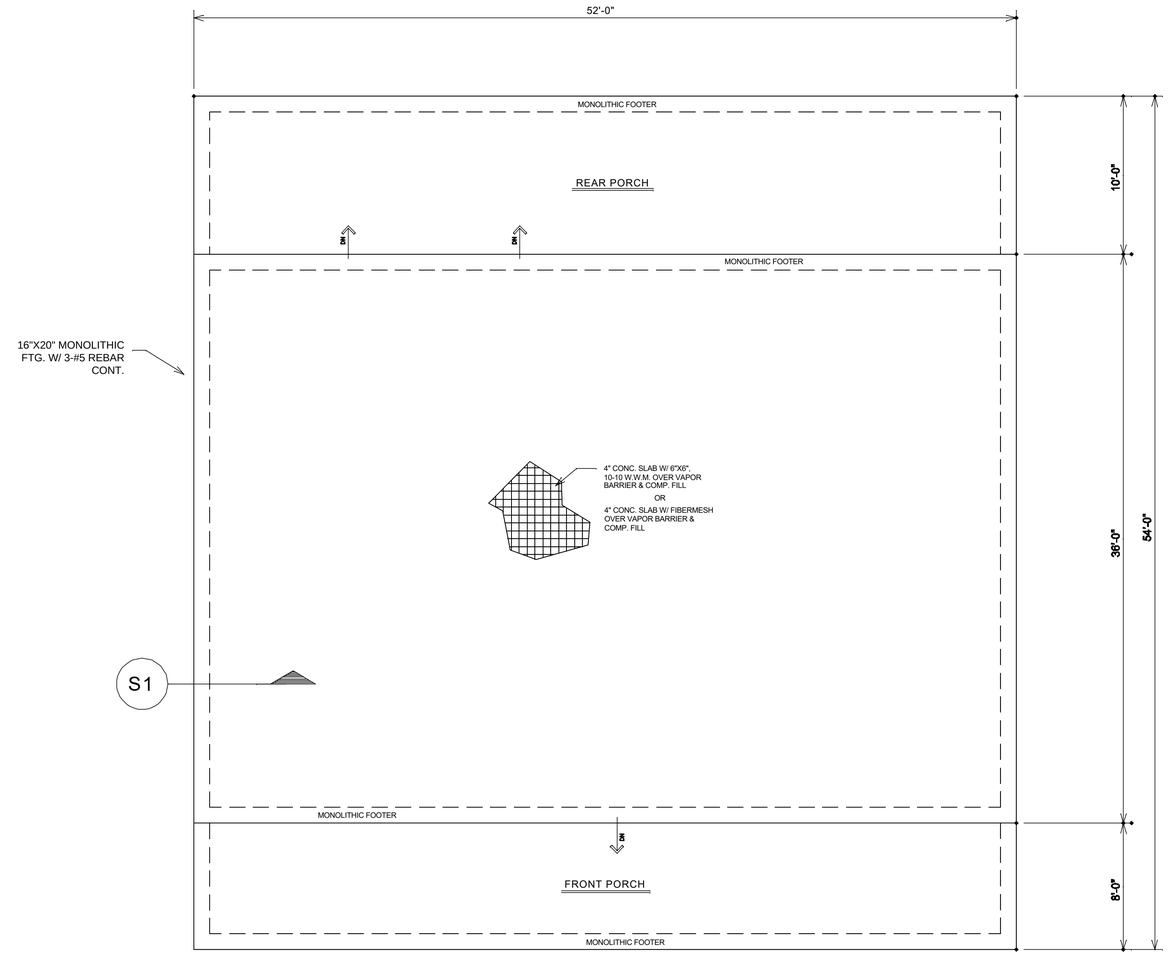
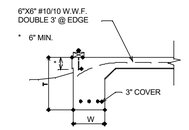


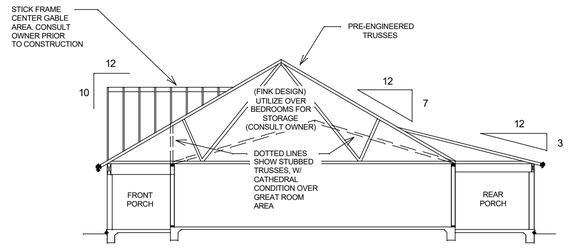
SHEET INDEX	
PAGE	DESCRIPTION
1	FOUNDATION PLAN
2	FLOOR PLAN
3	ELEVATIONS
4	**ELECTRICAL/MECHANICAL PLAN**
4	CABINET ELEVATIONS
5	TYPICAL WALL SECTION

	T[1]	W	REINFORCING
1 STORY	20"	16"	3-#5

1. THICKNESS OF FOOTING INCLUDES THICKNESS OF SLAB.



SECTION S1 1/2"=1'-0"



BASIC TRUSS DESIGN 1/8"=1'-0"

JAY DYLAN LAIRD LICENSE No. 82367 STATE OF FLORIDA PROFESSIONAL ENGINEER

NOTE: CONTRACTOR SHALL VERIFY ALL PERTINENT INFORMATION PRIOR TO CONSTRUCTION. ALL BEAMS, GIRDERS, BLOCKING, ETC. SHALL BE DESIGNED AND LOCATED BY OTHERS. ADDITIONAL BLOCKING, BEAMS, PIER SUPPORTS, ETC. AS NEEDED FOR LOAD BEARING PURPOSES SHALL BE DETERMINED BY CONTRACTOR PRIOR TO CONSTRUCTION.

CONTRACTOR TO VERIFY ALL DIMENSIONS AND REPORT DISCREPANCIES TO SPIERS DRAFTING SERVICE PRIOR TO CONSTRUCTION.

CONTRACTOR TO COMPLY WITH ALL BUILDING CODES AND ORDINANCES OF COLUMBIA COUNTY, FLORIDA

Original Paper Size
34"x22"

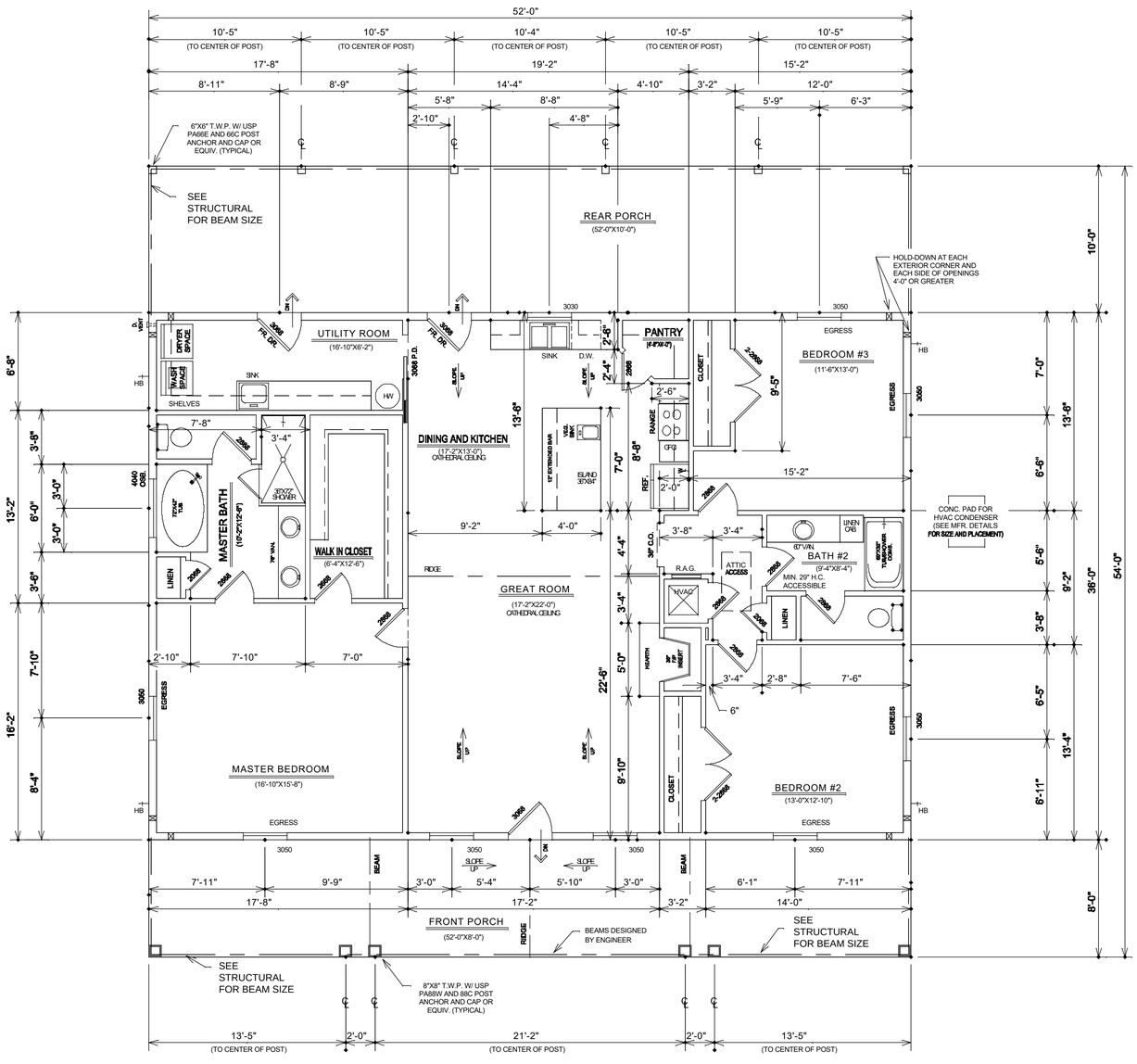
PROPOSED RESIDENCE FOR:
TROY POOLE

SPIERS DRAFTING SERVICE
850-836-4446 Ponce De Leon, FL

SCALE: 1/4"=1'-0"		DRAWN BY: LBS
DATE:		

FOUNDATION PLAN

PLAN # 071023 R | 1 OF 5



SQUARE FOOTAGE INFORMATION

HEATED AREA	1872 sq ft
COVERED PORCHES	980 sq ft
TOTAL SQ. FT.	2852 sq ft

NOTE:

8' CEILING HT. THROUGHTOUT HOUSE UNLESS OTHERWISE NOTED.

6" EXTERIOR WALLS

UTILIZE FINK TRUSS OVER BEDROOMS FOR STORAGE. SEE OWNER.

ROOF WILL BE CONSTRUCTED BY A TRUSS MANUFACTURER USING PRE-ENGINEERED ROOF TRUSSES

TRUSSES SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER WHO IS LICENCED BY THE STATE OF FL

TRUSSES SHALL BE DESIGNED FOR 140 MILE PER HOUR WIND SPEED.

JAY DYLAN LAIRD LICENSE No. 82367 STATE OF FLORIDA PROFESSIONAL ENGINEER

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SCALE: 1/4"=1'-0"		DRAWN BY: LBS
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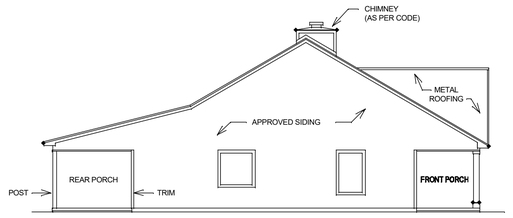
FLOOR PLAN

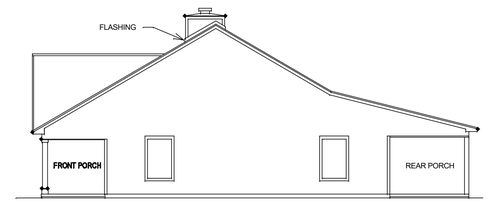
PLAN # 071023 R

FRONT ELEVATION 1/4"=1'-0"

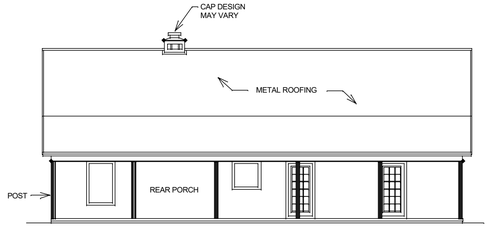


LEFT ELEV. 1/8"=1'-0"

PITCH:
MAIN ROOF ——— 7:12
FRONT GABLE ——— 10:12
REAR PORCH ——— 3:12
OVERHANG: ——— 12"



RIGHT ELEV. 1/8"=1'-0"



REAR ELEV. 1/8"=1'-0"

JAY DYLAN LAIRD
LICENSE
No. 82367
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Original Paper Size
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CONTRACTOR TO VERIFY ALL DIMENSIONS AND REPORT DISCREPANCIES TO SPIERS DRAFTING SERVICE PRIOR TO CONSTRUCTION.

