DATE 04/02/2007

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000025685

APPLICANT KATIE REED PHONE 752-4072

ADDRESS 2230 SE BAYA DRIVE LAKE CITY FL 32025

OWNER CARL & LYNN WAGONER PHONE 407 423-3694

ADDRESS 274 NW EVERETT TERR WHITE SPRING FL 32096

CONTRACTOR DON REED CONSTRUCTION PHONE 752-4072

LOCATION OF PROPERTY 41N, TL ON SUWANNEE VALLEY RD, TR ON EVERETT TERR, 1/4 MILE ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 136900.00

HEATED FLOOR AREA 2738.00 TOTAL AREA 4376.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 8/12 FLOOR SLAB

LAND USE & ZONING ESA-2 MAX. HEIGHT 28

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE AE DEVELOPMENT PERMIT NO. 07-005

PARCEL ID 20-2S-16-01660-006 SUBDIVISION LEVINGS UNREC

LOT 6 BLOCK PHASE UNIT TOTAL ACRES

000001359 CGC036224 x Katie Reed

Culvert Permit No. | Culvert Waiver | Contractor's License Number | Applicant/Owner/Contractor

WAIVER 07-0180-N BK JH

Driveway Connection | Septic Tank Number | LU & Zoning checked by | Approved for Issuance | New Resident

COMMENTS: ONE FOOT RISE LETTER ON FILE, ELEVATION CERTIFICATE NEEDED BEFORE POWER, NOC ON FILE

Check # or Cash 6401

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by | Foundation date/app. by | Monolithic date/app. by

Under slab rough-in plumbing date/app. by | Slab date/app. by | Sheathing/Nailing date/app. by

Framing date/app. by | Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by | Heat & Air Duct date/app. by | Peri. beam (Lintel) date/app. by

Permanent power date/app. by | C.O. Final date/app. by | Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by | Pool date/app. by

Reconnection date/app. by | Pump pole date/app. by | Utility Pole date/app. by

M/H Pole date/app. by | Travel Trailer date/app. by | Re-roof date/app. by

BUILDING PERMIT FEE $ 685.00 CERTIFICATION FEE $ 21.88 SURCHARGE FEE $ 21.88

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 803.76

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0703-48 Date Received 3-19-07 By G Permit # 1359/25685

Application Approved by - Zoning Official BLK Date 03.04.07 Plans Examiner OK JTH Date 3-27-07

Flood Zone AE Development Permit YES Zoning ESA-2 Land Use Plan Map Category ESA

Comments Elevation cert needed before power, Suwannee River
1000 one foot rise letter on file

Applicants Name Katie Reed Phone 386-752-4072

Address 2230 SE Baya Dr. #101 Lake City, FL 32025

Owners Name Carl & Lynn Wagoner Phone 407-423-3694

911 Address 274 NW Everett Terrace White Springs, FL 32096

Contractors Name Don Reed Construction Phone 386-752-4072

Address 2230 SE Baya Dr. #101 Lake City, FL 32025

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Mark Disosway P.O. Box 868 Lake City, FL 32056

Mortgage Lenders Name & Address First Federal Savings Bank 4307 us 90 w Lake City, FL 3205

Circle the correct power company – FL Power & Light – Clay Elec. – (Suwannee Valley Elec.) – Progressive Energy

Property ID Number 20-2S-16-01616D-0006 Estimated Cost of Construction $352,000.00

Subdivision Name Levings S/D Unrec. Lot 16 Block ___ Unit ___ Phase ___

Driving Directions Highway 41 N; TL on Suwannee Valley Rd; TR on Everett Terrace; 1/4 mile on left

Type of Construction SFD / New Construction Number of Existing Dwellings on Property Ø

Total Acreage 13.05 Lot Size ___ Do you need a - Culvert Permit or (Culvert Waiver) or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 350' Side 150' Side 206' Rear 894'

Total Building Height 28'6" Number of Stories 1 Heated Floor Area 2738 (TOTAL 4376) Roof Pitch 8/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Don Reed

DON REED

Owner Builder or Agent (Including Contractor)

Contractor Signature

Contractors License Number CGC036224

Competency Card Number ___

STATE OF FLORIDA
COUNTY OF COLUMBIA

NOTARY STAMP/SEAL

Sworn to (or affirmed) and subscribed before me this 19th day of March 2007.

Personally known ✓ or Produced Identification ___

Jamie T. Revis

Notary Signature

JAMIE T. REVIS
Notary Public - State of Florida
My Commission Expires Jul 19, 2009
Commission # DD 452695
Bonded By National Notary Assn.

25685

MEMO OF REVIEW FOR CORRECTNESS AND COMPLETION

In accordance with participation in the NFIP/CRS program, all elevation certificates are required to be reviewed for correctness and completion prior to acceptance by the community. This completed form shall be attached to all elevation certificates maintained on file and provided with requested copies of elevation certificates.

_____ The attached elevation certificate requires corrections by the surveyor of section(s) _____ prior to acceptance by the community.
X The attached elevation certificated is complete and correct.
_____ Minor corrections have been made in the below marked sections by the authorized Community Official.

SECTION A - PROPERTY INFORMATION

A1. Building Owner's Name	For Insurance Company Use:
A2. Building Street Address (including Apt., Unit, Suite, and/or Bldg. No.) or P.O. Route and Box No.	Policy Number
City State ZIP Code	Company NAIC Number

A3. Property Description (Lot and Block Numbers, Tax Parcel Number, Legal Description, etc.)

A4. Building Use (e.g., Residential, Non-Residential, Addition, Accessory, etc.) _____
A5. Latitude/Longitude: Lat. _____ Long. _____ Horizontal Datum: ☐ NAD 1927 ☐ NAD 1983
A6. Attach at least 2 photographs of the building if the Certificate is being used to obtain flood insurance.
A7. Building Diagram Number_____
A8. For a building with a crawl space or enclosure(s), provide:
a) Square footage of crawl space or enclosure(s) _____ sq ft
b) No. of permanent flood openings in the crawl space or enclosure(s) walls within 1.0 foot above adjacent grade _____
c) Total net area of flood openings in A8.b _____ sq in

A9. For a building with an attached garage, provide:
a) Square footage of attached garage _____ sq ft
b) No. of permanent flood openings in the attached garage walls within 1.0 foot above adjacent grade _____
c) Total net area of flood openings in A9.b _____ sq in

SECTION B - FLOOD INSURANCE RATE MAP (FIRM) INFORMATION

B1. NFIP Community Name & Community Number	B2. County Name				B3. State
B4. Map/Panel Number	B5. Suffix	B6. FIRM Index Date	B7. FIRM Panel Effective/Revised Date	B8. Flood Zone(s)	B9. Base Flood Elevation(s) (Zone AO, use base flood depth)

B10. Indicate the source of the Base Flood Elevation (BFE) data or base flood depth entered in Item B9.
☐ FIS Profile ☐ FIRM ☐ Community Determined ☐ Other (Describe) _____
B11. Indicate elevation datum used for BFE in Item B9: ☐ NGVD 1929 ☐ NAVD 1988 ☐ Other (Describe) _____
B12. Is the building located in a Coastal Barrier Resources System (CBRS) area or Otherwise Protected Area (OPA)? ☐ Yes ☐ No
Designation Date_____ ☐ CBRS ☐ OPA

COMMENTS: _____

Date of Review: 11-13-10 BOARD MEETS FIRST THURSDAY AT 7:00 P.M. AND THIRD THURSDAY AT 6:00 P.M. **Community Official:** _____

All elevation certificates shall be maintained by the community and copies with the attached memo made available upon request.

C5485

U.S. DEPARTMENT OF HOMELAND SECURITY
Federal Emergency Management Agency
National Flood Insurance Program

ELEVATION CERTIFICATE

OMB No. 1660-0008
Expires February 28, 2009

Important: Read the instructions on pages 1-8.

SECTION A - PROPERTY INFORMATION

For Insurance Company Use:

A1. Building Owner's Name CARLWAGNER & WANDA WAGNER

Policy Number

A2. Building Street Address (including Apt., Unit, Suite, and/or Bldg. No.) or P.O. Route and Box No.
274 NW EVERETT TER

Company NAIC Number

City WHITE SPRINGS State FL ZIP Code 32096

A3. Property Description (Lot and Block Numbers, Tax Parcel Number, Legal Description, etc.)
LOT 6 LEVINGS S/D UNRECORDED, PARCEL NO. 20-2S-16-0166-006

A4. Building Use (e.g., Residential, Non-Residential, Addition, Accessory, etc.) RESIDENTIAL

A5. Latitude/Longitude: Lat. 30DEG 17.781' Long. 82DEG 44.235' Horizontal Datum: ☐ NAD 1927 ☒ NAD 1983

A6. Attach at least 2 photographs of the building if the Certificate is being used to obtain flood insurance.

A7. Building Diagram Number 1

A8. For a building with a crawl space or enclosure(s), provide
- a) Square footage of crawl space or enclosure(s) NA
- b) No. of permanent flood openings in the crawl space or enclosure(s) walls within 1.0 foot above adjacent grade NA
- c) Total net area of flood openings in A8.b NA

A9. For a building with an attached garage, provide:
- a) Square footage of attached garage NA sq ft
- b) No. of permanent flood openings in the attached garage walls within 1.0 foot above adjacent grade NA
- c) Total net area of flood openings in A9.b NA sq in

SECTION B - FLOOD INSURANCE RATE MAP (FIRM) INFORMATION

B1. NFIP Community Name & Community Number	B2. County Name	B3. State
COLUMBIA UNINCORPORATED 120070	COLUMBIA	FL

B4. Map/Panel Number	B5. Suffix	B6. FIRM Index Date	B7. FIRM Panel Effective/Revised Date	B8. Flood Zone(s)	B9. Base Flood Elevation(s) (Zone AO, use base flood depth)
1200700150	B	JAN. 6, 1988	JAN 6, 1988	AE	88

B10. Indicate the source of the Base Flood Elevation (BFE) data or base flood depth entered in Item B9.
☐ FIS Profile ☒ FIRM ☐ Community Determined ☐ Other (Describe) _____

B11. Indicate elevation datum used for BFE in Item B9: ☒ NGVD 1929 ☐ NAVD 1988 ☐ Other (Describe) _____

B12. Is the building located in a Coastal Barrier Resources System (CBRS) area or Otherwise Protected Area (OPA)? ☐ Yes ☒ No
Designation Date _____ ☐ CBRS ☐ OPA

SECTION C - BUILDING ELEVATION INFORMATION (SURVEY REQUIRED)

C1. Building elevations are based on: ☐ Construction Drawings* ☐ Building Under Construction* ☒ Finished Construction
*A new Elevation Certificate will be required when construction of the building is complete.

C2. Elevations – Zones A1-A30, AE, AH, A (with BFE), VE, V1-V30, V (with BFE), AR, AR/A, AR/AE, AR/A1-A30, AR/AH, AR/AO. Complete Items C2.a-g below according to the building diagram specified in Item A7.

Benchmark Utilized LOCAL Vertical Datum NGVD 1929

Conversion/Comments NA

			Check the measurement used.	
a)	Top of bottom floor (including basement, crawl space, or enclosure floor)	93.41	☒ feet	☐ meters (Puerto Rico only)
b)	Top of the next higher floor	NA	☒ feet	☐ meters (Puerto Rico only)
c)	Bottom of the lowest horizontal structural member (V Zones only)	NA	☒ feet	☐ meters (Puerto Rico only)
d)	Attached garage (top of slab)	91.74	☒ feet	☐ meters (Puerto Rico only)
e)	Lowest elevation of machinery or equipment servicing the building (Describe type of equipment in Comments)	91.52	☒ feet	☐ meters (Puerto Rico only)
f)	Lowest adjacent (finished) grade (LAG)	91.1	☒ feet	☐ meters (Puerto Rico only)
g)	Highest adjacent (finished) grade (HAG)	92.3	☒ feet	☐ meters (Puerto Rico only)

SECTION D - SURVEYOR, ENGINEER, OR ARCHITECT CERTIFICATION

This certification is to be signed and sealed by a land surveyor, engineer, or architect authorized by law to certify elevation information. *I certify that the information on this Certificate represents my best efforts to interpret the data available. I understand that any false statement may be punishable by fine or imprisonment under 18 U.S. Code, Section 1001.*

☒ Check here if comments are provided on back of form.

PLACE SEAL HERE

Certifier's Name WILLIAM N. KITCHEN License Number LS 5490

Title PROFESSIONAL SURVEYOR AND MAPPER Company Name WILLIAM N. KITCHEN, P.S.M.

Address 152 N. MARION AVENUE City LAKE CITY State FL ZIP Code 32055

Signature William N. Kitchen Date October 16, 2007 Telephone (386) 755-7786

IMPORTANT: In these spaces, copy the corresponding information from Section A.	For Insurance Company Use:
Building Street Address (including Apt., Unit, Suite, and/or Bldg. No.) or P.O. Route and Box No. 274 NW EVERETT TER.	Policy Number
City WHITE SPRINGS State FL ZIP Code 32096	Company NAIC Number

SECTION D - SURVEYOR, ENGINEER, OR ARCHITECT CERTIFICATION (CONTINUED)

Copy both sides of this Elevation Certificate for (1) community official, (2) insurance agent/company, and (3) building owner.

Comments C2e= AC, GENERATOR, HOT WATER HEATER

Signature William H. Kitchen Date 10-16-2007 ☐ Check here if attachments

SECTION E - BUILDING ELEVATION INFORMATION (SURVEY NOT REQUIRED) FOR ZONE AO AND ZONE A (WITHOUT BFE)

For Zones AO and A (without BFE), complete Items E1-E5. If the Certificate is intended to support a LOMA or LOMR-F request, complete Sections A, B, and C. For Items E1-E4, use natural grade, if available. Check the measurement used. In Puerto Rico only, enter meters.

E1. Provide elevation information for the following and check the appropriate boxes to show whether the elevation is above or below the highest adjacent grade (HAG) and the lowest adjacent grade (LAG).
a) Top of bottom floor (including basement, crawl space, or enclosure) is _ ☒ feet ☐ meters ☒ above or ☐ below the HAG.
b) Top of bottom floor (including basement, crawl space, or enclosure) is._ ☒ feet ☐ meters ☒ above or ☐ below the LAG.

E2. For Building Diagrams 6-8 with permanent flood openings provided in Section A Items 8 and/or 9 (see page 8 of Instructions), the next higher floor (elevation C2.b in the diagrams) of the building is _____ ☐ feet ☐ meters ☐ above or ☐ below the HAG.

E3. Attached garage (top of slab) is_._____ ☐ feet ☐ meters ☐ above or ☐ below the HAG.

E4. Top of platform of machinery and/or equipment servicing the building is _ ☒ feet ☐ meters ☒ above or ☒ below the HAG.

E5. Zone AO only: If no flood depth number is available, is the top of the bottom floor elevated in accordance with the community's floodplain management ordinance? ☐ Yes ☐ No ☐ Unknown. The local official must certify this information in Section G.

SECTION F - PROPERTY OWNER (OR OWNER'S REPRESENTATIVE) CERTIFICATION

The property owner or owner's authorized representative who completes Sections A, B, and E for Zone A (without a FEMA-issued or community-issued BFE) or Zone AO must sign here. *The statements in Sections A, B, and E are correct to the best of my knowledge.*

Property Owner's or Owner's Authorized Representative's Name

Address City State ZIP Code

Signature Date Telephone

Comments

☐ Check here if attachments

SECTION G - COMMUNITY INFORMATION (OPTIONAL)

The local official who is authorized by law or ordinance to administer the community's floodplain management ordinance can complete Sections A, B, C (or E), and G of this Elevation Certificate. Complete the applicable item(s) and sign below. Check the measurement used in Items G8. and G9.

G1. ☐ The information in Section C was taken from other documentation that has been signed and sealed by a licensed surveyor, engineer, or architect who is authorized by law to certify elevation information. (Indicate the source and date of the elevation data in the Comments area below.)

G2. ☐ A community official completed Section E for a building located in Zone A (without a FEMA-issued or community-issued BFE) or Zone AO.

G3. ☐ The following information (Items G4.-G9.) is provided for community floodplain management purposes.

G4. Permit Number	G5. Date Permit Issued	G6. Date Certificate Of Compliance/Occupancy Issued

G7. This permit has been issued for: ☐ New Construction ☐ Substantial Improvement

G8. Elevation of as-built lowest floor (including basement) of the building: _____._____ ☐ feet ☐ meters (PR) Datum _____

G9. BFE or (in Zone AO) depth of flooding at the building site: _____._____ ☐ feet ☐ meters (PR) Datum _____

Local Official's Name Title

Community Name Telephone

Signature Date

Comments

BUILDING DIAGRAMS

The following eight diagrams illustrate various types of buildings. Compare the features of the building being certified with the features shown in the diagrams and select the diagram most applicable. Enter the diagram number in Item A7., the square footage of crawl space or enclosure(s) and the area of flood openings in square inches in Items A8.a-c, the square footage of attached garage and the area of flood openings in square inches in Items A9.a-c, and the elevations in Items C2.a-g.

In A zones, the floor elevation is taken at the top finished surface of the floor indicated; in V zones, the floor elevation is taken at the bottom of the lowest horizontal structural member (see drawing in instructions for Section C).

DIAGRAM 1

All slab-on-grade single- and multiple-floor buildings (other than split-level) and high-rise buildings, either detached or row type (e.g., townhouses); with or without attached garage.

Distinguishing Feature – The bottom floor is at or above ground level (grade) on at least one side. *

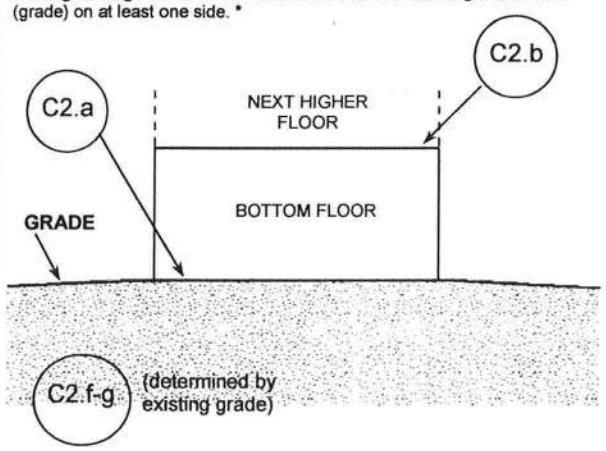


DIAGRAM 2

All single- and multiple-floor buildings with basement (other than split-level) and high-rise buildings with basement, either detached or row type (e.g., townhouses); with or without attached garage.

Distinguishing Feature – The bottom floor (basement or underground garage) is below ground level (grade) on all sides. Buildings constructed above crawl spaces that are below grade on all sides should also use this diagram.*

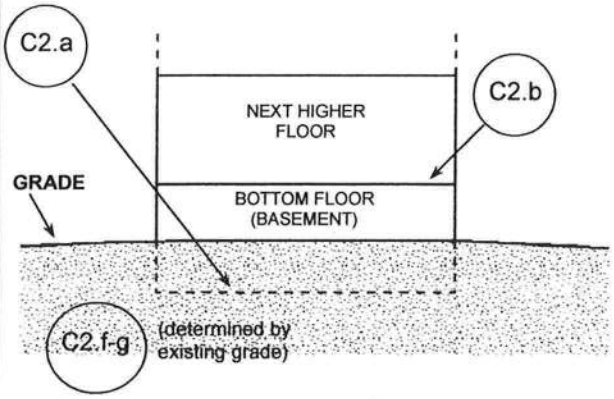


DIAGRAM 3

All split-level buildings that are slab-on-grade, either detached or row type (e.g., townhouses); with or without attached garage.

Distinguishing Feature – The bottom floor (excluding garage) is at or above ground level (grade) on at least one side .*

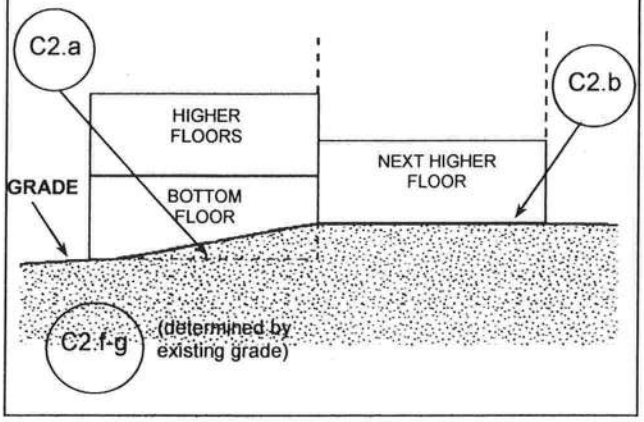
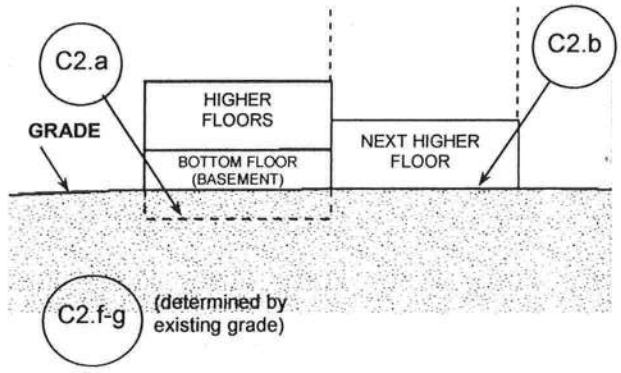


DIAGRAM 4

All split-level buildings (other than slab-on-grade), either detached or row type (e.g., townhouses); with or without attached garage.

Distinguishing Feature – The bottom floor (basement or underground garage) is below ground level (grade) on all sides. Buildings constructed above crawl spaces that are below grade on all sides should also use this diagram. *



* A floor that is below ground level (grade) on all sides is considered a basement even if the floor is used for living purposes, or as an office, garage, workshop, etc.

Columbia County Building Department
Flood Development Permit

Development Permit
F 023- 07-005

DATE 04/02/2007 BUILDING PERMIT NUMBER 000025685

APPLICANT KATIE REED PHONE 752-4072

ADDRESS 2230 SE BAYA DRIVE LAKE CITY FL 32025

OWNER CARL & LYNN WAGONER PHONE 407 423-3694

ADDRESS 274 NW EVERETT TERR WHITE SPRING FL 32096

CONTRACTOR DON REED CONSTRUCTION PHONE 752-4072

ADDRESS 2230 SE BAYA DRIVE LAKE CITY FL 32025

SUBDIVISION LEVINGS UNREC Lot 6 Block ____ Unit ____ Phase ____

TYPE OF DEVELOPMENT SFD,UTILITY PARCEL ID NO. 20-2S-16-01660-006

FLOOD ZONE AE BY BK 1-6-88 FIRM COMMUNITY #. 120070 - PANEL #. 105 B

FIRM 100 YEAR ELEVATION 88' PLAN INCLUDED YES or NO

REQUIRED LOWEST HABITABLE FLOOR ELEVATION 89'

IN THE REGULATORY FLOODWAY YES or (NO) RIVER Suwannee River

SURVEYOR / ENGINEER NAME Mark Disosway LICENSE NUMBER 53915

✓ ONE FOOT RISE CERTIFICATION INCLUDED

_____ ZERO RISE CERTIFICATION INCLUDED

_____ SRWMD PERMIT NUMBER _____
(INCLUDING THE ONE FOOT RISE CERTIFICATION)

DATE THE FINISHED FLOOR ELEVATION CERTIFICATE WAS PROVIDED _____

INSPECTED DATE _____ BY _____

COMMENTS _____

_____

135 NE Hernando Ave., Suite B-21
Lake City, Florida 32055
Phone: 386-758-1008
Fax: 386-758-2160

COLUMBIA COUNTY GATEWAY TO FLORIDA

PERMIT EXPIRES ONE YEAR FROM THE DATE OF ISSUANCE

0703-48

Mark Disosway, P.E.
POB 868, Lake City, FL 32056, Ph 386-754-5419, Fax 386-269-4871

One Foot Rise Analysis and Certification, 100 Year Base Flood

Wagoner, Carl D & Wanda L Residence, 274 NW Everitt Terrace White Springs, Florida 32096, 20-2S-16-01660-006, Columbia Co, FL

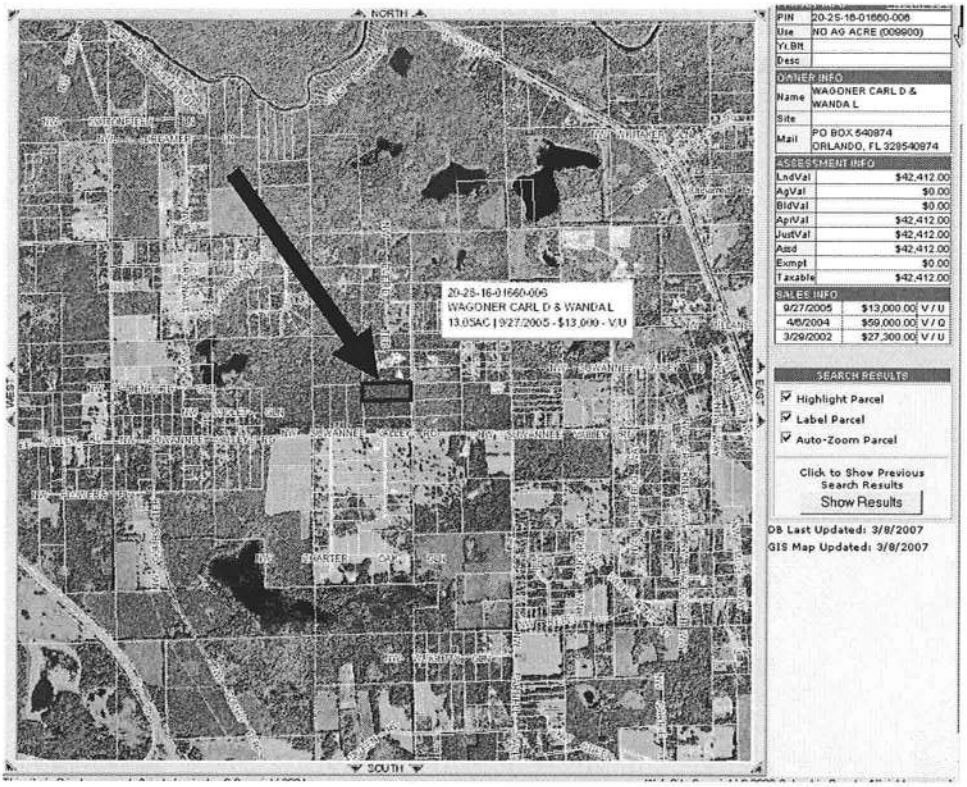
- PROPERTY DESCRIPTION: 274 NW Everitt Terrace White Springs, Florida 32096, 20-2S-16-01660-006, Columbia Co, FL
- OWNER: Wagoner, Carl D & Wanda L
- CONTRACTOR: Don Reed
- PROJECT: A 3963 ft2 addition to existing house on slab on grade stem wall foundation with filled stem wall.
- BASE FLOOD ELEVATION: 88' (Per survey by Wm Kitchen, Dated 20Oct06, Job No.06560.)
- FLOOD ZONE: AE
- BASIN AREA AT BASE FLOOD ELEVATION: 560 Acres (Calculated from FEMA flood plain data.)
- PROPOSED BUILDING AREA: Stem wall filled area 3963 ft2.
- PROPOSED BUILDING VOLUME BELOW FLOODPLAIN: (Slab) 3963 ft2 x 2' = 0 ft3.
- EXISTING GRADE ELEVATION AT BUILDING LOCATION: above 88' for one foot rise calculations. (Note: Existing grade at building location based on survey by Wm Kitchen, Dated 20Oct06, Job No.06560 which includes topo data.)
- CALCULATIONS: The project only requires volume calculations in this area since it is not a flowing or riverine area.

Floodplain volume removed = 0 ft3

Floodplain level increase = (0 ft3) / 43560 ft^2/acre / 560 acres = 0.0000 ft

CERTIFICATION:
I hereby certify that construction of Harold L. and Wagoner, Carl D & Wanda L Residence, 274 NW Everett Terrace White Springs, Florida 32096, 20-2S-16-01660-006, Columbia Co, FL will increase flood elevations less than one foot at the project location, to the best of my knowledge.

