	0.53
E	LOCK 5.0x 5.0	Ctr	-0.5	0.65
H	LOCK 3.0x 4.0	Ctr	Ctr	0.63

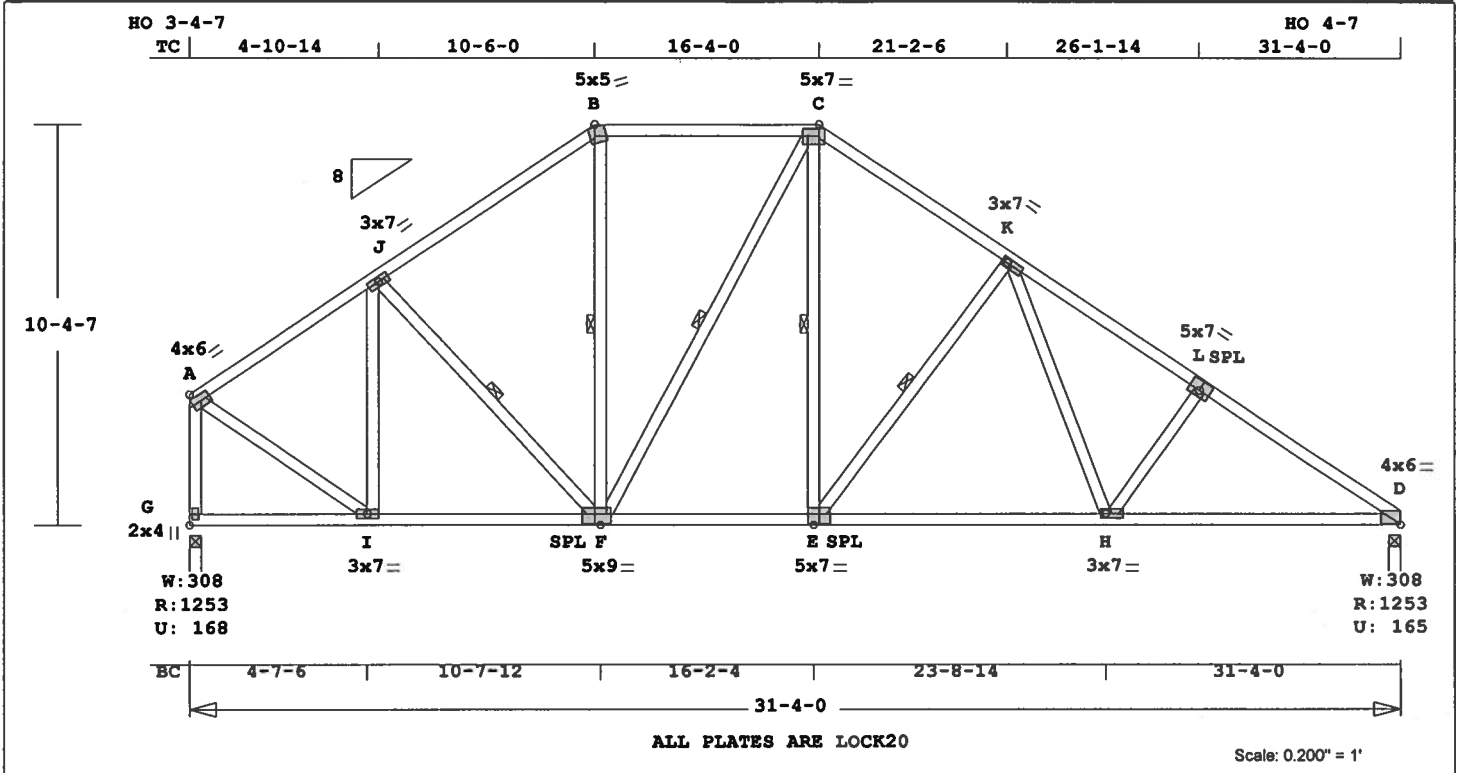
REVIEWED BY:  
 Robbins Engineering, Inc.  
 6904 Parke East Blvd.  
 Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.  
 NOTES:  
 Trusses Manufactured by: Mayo Truss Co. Inc.  
 Analysis Conforms To: FBC2004  
 Design checked for 10 psf non-concurrent LL on BC.  
 Wind Loads - ANSI / ASCE 7-02  
 Truss is designed as Components and Claddings\* for Exterior zone location.  
 Wind Speed: 110 mph  
 Mean Roof Height: 15-0  
 Exposure Category: B  
 Occupancy Factor : 1.00  
 Building Type: Enclosed  
 TC Dead Load: 5.0 psf  
 BC Dead Load: 5.0 psf  
 Max comp. force 1512 Lbs  
 Max tens. force 1261 Lbs  
 Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
 Robbins Engineering  
 6904 Parke East Blvd  
 Tampa, FL, 33610  
 FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>A11</b>	Quan 1	Type HIPP	Span '310400'	Pl-H1 8	Left OH 0	Right OH 0	Engineering <b>T2566261</b>
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 264.7 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

K -L 0.23 1657 C 0.08 0.15  
L -D 0.21 1827 C 0.07 0.14

Tampa, FL 33610

CSI	-Size-	-----Lumber-----
TC	0.32	2x 4 SP-#2
BC	0.45	2x 4 SP-#2
WB	0.21	2x 4 SP-#2

-----Bottom Chords-----

G -I	0.17	225 T	0.00	0.17
I -F	0.25	818 T	0.08	0.17
F -E	0.33	967 T	0.10	0.23
E -H	0.42	1249 T	0.12	0.30
H -D	0.45	1520 T	0.15	0.30

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	10- 6- 0
TC Cont.	10- 6- 0	16- 4- 0
TC Cont.	16- 4- 0	31- 4- 0
BC Cont.	0- 0- 0	31- 4- 0

WB 1 rows CLB on J -F  
WB 1 rows CLB on F -B  
WB 1 rows CLB on F -C  
WB 1 rows CLB on E -C  
WB 1 rows CLB on E -K

Attach CLB with (2)-10d nails at each web.

-----Webs-----

G -A	0.18	1215 C	WindLd
A -I	0.18	990 T	
I -J	0.21	441 C	
J -F	0.02	108 T	1 Br
F -B	0.04	310 T	1 Br
F -C	0.08	200 C	1 Br
E -C	0.09	514 T	1 Br
E -K	0.11	461 C	1 Br
K -H	0.06	411 T	
H -L	0.05	256 C	

Trusses Manufactured by: Mayo Truss Co. Inc.  
Analysis Conforms To: FBC2004  
Design checked for 10 psf non-concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as Components and Claddings\* for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 1827 Lbs  
Max tens. force 1520 Lbs  
Quality Control Factor 1.25

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0

Total 40.0 Spacing 24.0"  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

TL Defl -0.18" in E -H L/999  
LL Defl -0.07" in E -H L/999  
Shear // Grain in B -C 0.21

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x	6.0	0.1	0.1
J	LOCK	3.0x	7.0	Ctr	Ctr
B	LOCK	5.0x	5.0	0.9-3.1	0.60
C	LOCK	5.0x	7.0	Ctr-0.2	0.90
K	LOCK	3.0x	7.0	Ctr	Ctr
L	LOCK	5.0x	7.0	0.3	0.5
D	LOCK	4.0x	6.0-0.5	0.4	0.66
G	LOCK	2.0x	4.0	Ctr	Ctr
I	LOCK	3.0x	7.0	Ctr	Ctr
F	LOCK	5.0x	9.0	0.5-0.5	0.70
E	LOCK	5.0x	7.0	Ctr-0.5	0.70
H	LOCK	3.0x	7.0	Ctr	Ctr

Jt	Down	Uplift	Horiz-
G	1253	168 U	266 R
D	1253	166 U	214 R

Jt	Brg Size	Required
G	3.5"	1.5"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

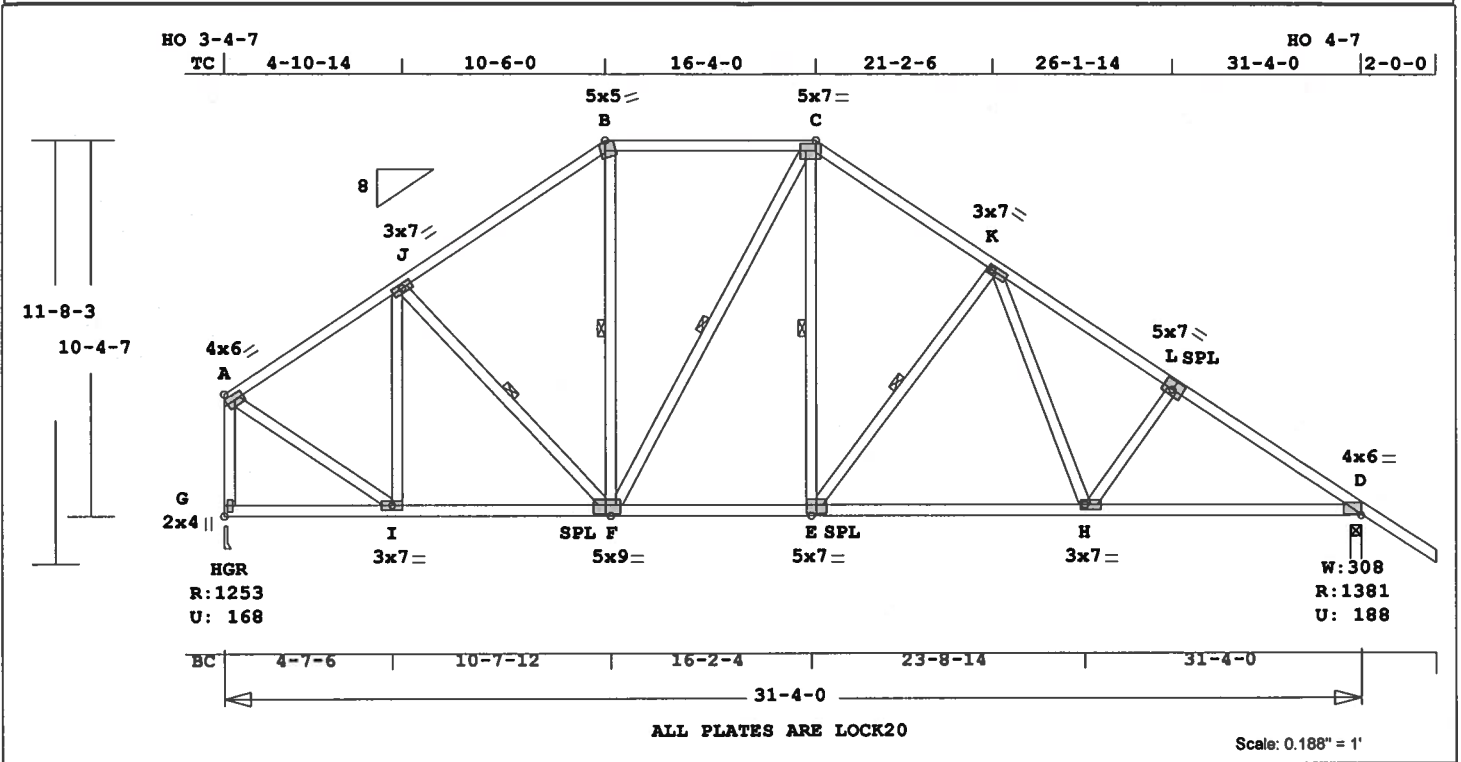
Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -J	0.29	963 C	0.00	0.29	
J -B	0.29	1051 C	0.00	0.29	
B -C	0.32	870 C	0.01	0.31	
C -K	0.22	1167 C	0.00	0.22	

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>A12</b>	Quan 1	Type HIPP	Span 310400'	P1-H1 8	Left OH 0	Right OH 2- 0- 0	Engineering <b>T2566262</b>
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 269.3 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI	-Size-	---Lumber---
TC	0.32	2x 4 SP-#2
BC	0.45	2x 4 SP-#2
WB	0.21	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0 10- 6- 0	10- 6- 0
TC	24.0"	10- 6- 0 16- 4- 0
TC Cont.	16- 4- 0 31- 4- 0	31- 4- 0
BC Cont.	0- 0- 0 31- 4- 0	31- 4- 0
WB	1 rows CLB on J -F	
WB	1 rows CLB on F -B	
WB	1 rows CLB on F -C	
WB	1 rows CLB on E -C	
WB	1 rows CLB on E -K	

Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber	Duration Factor 1.25	
Plate	Duration Factor 1.25	
TC	Fb=1.15	Fc=1.10 Ft=1.10
BC	Fb=1.10	Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
G	1253	168 U	266 R
D	1381	188 U	214 R

Jt	Brg Size	Required
G	3.5"	1.5"
D	3.5"	1.6"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
---Top Chords---			
A	-J	0.29	963 C 0.00 0.29
J	-B	0.29	1051 C 0.00 0.29
B	-C	0.32	870 C 0.01 0.31
C	-K	0.22	1167 C 0.00 0.22

K	-L	0.23	1657 C	0.08	0.15
L	-D	0.21	1827 C	0.07	0.14
-----Bottom Chords-----					
G	-I	0.17	225 T	0.00	0.17
I	-F	0.25	818 T	0.08	0.17
F	-E	0.33	967 T	0.10	0.23
E	-H	0.42	1248 T	0.12	0.30
H	-D	0.45	1520 T	0.15	0.30
-----Webs-----					
G	-A	0.18	1215 C	WindLd	
A	-I	0.18	990 T		
I	-J	0.21	441 C		
J	-F	0.02	108 T		1 Br
F	-B	0.04	310 T		1 Br
F	-C	0.08	200 C		1 Br
E	-C	0.09	514 T		1 Br
E	-K	0.11	461 C		1 Br
K	-H	0.06	411 T		
H	-L	0.05	256 C		

TL Defl	-0.18"	in E -H	L/999
LL Defl	-0.07"	in E -H	L/999
Shear // Grain		in B -C	0.21

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	- LOCK 20 Ga, Gross Area					
Plate	- RHS 20 Ga, Gross Area					
Jt	Type	Plt Size	X	Y	JSI	
A	LOCK	4.0x	6.0	0.1	0.1	0.66
J	LOCK	3.0x	7.0	Ctr	Ctr	0.42
B	LOCK	5.0x	5.0	0.9-3.1	0.60	
C	LOCK	5.0x	7.0	Ctr-0.2	0.90	
K	LOCK	3.0x	7.0	Ctr	Ctr	0.41
L	LOCK	5.0x	7.0	0.3	0.5	0.68
D	LOCK	4.0x	6.0-0.5	0.4	0.66	
G	LOCK	2.0x	4.0	Ctr	Ctr	0.44
I	LOCK	3.0x	7.0	Ctr	Ctr	0.41
F	LOCK	5.0x	9.0	0.5-0.5	0.70	
E	LOCK	5.0x	7.0	Ctr-0.5	0.70	
H	LOCK	3.0x	7.0	Ctr	Ctr	0.40

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.

Tampa, FL 33610

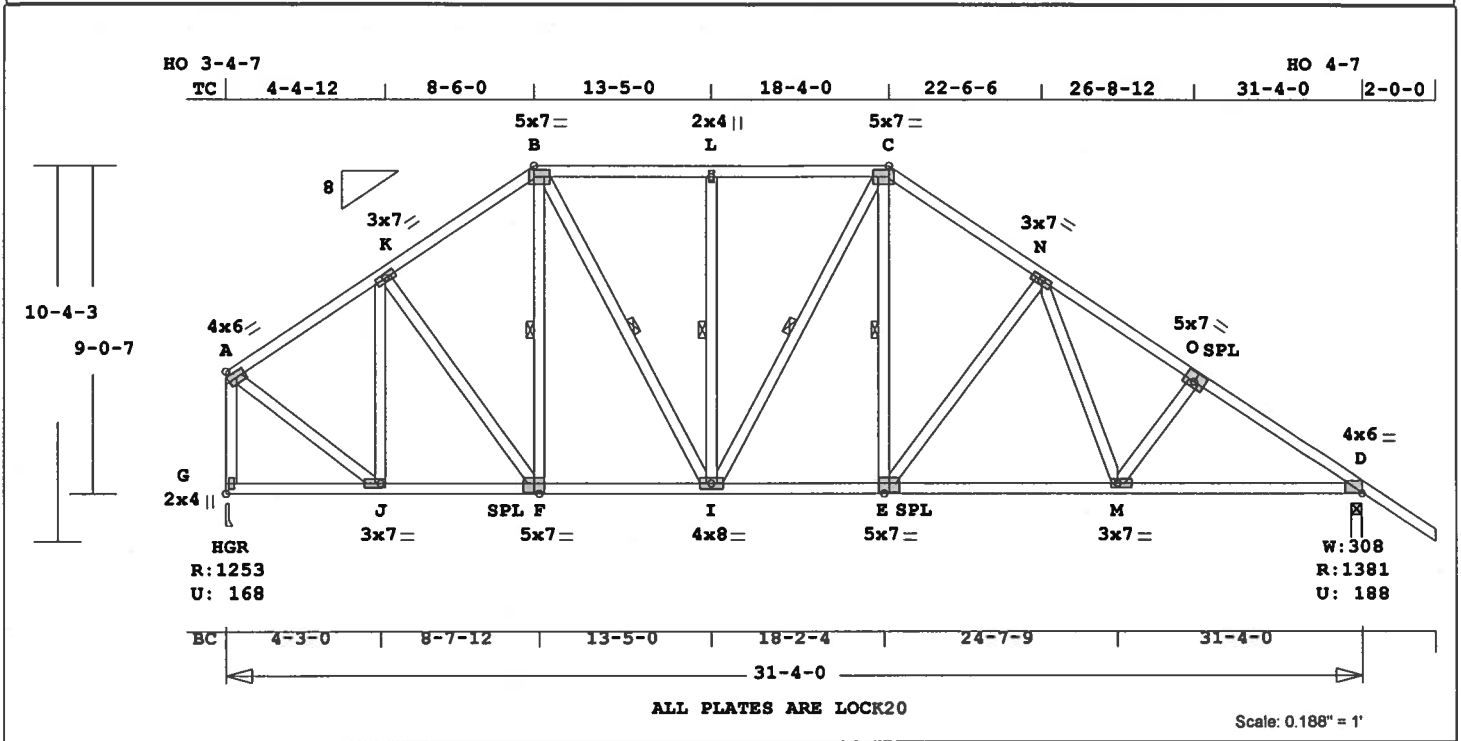
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor: 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 1827 Lbs  
Max tens. force 1520 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>A13</b>	Quan 1	Type HIPP	Span '310400'	P1-H1 8	Left OH 0	Right OH 2- 0- 0	Engineering <b>T2566263</b>
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 285.5 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI -Size-	Lumber	SP-#2
TC 0.25	2x 4	SP-#2
BC 0.36	2x 4	SP-#2
WB 0.25	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	31- 4- 0
BC Cont.	0- 0- 0	31- 4- 0
WB 1 rows	CLB on F -B	
WB 1 rows	CLB on B -I	
WB 1 rows	CLB on I -L	
WB 1 rows	CLB on I -C	
WB 1 rows	CLB on E -C	

Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
G	1253	168 U	236 R
D	1381	188 U	183 R

Jt	Brg Size	Required
G	3.5"	1.5"
D	3.5"	1.6"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd

---Top Chords---				
A -K	0.18	916 C	0.00	0.18
K -B	0.19	1067 C	0.06	0.13
B -L	0.25	1098 C	0.00	0.25
L -C	0.25	1098 C	0.00	0.25
C -N	0.19	1302 C	0.01	0.18
N -O	0.19	1702 C	0.08	0.11
O -D	0.18	1851 C	0.08	0.10
---Bottom Chords---				
G -J	0.10	194 T	0.00	0.10

J -F	0.18	774 T	0.08	0.10
F -I	0.20	885 T	0.14	0.06
I -E	0.28	1081 T	0.18	0.10
E -M	0.34	1316 T	0.13	0.21
M -D	0.36	1536 T	0.25	0.11
-----Webs-----				
G -A	0.18	1216 C	WindLd	
A -J	0.18	975 T		
J -K	0.22	501 C		
K -F	0.03	185 T		
F -B	0.01	74 T		1 Br
B -I	0.08	444 T		1 Br
I -L	0.08	330 C		1 Br
I -C	0.02	89 T		1 Br
E -C	0.07	439 T		1 Br
E -N	0.25	389 C		
N -M	0.05	343 T		
M -O	0.04	219 C		

TL Defl	-0.13"	in E -M	L/999
LL Defl	-0.06"	in E -M	L/999
Shear // Grain		in B -L	0.21

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 4.0x 6.0 0.1 0.1 0.66  
K LOCK 3.0x 7.0 Ctr Ctr 0.44  
B LOCK 5.0x 7.0 Ctr-0.2 0.90  
L LOCK 2.0x 4.0 Ctr Ctr 0.42  
C LOCK 5.0x 7.0 Ctr-0.2 0.90  
N LOCK 3.0x 7.0 Ctr Ctr 0.52  
O LOCK 5.0x 7.0 0.3 0.5 0.68  
D LOCK 4.0x 6.0-0.5 0.4 0.66  
G LOCK 2.0x 4.0 Ctr Ctr 0.44  
J LOCK 3.0x 7.0 Ctr Ctr 0.42  
F LOCK 5.0x 7.0 Ctr-0.5 0.70  
I LOCK 4.0x 8.0 Ctr Ctr 0.46  
E LOCK 5.0x 7.0 Ctr-0.5 0.70  
M LOCK 3.0x 7.0 1.1 Ctr 0.54

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:

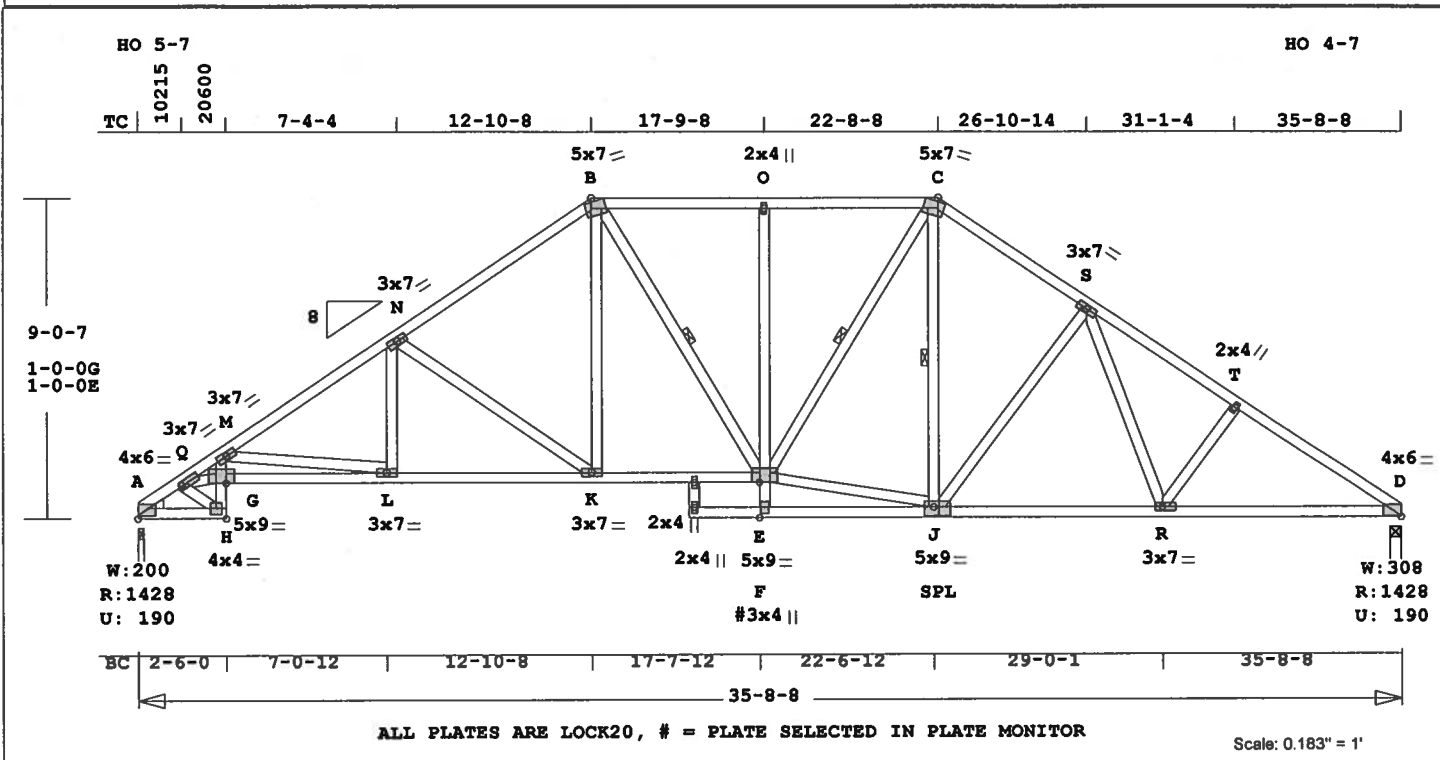
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor: 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 1851 Lbs  
Max tens. force 1536 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>A14</b>	Quan 1	Type SP	Span 350808	Pl-H1 8	Left OH 0	Right OH 0	Engineering T2566264
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 305.1 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI	Size	Lumber
TC	0.39 2x 4	SP-#2
BC	0.97 2x 4	SP-#2
CW	0.73 2x 4	SP-#2
WB	0.62 2x 4	SP-#2
PB	--- 2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0-0-0	35-8-8
BC Cont.	0-0-0	35-8-8
WB	1 rows CLB	on B - E
WB	1 rows CLB	on E - C
WB	1 rows CLB	on J - C

Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1428	190 U	199 R
D	1428	190 U	199 R

Jt	Brg Size	Required
A	2.0"	1.7"
D	3.5"	1.7"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	F Lbs	Axl-C81-Bnd
-----Top Chords-----			
A-Q	0.32	1928 C	0.02 0.30
Q-M	0.33	4390 C	0.13 0.20
M-N	0.39	2494 C	0.07 0.32
N-B	0.33	1823 C	0.02 0.31
B-O	0.26	1637 C	0.01 0.25
O-C	0.26	1635 C	0.01 0.25
C-S	0.19	1614 C	0.08 0.11
S-T	0.20	2003 C	0.09 0.11
T-D	0.20	2151 C	0.03 0.17
-----Bottom Chords-----			
A-H	0.35	1433 T	0.24 0.11
G-L	0.97	3821 T	0.64 0.33
L-K	0.51	2098 T	0.35 0.16
K-E	0.32	1507 T	0.25 0.07
F-J	0.19	18 C	0.00 0.19
J-R	0.38	1572 T	0.26 0.12

R-D	0.42	1783 T	0.29	0.13
-----Chord-Webs-----				
H-G	0.73	792 T	0.14	0.59
G-M	0.42	1129 T	0.20	0.22
F-E	0.03	69 T	0.00	0.03
E-O	0.11	331 C	0.06	0.05
-----Webs-----				
Q-H	0.13	1403 C		
Q-G	0.62	3399 T		
M-L	0.52	1740 C		
L-N	0.07	454 T		
N-K	0.42	707 C		
K-B	0.09	511 T		
B-E	0.04	248 T		1 Br
E-C	0.10	554 T		1 Br
E-J	0.25	1355 T		
J-C	0.03	241 T		1 Br
J-S	0.25	384 C		
S-R	0.05	330 T		
R-T	0.03	215 C		

TL Defl	-0.31"	in G-L	L/999
LL Defl	-0.15"	in G-L	L/999
Shear // Grain	0.43	in A-Q	

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	1.3	0.7	0.92
Q	LOCK	3.0x 7.0	0.8	0.6	0.95
M	LOCK	3.0x 7.0	Ctr	Ctr	0.50
N	LOCK	3.0x 7.0	Ctr	Ctr	0.44
B	LOCK	5.0x 7.0	1.6-3.4	0.70	
O	LOCK	2.0x 4.0	Ctr	Ctr	0.40
C	LOCK	5.0x 7.0	1.6-3.4	0.70	
S	LOCK	3.0x 7.0	Ctr	Ctr	0.53
T	LOCK	2.0x 4.0	Ctr	Ctr	0.45
D	LOCK	4.0x 6.0	0.5	0.4	0.70
H	LOCK	4.0x 4.0	Ctr	Ctr	0.84
G	LOCK	5.0x 9.0	Ctr	0.8	0.64
L	LOCK	3.0x 7.0	Ctr	Ctr	0.85
K	LOCK	3.0x 7.0	Ctr	Ctr	0.42
E	LOCK	5.0x 9.0	Ctr	0.7	0.67
F#	LOCK	3.0x 4.0	Ctr	Ctr	0.58
J	LOCK	5.0x 9.0	0.5-0.5	0.78	
R	LOCK	3.0x 7.0	1.1	Ctr	0.56

# = Plate Monitor used

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

Design checked for 10 psf non-concurrent LL on BC.

NOTE: USER MODIFIED PLATES

This design may have plates selected through a plate monitor.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings\* for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

Max comp. force 4390 Lbs

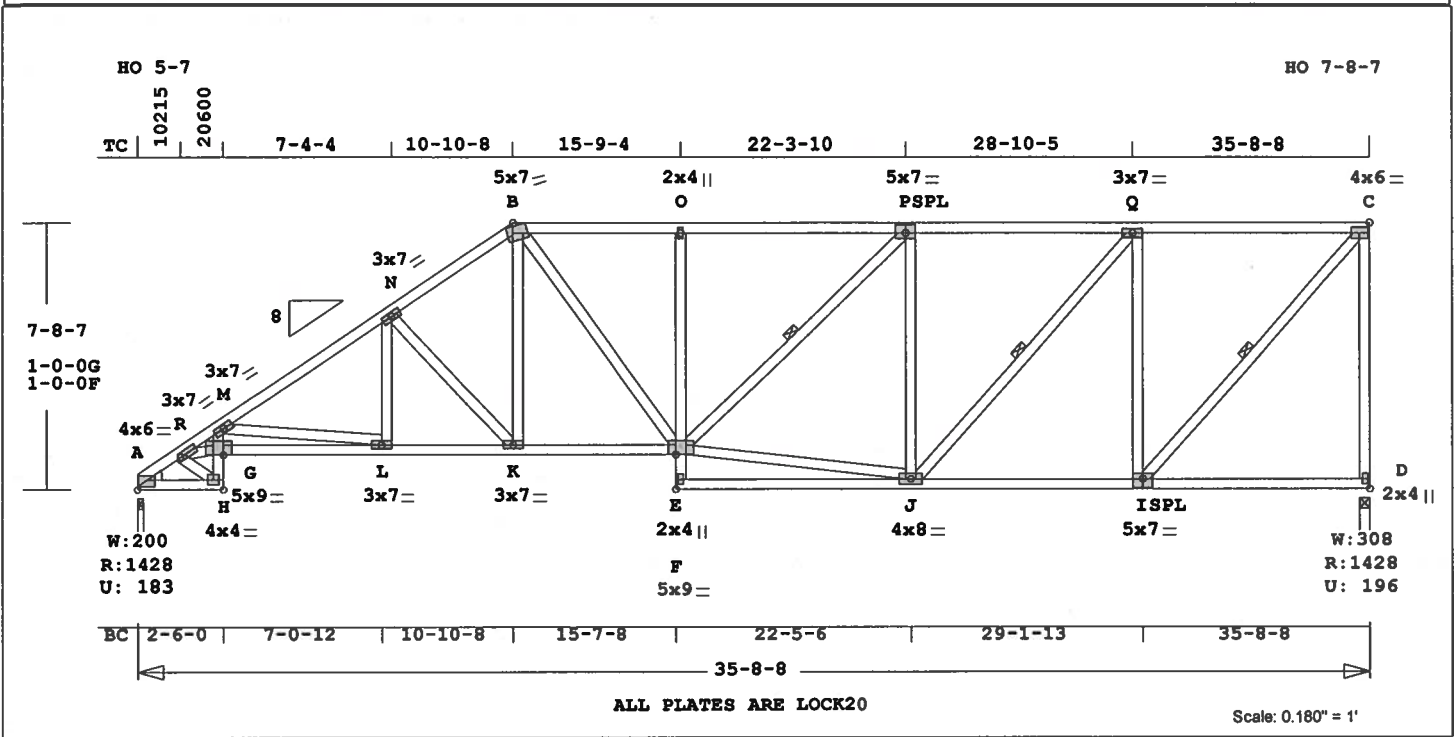
Max tens. force 3821 Lbs

Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>A15</b>	Quan 1	Type SP	Span '350808'	P1-H1 8	Left OH 0	Right OH 0	Engineering <b>T2566265</b>
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TONY WILLIAMS



ALL PLATES ARE LOCK20

Scale: 0.180" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 316.7 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI	Size	---Lumber---
TC	0.48	2x 4 SP-#2
BC	0.97	2x 4 SP-#2
CW	0.73	2x 4 SP-#2
WB	0.94	2x 4 SP-#2
PB	---	2x 4 SP-#2

Brace truss as follows:  
O.C. From To  
TC Cont. 0-0-0 35- 8- 8  
BC Cont. 0-0-0 35- 8- 8  
WB 1 rows CLB on F - P  
WB 1 rows CLB on J - Q  
WB 1 rows CLB on I - C  
Attach CLB with (2)-10d nails  
at each web.

psf-I'd	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)			
Jt	Down	Uplift	Horiz-
A	1428	184 U	134 R
D	1428	197 U	262 R

Jt	Brg Size	Required
A	2.0"	1.7"
D	3.5"	1.7"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Ax1-CSI-Bnd
---Top Chords---			
A -R	0.32	1925 C	0.02 0.30
R -M	0.34	4414 C	0.13 0.21
M -N	0.41	2482 C	0.07 0.34
N -B	0.23	2011 C	0.02 0.21
B -O	0.36	1957 C	0.02 0.34
O -P	0.39	1951 C	0.02 0.37
P -Q	0.48	1617 C	0.01 0.47
Q -C	0.47	1067 C	0.00 0.47
---Bottom Chords---			
A -H	0.35	1430 T	0.24 0.11
G -L	0.97	3854 T	0.64 0.33
L -K	0.48	2081 T	0.34 0.14
K -F	0.36	1666 T	0.27 0.09

E -J	0.27	47 T	0.00 0.27
J -I	0.38	1067 T	0.11 0.27
I -D	0.26	205 T	0.00 0.26
---Chord Webs---			
H -G	0.73	790 T	0.14 0.59
G -M	0.42	1132 T	0.20 0.22
E -F	0.10	107 T	0.01 0.09
F -O	0.14	381 C	0.04 0.10
---Webs---			
R -H	0.13	1398 C	
R -G	0.63	3428 T	
M -L	0.53	1791 C	
L -N	0.07	414 T	
N -K	0.21	585 C	
K -B	0.09	500 T	
B -F	0.13	486 T	
F -P	0.08	463 T	1 Br
F -J	0.29	1587 T	
J -P	0.48	711 C	
J -Q	0.15	830 T	1 Br
I -Q	0.73	1063 C	
I -C	0.29	1611 T	1 Br
D -C	0.94	1372 C	WindLd

TL Defl -0.33" in E -J L/999  
LL Defl -0.15" in K -F L/999  
Shear // Grain in A -R 0.43

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	LOCK	20 Ga,	Gross Area
Plate	RHS	20 Ga,	Gross Area
Jt Type	Plt Size	X	Y JSI
A	LOCK	4.0x 6.0	1.3 0.7 0.92
R	LOCK	3.0x 7.0	0.8 0.6 0.95
M	LOCK	3.0x 7.0	Ctr Ctr 0.51
N	LOCK	3.0x 7.0	Ctr Ctr 0.44
B	LOCK	5.0x 7.0	1.6-3.4 0.70
O	LOCK	2.0x 4.0	Ctr Ctr 0.40
P	LOCK	5.0x 7.0	Ctr 0.5 0.75
Q	LOCK	3.0x 7.0	Ctr Ctr 0.46
C	LOCK	4.0x 6.0	Ctr Ctr 0.74
H	LOCK	4.0x 4.0	Ctr Ctr 0.84
G	LOCK	5.0x 9.0	Ctr 0.8 0.65
L	LOCK	3.0x 7.0	Ctr Ctr 0.85
K	LOCK	3.0x 7.0	Ctr Ctr 0.46
F	LOCK	5.0x 9.0	Ctr 0.7 0.64
E	LOCK	2.0x 4.0	Ctr Ctr 0.58
J	LOCK	4.0x 8.0	Ctr Ctr 0.64
I	LOCK	5.0x 7.0	Ctr-0.5 0.75
D	LOCK	2.0x 4.0	Ctr Ctr 0.55

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

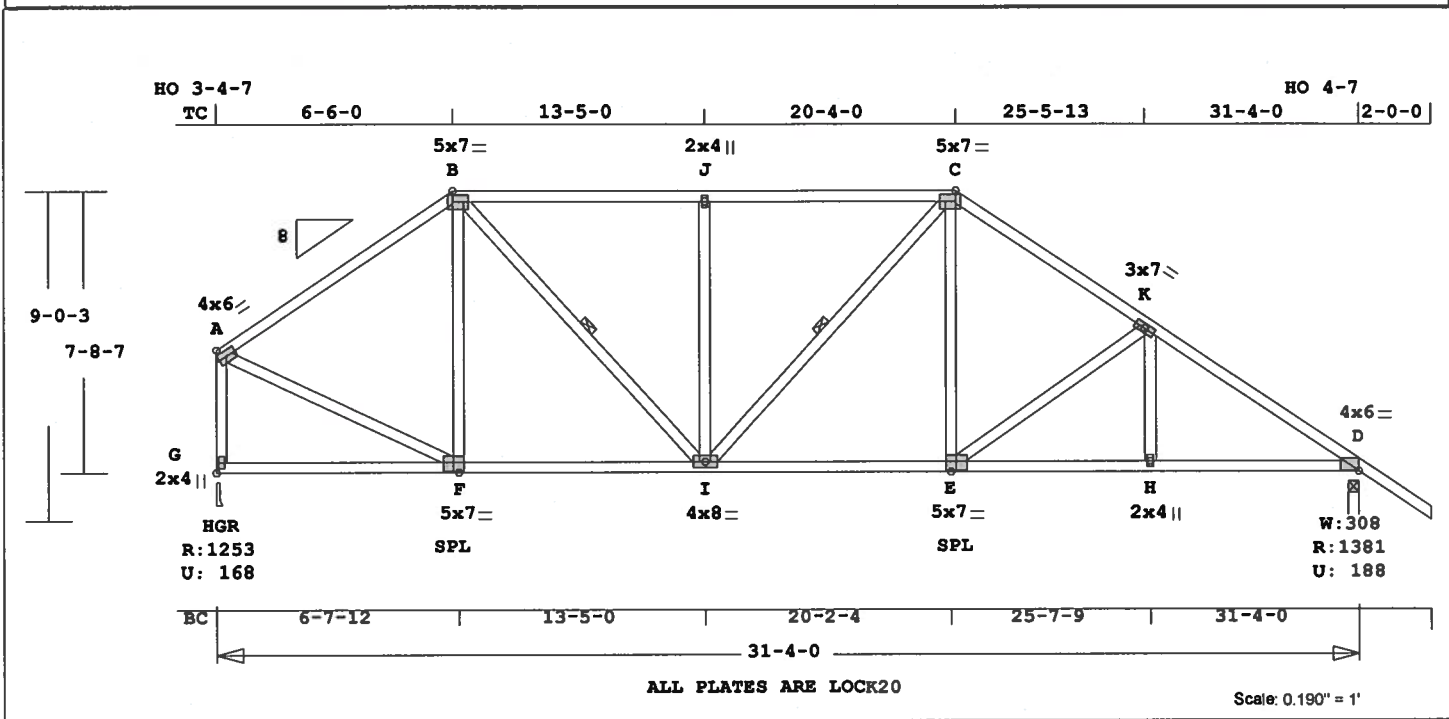
REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 4414 Lbs  
Max tens. force 3854 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>A16</b>	Quan 1	Type HIPP	Span '310400'	Pl-H1 8	Left OH 0	Right OH 2-0-0	Engineering <b>T2566266</b>
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TONY WILLIAMS



ALL PLATES ARE LOCK20

Scale: 0.190" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 244.2 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

	CSI	-Size-	-----Lumber-----
TC	0.49	2x 4	SP-#2
BC	0.37	2x 4	SP-#2
WB	0.30	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	31- 4- 0
BC	Cont.	0- 0- 0	31- 4- 0
WB	1 rows CLB	on B -I	
WB	1 rows CLB	on I -C	

Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
G	1253	168 U	205 R
D	1381	188 U	153 R

Jt	Brg Size	Required
G	3.5"	1.5"
D	3.5"	1.6"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----				
A -B	0.49	1058 C	0.02	0.47
B -J	0.49	1318 C	0.01	0.48
J -C	0.49	1318 C	0.01	0.48
C -K	0.27	1454 C	0.01	0.26
K -D	0.27	1815 C	0.08	0.19
-----Bottom Chords-----				

G -F	0.28	164 T	0.00	0.28
F -I	0.37	893 T	0.09	0.28
I -E	0.35	1208 T	0.12	0.23
E -H	0.37	1509 T	0.25	0.12
H -D	0.31	1509 T	0.25	0.06

-----Webs-----				
G -A	0.17	1199 C	WindLd	
A -F	0.18	988 T		
F -B	0.19	281 C		
B -I	0.11	626 T		1 Br
I -J	0.30	446 C		
J -C	0.02	162 T		1 Br
E -C	0.06	402 T		
E -K	0.19	367 C		
H -K	0.03	205 T		

TL Defl	-0.14"	in I -E	L/999
LL Defl	-0.06"	in I -E	L/999
Shear // Grain		in J -C	0.30

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	0.1	0.1	0.66
B	LOCK	5.0x 7.0	Ctr-0.2	0.90	
J	LOCK	2.0x 4.0	Ctr	Ctr	0.42
C	LOCK	5.0x 7.0	Ctr-0.2	0.90	
K	LOCK	3.0x 7.0	Ctr	Ctr	0.42
D	LOCK	4.0x 6.0	-0.5	0.4	0.66
G	LOCK	2.0x 4.0	Ctr	Ctr	0.46
F	LOCK	5.0x 7.0	Ctr-0.5	0.70	
I	LOCK	4.0x 8.0	Ctr	Ctr	0.41
E	LOCK	5.0x 7.0	Ctr-0.5	0.70	
H	LOCK	2.0x 4.0	Ctr	Ctr	0.42

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

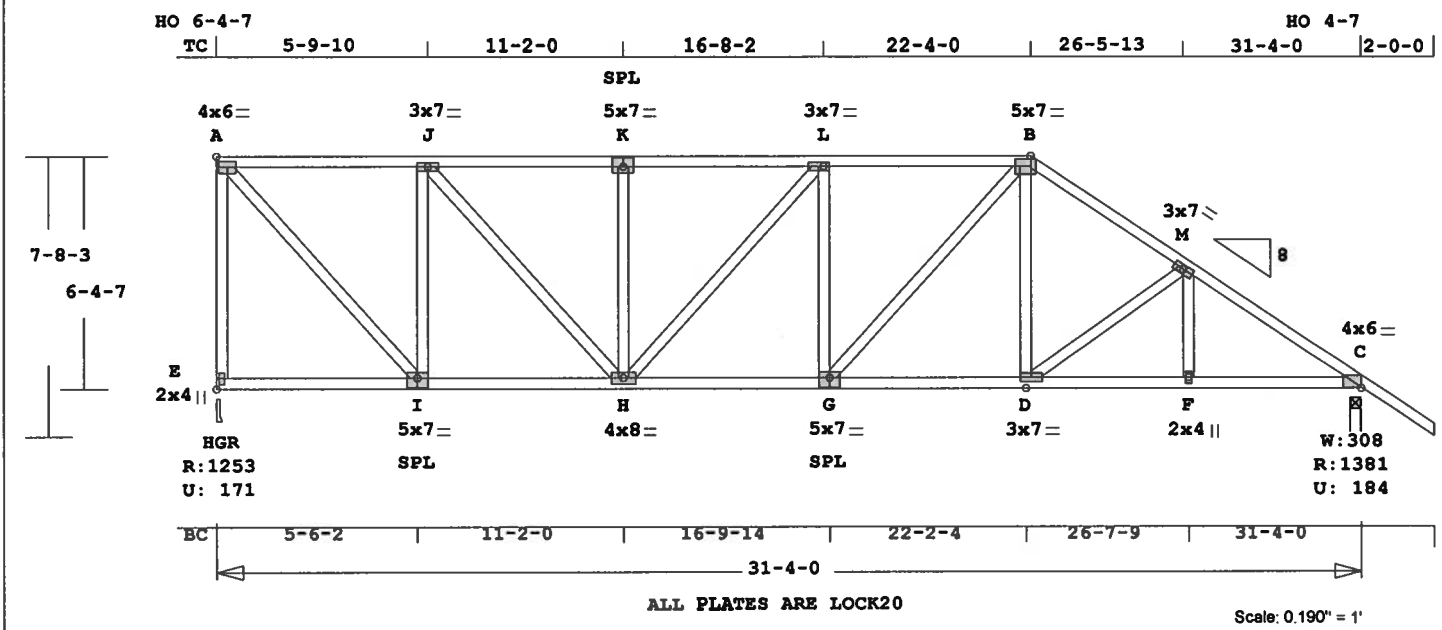
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 1815 Lbs  
Max tens. force 1509 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>A17</b>	Quan 1	Type HHIP	Span '310400'	Pl-H1 8	Left OH 0	Right OH 2- 0- 0	Engineering <b>T2566267</b>
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 255.5 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI	-Size-	---	Lumber	----
TC	0.34	2x 4	SP-#2	
BC	0.36	2x 4	SP-#2	
WB	0.55	2x 4	SP-#2	

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	31- 4- 0	
BC Cont.	0- 0- 0	31- 4- 0	

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
E	1253	172 U	215 R
C	1381	184 U	111 R

Jt	Brg Size	Required
E	3.5"	1.5"
C	3.5"	1.6"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI	Bnd
-----Top Chords-----					
A	-J	0.33	966 C	0.00	0.33
J	-K	0.34	1479 C	0.01	0.33
K	-L	0.32	1479 C	0.01	0.31
L	-B	0.32	1614 C	0.01	0.31
B	-M	0.21	1582 C	0.01	0.20
M	-C	0.19	1851 C	0.08	0.11
-----Bottom Chords-----					
E	-I	0.19	168 T	0.00	0.19
I	-H	0.29	965 T	0.10	0.19
H	-G	0.36	1614 T	0.27	0.09
G	-D	0.31	1318 T	0.22	0.09

D	-F	0.33	1533 T	0.25	0.08
F	-C	0.34	1533 T	0.25	0.09

-----Webs-----					
E	-A	0.55	1205 C	WindLd	
A	-I	0.39	1438 T		
I	-J	0.43	941 C		
J	-H	0.21	764 T		
H	-K	0.14	312 C		
H	-L	0.16	201 C		
G	-L	0.09	210 C		
G	-B	0.12	439 T		
D	-B	0.04	314 T		
D	-M	0.10	282 C		
F	-M	0.02	155 T		

TL Defl	-0.15"	in H	-G	L/999
LL Defl	-0.07"	in H	-G	L/999
Shear // Grain			A -J	0.25