CONTRACTOR TO COMPLY WITH ALL BUILDING CODES AND ORDINANCES OF COLUMBIA COUNTY, FLORIDA

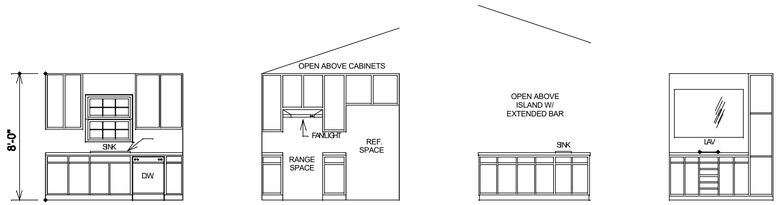
PROPOSED RESIDENCE FOR:
TROY POOLE

SPIERS DRAFTING SERVICE
850-836-4446 Ponce De Leon, FL

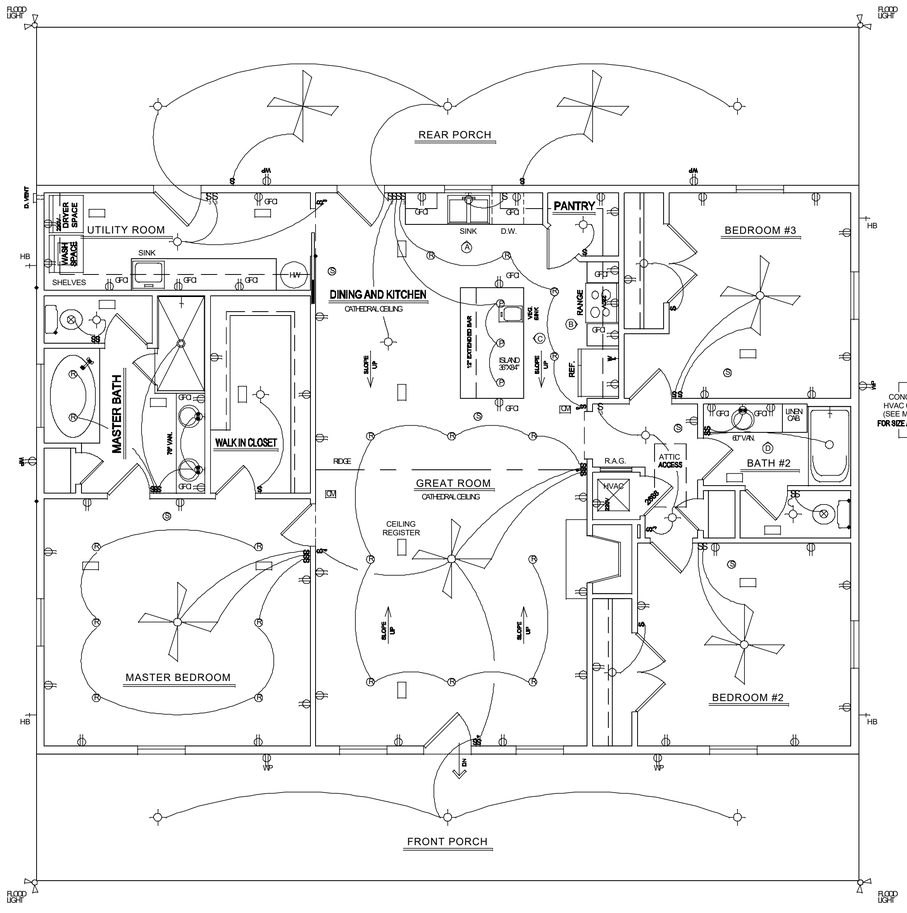
SCALE: AS SHOWN		DRAWN BY: LBS
DATE:		

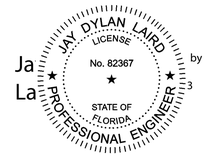
ELEVATIONS

PLAN # 071023 R — 3 OF 5



CABINET ELEVATIONS 1/4"=1'-0"





NOTE: MECHANICAL PLAN IS TO SHOW HEATED AREAS ONLY. MECHANICAL CONTRACTOR SHALL DETERMINE ADEQUATE SIZE OF HEAT/COOL UNIT, AS WELL AS EXACT SIZE AND LOCATION OF CEILING REGISTERS.

LEGEND

- CEILING HUNG LIGHT FIXTURE
- WALL HUNG LIGHT FIXTURE
- RECESSED DOWNLIGHT
- PENDANT DOWNLIGHT
- CEILING LIGHT/EXAUST FAN COMB.
- EXAUST FAN
- WALL RECEPTICLE
- WATER-PROOF RECEPTICLE
- 220 VOLT RECEPTICLE
- FLOOR RECEPTICLE
- GAS VALVE
- SWITCH
- 3-WAY SWITCH
- 4-WAY SWITCH
- SMOKE DETECTOR
- CARBON MONOXIDE DETECTOR
- GROUND-FAULT CIRCUIT INTERRUPTER
- HOSE BIB
- WATER LINE FOR ICE MAKER

Original Paper Size
34"x22"

CONTRACTOR TO VERIFY ALL DIMENSIONS AND REPORT DISCREPANCIES TO SPIERS DRAFTING SERVICE PRIOR TO CONSTRUCTION.

CONTRACTOR TO COMPLY WITH ALL BUILDING CODES AND ORDINANCES OF COLUMBIA COUNTY, FLORIDA

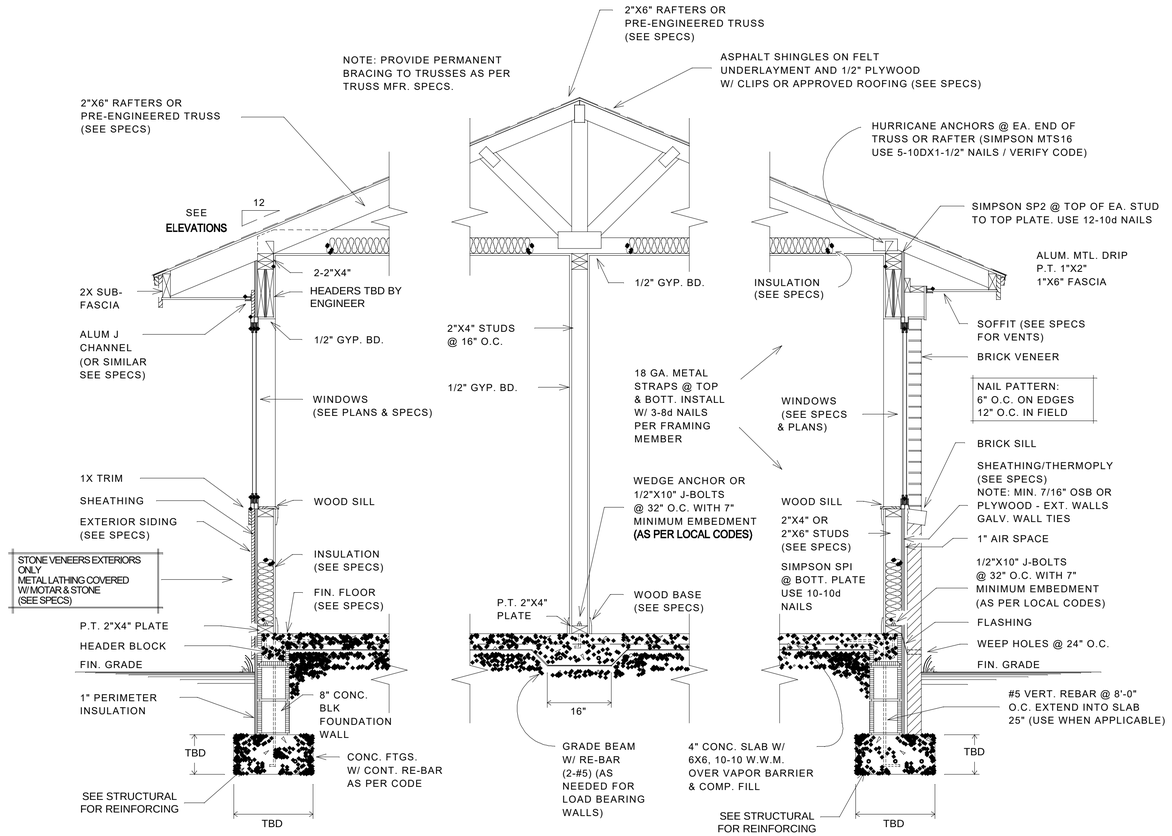
PROPOSED RESIDENCE FOR:
TROY POOLE

SPIERS DRAFTING SERVICE
850-836-4446 Ponce De Leon, FL

SCALE: 1/4"=1'-0"		DRAWN BY: LBS
DATE:		

ELECTRICAL/MECHANICAL PLANS, CABINET ELEVATIONS

PLAN # 071023 R | 4 OF 5



TYPICAL SECTIONS

(NOT TO SCALE)

NOTE: VERIFY PERTINENT INFORMATION PRIOR TO
CONSTRUCTI0N & USE ONLY THOSE DETAILS THAT
APPLY TO PLANS AND ARE REQUIRED BY CODES

JAY DYLAN LAIRD
LICENSE
No. 82367
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Original Paper Size

34"x22"

(850-836-4446)

SPIERS DRAFTING SERVICE
Ponce De Leon, FL

SCALE: AS SHOWN		DRAWN BY: JCS
DATE:		

TYPICAL SECTIONS

PLAN #	LAST PAGE

STRUCTURAL NOTES:

GENERAL:

1. NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, ENGINEER, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS. NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE STRUCTURAL ENGINEER OF RECORD OR ANY OF THE STRUCTURAL ENGINEER OF RECORD'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.
2. CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.
3. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
4. CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL GOVERN.
5. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.
6. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DOCUMENTS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SEE THE ARCHITECTURAL DRAWINGS.
7. CONTRACTOR SHALL OBTAIN AND COORDINATE EDGE OF SLAB DIMENSIONS, OPENING LOCATIONS AND DIMENSIONS, DEPRESSED SLAB LOCATIONS AND EXTENTS, SLAB SLOPES, CURB LOCATIONS, AND CMU WALL LOCATIONS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION.
8. CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY.
9. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, SAFETY, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
10. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR. CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTIBILITY ANALYSIS, AND ERECTION PROCEDURES, INCLUDING DESIGN AND ERECTION OF FALSEWORK, TEMPORARY BRACING, ETC.
11. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.
12. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED. ELECTRONIC DRAWING FILES WILL NOT BE PROVIDED TO THE CONTRACTOR.
13. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE ARCHITECT/STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE ARCHITECT/STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS.
14. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.
15. WHERE A SECTION OR DETAIL IS SHOWN OR DETAILED FOR ONE CONDITION, IT SHALL APPLY TO ALL SIMILAR AND LIKE CONDITIONS. DETAILS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR. THE CONTRACTOR SHALL CONSIDER ALL OF THE CONTRACT DOCUMENTS IN DETERMINING SIMILAR AND LIKE CONDITIONS.
16. SIGNATURE AND REGISTRATION SEAL OF THE STRUCTURAL ENGINEER THAT MAY BE AFFIXED TO THESE DRAWINGS RELATES ONLY TO THE STRUCTURAL DESIGN OF THE PROJECT.

CODE/DESIGN CRITERIA:

1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH:
   - 1.1. THE FLORIDA BUILDING CODE, 2023 EDITION
   - 1.2. AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (TIMBER CONSTRUCTION MANUAL, LATEST EDITION)
   - 1.3. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (A.C.I. 318).
   - 1.4. BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES (A.C.1.531).
     AMERICAN INSTITUTE OF STEEL CONSTRUCTION, LATEST EDITION.
2. GRAVITY LOADS
   (PONDING AND DRIFT EFFECTS HAVE BEEN INCLUDED IN THE DESIGN)
   - 2.1. UNIFORM FLOOR LIVE LOADS
     (REDUCED AS ALLOWED BY THE BUILDING CODE):

ROOMS OTHER THAN SLEEPING	40 PSF
SLEEPING ROOMS	30 PSF
EXTERIOR BALCONIES	60 PSF
STAIRS	40 PSF (300-LB POINT LOAD)
HANDRAILS/GUARDRAIL	50 PLF (200-LB POINT LOAD)
GUARD INFILL	50 PSF
PASSENGER VEHICLE GARAGES	50PSF (200-LB POINT LOAD)

   - 2.2. UNIFORM ROOF LIVE LOADS
     (REDUCED AS ALLOWED BY THE BUILDING CODE):

ROOF	20 PSF
ATTIC WITHOUT STORAGE	10 PSF
ATTIC WITH STORAGE	20 PSF
HABITABLE ATTIC (W/FIXED STAIRS)	30 PSF

CODE/DESIGN CRITERIA CONT:

3. ESTIMATED DEFLECTIONS (IN INCHES) ARE AS FOLLOWS:

	LIVE LOAD	DEAD + LIVE LOAD
ROOF MEMBERS:	L/360 OR 1 IN.	L/240
FLOOR MEMBERS:	L/360	L/240

   WHERE, L = SPAN LENGTH (IN INCHES) BETWEEN CENTERLINES OF SUPPORTS. FOR CANTILEVERS, L IS TWICE THE LENGTH OF THE CANTILEVER.

4. SPECIAL INSPECTIONS:
   - 4.1. THE FOLLOWING TYPES OF WORK REQUIRE SPECIAL INSPECTION:

     FOUNDATION ANCHORS & REINFORCING STEEL
     SHEARWALL CONSTRUCTION
     DIAPHRAM NAILING AND HOLDOWN ATTACHMENT
5. NO PROVISIONS HAVE BEEN MADE FOR FUTURE HORIZONTAL OR VERTICAL EXPANSION.

FOUNDATION:

1. FOUNDATION DESIGN IS BASED ON AN ASSUMED ALLOWABLE BEARING PRESSURE OF 2000 PSF. STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT FROM THOSE ASSUMED FOR DESIGN.
2. STRUCTURAL TESTING/INSPECTION AGENCY SHALL CERTIFY THE BEARING MEDIUM.
3. NO FOOTINGS SHALL BE POURED IN STANDING WATER
4. ANY SOIL CONDITION ENCOUNTERED DURING EXCAVATION THAT IS CONTRARY TO THE CONDITIONS USED FOR DESIGN OF FOOTINGS AS OUTLINED IN THE DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD BEFORE PROCEEDING.
5. BACKFILL OF FOUNDATION SHALL BE DONE SIMULTANEOUSLY ON BOTH SIDES TO PREVENT FOUNDATION SHIFT

CONCRETE MASONRY:

1. HOLLOW CONCRETE BLOCK (MASONRY) UNITS SHALL CONFORM TO ASTM C90, WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI ON THE NET AREA AND 1,000 PSI ON THE GROSS AREA (f'm = 1,500 PSI).
2. CONCRETE MASONRY WORK SHALL ALSO CONFORM TO ACI 530, BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES AND ACI 530.1, SPECIFICATION FOR MASONRY STRUCTURES.
3. ALL MORTAR FOR MASONRY SHALL CONFORM TO ASTM C270, TYPE "M" (WALLS BELOW GRADE) OR "S". ALL GROUT FOR USE IN MASONRY SHALL CONFORM TO ASTM C476, MINIMUM 2,500 PSI AT 28 DAYS.
4. MINIMUM VERTICAL WALL REINFORCEMENT SHALL BE #5 @ 32" UNLESS NOTED OTHERWISE WITH MINIMUM LAP LENGTH OF 12" OR 48 BAR DIAMETERS, WHICHEVER IS MORE.
5. CONTINUOUS BARS SHALL HAVE BASIC CLASS "C" TENSION LAPS WITH CORNER BARS AT ALL CORNERS AND END WALL INTERSECTIONS.
6. ALL VERTICAL REINFORCEMENT IN MASONRY SHALL HAVE CLASS "C" TENSION LAPS.
7. REINFORCING IN MASONRY WALL FOOTINGS SHALL BE CONTINUOUS.