30MAR07



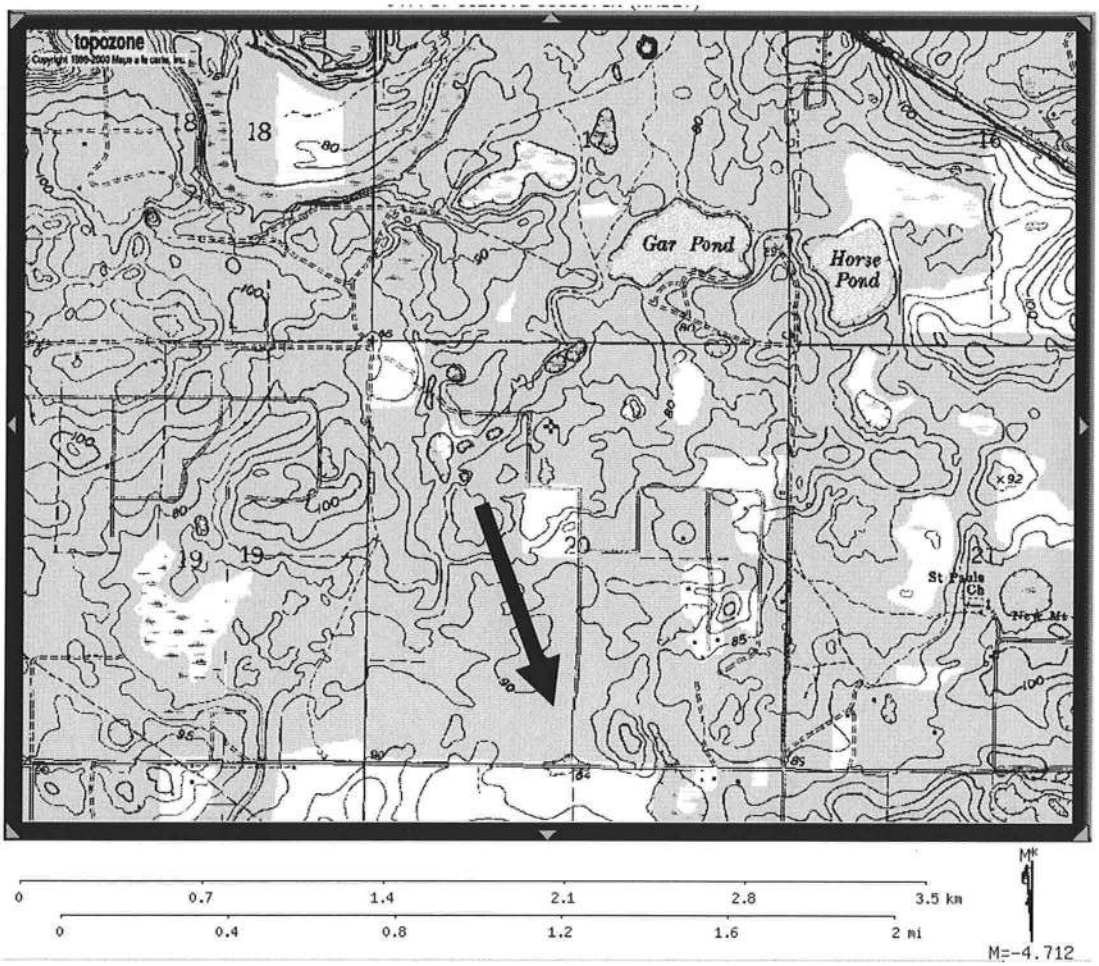
PIN	20-2S-18-01660-006
Use	NO AG ACRE (009900)
YLBN	
Desc	
OWNER INFO	
Name	WAGONER CARL D & WANDA L
Site	
Mail	PO BOX 540874 ORLANDO, FL 328540874
ASSESSMENT INFO	
LndVal	$42,412.00
AgVal	$0.00
BldVal	$0.00
ApVal	$42,412.00
JustVal	$42,412.00
Assd	$42,412.00
Exmpt	$0.00
Taxable	$42,412.00
SALES INFO	
9/27/2005	$13,000.00 V / U
4/6/2004	$59,000.00 V / Q
3/29/2002	$27,300.00 V / U

SEARCH RESULTS

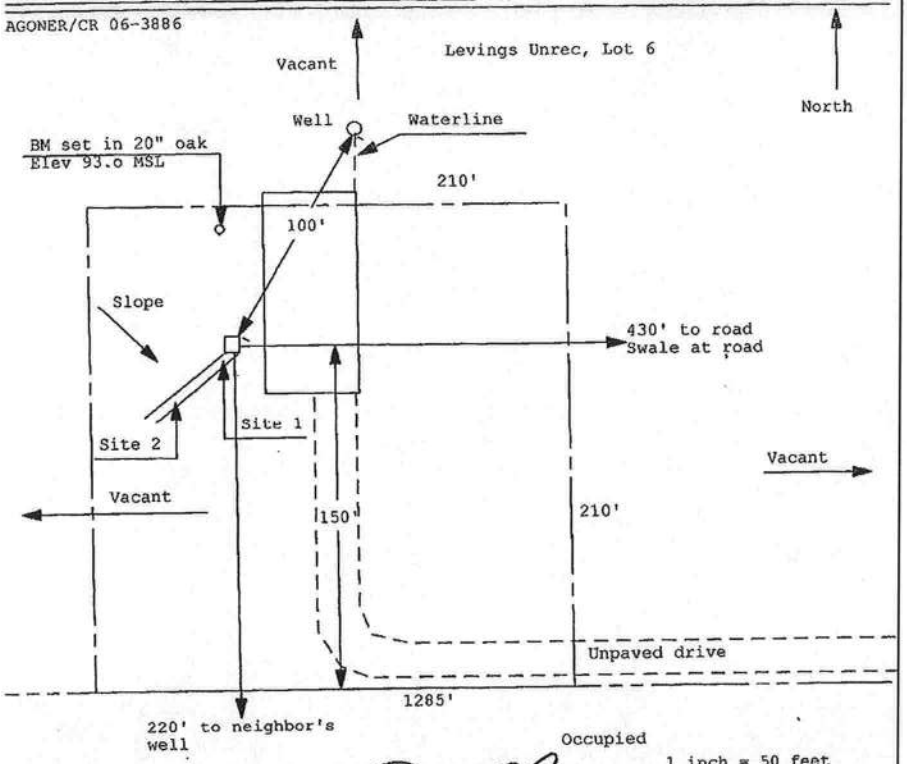
☑ Highlight Parcel
☑ Label Parcel
☑ Auto-Zoom Parcel

Click to Show Previous Search Results
Show Results

DB Last Updated: 3/8/2007
GIS Map Updated: 3/8/2007



ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

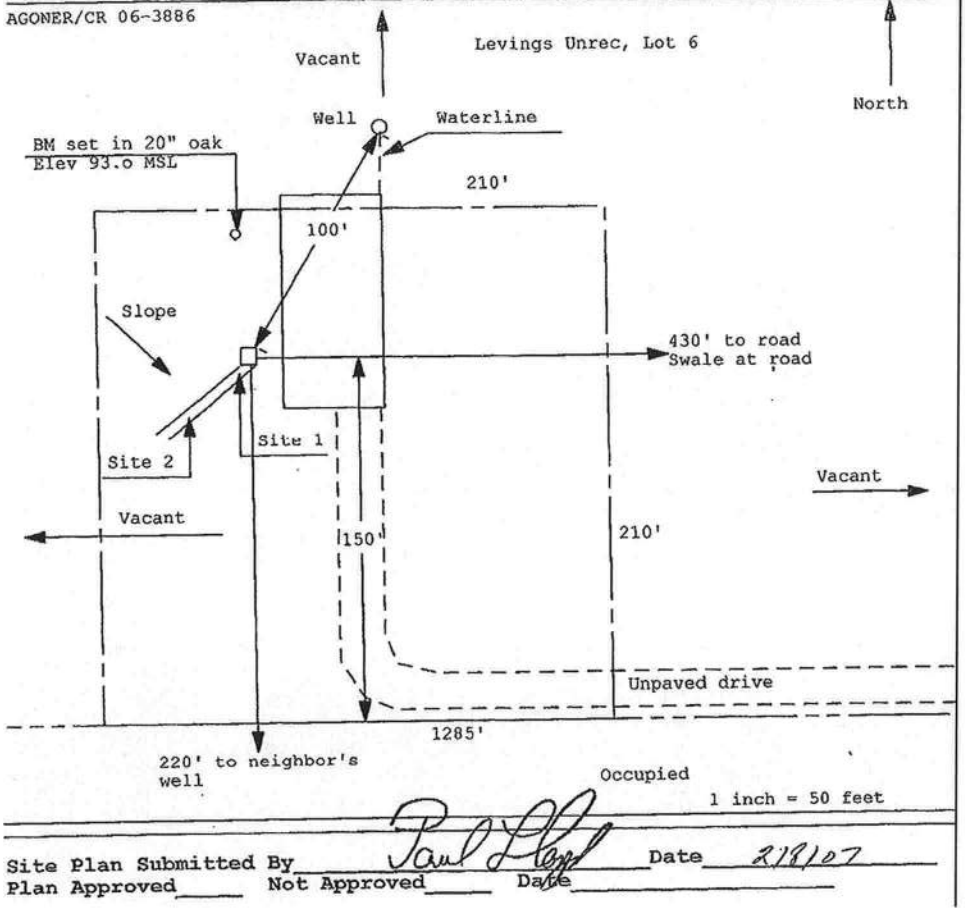


Site Plan Submitted By Paul Lloyd Date 2/8/07

Plan Approved____ Not Approved____ Date____

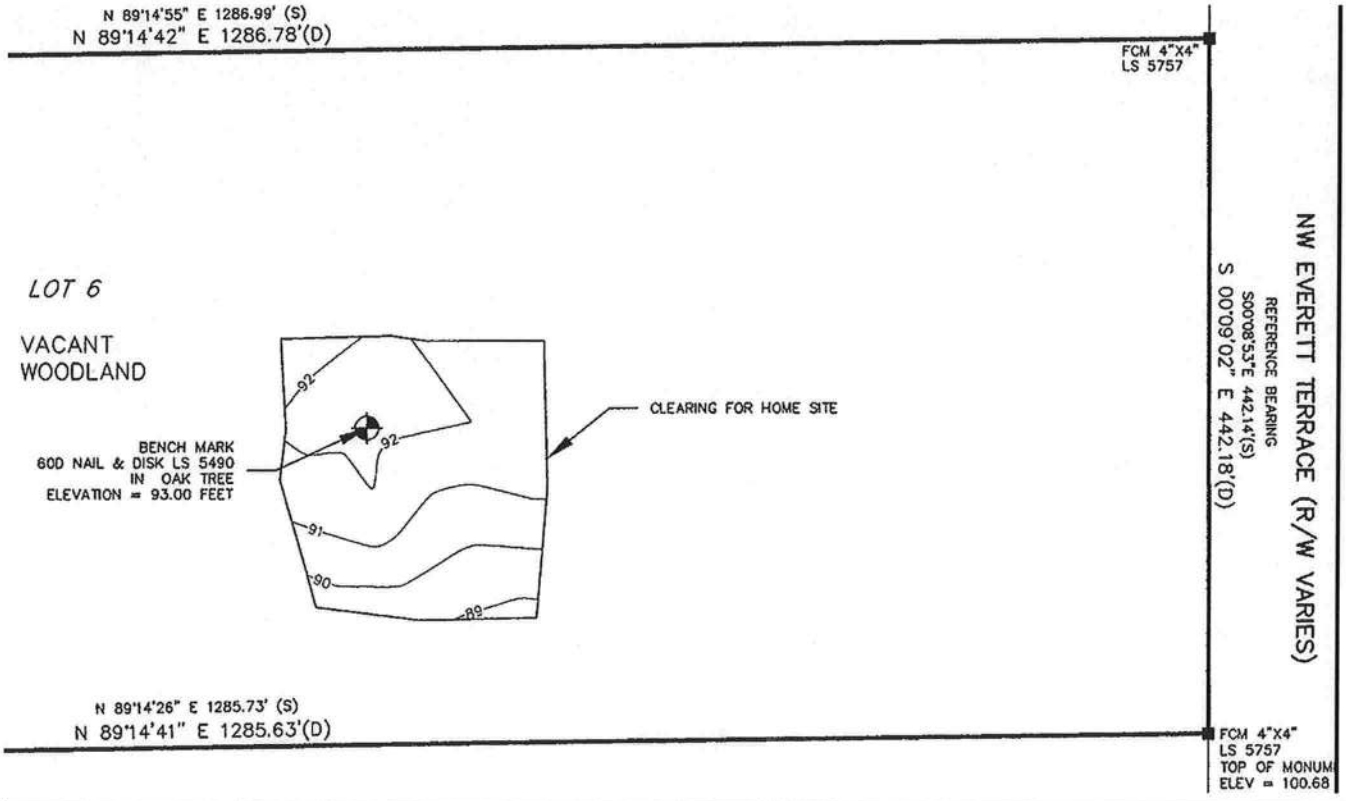
ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

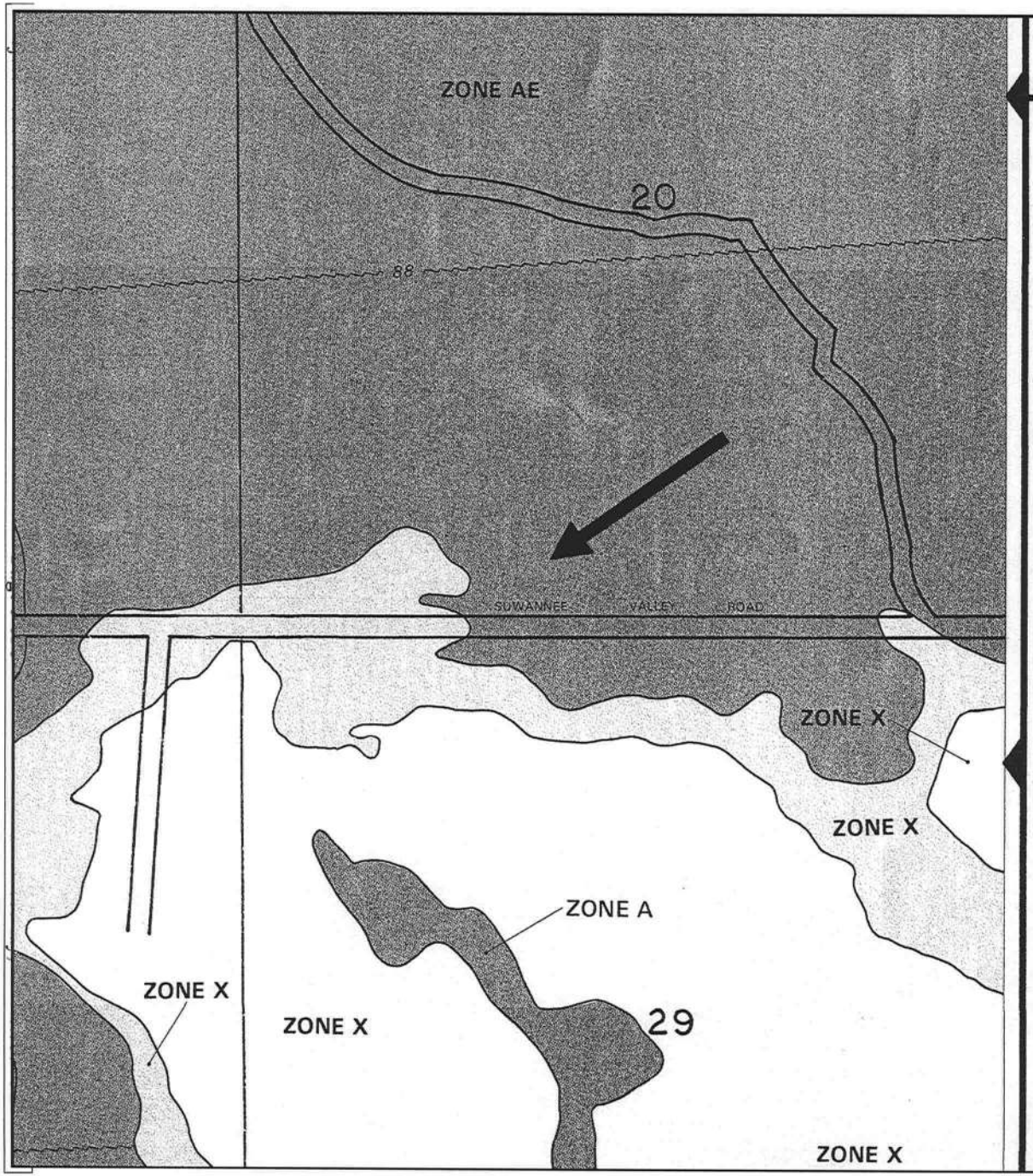
AGONER/CR 06-3886



Site Plan Submitted By Paul Lloyd Date 2/8/07

Plan Approved____ Not Approved____ Date_____





N

APPROXIMATE SCALE IN FEET

1000 0 1000

NATIONAL FLOOD INSURANCE PROGRAM

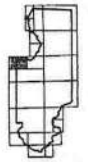
FIRM

FLOOD INSURANCE RATE MAP

COLUMBIA COUNTY, FLORIDA
(UNINCORPORATED AREAS)

PANEL 105 OF 290

PANEL LOCATION



COMMUNITY-PANEL NUMBER
120070 0105 B

EFFECTIVE DATE:
JANUARY 6, 1988

Federal Emergency Management Agency

THIS INSTRUMENT WAS PREPARED BY:
FIRST FEDERAL SAVINGS BANK OF FLORIDA
4705 WEST U.S. HIGHWAY 90
P.O. BOX 2029
LAKE CITY, FLORIDA 32056

Inst:2007004651 Date:02/26/2007 Time:17:00
DC,P.DeWitt Cason,Columbia County B:1111 P:2610

PERMIT NO._____ TAX FOLIO NO._____

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF Columbia

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: SEE EXHIBIT "A" ATTACHED HERETO AND MADE A PART HEREOF.

2. General description of improvement: **Construction of Dwelling**

3. Owner information:
   a. Name and address: CARL D. WAGONER & WANDA L. WAGONER, Husband & Wife 1400 Clouser Ave., Orlando, FL 32804

   b. Interest in property: **Fee Simple**

   c. Name and address of fee simple title holder (if other than Owner): **NONE**

4. Contractor (name and address): Don Reed Construction, Inc. 2230 SW Baya Drive, Lake City, FL 32025

5. Surety:
   a. Name and address: _____

   b. Amount of bond: _____

6. Lender: **FIRST FEDERAL SAVINGS BANK OF FLORIDA**
   4705 WEST U.S. HIGHWAY 90
   P. O. BOX 2029
   LAKE CITY, FLORIDA 32056

7. Persons within the State of Florida designated by Owner upon whom notices or other document may be served as provided by Section 713.13 (1) (a) 7., Florida Statutes: NONE

8. In addition to himself, Owner designates **PAULA HACKER of FIRST FEDERAL SAVINGS BANK OF FLORIDA, 4705 West U.S. Highway 90 / P. O. Box 2029, Lake City, Florida 32056** to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

Carl D. Wagoner
Borrower Name

Wanda L. Wagoner
Co-Borrower Name

The foregoing instrument was acknowledged before me this 20th day of February, 2007 20 07 by CARL D. & WANDA L. WAGONER, who is personally known to me or who has produced driver's license for identification.

Notary Public
My Commission Expires:

TERRY MCDAVID
MY COMMISSION # DD 500788
EXPIRES: January 16, 2010
Bonded Thru Notary Public Underwriters

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Sharon Teagle
Deputy Clerk
Date 02-26-2007

CLERK CIRCUIT COURT
COLUMBIA COUNTY, FLORIDA

Inst:2007004651 Date:02/26/2007 Time:17:00
_____DC,P.DeWitt Cason,Columbia County B:1111 P:2611

EXHIBIT "A"

A part of the SW 1/4 of Section 20, Township 2 South, Range 16 East, Columbia County, Florida, more particularly described as follows: Commence at the SE Corner of the SE 1/4 of SW 1/4 of said Section 20 and run N 00 deg. 18'00" W, a distance of 908.53 feet to the Point of Beginning, thence continue N 00 deg. 18'00" W, a distance of 442.18 feet, thence N 89 deg. 14'42" E, a distance of 1286.78 feet to the West maintained right of way line of Everett Lane, thence S 00 deg. 09'02" E, along said maintained right of way line, a distance of 442.18 feet; thence S 89 deg. 14'41" W, a distance of 1285.63 feet to the Point of Beginning.

Inst:2005027879 Date:11/08/2005 Time:12:53
Doc Stamp-Deed : 91.00
MK DC,P.DeWitt Cason,Columbia County B:1064 P:1277

Prepared by and return to:
Marie E. Henkel
Attorney at Law
M.E. Henkel, P.A.
3560 S. Magnolia Ave.
Orlando, FL 32806
407-438-6738
File Number: 05-283
Will Call No.:

[Space Above This Line For Recording Data]

Warranty Deed

This Warranty Deed made this 27 day of September, 2005 between **Cari D. Wagoner and Wanda L. Wagoner, husband and wife,** whose post office address is P.O. Box 540874, Orlando, FL 32854, grantor, and **Harold L. Bass and Barbara R. Bass, husband and wife** whose post office address is 3430 Henry J Ave., Saint Cloud, FL 34772, grantee:

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth, that said grantor, for and in consideration of the sum of TEN AND NO/100 DOLLARS ($10.00) and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained, and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in **Columbia County, Florida** to-wit:

An undivided one-half interest in and to:
Tract #5: **A PART OF THE SW1/4 OF SECTION 20, TOWNSHIP 2 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA MORE PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT THE SE CORNER OF THE SW 1/4 OF SW 1/4 OF SAID SECTION 20, AND RUN N00° 18'00"W A DISTANCE OF 465.65 FEET TO THE POINT OF BEGINNING, THENCE CONTINUE N00° 18'00"W, A DISTANCE OF 442.88 FEET, THENCE N89° 14'41"E A DISTANCE OF 1285.63 FEET TO THE WEST MAINTAINED RIGHT OF WAY LINE OF EVERETT LANE, THENCE S00° 09'02"E, ALONG SAID MAINTAINED RIGHT OF WAY LINE, A DISTANCE OF 442.88 FEET, THENCE S89° 14'39"W A DISTANCE OF 1284.47 FEET TO THE POINT OF BEGINNING.**

Parcel Identification Number: 20-2S-[redacted]

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to **December 31, 2004**.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Inst:2005027879 Date:11/08/2005 Time:12:53
Doc Stamp-Deed : 91.00
DC,P.DeWitt Cason,Columbia County B:1064 P:1278

Signed, sealed and delivered in our presence:

Witness Name: Janet M Jansen

Witness Name: Debbie Jaramillo

Witness Name: Janet M Jansen

Witness Name: Debbie Jaramillo

Carl D. Wagoner (Seal)
FL DL [redacted]

Wanda L. Wagoner
FL DL [redacted]

State of Florida
County of Orange Osceola

The foregoing instrument was acknowledged before me this 27 day of September, 2005 by Carl D. Wagoner and Wanda L. Wagoner, who [ ] are personally known or [X] have produced a driver's license as identification.

[Notary Seal]

Ranjena K-Pereira
My Commission DD083807
Expires January 13, 2006

Notary Public

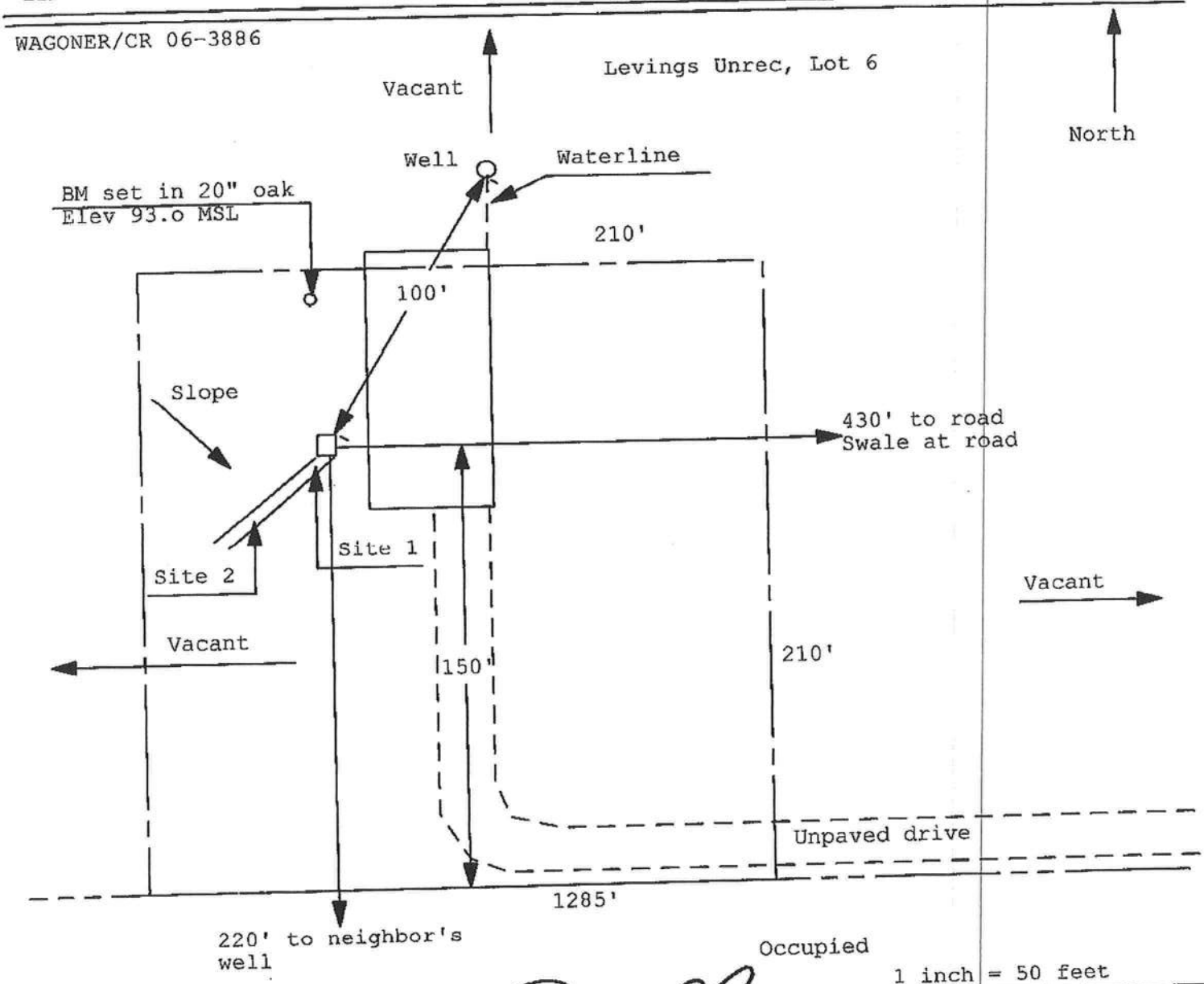
Printed Name: RANJANA K-PEREIRA

My Commission Expires: January 13, 2006

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 07-00180N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



Site Plan Submitted By Paul Lloyd Date 2/8/07

Plan Approved ✓ Not Approved ____ Date ____

By Sallie Shaddox ES II 31407 CPHU

Notes: ____

Columbia CHD

FORM 600A-2004 EnergyGauge® 4.0

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Wagoner Residence**	Builder:	**Don Reed**
Address:	**Lot: 6, Sub: Everet Lane, Plat:**	Permitting Office:	Columbia
City, State:	**Lake City, FL**	Permit Number:	25685
Owner:	**Lynn Wagoner**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 42.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes		
6. Conditioned floor area (ft²)	2773 ft²	c. N/A	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor:	Description Area	13. Heating systems	
(or Single or Double DEFAULT)	7a. (Dble Default) 178.5 ft²	a. Electric Heat Pump	Cap: 42.0 kBtu/hr
b. SHGC:			HSPF: 7.20
(or Clear or Tint DEFAULT)	7b. (Clear) 178.5 ft²	b. N/A	
8. Floor types			
a. Slab-On-Grade Edge Insulation	R=0.0, 228.0(p) ft	c. N/A	
b. N/A			
c. N/A		14. Hot water systems	
9. Wall types		a. Electric Resistance	Cap: 50.0 gallons
a. Concrete, Int Insul, Exterior	R=8.0, 1710.0 ft²		EF: 0.92
b. Frame, Wood, Adjacent	R=13.0, 342.0 ft²	b. N/A	
c. N/A			
d. N/A		c. Conservation credits	
e. N/A		(HR-Heat recovery, Solar DHP-Dedicated heat pump)	
10. Ceiling types		15. HVAC credits	
a. Under Attic	R=30.0, 2773.0 ft²	(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 124.0 ft		
b. N/A			

Glass/Floor Area: 0.11

Total as-built points: 32007
Total base points: 37369

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 11-16-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Don Reed

DATE: 11-18-06

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 6, Sub: Everet Lane, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	2773.0	20.04	10002.8	Double, Clear	S	9.0	8.0	144.0	35.87	0.50	2596.4
				Double, Clear	W	1.5	2.0	9.0	38.52	0.60	208.9
				Double, Clear	W	1.5	7.0	36.0	38.52	0.94	1302.2
				Double, Clear	E	1.5	7.0	18.0	42.06	0.94	710.5
				Double, Clear	N	1.5	5.0	24.0	19.20	0.92	421.9
				Double, Clear	N	1.5	7.0	36.0	19.20	0.96	660.1
				Double, Clear	N	11.0	7.0	36.0	19.20	0.65	447.8
				As-Built Total:				**303.0**			**6347.7**
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Adjacent	342.0	0.70	239.4	Concrete, Int Insul, Exterior			8.0	1710.0	0.63		1068.8
Exterior	1710.0	1.70	2907.0	Frame, Wood, Adjacent			13.0	342.0	0.60		205.2
Base Total:	**2052.0**		**3146.4**	**As-Built Total:**				**2052.0**			**1273.9**
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points
Adjacent	20.0	2.40	48.0	Exterior Insulated				20.0	4.10		82.0
Exterior	60.0	6.10	366.0	Adjacent Insulated				20.0	1.60		32.0
				Exterior Insulated				40.0	4.10		164.0
Base Total:	**80.0**		**414.0**	**As-Built Total:**				**80.0**			**278.0**
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points
Under Attic	2773.0	1.73	4797.3	Under Attic			30.0	2773.0	1.73 X 1.00		4797.3
Base Total:	**2773.0**		**4797.3**	**As-Built Total:**				**2773.0**			**4797.3**
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Slab	228.0(p)	-37.0	-8436.0	Slab-On-Grade Edge Insulation			0.0	228.0(p	-41.20		-9393.6
Raised	0.0	0.00	0.0								
Base Total:			**-8436.0**	**As-Built Total:**				**228.0**			**-9393.6**
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points
	2773.0	10.21	28312.3					2773.0	10.21		28312.3

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 6, Sub: Everet Lane, Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 38236.8			**Summer As-Built Points:**					**31615.7**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 42000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)					
38236.8	**0.4266**	**16311.8**	31616 **31615.7**	1.00 **1.00**	(1.09 x 1.147 x 0.91) **1.138**	0.263 **0.263**	1.000 **1.000**	9443.4 **9443.4**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 6, Sub: Everet Lane, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	2773.0	12.74	6359.0	Double, Clear	S	9.0	8.0	144.0	13.30	2.94	5620.4	
				Double, Clear	W	1.5	2.0	9.0	20.73	1.13	211.7	
				Double, Clear	W	1.5	7.0	36.0	20.73	1.02	758.5	
				Double, Clear	E	1.5	7.0	18.0	18.79	1.03	347.3	
				Double, Clear	N	1.5	5.0	24.0	24.58	1.00	592.1	
				Double, Clear	N	1.5	7.0	36.0	24.58	1.00	886.1	
				Double, Clear	N	11.0	7.0	36.0	24.58	1.02	905.3	
				As-Built Total:				**303.0**			**9321.4**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	342.0	3.60	1231.2	Concrete, Int Insul, Exterior			8.0	1710.0	4.20		7182.0	
Exterior	1710.0	3.70	6327.0	Frame, Wood, Adjacent			13.0	342.0	3.30		1128.6	
Base Total:	**2052.0**		**7558.2**	**As-Built Total:**				**2052.0**			**8310.6**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	20.0	11.50	230.0	Exterior Insulated				20.0	8.40		168.0	
Exterior	60.0	12.30	738.0	Adjacent Insulated				20.0	8.00		160.0	
				Exterior Insulated				40.0	8.40		336.0	
Base Total:	**80.0**		**968.0**	**As-Built Total:**				**80.0**			**664.0**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	2773.0	2.05	5684.6	Under Attic			30.0	2773.0	2.05 X	1.00	5684.6	
Base Total:	**2773.0**		**5684.6**	**As-Built Total:**				**2773.0**			**5684.6**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	228.0(p)	8.9	2029.2	Slab-On-Grade Edge Insulation			0.0	228.0(p	18.80		4286.4	
Raised	0.0	0.00	0.0									
Base Total:			**2029.2**	**As-Built Total:**				**228.0**			**4286.4**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	2773.0	-0.59	-1636.1					2773.0	-0.59		-1636.1	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 6, Sub: Everet Lane, Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**20963.0**	**Winter As-Built Points:**					**26630.9**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 42000 btuh ,EFF(7.2) Ducts:Unc(S),Unc(R),Int(AH),R6.0					
20963.0	0.6274	13152.2	26630.9	1.00	1.162	0.474	1.000	14658.3

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 6, Sub: Everet Lane, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2635.00	7905.0	50.0	0.92	3		1.00	2635.00	1.00	7905.0
				As-Built Total:							**7905.0**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
16312	**13152**	**7905**	**37369**	**9443**	**14658**	**7905**	**32007**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 6, Sub: Everet Lane, Plat: , Lake City, FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.0

The higher the score, the more efficient the home.

Lynn Wagoner, Lot: 6, Sub: Everet Lane, Plat: , Lake City, FL,

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	Yes	
6.	Conditioned floor area (ft²)	2773 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT)	7a. Description (Dble Default)	Area 178.5 ft²
b.	SHGC: (or Clear or Tint DEFAULT)	7b. (Clear)	178.5 ft²
8.	Floor types		
a.	Slab-On-Grade Edge Insulation	R=0.0, 228.0(p) ft	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Concrete, Int Insul, Exterior	R=8.0, 1710.0 ft²	
b.	Frame, Wood, Adjacent	R=13.0, 342.0 ft²	
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 2773.0 ft²	
b.	N/A		
c.	N/A		
11.	Ducts		
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 124.0 ft	
b.	N/A		

12.	Cooling systems		
a.	Central Unit	Cap: 42.0 kBtu/hr	
		SEER: 13.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 42.0 kBtu/hr	
		HSPF: 7.20	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 50.0 gallons	
		EF: 0.92	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Residential System Sizing Calculation

Summary

Lynn Wagoner

Lake City, FL

Project Title:
Wagoner Residence

Code Only
Professional Version
Climate: North

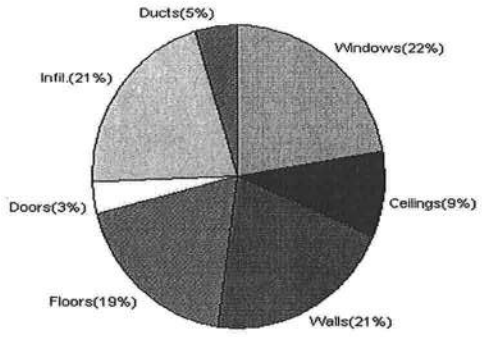
11/16/2006

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**38345**	**Btuh**	**Total cooling load calculation**	**36457**	**Btuh**
Submitted heating capacity	42000	Btuh	Submitted cooling capacity	42000	Btuh
Submitted as % of calculated	109.5	%	Submitted as % of calculated	115.2	%

WINTER CALCULATIONS

Winter Heating Load (for 2773 sqft)

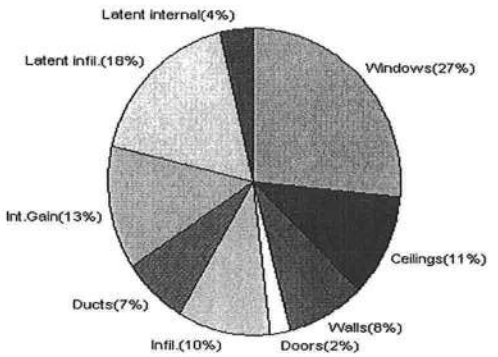
Load component			Load	
Window total	303	sqft	8575	Btuh
Wall total	2052	sqft	7900	Btuh
Door total	80	sqft	1288	Btuh
Ceiling total	2773	sqft	3605	Btuh
Floor total	228	ft	7205	Btuh
Infiltration	185	cfm	7947	Btuh
Subtotal			**36519**	**Btuh**
Duct loss			1826	Btuh
TOTAL HEAT LOSS			**38345**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 2773 sqft)

Load component			Load	
Window total	303	sqft	9774	Btuh
Wall total	2052	sqft	3057	Btuh
Door total	80	sqft	811	Btuh
Ceiling total	2773	sqft	3938	Btuh
Floor total			0	Btuh
Infiltration	185	cfm	3668	Btuh
Internal gain			4800	Btuh
Subtotal(sensible)			**26048**	**Btuh**
Duct gain			2605	Btuh
Total sensible gain			**28653**	**Btuh**
Latent gain(infiltration)			6424	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**7804**	**Btuh**
TOTAL HEAT GAIN			**36457**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.