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	-	LOCK	20 Ga,	Gross Area
Plate	-	RHS	20 Ga,	Gross Area
Jt Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr Ctr	0.66
J	LOCK	3.0x 7.0	Ctr Ctr	0.45
K	LOCK	5.0x 7.0	Ctr	0.5 0.70
L	LOCK	3.0x 7.0	Ctr Ctr	0.45
B	LOCK	5.0x 7.0	Ctr-0.2	0.90
C	LOCK	3.0x 7.0	Ctr Ctr	0.42
C	LOCK	4.0x 6.0-0.5	0.4	0.66
E	LOCK	2.0x 4.0	Ctr Ctr	0.48
I	LOCK	5.0x 7.0	Ctr-0.5	0.70
H	LOCK	4.0x 8.0	Ctr Ctr	0.41
G	LOCK	5.0x 7.0	Ctr-0.5	0.70
D	LOCK	3.0x 7.0	Ctr Ctr	0.41
F	LOCK	2.0x 4.0	Ctr Ctr	0.42

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

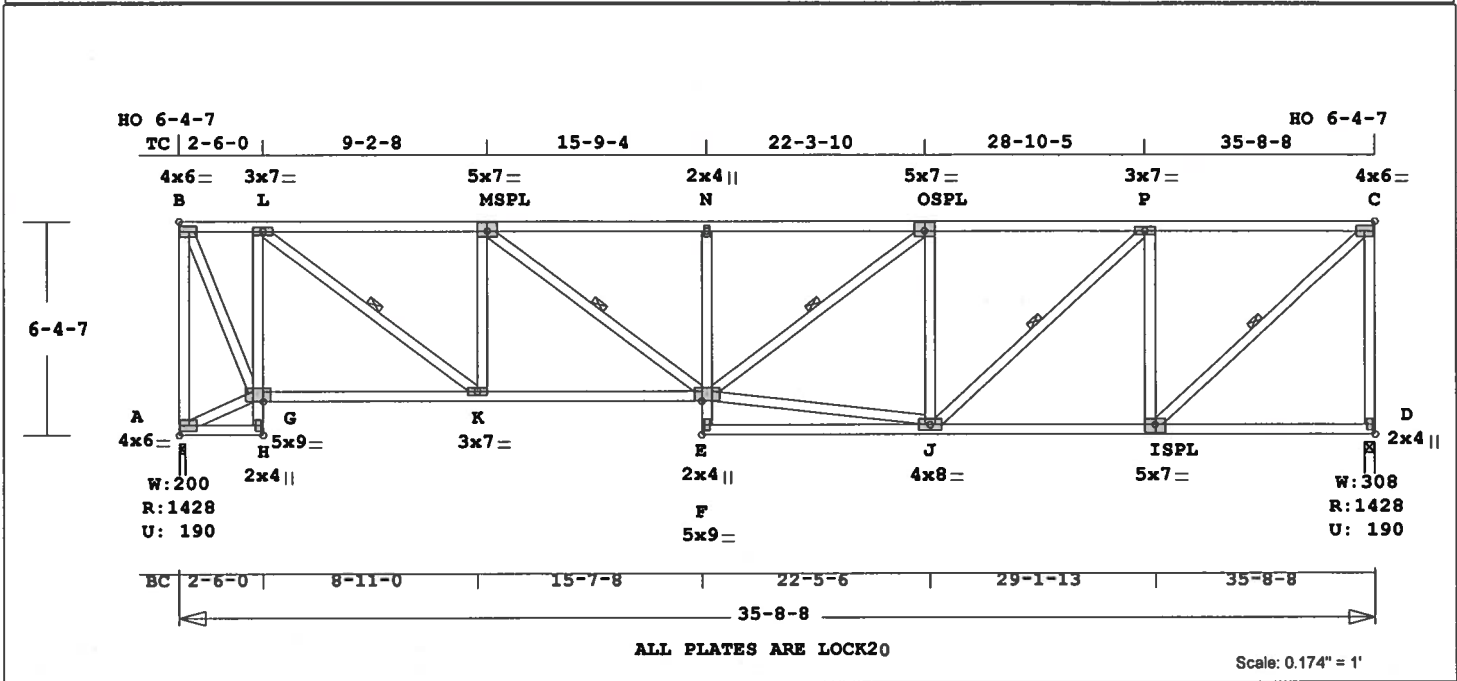
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 1851 Lbs  
Max tens. force 1614 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555



Job <b>ELIXSON-WILL</b>	Mark <b>A18</b>	Quan 1	Type SP	Span '350808'	Pl-H1 60407	Left OH 0	Right OH 0	Engineering <b>T2566268</b>
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TONY WILLIAMS



ALL PLATES ARE LOCK20

Scale: 0.174" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 313.1 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI	-Size-	-----Lumber-----
TC	0.49	2x 4 SP-#2
BC	0.48	2x 4 SP-#2
CW	0.36	2x 4 SP-#2
WB	0.65	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	35- 8- 8
BC Cont.	0- 0- 0	35- 8- 8
WB 1 rows CLB	on L -K	
WB 1 rows CLB	on M -F	
WB 1 rows CLB	on F -O	
WB 1 rows CLB	on J -P	
WB 1 rows CLB	on I -C	

Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1428	190 U	185 R
D	1428	190 U	185 R

Jt	Brg Size	Required
A	2.0"	1.7"
D	3.5"	1.7"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI	Bnd
-----Top Chords-----					
B -L	0.29	612 C	0.00	0.29	
L -M	0.44	1923 C	0.02	0.42	
M -N	0.46	2478 C	0.04	0.42	
N -O	0.42	2467 C	0.04	0.38	
O -P	0.49	1971 C	0.02	0.47	
P -C	0.48	1301 C	0.01	0.47	
-----Bottom Chords-----					
A -H	0.03	69 C	0.00	0.03	
G -K	0.32	619 T	0.06	0.26	
K -F	0.48	1923 T	0.32	0.16	
E -J	0.26	53 C	0.00	0.26	

J -I	0.39	1301 T	0.13	0.26
I -D	0.26	139 T	0.00	0.26

-----Chord-Webs-----				
H -G	0.12	43 T	0.00	0.12
G -L	0.36	1317 C	0.22	0.14
E -F	0.12	108 T	0.01	0.11
F -N	0.15	400 C	0.01	0.14

-----Webs-----				
A -B	0.65	1409 C	WindLd	
A -G	0.02	176 T		
B -G	0.28	1535 T		
L -K	0.30	1635 T		1 Br
K -M	0.27	836 C		
M -F	0.12	696 T		1 Br
F -O	0.11	623 T		1 Br
F -J	0.35	1939 T		
J -O	0.35	763 C		
J -P	0.16	913 T		1 Br
I -P	0.49	1063 C		
I -C	0.32	1774 T		1 Br
D -C	0.63	1371 C	WindLd	

TL Defl	-0.31"	in E -J	L/999
LL Defl	-0.13"	in K -F	L/999
Shear // Grain		in P -C	0.30

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate - LOCK	20 Ga,	Gross Area
Plate - RHS	20 Ga,	Gross Area
Jt Type	Plt Size	X Y JSI
B LOCK	4.0x 6.0	Ctr Ctr 0.74
L LOCK	3.0x 7.0	Ctr Ctr 0.59
M LOCK	5.0x 7.0	Ctr 0.5 0.75
N LOCK	2.0x 4.0	Ctr Ctr 0.40
O LOCK	5.0x 7.0	Ctr 0.5 0.75
P LOCK	3.0x 7.0	Ctr Ctr 0.45
C LOCK	4.0x 6.0	Ctr Ctr 0.74
A LOCK	4.0x 6.0	Ctr Ctr 0.74
H LOCK	2.0x 4.0	Ctr Ctr 0.58
G LOCK	5.0x 9.0	Ctr 0.8 0.91
K LOCK	3.0x 7.0	Ctr Ctr 0.58
F LOCK	5.0x 9.0	Ctr 0.8 0.64
E LOCK	2.0x 4.0	Ctr Ctr 0.58
J LOCK	4.0x 8.0	Ctr Ctr 0.74
I LOCK	5.0x 7.0	Ctr-0.5 0.75
D LOCK	2.0x 4.0	Ctr Ctr 0.55

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.

Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.  
NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

Design checked for 10 psf non-  
concurrent LL on BC.  
Provide drainage to prevent  
water ponding.

This truss must be installed  
as shown. It cannot be  
installed upside-down.

Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

Max comp. force 2478 Lbs

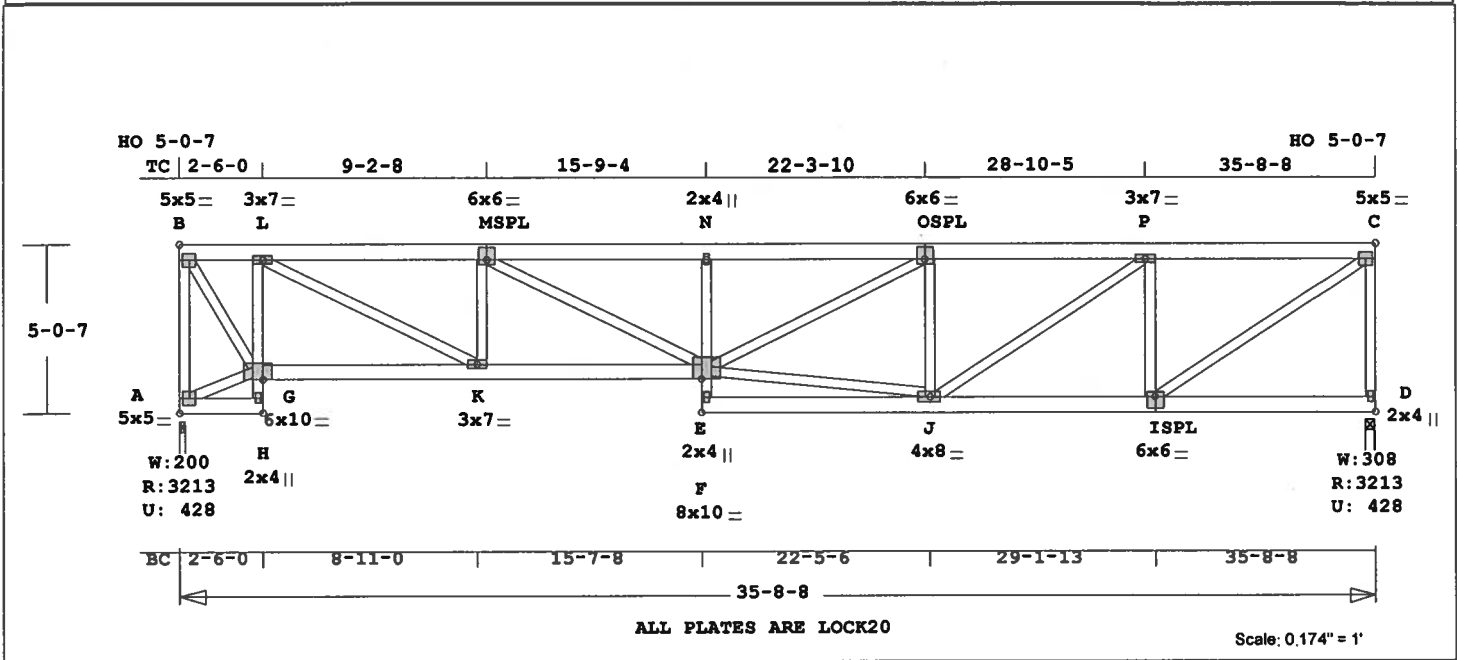
Max tens. force 1939 Lbs

Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>A19</b>	Quan 1*2P	Type SP	Span '350808'	Pl-H1 50007	Left OH 0	Right OH 0	Engineering <b>T2566269</b>
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TONY WILLIAMS



ALL PLATES ARE LOCK 20

Scale: 0.174" = 1'

Online Plus -- Version 21.0.032  
 RUN DATE: 05-JUN-07  
 \*\*\*\*\*  
 \* 2-Ply Truss \*  
 \*\*\*\*\*

CSI	Size	Lumber
TC	0.29	2x 6 SP-#2
BC	0.58	2x 6 SP-#2
CW	0.45	2x 4 SP-#2
WB	0.50	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0-0-0	35-8-8
BC Cont.	0-0-0	35-8-8

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.00	Fc=1.00	Ft=1.00
BC Fb=1.00	Fc=1.00	Ft=1.00

Total Load Reactions (lbs)

Jt	Down	Uplift	Horiz
A	3214	428	U 136 R
D	3214	428	U 136 R

Jt	Brg Size	Required
A	2.0"	1.9"
D	3.5"	1.9"

LC# 1 Girder Loading

Dur Fctrs	Lbr	1.25	Plt	1.25
plf - Dead	TC V	45	90	0.0' 35.7'
plf - Live	BC V	45	0	0.0' 35.7'

Plus 9 Wind Load Case(s)  
 Plus 1 UBC LL Load Case(s)

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
B-L	0.18	1950	C	0.00	0.18
L-M	0.25	6110	C	0.03	0.22
M-N	0.27	7798	C	0.05	0.22
N-O	0.26	7761	C	0.06	0.20
O-P	0.29	5865	C	0.02	0.27
P-C	0.28	3866	C	0.01	0.27
-----Bottom Chords-----					
A-H	0.02	92	C	0.00	0.02
G-K	0.23	1976	T	0.08	0.15
K-F	0.58	6110	T	0.40	0.18
E-J	0.15	423	T	0.01	0.14
J-I	0.31	3865	T	0.25	0.06
I-D	0.14	101	T	0.00	0.14
-----Chord-Webs-----					
H-G	0.08	98	T	0.00	0.08
G-L	0.13	2963	C	0.13	0.00

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 346.9 LBS

E-F	0.45	250	T	0.01	0.44
F-N	0.14	883	C	0.00	0.14

-----Webs-----

A-B	0.17	3161	C	WindLd
A-G	0.01	148	T	
B-G	0.34	3713	T	
L-K	0.43	4685	T	
K-M	0.09	1828	C	
M-F	0.17	1913	T	
F-O	0.19	2150	T	
F-J	0.50	5503	T	
J-O	0.10	1898	C	
J-P	0.22	2439	T	
I-P	0.13	2381	C	
I-C	0.43	4715	T	
D-C	0.17	3079	C	WindLd

TL Defl	-0.37"	in K-F	L/999
LL Defl	-0.18"	in K-F	L/999
Shear // Grain		in E-F	0.27

Plates for each ply each face.  
 PLATING CONFORMS TO TPI.  
 REPORTS: SBCCI 9761  
 ROBBINS ENGINEERING, INC.  
 BASED ON SP LUMBER  
 USING GROSS AREA TEST.

Plate - LOCK	20 Ga,	Gross Area
Plate - RHS	20 Ga,	Gross Area
Jt Type	Plt Size	X Y JSI
B	LOCK	5.0x 5.0 Ctr Ctr 0.76
L	LOCK	3.0x 7.0 Ctr Ctr 0.78
M	LOCK	6.0x 6.0 Ctr 1.2 0.60
N	LOCK	2.0x 4.0 Ctr Ctr 0.30
O	LOCK	6.0x 6.0 Ctr 1.2 0.60
P	LOCK	3.0x 7.0 Ctr Ctr 0.42
C	LOCK	5.0x 5.0 Ctr Ctr 0.76
A	LOCK	5.0x 5.0 Ctr Ctr 0.76
H	LOCK	2.0x 4.0 Ctr Ctr 0.91
G	LOCK	6.0x10.0 Ctr 0.2 0.93
K	LOCK	3.0x 7.0 Ctr Ctr 0.78
F	LOCK	8.0x10.0 Ctr 1.3 0.64
E	LOCK	2.0x 4.0 Ctr Ctr 0.91
J	LOCK	4.0x 8.0-0.5 Ctr 0.89
I	LOCK	6.0x 6.0 Ctr-1.2 0.81
D	LOCK	2.0x 4.0 Ctr Ctr 0.82

REVIEWED BY:  
 Robbins Engineering, Inc.  
 6904 Parke East Blvd.  
 Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
 NOTES AND SYMBOLS SHEET FOR  
 ADDITIONAL SPECIFICATIONS.  
 NOTES:  
 Trusses Manufactured by:  
 Mayo Truss Co. Inc.  
 Analysis Conforms To:  
 FBC2004  
 Girder Common  
 Loading TC and BC

Span 7-0-0  
 2 COMPLETE TRUSSES REQUIRED.  
 Fasten together in staggered  
 pattern. (1/2" bolts -OR-  
 SDS3 screws -OR- 10d nails  
 as each layer is applied.)

Rows	Nails	Screws	Bolts
TC	2	12	24
BC	2	12	24
WB	1	8	8

Design checked for 10 psf non-  
 concurrent LL on BC.  
 Provide drainage to prevent  
 water ponding.

Use properly rated hangers for  
 loads framing into girder  
 truss.

This truss must be installed  
 as shown. It cannot be  
 installed upside-down.

Wind Loads - ANSI / ASCE 7-02  
 Truss is designed as

Components and Claddings\*  
 for Exterior zone location.

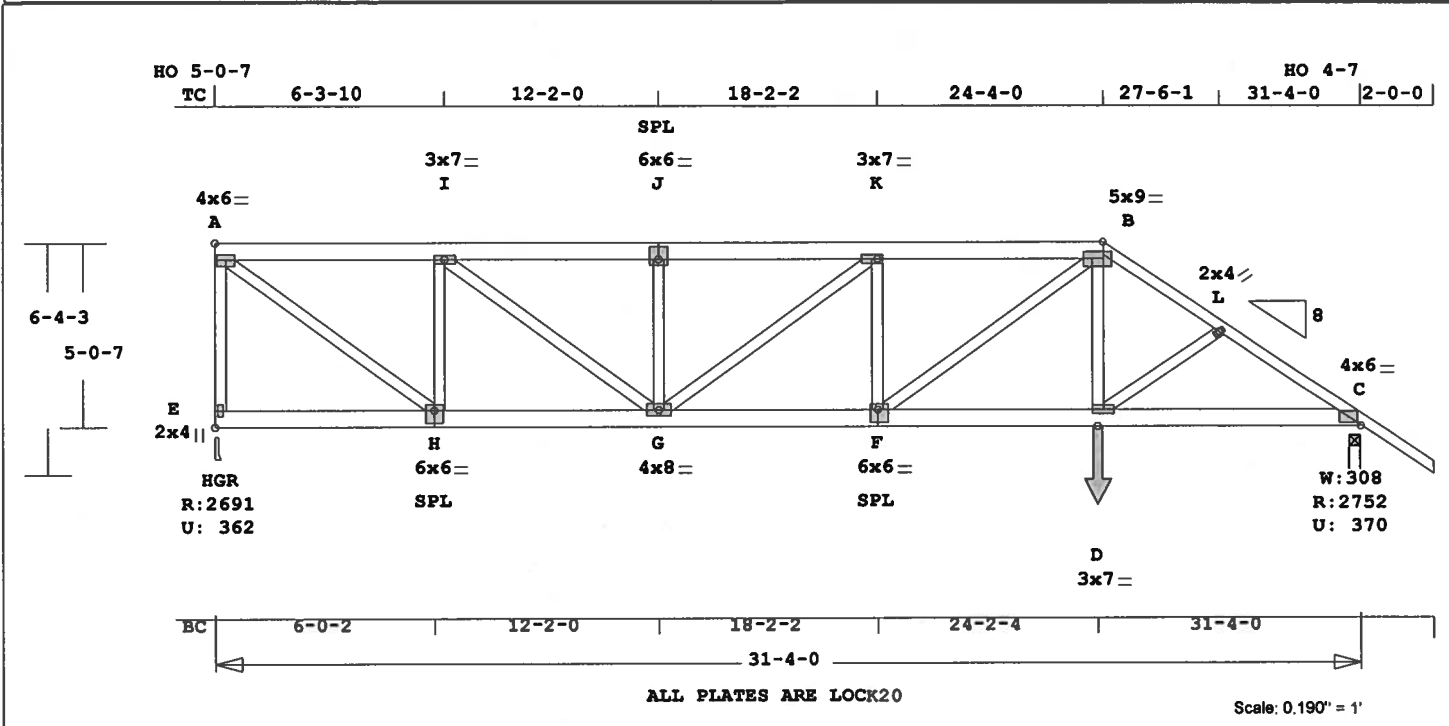
Wind Speed: 110 mph  
 Mean Roof Height: 15-0  
 Exposure Category: B  
 Occupancy Factor: 1.00  
 Building Type: Enclosed

TC Dead Load:	5.0 psf
BC Dead Load:	5.0 psf
Max comp. force	7798 Lbs
Max tens. force	6110 Lbs
Quality Control Factor	1.25

Michael S. Magid, FL Lic. #53681  
 Robbins Engineering  
 6904 Parke East Blvd  
 Tampa, FL 33610  
 FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>A20</b>	Quan 1*2P	Type HHIP	Span '310400'	Pl-H1 8	Left OH 0	Right OH 2-0-0	Engineering <b>T2566270</b>
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 278.0 LBS

Online Plus -- Version 21.0.032  
 RUN DATE: 05-JUN-07  
 \*\*\*\*\*  
 \* 2-Ply Truss \*  
 \*\*\*\*\*

CSI	Size	Lumber
TC	0.28 2x 4	SP-#2
--	0.25 2x 6	SP-#2
A	-J J -B	
BC	0.38 2x 6	SP-#2
WB	0.36 2x 4	SP-#2

Brace truss as follows:  
 O.C. From To  
 TC Cont. 0-0-0 31-4-0  
 BC Cont. 0-0-0 31-4-0

psf-Ld Dead Live  
 TC 10.0 20.0  
 BC 10.0 0.0  
 TC+BC 20.0 20.0  
 Total 40.0 Spacing 24.0"  
 Lumber Duration Factor 1.25  
 Plate Duration Factor 1.25  
 TC Fb=1.00 Fc=1.00 Ft=1.00  
 BC Fb=1.00 Fc=1.00 Ft=1.00

Total Load Reactions (Lbs)  
 Jt Down Uplift Horiz-  
 E 2692 362 U 161 R  
 C 2752 371 U 84 R

Jt Brg Size Required  
 E 3.5" 1.6"  
 C 3.5" 1.6"

LC# 1 Girder Loading

Dur	Fctrs	Lbr	1.25	Plt	1.25
plf	Dead	Live*	From	To	
TC V	20	40	0.0'	31.3'	
BC V	20	0	0.0'	31.3'	
TC V	25	50	1.0'	24.3'	
TC V	-20	-40	0.0'	1.0'	
BC V	25	0	1.0'	24.2'	
BC V	-20	0	0.0'	1.0'	
BC V	280	280	24.2'	CL-LB	

Plus 9 Wind Load Case(s)  
 Plus 1 UBC LL Load Case(s)

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A	-I	0.22	3133	C	0.00 0.22
I	-J	0.23	4693	C	0.01 0.22
J	-K	0.24	4693	C	0.01 0.23
K	-B	0.25	4933	C	0.02 0.23
B	-L	0.28	4167	C	0.03 0.25
L	-C	0.13	4283	C	0.04 0.09

-----Bottom Chords-----  
 E -H 0.12 126 T 0.00 0.12  
 H -G 0.26 3133 T 0.20 0.06  
 G -F 0.38 4933 T 0.32 0.06  
 F -D 0.30 3496 T 0.23 0.07  
 D -C 0.28 3540 T 0.23 0.05

-----Webs-----  
 E -A 0.14 2611 C WindLd  
 A -H 0.36 3939 T  
 H -I 0.11 2088 C  
 I -G 0.18 1960 T  
 G -J 0.04 759 C  
 J -K 0.02 302 C  
 K -L 0.04 744 C  
 F -B 0.16 1781 T  
 D -B 0.06 787 T  
 D -L 0.00 121 C

TL Defl -0.18" in G -F L/999  
 LL Defl -0.09" in G -F L/999  
 Shear // Grain in K -B 0.20