REINFORCEMENT:

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
2. WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM A185 AND HAVE MINIMUM SIDE AND END LAPS OF 8".
3. REINFORCING BARS PLACING ACCESSORIES: IN ACCORDANCE WTH CRSI SPECIFICATIONS.
4. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE.
5. PROVIDE DOWELS FROM FOUNDATIONS THE SAME SIZE AND NUMBER AS THE VERTICAL WALL OR COLUMN REINFORCING, UNLESS NOTED OTHERWISE.
6. PLACE REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:
   - 6.1. CAST-IN-PLACE (NON POST-TENSIONED) CONCRETE REINFORCEMENT COVER

PERMANENTLY EXPOSED TO EARTH:	
CAST AGAINST THE EARTH	3" CLEAR
EXPOSED TO EARTH OR WEATHER:	
FOR BARS LARGER THAN A NO. 5 BAR	2" CLEAR
NO. 5 BARS OR SMALLER	1-1/2" CLEAR

7. MASONRY REINFORCING STEEL SHALL BE PLACED IN THE CENTER OF CMU CELLS, UNLESS NOTED OTHERWISE.
8. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER. REINFORCING STEEL SPLICES SHALL BE AS FOLLOWS, UNLESS NOTED OTHERWISE:

CONCRETE REINFORCEMENT:	NO. 5 BARS (36") FOR 3000PSI CONC.
MASONRY REINFORCEMENT:	NO. 5 BARS (30")

9. ADHESIVE FOR REINFORCING DOWELS IN EXISTING CONCRETE SHALL BE EITHER THE HIT HY150 INJECTION ADHESIVE SUPPLIED BY HILTI FASTENING SYSTEMS, THE EPCON SYSTEM CERAMIC 6 EPOXY ADHESIVE SUPPLIED BY ITW RAMSET/RED HEAD, POWER-FAST EPOXY INJECTION GEL SUPPLIED BY POWERS FASTENING, OR APPROVED EQUAL. MINIMUM EMBEDMENT LENGTH SHALL BE 12 BAR DIAMETERS, UNLESS NOTED OTHERWISE.

CAST-IN-PLACE CONCRETE:

1. CONCRETE WORK SHALL CONFORM TO ACI 318 AND CRSI STANDARDS.
2. MINIMUM CONCRETE COVERAGE OF REINFORCEMENT: FOOTINGS: 3" BOTTOM AND 3" SIDES.
3. EARTH SUPPORTED SLAB: 4 INCHES THICK REINFORCED WITH 6 X 6 X W2.1 X W2.1 W.W.F. THE SLAB SHALL BE PLACED OVER POLYETHYLENE VAPOR BARRIER OF NOT LESS THAN .006 INCH NOMINAL THICKNESS. IN LIEU OF WELDED WIRE FABRIC, CONCRETE SLAB CAN BE TREATED WITH SYNTHETIC REINFORCING FIBERS AS MANUFACTURED BY FIBERMESH COMPANY AND IN ACCORDANCE WITH ASTM STANDARD SPECIFICATION FOR FIBER REINFORCED CONCRETE AND SHOTCRETE C1116. THE DOSAGE SHALL BE ONE AND ONE HALF 1 1/2 POUNDS FIBERS PER CUBIC YARD OF CONCRETE.
4. ANCHOR BOLTS IN CMU BLOCKS SHALL CONFORM TO ASTM A36 AND SHALL BE $\frac{1}{2}$" OR 5/8" DIAMETER WITH 7" MIN. DEPTH IN CONCRETE.
5. DETAIL REINFORCING IN ACCORDANCE WITH A.C.I. 315. REINFORCING SHALL NOT BE WELDED, EXCEPT AS SHOWN WHERE ASTM A708 BARS ARE USED.
6. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM SPECIFIED 28-DAY COMPRESSIVE STRENGTH:

   NORMAL WEIGHT STRUCTURAL CONCRETE:

FOOTINGS, PEDESTALS	3,000 PSI
SLABS-ON-GRADE	3,000 PSI

7. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, CLIPS OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND FOR LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS.
8. CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER. NO HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED EXCEPT THOSE SHOWN ON THE STRUCTURAL DRAWINGS.
9. DEFECTIVE AREAS IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.01 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE STRUCTURAL ENGINEER.

EXTERIOR WINDOWS & DOORS:

NOTES:

1. THIS STRUCTURE IS DESIGNED AS AN ENCLOSED BUILDING IN ACCORDANCE WITH FLORIDA BUILDING CODE CHAPTER 16. ALL WINDOWS AND EXTERIOR DOORS SHALL BE RATED FOR 140 MPH WIND PRESSURE.
2. FOR THIS ENCLOSED BUILDING, DESIGN PRESSURES ARE INDICATED ON THE WIND LOAD TABLE THIS SHEET.
3. TO COMPLY WITH THE FLORIDA BLDG CODE, 2023 RESIDENTIAL SHUTTERS OR IMPACT RESISTANT GLAZING ARE REQUIRED.
4. SHUTTERS OR IMPACT RESISTANT GLAZING MUST HAVE FLORIDA PRODUCT APPROVAL NUMBERS.

WOOD:

1. STRUCTURAL GLUED LAMINATED TIMBER SHALL BE PRODUCED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC). MINIMUM ALLOWABLE BENDING STRESS SHALL BE 2,400 PSI (DRY CONDITIONS).
2. PROVIDE DRESSED SEASONED LUMBER, S4S, WITH A MAXIMUM MOISTURE CONTENT OF 19% AT TIME OF DRESSING AS LISTED BELOW.
3. INTERIOR AND EXTERIOR LOAD-BEARING WALLS: SOUTHERN PINE, NO. 2 GRADE.
4. LINTELS, FLOOR JOISTS AND BEAMS: SOUTHERN PINE, NO. 2 GRADE WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE FOUNDATION GRADE PRESSURE-TREATED. USE GALVANIZED NAILS IN PRESSURE-TREATED WOOD.

ENGINEERED LUMBER PRODUCTS

1. PARALLEL STRAND LUMBER (PSL) SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE STRESSES AND PROPERTIES:

ALLOWABLE BENDING STRESS	FB = 2900 PSI
COMPRESSION PERPENDICULAR TO GRAIN	FCPER = 650 PSI
COMPRESSION PARALLEL TO GRAIN	FCPAR = 2900 PSI
HORIZONTAL SHEAR	FV = 290 PSI
MODULUS OF ELASTICITY	E = 2,000,000 PSI

2. LAMINATED VENEER LUMBER (LVL) SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE STRESSES AND PROPERTIES:

ALLOWABLE BENDING STRESS	FB = 2600 PSI
COMPRESSION PERPENDICULAR TO GRAIN	FCPER = 750 PSI
COMPRESSION PARALLEL TO GRAIN	FCPAR = 2310 PSI
HORIZONTAL SHEAR	FV = 285 PSI
MODULUS OF ELASTICITY	E = 1,900,00 PSI

3. LAMINATED STRAND LUMBER (LSL) SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE STRESSES AND PROPERTIES:

ALLOWABLE BENDING STRESS	FB = 1700 PSI
COMPRESSION PERPENDICULAR TO GRAIN	FCPER = 650 PSI
COMPRESSION PARALLEL TO GRAIN	FCPAR = 1400 PSI
HORIZONTAL SHEAR	FV = 285 PSI
MODULUS OF ELASTICITY	E = 1,300,000 PSI

STRUCTURAL PANELS

1. FLOOR PANELS SHALL BE CONSTRUCTED WITH TONGUE AND GROOVE APA RATED STURD-I-FLOOR. FLOOR PANELS SHALL BE GLUED AND NAILED.
2. WALL PANELS SHALL BE CONSTRUCTED WITH APA RATED SHEATHING. NAIL PANELS w/10d COMMON NAILS AT 3" O.C. ALONG SUPPORTED PANEL EDGES AND AT 10" O.C. AT INTERMEDIATE SUPPORTS. FOR SHEAR WALLS, USE 10d NAILS @ 3" O.C.
3. ROOF PANELS SHALL BE CONSTRUCTED WITH 5/8" APA RATED SHEATHING.

PLYWOOD SHEATHING:

1. EACH CONSTRUCTION AND INDUSTRIAL PANEL SHALL BEI DENTIFIED WITH THE APPROPRIATE TRADEMARK OF THE AMERICAN PLYWOOD ASSOCIATION AND SHALL MEET THE REQUIREMENTS OF THE LARGEST EDITION OF U.S. PRODUCT STANDARDS PS 1 OR PRP-180 PERFORMANCE STANDARDS. ALL PANELS WHICH HAVE ANY EDGE OR SURFACE PERMANENTLY EXPOSED TO WEATHER SHALL BE CLASSIFIED EXTERIOR.
2. PANEL ROOF WALL AND FLOOR SHEATHING SHALL BE ½ THICK APA STRUCTURAL 1 RATED SHEATHING EXP 2 (UNLESS OTHERWISE NOTED ON PLANS). SHEATHING PERMANENTLY EXPOSED TO WEATHER SHALL BE CLASSIFIED EXTERIOR.
3. NAIL PANELS WITH 10d COMMON NAILS AT 3" O.C. ALONG SUPPORTED PANEL EDGES AND 6 O.C. AT INTERMEDIATE SUPPORTS, OR AS INDICATED ON PLANS.
4. ALL BEARING STUD WALLS SHALL HAVE SOLID BLOCKING AT MID-HEIGHT OR AS OTHERWISE NOTED ON BUILDING SECTIONS.
5. PREFABRICATED WOOD STRUCTURAL MEMBERS, INCLUDING TRUSSES SHALL BE DESIGNED SPECIFICALLY FOR THIS PROJECT OR A 140 MPH WIND LOAD IAW ASCE 7-22 LATERAL LOAD AND SEALED BY A FLORIDA REGISTERED ENGINEER.
6. REVIEW ALL DRAWINGS INCLUDING MECHANICAL, ELECTRICAL PLUMBING ETC. TO ASCERTAIN LOADS FROM EQUIPMENT OPENINGS FOR DUCTS ETC. AND PROVIDE MODIFICATION TO TRUSSES IF REQUIRED TO SUPPORT SAME.
7. USE SIMPSON SP1 & SP2 (OR EQUAL) TO SECURE STUDS TO BOTTOM AND TOP PLATES, OR AS INDICATED ON PLAN.
8. USE TWO (2) SIMPSON LSTA21 (OR EQUAL) TO SECURE EACH BEAM HEADER BEARING END TO EACH SUPPORT, OR AS INDICATED PER PLAN.
9. USE SIMPSON LSTA21 STRAP TIES (OR EQUAL) OR SIMPSON SP4 (OR EQUAL) AT TOP OF EACH EXTERIOR WINDOW AND DOOR FRAME OPENING, OR AS INDICATED PER PLAN.
10. CUTTING NOTCHING BORED HOLES IN STUD WALLS, RAFTERS, ETC., SHALL BE DONE IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2023 RESIDENTIAL.
11. RIDGE BOARDS WHERE INDICATED ON FRAMING PLANS SHALL NOT BE LESS THAN 1" IN THICKNESS, AND NOT LESS IN DEPTH THAN CUT END RAFTERS. RAFTERS SHALL BE PLACED DIRECTLY OPPOSITE EACH OTHER AND NAILED TO RIDGE BOARD.
12. ALL WOOD BUILT-UP GIRDERS BEAMS, STUDS TO SOLE PLATES, ETC. TO BE CONNECTED AS PER 2023 FLORIDA BUILDING CODE.
13. OPENING IN EXTERIOR WALLS, A WALL STUD SHALL BE AT EACH SIDE OF THE OPENING WITH THE ENDS OF THE HEADER SUPPORTED AS FOLLOWS (UNLESS OTHERWISE NOTED):
    - 13.1. FOR OPENINGS LESS THAN 3 FEET IN WIDTH, EACH SIDE OF HEADER SHALL REST ON A SINGLE HEADER STUD OR MAY BE SUPPORTED BY FRAMING ANCHORS ATTACHED TO WALL STUD.
    - 13.2. FOR OPENINGS OVER 3 FEET TO LESS THAN 6'-0", EACH END SHALL BEAR ON A SINGLE HEADER STUD.
    - 13.3. FOR OPENINGS MORE THAN 6'-0" AND LESS THAN 12' IN WIDTH, EACH END SHALL BEAR ON A DOUBLE HEADER STUD.
14. ANY HEADER SUPPORTING CONCENTRATED LOADS FROM BEAMS ABOVE, EACH END SHALL BEAR ON DOUBLE HEADER STUDS.
15. WHERE WOOD BEAMS BEAR ON STUD WALLS, PROVIDE MINIMUM DOUBLE OR TRIPLE STUDS, DEPENDING ON BEAM WIDTH AND LOADS, UNDER FOUNDATION.

FRAMING

1. ROOF SHEATHING: 5/8 CDX - NAIL 10d RING SHANK FULL HEAD - 3" PERIM./4" FIELD.
2. ROOF RAFTERS: 2x8 - 24" O.C. (MAX. UNBRACED HORIZONTAL SPAN - 12'-6" OR PRE-ENGINEERED ROOF TRUSSES.
3. WALL FRAMING: 2x6 OR 2x4 @ 16" O.C. - LODGE POLE MAY BE USED FOR INTERIOR STUD FRAMING. SYP SHALL BE USED FOR TOP, BOTTOM PLATES, AND EXTERIOR WALL STUDS.
4. SECURE ROOF RAFTERS TO TOP PLATE WITH SIMPSON H10, OR H10-2, OR MSTD12/16. CLIPS OR EQUAL OR GREATER UPLIFT CAPACITY MAY BE SUBSTITIUDED.
5. WALL SHEATHING: 1/2 CDX - NAIL 8d COMMON - 4" PERIM./4" FIELD.
6. STRAP TIES: SIMPSON SP1 AND SP2 OR SIMPSON LSTA21 - 20 GA. - (16) 10d COMMON. TIE SPACING: 32" O.C.
7. TOP PLATE NAILING: 24" O.C. - USE 16d COMMON. USP CONNECTORS MAY BE USED IN LIEU OF SIMPSON.
8. IF USED, RUN $\frac{1}{2}$" OR $\frac{5}{8}$" ALL THREAD RODS ON 72" CENTERS, PLACE ALL THREAD RODS EACH SIDE OF BEARING OPENINGS GREATER THAT 3'-0", AND WITHIN 12" OF ALL CORNERS.
9. ALL COLUMN TO BEAM CONNECTIONS SHALL BE SIMPSON CC OR ECC L/R. STRAPPED CONNECTIONS ARE NOT ALLOWED.