PREPARED BY: _____

DATE: 11-16-06

System Sizing Calculations - Winter

Residential Load - Component Details

Lynn Wagoner

Lake City, FL

Project Title:
Wagoner Residence

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

11/16/2006

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	144.0	28.3	4075 Btuh
2	2, Clear, Metal, DEF	E	9.0	28.3	255 Btuh
3	2, Clear, Metal, DEF	E	36.0	28.3	1019 Btuh
4	2, Clear, Metal, DEF	W	18.0	28.3	509 Btuh
5	2, Clear, Metal, DEF	S	24.0	28.3	679 Btuh
6	2, Clear, Metal, DEF	S	36.0	28.3	1019 Btuh
7	2, Clear, Metal, DEF	S	36.0	28.3	1019 Btuh
	Window Total		303		8575 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Concrete - Exterior	8.0	1710	4.3	7353 Btuh
2	Frame - Adjacent	13.0	342	1.6	547 Btuh
	Wall Total		2052		7900 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exter		20	18.3	367 Btuh
2	Insulated - Adjac		20	9.4	188 Btuh
3	Insulated - Exter		40	18.3	733 Btuh
	Door Total		80		1288Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	2773	1.3	3605 Btuh
	Ceiling Total		2773		3605Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	228.0 ft(p)	31.6	7205 Btuh
	Floor Total		228		7205 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	27730(sqft)	185	7947 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			185	7947 Btuh

Totals for Heating	**Subtotal**	**36519 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1826 Btuh**
	Total Btuh Loss	**38345 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

Lynn Wagoner

Lake City, FL

Project Title:
Wagoner Residence

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F

11/16/2006

Window	Type Panes/SHGC/U/InSh/ExSh Ornt		Overhang Len	 Hgt	Window Area(sqft) Gross	 Shaded	 Unshaded	HTM Shaded	 Unshaded	Load	
1	2, Clear, DEF, N, N	N	9	8	144.0	0.0	144.0	22	22	3168	Btuh
2	2, Clear, DEF, N, N	E	1.5	2	9.0	2.2	6.8	22	72	536	Btuh
3	2, Clear, DEF, N, N	E	1.5	7	36.0	1.5	34.5	22	72	2518	Btuh
4	2, Clear, DEF, N, N	W	1.5	7	18.0	0.7	17.3	22	72	1259	Btuh
5	2, Clear, DEF, N, N	S	1.5	5	24.0	12.0	12.0	22	37	708	Btuh
6	2, Clear, DEF, N, N	S	1.5	7	36.0	36.0	0.0	22	37	792	Btuh
7	2, Clear, DEF, N, N	S	11	7	36.0	36.0	0.0	22	37	792	Btuh
	Window Total				303					9774	Btuh

Walls	Type	R-Value	Area	HTM	Load	
1	Concrete - Exterior	8.0	1710.0	1.6	2702	Btuh
2	Frame - Adjacent	13.0	342.0	1.0	356	Btuh
	Wall Total		2052.0		3057	Btuh

Doors	Type	Area	HTM	Load	
1	Insulated - Exter	20.0	10.1	203	Btuh
2	Insulated - Adjac	20.0	10.1	203	Btuh
3	Insulated - Exter	40.0	10.1	406	Btuh
	Door Total	80.0		811	Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load	
1	Under Attic/Dark	30.0	2773.0	1.4	3938	Btuh
	Ceiling Total		2773.0		3938	Btuh

Floors	Type	R-Value	Size	HTM	Load	
1	Slab-On-Grade Edge Insulation	0.0	228.0 ft(p)	0.0	0	Btuh
	Floor Total		228.0		0	Btuh

Infiltration	Type	ACH	Volume	CFM=	Load	
	Natural	0.40	27730	185.2	3668	Btuh
	Mechanical			0	0	Btuh
	Infiltration Total			185	3668	Btuh

Internal gain		Occupants		Btuh/occupant		Appliance	Load	
		6	X	300	+	3000	4800	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Lynn Wagoner

Lake City, FL

Project Title:
Wagoner Residence

Code Only
Professional Version
Climate: North

11/16/2006

Totals for Cooling	**Subtotal**	**26048 Btuh**
	Duct gain(using duct multiplier of 0.10)	**2605 Btuh**
	Total sensible gain	**28653 Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**6424 Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380 Btuh**
	Latent other gain	**0 Btuh**
	TOTAL GAIN	**36457 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC

SERIES/MODEL: 420/430/440
PRODUCT TYPE: Aluminum Sliding Glass Door

	Summary of Results		
Title	**Test Specimen #1**	**Test Specimen #2**	**Test Specimen #3**
Rating	SGD-R25 182 x 96	SGD-R35 182 x 80	SGD-R40 144 x 96
Operating Force	17 lbf max.	17 lbf max.	N/A
Air Infiltration	0.23 cfm/ft^2	0.27 cfm/ft^2	N/A
Water Resistance Test Pressure	3.75/6.0/9.0 psf	6.0 psf	N/A
Uniform Load Deflection Test Pressure	±35.0 psf	±35.0 psf	+40.0 psf/-40.1 psf
Uniform Load Structural Test Pressure	±37.5 psf	±52.5 psf	+60.0 psf/-60.2 psf
Forced Entry Resistance	Grade 10	Grade 10	N/A

Reference should be made to ATI Report No. 52112.01-122-47 for complete test specimen description and data.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No.:	52112.01-122-47
Revision 1:	09/13/04
Test Dates:	06/30/04
Through:	08/12/04
Report Date:	08/30/04
Expiration Date:	07/02/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on three Series/Model 420/430/440, aluminum sliding glass doors at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: SGD-R25 182 x 96; Test Specimen #2: SGD-R35 182 x 80; Test Specimen #3: SGD-R40 144 x 96. Test specimen description and results are reported herein.

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 420/430/440

Product Type: Aluminum Sliding Glass Door

Test Specimen #1: SGD-R25 182 x 96 (XXO)

Overall Size: 15' 1-3/4" wide by 8' 0" high

Active Door Panel Size (2): 5' 0-1/2" wide by 7' 11" high

Fixed Door Panel Size: 5' 1" wide by 7' 11" high

Screen Size: 5' 0-3/8" wide by 7' 11" high

Overall Area: 121.2 ft^2

Reinforcement: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520).

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Test Specimen Description: (Continued)

Test Specimen #2: SGD-R35 182 x 80 (OXX)

Overall Size: 15' 1-3/4" wide by 6' 8" high

Active Door Panel Size (2): 5' 0-1/2" wide by 6' 7" high

Fixed Door Panel Size: 4' 8-7/8" wide by 6' 2-5/8" high

Screen Size: 5' 0-3/8" wide by 6' 7" high

Overall Area: 101 ft^2

Reinforcement: No reinforcement was utilized.

Test Specimen #3: SGD-R40 144 x 96 (XOX)

Overall Size: 12' 0" wide by 8' 0" high

Active Door Panel Size: 3' 8-1/4" wide by 7' 10-1/2" high

Fixed Door Panel Size (2): 3' 8-3/4" wide by 7' 6-1/2" high

Screen Size: 3' 11-1/2" wide by 7' 11-3/8" high

Overall Area: 96 ft^2

Reinforcement: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520). The interlock utilized an aluminum reinforcement (Drawing #SECT4237).

The following descriptions apply to all specimens.

Finish: All aluminum was white.

Glazing Details: All glazing consisted of a single sheet of 3/16" thick clear tempered glass that was channel glazed with a wrap around rubber gasket.

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.187" backed by 0.270" high polypile with center fin	2 Rows	Stiles

Frame Construction: The frame was constructed of extruded aluminum. Corners were coped, butted, sealed, and fastened with two #8 by 5/8" screws.

Door Panel Construction: The door panels were constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" by 3/4" screw at the bottom and two #8 by 3/4" screws at the top.

Screen Construction: The screen was constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" by 3/4" and one #8 by 1" screw at the bottom and one #8 by 1" screw at the top.

Hardware:

Description	Quantity	Location
Locking handle	1	44" from active panel bottom
Roller assembly	2	3" from bottom rail ends
Screen locking handle	1	46" from screen bottom rail

Drainage:

Description	Quantity	Location
Sloped sill	1	Sill

Installation: The units were installed into a #2 Spruce-Pine-Fir wood test buck. The units were fastened to the test buck with two rows of #8 by 1-1/4" screws, 8" from each end and 23" on center. The exterior perimeter was sealed with silicone.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO)			
2.2.1.6.1	Operating Force	17 lbf	20 lbf max.
	Breakaway force	24 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283		
	1.57 psf (25 mph)	0.23 cfm/ft^2	0.3 cfm/ft^2 max.

***Note #1**: The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547		
	(with and without screen)		
	2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330		
	(Deflections reported were taken on the meeting rail)		
	(Loads were held for 52 seconds)		
	15.0 psf (positive)	0.56"	See Note #2
	15.0 psf (negative)	0.57"	See Note #2

***Note #2**: The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

2.1.4.2	Uniform Load Structural per ASTM E 330		
	(Permanent sets reported were taken on the meeting stile)		
	(Loads were held for 10 seconds)		
	22.5 psf (positive)	0.02"	0.30" max.
	22.5 psf (negative)	0.03"	0.30" max.
2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction - 70 lbs		
	Locking stile	0.12"/24%	0.50"/100%
	Interlock stile	0.12"/24%	0.50"/100%

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO) (Continued)			
2.2.1.6.2	Deglazing Test per ASTM E 987 In remaining direction - 50 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 842		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A6	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) 3.75 psf	No leakage	No leakage
4.3	Water Resistance per ASTM E 547 (with and without screen) (with sill riser) 6.0 psf	No leakage	No leakage
4.3	Water Resistance per ASTM E 547 (with and without screen) (with 2-5/8" Dade County sill extension) 9.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	35.0 psf (positive)	2.98"	See Note #2
	35.0 psf (negative)	2.52"	See Note #2

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO) (Continued)			
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	37.5 psf (positive)	0.20"	0.36" max.
	37.5 psf (negative)	0.19"	0.36" max.
Test Specimen #2: SGD-R35 182 x 80 (OXX)			
2.2.1.6.1	Operating Force	17 lbf	20 lbf max.
	Breakaway force	21 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283		
	1.57 psf (25 mph)	0.27 cfm/ft^2	0.3 cfm/ft^2 max.
	Note #1: *The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*		
2.1.3	Water Resistance per ASTM E 547 (with and without screen)		
	2.86 psf	No leakage	No leakage
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Locking stile	0.12"/24%	0.50"/100%
	Interlock stile	0.12"/24%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 842		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A6	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #2: SGD-R35 182 x 80 (OXX) (Continued)			
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) (with sill riser) 6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	35.0 psf (positive)	1.28"	See Note #2
	35.0 psf (negative)	1.33"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	52.5 psf (positive)	0.13"	0.30" max.
	52.5 psf (negative)	0.15"	0.30" max.
Test Specimen #3: SGD-R40 144 x 96 (XOX)			
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	40.0 psf (positive)	1.42"	See Note #2
	40.1 psf (negative)	1.28"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	60.0 psf (positive)	0.27"	0.37" max.
	60.2 psf (negative)	0.30"	0.37" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess vlm

Digitally Signed for: Mark A. Hess by Vicki L. McElwain

Mark Hess
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

MH:vlm

MI WINDOWS AND DOORS, INC.

420 / 430 / 440 SERIES ALUMINUM SLIDING GLASS DOOR

COMAPARATIVE ANALYSIS CHART IN DESIGN PRESSURE

STEEL AND ALUMINUM REINFORCING

09/08/2004

SGD ALUM & STL REINF

PANEL WIDTH >>	24	30	36	48
PANEL HEIGHT 80	85	71	62	51
96	69	57	49	40

TEST REPORT NO: ATI-52112.01-122-47
DESIGN PRESSURE ACHIEVED IN TEST: POS. & NEG. 40.0 PSF
WATER TEST PRESSURE:
- 1-3/8 IN. SILL RISER: 3.75 PSF
- 1-7/8 IN. SILL RISER: 6.0 PSF
- 2-5/8 IN. SILL RISER: 9.0 PSF

OVERALL TEST SIZE: 12'-0" X 8'-0" NOMINAL

OVERALL PANEL SIZE: 48 IN. X 96 IN. NOMINAL
GLAZING: SINGLE PC. OF 3/16 IN. THK. TEMPERED GLASS
REINFORCING: STEEL IN INTERLOCKING STILES AND INTERMEDIATE JAMB. ADDITIONAL ALUM. REINFORCING ON EXTERIOR OF OPERATING INTERLOCK STILE.
CONFIGURATION: XOX

LIMITATIONS:
THE ABOVE ARE POSITIVE AND NEGATIVE STRUCTURAL DESIGN LOADS FROM COMPARATIVE ANALYSIS & HAVE NOT BEEN CAPPED BY RESULTS OF WATER PERFORMANCE TESTING.
WHERE LOCAL CODE REQUIRES WATER RESISTANCE TESTING TO PASS A MIN. 15% OF DESIGN PRESSURE, ALLOWABLE POSITIVE DESIGN PRESSURE WOULD BE CAPPED AS FOLLOWS:
- WHERE 1-3/8 IN. SILL RISER IS EMPLOYED POSITIVE DESIGN PRESSURE IS CAPPED AT 25.0 PSF.
- WHERE 1-7/8 IN. SILL RISER IS EMPLOYED POSITIVE DESIGN PRESSURE IS CAPPED AT 40.0 PSF.
- WHERE 2-5/8 IN. SILL RISER IS EMPLOYED POSITIVE DESIGN PRESSURE IS CAPPED AT 60.0 PSF.

PANEL WIDTHS AND HEIGHTS ARE NOMINAL, IN INCHES.

PREPARED BY:
PRODUCT TECHNOLOGY CORPORATION
1150 LOUISIANA AVENUE, SUITE 6
WINTER PARK, FLORIDA 32789
PHONE 407 622-6334 FAX 407 622.6335
www.ptc-corp.com

MI WINDOWS AND DOORS, INC.

420 / 430 / 440 SERIES ALUMNUM SLIDING GLASS DOOR

09/08/2004

COMAPARATIVE ANALYSIS CHART IN DESIGN PRESSURE

SGD non-Reinf

PANEL WIDTH >>	24	30	36	48	60
PANEL HEIGHT 80	64	54	47	39	35

TEST REPORT NO: ATI-52112.01-122-47
DESIGN PRESSURE ACHIEVED IN TEST: POS. & NEG. 35.0 PSF
WATER TEST PRESSURE:
- 1-3/8 IN. SILL RISER: 3.75 PSF
- 1-7/8 IN. SILL RISER: 6.0 PSF
- 2-5/8 IN. SILL RISER: 9.0 PSF

OVERALL SIZE TESTED: 15'-0" X 6'-8" NOMINAL
OVERALL PANEL SIZE TESTED: 5'-0" X 6'-8" NOMINAL
GLAZING: SINGLE PC. OF 3/16 IN THICK TEMP. GLASS
REINFORCING: NONE
CONFIGURATION TESTED: XXO

LIMITATIONS:
THE ABOVE ARE POSITIVE AND NEGATIVE STRUCTURAL DESIGN LOADS FROM COMPARATIVE ANALYSIS & HAVE NOT BEEN CAPPED BY RESULTS OF WATER PERFORMANCE TESTING.
WHERE LOCAL CODE REQUIRES WATER RESISTANCE TESTING TO PASS A MIN. 15% OF DESIGN PRESSURE, ALLOWABLE POSITIVE DESIGN PRESSURE WOULD BE CAPPED AS FOLLOWS:
- WHERE 1-3/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURE = 25.0 PSF
- WHERE 1-7/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURE = 40.0PSF
- WHERE 2-5/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURE = 60.0 PSF

PANEL WIDTHS AND HEIGHTS ARE NOMINAL, IN INCHES.

PREPARED BY:
PRODUCT TECHNOLOGY CORPORATION
1150 LOUISIANA AVENUE, SUITE 6
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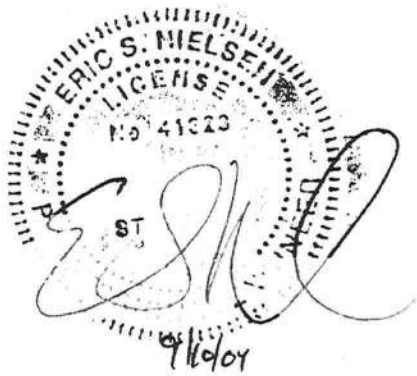

MI WINDOWS AND DOORS, INC.

420 / 430 / 440 SERIES ALUMINUM SLIDING GLASS DOOR | STEEL REINFORCED | 09/08/2004

COMAPARATIVE ANALYSIS CHART IN DESIGN PRESSURE | SGD STL REINF

PANEL WIDTH >>	24	30	36	48	60
PANEL HEIGHT 80	61	51	44	37	33
96	49	41	35	29	25

TEST REPORT NO: ATI-52112.01-122-47
DESIGN PRESSURE ACHIEVED IN TEST: POS. & NEG. 25.0 PSF
WATER TEST PRESSURE:
- 1-3/8 IN. SILL RISER: 3.75 PSF
- 1-7/8 IN. SILL RISER: 6.0 PSF
- 2-5/8 IN. SILL RISER: 9.0 PSF

OVERALL SIZE TESTED: 15'-0" X 8'-0" NOMINAL
OVERALL PANEL SIZE TESTED: 60 IN. X 96 IN. NOMINAL
GLAZING: SINGLE PC. OF 3/16 IN. THK. TEMPERED GLASS
REINFORCING: STEEL IN INTERLOCKING STILES, AND FIXED INTERMEDIATE JAMB
CONFIGURATION TESTED: OXX

LIMITATIONS:
THE ABOVE ARE POSITIVE AND NEGATIVE STRUCTURAL DESIGN LOADS FROM COMPARATIVE ANALYSIS & HAVE NOT BEEN CAPPED BY RESULTS OF WATER PERFORMANCE TESTING.
WHERE LOCAL CODE REQUIRES WATER RESISTANCE TESTING TO PASS A MIN. 15% OF DESIGN PRESSURE, ALLOWABLE POSITIVE DESIGN PRESSURE WOULD BE CAPPED AS FOLLOWS:
- WHERE 1-3/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURES ARE CAPPED AT 25.0 PSF.
- WHERE 1-7/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURES ARE CAPPED AT 40.0 PSF.
- WHERE 2-5/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURES ARE CAPPED AT 60.0 PSF.

PANEL WIDTHS AND HEIGHTS ARE NOMINAL, IN INCHES.

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CAPITOL WINDOWS AND DOORS

- Series 3540 Single Hung and Fixed Windows
- Series 8540 Single Hung and Fixed Windows

NOTE: SEE INDIVIDUAL TEST REPORT(S) FOR DP RATINGS AND MAXIMUM ALLOWABLE SIZES.

INSTALLATION INSTRUCTIONS FOR "APPROVED FOR FLORIDA" VINYL FIN WINDOWS

1. Storage: Do not lay windows flat, lean multiple units against poles, or store in the sun before installing.

2. Handle units one at a time in the closed and locked position. Place a continuous bead of adhesive caulk such as silicone or urethane on the back side of "nail fins" (mounting flanges) before placing in opening.

3. Place shims under corners of sill. In the closed and locked position, set unit into opening and make sure that there is 3/16" ± 1/16" clearance around the frame. Starting at the center of the longest frame member, place #8 sheet metal or wood screws (with a minimum of 1" penetration into the framing) in every other pre-punched slot which are on 4" to 5" centers (max. spacing 10"). Make sure that screws are driven in straight in order to avoid twisting or bowing. Make sure that the head and sill are straight and level and the jambs are straight and plumb. Check operation of unit frequently as fasteners are set.

4. **Note: Adherence to the above screw spacing and caulk requirements will allow this product to be used for design pressures (DP's) up to and including ± 47.2**

5. Caulk entire perimeter of the fin to mounting surface joint. Also caulk over screw heads and unused slots. Note: This step can be eliminated if 4" wide adhesive type flashing is used (sill 1st., jambs 2nd., head 3rd.).

6. Fill voids between window frame and construction with loose batten type insulation or non-expanding aerosol foam specifically formulated for windows. The use of expanding aerosol type insulating foam, which can bow the frame, voids all stated warranties. The use of muriatic acid for brick clean-up may damage the coil spring sash balance system. Windows must be masked off to avoid muriatic acid exposure, which will void the warranty.

7. Remove plaster, mortar, paint and any other debris that may have collected on the unit and make sure that sash/vent tracks are also clean. Do not use abrasives, solvents, ammonia, vinegar, alkaline, or acid solutions for clean-up, especially with insulated glass units as their use could cause chemical breakdown of the glass seal. Take care not to scratch glass, scratches severely weaken glass and it could eventually break from thermal expansion and contraction. Clean units with water and mild detergent.

8. For structures finished in brick or stone, allow ¼" gap under the sill, then caulk this joint when complete. Also caulk the head and jamb joints in the same manner.

9. If one or more holes are field drilled (by others) in any area of the window sill for any purpose (such as security systems), the warranty will be void. Adding holes can cause water leaks and interior damage.

- CAUTION -

Capitol Windows & Doors or its representatives are unable to control and cannot assume responsibility for the selection and placement of their products in a building or structure in a manner required by laws, statutes, and/or building codes. The purchaser is solely responsible for knowledge of and adherence to the same. BetterBilt window products are not provided with safety glazing unless specifically ordered with such. Many laws and codes require safety glazing (tempered glass) near doors, bathtubs, and shower enclosures. Also be aware of other code requirements such as emergency egress, structural performance, and energy performance.

Headquarters: M I. Home Products, 650 West Market St., Gratz, PA 17030 (717) 365-3300 www.mihp.com

FL 721

June 26, 2003

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.

SERIES/MODEL: 3540
PRODUCT TYPE: PVC Triple Single Hung

Title	Summary of Results
Rating	H-R30* 108 x 74
Operating Force	17 lbf max.
Air Infiltration	0.11 cfm/ft^2
Water Resistance Test Pressure	4.50 psf
Uniform Load Deflection Test Pressure	±47.2 psf
Uniform Load Structural Test Pressure	+52.5 psf, -70.8 psf
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 50172.01-122-47 for complete test specimen description and data.

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Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No.:	50172.01-122-47
Revision 1:	08/30/04
Test Dates:	06/11/04
Through:	07/07/04
Report Date:	07/27/04
Expiration Date:	07/07/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on a Series/Model 3540, triple single hung window at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R30* 108 x 74 rating. Reference should be made to Report No. 01-45617.02 for Gateway Performance results. Test specimen description and results are reported herein.

***General Note**: An asterisk (*) next to the performance grade indicates that the size tested for optional performance was smaller than the Gateway test size for the product type and class.*

Test Specification: The test specimen was evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 3540

Product Type: PVC Triple Single Hung

Overall Size: 8' 11-5/8" wide by 6' 1-3/4" high

Interior Sash Size (3): 2' 9-3/4" wide by 3' 0-1/8" high

Fixed Daylight Opening Size (3): 2' 7-3/4" wide by 2' 9-3/16" high

Screen Size: 2' 9" wide by 2' 11-1/4" high

Overall Area: 55.1 ft^2

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Test Specimen Description: (Continued)

Finish: All PVC was white.

Glazing Details: All glazing consisted of 7/8" thick sealed insulating glass units that were comprised of two sheets of 3/32" thick clear annealed glass and a metal reinforced butyl spacer system. The glass was interior glazed against a double-sided adhesive glazing tape and secured with vinyl glazing beads.

Weatherstripping:

Description	Quantity	Location
0.187" backed by 0.250" high polypile	1 Row	Meeting rail, stiles
0.187" backed by 0.250" high polypile	1 Row	Sill leg
0.187" backed by 0.310" high polypile	1 Row	Stiles
0.187" backed, 1/4 foam filled single leaf vinyl bulb gasket	1 Row	Bottom rail
0.187" backed, 1/8 foam filled vinyl bulb gasket	1 Row	Fixed meeting rail

Frame Construction: The frame was constructed of extruded PVC members. Corners were mitered and welded. End caps were utilized on the ends of the meeting rail and secured with three #6 by 5/8" screws per cap. The fixed meeting rail was then secured to the frame utilizing three #6 by 5/8" screws.

Sash Construction: The sash was constructed of extruded PVC members. Corners were mitered and welded.

Screen Construction: The screen was constructed of roll-formed aluminum. Corners were square-cut and secured with vinyl corner keys. The mesh was secured with a flexible vinyl spline.

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Constant force balances	6	One per jamb
Metal cam locks with adjacent keepers	6	Meeting rail, 7" from each end
Plastic tilt latches	6	Each end of the interior meeting rail
Metal pivot pins	6	Each end of the bottom rail

Drainage:

Description	Quantity	Location
3/32" by 1/2" weepslot	12	Bottom rail, 2 at each end
1/8" by 1" weepslot	2	Sill, 3" from each end
3/16" by 1/2" weepslot	2	Screen track, 2-1/2" from each end

Reinforcement: The interior meeting rail and bottom rail utilized a roll-formed "I beam" steel reinforcement (Drawing #GVL-451-020). The fixed meeting rail utilized a steel reinforcement (Drawing #RF-104S-020). The intermediate frame rails utilized a steel reinforcement (Drawing #2.75x.125 steel plate).

Installation: The unit was installed into a wood test buck. The nail fin was set against a silicone bedding and fastened to the buck with #6 by 1-5/8" screws, 2" from corners and 8" on center. 3/4" washers were utilized along the entire length of the sill, at midspan of the head and jambs, and at all corners.

Test Results: The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.6.1.1	Operating Force	17 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.11 cfm/ft^2	0.3 cfm/ft^2 max.

***Note #1**: The tested specimen meets (or exceeds) the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.3	Water Resistance per ASTM E 547 (with and without screen)		See Note #2

***Note #2**: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance".*

2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the mullion) (Loads were held for 52 seconds)		
	35.0 psf (positive)	0.39"	See Note #3
	35.0 psf (negative)	0.54"	See Note #3

***Note #3**: The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the mullion) (Loads were held for 10 seconds)		
	52.5 psf (positive)	<0.01"	0.27" max.
	52.5 psf (negative)	0.07"	0.27" max.
2.2.6.1.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Interior meeting rail	0.13"/26%	0.50"/100%
	Bottom rail	0.11"/22%	0.50"/100%
	In remaining direction - 50 lbs		
	Left stile	0.09"/18%	0.50"/100%
	Right stile	0.10"/20%	0.50"/100%
2.1.7	Welded Corner Test	Meets as stated	Meets as stated

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1	No entry	No entry
	Test A2	No entry	No entry
	Test A3	No entry	No entry
	Test A4	No entry	No entry
	Test A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen)		
	4.50 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the mullion) (Loads were held for 52 seconds)		
	47.2 psf (positive)	0.73"	See Note #3
	47.2 psf (negative)	0.92"	See Note #3
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the mullion) (Loads were held for 10 seconds)		
	52.5 psf (positive)	<0.01"	0.27" max.
	70.8 psf (negative)	0.21"	0.27" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Digitally Signed by: Jeramie D. Grabosch

Jeramie D. Grabosch
Technician

Digitally Signed by: Steven M. Urich

Steven. M. Urich, P.E.
Senior Project Engineer

JDG:vlm

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 3540 Picture Window
with Continuous Head and Sill
PRODUCT TYPE: PVC Triple Fixed Window

Title	Summary of Results
Rating	F-C40 108 x 74
Uniform Load Deflection Test Pressure	±47.0 psf
Uniform Structural Load Test Pressure	±70.5 psf

Reference should be made to ATI Report Identification No. 01-51007.02 for complete test specimen description and data.

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phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030

ATI Report Identification No.: 01-51007.02
Test Date: 04/30/04
Report Date: 06/12/04
Expiration Date: 04/30/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc to witness testing on a Series/Model 3540 Picture Window with Continuous Head and Sill, PVC triple fixed window at MI Home Products, Inc.'s test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a F-C40 108 x 74 rating. Test specimen description and results are reported herein. Reference should be made to ATI Report Identification No. 01-51007.01 for air infiltration and water penetration test results.

Test Specification: The test specimen was evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 3540 Picture Window with Continuous Head and Sill

Product Type: PVC Triple Fixed Window

Overall Size: 8' 11-9/16" wide by 6' 1-1/2" high

Daylight Opening Size (3): 2' 7-3/4" wide by 5' 9-1/2" high

Overall Area: 54.90 ft^2

Finish: All PVC was white.

Glazing Details: The window utilized 7/8" thick, sealed insulating glass units fabricated of two sheets of 1/8" thick, clear annealed glass and metal reinforced butyl spacer system. The glass was interior glazed against a dual-sided adhesive foam tape and secured utilizing extruded snap-in glazing beads.

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www.archtest.com

Test Specimen Description: (Continued)

Frame Construction: The frame was constructed of extruded PVC members with mitered and welded corners. An extruded PVC snap-in filler piece was utilized around the entire interior perimeter of the each opening. The mullions were coped, butted, sealed with single-sided adhesive foam pads and secured to the head and sill utilizing two #8 by 2-1/2" long screws and two #8 by 1-1/4" screws. Sealant was utilized at the exterior face of the mullion to jamb joinery.

Drainage:

Description	Quantity	Location
5/8" wide by 1/4" high weepslot	2 Per window	Corners of the sill draining the interior sill hollow
5/8" wide by 1/4" high weepslot	2 Per window	Corners of the sill draining the intermediate sill hollow
1" long by 1/8" high weepslot	2 Per window	Exterior sill face draining the exterior sill hollow
1/2" long by 3/16" high weepslot	2 Per window	2" from corners draining the glazing pocket
1/2" long by 1/8" high weepslot	2 Per window	1" from corners draining the glazing channel

Reinforcement: A 1/8" by 2-1/2" strip of steel was utilized in the mullions.

Installation: The window was installed into a Spruce-Pine-Fir wood test buck utilizing an integral nailing fin. The nailing fin was bedded in silicone and secured to the test buck utilizing 1-5/8" wood screws 3" from the corners and 8" on center. Washers were utilized at the corners and midspan of the head, jambs and entire sill. A 3/4" by 3/4" wood blind stop was secured to the sill utilizing 1-5/8" wood screws 2" from the end and 8" on center.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the mullion) (Loads were held for 52 seconds)		
	47.0 psf (positive)	0.63"	See Note #2
	47.0 psf (negative)	0.69"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the mullion) (Loads were held for 10 seconds)		
	70.5 psf (positive)	0.03"	0.28" max.
	70.5 psf (negative)	0.11"	0.28" max.

***Note #2:** The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Digitally Signed by: Eric Westphal

Eric Westphal
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

EW:nlb
01-51007.02

CAPITOL™ WINDOWS AND DOORS

- Series 3540 Single Hung and Fixed Windows
- Series 8540 Single Hung and Fixed Windows

NOTE: SEE INDIVIDUAL TEST REPORT(S) FOR DP RATINGS AND MAXIMUM ALLOWABLE SIZES.