Plates for each ply each face.  
 PLATING CONFORMS TO TPI.  
 REPORTS: SBCCI 9761  
 ROBBINS ENGINEERING, INC.  
 BASED ON SP LUMBER  
 USING GROSS AREA TEST.  
 Plate - LOCK 20 Ga, Gross Area  
 Plate - RHS 20 Ga, Gross Area  
 Jt Type Plt Size X Y JSI  
 A LOCK 4.0x 6.0 Ctr Ctr 0.85  
 I LOCK 3.0x 7.0 Ctr Ctr 0.41  
 J LOCK 6.0x 6.0 Ctr 1.2 0.56  
 K LOCK 3.0x 7.0 Ctr Ctr 0.41  
 B LOCK 5.0x 9.0 Ctr Ctr 0.95  
 L LOCK 2.0x 4.0 Ctr Ctr 0.42  
 C LOCK 4.0x 6.0 Ctr Ctr 0.63  
 E LOCK 2.0x 4.0 Ctr Ctr 0.73  
 H LOCK 6.0x 6.0 Ctr-1.2 0.68  
 G LOCK 4.0x 8.0 Ctr Ctr 0.42  
 F LOCK 6.0x 6.0 0.5-1.2 0.56  
 D LOCK 3.0x 7.0 Ctr Ctr 0.41

REVIEWED BY:  
 Robbins Engineering, Inc.  
 6904 Parke East Blvd.  
 Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
 NOTES AND SYMBOLS SHEET FOR  
 ADDITIONAL SPECIFICATIONS.  
 NOTES:  
 Trusses Manufactured by:  
 Mayo Truss-Co. Inc.  
 Analysis Conforms to:  
 FBC2004  
 Girder Half Hip

Framing King Jacks  
 Jack Open Faced  
 Setback 7-0-0  
 2 COMPLETE TRUSSES REQUIRED.  
 Fasten together in staggered  
 pattern. (1/2" bolts -OR-  
 SDS3 screws -OR- 10d nails  
 as each layer is applied.)  
 -----Spacing (In)-----  

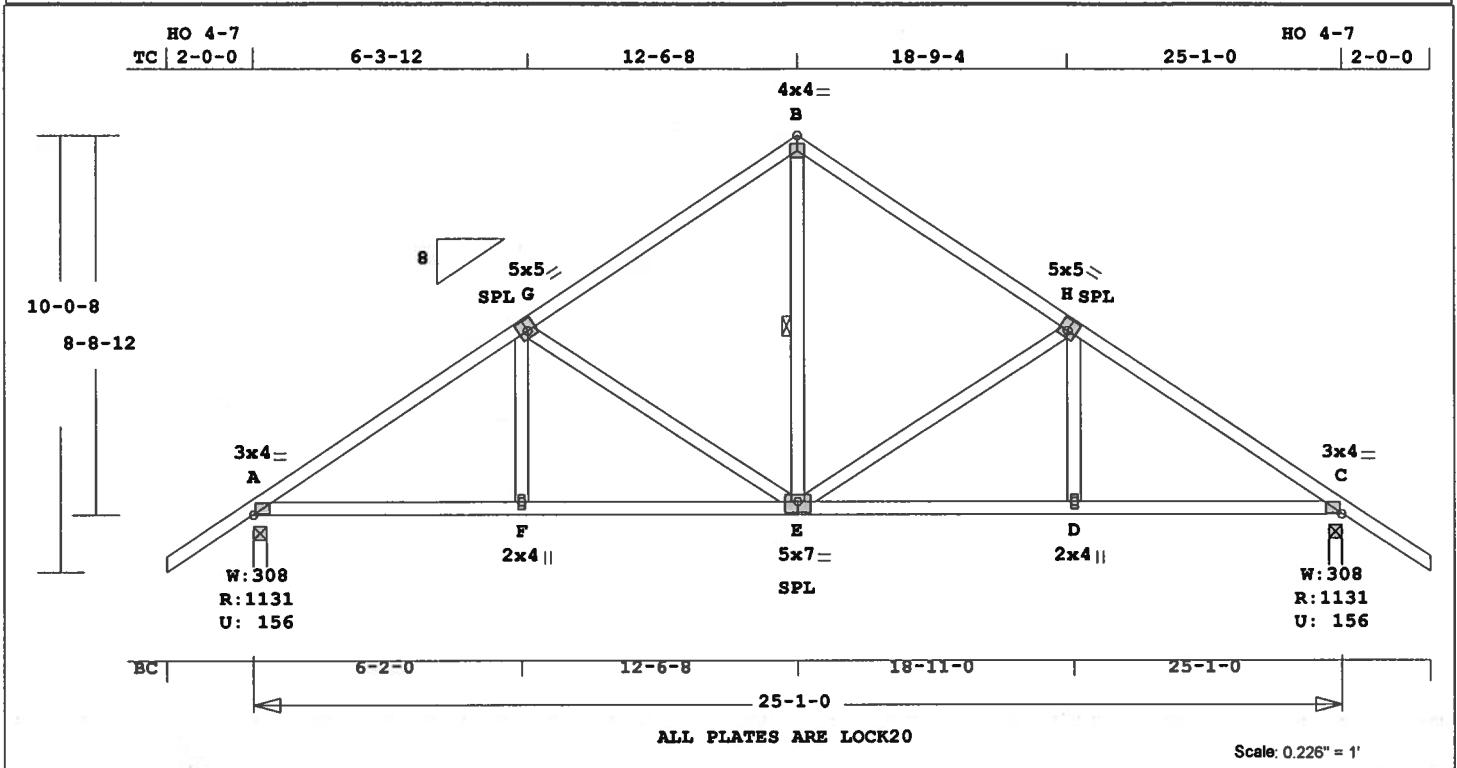
Rows	Nails	Screws	Bolts
TC 1	12	24	0
BC 2	12	24	0
WB 1	8	8	

 Plus clusters of nails where  
 shown.  
 OH Loading  
 Soffit psf 2.0  
 Design checked for 10 psf non-  
 concurrent LL on BC.  
 Wind Loads - ANSI / ASCE 7-02  
 Truss is designed as  
 Components and Claddings\*  
 for Exterior zone location.  
 Wind Speed: 110 mph  
 Mean Roof Height: 15-0  
 Exposure Category: B  
 Occupancy Factor: 1.00  
 Building Type: Enclosed  
 TC Dead Load: 5.0 psf  
 BC Dead Load: 5.0 psf  
 Max comp. force 4933 Lbs  
 Max tens. force 4933 Lbs  
 Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
 Robbins Engineering  
 6904 Parke East Blvd  
 Tampa, FL, 33610  
 FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>BI</b>	Quan 19	Type TR	Span '250100'	Pl-H1 8	Left OH 2- 0- 0	Right OH 2- 0- 0	Engineering <b>T2566271</b>
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 171.7 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

	CSI	-Size-	-----Lumber-----
TC	0.37	2x 4	SP-#2
BC	0.33	2x 4	SP-#2
WB	0.33	2x 4	SP-#2

Brace truss as follows:  
O.C. From To  
TC Cont. 0- 0- 0 25- 1- 0  
BC Cont. 0- 0- 0 25- 1- 0  
WB 1 rows CLB on E -B  
Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)			
Jt	Down	Uplift	Horiz-
A	1131	156 U	182 R
C	1131	156 U	182 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
-----Top Chords-----			
A -G	0.37	1373 C	0.01 0.36
G -B	0.36	916 C	0.00 0.36
B -H	0.36	916 C	0.00 0.36
H -C	0.37	1373 C	0.01 0.36
-----Bottom Chords-----			

A -F	0.31	1149 T	0.12 0.19
F -E	0.33	1149 T	0.12 0.21
E -D	0.33	1149 T	0.12 0.21
D -C	0.31	1149 T	0.12 0.19
-----Webs-----			
F -G	0.03	250 T	
G -E	0.33	469 C	
E -B	0.11	642 T	1 Br
E -H	0.33	469 C	
D -H	0.03	250 T	

TL Defl -0.09" in E -D L/999  
LL Defl -0.04" in E -D L/999  
Shear // Grain in G -B 0.21

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.80  
G LOCK 5.0x 5.0-0.3 0.5 0.61  
B LOCK 4.0x 4.0 Ctr Ctr 0.68  
H LOCK 5.0x 5.0 0.3 0.5 0.61  
C LOCK 3.0x 4.0 Ctr Ctr 0.80  
F LOCK 2.0x 4.0 Ctr Ctr 0.40  
E LOCK 5.0x 7.0 Ctr-0.5 0.63  
D LOCK 2.0x 4.0 Ctr Ctr 0.40

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.

Analysis Conforms To:  
FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings\*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

Max comp. force 1373 Lbs

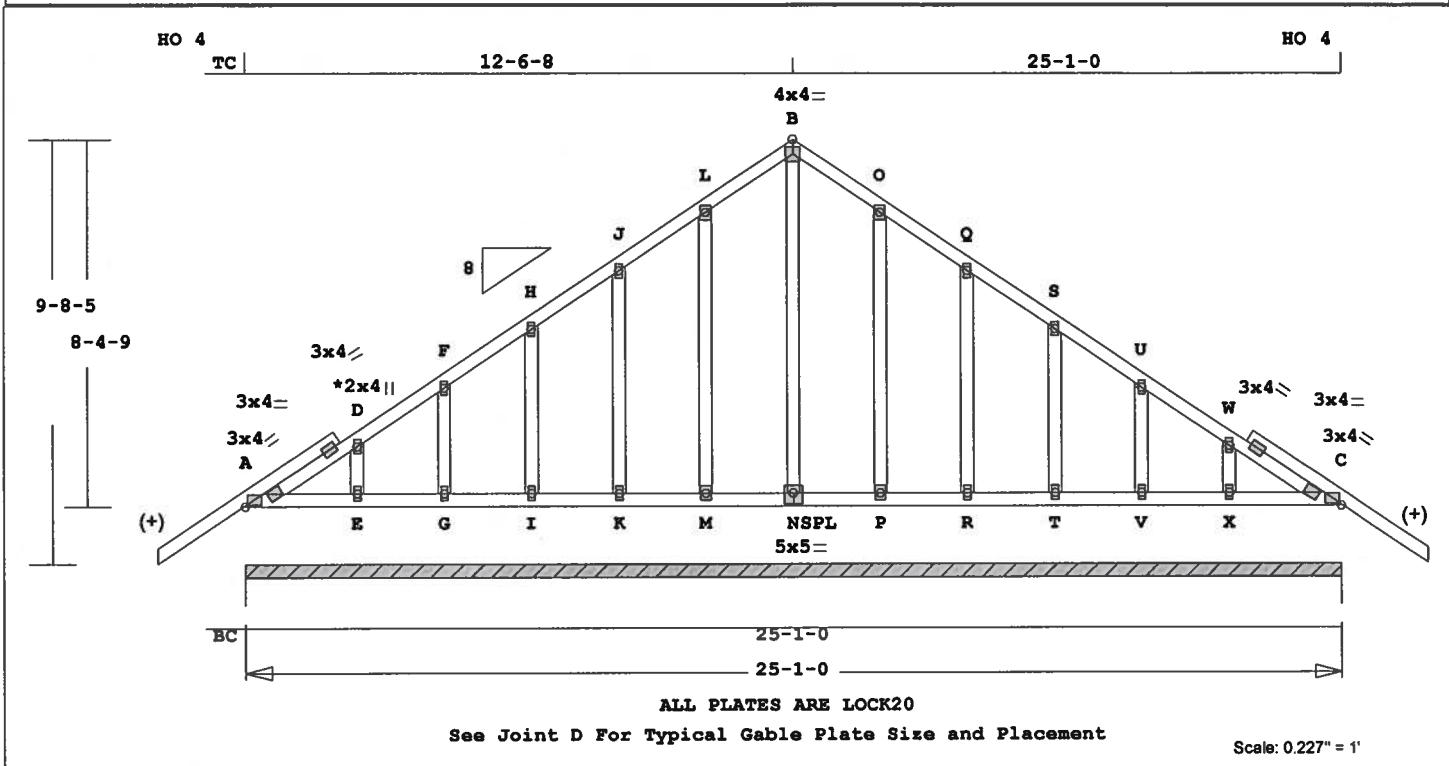
Max tens. force 1149 Lbs

Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>B2</b>	Quan 1	Type TR	Span 250'10"	Pl-H1 8	Left OH 0	Right OH 0	Engineering T2566272
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TONY WILLIAMS



ALL PLATES ARE LOCK20  
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.227" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 208.5 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI	Size	Lumber
TC	0.04	2x 4 SP-#2 (+)
BC	0.04	2x 4 SP-#2
GW	0.09	2x 4 SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.	0-0-0	25-1-0	
BC Cont.	0-0-0	25-1-0	

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15 Fc=1.10 Ft=1.10		
BC Fb=1.10 Fc=1.10 Ft=1.10		

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz
A	2007	267 U	174 R

Jt	Brg Size	Required
A	301.0"	0"-to- 301"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd

Top Chords			
A-D	0.04	162 C	0.00 0.04
D-F	0.04	109 C	0.00 0.04
F-H	0.03	70 C	0.00 0.03
H-J	0.03	53 C	0.00 0.03
J-L	0.03	100 C	0.00 0.03
L-B	0.03	165 T	0.00 0.03
B-O	0.03	165 T	0.00 0.03
O-Q	0.03	100 C	0.00 0.03
Q-S	0.03	53 C	0.00 0.03
S-U	0.03	70 C	0.00 0.03
U-W	0.04	109 C	0.00 0.04
W-C	0.04	162 C	0.00 0.04
Bottom Chords			
A-E	0.04	15 T	0.00 0.04
E-G	0.02	0 T	0.00 0.02
G-I	0.02	0 T	0.00 0.02
I-K	0.02	0 T	0.00 0.02
K-M	0.02	0 T	0.00 0.02
M-N	0.02	0 T	0.00 0.02
N-P	0.02	0 T	0.00 0.02
P-R	0.02	0 T	0.00 0.02
R-T	0.02	0 T	0.00 0.02
T-V	0.02	0 T	0.00 0.02
V-X	0.02	0 T	0.00 0.02

X	C	0.04	15 T	0.00	0.04
-----Gable Webs-----					
E-D	0.01	141 C			
G-F	0.01	119 C			
I-H	0.02	124 C			
K-J	0.04	126 C			
M-L	0.07	122 C			
N-B	0.09	114 C			
P-O	0.07	122 C			
R-Q	0.04	126 C			
T-S	0.02	124 C			
V-U	0.01	119 C			
X-W	0.01	141 C			

TL Defl	0.00"	in A -E	L/999
LL Defl	0.00"	in A -E	L/999
Shear // Grain	in A -D	0.08	

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761

ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area	
Plate - RHS 20 Ga, Gross Area	
Jt Type Plt Size X Y JSI	
A LOCK 3.0x 4.0 Ctr Ctr 0.80	
D LOCK 2.0x 4.0 Ctr Ctr 0.00	
F LOCK 2.0x 4.0 Ctr Ctr 0.00	
H LOCK 2.0x 4.0 Ctr Ctr 0.00	
J LOCK 2.0x 4.0 Ctr Ctr 0.00	
L LOCK 2.0x 4.0 Ctr Ctr 0.00	
B LOCK 4.0x 4.0 Ctr Ctr 0.68	
O LOCK 2.0x 4.0 Ctr Ctr 0.00	
Q LOCK 2.0x 4.0 Ctr Ctr 0.00	
S LOCK 2.0x 4.0 Ctr Ctr 0.00	
U LOCK 2.0x 4.0 Ctr Ctr 0.00	
C LOCK 3.0x 4.0 Ctr Ctr 0.80	
E LOCK 2.0x 4.0 Ctr Ctr 0.00	
G LOCK 2.0x 4.0 Ctr Ctr 0.00	
I LOCK 2.0x 4.0 Ctr Ctr 0.00	
K LOCK 2.0x 4.0 Ctr Ctr 0.00	
M LOCK 2.0x 4.0 Ctr Ctr 0.00	
N LOCK 5.0x 5.0 Ctr-0.5 0.63	
P LOCK 2.0x 4.0 Ctr Ctr 0.00	
R LOCK 2.0x 4.0 Ctr Ctr 0.00	
T LOCK 2.0x 4.0 Ctr Ctr 0.00	
V LOCK 2.0x 4.0 Ctr Ctr 0.00	
X LOCK 2.0x 4.0 Ctr Ctr 0.00	

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parka East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

WARNING Do Not Cut overframe  
member between outside of  
truss and first tie-plate  
to inside of heel plate.  
Design checked for 10 psf non-  
concurrent LL on BC.

Refer to Gen Det 3 series for  
web bracing and plating.

Wind Loads - ANSI / ASCE 7-02  
Truss is designed as

Components and Claddings\*  
for Exterior zone location.

Wind Speed: 110 mph  
Mean Roof Height: 15-0

Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed

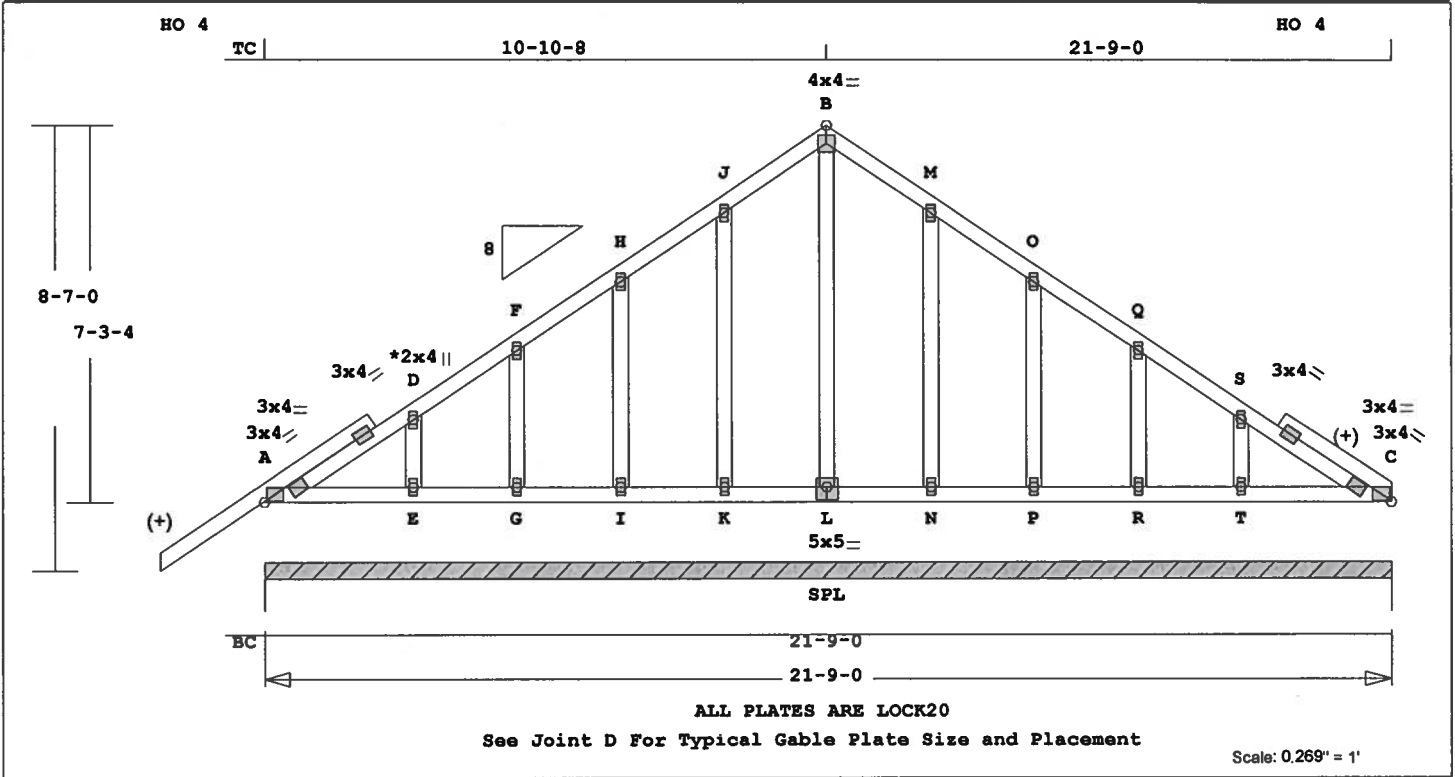
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf

Max comp. force 162 Lbs  
Max tens. force 165 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>CI</b>	Quan 1	Type TR	Span 210900'	P1-H1 8	Left OH 0	Right OH 0	Engineering T2566273
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 166.7 LBS  
 Online Plus -- Version 21.0.032  
 RUN DATE: 05-JUN-07  
 CSI -Size- ---Lumber---  
 TC 0.05 2x 4 SP-#2 (+)  
 BC 0.05 2x 4 SP-#2  
 GW 0.05 2x 4 SP-#2

Brace truss as follows:  
 O.C. From To  
 TC Cont. 0- 0- 0 21- 9- 0  
 BC Cont. 0- 0- 0 21- 9- 0

psf-Ld Dead Live  
 TC 10.0 20.0  
 BC 10.0 0.0  
 TC+BC 20.0 20.0  
 Total 40.0 Spacing 24.0"  
 Lumber Duration Factor 1.25  
 Plate Duration Factor 1.25  
 TC Fb=1.15 Fc=1.10 Ft=1.10  
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)  
 Jt Down Uplift Horiz-  
 A 1740 232 U 147 R

Jt Brg Size Required  
 A 261.0" 0"-to- 261"

Plus 9 Wind Load Case(s)  
 Plus 1 UBC LL Load Case(s)

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
-----Top Chords-----					
A - D	0.05	118	C	0.00	0.05
D - F	0.05	71	C	0.00	0.05
F - H	0.03	61	C	0.00	0.03
H - J	0.03	93	C	0.00	0.03
J - B	0.03	150	C	0.00	0.03
B - M	0.03	150	C	0.00	0.03
M - O	0.03	93	C	0.00	0.03
O - Q	0.03	61	C	0.00	0.03
Q - S	0.05	71	C	0.00	0.05
S - C	0.05	118	C	0.00	0.05
-----Bottom Chords-----					
A - E	0.05	11	T	0.00	0.05
E - G	0.03	0	T	0.00	0.03
G - I	0.02	0	T	0.00	0.02
I - K	0.02	0	T	0.00	0.02
K - L	0.02	0	T	0.00	0.02
L - N	0.02	0	T	0.00	0.02
N - P	0.02	0	T	0.00	0.02
P - R	0.02	0	T	0.00	0.02
R - T	0.03	0	T	0.00	0.03

T - C	0.05	11	T	0.00	0.05
-----Gable Webs-----					
E - D	0.01	155	C		
G - F	0.01	116	C		
I - H	0.03	128	C		
K - J	0.04	122	C		
L - B	0.05	98	C		
N - M	0.04	122	C		
P - O	0.03	128	C		
R - Q	0.01	116	C		
T - S	0.01	155	C		

TL Defl 0.00" in T - C L/999  
 LL Defl 0.00" in T - C L/999  
 Shear // Grain in A - D 0.09