FABRICATED WOOD TRUSSES

1. DESIGN OF WOOD TRUSSES SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS, DESIGN LOAD DATA, AND SUPPORT REACTIONS SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS WITH REGARD TO TRUSS CONFIGURATION, AND THE CONTRACTOR'S INTERPRETATION OF DESIGN LOADS AND DETAILS. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN OF THE TRUSSES OR TRUSS CONNECTIONS NOT SPECIFICALLY DETAILED IN THE CONTRACT DOCUMENTS.
2. ERECTION AND TEMPORARY BRACING OF PREFABRICATED WOOD TRUSSES SHALL BE IN CONFORMANCE WITH THE RECOMMENDATIONS OF THE TRUSS MANUFACTURER AND THE TRUSS PLATE INSTITUTE'S "BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS".

FABRICATED WOOD TRUSSES CONT

3. SECURE EACH COMMON ROOF TRUSS/RAFTER TO TOP PLATE WITH SIMPSOM H-10 ORH-7 HURRICANE CLIP AT ALL BEARING POINTS, USE SIMPSON H-7 AT GIRDER TRUSSES. PROVIDE A MINIMUM OF TWO STUDS UNDER GIRDER TRUSS END BEARING.
4. CONTRACTOR TO REFER TO "STANDARD FOR HURRICANE RESISTANT CONSTRUCTIONSSTD 10-93 FOR FRAMING REQUIREMENTS OF WOOD FRAMED WALL SYSTEMS, TABLE 305C AND FIGURE 306D.
5. TRUSS LAYOUT AS SHOWN ON PLANS IS SCHEMATIC AND MAY BE MODIFIED WITH APPROVAL OF THE ENGINEER.
6. ALTHOUGH WEB LAYOUT MAY BE SHOWN ON PLANS IT IS THE RESPONSIBILITY OF THE TRUSS DESIGNER TO ACCEPT, APPROVE, OR MODIFY, AS REQUIRED FOR THE DESIGN PURPOSE.
7. WOOD-TO-WOOD FRAMED CONNECTIONS ARE TO BE MADE WITH BOLTS AND/OR JOIST HANGERS AS SHOWN. TOE-NAILING IS NOT PERMITTED.
8. MAXIMUM SPANS OF DIMENSIONAL LUMBER USED FOR JACK RAFTERS AT HIPPED ROOF SECTIONS SHALL BE IN ACCORDANCE WITH SPAN TABLES FOR JOISTS AND RAFTERS AS PUBLISHED BY THE NATIONAL WOOD PRODUCTS ASSOCIATION.
9. HIP RAFTERS SHALL BE 2 INCHES DEEPER THAN JACK RAFTERS.
10. ALL TRUSSES AND RAFTERS SHALL BE STRAPPED OR HURRICANE CLIPPED TO SUPPORTING MEMBERS AT ALL BEARING POINTS.
11. SECURE EACH ROOF TRUSS/RAFTER TO TOP PLATE WITH SIMPSON HURRICANE CLIPS (OR EQUAL) AS INDICATED ON PLANS. CONTRACTOR TO SUBMIT SHOP DRAWINGS OF TRUSSES TO ENGINEER TO VERIFY/MODIFY UPLIFT CONNECTORS.
12. ALL EXTERIOR WALL FRAMING SHALL BE 2"X4" OR 2"X6" AT 16" O.C. UNLESS NOTED OTHERWISE. 7/16" OSB SHEATHING OR 1/2" CDX PLYWOOD PANELS SHOULD EXTEND TO THE TOP PLATE AND BOTTOM OF EXTERIOR GIRDERS OR SILL PLATE. NAIL PLYWOOD AT 4" O.C. AT ALL EDGES AND 6" O.C. AT INTERMEDIATE SUPPORTS OR AS INDICATED PER PLAN.
13. USE SIMPSON ST18 (OR EQUAL) RIDGE/RAFTER CONNECTORS OR SIMPSON RR STRAPS AT ALL RAFTERS RIDGE BEAMS OR AS INDICATED PER PLAN.

FLOOR TRUSS LOADS:

1. FLOOR TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS UNLESS A SPECIAL LOADING PATTERN IS PROVIDED BY THE STRUCTURAL ENGINEER OF RECORD.

TOP CHORD LIVE LOAD	40 PSF
TOP CHORD DEAD LOAD	25 PSF
BOTTOM CHORD DEAD LOAD	10 SPF
TOTAL	75 PSF

ROOF TRUSS LOADS:

1. ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS UNLESS A SPECIAL LOADING PATTERN IS PROVIDED BY THE STRUCTURAL ENGINEER OF RECORD.

TOP CHORD LIVE LOAD	20 PSF
TOP CHORD DEAD LOAD	15 PSF
BOTTOM CHORD DEAD LOAD	10 SPF
TOTAL	45 PSF
	(50 PSF FOR TILE ROOF)

NOTES TO GENERAL CONTRACTOR:

IN CASE OF CONFLICT BETWEEN THE DRAWINGS AND THE SPECIFICATIONS OR NOTES, E.O.R. SHALL BE NOTIFIED PRIOR TO THE EXECUTION OF ANY WORK.

IN CASE OF CONFLICT AS TO THE TYPE OF MATERIALS OR THE QUALITY OF CONSTRUCTION, THE MATERIAL OUTLINE SHALL PREVAIL; IF CLARIFICATION IS NEEDED, NOTIFY ENGINEER PRIOR TO EXECUTION OF ANY WORK.

IN CASE OF CONFLICT BETWEEN THE DOCUMENTS AND THE BUILDING CODE, THE BUILDING CODE PREVAILS.

NOTIFY ENGINEER OF ERRORS OR OMISSIONS IMMEDIATELY UPON DISCOVERY AND STOP WORK UNTIL RESOLVED.

NO CHANGES SHALL BE MADE WITHOUT WRITTEN APPROVAL FROM ENGINEER.

DETAILS ON ONE SHEET GENERALLY APPLY TO ALL SIMILAR SITUATIONS EVEN IF NOT SHOWN OR REFERENCED TO OTHER DRAWINGS. IF A CONFLICT OCCURS, NOTIFY STRUCTURAL ENGINEER PRIOR TO PROCEEDING.

NAIL PATTERNS AND OTHER DETAILS NOT DRAWN ARE TO BE INSTALLED ACCORDING TO CODE AND TO ACCEPTABLE STANDARD PRACTICE FOR QUALITY CUSTOM CONSTRUCTION; STRUCTURAL ENGINEER SHALL BE THE SOLE JUDGE OR ARBITRATOR AS TO WHAT DETERMINES "ACCEPTABLE QUALITY CUSTOM CONSTRUCTION".

MAINTAIN A STAMPED AND SEALED SET OF CURRENT CONSTRUCTION DOCUMENTS, INCLUDING ANY AND ALL ADDENDA, SHOP DRAWING AND SKETCH DESIGN/DETAIL DRAWINGS ON SITE AND ACCESSIBLE TO ALL INVOLVED PARTIES.

DIMENSIONS SHOWN ARE FRAMING OR MASONRY DIMENSIONS UNLESS NOTED AS SURFACE DIMENSIONS.

DO NOT SCALE THE DRAWING, CALL STRUCTURAL ENGINEER FOR CLARIFICATION AS NECESSARY.

COUNTY CERTIFICATION:

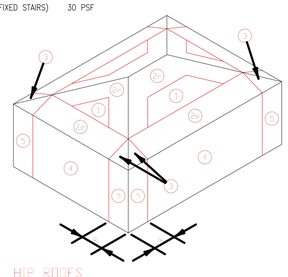
I CERTIFY THAT THE DESIGN PLANS AND SPECIFICATIONS FOR THIS CONSTRUCTION ARE IN COMPLIANCE WITH THE CRITERIA ESTABLISHED BY THE 2023 FLORIDA BUILDING CODE.

THIS BUILDING AND/OR STRUCTURE IS DESIGNED TO WITHSTAND WIND VELOCITY OF 140 MPH.

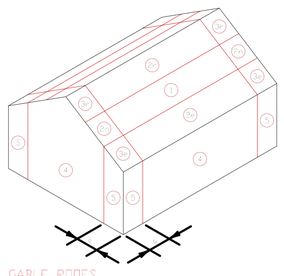
ALSO, UPON COMPLETION OF THIS BUILDING AND/OR STRUCTURE, I WILL CERTIFY AT THAT TIME THE BUILDING AND/OR STRUCTURE HAS COMPLIED WITH THIS SPECIFIC BUILDING DESIGN. THIS MUST BE ON FILE AT THE COUNTY BUILDING DEPARTMENT BEFORE RECEIVING AN INSPECTION FOR POWER.

I UNDERSTAND THAT ANY CHANGE IN DESIGN OR SPECIFICATION MUST BE SUBMITTED IN WRITING BY ME TO THE BUILDING DEPARTMENT.

ALL DRAWINGS AND/OR CORRESPONDENCE SHALL BE SIGNED AND SEALED.



HIP ROOFS



GABLE ROOFS

Wind Pressures [Normal to Ridge]
All wind pressures include a Load Factor (LF) of 0.6

Elev ft	GC_{pi}	q_i psf	K_z	K_{zt}	q_z psf	Windward Press psf	Leeward Press psf	Side Press psf	Total Press psf	Minimum Pressure* psf
9.500	0.18	26.06	0.851	1.000	25.62	10.82	-13.40	-17.17	24.23	9.60
------	------	------	------	------	------	------	------	------	------	------
9.500	-0.18	26.06	0.851	1.000	25.62	18.80	-5.43	-9.19	24.23	9.60

Wind Pressures [Parallel to Ridge]
All wind pressures include a Load Factor (LF) of 0.6

Elev ft	GC_{pi}	q_i psf	K_z	K_{zt}	q_z psf	Windward Press psf	Leeward Press psf	Side Press psf	Total Press psf	Minimum Pressure* psf
23.111	0.18	26.06	0.930	1.000	27.99	12.19	-12.72	-17.17	24.91	9.60
16.306	0.18	26.06	0.866	1.000	26.06	11.08	-12.72	-17.17	23.80	9.60
9.500	0.18	26.06	0.851	1.000	25.62	10.82	-12.72	-17.17	23.54	9.60
------	------	------	------	------	------	------	------	------	------	------
23.111	-0.18	26.06	0.930	1.000	27.99	20.16	-4.74	-9.19	24.91	9.60
16.306	-0.18	26.06	0.866	1.000	26.06	19.05	-4.74	-9.19	23.80	9.60
9.500	-0.18	26.06	0.851	1.000	25.62	18.80	-4.74	-9.19	23.54	9.60

Roof Wind Pressures [Normal to Ridge]
All wind pressures include a Load Factor (LF) of 0.6

Component	Description	Location	Start ft	End ft	GC_{pi}	C_{pMin}	C_{pMax}	$PCpMin_{psf}$	$PCpMax_{psf}$	P_{min} psf
OH	Overhang Top	356	All	All	0	0.257	-0.195	4.84	-3.66	4.80
OH	Overhang Leeward	278	All	All	0	-0.600	-0.600	-11.30	-11.30	4.80
OH_Bot	Overhang Bottom	3	All	All	0	0.800	0.800	15.07	15.07	4.80
Roof	Roof Windward	1	All	All	0.18	0.257	-0.195	0.85	-7.65	4.80
Roof	Roof Leeward	2	All	All	0.18	-0.600	-0.600	-15.29	-15.29	4.80
------	------	------	------	------	------	------	------	------	------	------
OH	Overhang Top	356	All	All	0	0.257	-0.195	4.84	-3.66	4.80
OH	Overhang Leeward	278	All	All	0	-0.600	-0.600	-11.30	-11.30	4.80
OH_Bot	Overhang Bottom	3	All	All	0	0.800	0.800	15.07	15.07	4.80
Roof	Roof Windward	1	All	All	-0.18	0.257	-0.195	8.83	0.32	4.80
Roof	Roof Leeward	2	All	All	-0.18	-0.600	-0.600	-7.31	-7.31	4.80

Roof Wind Pressures [Parallel to Ridge]
All wind pressures include a Load Factor (LF) of 0.6

Component	Description	Location	Start ft	End ft	GC_{pi}	C_{pMin}	C_{pMax}	$PCpMin_{psf}$	$PCpMax_{psf}$	P_{min} psf
OH_Bot	Overhang Bottom	57	All	All	0	0.800	0.800	15.07	15.07	4.80
OH_Top	Overhang Top (0 to h)	3457	0.000	16.306	0	-0.900	-0.180	-16.95	-3.39	4.80
OH_Top	Overhang Top (h to 2*h)	34	16.306	32.611	0.18	-0.500	-0.180	-13.40	-7.38	4.80
OH_Top	Overhang Top (>= 2*h)	3468	32.611	54.660	0.18	-0.300	-0.180	-9.64	-7.38	4.80
Roof	Roof (0 to h)	12	1.330	16.306	0.18	-0.900	-0.180	-20.94	-7.38	4.80
Roof	Roof (h to 2*h)	12	16.306	32.611	0.18	-0.500	-0.180	-13.40	-7.38	4.80
Roof	Roof (>= 2*h)	12	32.611	53.330	0.18	-0.300	-0.180	-9.64	-7.38	4.80
------	------	------	------	------	------	------	------	------	------	------
OH_Bot	Overhang Bottom	57	All	All	0	0.800	0.800	15.07	15.07	4.80
OH_Top	Overhang Top (0 to h)	3457	0.000	16.306	0	-0.900	-0.180	-16.95	-3.39	4.80
OH_Top	Overhang Top (h to 2*h)	34	16.306	32.611	-0.18	-0.500	-0.180	-5.43	0.60	4.80
OH_Top	Overhang Top (>= 2*h)	3468	32.611	54.660	-0.18	-0.300	-0.180	-1.66	0.60	4.80
Roof	Roof (0 to h)	12	1.330	16.306	-0.18	-0.900	-0.180	-12.96	0.60	4.80
Roof	Roof (h to 2*h)	12	16.306	32.611	-0.18	-0.500	-0.180	-5.43	0.60	4.80
Roof	Roof (>= 2*h)	12	32.611	53.330	-0.18	-0.300	-0.180	-1.66	0.60	4.80

Wind Pressures for C&C Ch 30 Pt 1 Roof & Wall
All wind pressures include a Load Factor (LF) of 0.6