INSTALLATION INSTRUCTIONS FOR "APPROVED FOR FLORIDA" VINYL FIN WINDOWS

1. Storage: Do not lay windows flat, lean multiple units against poles, or store in the sun before installing.

2. Handle units one at a time in the closed and locked position. Place a continuous bead of adhesive caulk such as silicone or urethane on the back side of "nail fins" (mounting flanges) before placing in opening.

3. Place shims under corners of sill. In the closed and locked position, set unit into opening and make sure that there is 3/16" ± 1/16" clearance around the frame. Starting at the center of the longest frame member, place #8 sheet metal or wood screws (with a minimum of 1" penetration into the framing) in every other pre-punched slot which are on 4" to 5" centers (max. spacing 10"). Make sure that screws are driven in straight in order to avoid twisting or bowing. Make sure that the head and sill are straight and level and the jambs are straight and plumb. Check operation of unit frequently as fasteners are set.

4. **Note: Adherence to the above screw spacing and caulk requirements will allow this product to be used for design pressures (DP's) up to and including ± 47.2**

5. Caulk entire perimeter of the fin to mounting surface joint. Also caulk over screw heads and unused slots. Note: This step can be eliminated if 4" wide adhesive type flashing is used (sill 1st., jambs 2nd, head 3rd).

6. Fill voids between window frame and construction with loose batten type insulation or non-expanding aerosol foam specifically formulated for windows. The use of expanding aerosol type insulating foam, which can bow the frame, voids all stated warranties. The use of muriatic acid for brick clean-up may damage the coil spring sash balance system. Windows must be masked off to avoid muriatic acid exposure, which will void the warranty.

7. Remove plaster, mortar, paint and any other debris that may have collected on the unit and make sure that sash/vent tracks are also clean. Do not use abrasives, solvents, ammonia, vinegar, alkaline, or acid solutions for clean-up, especially with insulated glass units as their use could cause chemical breakdown of the glass seal. Take care not to scratch glass; scratches severely weaken glass and it could eventually break from thermal expansion and contraction. Clean units with water and mild detergent.

8. For structures finished in brick or stone, allow ¼" gap under the sill, then caulk this joint when complete. Also caulk the head and jamb joints in the same manner.

9. If one or more holes are field drilled (by others) in any area of the window sill for any purpose (such as security systems), the warranty will be void. Adding holes can cause water leaks and interior damage.

- CAUTION -

Capitol Windows & Doors or its representatives are unable to control and cannot assume responsibility for the selection and placement of their products in a building or structure in a manner required by laws, statutes, and/or building codes. The purchaser is solely responsible for knowledge of and adherence to the same. BetterBilt window products are not provided with safety glazing unless specifically ordered with such. Many laws and codes require safety glazing (tempered glass) near doors, bathtubs, and shower enclosures. Also be aware of other code requirements such as emergency egress, structural performance, and energy performance.

Headquarters: M.I. Home Products 650 West Market St. Gratz, PA 17030 (717) 365-3300 www.mihp.com

FL 721

June 26, 2003

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.

SERIES/MODEL: 3540
PRODUCT TYPE: PVC Triple Single Hung

Title	Summary of Results
Rating	H-R30* 108 x 74
Operating Force	17 lbf max.
Air Infiltration	0.11 cfm/ft^2
Water Resistance Test Pressure	4.50 psf
Uniform Load Deflection Test Pressure	±47.2 psf
Uniform Load Structural Test Pressure	+52.5 psf, -70.8 psf
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 50172.01-122-47 for complete test specimen description and data.

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Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No.:	50172.01-122-47
Revision 1:	08/30/04
Test Dates:	06/11/04
Through:	07/07/04
Report Date:	07/27/04
Expiration Date:	07/07/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on a Series/Model 3540, triple single hung window at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R30* 108 x 74 rating. Reference should be made to Report No. 01-45617.02 for Gateway Performance results. Test specimen description and results are reported herein.

***General Note**: An asterisk (*) next to the performance grade indicates that the size tested for optional performance was smaller than the Gateway test size for the product type and class.*

Test Specification: The test specimen was evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 3540

Product Type: PVC Triple Single Hung

Overall Size: 8' 11-5/8" wide by 6' 1-3/4" high

Interior Sash Size (3): 2' 9-3/4" wide by 3' 0-1/8" high

Fixed Daylight Opening Size (3): 2' 7-3/4" wide by 2' 9-3/16" high

Screen Size: 2' 9" wide by 2' 11-1/4" high

Overall Area: 55.1 ft^2

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Test Specimen Description: (Continued)

Finish: All PVC was white.

Glazing Details: All glazing consisted of 7/8" thick sealed insulating glass units that were comprised of two sheets of 3/32" thick clear annealed glass and a metal reinforced butyl spacer system. The glass was interior glazed against a double-sided adhesive glazing tape and secured with vinyl glazing beads.

Weatherstripping:

Description	Quantity	Location
0.187" backed by 0.250" high polypile	1 Row	Meeting rail, stiles
0.187" backed by 0.250" high polypile	1 Row	Sill leg
0.187" backed by 0.310" high polypile	1 Row	Stiles
0.187" backed, 1/4 foam filled single leaf vinyl bulb gasket	1 Row	Bottom rail
0.187" backed, 1/8 foam filled vinyl bulb gasket	1 Row	Fixed meeting rail

Frame Construction: The frame was constructed of extruded PVC members. Corners were mitered and welded. End caps were utilized on the ends of the meeting rail and secured with three #6 by 5/8" screws per cap. The fixed meeting rail was then secured to the frame utilizing three #6 by 5/8" screws.

Sash Construction: The sash was constructed of extruded PVC members. Corners were mitered and welded.

Screen Construction: The screen was constructed of roll-formed aluminum. Corners were square-cut and secured with vinyl corner keys. The mesh was secured with a flexible vinyl spline.

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Constant force balances	6	One per jamb
Metal cam locks with adjacent keepers	6	Meeting rail, 7" from each end
Plastic tilt latches	6	Each end of the interior meeting rail
Metal pivot pins	6	Each end of the bottom rail

Drainage:

Description	Quantity	Location
3/32" by 1/2" weepslot	12	Bottom rail, 2 at each end
1/8" by 1" weepslot	2	Sill, 3" from each end
3/16" by 1/2" weepslot	2	Screen track, 2-1/2" from each end

Reinforcement: The interior meeting rail and bottom rail utilized a roll-formed "I beam" steel reinforcement (Drawing #GVL-451-020). The fixed meeting rail utilized a steel reinforcement (Drawing #RF-104S-020). The intermediate frame rails utilized a steel reinforcement (Drawing #2.75x.125 steel plate).

Installation: The unit was installed into a wood test buck. The nail fin was set against a silicone bedding and fastened to the buck with #6 by 1-5/8" screws, 2" from corners and 8" on center. 3/4" washers were utilized along the entire length of the sill, at midspan of the head and jambs, and at all corners.

Test Results: The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.6.1.1	Operating Force	17 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.11 cfm/ft^2	0.3 cfm/ft^2 max.

***Note #1**: The tested specimen meets (or exceeds) the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.3	Water Resistance per ASTM E 547 (with and without screen)		See Note #2

***Note #2**: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance".*

2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the mullion) (Loads were held for 52 seconds)		
	35.0 psf (positive)	0.39"	See Note #3
	35.0 psf (negative)	0.54"	See Note #3

***Note #3**: The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the mullion) (Loads were held for 10 seconds)		
	52.5 psf (positive)	<0.01"	0.27" max.
	52.5 psf (negative)	0.07"	0.27" max.
2.2.6.1.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Interior meeting rail	0.13"/26%	0.50"/100%
	Bottom rail	0.11"/22%	0.50"/100%
	In remaining direction - 50 lbs		
	Left stile	0.09"/18%	0.50"/100%
	Right stile	0.10"/20%	0.50"/100%
2.1.7	Welded Corner Test	Meets as stated	Meets as stated

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1	No entry	No entry
	Test A2	No entry	No entry
	Test A3	No entry	No entry
	Test A4	No entry	No entry
	Test A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen)		
	4.50 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the mullion) (Loads were held for 52 seconds)		
	47.2 psf (positive)	0.73"	See Note #3
	47.2 psf (negative)	0.92"	See Note #3
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the mullion) (Loads were held for 10 seconds)		
	52.5 psf (positive)	<0.01"	0.27" max.
	70.8 psf (negative)	0.21"	0.27" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Jeramie D. Grabosch

Digitally Signed by: Jeramie D. Grabosch

Jeramie D. Grabosch
Technician

Digitally Signed by: Steven M. Urich

Steven. M. Urich, P.E.
Senior Project Engineer

JDG:vlm

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 3540 Picture Window
with Continuous Head and Sill
PRODUCT TYPE: PVC Triple Fixed Window

Title	Summary of Results
Rating	F-C40 108 x 74
Uniform Load Deflection Test Pressure	±47.0 psf
Uniform Structural Load Test Pressure	±70.5 psf

Reference should be made to ATI Report Identification No. 01-51007.02 for complete test specimen description and data.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030

ATI Report Identification No.: 01-51007.02
Test Date: 04/30/04
Report Date: 06/12/04
Expiration Date: 04/30/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc to witness testing on a Series/Model 3540 Picture Window with Continuous Head and Sill, PVC triple fixed window at MI Home Products, Inc.'s test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a F-C40 108 x 74 rating. Test specimen description and results are reported herein. Reference should be made to ATI Report Identification No. 01-51007.01 for air infiltration and water penetration test results.

Test Specification: The test specimen was evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 3540 Picture Window with Continuous Head and Sill

Product Type: PVC Triple Fixed Window

Overall Size: 8' 11-9/16" wide by 6' 1-1/2" high

Daylight Opening Size (3): 2' 7-3/4" wide by 5' 9-1/2" high

Overall Area: 54.90 ft^2

Finish: All PVC was white.

Glazing Details: The window utilized 7/8" thick, sealed insulating glass units fabricated of two sheets of 1/8" thick, clear annealed glass and metal reinforced butyl spacer system. The glass was interior glazed against a dual-sided adhesive foam tape and secured utilizing extruded snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Test Specimen Description: (Continued)

Frame Construction: The frame was constructed of extruded PVC members with mitered and welded corners. An extruded PVC snap-in filler piece was utilized around the entire interior perimeter of the each opening. The mullions were coped, butted, sealed with single-sided adhesive foam pads and secured to the head and sill utilizing two #8 by 2-1/2" long screws and two #8 by 1-1/4" screws. Sealant was utilized at the exterior face of the mullion to jamb joinery.

Drainage:

Description	Quantity	Location
5/8" wide by 1/4" high weepslot	2 Per window	Corners of the sill draining the interior sill hollow
5/8" wide by 1/4" high weepslot	2 Per window	Corners of the sill draining the intermediate sill hollow
1" long by 1/8" high weepslot	2 Per window	Exterior sill face draining the exterior sill hollow
1/2" long by 3/16" high weepslot	2 Per window	2" from corners draining the glazing pocket
1/2" long by 1/8" high weepslot	2 Per window	1" from corners draining the glazing channel

Reinforcement: A 1/8" by 2-1/2" strip of steel was utilized in the mullions.

Installation: The window was installed into a Spruce-Pine-Fir wood test buck utilizing an integral nailing fin. The nailing fin was bedded in silicone and secured to the test buck utilizing 1-5/8" wood screws 3" from the corners and 8" on center. Washers were utilized at the corners and midspan of the head, jambs and entire sill. A 3/4" by 3/4" wood blind stop was secured to the sill utilizing 1-5/8" wood screws 2" from the end and 8" on center.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330		
	(Deflections reported were taken on the mullion)		
	(Loads were held for 52 seconds)		
	47.0 psf (positive)	0.63"	See Note #2
	47.0 psf (negative)	0.69"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330		
	(Permanent sets reported were taken on the mullion)		
	(Loads were held for 10 seconds)		
	70.5 psf (positive)	0.03"	0.28" max.
	70.5 psf (negative)	0.11"	0.28" max.

***Note #2**: The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Digitally Signed by: Eric Westphal

Eric Westphal
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

EW:nlb
01-51007.02

HALL'S PUMP & WELL SERVICE, INC.

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DONALD AND MARY HALL
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June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

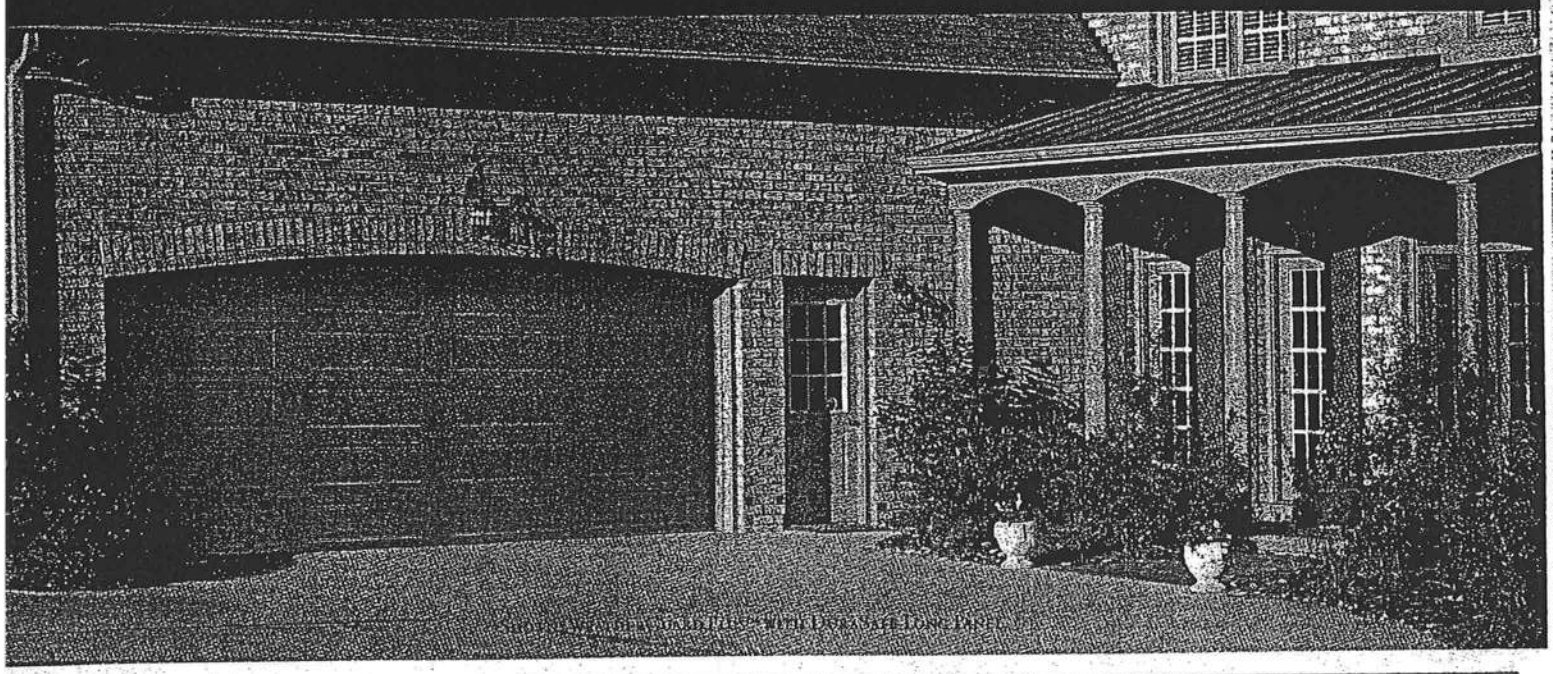
If you have any questions please feel free to call our office anytime.

Thank you,

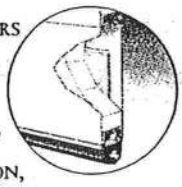
Donald D. Hall
DDH/jk

Amarr GARAGE DOORS

BEST | WEATHERGUARD™ SERIES
FEATURING OUR **DuraSafe System**



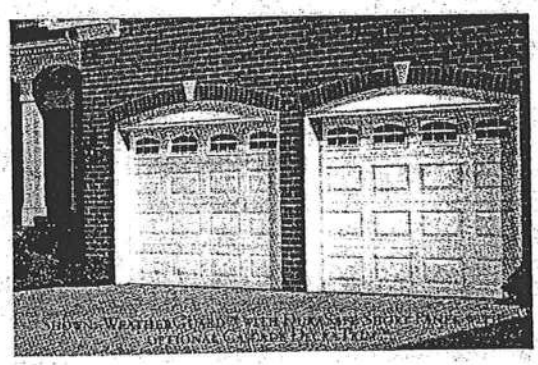
WEATHERGUARD PLUS™ WITH **DuraSafe**



THE WEATHERGUARD PLUS OFFERS DISCERNING HOMEOWNERS A MASTERFUL COMBINATION OF PREMIUM FEATURES. SUPERIOR TRIPLE-LAYER CONSTRUCTION, 2" (5.1 CM) POLYSTYRENE INSULATION, AN R-VALUE OF 8.34, AND UNMATCHED BEAUTY PUT THE WEATHERGUARD PLUS AT THE TOP OF ITS CLASS.

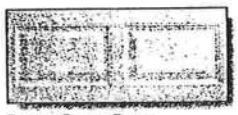
WEATHERGUARD™ WITH **DuraSafe**

TOP-QUALITY TRIPLE-LAYER CONSTRUCTION AND 1 3/8" (3.5 CM) POLYSTYRENE INSULATION MAKE OUR WEATHERGUARD STEEL DOOR STRONG, QUIET, AND ENERGY EFFICIENT. FEATURING AN R-VALUE OF 5.73, THE WEATHERGUARD IS THE PERFECT ADDITION TO YOUR HOME FOR YEARS OF TROUBLE FREE SERVICE AND GREAT LOOKS.

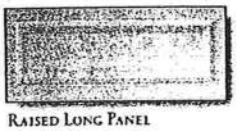


DESIGN ELEMENTS

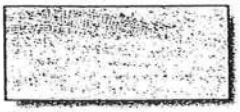
THE WEATHERGUARD SERIES DOORS ARE AVAILABLE WITH A RAISED SHORT, RAISED LONG, OR FLUSH PANEL DESIGN IN YOUR CHOICE OF FOUR COLORS.*



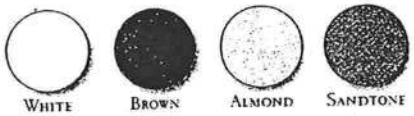
RAISED SHORT PANEL



RAISED LONG PANEL



FLUSH PANEL

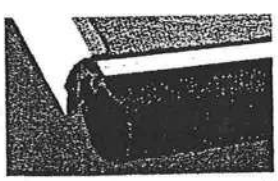


WHITE BROWN ALMOND SANDTONE

* ACTUAL PAINT COLORS MAY VARY FROM SAMPLES SHOWN.

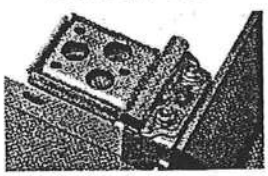
Bottom Seal

NEW ALUMINUM BOTTOM SEAL MEANS EASY AND FAST INSTALLATION AND MAINTENANCE... AS WELL AS A BETTER SEAL AGAINST THE ELEMENTS.



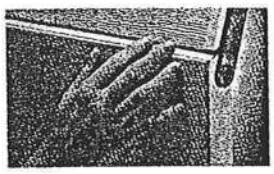
Bottom Bracket

NEW TAMPER RESISTANT BOTTOM BRACKET HELPS PREVENT ACCIDENTS, YET ALLOWS FOR ROLLER MAINTENANCE/CHANGE WITHOUT DISASSEMBLY. FULL LENGTH ROLLER TUBE PREVENTS SLIP-OUTS.



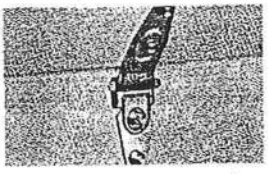
Door Sections

THE SECTION JOINT OF THE FUTURE: TODAY. NEW SECTION PROFILE ASSURES PINCH RESISTANCE BOTH INSIDE AND OUT, EXCEEDING INDUSTRY STANDARDS - NEITHER FINGERS NOR WEATHER GETS IN.



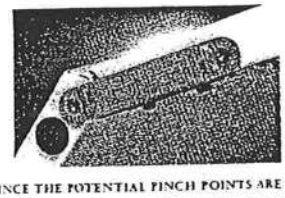
Center Hinge

FLUSH MOUNT INBOARD DESIGN CENTER HINGES PROVIDE PINCH RESISTANT PROTECTION AND A LOW PROFILE CLEAN LOOK ON THE INSIDE OF THE DOOR.



End Hinge

WITH MOST OF ITS ACTION HIDDEN INSIDE THE DOOR, OUR RE-ENGINEERED END HINGES LEAVE NO ROOM FOR EVEN THE SMALLEST FINGERS.

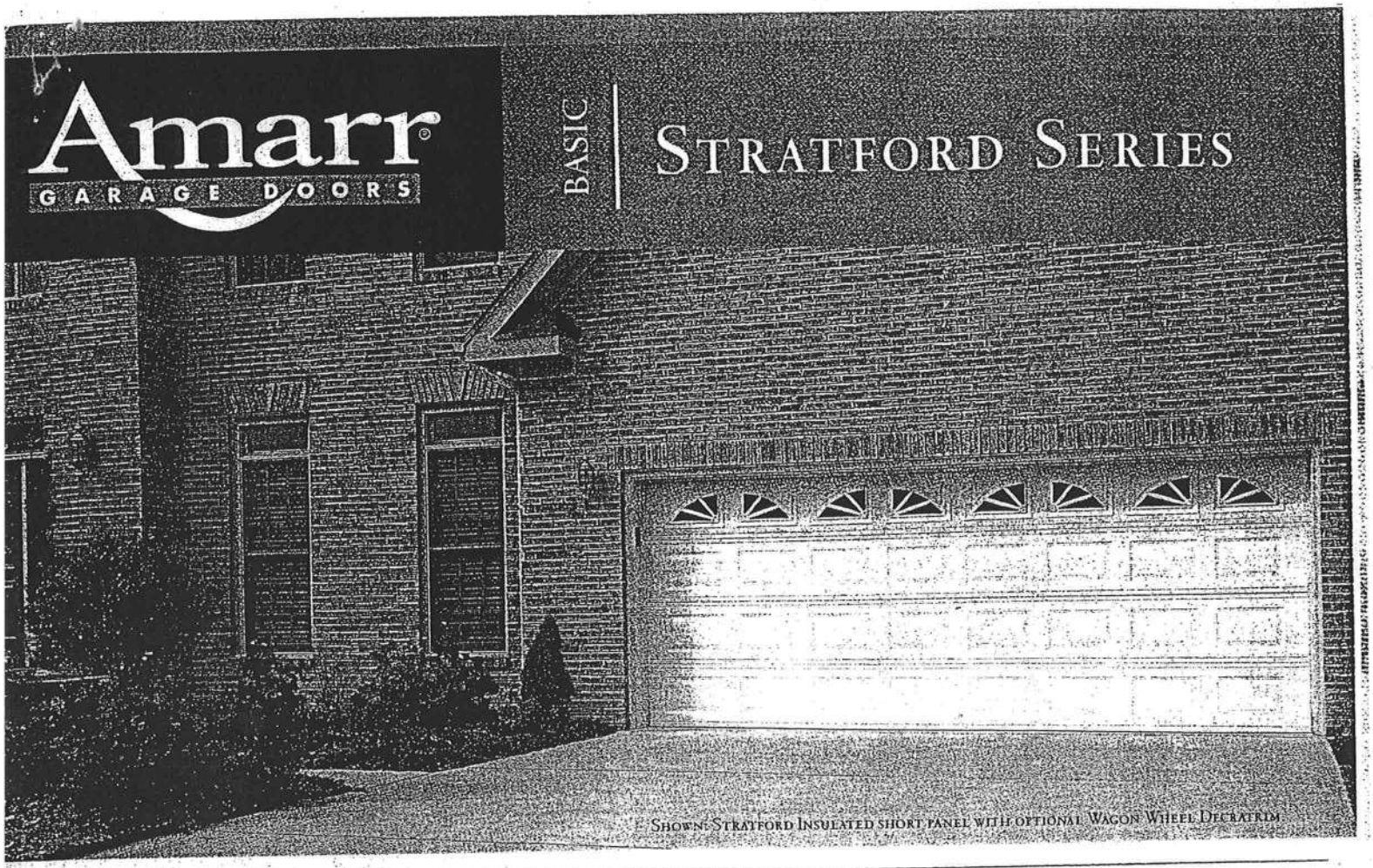


AMARR DURASAFE DOORS UNDER 8'9" WILL BE SUPPLIED WITH DURASAFE HARDWARE. DASMA STANDARDS FOR PINCH-RESISTANCE DO NOT APPLY TO DOORS OVER 8' HIGH SINCE THE POTENTIAL PINCH POINTS ARE ABOVE TYPICAL GRASPING HEIGHTS; AMARR DOORS OVER 8'9" ARE SUPPLIED WITH CONVENTIONAL HARDWARE. THE BOTTOM BRACKET, DOOR SECTIONS, CENTER HINGE AND END HINGE SHOWN ABOVE ARE PATENTED. DOORS SHOWN ARE ELECTRICALLY OPERATED. NON-ELECTRICALLY OPERATED DOORS SHOULD HAVE EXTERIOR AND INTERIOR LIFT HANDLES ATTACHED TO THE DOOR.

Amarr
GARAGE DOORS

BASIC

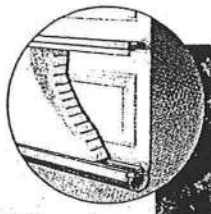
STRATFORD SERIES



SHOWN: STRATFORD INSULATED SHORT PANEL WITH OPTIONAL WAGON WHEEL DECRATRIM

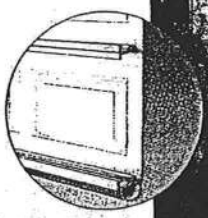
STRATFORD INSULATED

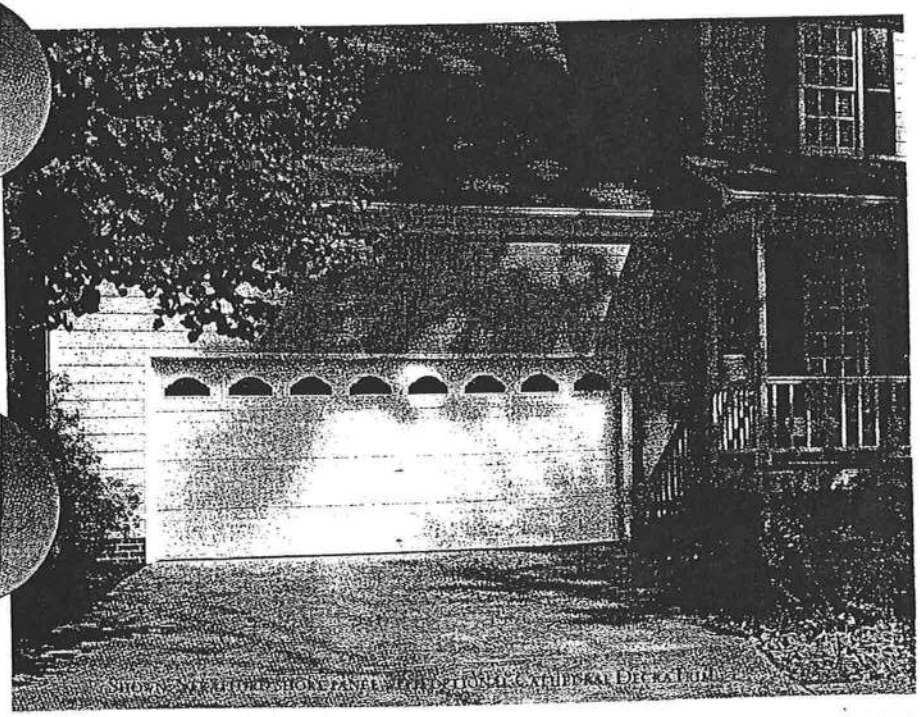
THE 2" (5.1 CM) THICK STRATFORD INSULATED PROVIDES HOMEOWNERS EXCELLENT THERMAL PROTECTION AND HANDSOME GOOD LOOKS. FEATURES INCLUDE DOUBLE-LAYER CONSTRUCTION OF STURDY 25-GAUGE STEEL, AND 1 7/16" (3.7 CM) POLYSTYRENE INSULATION WITH LAMINATED BACKING AND AN R-VALUE OF 5.65.



STRATFORD

A SUPERLATIVE ADDITION TO ANY HOME, THE STRATFORD'S DURABLE SINGLE-LAYER CONSTRUCTION, 25-GAUGE STEEL, AND ATTRACTIVE DESIGN PROVIDE HOMEOWNERS WITH EXCEPTIONAL VALUE.

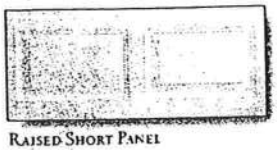




SHOWN: WEATHERED SHORT PANEL WITH OPTIONAL DECRATRIM

DESIGN ELEMENTS

THE STRATFORD SERIES DOORS ARE AVAILABLE WITH A RAISED SHORT PANEL DESIGN IN YOUR CHOICE OF THREE COLORS.*



RAISED SHORT PANEL

WHITE ALMOND SANDTONE

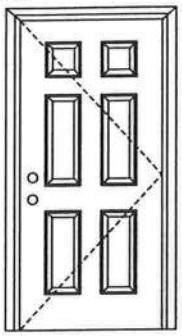
* ACTUAL PAINT COLORS MAY VARY FROM SAMPLES SHOWN.

X
Opaque Inswing Unit

COP-WL-MA0101-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure

+76.0/-76.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

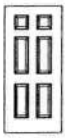
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

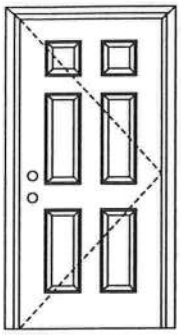
Masonite®

X
Opaque Outswing Unit

COP-WL-MA0121-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+76.0/-76.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0011-02.

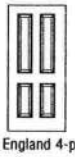
MINIMUM INSTALLATION DETAIL:

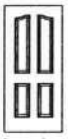
Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Neo-Textured Fiberglass Entry Doors

March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Masonite®

X
Opaque Inswing Unit

COP-WL-MA0101-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

NCTL 210-1973-1, 2, 3

Certifying Engineer and License Number: Barry Portney, P.E. 16258

CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996.

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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X
Opaque Inswing Unit

COP-WL-MA0101-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

NCTL 210-1973-1, 2, 3

Certifying Engineer and License Number: Barry Portney, P.E. 16258

CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996.