Plates for each ply each face.  
 PLATING CONFORMS TO TPI.  
 REPORTS: SBCCI 9761  
 ROBINS ENGINEERING, INC.  
 BASED ON SP LUMBER  
 USING GROSS AREA TEST.  
 Plate - LOCK 20 Ga, Gross Area  
 Plate - RHS 20 Ga, Gross Area  
 Jt Type Plt Size X Y JSI  
 A LOCK 3.0x 4.0 Ctr Ctr 0.75  
 D LOCK 2.0x 4.0 Ctr Ctr 0.00  
 F LOCK 2.0x 4.0 Ctr Ctr 0.00  
 H LOCK 2.0x 4.0 Ctr Ctr 0.00  
 J LOCK 2.0x 4.0 Ctr Ctr 0.00  
 B LOCK 4.0x 4.0 Ctr Ctr 0.64  
 M LOCK 2.0x 4.0 Ctr Ctr 0.00  
 O LOCK 2.0x 4.0 Ctr Ctr 0.00  
 Q LOCK 2.0x 4.0 Ctr Ctr 0.00  
 S LOCK 2.0x 4.0 Ctr Ctr 0.00  
 C LOCK 3.0x 4.0 Ctr Ctr 0.75  
 E LOCK 2.0x 4.0 Ctr Ctr 0.00  
 G LOCK 2.0x 4.0 Ctr Ctr 0.00  
 I LOCK 2.0x 4.0 Ctr Ctr 0.00  
 K LOCK 2.0x 4.0 Ctr Ctr 0.00  
 L LOCK 5.0x 5.0 Ctr-0.5 0.59  
 N LOCK 2.0x 4.0 Ctr Ctr 0.00  
 P LOCK 2.0x 4.0 Ctr Ctr 0.00  
 R LOCK 2.0x 4.0 Ctr Ctr 0.00  
 T LOCK 2.0x 4.0 Ctr Ctr 0.00

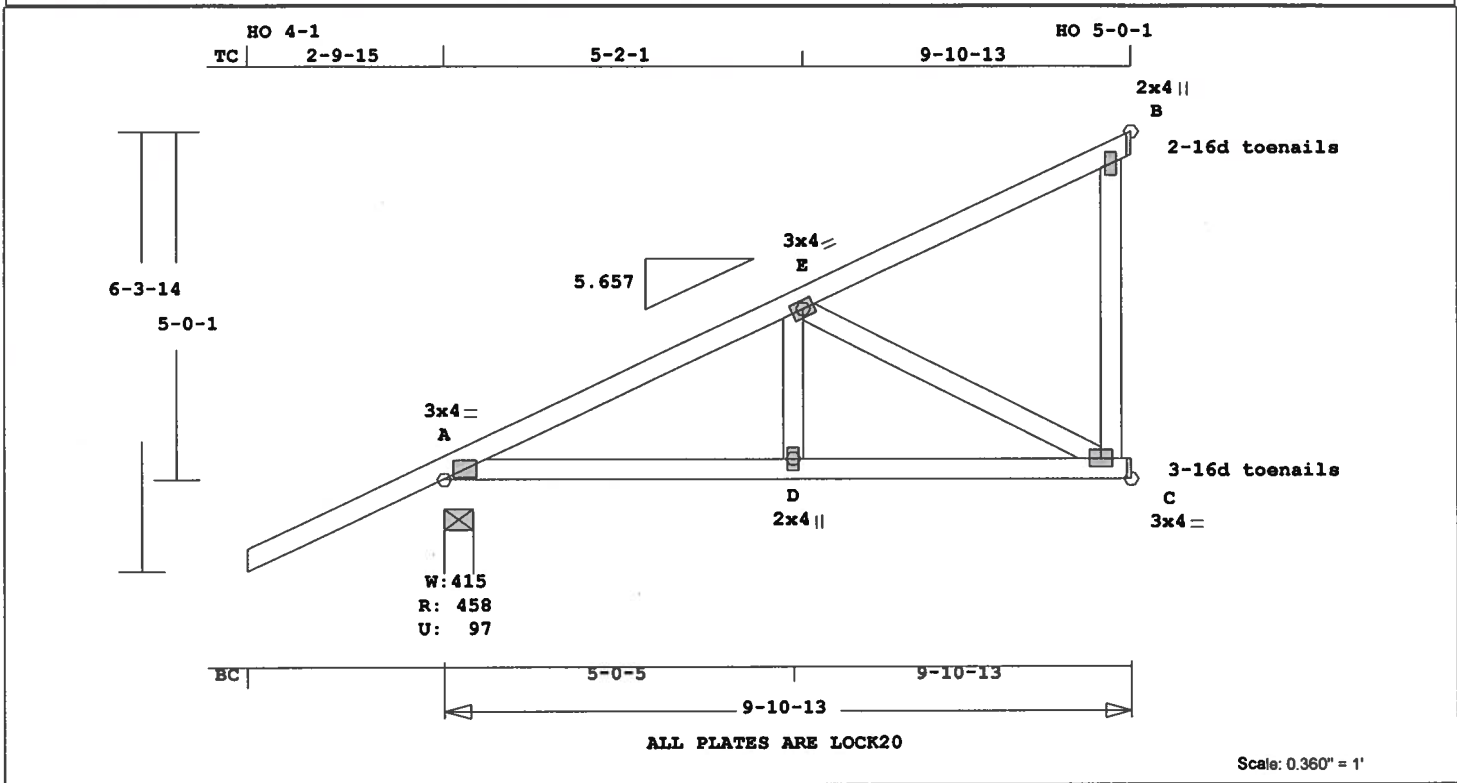
REVIEWED BY:  
 Robbins Engineering, Inc.  
 6904 Parke East Blvd.  
 Tampa, FL 33610

REFER TO ROBINS ENG. GENERAL  
 NOTES AND SYMBOLS SHEET FOR  
 ADDITIONAL SPECIFICATIONS.

NOTES:  
 Trusses Manufactured by:  
 Mayo Truss Co. Inc.  
 Analysis Conforms To:  
 FBC2004  
 WARNING Do Not Cut overframe  
 member between outside of  
 truss and first tie-plate  
 to inside of heel plate.  
 Design checked for 10 psf non-  
 concurrent LL on BC.  
 Refer to Gen Det 3 series for  
 web bracing and plating.  
 Wind Loads - ANSI / ASCE 7-02  
 Truss is designed as  
 Components and Claddings\*  
 for Exterior zone location.  
 Wind Speed: 110 mph  
 Mean Roof Height: 15-0  
 Exposure Category: B  
 Occupancy Factor : 1.00  
 Building Type: Enclosed  
 TC Dead Load: 5.0 psf  
 BC Dead Load: 5.0 psf  
 Max comp. force 155 Lbs  
 Max tens. force 150 Lbs  
 Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
 Robbins Engineering  
 6904 Parke East Blvd  
 Tampa, FL, 33610  
 FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>CJI</b>	Quan 1	Type MONO.DD	Span 91013	P1-H1 5.657	Left OH 2- 9-15	Right OH 0	Engineering T2566274
TONY WILLIAMS								



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 66.9 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

	CSI	-Size-	-----Lumber-----
TC	0.43	2x 4	SP-#2
BC	0.25	2x 4	SP-#2
WB	0.17	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	9-10-13	
BC Cont.	0- 0- 0	9-10-13	

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.00	Fc=1.00	Ft=1.00
BC Fb=1.00	Fc=1.00	Ft=1.00

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	458	97 U	119 R
C	348	16 U	
B	243	87 U	170 R

Jt	Brg Size	Required
A	4.9"	1.5"
C	1.5"	1.5"
B	1.5"	1.5"

LC# 1 Girder Loading

Dur	Fctrs	- Lbr	1.25	Plt	1.25
plf	- Dead	Live*	From	To	
TC V	20	40	0.0'	9.9'	
BC V	20	0	0.0'	9.9'	
TC V	-20	-40	0.0'		
		22	45		9.9'
BC V	-20	0	0.0'		
		22	0		9.9'

Plus 7 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----					
A -E	0.35	500 C	0.02	0.33	
E -B	0.43	107 T	0.00	0.43	
-----Bottom Chords-----					
A -D	0.22	472 T	0.05	0.17	
D -C	0.25	472 T	0.05	0.20	
-----Webs-----					
D -E	0.03	236 T			
E -C	0.17	533 C			
C -B	0.10	0 T	WindLd		

TL Defl -0.05" in D -C L/999  
LL Defl -0.02" in D -C L/999  
Shear // Grain in E -B 0.28

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.61  
E LOCK 3.0x 4.0 Ctr Ctr 0.45  
B LOCK 2.0x 4.0 Ctr Ctr 0.38  
D LOCK 2.0x 4.0 Ctr Ctr 0.38  
C LOCK 3.0x 4.0 Ctr Ctr 0.54

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

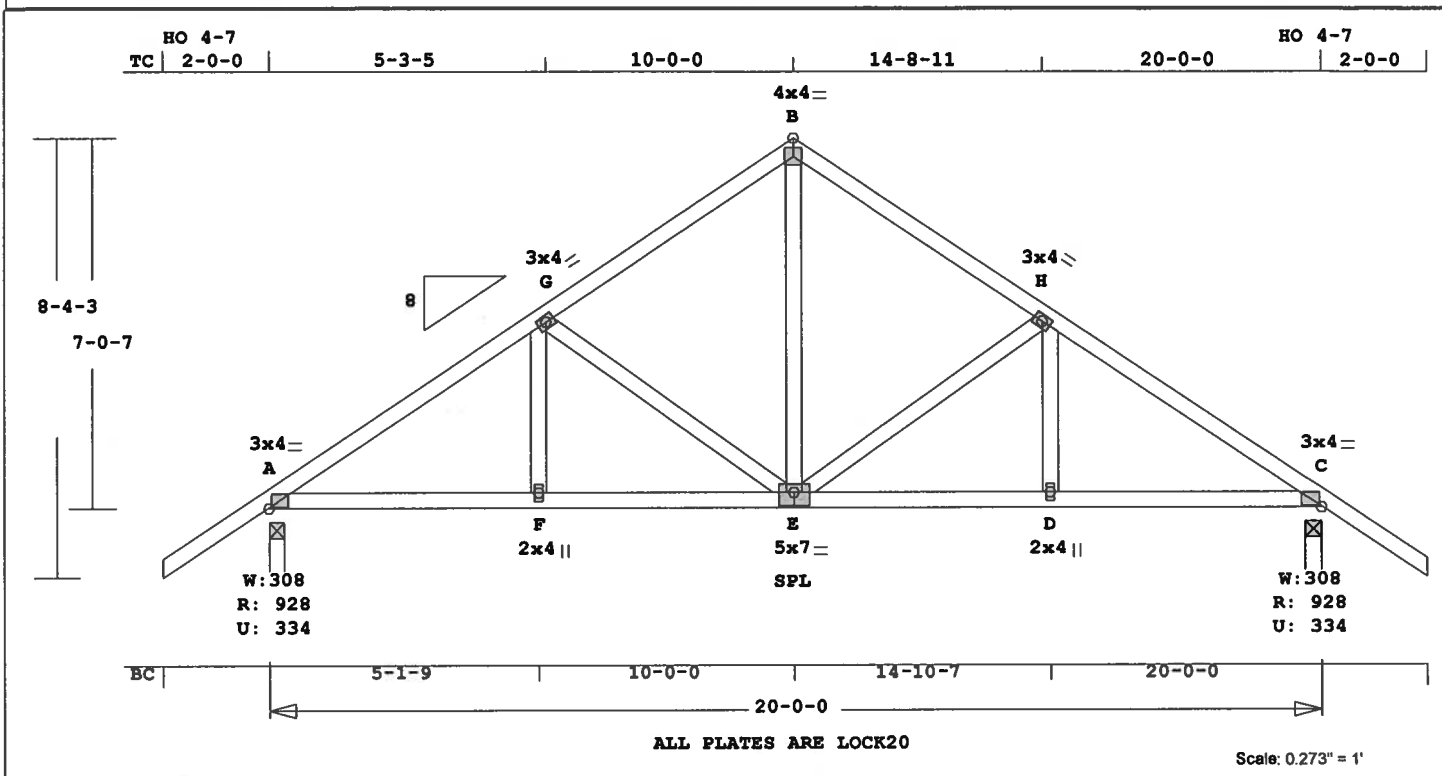
For proper installation of  
toe-nails, refer to the 2001  
National Design Specification  
(NDS) for Wood Construction

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:

FBC2004  
Girder King Jack  
Loading TC and BC  
Setback 7- 0- 0  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Use properly rated hangers for  
loads framing into girder  
truss.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 533 Lbs  
Max tens. force 472 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>DI</b>	Quan 6	Type TR	Span '20000'	Pl-H1 8	Left OH 2-0-0	Right OH 2-0-0	Engineering <b>T2566275</b>
TONY WILLIAMS								



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 138.5 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

	CSI	-Size-	---	Lumber	----
TC	0.28	2x 4	SP-#2		
BC	0.21	2x 4	SP-#2		
WB	0.36	2x 4	SP-#2		

Brace truss as follows:

	O.C.	From	To
TC Cont.	0-0-0	20-0-0	0-0-0
BC Cont.	0-0-0	20-0-0	0-0-0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	24.0
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	928	335 U	141 R
C	928	335 U	141 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----				
A -G	0.28	1066 C	0.12	0.16
G -B	0.25	735 T	0.09	0.16
B -H	0.25	735 T	0.09	0.16
H -C	0.28	1066 C	0.12	0.16
-----Bottom Chords-----				
A -F	0.20	891 T	0.09	0.11
F -E	0.21	891 T	0.14	0.07

E -D	0.21	890 T	0.14	0.07
D -C	0.20	890 T	0.09	0.11
-----Webs-----				
F -G	0.03	196 T		
G -E	0.15	417 T		
E -B	0.36	663 C		
E -H	0.15	417 T		
D -H	0.03	196 T		

TL Defl	-0.05"	in F -E	L/999
LL Defl	-0.02"	in F -E	L/999
Shear // Grain		in A -G	0.16

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.72  
G LOCK 3.0x 4.0 Ctr Ctr 0.62  
B LOCK 4.0x 4.0 Ctr Ctr 0.62  
H LOCK 3.0x 4.0 Ctr Ctr 0.62  
C LOCK 3.0x 4.0 Ctr Ctr 0.72  
F LOCK 2.0x 4.0 Ctr Ctr 0.40  
E LOCK 5.0x 7.0 Ctr-0.5 0.57  
D LOCK 2.0x 4.0 Ctr Ctr 0.40

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:

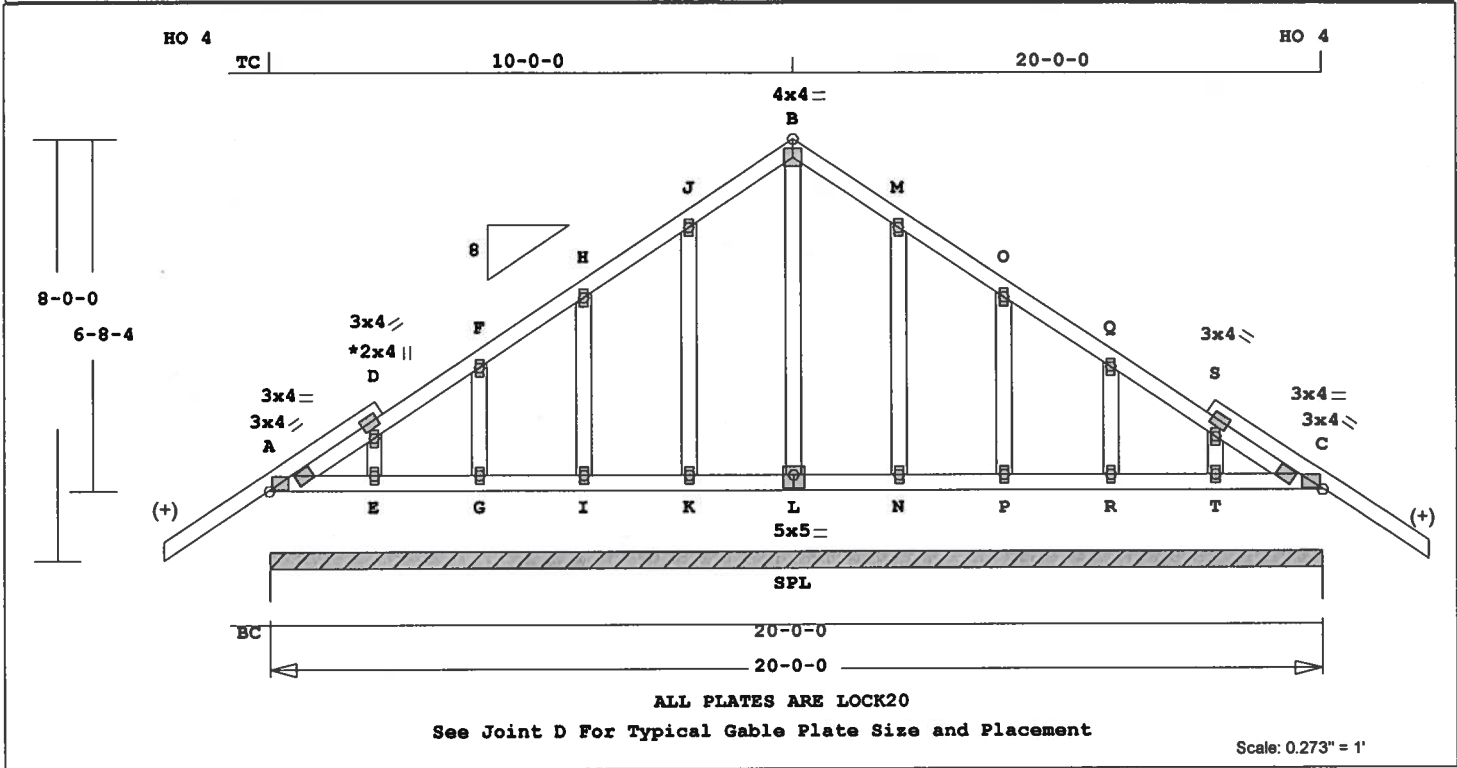
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
User-defined wind-exposed BC  
regions --From-- --To--  
0-0-0 20-0-0  
Max comp. force 1066 Lbs  
Max tens. force 964 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555



Job <b>ELIXSON-WILL</b>	Mark <b>D2</b>	Quan <b>1</b>	Type <b>TR</b>	Span <b>20000'</b>	P1-H1 <b>8</b>	Left OH <b>0</b>	Right OH <b>0</b>	Engineering <b>T2566276</b>
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 154.1 LBS  
 Online Plus -- Version 21.0.032  
 RUN DATE: 05-JUN-07

CSI -Size- ---Lumber---  
 TC 0.03 2x 4 SP-#2 (+)  
 BC 0.02 2x 4 SP-#2  
 GW 0.04 2x 4 SP-#2

Brace truss as follows:  
 O.C. From To  
 TC Cont. 0- 0- 0 20- 0- 0  
 BC Cont. 0- 0- 0 20- 0- 0

psf-Ld Dead Live  
 TC 10.0 20.0  
 BC 10.0 0.0  
 TC+BC 20.0 20.0  
 Total 40.0 Spacing 24.0"  
 Lumber Duration Factor 1.25  
 Plate Duration Factor 1.25  
 TC Fb=1.15 Fc=1.10 Ft=1.10  
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)  
 Jt Down Uplift Horiz-  
 A 1600 104 U 133 R

Jt Brg Size Required  
 A 240.0" 0"-to- 240"

Plus 9 Wind Load Case(s)  
 Plus 1 UBC LL Load Case(s)

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -D	0.03	103	C	0.00	0.03
D -F	0.03	74	C	0.00	0.03
F -H	0.03	50	C	0.00	0.03
H -J	0.03	53	C	0.00	0.03
J -B	0.03	107	T	0.00	0.03
B -M	0.03	107	T	0.00	0.03
M -O	0.03	53	C	0.00	0.03
O -Q	0.03	50	C	0.00	0.03
Q -S	0.03	74	C	0.00	0.03
S -C	0.03	103	C	0.00	0.03
-----Bottom Chords-----					
A -E	0.02	13	T	0.00	0.02
E -G	0.02	0	T	0.00	0.02
G -I	0.02	0	T	0.00	0.02
I -K	0.02	0	T	0.00	0.02
K -L	0.02	0	T	0.00	0.02
L -N	0.02	0	T	0.00	0.02
N -P	0.02	0	T	0.00	0.02
P -R	0.02	0	T	0.00	0.02
R -T	0.02	0	T	0.00	0.02

T -C	0.02	13	T	0.00	0.02
-----Gable Webs-----					
E -D	0.01	122	C		
G -F	0.01	121	C		
I -H	0.02	119	C		
K -J	0.04	123	C		
L -B	0.03	69	C		
N -M	0.04	123	C		
P -O	0.02	119	C		
R -Q	0.01	121	C		
T -S	0.01	122	C		

TL Defl 0.00" in T -C L/999  
 LL Defl 0.00" in E -G L/999  
 Shear // Grain in D -D 0.06

Plates for each ply each face.  
 PLATING CONFORMS TO TPI.  
 REPORTS: SBCCI 9761  
 ROBBINS ENGINEERING, INC.  
 BASED ON SP LUMBER  
 USING GROSS AREA TEST.  
 Plate - LOCK 20 Ga, Gross Area  
 Plate - RHS 20 Ga, Gross Area  
 Jt Type Plt Size X Y JSI  
 A LOCK 3.0x 4.0 Ctr Ctr 0.72  
 D LOCK 2.0x 4.0 Ctr Ctr 0.00  
 F LOCK 2.0x 4.0 Ctr Ctr 0.00  
 H LOCK 2.0x 4.0 Ctr Ctr 0.00  
 J LOCK 2.0x 4.0 Ctr Ctr 0.00  
 B LOCK 4.0x 4.0 Ctr Ctr 0.62  
 M LOCK 2.0x 4.0 Ctr Ctr 0.00  
 O LOCK 2.0x 4.0 Ctr Ctr 0.00  
 Q LOCK 2.0x 4.0 Ctr Ctr 0.00  
 S LOCK 2.0x 4.0 Ctr Ctr 0.00  
 C LOCK 3.0x 4.0 Ctr Ctr 0.72  
 E LOCK 2.0x 4.0 Ctr Ctr 0.00  
 G LOCK 2.0x 4.0 Ctr Ctr 0.00  
 I LOCK 2.0x 4.0 Ctr Ctr 0.00  
 K LOCK 2.0x 4.0 Ctr Ctr 0.00  
 L LOCK 5.0x 5.0 Ctr-0.5 0.57  
 N LOCK 2.0x 4.0 Ctr Ctr 0.00  
 P LOCK 2.0x 4.0 Ctr Ctr 0.00  
 R LOCK 2.0x 4.0 Ctr Ctr 0.00  
 T LOCK 2.0x 4.0 Ctr Ctr 0.00

REVIEWED BY:  
 Robbins Engineering, Inc.  
 6904 Parke East Blvd.  
 Tampa, FL 33610

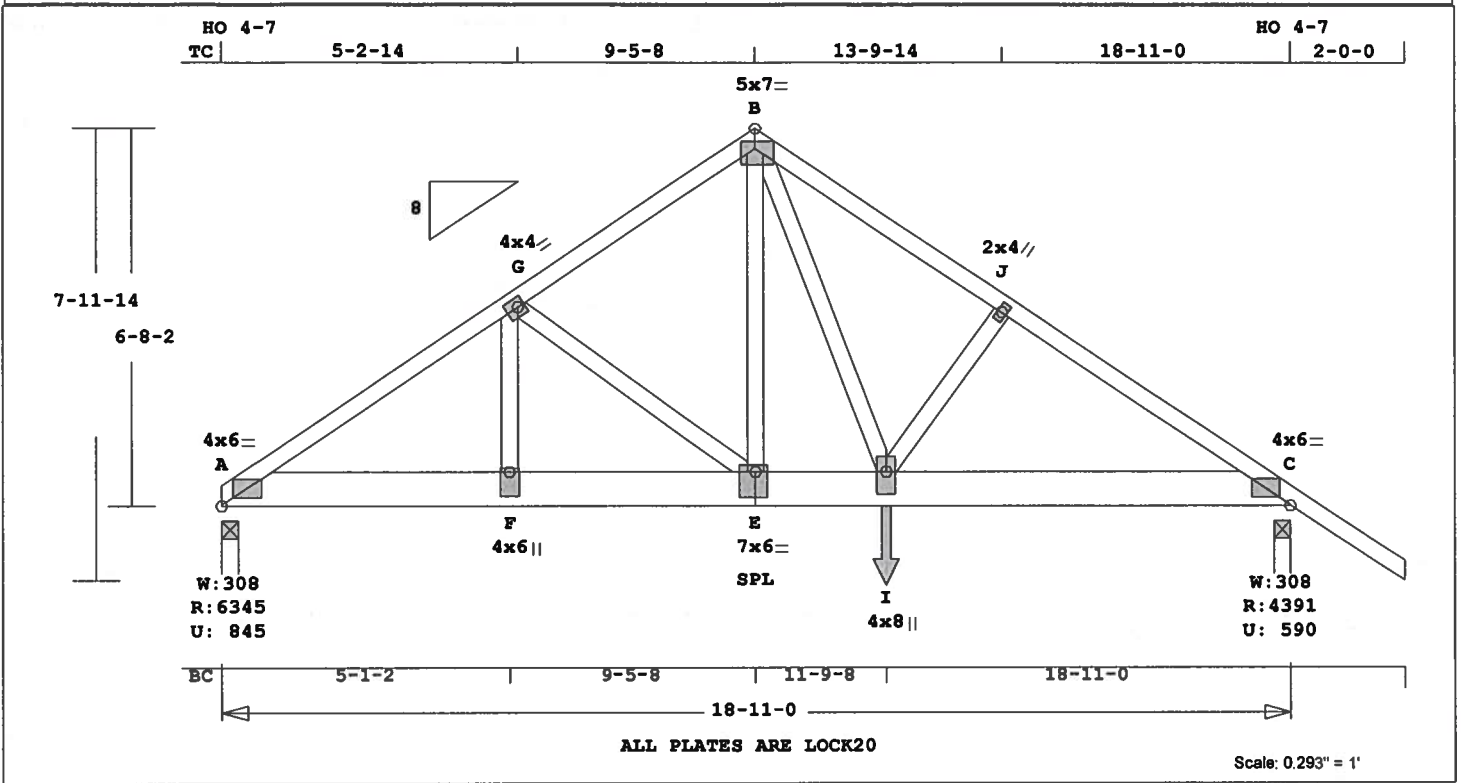
REFER TO ROBBINS ENG. GENERAL  
 NOTES AND SYMBOLS SHEET FOR  
 ADDITIONAL SPECIFICATIONS.