Description	Zone	Width ft	Span ft	Area ft^2	1/3 Rule	Figure	GCp Max	GCp Min	p Max psf	p Min psf
Zone 1	1	1.000	1.000	1.00	No	30.3-2D	0.900	-1.800	23.93	-43.87
Zone 2	2	1.000	1.000	1.00	No	30.3-2D	0.900	-2.000	23.93	-48.30
Zone 3	3	1.000	1.000	1.00	No	30.3-2D	0.900	-2.500	23.93	-59.38
Zone 4	4	1.000	1.000	1.00	No	30.3-1	1.000	-1.100	26.14	-28.36
Zone 5	5	1.000	1.000	1.00	No	30.3-1	1.000	-1.400	26.14	-35.00
3068 EXT DOOR	4	3.000	6.670	20.01	No	30.3-1	0.947	-1.047	24.96	-27.18
4040 WINDOW	4	4.000	4.000	16.00	No	30.3-1	0.964	-1.064	25.34	-27.56
3050 WINDOW	4	3.000	5.000	15.00	No	30.3-1	0.969	-1.069	25.45	-27.67
3030 WINDOW	5	3.000	3.000	9.00	No	30.3-1	1.000	-1.400	26.14	-35.00

CalculationsPreparedby:
JL ENGINEERING & CONSULTING LLC
490 LAKESIDE DRIVE
DEFUNIAK SPRINGS,FL,32435
Date:May 09,2024
Designer:JDL

CalculationsPreparedFor:
Client: Lamneck/Poole
Project#: 25-2024
Location: DFS,FL
Description: SINGLE FAMILY RESIDENCE

General:
```
WindLoadStandard            =  ASCE7-22    | BasicWindSpeed                 =  140.0mph
ExposureClassification      =  C           | RiskCategory                   =  II
StructureType               =  Building    | DesignBasisforWindPressures    =  ASD
MWFRSAnalysisMethod         =  Ch27        | C&CAnalysisMethod              =  Ch30Pt1
DynamicTypeofStructure      =  Rigid       | ShowAdvancedOptions            =  False
```

Building:
```
Roof   =  RoofType                =  Gabled      | Encl   =  EnclosureClassification   =  Enclosed
Help   =  HelponBuildingRoofType  =  Help        | Pitch=     PitchofRoof               =  7.0:12
θ      =  SlopeofRoof             =  30.26Deg    | RHt    =  RidgeHeight               =  23.111ft
EHt    =  EaveHeight              =  9.500ft     | W      =  BuildingWidth             =  44.000ft
L      =  BuildingLength          =  52.000ft    | OH     =  TypeofOverhang            =  AllSoffit
Par    =  Parapet                 =  None        | HTover =  OverrideMeanRoofHeight    =  False
Htman  =  MeanRoofHeight          =  16.306ft    | RAover =  OverrideRoofArea          =  False
GCpi_  =  OverrideGCpivalue       =  False       | IsElev=    BuildingisElevated        =  False
```

ExposureConstants[Tbl26.11-1]:
```
α=3-sGust-speedexponent              =  9.800   | Zg=NominalHtofBoundaryLayer        =  2460.000ft
â=Reciprocolofα                      =  0.102   | b=3secgustspeedfactor              =  1.000
αm=  MeanhourlyWind-SpeedExponent    =  0.156   | bm=MeanhourlyWindspeedExponent     =  0.660
c=TurbulenceIntensityFactor          =  0.200   | ε=IntegralLengthScaleExponent      =  0.2000
```

OverhangInputs:
```
Std        =  Overhangsonallsidesarethesame                    =  True
OHType     =  TypeofRoofWallIntersections                      =  Soffit
OH         =  OverhangofRoofBeyondWall                         =  1.330ft
```

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JAY DYLAN LAIRD

Jay Dylan Laird

Digitally signed by Jay Dylan Laird
Date: 2025.02.13 11:07:36 -06'00'

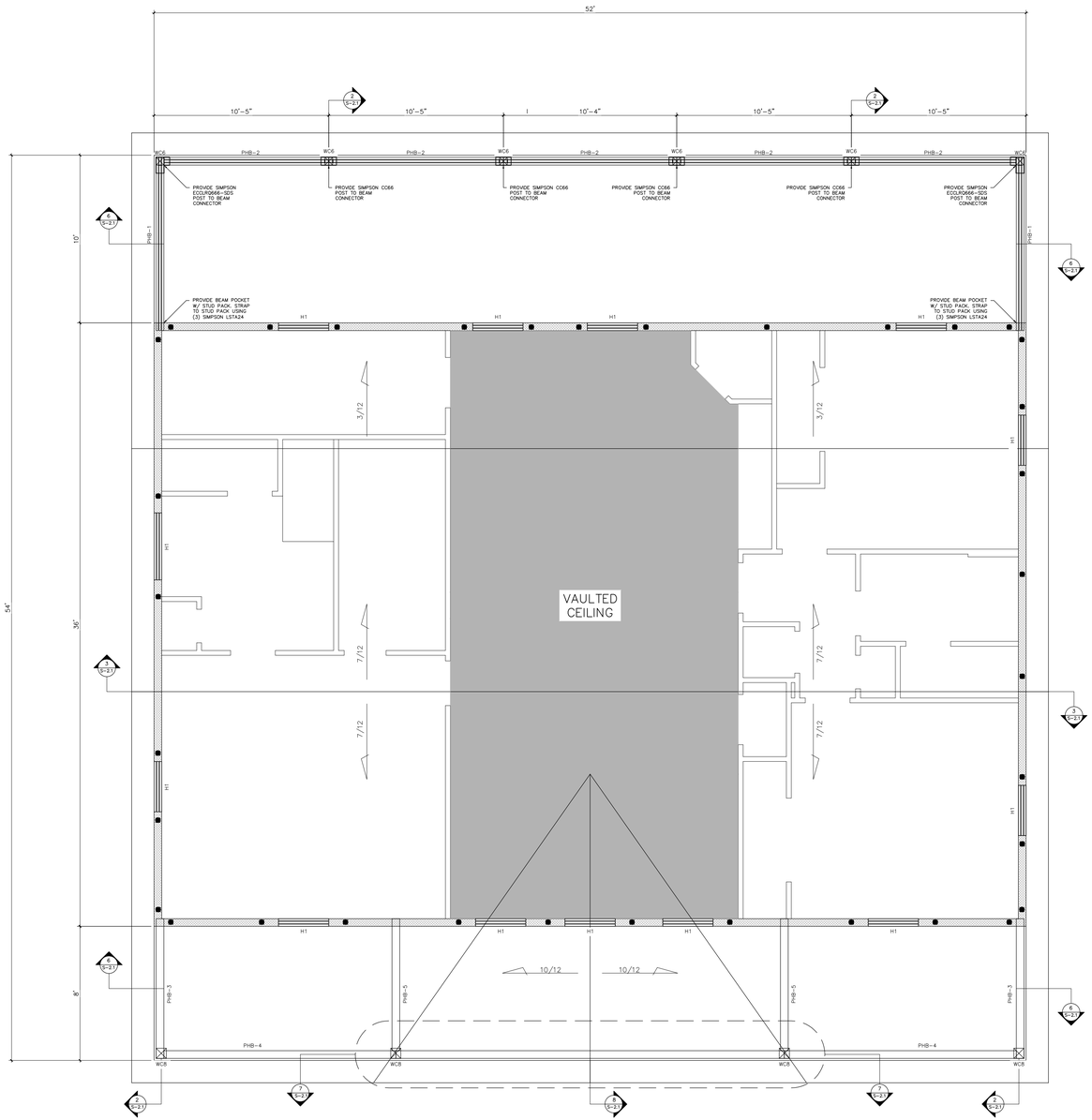
JL ENGINEERING & CONSULTING LLC
490 LAKESIDE DRIVE
DEFUNIAK SPRINGS, FL 32435
CONTACT: (850)-399-1188

No.	Revision/Issue	Date

Project Name and Address
LAMNECK/POOLE RESIDENCE

SHEET TITLE
GENERAL NOTES

Project: **25-2024**
Date: **05-06-2024**
Scale: **NTS**
Sheet: **S-1.0**



NOTE: SIMPSON CCTQ6X AND ECCLQ6X-SDS FRONT PORCH POST TO BEAM CONNECTORS SHALL HAVE THE FOLLOWING DIMENSIONS:
W1 = 5.5"
W2 = 7.5"
W3 = 5.5"

1 S-1.1 LOAD BEARING & ROOF FRAMING PLAN

SCALE: 1/4" = 1'=0"

FRAMING SCHEDULE

NO.	TYPE	SIZE	ANCHOR/STRAPPING SIDE 1	ANCHOR/STRAPPING SIDE 2	REMARKS
H1	WINDOW/DOOR HEADER	(3 PLY) 2X8 FLITCHED W/ (2) $\frac{3}{8}$" CDX/OSB PLYWOOD	SIMPSON LSTA 24	SIMPSON LSTA 24	CONNECT USING (2) ROWS 16D RING SHANK NAILS SPACED @ 16" O.C. EACH SIDE
WC6	PT WOOD COLUMN	PT 6X6	SEE DETAILS SHEET S-2.0	SIMPSON ECCLR/CC	SET PT COLUMN FLUSH WITH EDGE OF SLAB
WC8	PT WOOD COLUMN	PT 8x8	SEE DETAILS SHEET S-2.0	SEE DETAILS SHEET S-2.1	SET PT COLUMN FLUSH WITH EDGE OF SLAB
PHB-1	PORCH HEADER BEAM	(3 PLY) 2X12 FLITCHED W/ (2) $\frac{3}{8}$" CDX/OSB SPACERS	STUD PACK W/ SIMPSON LSTA 24	SIMPSON ECCLR/CC	CONNECT USING (2) ROWS 16D RING SHANK NAILS SPACED @ 12" O.C. EACH SIDE
PHB-2	PORCH HEADER BEAM	(3 PLY) 2X12 FLITCHED W/ (2) $\frac{3}{8}$" CDX/OSB SPACERS	SIMPSON ECCLR/CC	SIMPSON ECCLR/CC	CONNECT USING (2) ROWS 16D RING SHANK NAILS SPACED @ 12" O.C. EACH SIDE
PHB-3	PORCH HEADER BEAM	PRESSURE TREATED 5.5"X14" GULAM BEAM	STUD PACK W/ SIMPSON LSTA 24	SIMPSON ECCLQX	FASTEN TO BEAM TO SIMPSON CONNECTORS USING MFG RECOMMENDED FASTENERS ONLY
PHB-4	PORCH HEADER BEAM	PRESSURE TREATED 5.5"X14" GULAM BEAM	SIMPSON ECCLQX	SIMPSON CCTQ6X	FASTEN TO BEAM TO SIMPSON CONNECTORS USING MFG RECOMMENDED FASTENERS ONLY
PHB-5	PORCH HEADER BEAM	PRESSURE TREATED 5.5"X14" GULAM BEAM	STUD PACK W/ SIMPSON LSTA 24	SIMPSON CCTQ6X	FASTEN TO BEAM TO SIMPSON CONNECTORS USING MFG RECOMMENDED FASTENERS ONLY
PHB-6	PORCH HEADER BEAM	PRESSURE TREATED 5.5"X14" GULAM BEAM	SIMPSON CCTQ6X	SIMPSON CCTQ6X	FASTEN TO BEAM TO SIMPSON CONNECTORS USING MFG RECOMMENDED FASTENERS ONLY

INTERIOR NON LOAD BEARING WALL

LOAD BEARING WALL/SHEAR WALL

ROOF PLAN & FRAMING NOTES

1. FOR GENERAL NOTES AND DESIGN LIVE LOADS SEE SHEET S-1
2. FOR TYPICAL FRAMING DETAILS SEE S-SERIES SHEETS. SEE ARCHITECTURAL DWGS. FOR DIMENSIONS NOT SHOWN. IF A CONFLICT EXISTS, THE ARCH DIMENSIONS SHALL GOVERN. NOTIFY ENGINEER OF RECORD IN WRITING IF CONFLICT EXISTS.
3. ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT WITH CONCRETE/MASONRY SHALL BE PRESSURE TREATED.
4. ATTACH ALL ROOF TRUSSES TO TOP PLATE USING SIMPSON H10A/ H10A-2/ MTS18 HURRICANE TIES. SEE SHOP DRAWINGS FOR GIRDER TRUSS HOLD DOWN.
5. TRUSS TO TRUSS/BEAM CONNECTORS SHALL BE SPECIFIED BY TRUSS DESIGNER, OVERHANG: 1'-0"
6. ■ - INDICATES LOCATIONS OF $\frac{3}{4}$" HDG ALLTHREAD HOLD DOWNS OR HD3B HOLD DOWNS FOR SHEAR WALL ENDS.
7. TRUSS MFG TO PROVIDE SPACE FOR HVAC DUCT AND UNIT
8. ROOF PITCH TO BE 7:12 MAIN, SEE PLAN FOR VARIATIONS

FRAMING NOTES:

- STUD PACKS SHALL BE CONTINUOUS THROUGH ZONE OF GIRDER TRUSSES VIA. MATCHING BLOCKING.
- SHEAR WALLS WHERE NOTED TO HAVE $\frac{3}{8}$" PLYWOOD EACH SIDE NAILED @ 4" O/C EDGE AND 6" O/C FIELD. PLACE SIMPSON HD3B HOLD DOWN EACH END OF SHEAR WALL TO DOUBLE STUD PACK.
- ALL INTERIOR PLUMBING WALLS TO BE 2X6 TO ACCOMMODATE PIPING.