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886
COMPANY NAME
CITY, STATE

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Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

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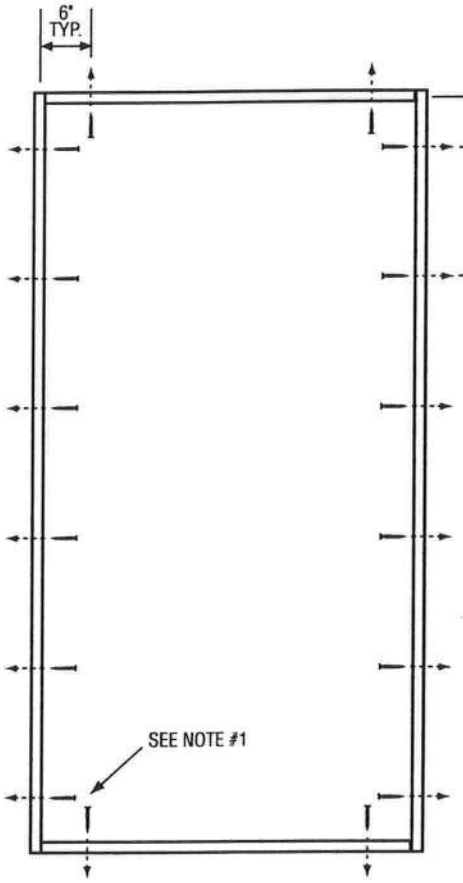
Masonite®

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey ETL

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0246*, 0266*, 3241*, 3246, 3261* or 3266**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 10, 2003

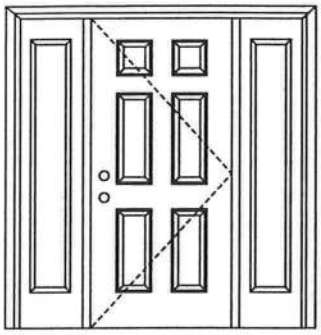
Masonite®

OXO
Opaque Outswing Unit

COP-WL-MA0124-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 5'4" x 6'8"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etisemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED on opaque panel, but is required on glazed panels.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0014-02 or MAD-WL-MA0017-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

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ARTEK™ Non-Textured Fiberglass Entry Doors

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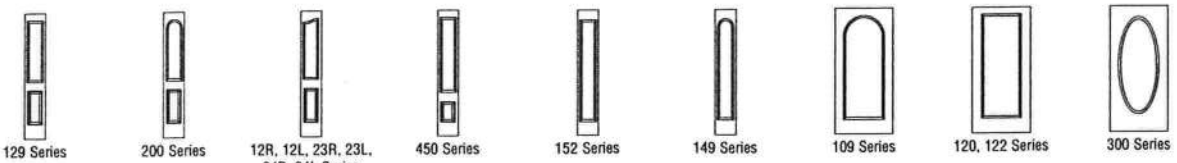
OXO
Opaque Outswing Unit

COP-WL-MA0124-02

FIBERGLASS DOORS

APPROVED SIDELITE STYLES:



129 Series | 200 Series | 12R, 12L, 23R, 23L, 24R, 24L Series | 450 Series | 152 Series | 149 Series | 109 Series | 120, 122 Series | 300 Series

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panel glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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FIBERGLASS ENTRY DOORS

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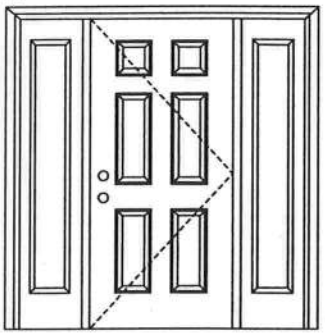
OXO
Opaque Inswing Unit

COP-WL-MA0104-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 5'4" x 6'8"

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED on opaque panel, but is required on glazed panels.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft
Wood-Grain Textured
FIBERGLASS ENTRY DOORS

ARTEK™
Non-Textured Fiberglass Entry Doors

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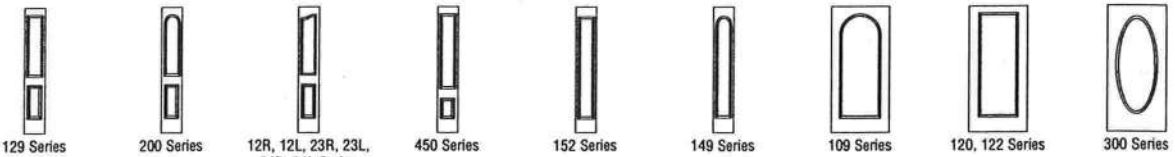
OXO
Opaque Inswing Unit

COP-WL-MA0104-02

FIBERGLASS DOORS

APPROVED SIDELITE STYLES:



129 Series | 200 Series | 12R, 12L, 23R, 23L, 24R, 24L Series | 450 Series | 152 Series | 149 Series | 109 Series | 120, 122 Series | 300 Series

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panel glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

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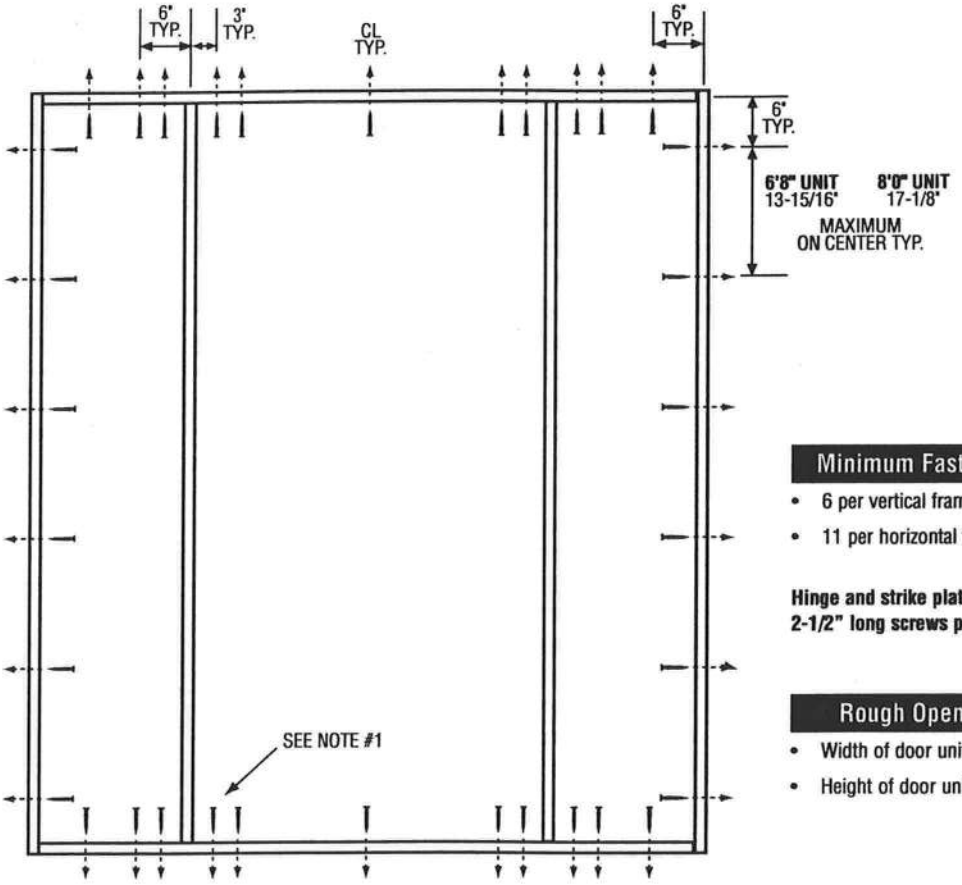
Masonite®

OXO
Unit

MID-WL-MA0004-02

SINGLE DOOR WITH 2 SIDELITES



Minimum Fastener Count

- 6 per vertical framing member
- 11 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0249*, 0269*, 3244*, 3249, 3264* or 3269**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Mrach 10, 2003

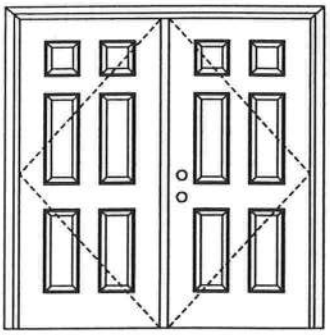
Masonite®

XX
Opaque Outswing Unit

COP-WL-MA0122-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Double Door
Maximum unit size = 6'0" x 6'8"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

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Masonite®

XX
Opaque Outswing Unit

COP-WL-MA0122-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

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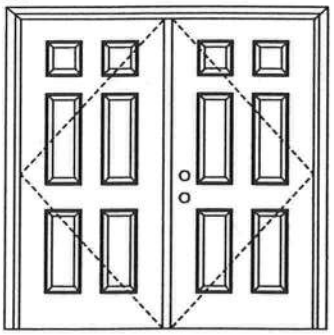
Masonite®

XX
Opaque Inswing Unit

COP-WL-MA0102-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

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XX
Opaque Inswing Unit

COP-WL-MA0102-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

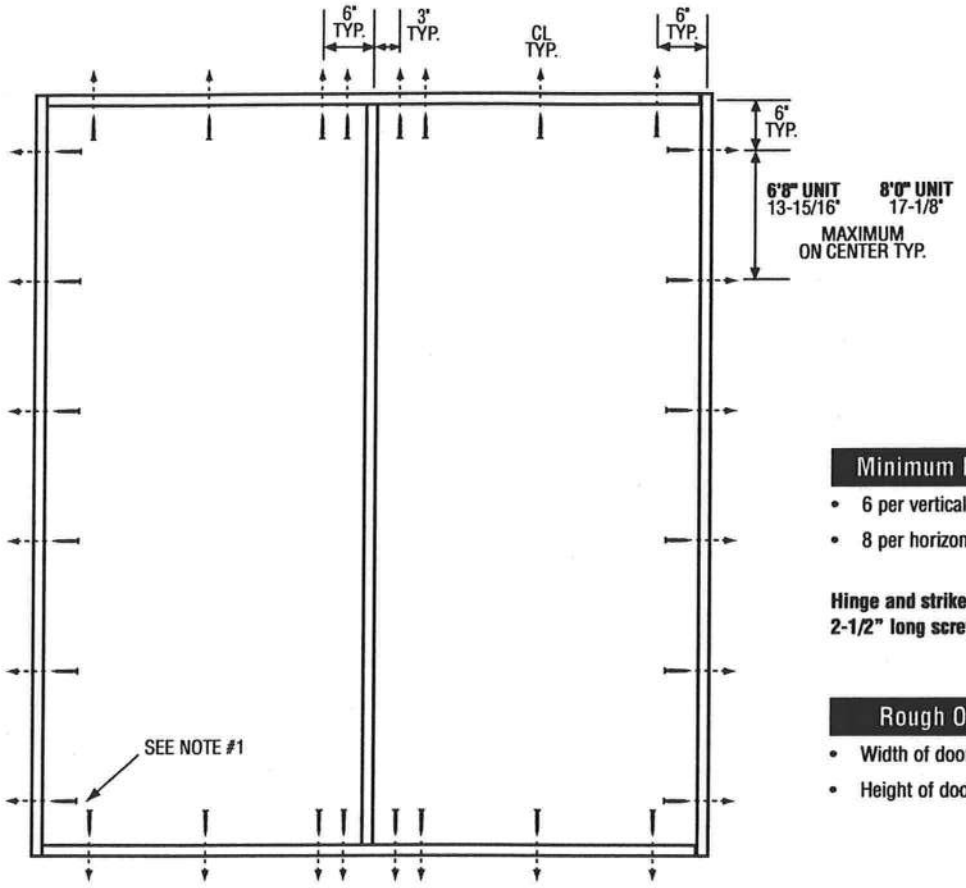
March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Masonite®

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0247*, 0267*, 3242*, 3247, 3262* or 3267**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 10, 2003

Masonite®

Project Information for:	L227913		
Builder:	DON REED	Date:	2/23/2007
Lot:	N/A	Start Number:	**1003**
Subdivision:	274 EVERETT TERRACE	SEI Ref:	L227913
County or City:	COLUMBIA COUNTY		
Truss Page Count:	40		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

REED, LARRY DON CGC 036224
Address: 2230 E BAYA AVE. STE 101
LAKE CITY ,FLORIDA 32025 Designer: **148**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0223071003	2/23/2007				
2	CJ3	0223071004	2/23/2007				
3	CJ5	0223071005	2/23/2007				
4	EJ7	0223071006	2/23/2007				
5	HGBL1	0223071007	2/23/2007				
6	HGBL1A	0223071008	2/23/2007				
7	HGBL1B	0223071009	2/23/2007				
8	HGBL2	0223071010	2/23/2007				
9	HGBL3	0223071011	2/23/2007				
10	HJ7	0223071012	2/23/2007				
11	PB1	0223071013	2/23/2007				
12	PB2	0223071014	2/23/2007				
13	PB3	0223071015	2/23/2007				
14	PB4	0223071016	2/23/2007				
15	PB5	0223071017	2/23/2007				
16	PB6	0223071018	2/23/2007				
17	PB7	0223071019	2/23/2007				
18	PB8	0223071020	2/23/2007				
19	T01	0223071021	2/23/2007				
20	T02	0223071022	2/23/2007				
21	T03	0223071023	2/23/2007				
22	T04	0223071024	2/23/2007				
23	T05	0223071025	2/23/2007				
24	T06	0223071026	2/23/2007				
25	T07	0223071027	2/23/2007				
26	T08	0223071028	2/23/2007				
27	T09	0223071029	2/23/2007				
28	T09	0223071030	2/23/2007				
29	T10	0223071031	2/23/2007				
30	T11	0223071032	2/23/2007				
31	T12	0223071033	2/23/2007				
32	T13	0223071034	2/23/2007				
33	T14	0223071035	2/23/2007				
34	T15	0223071036	2/23/2007				
35	T16	0223071037	2/23/2007				
36	T17	0223071038	2/23/2007				
37	T18	0223071039	2/23/2007				
38	T19	0223071040	2/23/2007				
39	T20	0223071041	2/23/2007				
40	T21	0223071042	2/23/2007				

LATERAL TOE-NAIL DETAIL

ST-TOENAIL

MITek Industries, Chesterfield, MO

NOTES:
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 30 DEGREES WITH THE MEMBER AND STARTED 1/3 THE LENGTH OF THE NAIL FROM THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE BOTTOM CHORD SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

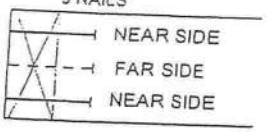
TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP
3.5" LONG	.131	83.3
	.135	89.6
	.162	118.3
3.25" LONG	.128	80.5
	.131	83.3
	.148	102.1
3.0" LONG	.120	70.5
	.128	80.5
	.131	83.3
	.148	102.1

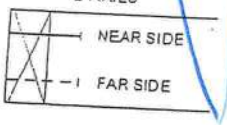
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

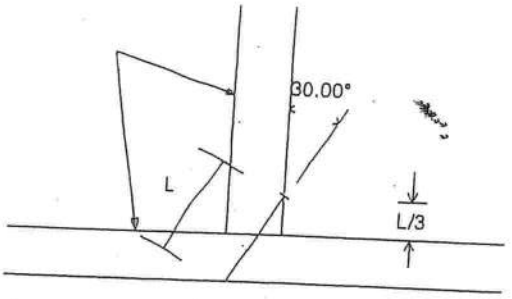
SQUARE CUT

SIDE VIEW (2x4, 2x6) 3 NAILS

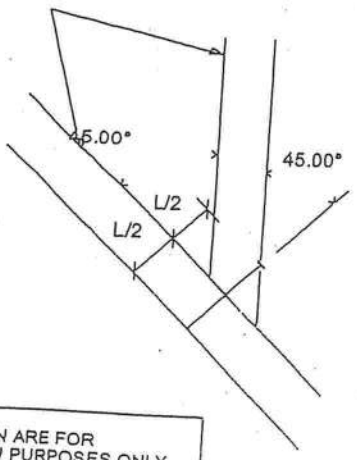


SIDE VIEW (2x3) 2 NAILS

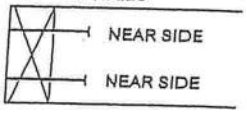




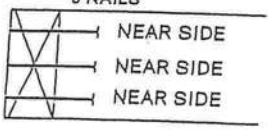
45 DEGREE ANGLE BEVEL CUT



SIDE VIEW (2x3, 2x4) 2 NAILS



SIDE VIEW (2x6) 3 NAILS



This detail may only be applied to Pre-engineered truss drawings signed and sealed by Structural Engineering and Inspections Inc.

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

FEB 23 2007

The seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any particular building design is the responsibility of the building designer.

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Licensee Information

Name:	**REED, LARRY DON (Primary Name)** **DON REED CONSTRUCTION INC (DBA Name)**
Main Address:	**2230 SE BAYA DRIVE SUITE 101** **LAKE CITY Florida 32025**
County:	**COLUMBIA**
License Mailing:	
LicenseLocation:	**2230 E BAYA AVE STE 101** **LAKE CITY FL 32025**
County:	**COLUMBIA**

License Information

License Type:	**Certified General Contractor**
Rank:	**Cert General**
License Number:	**CGC036224**
Status:	**Current,Active**
Licensure Date:	**03/08/1986**
Expires:	**08/31/2008**

Special Qualifications	**Qualification Effective**
Bldg Code Core Course Credit	
Qualified Business License Required	**08/13/2004**

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FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0223071003

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	CJ1	JACK	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:14:08 2007 Page 1

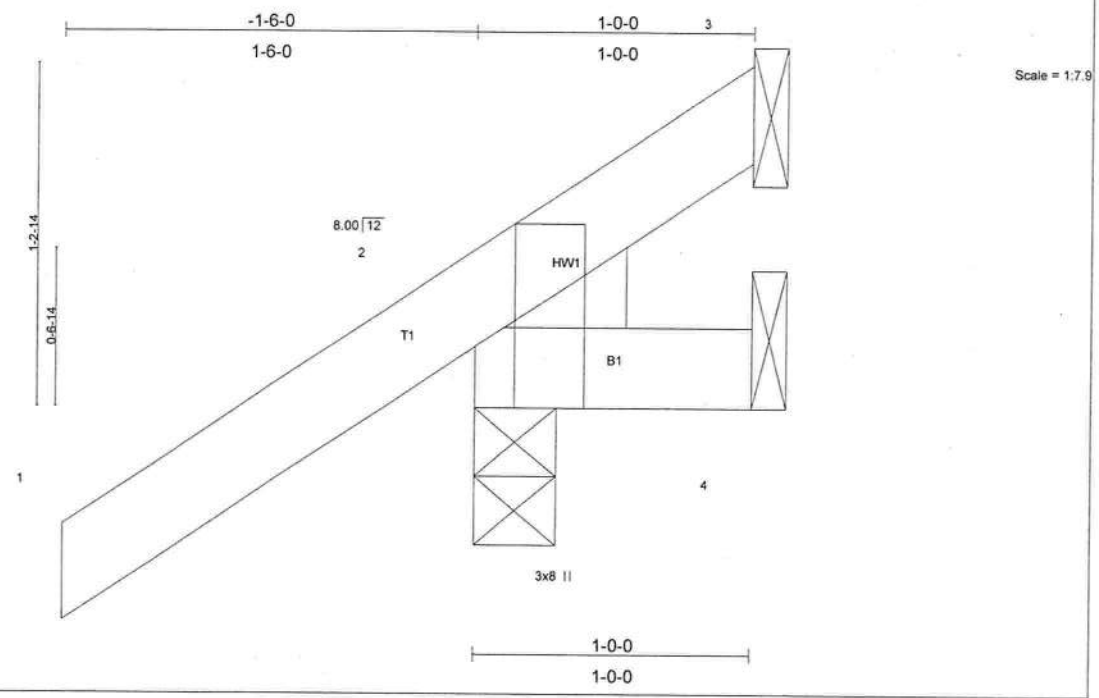


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=-40/Mechanical
Max Horz 2=90(load case 5)
Max Uplift2=-181(load case 5), 4=-11(load case 3), 3=-40(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=49(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-63/40
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 2, 11 lb uplift at joint 4 and 40 lb uplift at joint 3.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071004

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	CJ3	JACK	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:14:28 2007 Page 1

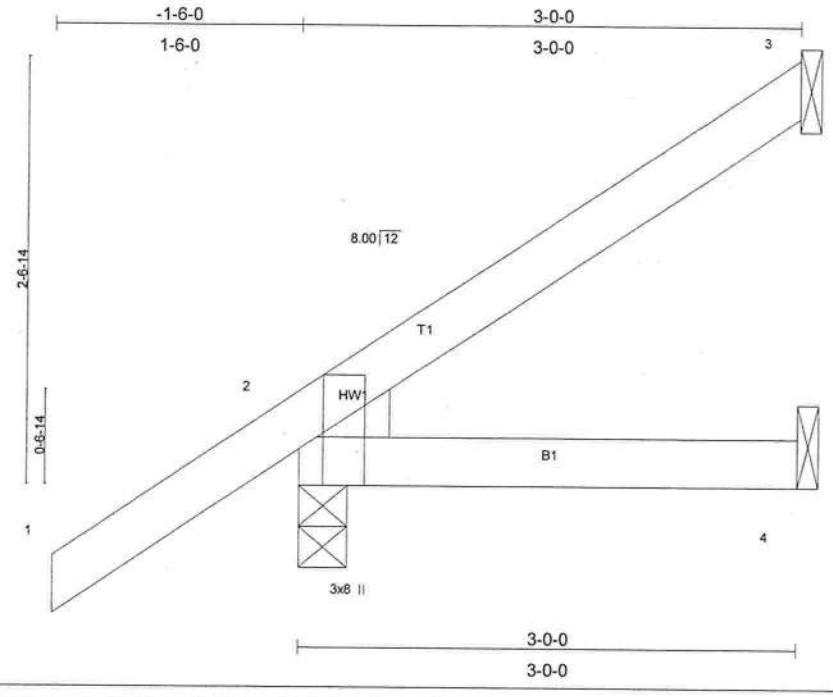


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=150(load case 5)
Max Uplift3=-59(load case 5), 2=-165(load case 5), 4=-33(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-63/21
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 3, 165 lb uplift at joint 2 and 33 lb uplift at joint 4.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071005

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	CJ5	JACK	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:14:48 2007 Page 1

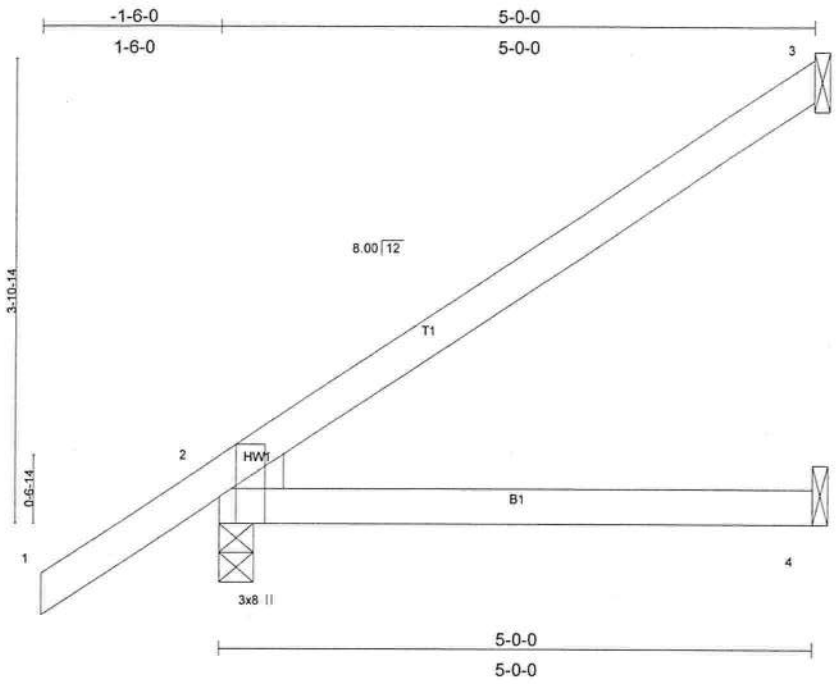


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.09	2-4	>663	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>774	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 20 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=212(load case 5)
Max Uplift3=-131(load case 5), 2=-188(load case 5), 4=-56(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-111/52
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 lb uplift at joint 3, 188 lb uplift at joint 2 and 56 lb uplift at joint 4.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071006

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	EJ7	MONO TRUSS	41	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:15:10 2007 Page 1

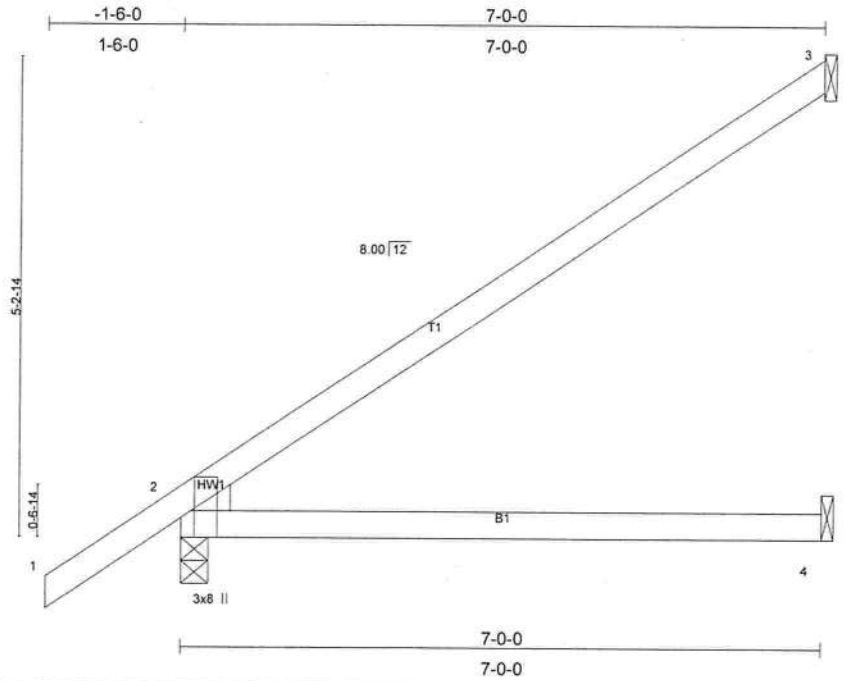


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.33	2-4	>245	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	0.28	2-4	>293	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 27 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=165/Mechanical, 2=385/0-3-8, 4=110/Mechanical
Max Horz 2=273(load case 5)
Max Uplift3=-181(load case 5), 2=-218(load case 5), 4=-79(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-139/75
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 3, 218 lb uplift at joint 2 and 79 lb uplift at joint 4.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ. FL 33549

Dwg.#0223071007

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	HGBL1	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 16:44:12 2007 Page 1

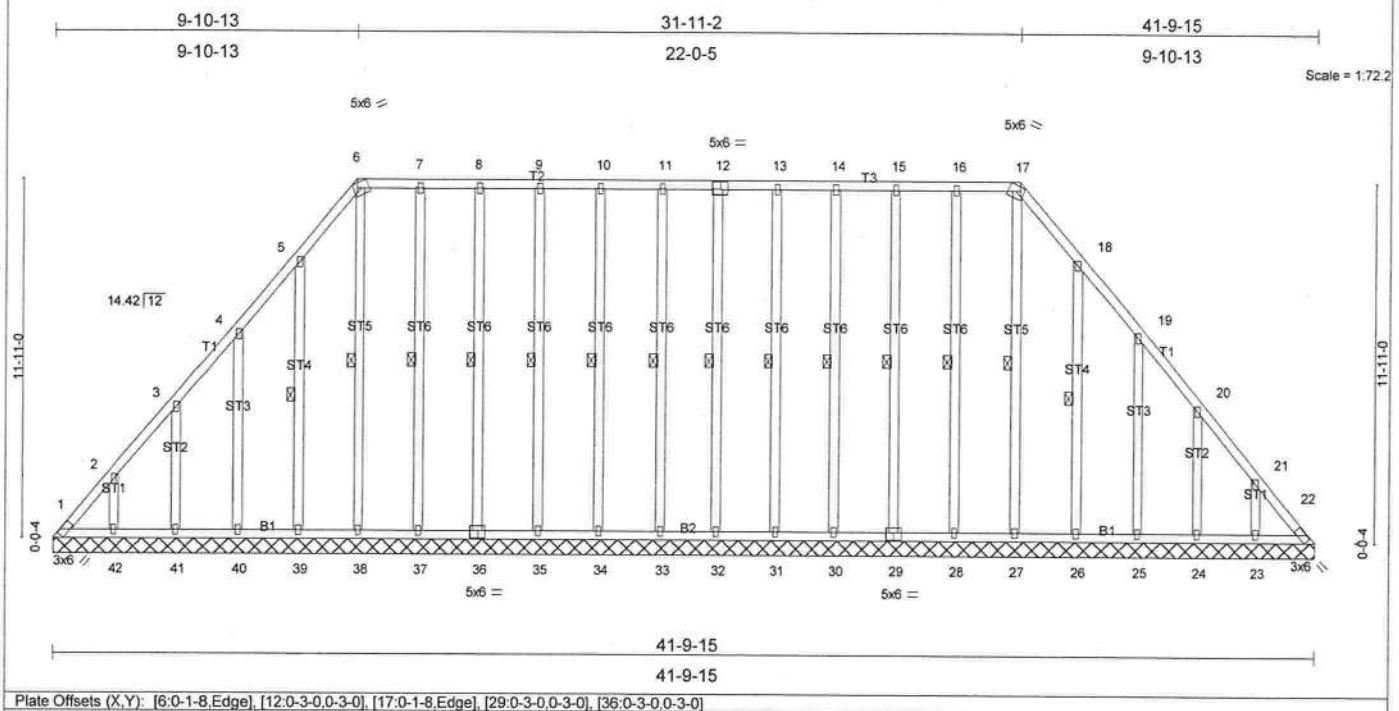


Plate Offsets (X,Y): [6:0-1-8,Edge], [12:0-3-0,0-3-0], [17:0-1-8,Edge], [29:0-3-0,0-3-0], [36:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.02	22	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 407 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 18-26, 17-27, 16-28, 15-29, 14-30, 13-31, 12-32, 5-39, 6-38, 7-37, 8-36, 9-35, 10-34, 11-33

REACTIONS (lb/size) 1=79/41-9-15, 22=79/41-9-15, 23=174/41-9-15, 24=167/41-9-15, 25=170/41-9-15, 26=163/41-9-15, 27=145/41-9-15, 28=170/41-9-15, 29=168/41-9-15, 30=168/41-9-15, 31=167/41-9-15, 32=159/41-9-15, 42=174/41-9-15, 41=167/41-9-15, 40=170/41-9-15, 39=163/41-9-15, 38=146/41-9-15, 37=170/41-9-15, 36=168/41-9-15, 35=168/41-9-15, 34=169/41-9-15, 33=162/41-9-15
Max Horz 1=-424(load case 3)
Max Uplift1=-219(load case 3), 22=-97(load case 4), 23=-203(load case 6), 24=-193(load case 6), 25=-206(load case 6), 26=-179(load case 6), 28=-86(load case 4), 29=-83(load case 3), 30=-80(load case 4), 31=-78(load case 4), 32=-76(load case 3), 42=-203(load case 5), 41=-194(load case 5), 40=-205(load case 5), 39=-182(load case 5), 38=-72(load case 4), 37=-79(load case 4), 36=-83(load case 3), 35=-80(load case 4), 34=-80(load case 3), 33=-78(load case 4)
Max Grav 1=354(load case 4), 22=282(load case 6), 23=174(load case 1), 24=167(load case 10), 25=170(load case 1), 26=163(load case 10), 27=150(load case 6), 28=174(load case 9), 29=168(load case 1), 30=168(load case 10), 31=167(load case 9), 32=159(load case 10), 42=174(load case 1), 41=167(load case 9), 40=170(load case 1), 39=163(load case 9), 38=183(load case 6), 37=174(load case 10), 36=168(load case 1), 35=168(load case 9), 34=169(load case 10), 33=162(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-471/306, 2-3=-364/286, 3-4=-248/262, 4-5=-132/243, 5-6=-70/326, 6-7=-20/249, 7-8=-19/249, 8-9=-19/249, 9-10=-19/249, 10-11=-19/249, 11-12=-19/249, 12-13=-20/249, 13-14=-20/249, 14-15=-20/249, 15-16=-20/249, 16-17=-21/249, 17-18=-71/303, 18-19=-75/158, 19-20=-88/104, 20-21=-223/127, 21-22=-387/148
BOT CHORD 1-42=-96/288, 41-42=-96/288, 40-41=-96/288, 39-40=-96/288, 38-39=-96/288, 37-38=-96/287, 36-37=-96/287, 35-36=-96/287, 34-35=-96/287, 33-34=-96/287, 32-33=-96/287, 31-32=-96/287, 30-31=-96/287, 29-30=-96/287, 28-29=-96/287, 27-28=-96/287, 26-27=-96/288, 25-26=-96/288, 24-25=-96/288, 23-24=-96/288, 22-23=-96/288
WEBS 21-23=-107/199, 20-24=-108/209, 19-25=-109/217, 18-26=-103/191, 17-27=-138/0, 16-28=-114/98, 15-29=-108/95, 14-30=-108/92, 13-31=-106/90, 12-32=-102/87, 2-42=-107/199, 3-41=-108/209, 4-40=-109/216, 5-39=-103/194, 6-38=-171/84, 7-37=-114/91, 8-36=-108/95, 9-35=-108/92, 10-34=-109/92, 11-33=-105/90

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 lb uplift at joint 1, 97 lb uplift at joint 22, 203 lb uplift at joint 23, 193 lb uplift at joint 24, 206 lb uplift at joint 25, 179 lb uplift at joint 26, 86 lb uplift at joint 28, 83 lb uplift at joint 29, 80 lb uplift at joint 30, 78 lb uplift at joint 31, 76 lb uplift at joint 32, 203 lb uplift at joint 42, 194 lb uplift at joint 41, 205 lb uplift at joint 40, 182 lb uplift at joint 39, 72 lb uplift at joint 38, 79 lb uplift at joint 37, 83 lb uplift at joint 36, 80 lb uplift at joint 35, 80 lb uplift at joint 34 and 78 lb uplift at joint 33.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071008