NOTES:  
 Trusses Manufactured by:  
 Mayo Truss Co. Inc.  
 Analysis Conforms To:  
 FBC2004  
 WARNING Do Not Cut overframe  
 member between outside of  
 truss and first tie-plate  
 to inside of heel plate.  
 Design checked for 10 psf non-  
 concurrent LL on BC.  
 Refer to Gen Det 3 series for  
 web bracing and plating.  
 Wind Loads - ANSI / ASCE 7-02  
 Truss is designed as  
 Components and Claddings\*  
 for Exterior zone location.  
 Wind Speed: 110 mph  
 Mean Roof Height: 15'-0  
 Exposure Category: B  
 Occupancy Factor : 1.00  
 Building Type: Open  
 TC Dead Load: 5.0 psf  
 BC Dead Load: 5.0 psf  
 Max comp. force 123 Lbs  
 Max tens. force 107 Lbs  
 Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
 Robbins Engineering  
 6904 Parke East Blvd  
 Tampa, FL, 33610  
 FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>E1</b>	Quan 1*2P	Type TR	Span 181100'	Pl-H1 8	Left OH 0	Right OH 2- 0- 0	Engineering T2566277
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TONY WILLIAMS



ALL PLATES ARE LOCK20

Scale: 0.293" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 163.4 LBS

Online Plus -- Version 21.0.032  
 RUN DATE: 05-JUN-07  
 \*\*\*\*\*  
 \* 2-Ply Truss \*  
 \*\*\*\*\*

CSI	-Size-	-----Lumber-----
TC	0.47	2x 4 SP-#2
BC	0.86	2x 8 SP-#2
WB	0.36	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	18-11- 0
BC Cont.	0- 0- 0	18-11- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.00	Fc=1.00	Ft=1.00
BC Fb=1.00	Fc=1.00	Ft=1.00

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	6346	845 U	131 R
C	4391	590 U	130 R

Jt	Brg Size	Required
A	3.5"	3.5"
C	3.5"	2.6"

LC# 1 Standard Loading

Dur	Fctrs	- Lbr	1.25	Plt	1.25
plf	- Dead	Live*	From	To	
TC V	20	40	0.0'	18.9'	
BC V	20	0	0.0'	18.9'	
BC V	293	293	0.0'	10.9'	
BC V	1346	1346	11.8'	CL-LB	

Plus 9 Wind Load Case(s)  
 Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A	-G	0.47	8579 C	0.22 0.25
B	-B	0.35	5800 C	0.08 0.27
G	-J	0.32	7096 C	0.14 0.18

J -C	0.25	7235 C	0.15	0.10
-----Bottom Chords-----				
A -F	0.86	7152 T	0.40	0.46
F -E	0.64	7152 T	0.40	0.24
E -I	0.42	4787 T	0.27	0.15
I -C	0.44	6026 T	0.34	0.10
-----Webs-----				
F -G	0.28	3120 T		
G -E	0.18	2877 C		
E -B	0.36	3943 T		
B -I	0.28	3120 T		
I -J	0.01	255 C		

TL Defl -0.17" in F -E L/999  
 LL Defl -0.08" in F -E L/999  
 Shear // Grain in A -F 0.52

Plates for each ply each face.  
 PLATING CONFORMS TO TPI.  
 REPORTS: SBCCI 9761  
 ROBBINS ENGINEERING, INC.  
 BASED ON SP LUMBER  
 USING GROSS AREA TEST.  
 Plate - LOCK 20 Ga, Gross Area  
 Plate - RHS 20 Ga, Gross Area  
 Jt Type Plt Size X Y JSI  
 A LOCK 4.0x 6.0 Ctr Ctr 0.99  
 G LOCK 4.0x 4.0-0.4-0.3 0.82  
 B LOCK 5.0x 7.0 0.5-1.0 0.95  
 J LOCK 2.0x 4.0 Ctr Ctr 0.37  
 C LOCK 4.0x 6.0 Ctr Ctr 0.84  
 F LOCK 4.0x 6.0 Ctr-2.2 0.85  
 E LOCK 7.0x 6.0-0.5-2.0 0.88  
 I LOCK 4.0x 8.0 Ctr-0.8 0.76

REVIEWED BY:  
 Robbins Engineering, Inc.  
 6904 Parke East Blvd.  
 Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
 NOTES AND SYMBOLS SHEET FOR  
 ADDITIONAL SPECIFICATIONS.

NOTES:  
 Trusses Manufactured by:  
 Mayo Truss Co. Inc.  
 Analysis Conforms To:  
 FBC2004  
 2 COMPLETE TRUSSES REQUIRED.  
 Fasten together in staggered

pattern. (1/2" bolts -OR-  
 SDS3 screws -OR- 10d nails  
 as each layer is applied.)  
 ----Spacing (in)----  

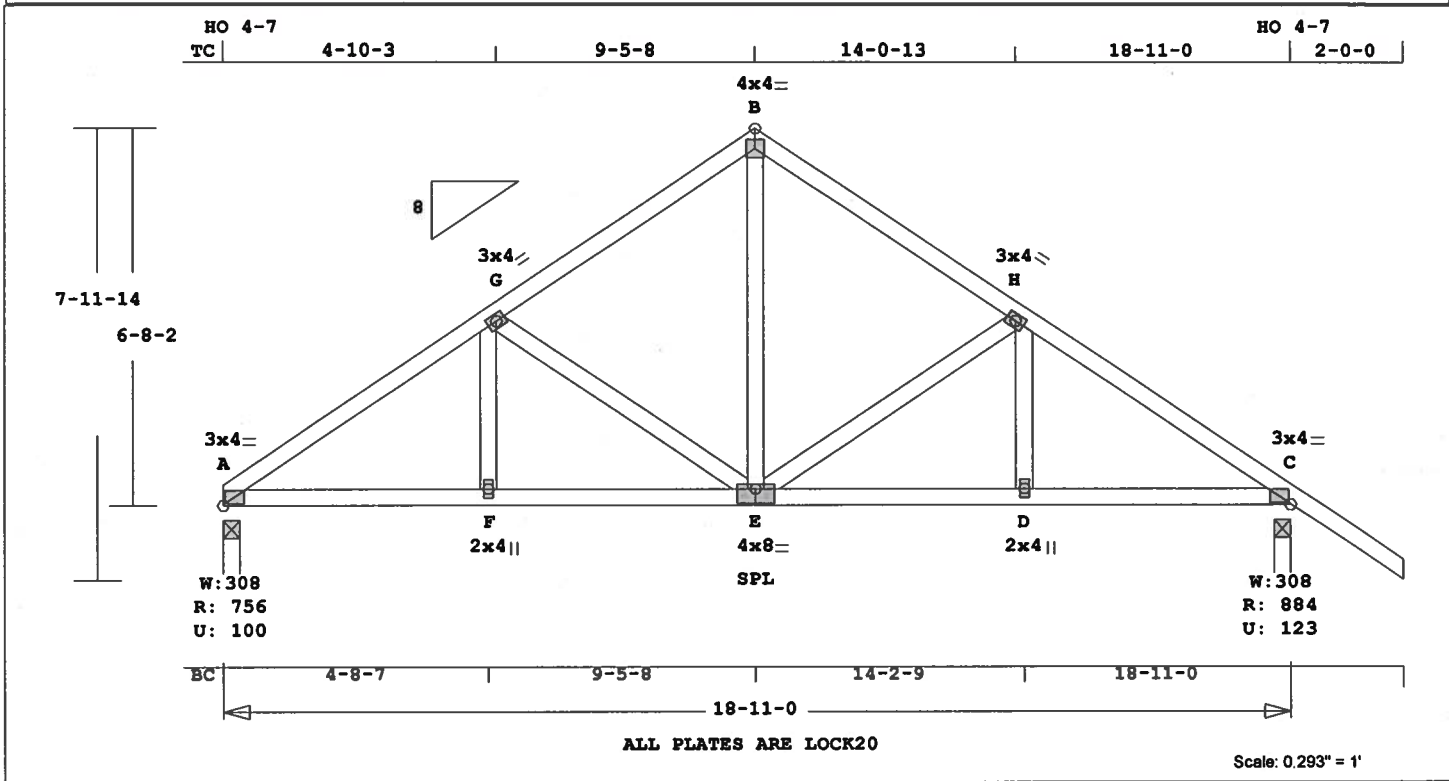
Rows	Nails	Screws	Bolts
TC 1	12	24	0
BC 2	12	23.5	0
WB 1	8	8	

 Plus clusters of nails where  
 shown.  
 OH Loading  
 Soffit psf 2.0  
 Design checked for 10 psf non-  
 concurrent LL on BC.  
 Wind Loads - ANSI / ASCE 7-02  
 Truss is designed as  
 Components and Claddings\*  
 for Exterior zone location.  
 Wind Speed: 110 mph  
 Mean Roof Height: 15-0  
 Exposure Category: B  
 Occupancy Factor : 1.00  
 Building Type: Enclosed  
 TC Dead Load: 5.0 psf  
 BC Dead Load: 5.0 psf  
 Max comp. force 8579 Lbs  
 Max tens. force 7152 Lbs  
 Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
 Robbins Engineering  
 6904 Parke East Blvd  
 Tampa, FL, 33610  
 FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>E2</b>	Quan 2	Type TR	Span 181100'	P1-H1 8	Left OH 0	Right OH 2-0-0	Engineering T2566278
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 126.7 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

	TC	BC	WB
CSI -Size-	2x 4	2x 4	2x 4
Lumber----	SP-#2	SP-#2	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	18-11- 0
BC Cont.	0- 0- 0	18-11- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	24.0
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=	1.15	Fc=1.10 Ft=1.10
BC Fb=	1.10	Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	757	101 U	132 R
C	885	123 U	132 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
-----Top Chords-----			
A -G	0.19	1012 C	0.00 0.19
G -B	0.19	684 C	0.00 0.19
B -H	0.19	684 C	0.00 0.19
H -C	0.19	1012 C	0.00 0.19
-----Bottom Chords-----			
A -F	0.18	846 T	0.14 0.04
F -E	0.21	846 T	0.14 0.07

E -D	0.21	846 T	0.14	0.07
D -C	0.18	846 T	0.14	0.04
-----Webs-----				
F -G	0.02	182 T		
G -E	0.13	338 C		
E -B	0.08	477 T		
E -H	0.13	338 C		
D -H	0.02	182 T		

TL Defl	-0.05"	in E -D	L/999
LL Defl	-0.02"	in E -D	L/999
Shear // Grain		in G -B	0.16

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.71  
G LOCK 3.0x 4.0 Ctr Ctr 0.60  
B LOCK 4.0x 4.0 Ctr Ctr 0.60  
H LOCK 3.0x 4.0 Ctr Ctr 0.60  
C LOCK 3.0x 4.0 Ctr Ctr 0.71  
F LOCK 2.0x 4.0 Ctr Ctr 0.40  
E LOCK 4.0x 8.0 Ctr-1.0 0.56  
D LOCK 2.0x 4.0 Ctr Ctr 0.40

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

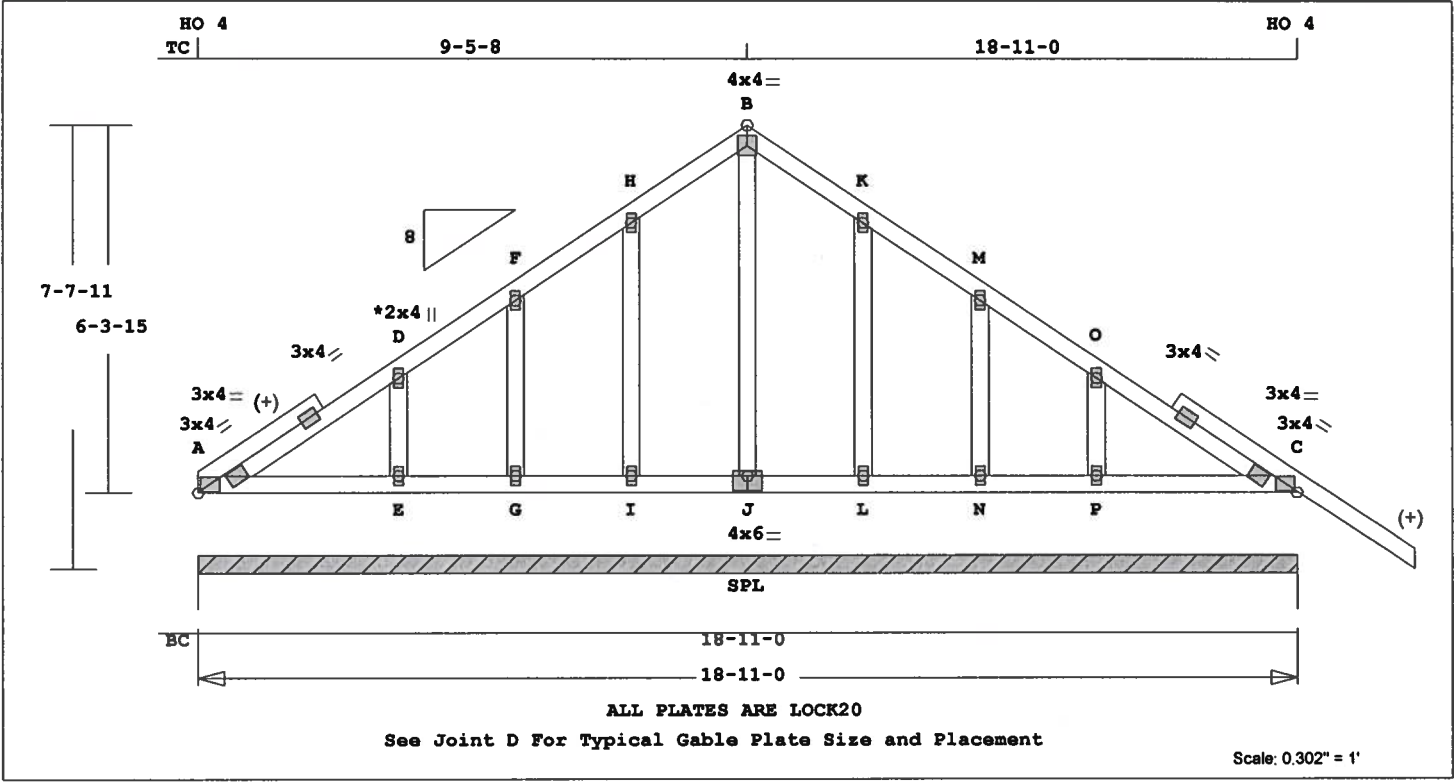
REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.  
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:

FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 1012 Lbs  
Max tens. force 846 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>E3</b>	Quan 1	Type TR	Span 181100'	Pl-H1 8	Left OH 0	Right OH 0	Engineering T2566279
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 137.2 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI	-Size-	---	Lumber----
TC	0.07	2x 4	SP-#2 (+)
BC	0.06	2x 4	SP-#2
GW	0.04	2x 4	SP-#2

Brace truss as follows:  
O.C. From To  
TC Cont. 0- 0- 0 18-11- 0  
BC Cont. 0- 0- 0 18-11- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (lbs)  
Jt Down Uplift Horiz-  
A 1513 202 U 125 R

Jt Brg Size Required  
A 227.0" 0"-to- 227"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
-----Top Chords-----					
A -D	0.07	79	C	0.00	0.07
D -F	0.07	101	C	0.00	0.07
F -H	0.03	104	C	0.00	0.03
H -B	0.03	163	C	0.00	0.03
B -K	0.03	163	C	0.00	0.03
K -M	0.03	104	C	0.00	0.03
M -O	0.07	101	C	0.00	0.07
O -C	0.07	79	C	0.00	0.07
-----Bottom Chords-----					
A -E	0.06	8	T	0.00	0.06
E -G	0.05	0	T	0.00	0.05
G -I	0.02	0	T	0.00	0.02
I -J	0.02	0	T	0.00	0.02
J -L	0.02	0	T	0.00	0.02
L -N	0.02	0	T	0.00	0.02
N -P	0.05	0	T	0.00	0.05

P -C	0.06	8	T	0.00	0.06
-----Gable Webs-----					
E -D	0.01	181	C		
G -F	0.01	110	C		
I -H	0.03	125	C		
J -B	0.04	97	C		
L -K	0.03	125	C		
N -M	0.01	110	C		
P -O	0.01	181	C		

TL Defl -0.01" in A -E L/999  
LL Defl 0.00" in A -E L/999  
Shear // Grain in A -D 0.11

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.71  
D LOCK 2.0x 4.0 Ctr Ctr 0.00  
F LOCK 2.0x 4.0 Ctr Ctr 0.00  
H LOCK 2.0x 4.0 Ctr Ctr 0.00  
B LOCK 4.0x 4.0 Ctr Ctr 0.60  
K LOCK 2.0x 4.0 Ctr Ctr 0.00  
M LOCK 2.0x 4.0 Ctr Ctr 0.00  
O LOCK 2.0x 4.0 Ctr Ctr 0.00  
C LOCK 3.0x 4.0 Ctr Ctr 0.71  
E LOCK 2.0x 4.0 Ctr Ctr 0.00  
G LOCK 2.0x 4.0 Ctr Ctr 0.00  
I LOCK 2.0x 4.0 Ctr Ctr 0.00  
J LOCK 4.0x 6.0 Ctr-1.0 0.56  
L LOCK 2.0x 4.0 Ctr Ctr 0.00  
N LOCK 2.0x 4.0 Ctr Ctr 0.00  
P LOCK 2.0x 4.0 Ctr Ctr 0.00

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

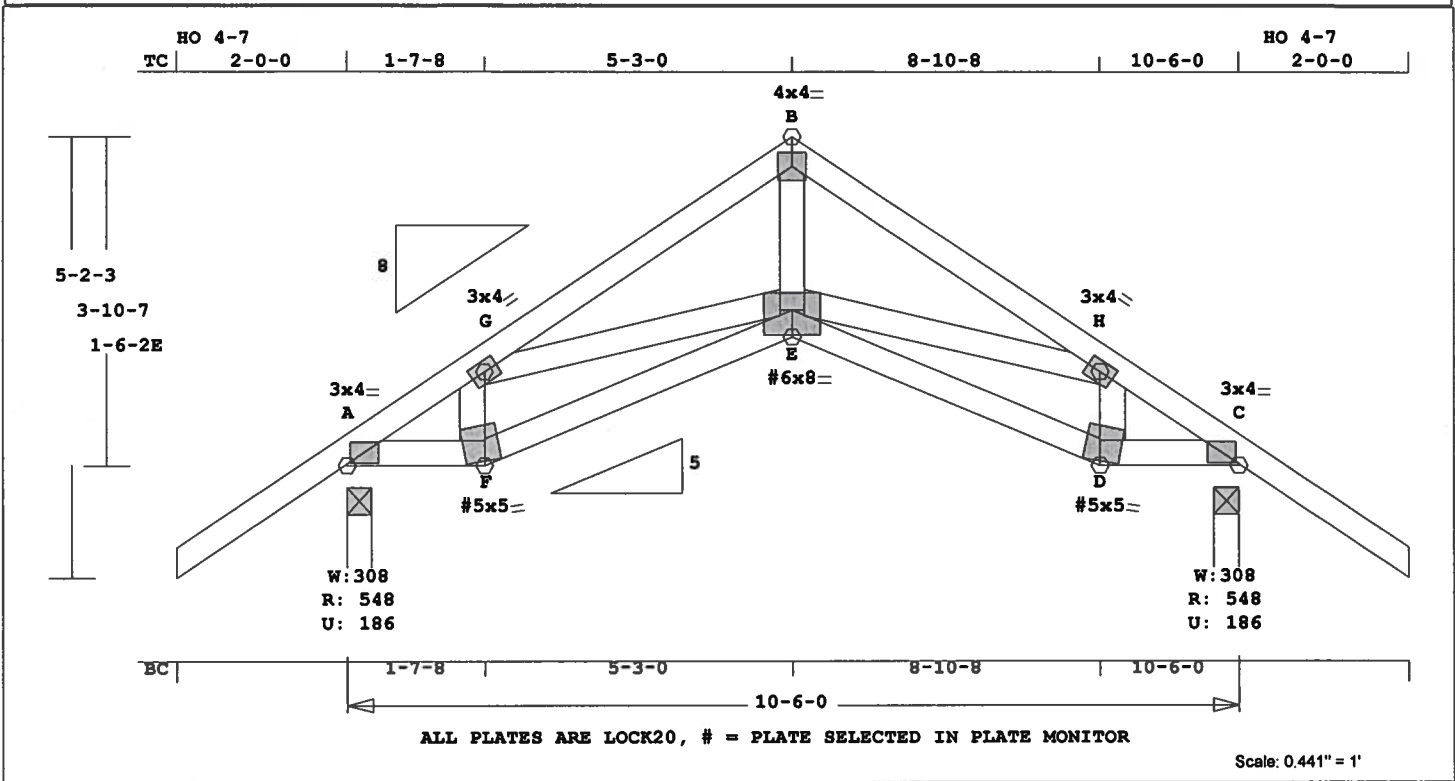
REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.  
NOTES:  
Trusses Manufactured by:

Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
WARNING Do Not Cut overframe  
member between outside of  
truss and first tie-plate  
to inside of heel plate.  
Design checked for 10 psf non-  
concurrent LL on BC.  
Refer to Gen Det 3 series for  
web bracing and plating.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 181 Lbs  
Max tens. force 148 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>F1</b>	Quan 2	Type CA55	Span 100600'	P1-H1 8	Left OH 2-0-0	Right OH 2-0-0	Engineering <b>T2566280</b>
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 75.1 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI	-Size-	----	Lumber	----
TC	0.15	2x 4	SP-#2	
BC	0.13	2x 4	SP-#2	
WB	0.09	2x 4	SP-#2	

Brace truss as follows:

	O.C.	From	To
TC Cont.	0-0-0	10-6-0	
BC Cont.	0-0-0	10-6-0	

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	548	186 U	70 R
C	548	186 U	70 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI	Bnd
-----Top Chords-----					
A -G	0.13	674 T	0.08	0.05	
G -B	0.15	661 C	0.08	0.07	
B -H	0.15	661 C	0.08	0.07	
H -C	0.13	674 T	0.08	0.05	
-----Bottom Chords-----					
A -F	0.12	535 C	0.08	0.04	
F -E	0.13	535 T	0.09	0.04	
E -D	0.13	535 T	0.09	0.04	
D -C	0.12	535 C	0.08	0.04	
-----Webs-----					

F -G	0.01	184 C
G -E	0.01	109 T
E -B	0.09	562 C
E -H	0.01	109 T
D -H	0.01	184 C

TL Defl	-0.03"	in E -D	L/999
LL Defl	-0.01"	in E -D	L/999
Hz Disp	LL	DL	TL
Jt C	0.01"	0.01"	0.02"
Shear // Grain	in G -B	0.11	

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.58  
G LOCK 3.0x 4.0 Ctr Ctr 0.49  
B LOCK 4.0x 4.0 Ctr Ctr 0.49  
H LOCK 3.0x 4.0 Ctr Ctr 0.49  
C LOCK 3.0x 4.0 Ctr Ctr 0.58  
F# LOCK 5.0x 5.0-0.6 3.1 0.41  
E# LOCK 6.0x 8.0 Ctr-0.5 0.42  
D# LOCK 5.0x 5.0 0.5 3.1 0.40

# = Plate Monitor used

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.  
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

NOTE: USER MODIFIED PLATES  
This design may have plates selected through a plate monitor.