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490 LAKESIDE DRIVE
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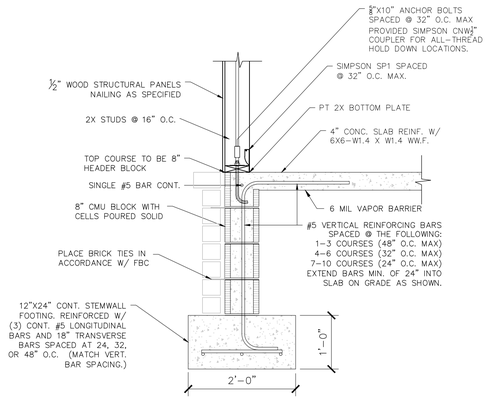
CONTACT: (850)-399-1188

No.	Revision/Issue	Date

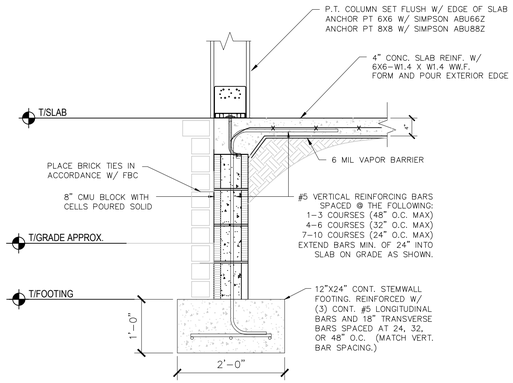
Project Name and Address
LAMNECK/POOLE RESIDENCE

SHEET TITLE
ROOF AND FRAMING PLAN

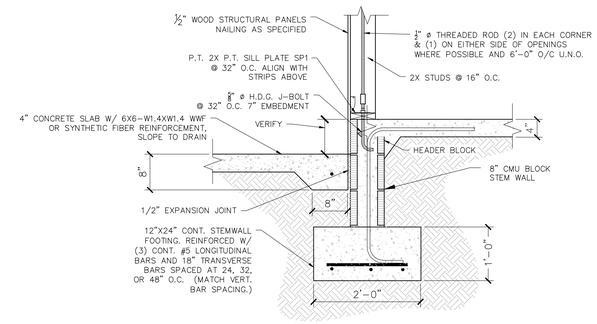
Project	Sheet
25-2024	S-1.1
Date: 05-06-2024	
Scale: 1/4" = 1'-0"	



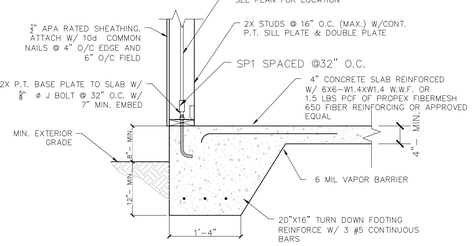
1 / S-2.0 **TYP. PERIMETER WALL FOOTING DETAIL**
N.T.S. ALT. DETAIL FOR CMU FOUNDATION W/ SLAB



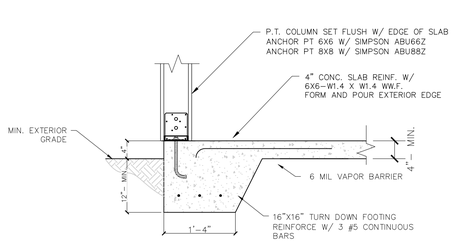
2 / S-2.0 **TYP. PORCH PERIMETER COLUMN DETAIL**
N.T.S. ALT. DETAIL FOR CMU FOUNDATION W/ SLAB



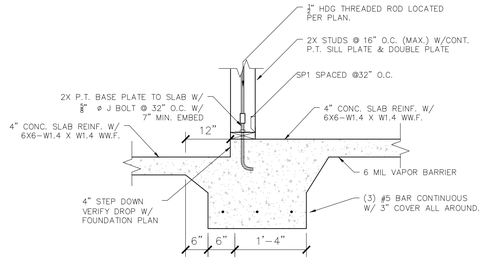
3 / S-2.0 **TYP. PORCH STEP DOWN DETAIL**
N.T.S. ALT. DETAIL FOR CMU FOUNDATION W/ SLAB



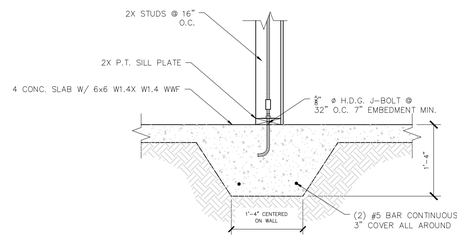
1B / S-2.0 **TYP EXT. WALL FOOTING (MONO)**
SCALE: 3/4" = 1'-0"



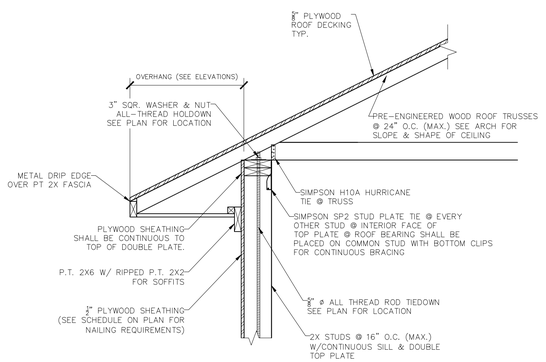
2B / S-2.0 **TYP PORCH COLUMN FOOTING (MONO)**
SCALE: 3/4" = 1'-0"



3B / S-2.0 **RECESS SLAB FOOTING (PORCH) MONO FOOTING**
SCALE: 3/4" = 1'-0"



4 / S-2.0 **TYP. INTERIOR LOAD BEARING FOOTING**
SCALE: 3/4" = 1'-0"



1 / S-2.1 **TYP. EXTERIOR WALL ROOF FRAMING DETAIL**
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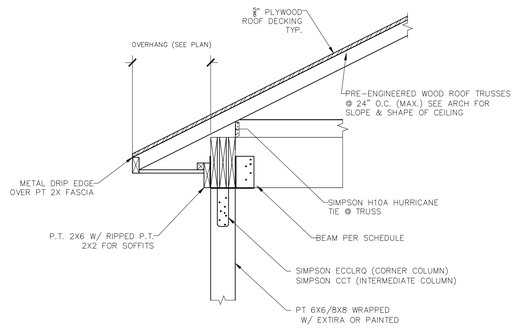

2 / S-2.1 **TYP. EXTERIOR COLUMN ROOF FRAMING DETAIL**
SCALE: N.T.S.

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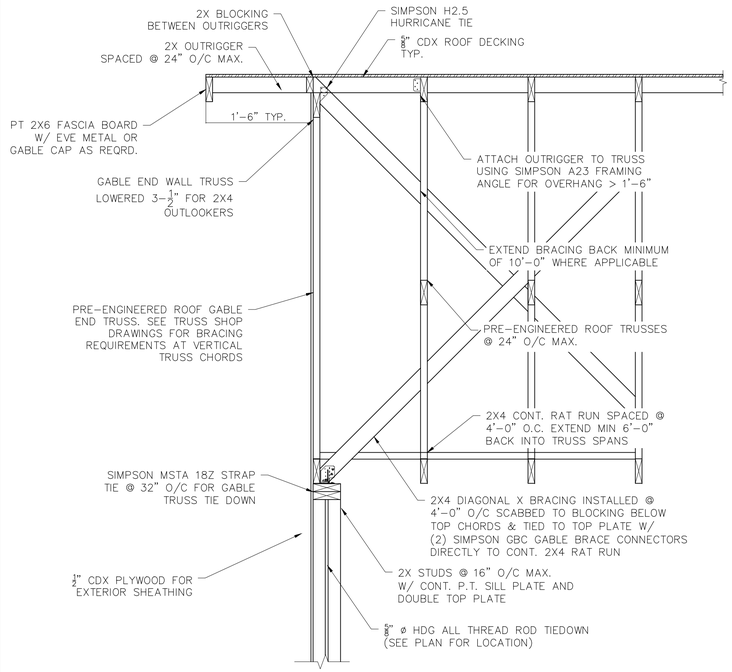
No.	Revision/Issue	Date

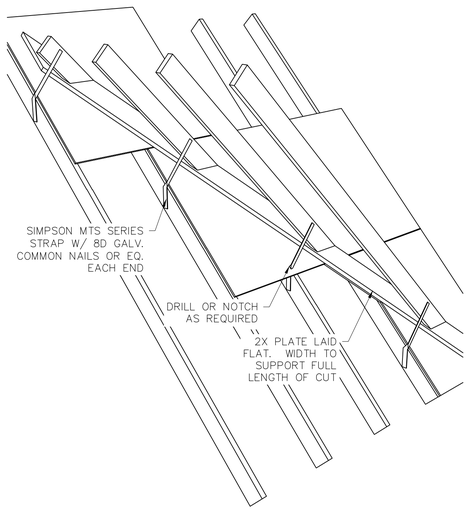
Project Name and Address
LAMNECK/POOLE RESIDENCE

SHEET TITLE
TYPICAL FOUNDATION & FLOOR FRAMING DETAILS

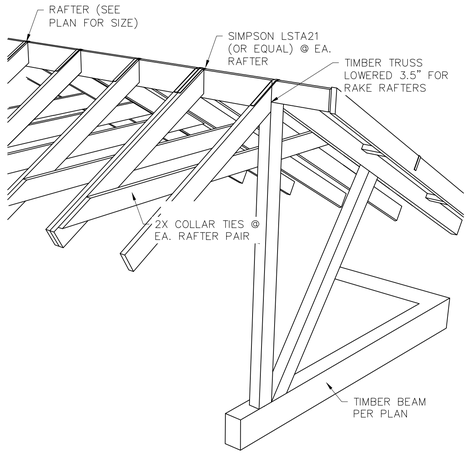
Project	Sheet
25-2024	S-2.0
Date: 05-06-2024	
Scale: NTS	



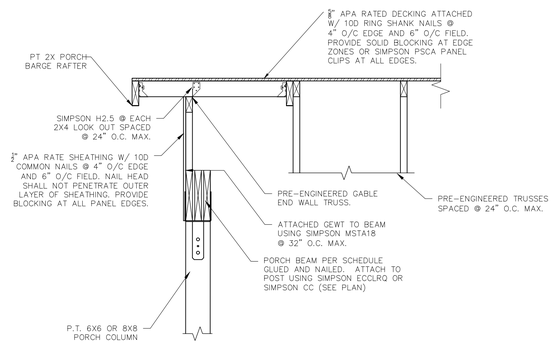
3 S-2.1 **TYP. GABLE END ROOF FRAMING DETAIL**
SCALE: 3/4" = 1'-0"



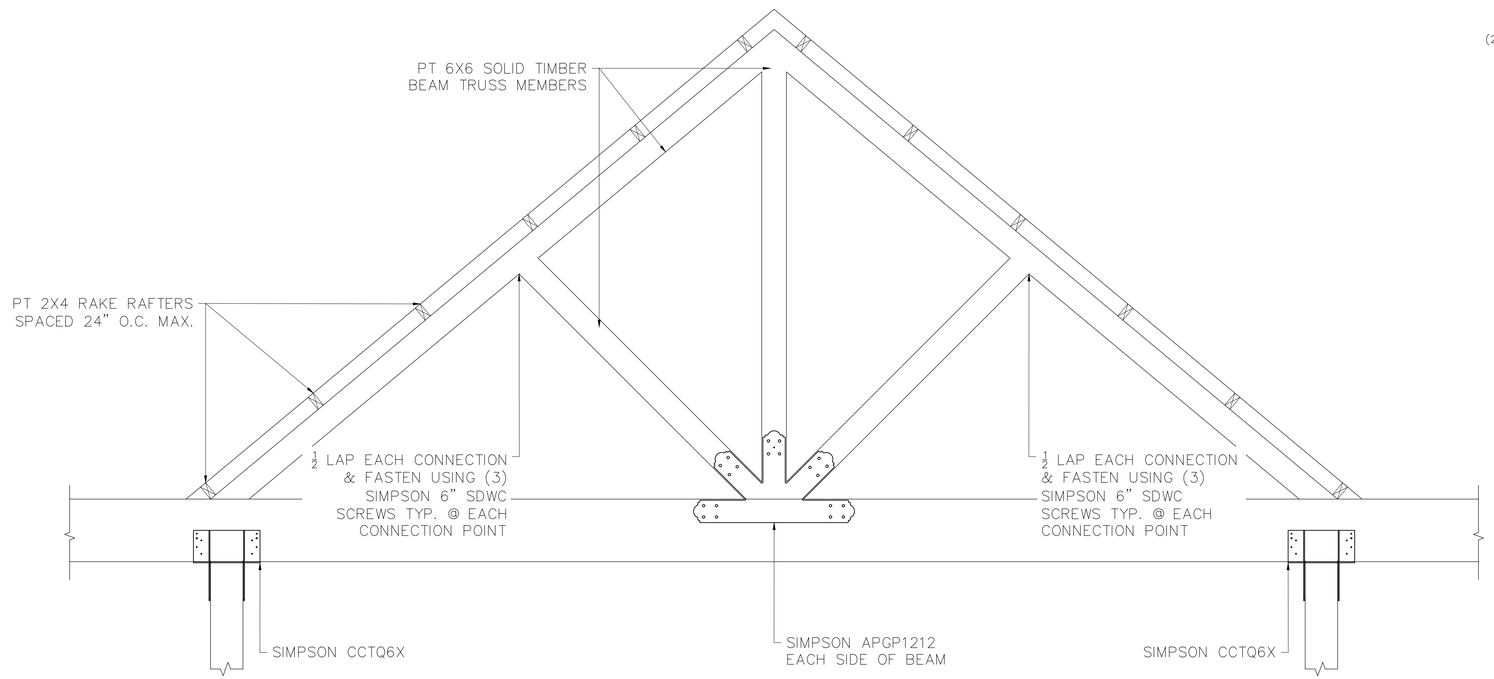
4 S-2.1 **LAY ON/ GABLE FRAME OVER VALLEY**
N.T.S.



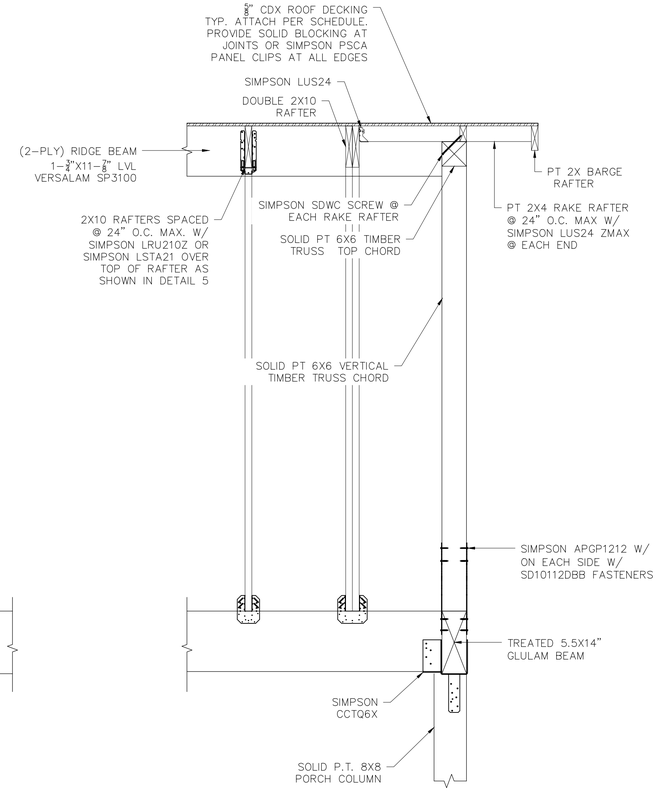
5 S-2.1 **GABLE END PORCH FRAME**
N.T.S.