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	HGBL1A	GABLE	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 16:44:25 2007 Page 1

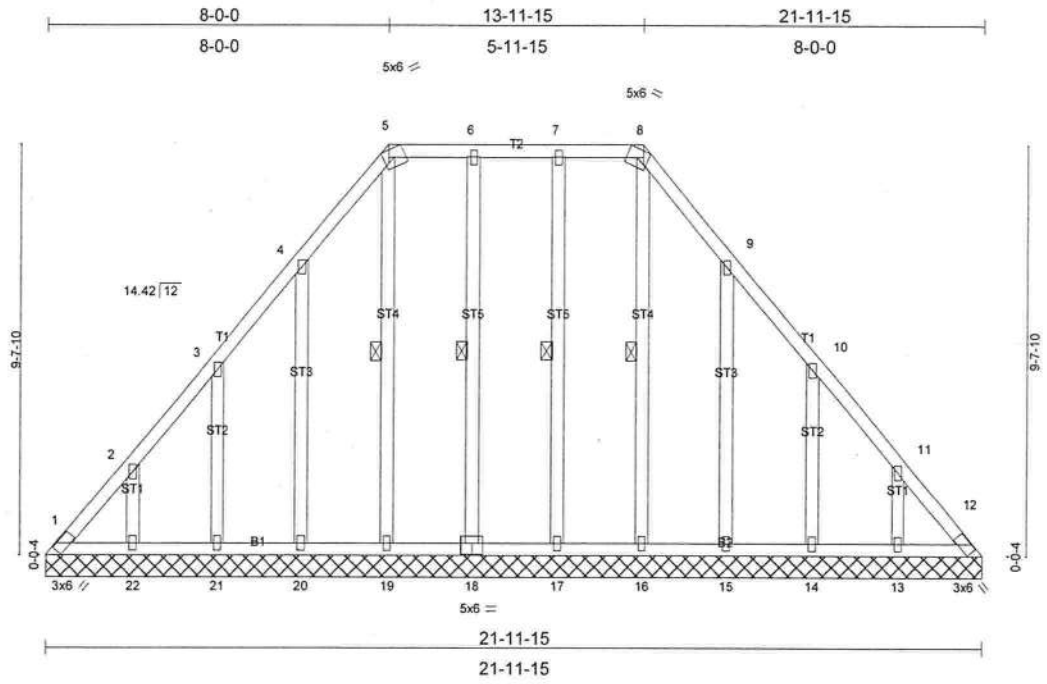


Plate Offsets (X,Y): [5:0-1-8,Edge], [8:0-1-8,Edge], [18:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.08	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 170 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 8-16, 7-17, 5-19, 6-18

REACTIONS (lb/size) 1=84/21-11-15, 12=84/21-11-15, 13=173/21-11-15, 14=168/21-11-15, 15=167/21-11-15, 16=140/21-11-15, 17=166/21-11-15, 22=173/21-11-15, 21=168/21-11-15, 20=167/21-11-15, 19=140/21-11-15, 18=166/21-11-15
Max Horz 1=-340(load case 3)
Max Uplift1=-166(load case 3), 12=-90(load case 4), 13=-202(load case 6), 14=-200(load case 6), 15=-193(load case 6), 17=-85(load case 4), 22=-202(load case 5), 21=-199(load case 5), 20=-195(load case 5), 19=-64(load case 4), 18=-80(load case 3)
Max Grav 1=261(load case 4), 12=211(load case 6), 13=174(load case 10), 14=168(load case 1), 15=168(load case 10), 16=145(load case 10), 17=171(load case 9), 22=174(load case 9), 21=168(load case 1), 20=168(load case 9), 19=145(load case 9), 18=171(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-350/238, 2-3=-243/217, 3-4=-126/196, 4-5=-80/254, 5-6=-26/209, 6-7=-25/209, 7-8=-26/209, 8-9=-80/246, 9-10=-80/100, 10-11=-143/117, 11-12=-296/139
BOT CHORD 1-22=-90/229, 21-22=-90/229, 20-21=-90/229, 19-20=-90/229, 18-19=-90/229, 17-18=-90/229, 16-17=-90/229, 15-16=-90/229, 14-15=-90/229, 13-14=-90/229, 12-13=-90/229
WEBS 11-13=-107/198, 10-14=-109/215, 9-15=-107/204, 8-16=-95/6, 7-17=-111/97, 2-22=-107/198, 3-21=-109/214, 4-20=-107/206, 5-19=-122/76, 6-18=-111/92

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 166 lb uplift at joint 1, 90 lb uplift at joint 12, 202 lb uplift at joint 13, 200 lb uplift at joint 14, 193 lb uplift at joint 15, 85 lb uplift at joint 17, 202 lb uplift at joint 22, 199 lb uplift at joint 21, 195 lb uplift at joint 20, 64 lb uplift at joint 19 and 80 lb uplift at joint 18.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071009

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	HGBL1B	GABLE	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 16:44:34 2007 Page 1

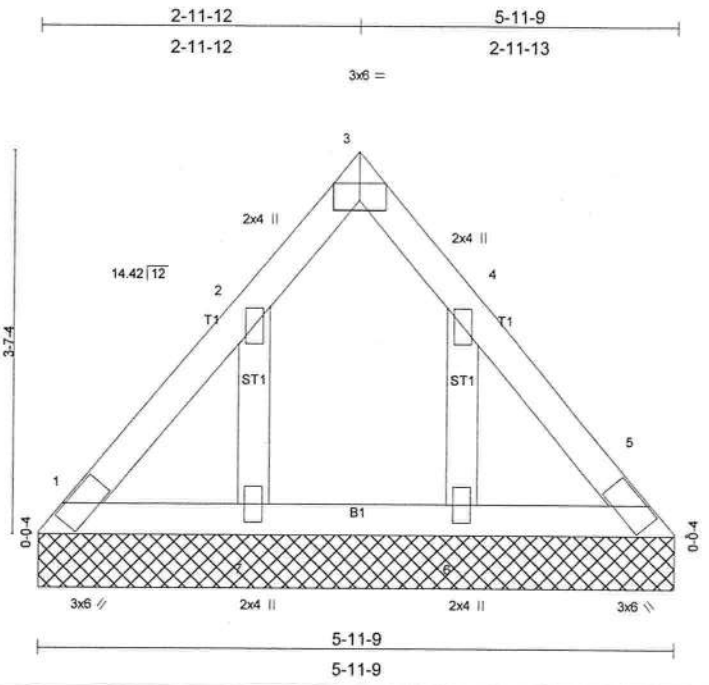


Plate Offsets (X,Y): [3:Edge,0-1-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.05	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.02	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 27 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=71/5-11-9, 5=71/5-11-9, 6=155/5-11-9, 7=155/5-11-9
Max Horz 1=-118(load case 3)
Max Uplift1=-11(load case 3), 5=-9(load case 4), 6=-162(load case 6), 7=-163(load case 5)
Max Grav 1=82(load case 5), 5=81(load case 6), 6=155(load case 1), 7=155(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-165/39, 2-3=-48/19, 3-4=-48/19, 4-5=-165/37
BOT CHORD 1-7=-24/154, 6-7=-24/154, 5-6=-24/154
WEBS 4-6=-94/219, 2-7=-94/219

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint 1, 9 lb uplift at joint 5, 162 lb uplift at joint 6 and 163 lb uplift at joint 7.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071010

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	HGBL2	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 16:44:46 2007 Page 1

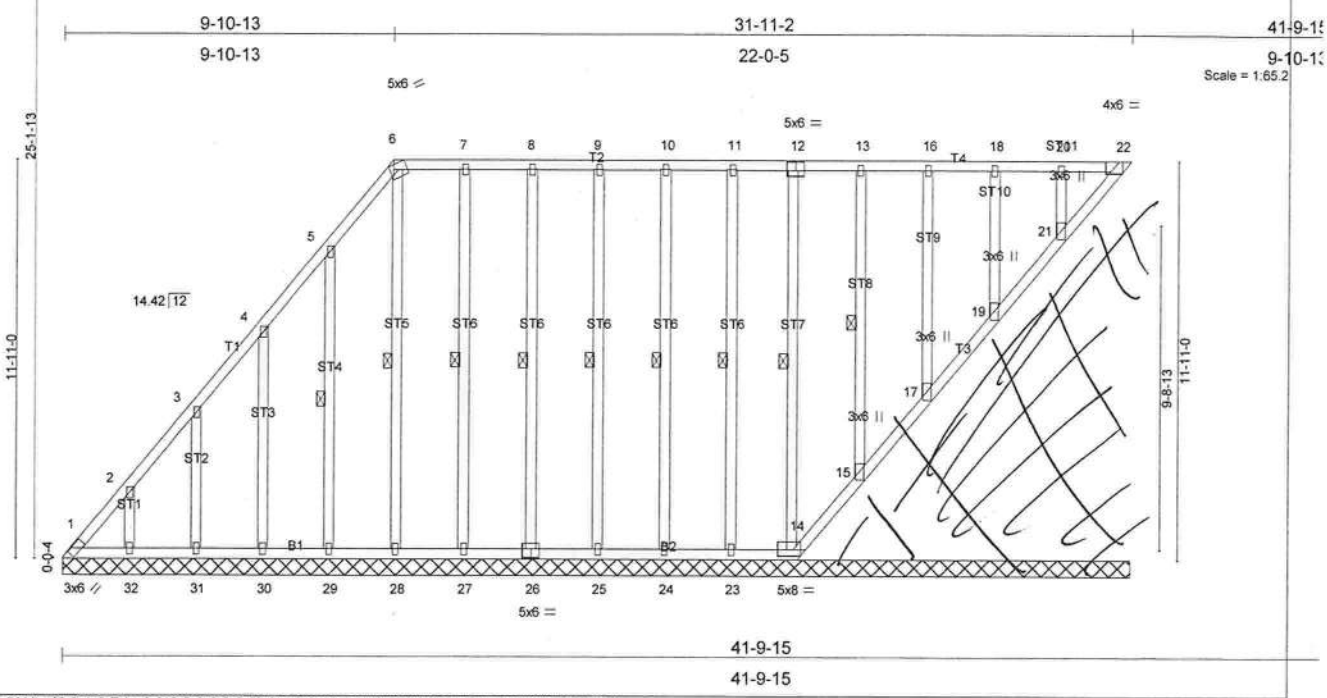


Plate Offsets (X,Y): [6:0-1-8,Edge], [12:0-3-0,0-3-0], [14:0-2-12,0-2-0], [22:0-1-14,Edge], [26:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.07	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.16	Horz(TL)	-0.03	21	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 294 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-0-15 oc bracing.
WEBS 1 Row at midpt 13-15, 12-14, 5-29, 6-28, 7-27, 8-26, 9-25, 10-24, 11-23

REACTIONS (lb/size) 1=71/31-11-2, 21=398/31-11-2, 19=371/31-11-2, 17=378/31-11-2, 15=377/31-11-2, 14=243/31-11-2, 32=174/31-11-2, 31=167/31-11-2, 30=169/31-11-2, 29=164/31-11-2, 28=152/31-11-2, 27=171/31-11-2, 26=168/31-11-2, 25=167/31-11-2, 24=171/31-11-2, 23=153/31-11-2, 22=145/31-11-2
Max Horz 1=986(load case 5)
Max Uplift1=-49(load case 3), 21=-246(load case 5), 19=-231(load case 6), 17=-234(load case 5), 15=-234(load case 6), 14=-73(load case 3), 32=-203(load case 5), 31=-195(load case 5), 30=-196(load case 5), 29=-201(load case 5), 28=-165(load case 5), 27=-91(load case 3), 26=-81(load case 4), 25=-80(load case 3), 24=-80(load case 4), 23=-77(load case 3), 22=-101(load case 4)
Max Grav 1=635(load case 5), 21=398(load case 1), 19=371(load case 10), 17=378(load case 1), 15=377(load case 10), 14=350(load case 5), 32=174(load case 1), 31=167(load case 1), 30=169(load case 1), 29=164(load case 1), 28=152(load case 1), 27=175(load case 10), 26=168(load case 1), 25=167(load case 1), 24=171(load case 1), 23=153(load case 10), 22=145(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-847/86, 2-3=-699/64, 3-4=-537/41, 4-5=-380/19, 5-6=-215/34, 6-7=-89/55, 7-8=-88/56, 8-9=-88/56, 9-10=-88/56, 10-11=-88/56, 11-12=-88/56, 12-13=-88/56, 13-16=-88/56, 16-18=-88/56, 18-20=-88/56, 20-22=-88/56, 14-15=-678/92, 15-17=-494/92, 17-19=-310/91, 19-21=-135/93, 21-22=-93/72
BOT CHORD 1-32=-484/14, 31-32=-484/14, 30-31=-484/14, 29-30=-484/14, 28-29=-484/14, 27-28=-482/13, 26-27=-482/13, 25-26=-482/13, 24-25=-482/13, 23-24=-482/13, 14-23=-482/13
WEBS 20-21=-202/108, 18-19=-185/107, 16-17=-189/108, 13-15=-188/107, 12-14=-139/94, 2-32=-107/196, 3-31=-108/212, 4-30=-109/207, 5-29=-104/213, 6-28=-92/228, 7-27=-115/103, 8-26=-108/93, 9-25=-108/92, 10-24=-108/92, 11-23=-103/89

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) Bearing at joint(s) 21, 19, 17, 15, 22 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 1, 246 lb uplift at joint 21, 231 lb uplift at joint 19, 234 lb uplift at joint 17, 234 lb uplift at joint 15, 73 lb uplift at joint 14, 203 lb uplift at joint 32, 195 lb uplift at joint 31, 196 lb uplift at joint 30, 201 lb uplift at joint 29, 165 lb uplift at joint 28, 91 lb uplift at joint 27, 81 lb uplift at joint 26, 80 lb uplift at joint 25, 80 lb uplift at joint 24, 77 lb uplift at joint 23 and 101 lb uplift at joint 22.
9) Non Standard bearing condition. Review required.
10) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

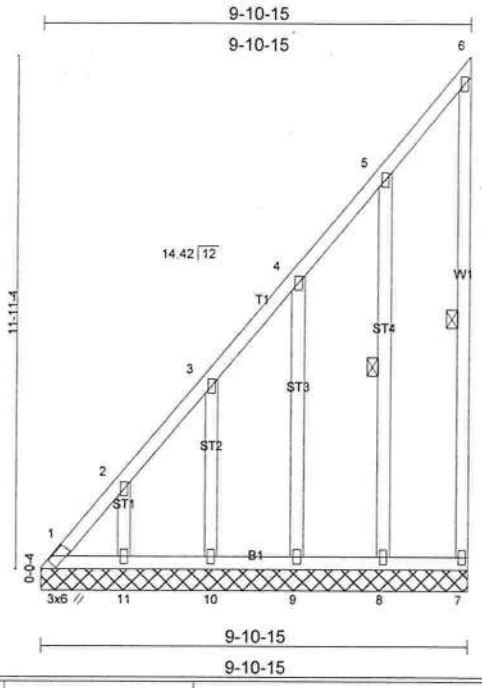
LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071011

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	HGBL3	GABLE	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 16:44:57 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.05	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	NO	WB	0.09	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 86 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-7, 5-8

REACTIONS (lb/size) 1=52/9-10-15, 7=66/9-10-15, 8=173/9-10-15, 9=168/9-10-15, 10=168/9-10-15, 11=169/9-10-15
Max Horz 1=525(load case 5)
Max Uplift1=-64(load case 3), 7=-82(load case 5), 8=-196(load case 5), 9=-199(load case 5), 10=-196(load case 5), 11=-198(load case 5)
Max Grav 1=570(load case 5), 7=66(load case 1), 8=173(load case 1), 9=168(load case 1), 10=168(load case 1), 11=169(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-848/105, 2-3=-671/84, 3-4=-474/61, 4-5=-280/40, 5-6=-82/32, 6-7=-42/93
BOT CHORD 1-11=-1/1, 10-11=-1/1, 9-10=-1/1, 8-9=-1/1, 7-8=-1/1
WEBS 5-8=-111/241, 4-9=-108/235, 3-10=-109/239, 2-11=-104/217

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) All plates are 2x4 MT20 unless otherwise indicated.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 1, 82 lb uplift at joint 7, 196 lb uplift at joint 8, 199 lb uplift at joint 9, 196 lb uplift at joint 10 and 198 lb uplift at joint 11.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071012

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	HJ7	MONO TRUSS	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:13:36 2007 Page 1

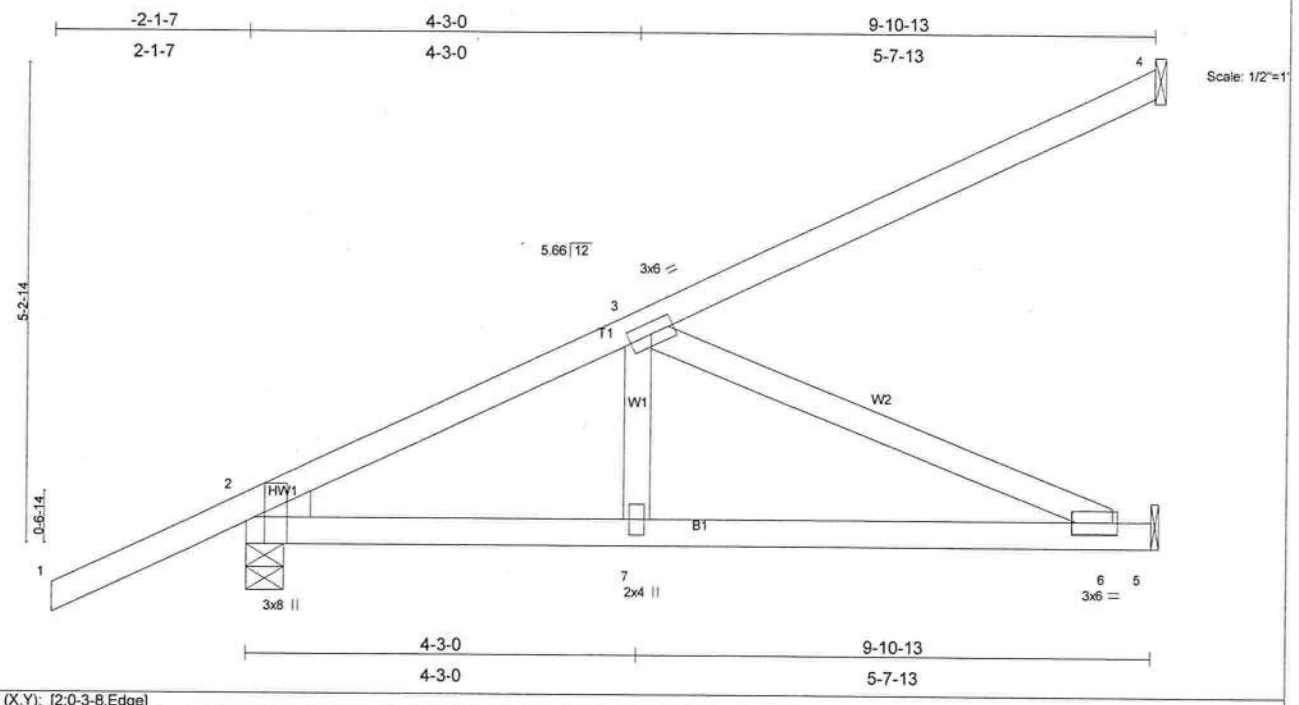


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.10	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.17	6-7	>668	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.39	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 47 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-7-12 oc bracing.

REACTIONS (lb/size) 4=266/Mechanical, 2=484/0-4-15, 5=391/Mechanical
Max Horz 2=332(load case 4)
Max Uplift4=-267(load case 4), 2=-294(load case 4), 5=-225(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-694/225, 3-4=-149/83
BOT CHORD 2-7=-481/602, 6-7=-481/602, 5-6=0/0
WEBS 3-7=-99/216, 3-6=-657/524

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 4, 294 lb uplift at joint 2 and 225 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071013

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	PB1	VALLEY	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:16:14 2007 Page 1

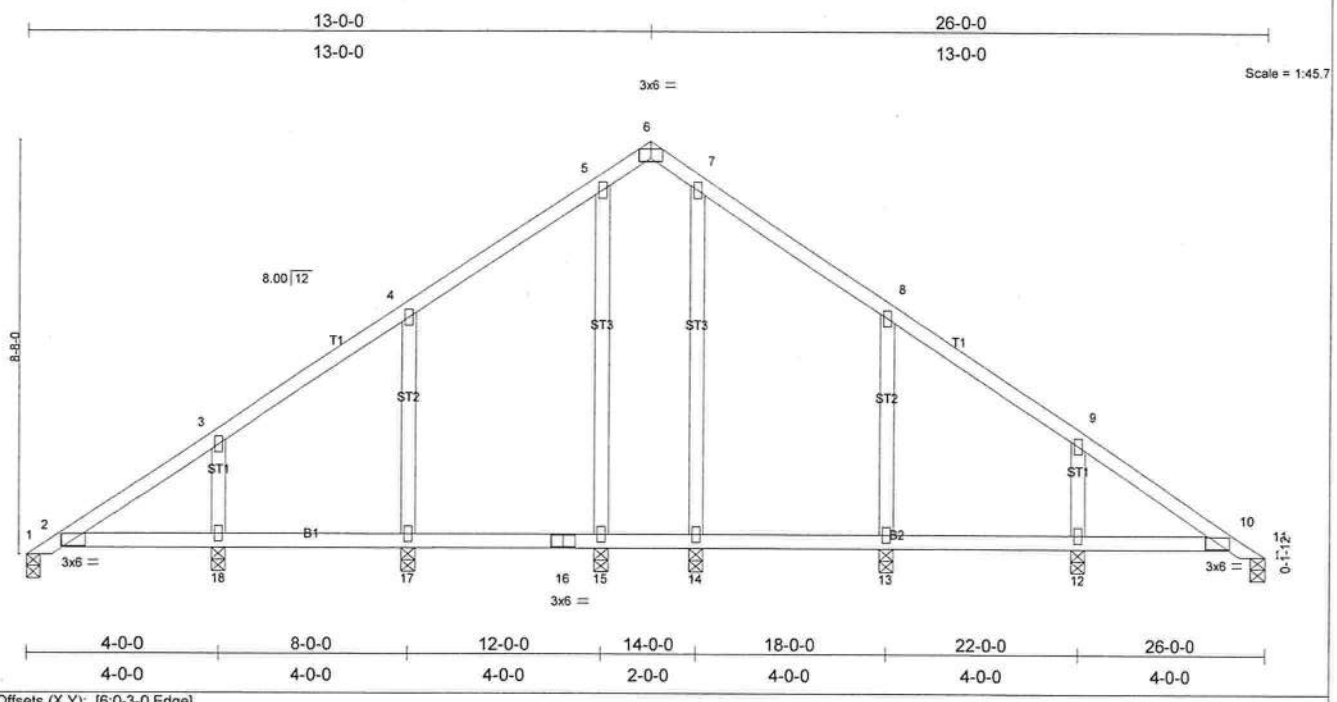


Plate Offsets (X,Y): [6:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.01	2-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	2-18	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	0.01	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 124 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=81/0-3-8, 11=81/0-3-8, 12=358/0-3-8, 13=341/0-3-8, 14=288/0-3-8, 18=358/0-3-8, 17=341/0-3-8, 15=288/0-3-8
Max Horz 1=-298(load case 3)
Max Uplift1=-82(load case 3), 12=-190(load case 6), 13=-231(load case 6), 14=-39(load case 6), 18=-202(load case 5), 17=-226(load case 5), 15=-96(load case 4)
Max Grav 1=132(load case 4), 11=90(load case 10), 12=360(load case 10), 13=341(load case 1), 14=288(load case 1), 18=360(load case 9), 17=341(load case 1), 15=288(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-323/303, 2-3=-274/179, 3-4=-178/158, 4-5=-76/149, 5-6=0/131, 6-7=-7/136, 7-8=-4/137, 8-9=-80/139, 9-10=-213/143, 10-11=-44/0
BOT CHORD 2-18=-66/235, 17-18=-66/235, 16-17=-66/235, 15-16=-66/235, 14-15=-66/235, 13-14=-66/235, 12-13=-66/235, 10-12=-66/235
WEBS 9-12=-222/211, 8-13=-221/257, 7-14=-202/56, 3-18=-222/217, 4-17=-221/253, 5-15=-202/112

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) Bearing at joint(s) 1, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 82 lb uplift at joint 1, 190 lb uplift at joint 12, 231 lb uplift at joint 13, 39 lb uplift at joint 14, 202 lb uplift at joint 18, 226 lb uplift at joint 17 and 96 lb uplift at joint 15.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071014

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	PB2	VALLEY	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:17:11 2007 Page 1

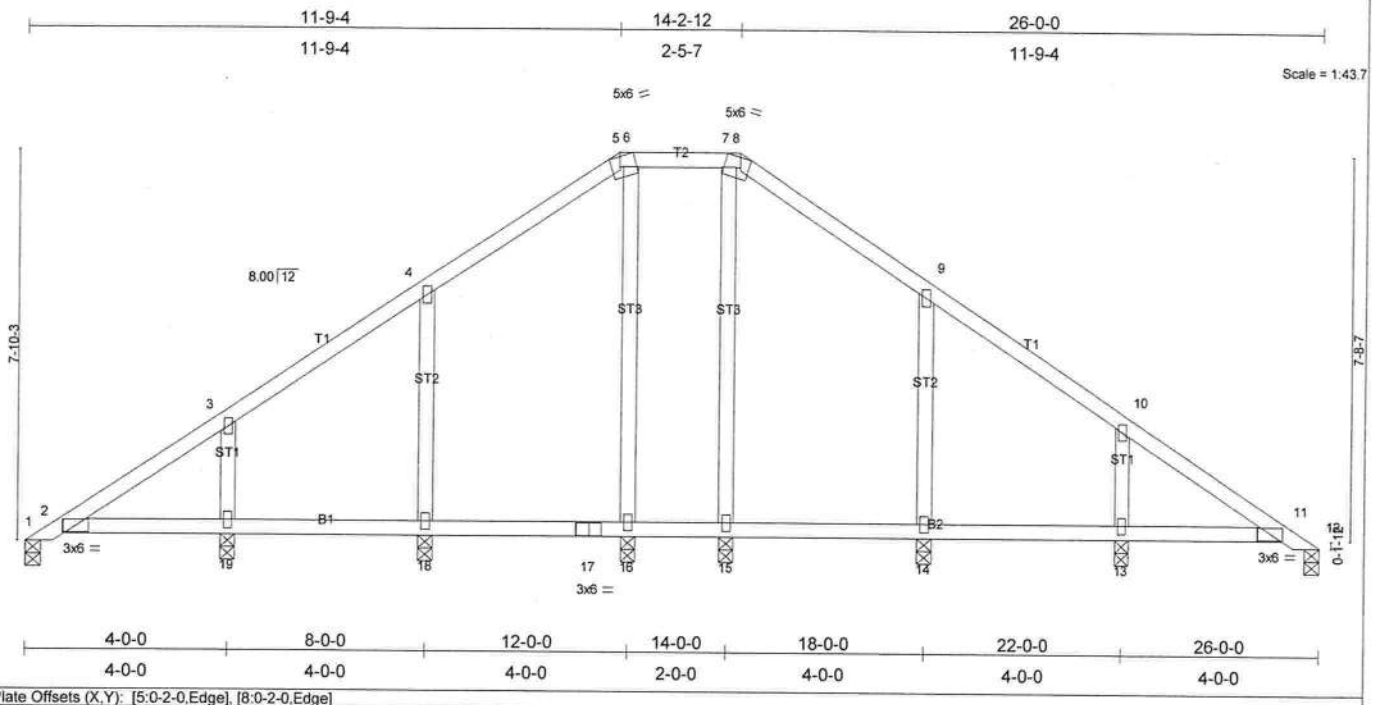


Plate Offsets (X,Y): [5:0-2-0,Edge], [8:0-2-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.01	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 123 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=93/0-3-8, 13=361/0-3-8, 14=344/0-3-8, 15=270/0-3-8, 19=361/0-3-8, 18=344/0-3-8, 16=270/0-3-8, 12=93/0-3-8
Max Horz 1=-270(load case 3)
Max Uplift1=-92(load case 3), 13=-209(load case 6), 14=-212(load case 6), 15=-38(load case 3), 19=-217(load case 5), 18=-212(load case 5), 16=-119(load case 4), 12=-3(load case 5)
Max Grav 1=110(load case 4), 13=362(load case 10), 14=344(load case 1), 15=287(load case 10), 19=362(load case 9), 18=344(load case 1), 16=287(load case 9), 12=98(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-288/284, 2-3=-234/208, 3-4=-137/191, 4-5=-36/224, 5-6=0/217, 6-7=0/217, 7-8=0/217, 8-9=-21/205, 9-10=-18/112, 10-11=-105/116, 11-12=-47/6
BOT CHORD 2-19=-44/142, 18-19=-44/142, 17-18=-44/142, 16-17=-44/142, 15-16=-44/142, 14-15=-44/142, 13-14=-44/142, 11-13=-44/142
WEBS 10-13=-222/222, 9-14=-224/240, 7-15=-200/55, 3-19=-222/226, 4-18=-224/240, 6-16=-200/134

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Bearing at joint(s) 1, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 92 lb uplift at joint 1, 209 lb uplift at joint 13, 212 lb uplift at joint 14, 38 lb uplift at joint 15, 217 lb uplift at joint 19, 212 lb uplift at joint 18, 119 lb uplift at joint 16 and 3 lb uplift at joint 12.