Wind Loads - ANSI / ASCE 7-02  
Truss is designed as Components and Claddings\* for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0

Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf

User-defined wind-exposed BC regions --From-- ---To---

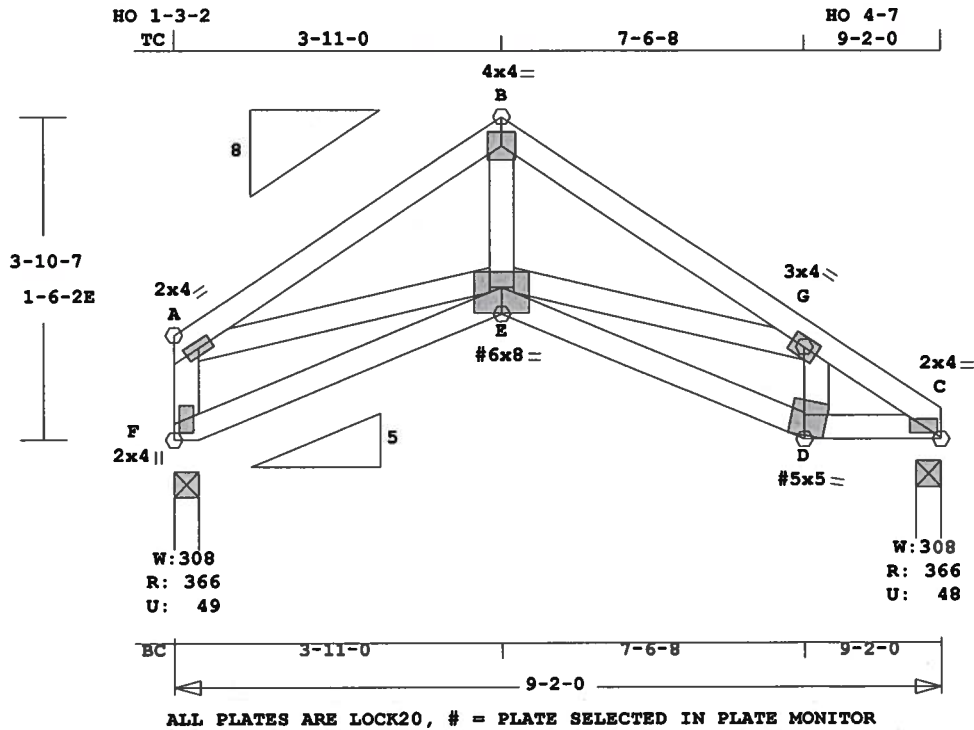
0-0-0 10-6-0  
Max comp. force 661 Lbs  
Max tens. force 674 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555



Job <b>ELIXSON-WILL</b>	Mark <b>F3</b>	Quan 2	Type SP	Span 90200'	P1-H1 8	Left OH 0	Right OH 0	Engineering <b>T2566282</b>
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TONY WILLIAMS



Scale: 0.433" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 60.6 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

	CSI	Size	Lumber
TC	0.14	2x 4	SP-#2
BC	0.16	2x 4	SP-#2
WB	0.08	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	9- 2- 0	
BC Cont.	0- 0- 0	9- 2- 0	

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15 Fc=1.10 Ft=1.10		
BC Fb=1.10 Fc=1.10 Ft=1.10		

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
F	367	49 U	85 R
C	367	48 U	68 R

Jt Brg Size Required

Jt	Brg Size	Required
F	3.5"	1.5"
C	3.5"	1.5"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Axl-C SI-Bnd

-----Top Chords-----				
A -B	0.14	523 C	0.00	0.14
B -G	0.11	528 C	0.00	0.11
G -C	0.08	482 C	0.02	0.06
-----Bottom Chords-----				
F -E	0.11	77 T	0.00	0.11
E -D	0.16	445 T	0.07	0.09
D -C	0.09	406 T	0.06	0.03

-----Webs-----			
F -A	0.03	317 C	WindLd
A -E	0.08	448 T	
E -B	0.06	354 T	
E -G	0.01	134 T	
D -G	0.01	167 C	

TL Defl	-0.03"	in F -E	L/999
LL Defl	-0.01"	in F -E	L/999
Hz Disp	LL	DL	TL
Jt C	0.01"	0.01"	0.01"
Shear // Grain	in A -B 0.13		

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	LOCK	20 Ga,	Gross Area	
Plate - RHS	20 Ga,	Gross Area		
Jt Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr Ctr	0.79
B	LOCK	4.0x 4.0	Ctr Ctr	0.47
G	LOCK	3.0x 4.0	Ctr Ctr	0.47
C	LOCK	2.0x 4.0	Ctr Ctr	0.68
F	LOCK	2.0x 4.0	Ctr Ctr	0.38
E#	LOCK	6.0x 8.0	Ctr-0.7	0.38
D#	LOCK	5.0x 5.0	0.6 3.0	0.38

# = Plate Monitor used

REVIEWED BY:

Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:

FBC2004

Design checked for 10 psf non-concurrent LL on BC.

NOTE: USER MODIFIED PLATES  
This design may have plates selected through a plate monitor.

Wind Loads - ANSI / ASCE 7-02  
Truss is designed as

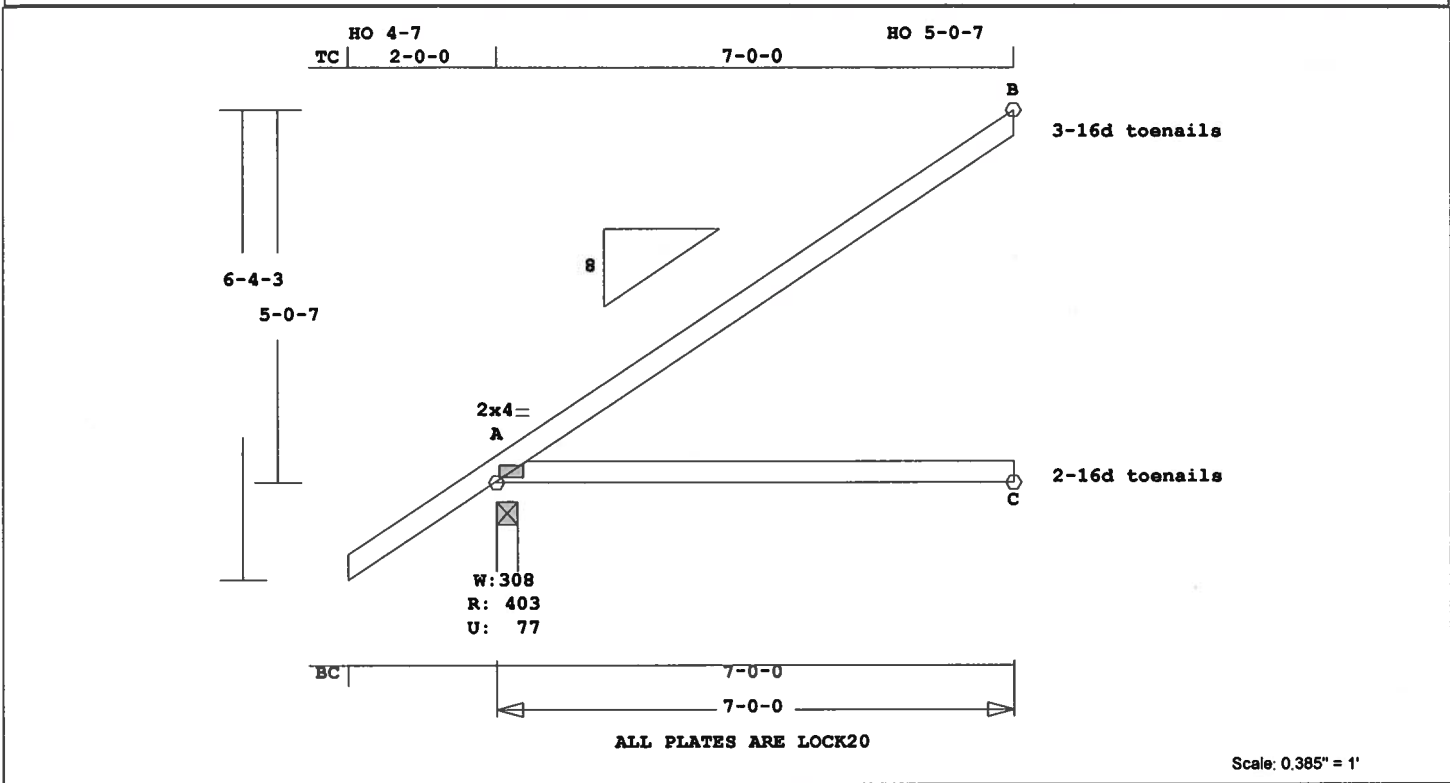
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 528 Lbs  
Max tens. force 448 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555





TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 34.0 LBS

Online Plus -- Version 21.0.032  
 RUN DATE: 05-JUN-07

CSI -Size- ----Lumber----  
 TC 0.47 2x 4 SP-#2  
 BC 0.35 2x 4 SP-#2

Brace truss as follows:  
 O.C. From To  
 TC Cont. 0- 0- 0 7- 0- 0  
 BC Cont. 0- 0- 0 7- 0- 0

psf-Ld Dead Live  
 TC 10.0 20.0  
 BC 10.0 0.0  
 TC+BC 20.0 20.0  
 Total 40.0 Spacing 24.0"  
 Lumber Duration Factor 1.25  
 Plate Duration Factor 1.25  
 TC Fb=1.15 Fc=1.10 Ft=1.10  
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)  
 Jt Down Uplift Horiz-  
 A 404 78 U 228 R  
 C 133  
 B 213 104 U 97 R

Jt Brg Size Required  
 A 3.5" 1.5"  
 C 3.5" 1.5"  
 B 3.5" 1.5"

Plus 8 Wind Load Case(s)  
 Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd  
 Top Chords  
 A -B 0.47 124 C 0.00 0.47  
 B -B 0.00 4 C

-----Bottom Chords-----  
 A -C 0.35 0 T 0.00 0.35

TL Defl -0.19" in A -C L/415  
 LL Defl -0.08" in A -C L/999  
 Shear // Grain in A -B 0.22

Plates for each ply each face.  
 PLATING CONFORMS TO TPI.  
 REPORTS: SBCCI 9761  
 ROBBINS ENGINEERING, INC.  
 BASED ON SP LUMBER  
 USING GROSS AREA TEST.  
 Plate - LOCK 20 Ga, Gross Area  
 Plate - RHS 20 Ga, Gross Area  
 Jt Type Plt Size X Y JSI  
 A LOCK 2.0x 4.0 Ctr Ctr 0.64

REVIEWED BY:  
 Robbins Engineering, Inc.  
 6904 Parke East Blvd.  
 Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
 NOTES AND SYMBOLS SHEET FOR  
 ADDITIONAL SPECIFICATIONS.

For proper installation of  
 toe-nails, refer to the 2001  
 National Design Specification  
 (NDS) for Wood Construction

NOTES:

Trusses Manufactured by:  
 Mayo Truss Co. Inc.  
 Analysis Conforms To:

FBC2004  
 OH Loading

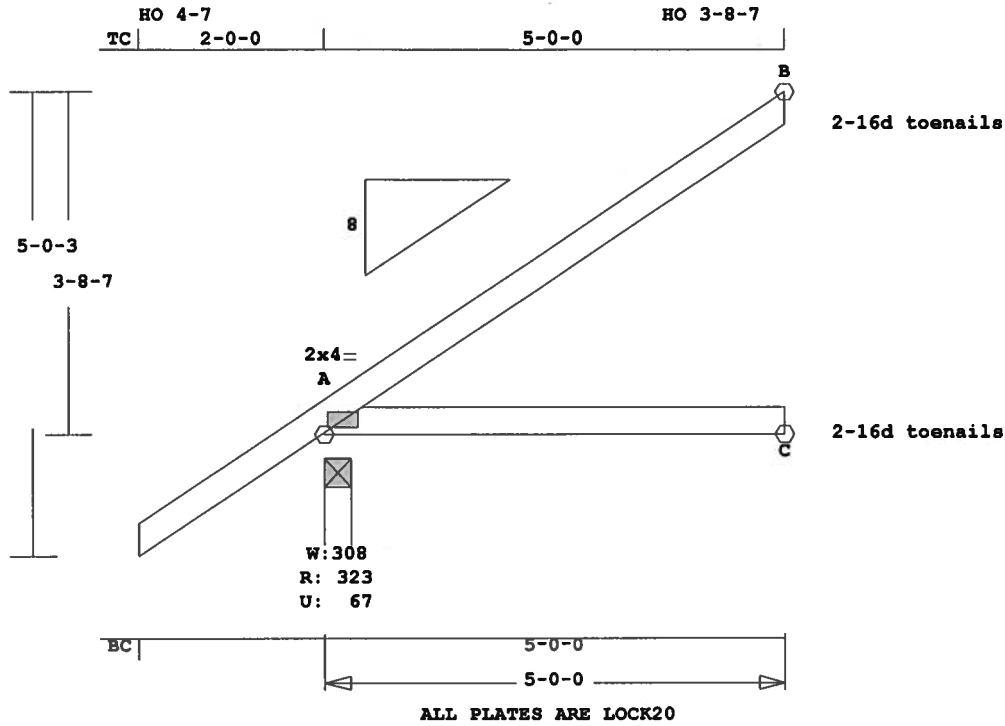
Soffit psf 2.0  
 Design checked for 10 psf non-

concurrent LL on BC.  
 Wind Loads - ANSI / ASCE 7-02  
 Truss is designed as  
 Components and Claddings\*  
 for Exterior zone location.  
 Wind Speed: 110 mph  
 Mean Roof Height: 15-0  
 Exposure Category: B  
 Occupancy Factor : 1.00  
 Building Type: Enclosed  
 TC Dead Load: 5.0 psf  
 BC Dead Load: 5.0 psf  
 Max comp. force 124 Lbs  
 Max tens. force 98 Lbs  
 Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
 Robbins Engineering  
 6904 Parke East Blvd  
 Tampa, FL, 33610  
 Ft.Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>J2</b>	Quan 2	Type JCA2	Span 5000'	P1-H1 8	Left OH 2- 0- 0	Right OH 0	Engineering T2566285
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TONY WILLIAMS



Scale: 0.479" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 25.7 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI	Size	Lumber
TC	0.22 2x 4	SP-#2
BC	0.16 2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 0
BC Cont.	0- 0- 0	5- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	324	68 U	168 R
C	95		
B	158	76 U	69 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"
B	3.5"	1.5"

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -B	0.22	93 C	0.00	0.22
B -B	0.00	4 C		

-----Bottom Chords-----  
A -C 0.16 0 T 0.00 0.16

TL Defl -0.04" in A -C L/999  
LL Defl -0.02" in A -C L/999  
Shear // Grain in A -B 0.14

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 2.0x 4.0 Ctr Ctr 0.60

REVIEWED BY:

Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

For proper installation of  
toe-nails, refer to the 2001  
National Design Specification  
(NDS) for Wood Construction

NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-

concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings\*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

Max comp. force 93 Lbs

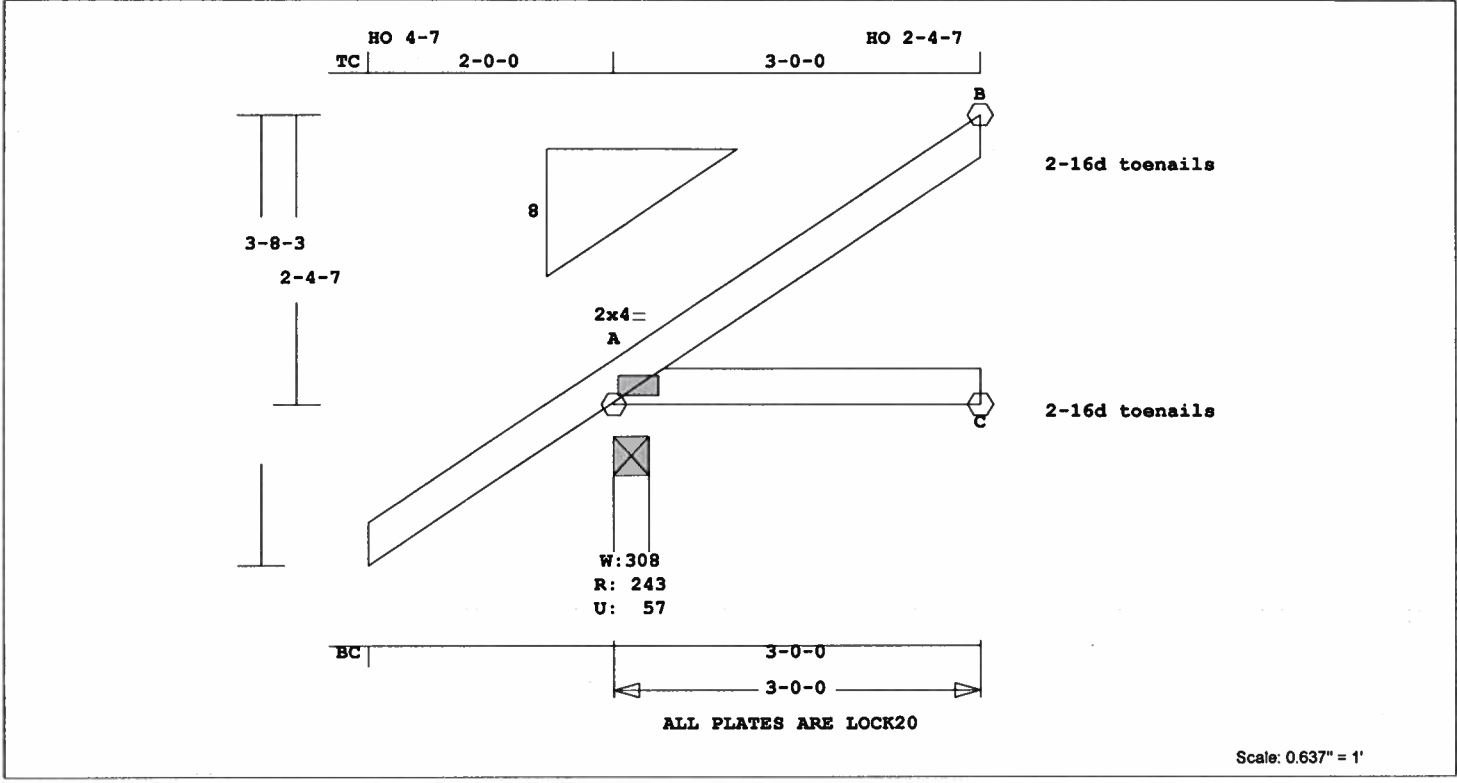
Max tens. force 73 Lbs

Quality Control Factor 1.25

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Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>J3</b>	Quan 2	Type JCA2	Span 30000'	P1-H1 8	Left OH 2- 0- 0	Right OH 0	Engineering T2566286
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 17.4 LBS

Online Plus -- Version 21.0.032  
 RUN DATE: 05-JUN-07

CSI	Size	Lumber
TC	0.06	2x 4 SP-#2
BC	0.05	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 0
BC Cont.	0- 0- 0	3- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	243	57 U	100 R
C	57		
B	99	48 U	41 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"
B	3.5"	1.5"

Plus 8 Wind Load Case(s)  
 Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
<b>Top Chords</b>				
A - B	0.06	58 C	0.00	0.06
B - B	0.00	4 C		

-----Bottom Chords-----  
 A - C 0.05 0 T 0.00 0.05

TL Defl 0.00" in A - C L/999  
 LL Defl 0.00" in A - C L/999  
 Shear // Grain in A - B 0.07

Plates for each ply each face.  
 PLATING CONFORMS TO TPI.  
 REPORTS: SBCCI 9761  
 ROBBINS ENGINEERING, INC.  
 BASED ON SP LUMBER  
 USING GROSS AREA TEST.  
 Plate - LOCK 20 Ga, Gross Area  
 Plate - RHS 20 Ga, Gross Area  
 Jt Type Plt Size X Y JSI  
 A LOCK 2.0x 4.0 Ctr Ctr 0.59

REVIEWED BY:  
 Robbins Engineering, Inc.  
 6904 Parke East Blvd.  
 Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
 NOTES AND SYMBOLS SHEET FOR  
 ADDITIONAL SPECIFICATIONS.