6 S-2.1 **BARGE RAFTER DETAIL @ COLUMN**
N.T.S.



7 S-2.1 **GABLE END FRONT PORCH FRAME**
N.T.S.



8 S-2.1 **GABLE END FRONT PORCH FRAME**
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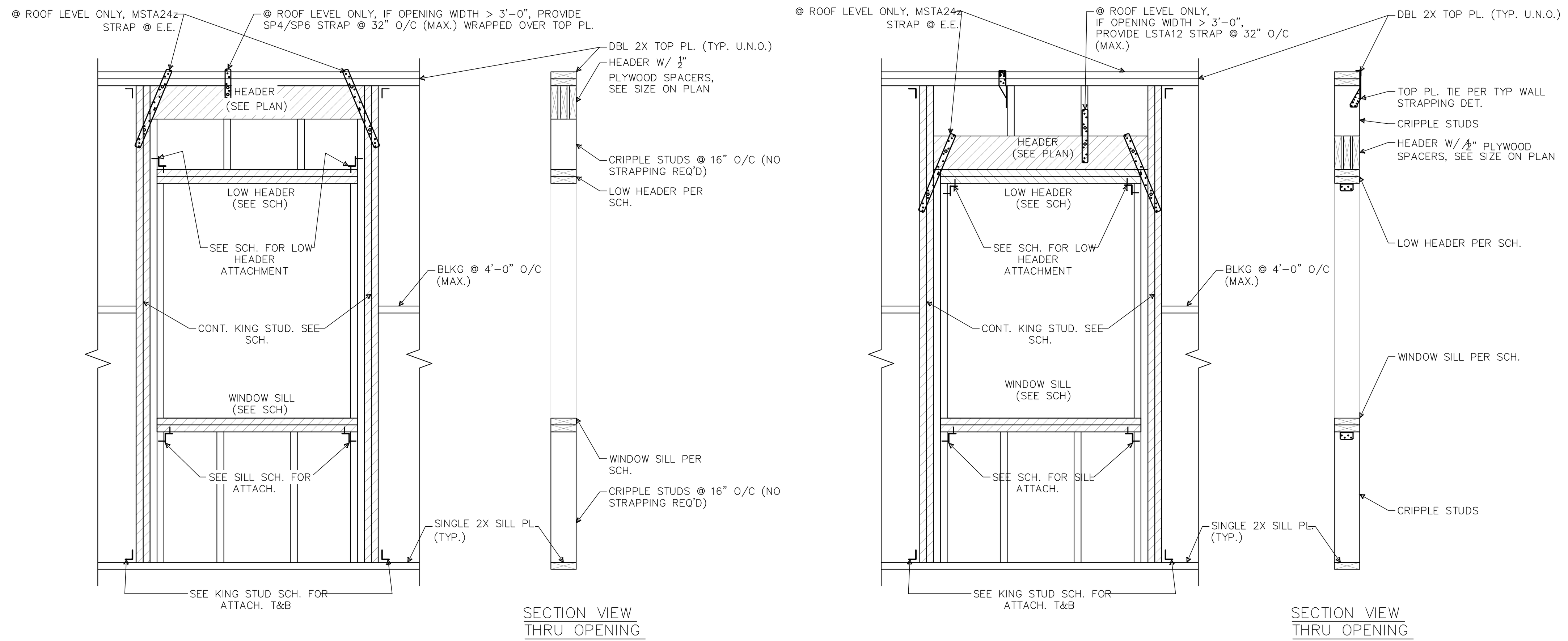
No.	Revision/Issue	Date

Project Name and Address
LAMNECK/POOLE RESIDENCE

SHEET TITLE
TYPICAL ROOF FRAMING DETAILS

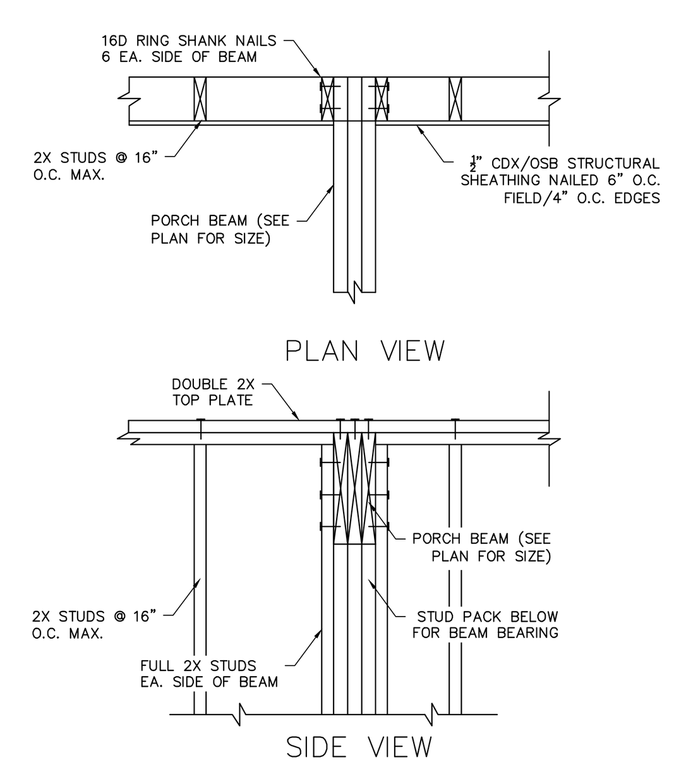
Project	Sheet
25-2024	S-2.1
Date: 05-06-2024	
Scale: NTS	



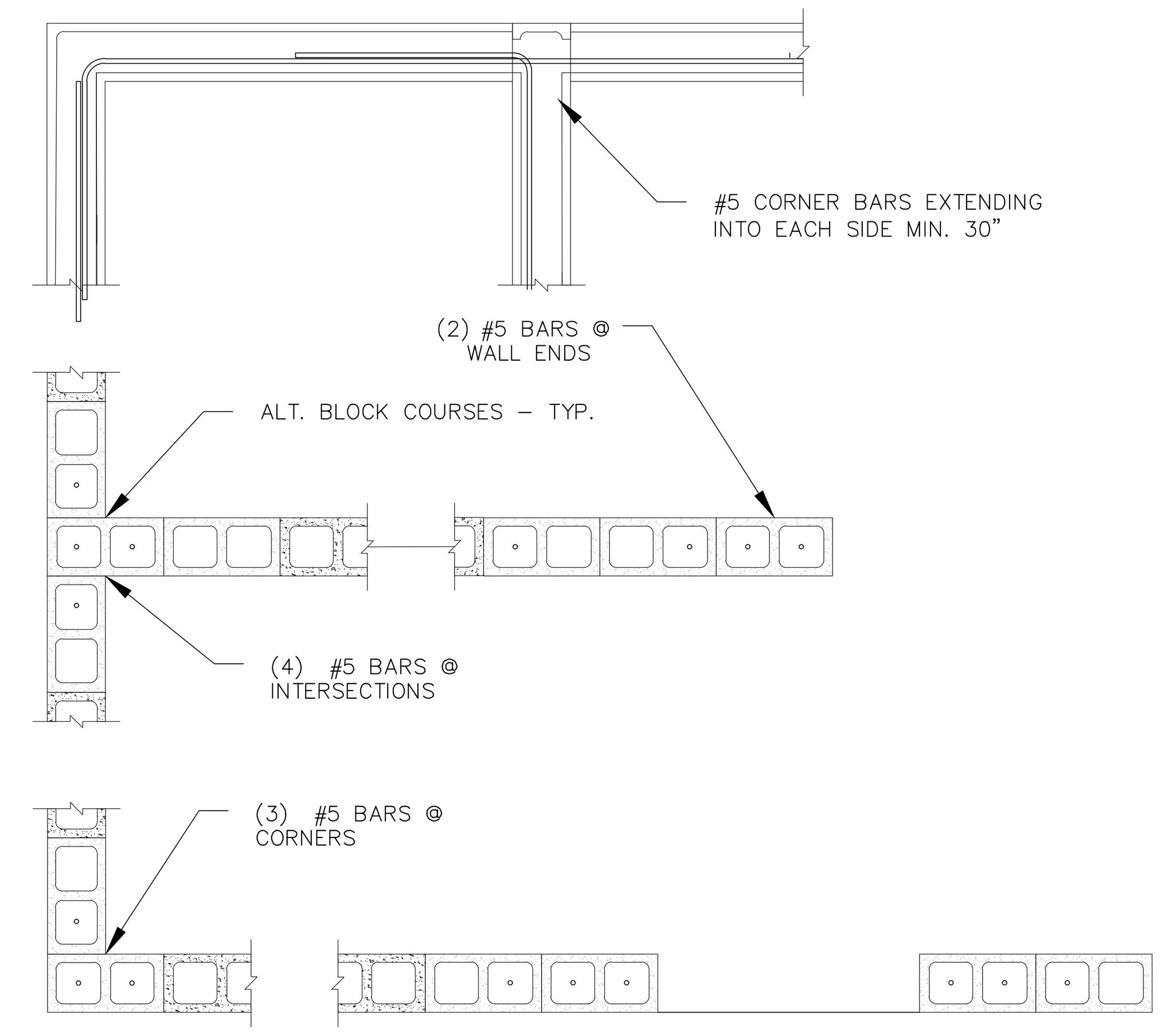
1 (S-3) OPTION 1&2 WALL OPENING W/ HEADER BEARING
SCALE: N.T.S.

WINDOW SILL & LOW HEADER SCHEDULE		
MAX. WIDTH OF OPENING	MIN. MEMBER SIZE	FASTENERS/ CONNECTOR @ EA. END
3'-0"	1 -2X6	4 – 3"×0.131" TOE NAILS
6'-6"	2 -2X6	2 – SIMPSON A23 ANGLES (1 TOP, 1 BOTTOM)
9'-6"	2 – 1-¾" X 5-½" LVL	SIMPSON HL35 ANGLEW/(4) ½"Ø CARRIAGE BOLTS INTO SILL PL./ LOW HEADER & (4) ½"X5" LONG LAG BOLTS INTO KING STUD
12'-0"	3 – 1-¾" X 5-½" LVL	SIMPSON HL35 ANGLEW/(4) ½"Ø CARRIAGE BOLTS INTO SILL PL./ LOW HEADER & (4) ½"X5" LONG LAG BOLTS INTO KING STUD
> 12'-0"	SEE PLAN OR CONSULT W/ EOR	

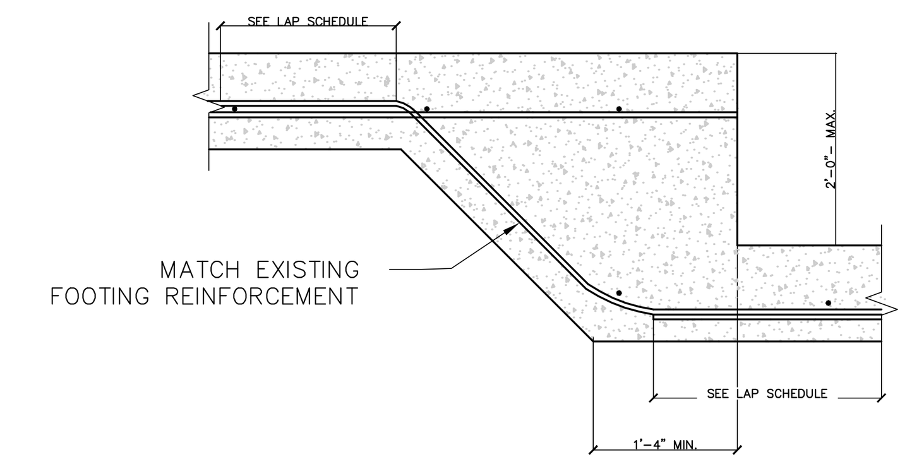
NOTE:
1. PROVIDE 2X6 TRIMMER BELOW EA. END OF WINDOW SILL & LOW HEADER.
2. SEE ARCH. DRAWINGS FOR OPENING WIDTH.
3. ALL WINDOW SILL AND LOW HEADER PLIES SHALL BE INSTALLED IN FLAT ORIENTATION.



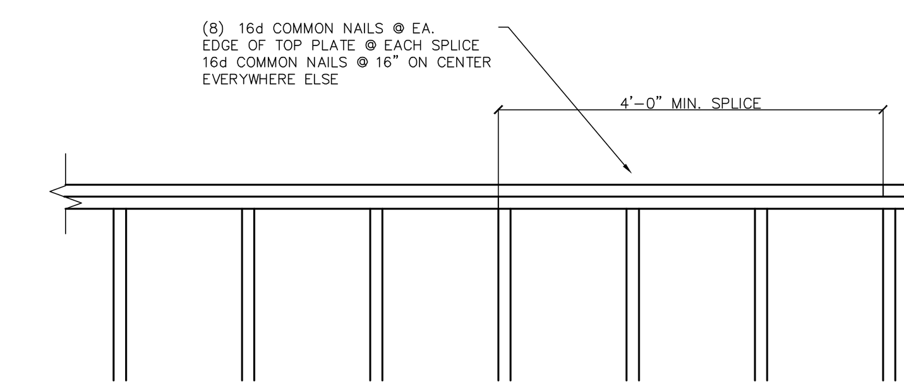
3 (S-3) TYPICAL PORCH BEAM POCKET
N.T.S.



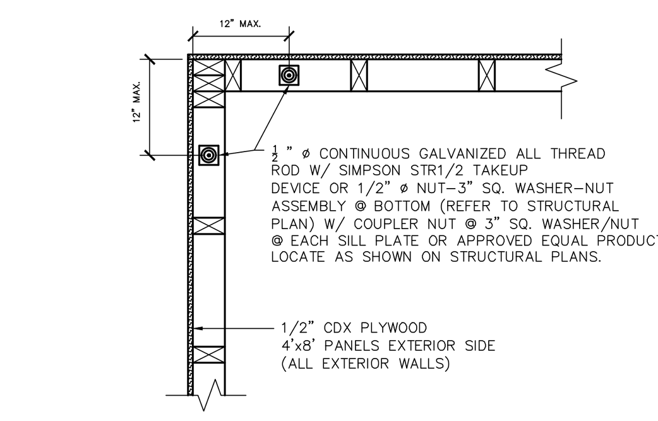
7 (S-3) TYPICAL VERT. & HORIZ. REINFORCING @ CMU WALLS
SCALE: N.T.S.