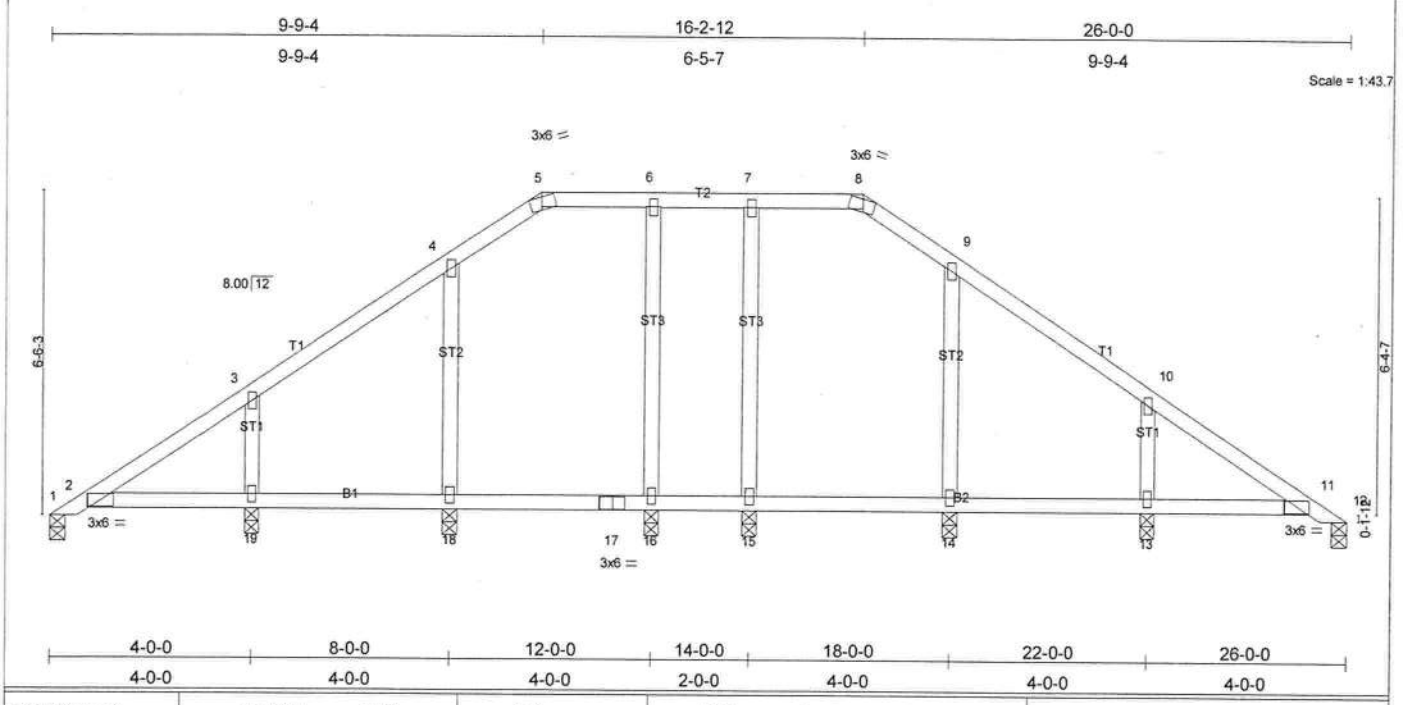
LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071015

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	PB3	VALLEY	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:18:01 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.01	2-19	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-19	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.10	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 118 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=131/0-3-8, 12=131/0-3-8, 13=375/0-3-8, 14=328/0-3-8, 15=235/0-3-8, 19=375/0-3-8, 18=328/0-3-8, 16=235/0-3-8
Max Horz 1=224(load case 4)
Max Uplift1=-65(load case 3), 12=-9(load case 4), 13=-214(load case 6), 14=-150(load case 6), 15=-87(load case 3), 19=-222(load case 5), 18=-157(load case 5), 16=-105(load case 4)
Max Grav 1=133(load case 9), 12=133(load case 10), 13=376(load case 10), 14=328(load case 1), 15=237(load case 10), 19=376(load case 9), 18=328(load case 1), 16=237(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-225/231, 2-3=-185/161, 3-4=-100/143, 4-5=-96/116, 5-6=-32/113, 6-7=-32/113, 7-8=-32/113, 8-9=-96/114, 9-10=-100/77, 10-11=-157/91, 11-12=-65/10
BOT CHORD 2-19=-73/185, 18-19=-73/185, 17-18=-73/185, 16-17=-73/185, 15-16=-73/185, 14-15=-73/185, 13-14=-73/185, 11-13=-73/185
WEBS 10-13=-230/232, 9-14=-209/177, 7-15=-149/104, 3-19=-230/235, 4-18=-209/185, 6-16=-149/120

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Bearing at joint(s) 1, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 65 lb uplift at joint 1, 9 lb uplift at joint 12, 214 lb uplift at joint 13, 150 lb uplift at joint 14, 87 lb uplift at joint 15, 222 lb uplift at joint 19, 157 lb uplift at joint 18 and 105 lb uplift at joint 16.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071016

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	PB4	VALLEY	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:18:53 2007 Page 1

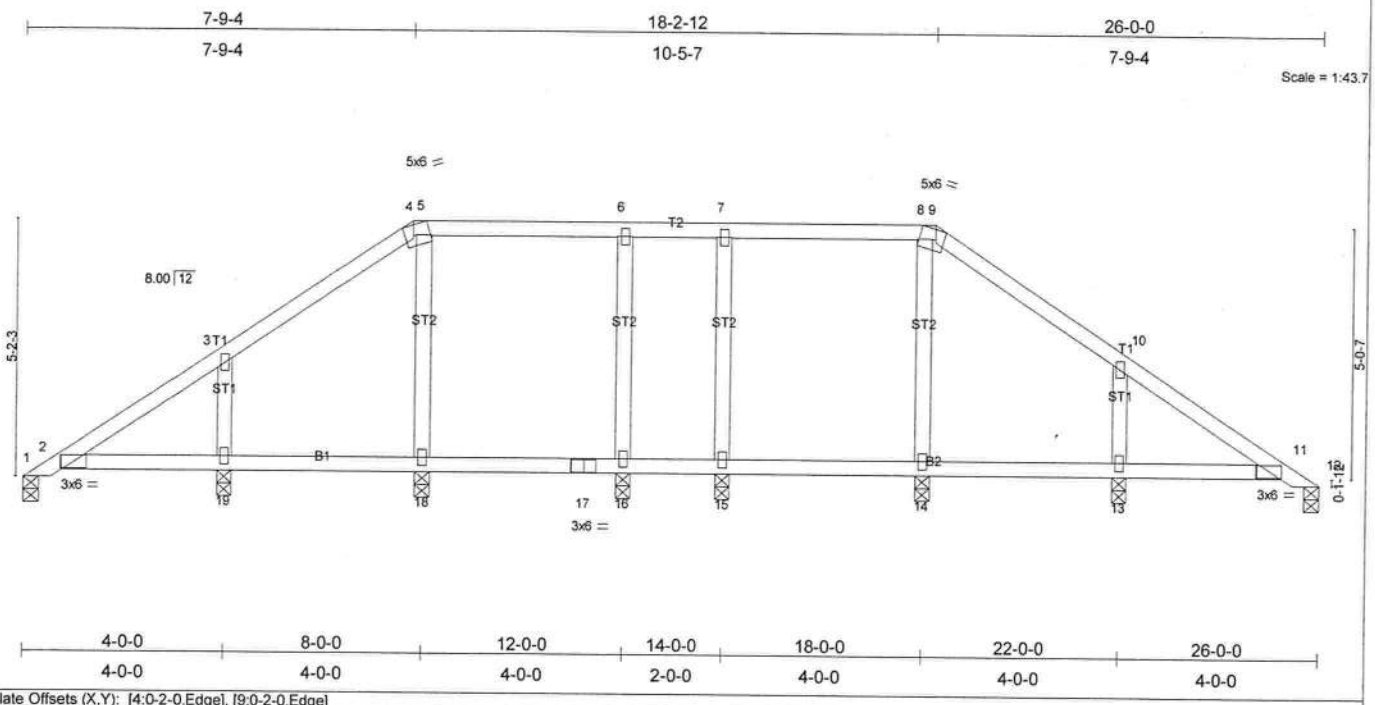


Plate Offsets (X,Y): [4:0-2-0,Edge], [9:0-2-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.01	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 112 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=81/0-3-8, 13=359/0-3-8, 14=388/0-3-8, 15=240/0-3-8, 19=359/0-3-8, 18=388/0-3-8, 16=240/0-3-8, 12=81/0-3-8
Max Horz 1=178(load case 4)
Max Uplift1=-46(load case 3), 13=-218(load case 6), 14=-99(load case 3), 15=-134(load case 4), 19=-224(load case 5), 18=-154(load case 4), 16=-135(load case 3), 12=-19(load case 6)
Max Grav 1=86(load case 9), 13=360(load case 10), 14=388(load case 1), 15=253(load case 9), 19=360(load case 9), 18=388(load case 1), 16=253(load case 10), 12=86(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-187/181, 2-3=-162/142, 3-4=-68/131, 4-5=-10/128, 5-6=-10/128, 6-7=-10/128, 7-8=-10/128, 8-9=-10/128, 9-10=-23/131, 10-11=-64/142, 11-12=-41/14
BOT CHORD 2-19=-65/108, 18-19=-65/108, 17-18=-65/108, 16-17=-65/108, 15-16=-65/108, 14-15=-65/108, 13-14=-65/108, 11-13=-65/108
WEBS 10-13=-222/228, 8-14=-267/126, 7-15=-168/152, 3-19=-222/232, 5-18=-267/183, 6-16=-168/153

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Bearing at joint(s) 1, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1, 218 lb uplift at joint 13, 99 lb uplift at joint 14, 134 lb uplift at joint 15, 224 lb uplift at joint 19, 154 lb uplift at joint 18, 135 lb uplift at joint 16 and 19 lb uplift at joint 12.

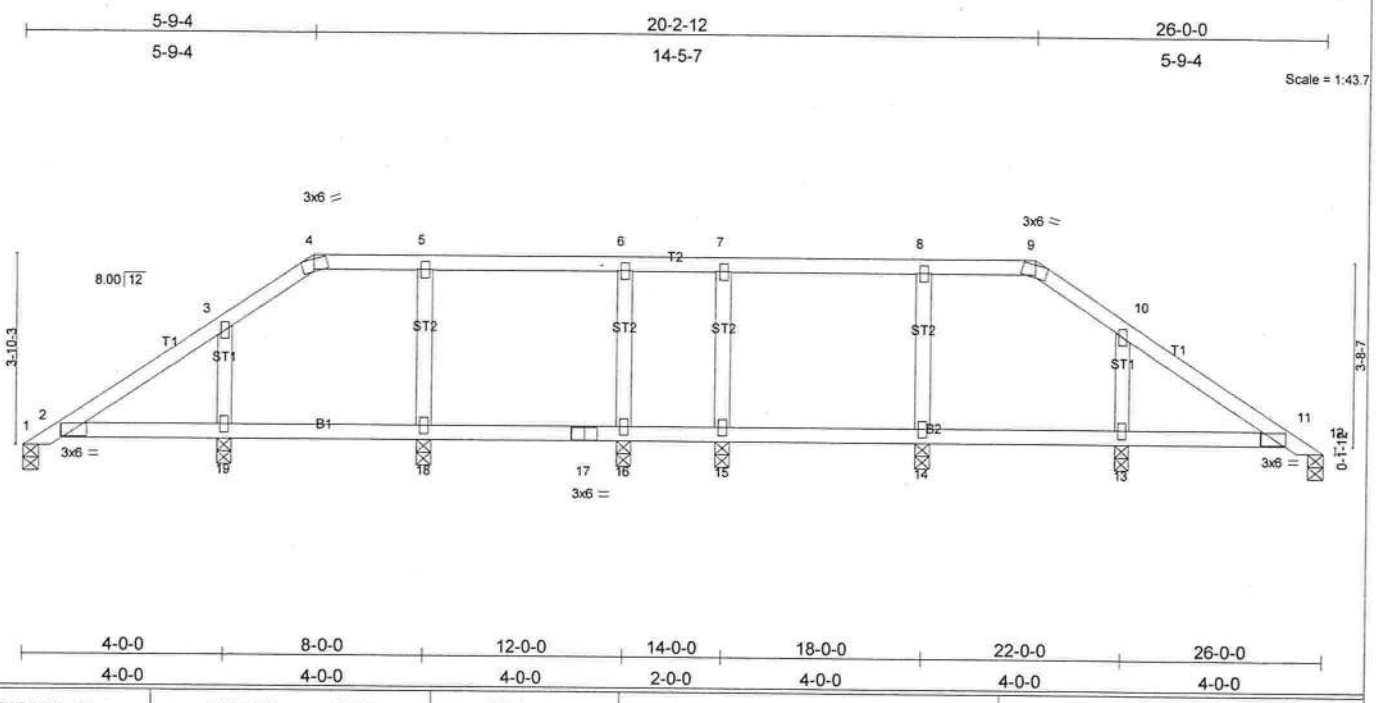
LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0223071017

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	PB5	VALLEY	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:19:52 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.01	2-19	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	2-19	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 103 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=112/0-3-8, 12=112/0-3-8, 13=369/0-3-8, 14=346/0-3-8, 15=241/0-3-8, 19=369/0-3-8, 18=346/0-3-8, 16=241/0-3-8
Max Horz 1=-131(load case 3)
Max Uplift1=-25(load case 3), 12=-20(load case 6), 13=-174(load case 6), 14=-149(load case 4), 15=-118(load case 3), 19=-186(load case 5), 18=-152(load case 4), 16=-119(load case 3)
Max Grav 1=116(load case 9), 12=116(load case 10), 13=369(load case 1), 14=346(load case 1), 15=242(load case 9), 19=369(load case 1), 18=346(load case 1), 16=242(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-123/131, 2-3=-89/95, 3-4=-59/68, 4-5=0/59, 5-6=0/59, 6-7=0/59, 7-8=0/59, 8-9=0/59, 9-10=-59/62, 10-11=-70/75, 11-12=-57/15
BOT CHORD 2-19=-38/117, 18-19=-38/117, 17-18=-38/117, 16-17=-38/117, 15-16=-38/117, 14-15=-38/117, 13-14=-38/117, 11-13=-38/117
WEBS 10-13=-226/180, 8-14=-227/173, 7-15=-157/135, 3-19=-226/189, 5-18=-227/182, 6-16=-157/137

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Bearing at joint(s) 1, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 1, 20 lb uplift at joint 12, 174 lb uplift at joint 13, 149 lb uplift at joint 14, 118 lb uplift at joint 15, 186 lb uplift at joint 19, 152 lb uplift at joint 18 and 119 lb uplift at joint 16.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071018

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	PB6	VALLEY	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:20:51 2007 Page 1

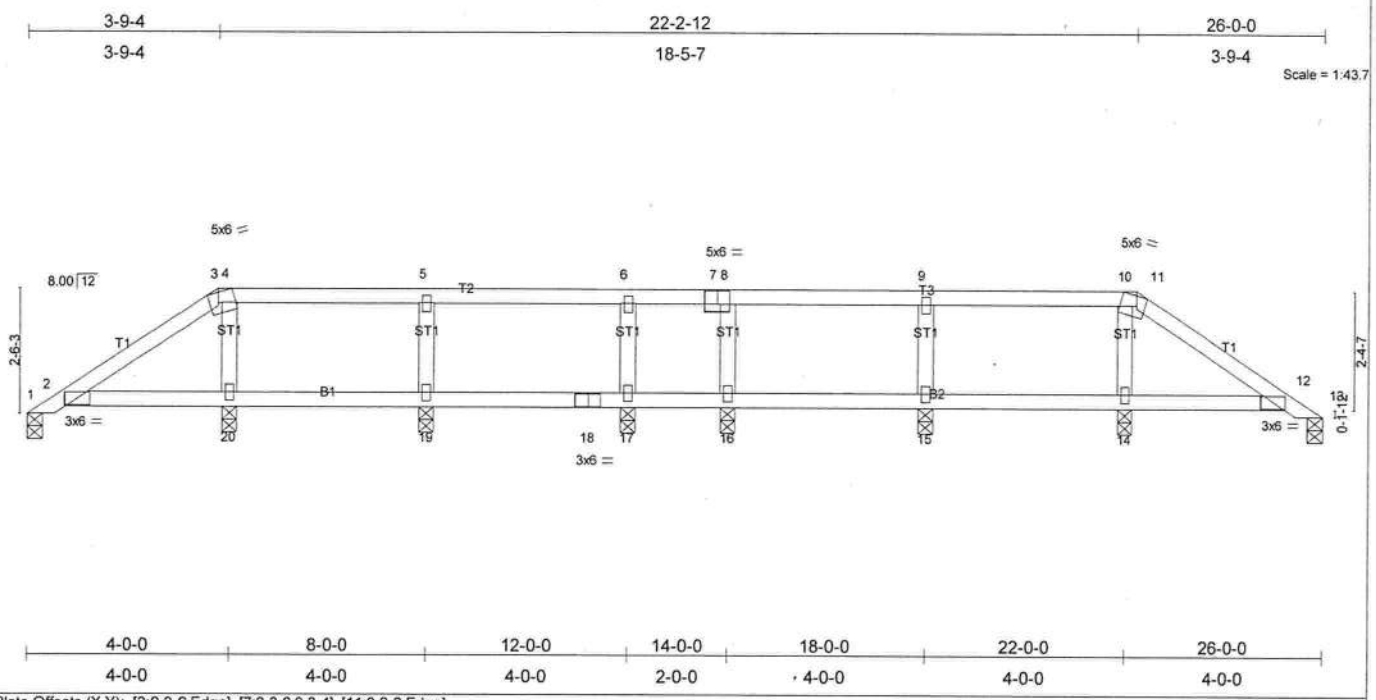


Plate Offsets (X,Y): [3:0-2-0,Edge], [7:0-3-0,0-3-4], [11:0-2-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.01	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 93 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=75/0-3-8, 13=75/0-3-8, 14=409/0-3-8, 15=343/0-3-8, 16=242/0-3-8, 20=409/0-3-8, 19=343/0-3-8, 17=242/0-3-8
Max Horz 1=85(load case 4)
Max Uplift1=-28(load case 6), 13=-38(load case 6), 14=-124(load case 6), 15=-187(load case 4), 16=-116(load case 3), 20=-158(load case 4), 19=-189(load case 3), 17=-117(load case 4)
Max Grav 1=85(load case 9), 13=85(load case 10), 14=409(load case 1), 15=360(load case 9), 16=244(load case 10), 20=409(load case 1), 19=360(load case 10), 17=244(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-78/77, 2-3=-58/151, 3-4=-5/79, 4-5=-5/79, 5-6=-5/79, 6-7=-5/79, 7-8=-5/79, 8-9=-5/79, 9-10=-5/79, 10-11=-5/79, 11-12=-34/151, 12-13=-41/24
BOT CHORD 2-20=-79/82, 19-20=-79/82, 18-19=-79/82, 17-18=-79/82, 16-17=-79/82, 15-16=-79/82, 14-15=-79/82, 12-14=-79/82
WEBS 10-14=-272/134, 9-15=-234/208, 8-16=-157/132, 4-20=-272/168, 5-19=-234/210, 6-17=-157/132

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Bearing at joint(s) 1, 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 1, 38 lb uplift at joint 13, 124 lb uplift at joint 14, 187 lb uplift at joint 15, 116 lb uplift at joint 16, 158 lb uplift at joint 20, 189 lb uplift at joint 19 and 117 lb uplift at joint 17.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0223071019

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	PB7	VALLEY	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:22:36 2007 Page 1

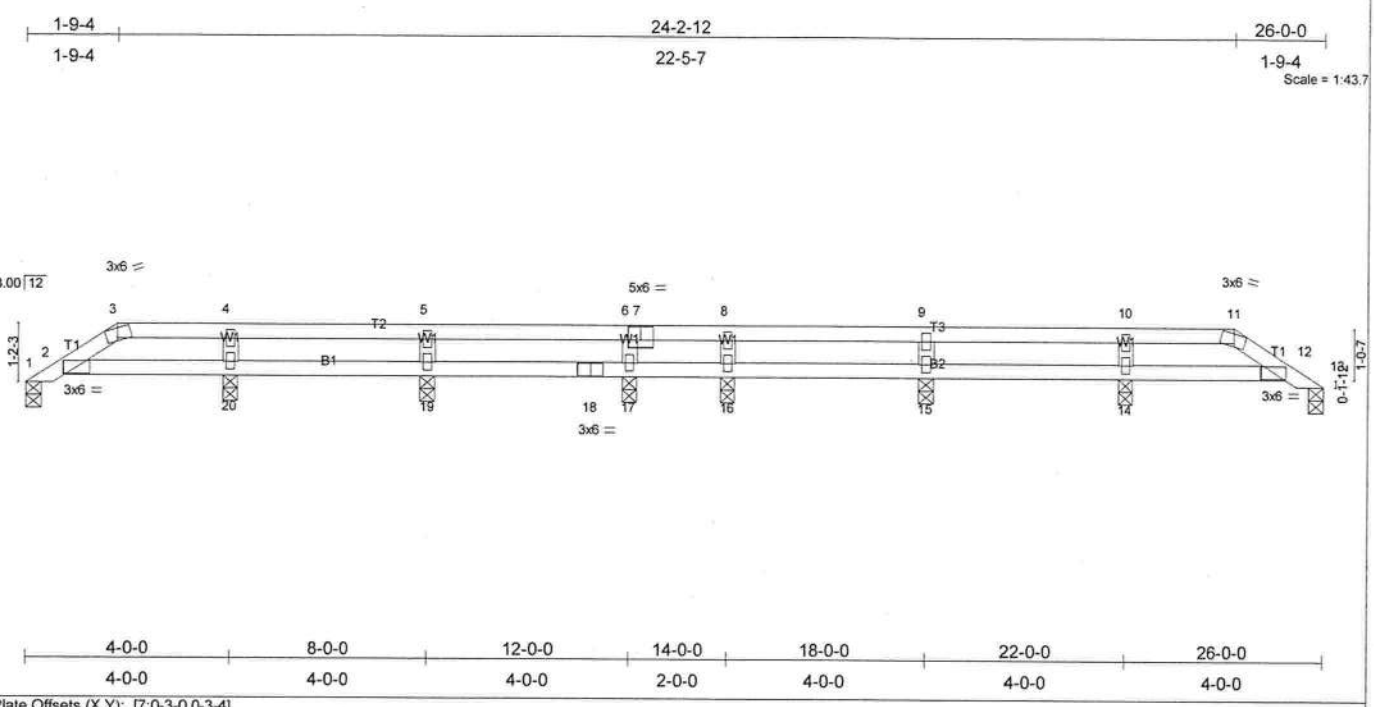


Plate Offsets (X,Y): [7:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.01	2-20	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.01	2-20	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 80 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=132/0-3-8, 13=132/0-3-8, 20=349/0-3-8, 19=346/0-3-8, 17=241/0-3-8, 16=241/0-3-8, 15=346/0-3-8, 14=349/0-3-8
Max Horz 1=-39(load case 3)
Max Uplift1=-47(load case 5), 13=-50(load case 6), 20=-164(load case 4), 19=-174(load case 3), 17=-115(load case 4), 16=-115(load case 3), 15=-172(load case 4), 14=-156(load case 3)
Max Grav 1=132(load case 1), 13=132(load case 1), 20=352(load case 9), 19=351(load case 10), 17=242(load case 9), 16=242(load case 10), 15=351(load case 9), 14=352(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-65/40, 2-3=-94/47, 3-4=-69/44, 4-5=-69/44, 5-6=-69/44, 6-7=-69/44, 7-8=-69/44, 8-9=-69/44, 9-10=-69/44, 10-11=-69/44, 11-12=-94/44, 12-13=-65/30
BOT CHORD 2-20=-31/69, 19-20=-31/69, 18-19=-31/69, 17-18=-31/69, 16-17=-31/69, 15-16=-31/69, 14-15=-31/69, 12-14=-31/69
WEBS 4-20=-213/173, 5-19=-228/198, 6-17=-155/131, 8-16=-155/131, 9-15=-228/197, 10-14=-213/169

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Bearing at joint(s) 1, 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 1, 50 lb uplift at joint 13, 164 lb uplift at joint 20, 174 lb uplift at joint 19, 115 lb uplift at joint 17, 115 lb uplift at joint 16, 172 lb uplift at joint 15 and 156 lb uplift at joint 14.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071020

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	PB8	VALLEY 3x6 =	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:22:58 2007 Page 1

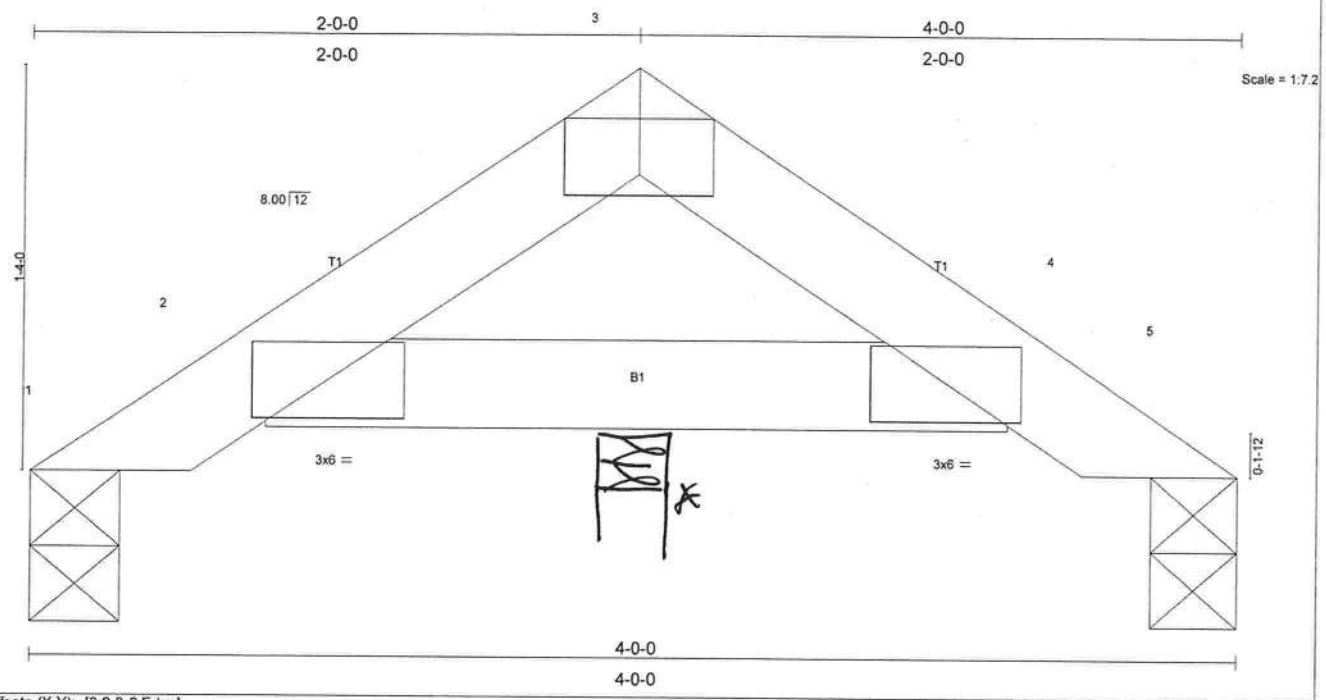


Plate Offsets (X,Y): [3:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=144/0-3-8, 5=144/0-3-8
Max Horz 1=43(load case 4)
Max Uplift1=-49(load case 5), 5=-49(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-71/41, 2-3=-174/69, 3-4=-174/69, 4-5=-71/32
BOT CHORD 2-4=-37/166

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 1 and 49 lb uplift at joint 5.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

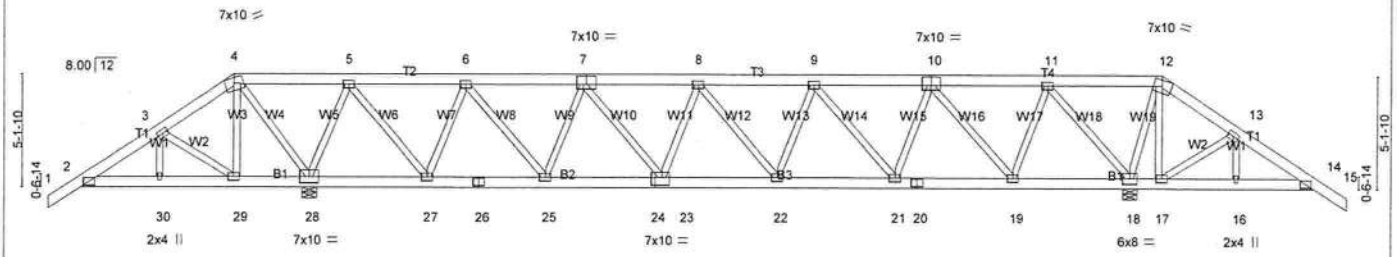
Dwg.#0223071021

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T01	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 11:37:54 2007 Page 1

-1-6-0	3-5-8	6-10-2	12-1-5	17-4-14	22-8-7	28-0-0	33-3-9	38-7-2	43-10-11	49-1-14	52-6-8	56-0-0	57-6-0
1-6-0	3-5-8	3-4-10	5-3-3	5-3-9	5-3-9	5-3-9	5-3-9	5-3-9	5-3-9	5-3-3	3-4-10	3-5-8	1-6-0

Scale = 1:99.2



7-0-0									49-1-14		
3-5-8	6-10-2	10-4-0	15-8-0	21-0-0	26-4-0	31-8-0	37-0-0	42-4-0	47-8-0	49-0-0	52-6-8	56-0-0
3-5-8	3-4-10	3-4-0	5-4-0	5-4-0	5-4-0	5-4-0	5-4-0	5-4-0	5-4-0	1-4-0	3-4-10	3-5-8
	0-1-14									0-1-14		

Plate Offsets (X,Y): [7:0-5-0,0-4-8], [10:0-5-0,0-4-8], [24:0-4-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	0.18	22-23	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.25	22-23	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.59	Horz(TL)	0.06	18	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 855 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 28=5330/0-8-0, 18=4788/0-8-0
Max Horz 28=166(load case 3)
Max Uplift28=-3207(load case 2), 18=-2827(load case 3)
Max Grav 28=5380(load case 8), 18=4856(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-331/421, 3-4=-463/733, 4-5=-1108/1601, 5-6=-2237/1515, 6-7=-4486/2964, 7-8=-5615/3707, 8-9=-5638/3754, 9-10=-4687/3116, 10-11=-2621/1772, 11-12=-600/892, 12-13=-474/742, 13-14=-326/416, 14-15=0/43
BOT CHORD 2-30=-320/389, 29-30=-320/389, 28-29=-521/565, 27-28=-301/536, 26-27=-2150/3118, 25-26=-2150/3118, 24-25=-3369/5002, 23-24=-3369/5002, 22-23=-3886/5774, 21-22=-3719/5497, 20-21=-2856/4214, 19-20=-2856/4214, 18-19=-1329/1865, 17-18=-531/573, 16-17=-318/387, 14-16=-318/387
WEBS 3-30=-123/178, 3-29=-268/230, 4-29=-500/666, 4-28=-1798/1365, 5-28=-3712/2510, 5-27=-2015/3483, 6-27=-2652/1836, 6-25=-1348/2397, 7-25=-1637/1200, 7-23=-561/1149, 8-23=-599/548, 8-22=-220/227, 9-22=-111/546, 9-21=-1455/993, 10-21=-763/1593, 10-19=-2706/1775, 11-19=-1327/2510, 11-18=-3898/2537, 12-18=-1180/994, 12-17=-324/302, 13-17=-268/237, 13-16=-139/202

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are 4x6 MT20 unless otherwise indicated.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3207 lb uplift at joint 28 and 2827 lb uplift at joint 18.
8) Girder carries hip end with 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 410 lb up at 49-0-0, and 539 lb down and 410 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-12=-118(F=-64), 12-15=-54, 2-29=-30, 17-29=-65(F=-35), 14-17=-30
Concentrated Loads (lb)
Vert: 29=-539(F) 17=-539(F)

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

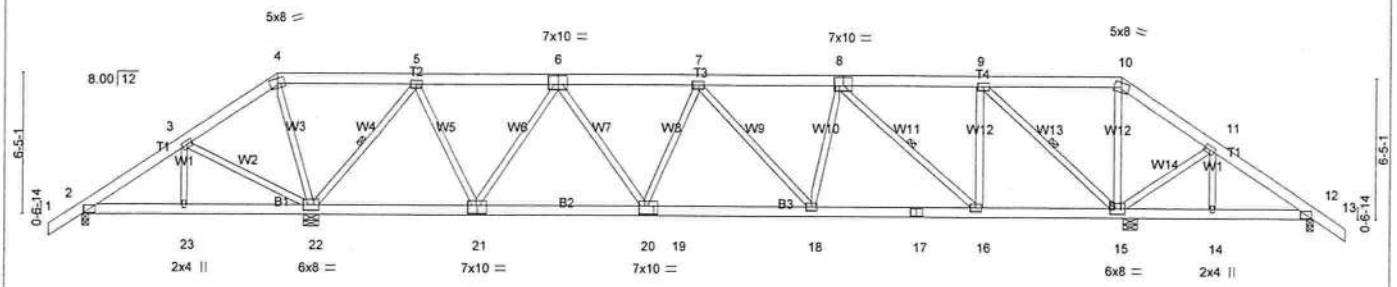
Dwg.#0223071022

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T02	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:47:50 2007 Page 1

-1-6-0	4-7-1	8-9-4	15-2-3	21-7-1	28-0-0	34-4-15	40-9-13	47-2-12	51-4-15	56-0-0	57-6-0
1-6-0	4-7-1	4-2-3	6-4-15	6-4-15	6-4-15	6-4-15	6-4-15	6-4-15	4-2-3	4-7-1	1-6-0

Scale = 1:99.2



4-7-1	10-4-0	17-11-7	25-6-15	33-2-6	40-9-13	47-2-12	47-8-0	51-4-15	56-0-0
4-7-1	5-8-15	7-7-7	7-7-7	7-7-7	7-7-7	6-4-15	0-5-4	3-8-15	4-7-1

Plate Offsets (X,Y): [6:0-5-0,0-4-8], [8:0-5-0,0-4-8], [19:0-2-12,0-4-8], [21:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.09	18-20	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.15	18-20	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.80	Horz(TL)	0.02	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 422 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-22, 8-16, 9-15