For proper installation of  
 toe-nails, refer to the 2001  
 National Design Specification  
 (NDS) for Wood Construction

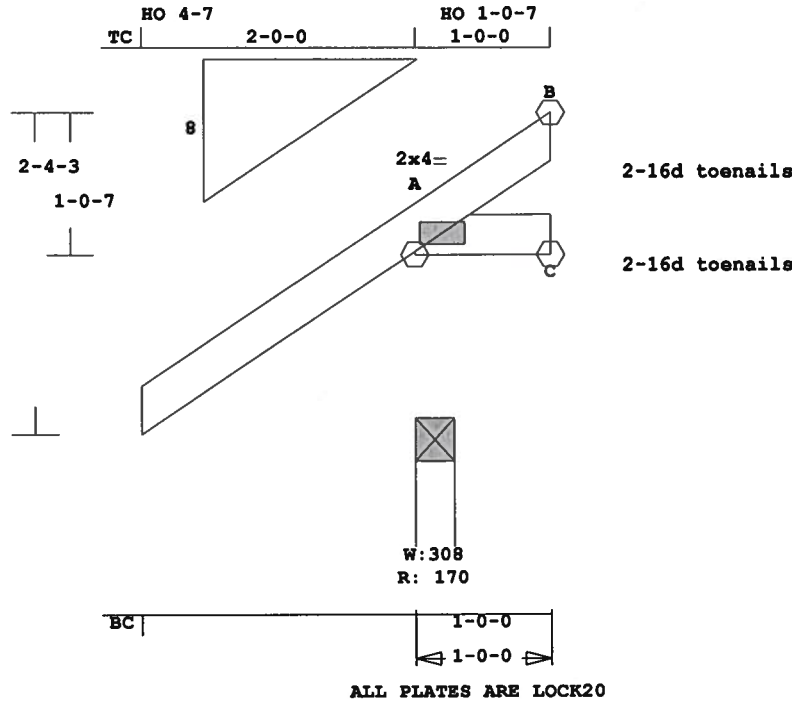
NOTES:  
 Trusses Manufactured by:  
 Mayo Truss Co. Inc.  
 Analysis Conforms To:  
 FBC2004  
 OH Loading  
 Soffit psf 2.0  
 Design checked for 10 psf non-

concurrent LL on BC.  
 Wind Loads - ANSI / ASCE 7-02  
 Truss is designed as  
 Components and Claddings\*  
 for Exterior zone location.  
 Wind Speed: 110 mph  
 Mean Roof Height: 15-0  
 Exposure Category: B  
 Occupancy Factor : 1.00  
 Building Type: Enclosed  
 TC Dead Load: 5.0 psf  
 BC Dead Load: 5.0 psf  
 Max comp. force 58 Lbs  
 Max tens. force 46 Lbs  
 Quality Control Factor 1.25

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 Robbins Engineering  
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 FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>J4</b>	Quan 2	Type JCA2	Span 10000'	P1-H1 8	Left OH 2- 0- 0	Right OH 0	Engineering T2566287
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TONY WILLIAMS



Scale: 0.710" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 9.1 LBS

Online Plus -- Version 21.0.032 A -C 0.00 13 T  
RUN DATE: 05-JUN-07

CSI -Size- ----Lumber----  
TC 0.00 2x 4 SP-#2  
BC 0.00 2x 4 SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	1- 0- 0	
BC Cont.	0- 0- 0	1- 0- 0	

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"

Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	170		32 R
B	33	16 U	
C	14		13 R

Jt	Brg Size	Required
A	3.5"	1.5"
B	1.5"	1.5"
C	1.5"	1.5"

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
-----Top Chords-----			
A -B	0.00	18 C	
-----Bottom Chords-----			

TL Defl 0.00" in A -C L/999  
LL Defl 0.00" in A -C L/999  
Shear // Grain in A -B 0.02

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 2.0x 4.0 Ctr Ctr 0.59

REVIEWED BY:

Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

For proper installation of  
toe-nails, refer to the 2001  
National Design Specification  
(NDS) for Wood Construction  
NOTES:

Trusses Manufactured by:

Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-  
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings\*  
for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

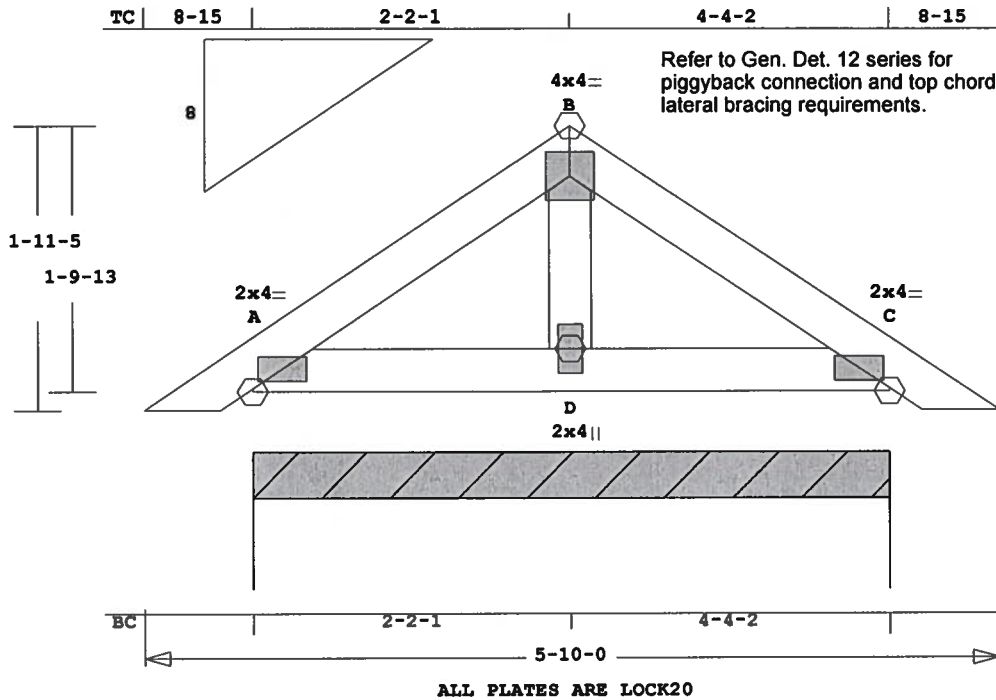
Max comp. force 18 Lbs

Max tens. force 15 Lbs

Quality Control Factor 1.25

Job <b>ELIXSON-WILL</b>	Mark <b>PI</b>	Quan 21	Type TR	Span 51000	P1-H1 8	Left OH 8-15	Right OH 8-15	Engineering <b>T2566288</b>
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TONY WILLIAMS



Refer to Gen. Det. 12 series for piggyback connection and top chord lateral bracing requirements.

ALL PLATES ARE LOCK20

Scale: 0.760" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 23.7 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI	Size	Lumber
TC	0.02 2x 4	SP-#2
BC	0.02 2x 4	SP-#2
WB	0.00 2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5-10- 0
BC Cont.	0- 0- 0	5-10- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=	1.15	Fc=1.10 Ft=1.10
BC Fb=	1.10	Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)			
Jt	Down	Uplift	Horiz-
A	376	51 U	27 R

Jt	Brg Size	Required
A	52.1"	0"-to- 52"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A	-B	0.02	75 C	0.00 0.02
B	-C	0.02	75 C	0.00 0.02
-----Bottom Chords-----				
A	-D	0.02	1 T	0.00 0.02
D	-C	0.02	1 T	0.00 0.02

-----Webs-----  
D -B 0.00 35 C

TL Defl 0.00" in D -C L/999  
LL Defl 0.00" in D -C L/999  
Shear // Grain in A -B 0.05

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 2.0x 4.0 Ctr Ctr 0.59  
B LOCK 4.0x 4.0 Ctr Ctr 0.42  
C LOCK 2.0x 4.0 Ctr Ctr 0.59  
D LOCK 2.0x 4.0 Ctr Ctr 0.38

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.

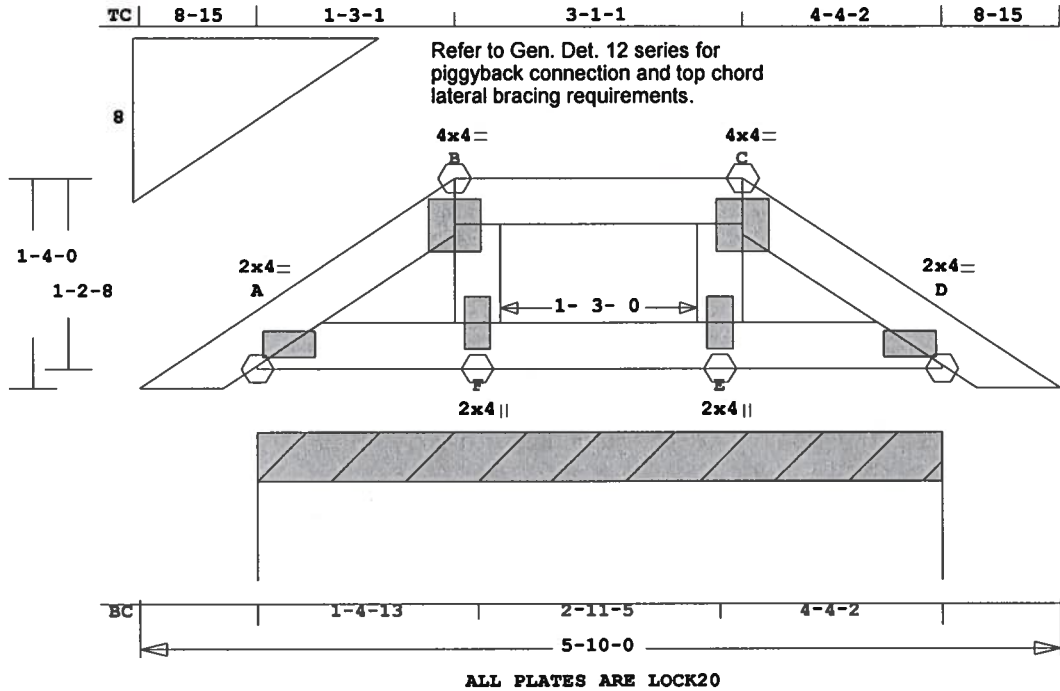
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Refer to Gen Det 3 series for

web bracing and plating.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 75 Lbs  
Max tens. force 59 Lbs  
Quality Control Factor 1.25

Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>P2</b>	Quan 2	Type HIPP	Span 51000'	P1-H1 8	Left OH 8-15	Right OH 8-15	Engineering <b>T2566289</b>
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TONY WILLIAMS



Scale: 0.819" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 23.1 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

CSI	Size	Lumber
TC	0.02 2x 4	SP-#2
BC	0.01 2x 4	SP-#2
WB	0.01 2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5-10- 0
BC Cont.	0- 0- 0	5-10- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)			
Jt	Down	Uplift	Horiz-
A	376	51 U	15 R

Jt	Brg Size	Required
A	52.1"	0"-to- 52"

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Ax1	CSI-Bnd
-----Top Chords-----				
A -B	0.00	44 C		
B -C	0.02	56 T	0.00	0.02
C -D	0.00	44 C		
-----Bottom Chords-----				
A -F	0.01	1 T	0.00	0.01

F -E	0.01	0 T	0.00	0.01
E -D	0.01	1 T	0.00	0.01
-----Webs-----				
F -B	0.01	89 T		
E -C	0.01	89 T		
TL Defl	0.00"	in F -E	L/999	
LL Defl	0.00"	in F -E	L/999	
Shear // Grain	in B -C	0.06		

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 2.0x 4.0 Ctr Ctr 0.59  
B LOCK 4.0x 4.0 Ctr Ctr 0.58  
C LOCK 4.0x 4.0 Ctr Ctr 0.58  
D LOCK 2.0x 4.0 Ctr Ctr 0.59  
F LOCK 2.0x 4.0 Ctr Ctr 0.38  
E LOCK 2.0x 4.0 Ctr Ctr 0.38

REVIEWED BY:  
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Tampa, FL 33610

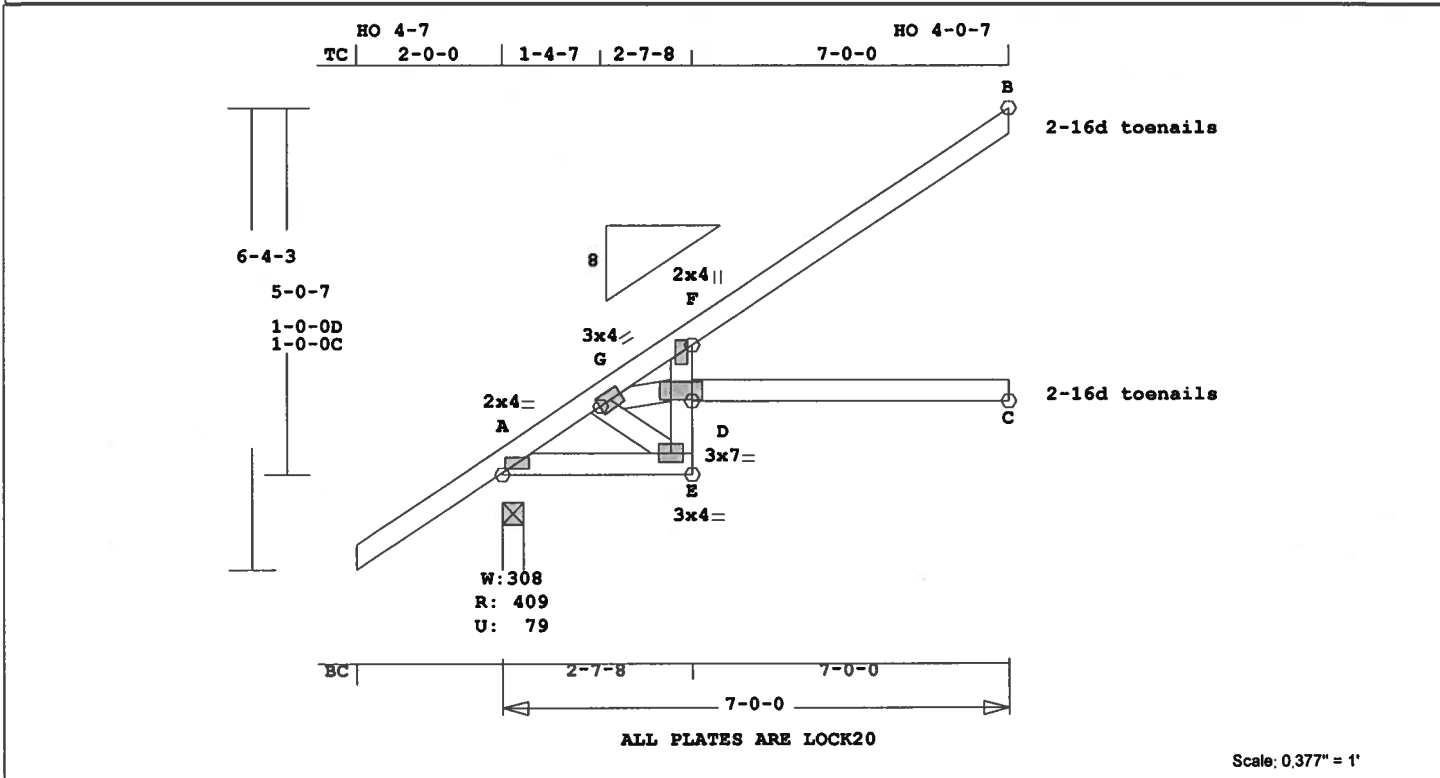
REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.  
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-concurrent LL on BC.  
Refer to Gen Det 3 series for web bracing and plating.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 61 Lbs  
Max tens. force 89 Lbs  
Quality Control Factor 1.25

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Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

Job <b>ELIXSON-WILL</b>	Mark <b>VJI</b>	Quan 6	Type SP	Span 7000'	P1-H1 8	Left OH 2- 0- 0	Right OH 0	Engineering <b>T2566290</b>
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TONY WILLIAMS



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 40.7 LBS

Online Plus -- Version 21.0.032  
RUN DATE: 05-JUN-07

	CSI	-Size-	---	Lumber	----
TC	0.38	2x 4	SP-#2		
BC	0.54	2x 4	SP-#2		
CW	0.20	2x 4	SP-#2		
WB	0.01	2x 4	SP-#2		

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0	
BC Cont.	0- 0- 0	7- 0- 0	

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	410	79 U	228 R
C	126	6 U	
B	179	82 U	97 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	1.5"	1.5"
B	1.5"	1.5"

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -G	0.02	353 C	0.00	0.02
G -F	0.22	222 C	0.00	0.22
F -B	0.38	97 C	0.00	0.38
-----Bottom Chords-----				

	Chord	Webs	-----
E -D	0.20	52 T	0.00 0.20
D -F	0.20	152 T	0.01 0.19
-----Webs-----			
G -E	0.01	128 C	
G -D	0.00	60 C	

TL Defl	-0.21"	in D -C	L/380
LL Defl	-0.10"	in D -C	L/792
Hz Disp	LL	DL	TL
Jt C	0.04"	0.04"	0.07"
Shear // Grain	in F -B		0.17

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 2.0x 4.0 Ctr Ctr 0.63  
G LOCK 3.0x 4.0 Ctr Ctr 0.39  
F LOCK 2.0x 4.0 Ctr Ctr 0.13  
E LOCK 3.0x 4.0 Ctr Ctr 0.77  
D LOCK 3.0x 7.0 Ctr Ctr 0.41

REVIEWED BY:  
Robbins Engineering, Inc.  
6904 Parke East Blvd.  
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

For proper installation of  
toe-nails, refer to the 2001  
National Design Specification

(NDS) for Wood Construction

NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

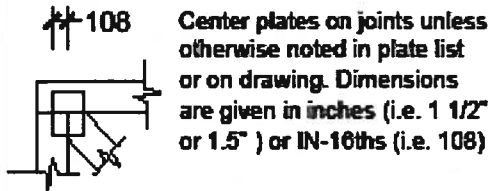
OH Loading

Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 353 Lbs  
Max tens. force 306 Lbs  
Quality Control Factor 1.25

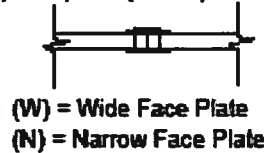
Michael S. Magid, FL Lic. #53681  
Robbins Engineering  
6904 Parke East Blvd  
Tampa, FL, 33610  
FL Cert.#5555

# ROBBINS ENG. GENERAL NOTES & SYMBOLS

## PLATE LOCATION

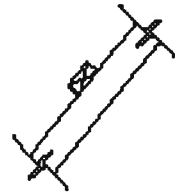


## FLOOR TRUSS SPLICE ( 3X2, 4X2, 6X2 )

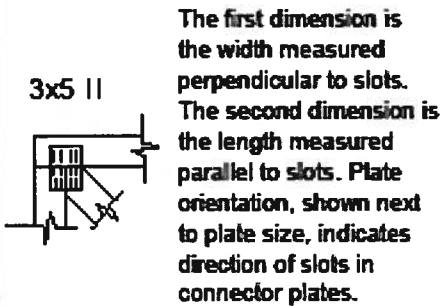


## LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

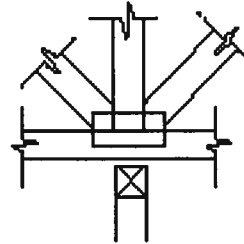
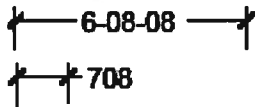


## PLATE SIZE AND ORIENTATION



## DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08 ). Dimensions less than one foot are shown in IN-SX only (i.e. 708).



W = Actual Bearing Width (IN-SX)  
R = Reaction (lbs.)  
U = Uplift (lbs.)

## BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted.

The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA ), " National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS FABRICATOR.

Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.



6904 Parke East Blvd.  
Tampa, FL 33610-4115  
Tel: 813-972-1135 Fax: 813-971-6117

www.robbseng.com





STATE OF FLORIDA  
DEPARTMENT OF HEALTH

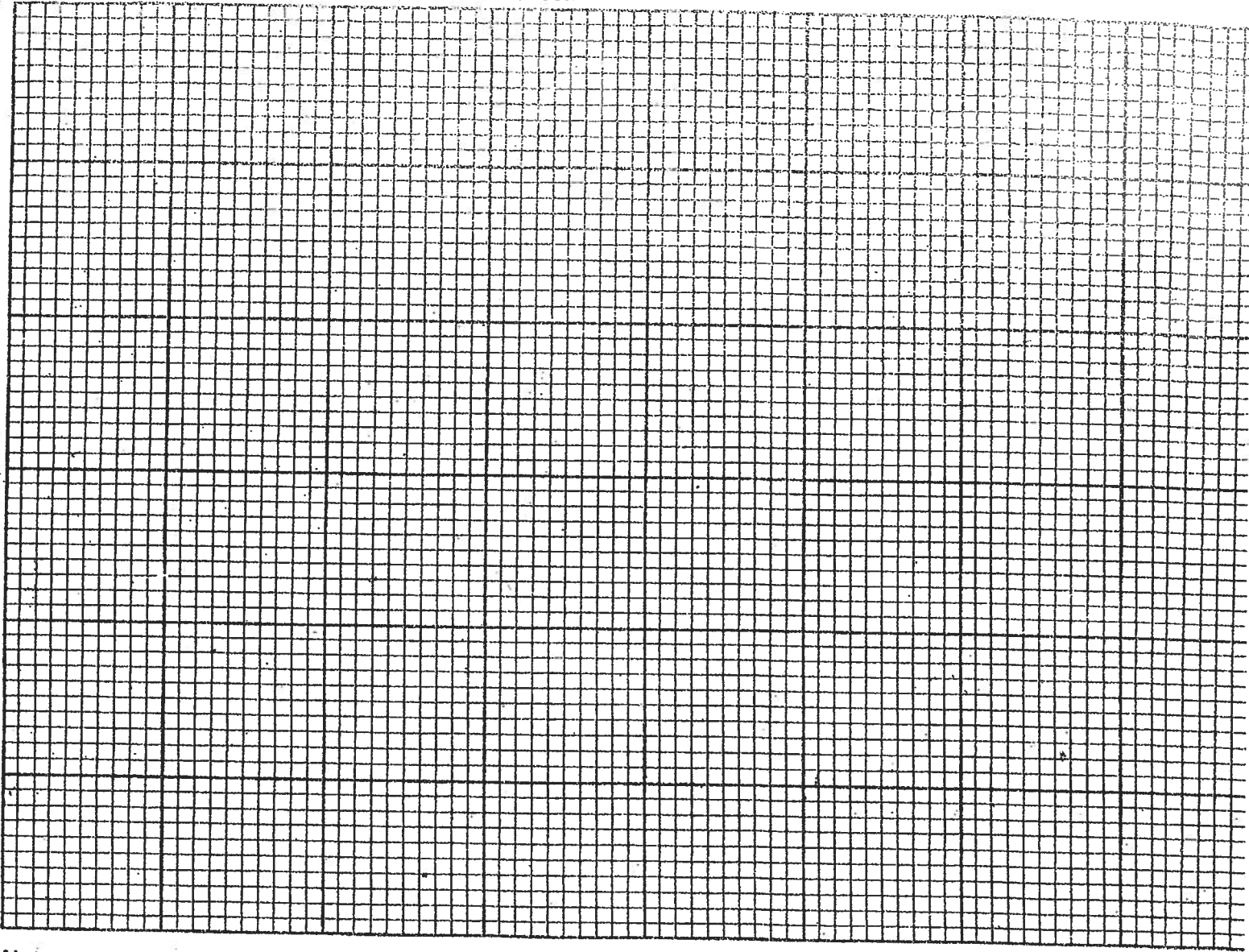
26182

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 09-03801

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: \_\_\_\_\_

*SE Attached*

Site Plan submitted by: *Willy* Signature *7/14/09* Title *Owner*

Plan Approved  Not Approved  Date *7-14-09*

By *Sallie Ford - Columbia - EHD Director* County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

# Notice of Preventative Treatments for Termites

**SHEFFIELD PEST CONTROL**

**904-964-9111 #26182**

307 SW Gerald Witt Lake City

Address of Treatment or Lot/Block of Treatment

10-9-07    6:00 PM    [Signature]

Date

Time

Applicator

Baseline Bifenoxin    650

Product Used    Chemical used    Number of Gallons Applied  
(Active Ingredients)

.02%    4060    \_\_\_\_\_

Percent Concentration    Area Treated (Sq. Ft.)    Linear Feet Treated

Stage of treatment (Horizontal, Vertical, Adjoining Slab, retreat of disturbed area)

As per 104.2.6-If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial and date this line:

\_\_\_\_\_

# GENERAL CONTRACTOR

OF

## OCCUPANCY

COLUMBIA COUNTY, FLORIDA

### Department of Building and Zoning Inspection

*This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.*

Parcel Number 13-5S-15-00453-002

Building permit No. 000026182

Use Classification SFD/UTILITY

Fire: 0.00

Permit Holder ANTHONY B. WILLIAMS, JR.

Waste: 0.00

Owner of Building ANTHONY B. & REBECCA WILLIAMS, JR.

Total: 0.00

Location: 307 SW GERALD WITT GLEN, LAKE CITY, FL

Date: 08/03/2009

*Wayne A. Rued*

Building Inspector

POST IN A CONSPICUOUS PLACE  
(Business Places Only)