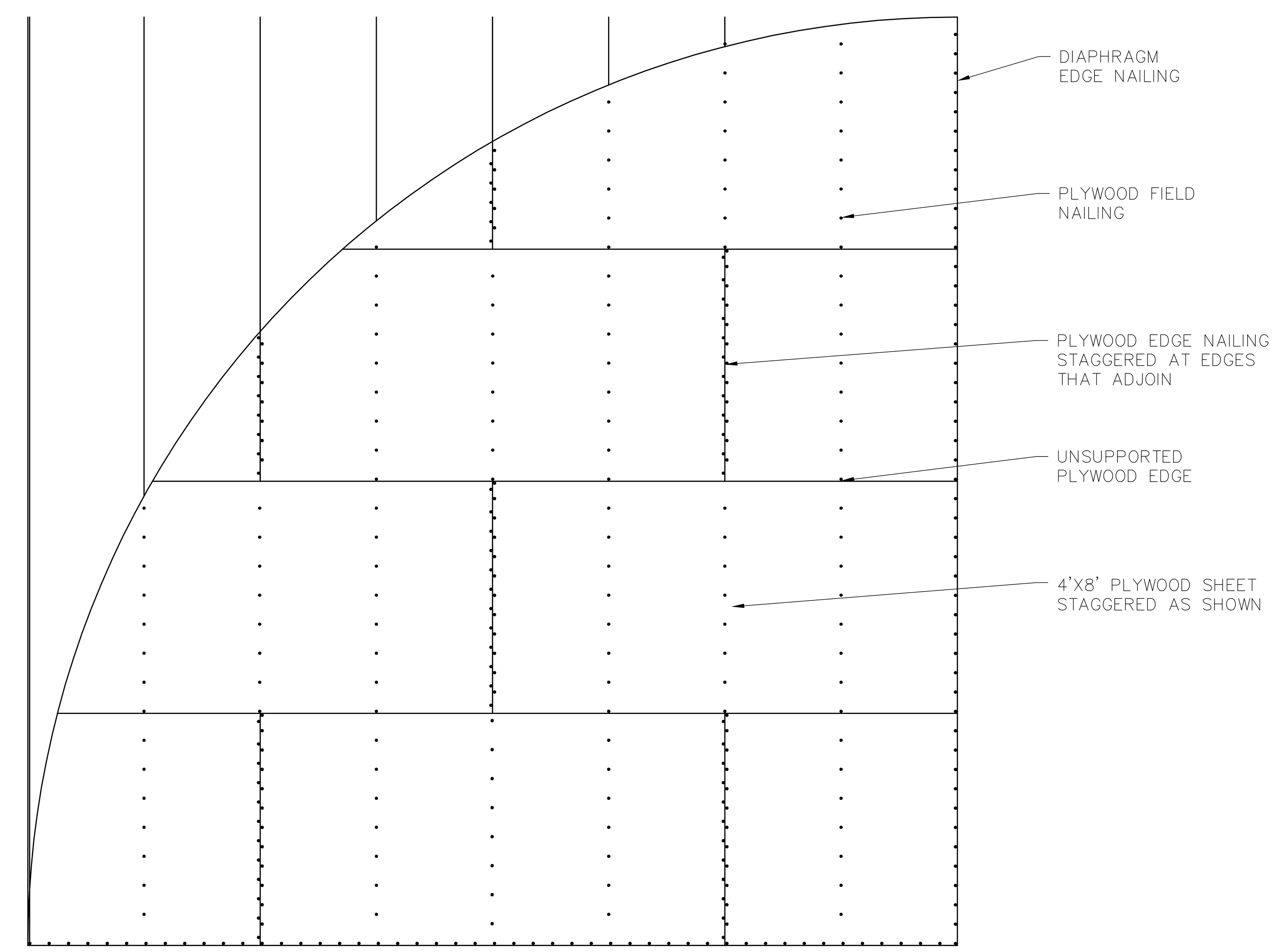
8 (S-3) TYP. CMU FOOTING STEP DETAIL
SCALE: N.T.S.



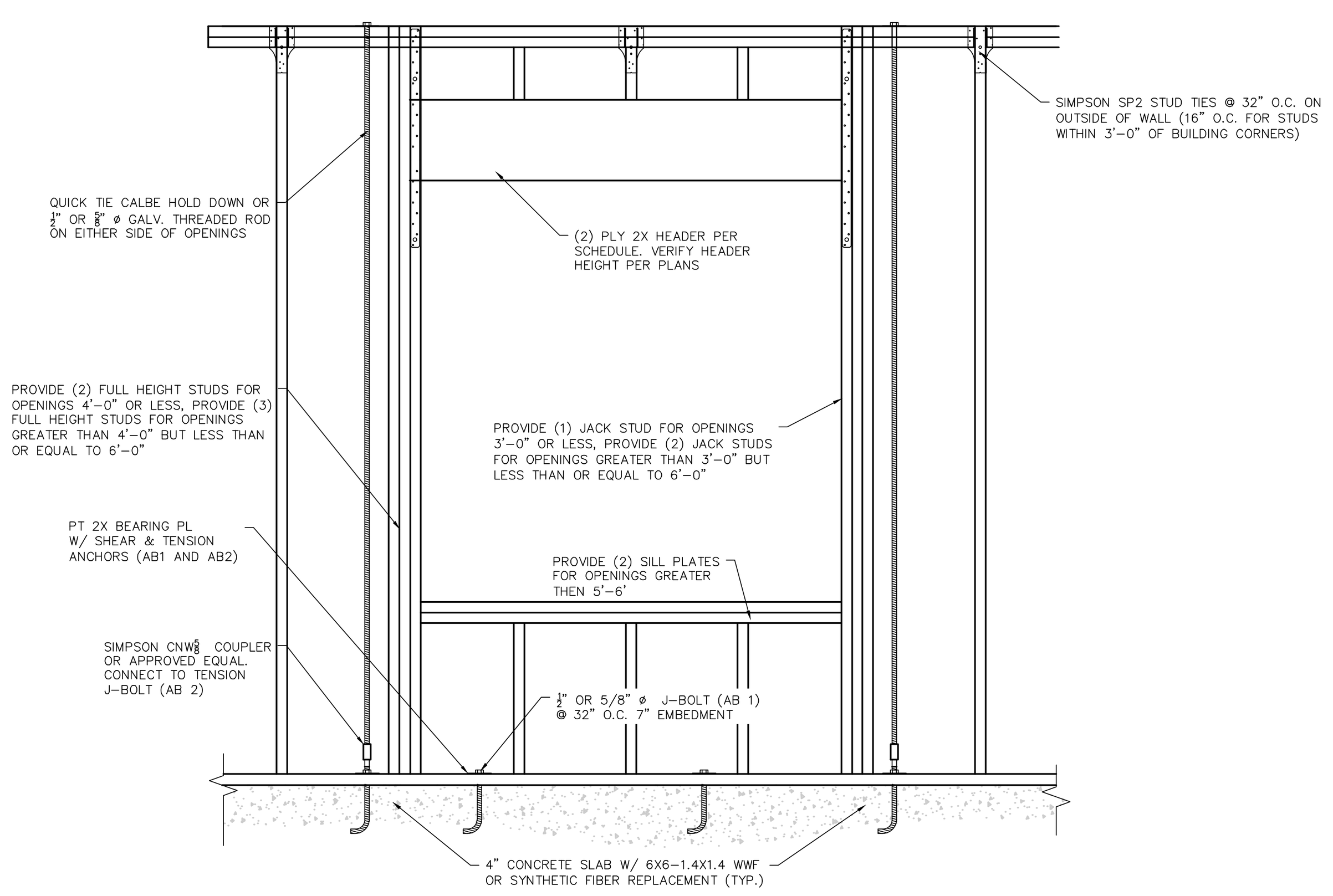
9 (S-3) WALL TOP PLATE SPLICE DETAIL
N.T.S.



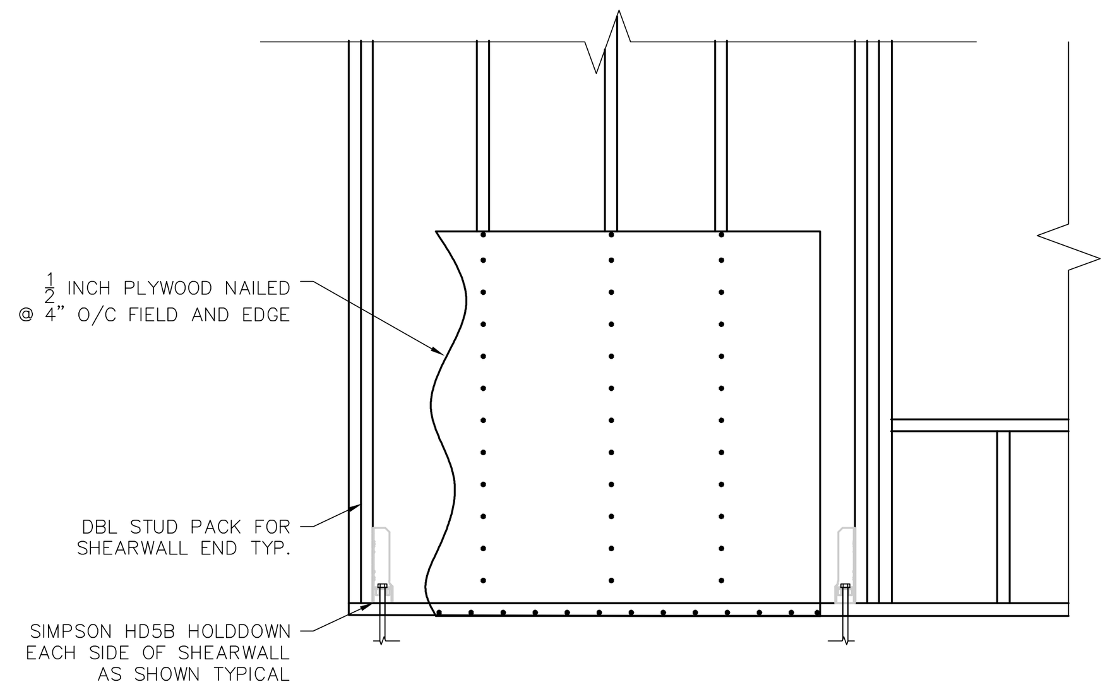
10 (S-3) THREADED ROD HOLDOWN DETAIL
N.T.S.



11 (S-3) ROOF/WALL PANEL SCHEDULE AND NAILING DETAIL
N.T.S.



2 (S-3) TYPICAL ALIGNED OPENING FRAMING
N.T.S.



6 (S-3) TYPICAL SHEARWALL DETAIL
SCALE: N.T.S.

JACK STUD & CONT. STUD REQUIREMENTS FOR ROUGH OPENINGS (R.O.)						
	UP TO 3'-0" R.O.		3'-1" TO 6'-0" R.O.		6'-0" TO 10'-0" R.O.	
	CONT.	JACK	CONT.	JACK	CONT.	JACK
2X EXTERIOR WALL SINGLE STORY	(2) 2X6	(1) 2X6	(3) 2X6	(2) 2X6	(4) 2X6	(2) 2X6

NAILING SCHEDULE	
JOISTS SITTING ON SILL OR GIRDER	(3) 8d TOENAILS, EACH SIDE
BLOCKING BETWEEN JOISTS/ RAFTERS	(2) 10d TOENAILS, EACH SIDE, EACH END
RIM BLOCKING BETWEEN JOISTS/ RAFTERS	(3) 10d TOENAILS EACH END
TOP PLATE TO STUD	(2) 16d END NAILS
STUD TO SILL PLATE	(2) 16d END NAILS OR (4) 8d TOENAILS
DOUBLE STUDS	(2) 10d @ 12" ON CENTER
DOUBLE TOP PLATES-BETWEEN SPLICE NAILING	16d @ 16" ON CENTER FACE NAILS
DOUBLE TOP PLATES-EA. SIDE OF SPLICED PLATE	(8) 16d
ELEVATED SILL PLATE TO 2X TRUSS RIBBON BOARD OR END WALL TRUSS	10d FACE NAILS @ 6" ON CENTER
BLOCKING TO TOP PLATE	(2) 10d TOENAILS, EACH SIDE
RIM JOIST OR BLOCKING TO TOP PLATE OR SILL PLATE	8d TOENAILS @ 6" ON CENTER
CONT. (2) AND (3) PIECE HEADERS	16d @ 16" ON CENTER ALONG EACH EDGE
BUILT UP CORNER STUDS	16d @ 24" ON CENTER
DECKING BOARDS	(2) #8 2 1/2" ST STEEL SCREWS @ EACH BEARING
CROSS BRIDGING	(2) 10d EACH END
HORIZONTAL BLOCKING BETWEEN WALL STUDS	(2) 10d TOENAILS EACH END

WALL SHEATHING SCHEDULE		
LOCATION	SHEATHING	NAILING
EXTERIOR WALL (TYP.)	1/2" PLYWOOD SHEATHING	10d COMMON NAILS @ 4" ON CENTER (MAX) @ PANEL EDGES & 6" ON CENTER (MAX) FIELD

ROOF DIAPHRAGM SCHEDULE				
LOCATION	SHEATHING	UNSUPPORTED EDGE	FIELD NAILING	EDGE NAILING
ROOF	5/8" APA RATED PLYWOOD	SIMPSON PSCL 19/32 PANEL SHEATHING CLIP OR EQUAL	10d RING SHANK NAILS @ 6" O/C	10d RING SHANK NAILS @ 4" O/C
ROOF OVERHANG	5/8" APA RATED PLYWOOD U.N.O. SEE ELEV FOR FINISHES	N/A	10d RING SHANK NAILS @ 3" O/C	10d RING SHANK NAILS @ 3" O/C

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Jay Dylan Laird

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DEFUNIAK SPRINGS, FL 32435
CONTACT: (850)-399-1188

No.	Revision/Issue	Date

Project Name and Address
LAMNECK/POOLE RESIDENCE

SHEET TITLE
TYPICAL FOUNDATION & FRAMING DETAILS

Project 25-2024
Date 05-06-2024
Scale NTS
Sheet **S-2.2**