REACTIONS (lb/size) 2=-119/0-3-8, 22=2651/0-8-0, 15=2440/0-8-0, 12=-115/0-3-8
Max Horz 2=211(load case 4)
Max Uplift2=-366(load case 10), 22=-1376(load case 4), 15=-1206(load case 3), 12=-343(load case 9)
Max Grav 2=136(load case 3), 22=2658(load case 9), 15=2450(load case 10), 12=189(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-353/838, 3-4=-540/1105, 4-5=-523/1095, 5-6=-531/342, 6-7=-1323/713, 7-8=-1316/704, 8-9=-517/322, 9-10=-372/815, 10-11=-457/1029, 11-12=-411/805, 12-13=0/44
BOT CHORD 2-23=-669/468, 22-23=-669/468, 21-22=-45/176, 20-21=-504/1017, 19-20=-698/1417, 18-19=-698/1417, 17-18=-610/1246, 16-17=-610/1246, 15-16=-254/517, 14-15=-641/340, 12-14=-641/340
WEBS 3-23=-197/148, 3-22=-362/352, 4-22=-898/508, 5-22=-1787/898, 5-21=-433/1136, 6-21=-948/530, 6-20=-198/567, 7-20=-262/223, 7-18=-159/134, 8-18=-58/363, 8-16=-1017/506, 9-16=-288/887, 9-15=-1809/862, 10-15=-828/427, 11-15=-314/306, 11-14=-160/117

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 4x6 MT20 unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 366 lb uplift at joint 2, 1376 lb uplift at joint 22, 1206 lb uplift at joint 15 and 343 lb uplift at joint 12.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

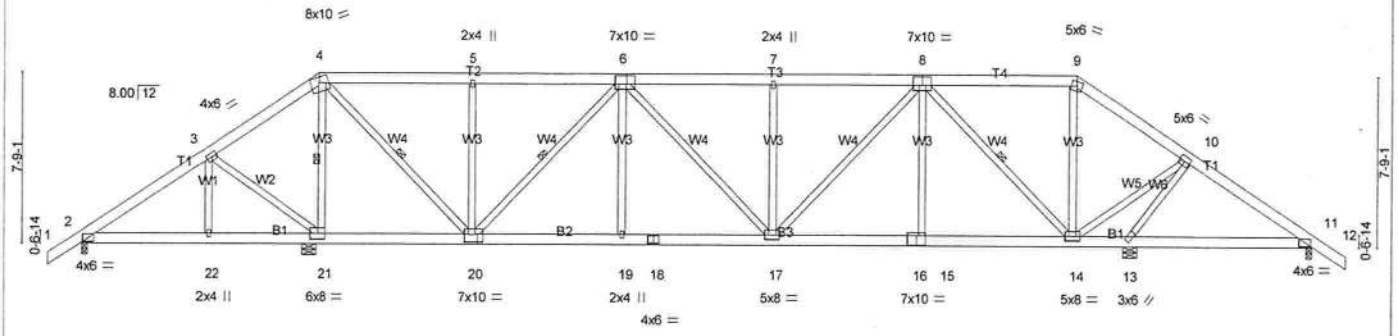
Dwg.#0223071023

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T03	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:49:13 2007 Page 1

-1-6-0	5-8-9	10-9-4	17-9-0	24-7-0	31-5-0	38-3-0	45-2-12	50-3-7	56-0-0	57-6-0
1-6-0	5-8-9	5-0-11	6-11-12	6-10-0	6-10-0	6-10-0	6-11-12	5-0-11	5-8-9	1-6-0

Scale = 1:99.3



5-8-9	10-4-0	10-9-4	17-9-0	24-7-0	31-5-0	38-3-0	45-2-12	47-8-0	56-0-0
5-8-9	4-7-7	0-5-4	6-11-12	6-10-0	6-10-0	6-10-0	6-11-12	2-5-4	8-4-0

Plate Offsets (X,Y): [6:0-4-0,0-4-8], [8:0-5-0,0-4-8], [16:0-5-0,0-0-4], [20:0-3-0,0-4-8], [21:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.11	17-19	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.17	17-19	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.03	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 443 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-21, 4-20, 6-20, 8-14

REACTIONS (lb/size) 2=158/0-3-8, 21=2482/0-8-0, 13=1793/0-8-0, 11=425/0-3-8
Max Horz 2=-257(load case 3)
Max Uplift2=-280(load case 5), 21=-1248(load case 4), 13=-863(load case 3), 11=-330(load case 6)
Max Grav 2=166(load case 9), 21=2482(load case 1), 13=1796(load case 10), 11=428(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-183/440, 3-4=-335/703, 4-5=-763/478, 5-6=-763/478, 6-7=-1682/893, 7-8=-1682/893, 8-9=-429/277, 9-10=-595/280, 10-11=-294/156, 11-12=0/44
BOT CHORD 2-22=-338/338, 21-22=-338/338, 20-21=-557/372, 19-20=-704/1495, 18-19=-704/1495, 17-18=-704/1495, 16-17=-602/1349, 15-16=-602/1349, 14-15=-602/1350, 13-14=-929/502, 11-13=0/182
WEBS 3-22=-205/147, 3-21=-391/391, 4-21=-2026/1015, 4-20=-872/1864, 5-20=-379/329, 6-20=-1084/497, 6-19=0/213, 6-17=-111/281, 7-17=-360/304, 8-17=-249/493, 8-15=0/241, 8-14=-1359/678, 9-14=-40/64, 10-14=-679/1661, 10-13=-1808/786

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2, 1248 lb uplift at joint 21, 863 lb uplift at joint 13 and 330 lb uplift at joint 11.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

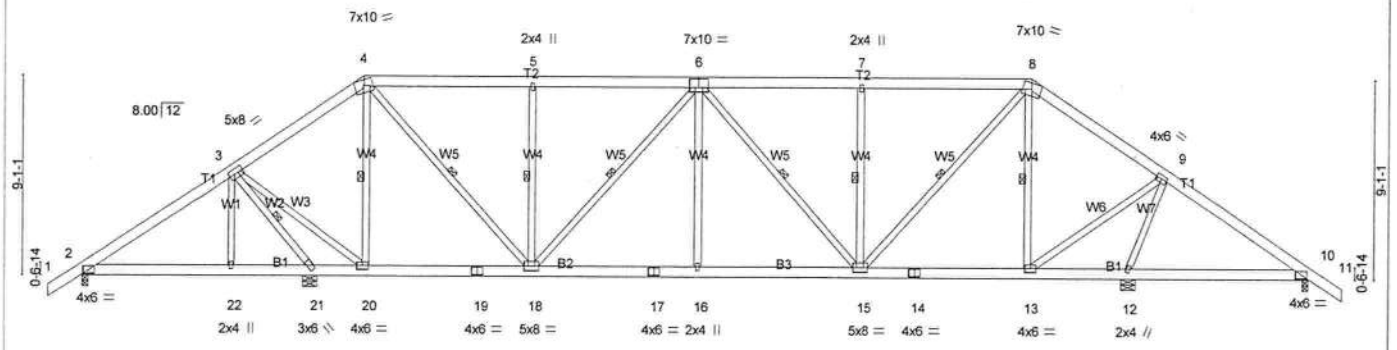
Dwg.#0223071024

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T04	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:50:13 2007 Page 1

-1-6-0	6-8-9	12-9-4	20-5-8	28-0-0	35-6-8	43-2-12	49-3-7	56-0-0	57-6-0
1-6-0	6-8-9	6-0-11	7-8-4	7-6-8	7-6-8	7-8-4	6-0-11	6-8-9	1-6-0

Scale = 1:99.3



6-8-9	10-4-0	12-9-4	20-5-8	28-0-0	35-6-8	43-2-12	47-8-0	56-0-0
6-8-9	3-7-7	2-5-4	7-8-4	7-6-8	7-6-8	7-8-4	4-5-4	8-4-0

Plate Offsets (X,Y): [6:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.11	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.18	15-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 452 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-21, 4-20, 4-18, 5-18, 6-18, 6-15, 7-15, 8-15, 8-13

REACTIONS (lb/size) 2=744/0-3-8, 21=1718/0-8-0, 12=1927/0-8-0, 10=468/0-3-8
Max Horz 2=304(load case 4)
Max Uplift2=-411(load case 5), 21=-853(load case 4), 12=-830(load case 3), 10=-342(load case 6)
Max Grav 2=747(load case 9), 21=1718(load case 1), 12=1927(load case 1), 10=475(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-798/381, 3-4=-889/406, 4-5=-1552/779, 5-6=-1552/779, 6-7=-1608/837, 7-8=-1609/836, 8-9=-1044/491, 9-10=-333/151, 10-11=0/44
BOT CHORD 2-22=-398/585, 21-22=-397/585, 20-21=-550/386, 19-20=-337/645, 18-19=-337/645, 17-18=-862/1850, 16-17=-862/1850, 15-16=-862/1850, 14-15=-293/776, 13-14=-293/776, 12-13=-481/274, 10-12=-0/204
WEBS 3-22=-175/64, 3-21=-1778/877, 3-20=-630/1508, 4-20=-937/518, 4-18=-679/1385, 5-18=-424/368, 6-18=-468/239, 6-16=0/215, 6-15=-387/208, 7-15=-424/371, 8-15=-648/1283, 8-13=-774/435, 9-13=-620/1550, 9-12=-1794/736

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 411 lb uplift at joint 2, 853 lb uplift at joint 21, 830 lb uplift at joint 12 and 342 lb uplift at joint 10.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

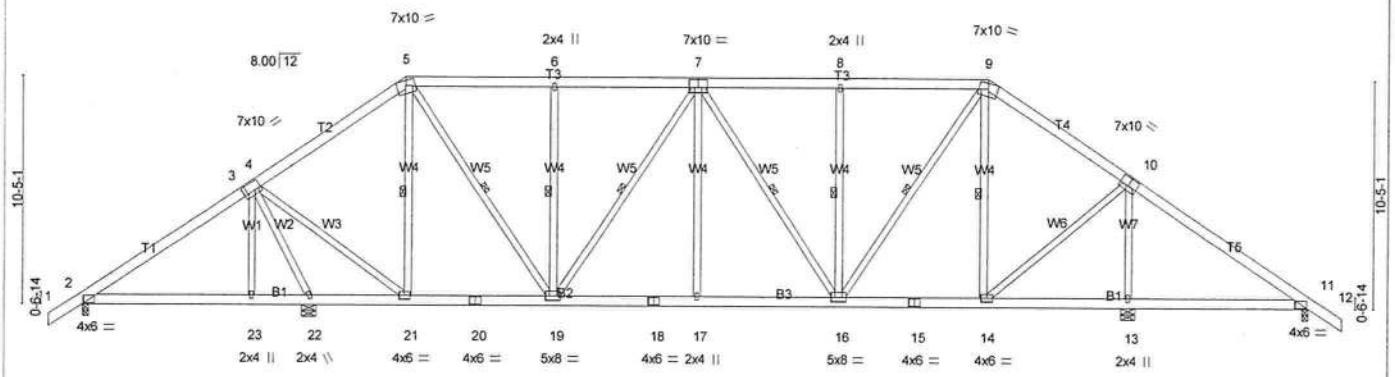
Dwg.#0223071025

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T05	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:51:30 2007 Page 1

-1-6-0	7-8-9	14-9-4	21-5-8	28-0-0	34-6-8	41-2-12	47-8-0	56-0-0	57-6-0
1-6-0	7-8-9	7-0-11	6-8-4	6-6-8	6-6-8	6-8-4	6-5-4	8-4-0	1-6-0

Scale = 1:99.3



7-8-9	10-4-0	14-9-4	21-5-8	28-0-0	34-6-8	41-2-12	47-8-0	56-0-0
7-8-9	2-7-7	4-5-4	6-8-4	6-6-8	6-6-8	6-8-4	6-5-4	8-4-0

Plate Offsets (X,Y): [3:0-3-6,0-4-8], [7:0-5-0,0-4-8], [10:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.09	16-17	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.14	16-17	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.99	Horz(TL)	0.03	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 471 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-21, 5-19, 6-19, 7-19, 7-16, 8-16, 9-16, 9-14

REACTIONS (lb/size) 2=753/0-3-8, 22=1697/0-8-0, 13=1994/0-8-0, 11=414/0-3-8
Max Horz 2=350(load case 4)
Max Uplift2=-408(load case 5), 22=-798(load case 4), 13=-793(load case 3), 11=-327(load case 6)
Max Grav 2=753(load case 1), 22=1697(load case 1), 13=1994(load case 1), 11=422(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-783/363, 3-4=-555/366, 4-5=-1122/486, 5-6=-1401/691, 6-7=-1401/692, 7-8=-1428/742, 8-9=-1428/742, 9-10=-1185/545, 10-11=-202/90, 11-12=0/44
BOT CHORD 2-23=-398/564, 22-23=-399/565, 21-22=-241/276, 20-21=-418/827, 19-20=-418/827, 18-19=-726/1594, 17-18=-726/1594, 16-17=-726/1594, 15-16=-322/885, 14-15=-322/885, 13-14=-26/130, 11-13=-4/121
WEBS 4-23=-225/123, 4-22=-1702/860, 4-21=-528/1319, 5-21=-658/369, 5-19=-542/1068, 6-19=-364/324, 7-19=-374/201, 7-17=0/192, 7-16=-331/192, 8-16=-365/322, 9-16=-535/1013, 9-14=-547/352, 10-14=-458/1116, 10-13=-1708/669

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 408 lb uplift at joint 2, 798 lb uplift at joint 22, 793 lb uplift at joint 13 and 327 lb uplift at joint 11.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

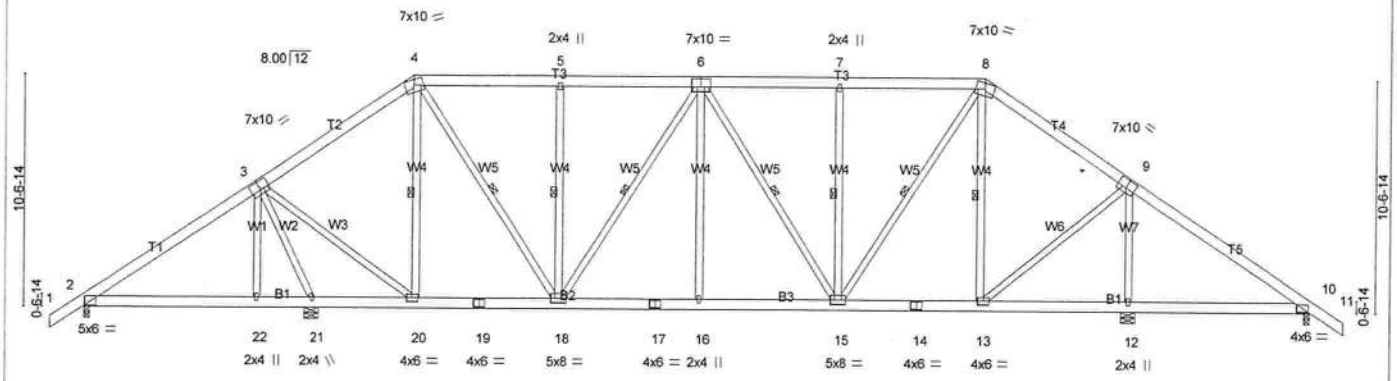
Dwg.#0223071026

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T06	HIP	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 10:57:31 2007 Page 1

-1-6-0	7-9-15	15-0-0	21-6-14	28-0-0	34-5-2	41-0-0	47-8-0	56-0-0	57-6-0
1-6-0	7-9-15	7-2-1	6-6-14	6-5-2	6-5-2	6-6-14	6-8-0	8-4-0	1-6-0

Scale = 1:99.3



7-9-15	10-4-0	15-0-0	21-6-14	28-0-0	34-5-2	41-0-0	47-8-0	56-0-0
7-9-15	2-6-1	4-8-0	6-6-14	6-5-2	6-5-2	6-6-14	6-8-0	8-4-0

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [6:0-5-0,0-4-8], [9:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.08	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.13	15-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.99	Horz(TL)	0.03	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 474 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-8.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-20, 4-18, 5-18, 6-18, 6-15, 7-15, 8-15, 8-13

REACTIONS (lb/size) 2=744/0-3-8, 21=1709/0-8-0, 12=1985/0-8-0, 10=419/0-3-8
Max Horz 2=-355(load case 3)
Max Uplift2=-404(load case 5), 21=-798(load case 4), 12=-783(load case 3), 10=-328(load case 6)
Max Grav 2=744(load case 1), 21=1709(load case 1), 12=1985(load case 1), 10=427(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-762/355, 3-4=-1131/488, 4-5=-1381/680, 5-6=-1380/680, 6-7=-1410/732, 7-8=-1411/732, 8-9=-1203/549, 9-10=-210/90, 10-11=0/44
BOT CHORD 2-22=-390/545, 21-22=-389/545, 20-21=-211/267, 19-20=-420/834, 18-19=-420/834, 17-18=-711/1567, 16-17=-711/1567, 15-16=-711/1567, 14-15=-324/898, 13-14=-324/898, 12-13=0/123, 10-12=0/123
WEBS 3-22=-233/128, 3-21=-1695/858, 3-20=-511/1290, 4-20=-639/358, 4-18=-531/1044, 5-18=-358/318, 6-18=-371/200, 6-16=0/189, 6-15=-321/189, 7-15=-358/317, 8-15=-523/982, 8-13=-520/341, 9-13=-444/1085, 9-12=-1696/661

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 2, 798 lb uplift at joint 21, 783 lb uplift at joint 12 and 328 lb uplift at joint 10.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

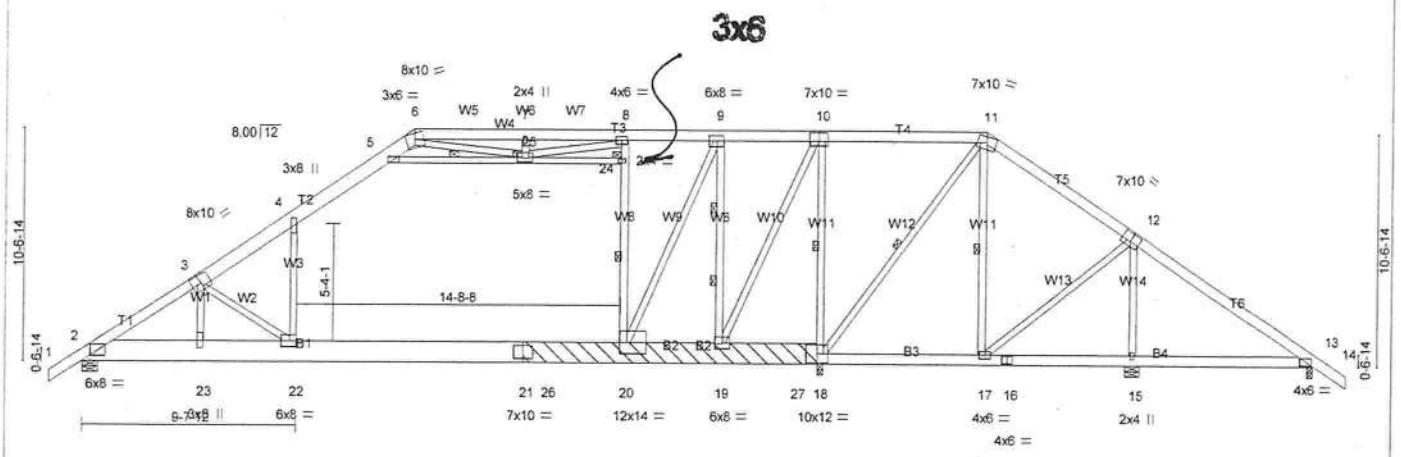
Dwg.#0223071027

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T07	ROOF TRUSS	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 12:30:10 2007 Page 1

-1-6-0	5-3-0	9-6-0	15-0-0	20-0-0	24-6-0	33-5-12	41-0-0	47-8-0	56-0-0	57-6-0
1-6-0	5-3-0	4-3-0	5-6-0	5-0-0	4-6-0	8-11-12	7-6-4	6-8-0	8-4-0	1-6-0

Scale = 1:99.3



5-3-0	9-6-0	24-6-0	33-5-12	41-0-0	47-8-0	56-0-0
5-3-0	4-3-0	15-0-0	8-11-12	7-6-4	6-8-0	8-4-0

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [9:0-3-8,0-3-0], [10:0-5-0,0-4-8], [12:0-5-0,0-4-8], [19:0-3-8,0-3-0], [20:0-3-8,0-6-0], [22:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.39	20-22	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.66	20-22	>603	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.95	Horz(TL)	0.05	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.16	20-22	>999	240	Weight: 613 lb	

LUMBER

TOP CHORD	2 X 6 SYP No.1D *Except* T2 2 X 8 SYP 2400F 2.0E
BOT CHORD	2 X 12 SYP No.2 *Except* B4 2 X 6 SYP No.1D, B3 2 X 6 SYP No.1D
WEBS	2 X 4 SYP No.3 *Except* W13 2 X 4 SYP No.2, W4 2 X 4 SYP No.2
LBR SCAB	18-21 2 X 12 SYP No.2 one side

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-2-6 oc purlins, except 2-0-0 oc purlins (5-3-1 max.): 6-11.
BOT CHORD	Rigid ceiling directly applied or 9-10-2 oc bracing.
WEBS	1 Row at midpt 20-24, 10-18, 11-18, 11-17, 5-25 2 Rows at 1/3 pts 9-19
JOINTS	1 Brace at Jt(s): 24, 25

REACTIONS (lb/size) 2=2417/0-8-0, 18=2229/0-3-8, 15=1047/0-8-0, 13=656/0-3-8
Max Horz 2=353(load case 4)
Max Uplift2=-435(load case 5), 18=-688(load case 3), 15=-436(load case 6), 13=-375(load case 6)
Max Grav 2=2418(load case 10), 18=2374(load case 11), 15=1047(load case 1), 13=656(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/53, 2-3=-3724/451, 3-4=-3300/294, 4-5=-2589/428, 5-6=-270/474, 6-7=-1319/667, 7-8=-1319/667, 8-9=-2466/463, 9-10=-1414/476, 10-11=-705/488, 11-12=-864/422, 12-13=-606/247, 13-14=0/44
BOT CHORD	2-23=-540/3080, 22-23=-541/3082, 21-22=-404/2470, 21-26=-404/2470, 20-26=-404/2470, 19-20=-371/1414, 19-27=-292/703, 18-27=-292/700, 17-18=-203/631, 16-17=-124/405, 15-16=-124/405, 13-15=-124/405
WEBS	3-23=-102/121, 3-22=-768/354, 4-22=0/1056, 20-24=-282/368, 8-24=-186/372, 10-19=-189/1721, 10-18=-1988/512, 11-18=-147/298, 11-17=-37/138, 12-17=-105/307, 12-15=-761/330, 5-25=-2588/207, 24-25=-36/13, 7-25=-75/178, 8-25=-1146/67, 6-25=-215/1457, 9-19=-2498/191, 9-20=-79/2609

NOTES

1) Attached 13-4-0 scab 18 to 21, front face(s) 2 X 12 SYP No.2 with 2 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except : starting at 11-4-0 from end at joint 18, nail 2 row(s) at 7 o.c. for 2-0-0; starting at 7-10-0 from end at joint 18, nail 2 row(s) at 4 o.c. for 2-0-0.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Provide adequate drainage to prevent water ponding.
5) Ceiling dead load (5.0 psf) on member(s). 4-5, 5-25, 24-25; Wall dead load (5.0psf) on member(s).4-22, 20-24
6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 20-22
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 435 lb uplift at joint 2, 688 lb uplift at joint 18, 436 lb uplift at joint 15 and 375 lb uplift at joint 13.
8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ FL 33549

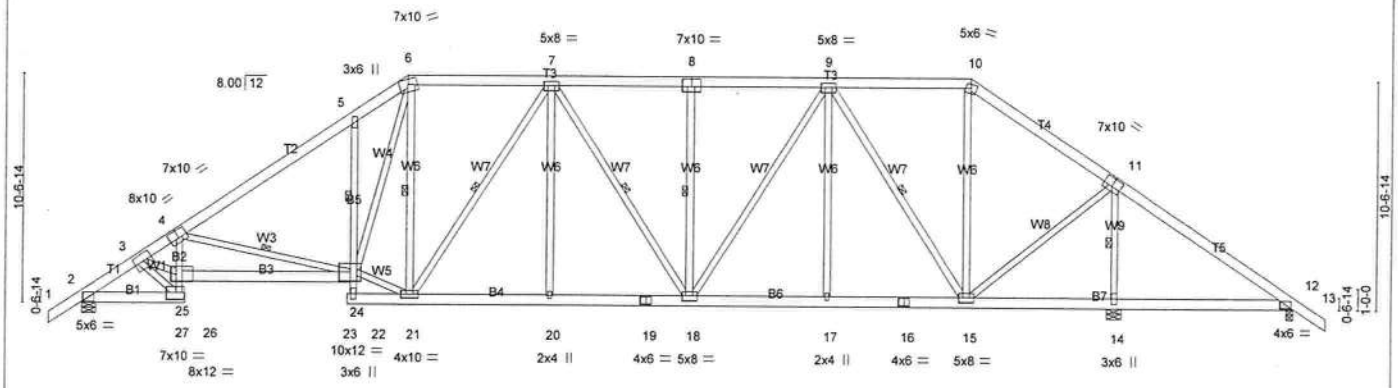
Dwg.#0223071028

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T08	SPECIAL	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 11:00:24 2007 Page 1

-1-6-0	2-8-7	4-9-0	12-3-0	15-0-0	21-6-14	28-0-0	34-5-2	41-0-0	47-8-0	56-0-0	57-6-0
1-6-0	2-8-7	2-0-9	7-6-0	2-9-0	6-6-14	6-5-2	6-5-2	6-6-14	6-8-0	8-4-0	1-6-0

Scale = 1:100.8



4-9-0	12-3-0	15-0-0	21-6-14	28-0-0	34-5-2	41-0-0	47-8-0	56-0-0
4-9-0	7-6-0	2-9-0	6-6-14	6-5-2	6-5-2	6-6-14	6-8-0	8-4-0

Plate Offsets (X,Y): [4:0-5-0,0-4-8], [8:0-5-0,0-4-8], [11:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.22	24-25	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.35	24-25	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.96	Horz(TL)	0.17	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 496 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D *Except*
B2 2 X 4 SYP No.2, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-6 oc purlins, except 2-0-0 oc purlins (5-9-6 max.): 6-10.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 5-24
WEBS 1 Row at midpt 4-24, 6-21, 7-21, 7-18, 8-18, 9-15, 11-14

REACTIONS (lb/size) 2=2029/0-8-0, 14=2831/0-8-0, 12=9/0-3-8
Max Horz 2=355(load case 4)
Max Uplift2=-657(load case 5), 14=-882(load case 3), 12=-262(load case 6)
Max Grav 2=2029(load case 1), 14=2831(load case 1), 12=111(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-2872/848, 3-4=-3984/1339, 4-5=-2943/1021, 5-6=-2823/1126, 6-7=-1932/804, 7-8=-2082/873, 8-9=-2082/873, 9-10=-870/504, 10-11=-1156/545, 11-12=-326/768, 12-13=0/44
BOT CHORD 2-27=-913/2109, 26-27=0/0, 25-27=-579/1511, 4-25=-115/555, 24-25=-1494/3594, 22-24=-7/17, 5-24=-264/361, 22-23=0/0, 21-22=-110/56, 20-21=-943/2171, 19-20=-943/2171, 18-19=-943/2171, 17-18=-693/1651, 16-17=-693/1651, 15-16=-693/1651, 14-15=-583/257, 12-14=-583/257
WEBS 3-27=-2010/815, 3-25=-1198/2952, 4-24=-1280/593, 21-24=-770/2012, 6-24=-608/1495, 6-21=-277/127, 7-21=-578/454, 7-20=0/180, 7-18=-192/99, 8-18=-339/284, 9-18=-376/826, 9-17=0/197, 9-15=-1492/726, 10-15=-85/305, 11-15=-725/1797, 11-14=-2527/770

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 657 lb uplift at joint 2, 882 lb uplift at joint 14 and 262 lb uplift at joint 12.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

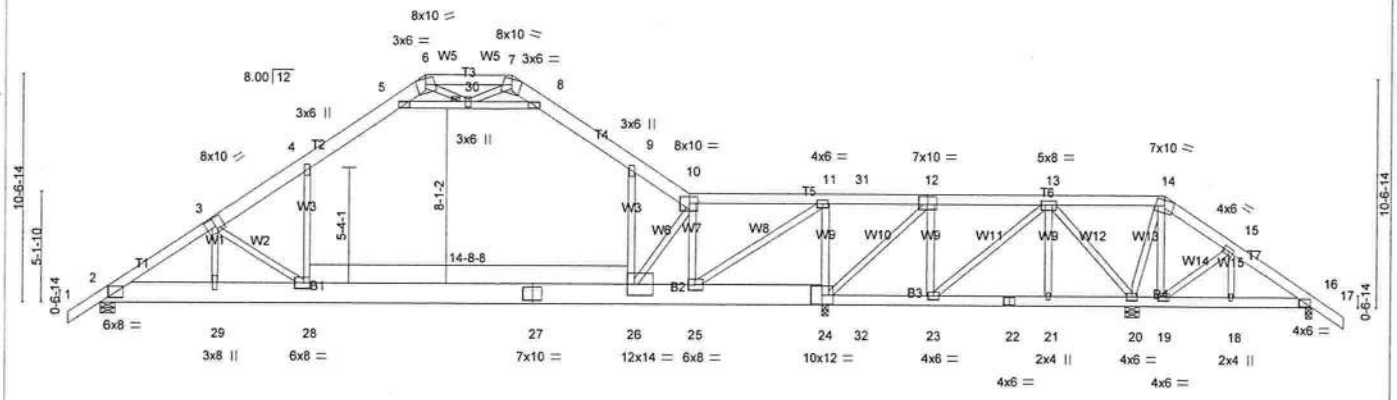
Dwg.#0223071029

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T09	ROOF TRUSS	1	3	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 16:46:30 2007 Page 1

-1-6-0	5-3-0	9-6-0	15-0-0	19-0-0	24-6-0	27-1-14	33-5-12	38-4-10	43-9-4	49-1-14	52-3-0	56-0-0	57-6-0
1-6-0	5-3-0	4-3-0	5-6-0	4-0-0	5-6-0	2-7-14	6-3-14	4-10-14	5-4-10	5-4-10	3-1-2	3-9-0	1-6-0

Scale = 1:100.8



5-3-0	9-6-0	24-6-0	33-5-12	38-4-10	43-9-4	47-8-0	49-0-0	49-1-14	52-3-0	56-0-0
5-3-0	4-3-0	15-0-0	8-11-12	4-10-14	5-4-10	3-10-12	1-4-0	0-1-14	3-1-2	3-9-0

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [12:0-5-0,0-4-8], [25:0-3-8,0-3-0], [26:0-3-8,0-6-0], [28:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.15	26-28	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.24	26-28	>999	240		
BCLL	10.0	Rep Stress Incr	NO	WB	0.35	Horz(TL)	0.01	20	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.05	26-28	>999	240	Weight: 1552 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except*
T2 2 X 8 SYP 2400F 2.0E, T4 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 6 SYP No.1D *Except*
B1 2 X 12 SYP No.2, B2 2 X 12 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2, W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 6-7, 10-14.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 30

REACTIONS (lb/size) 2=1932/0-8-0, 24=4755/0-3-8, 20=3090/0-8-0, 16=112/0-3-8
Max Horz 2=352(load case 3)
Max Uplift2=-347(load case 4), 24=-888(load case 5), 20=-1310(load case 3), 16=-326(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-2889/299, 3-4=-2429/143, 4-5=-1662/268, 5-6=0/719, 6-7=0/859, 7-8=0/822, 8-9=-1727/296, 9-10=-2358/236, 10-11=-684/274, 11-31=-91/1984, 12-31=-91/1984, 12-13=-49/730, 13-14=-40/734, 14-15=0/561, 15-16=0/325, 16-17=0/44
BOT CHORD 2-29=-299/2392, 28-29=-302/2394, 27-28=-123/1715, 26-27=-123/1715, 25-26=-272/794, 24-25=-2063/385, 24-32=-737/27, 23-32=-740/26, 22-23=-545/250, 21-22=-545/250, 20-21=-545/250, 19-20=-384/99, 18-19=-256/0, 16-18=-256/0
WEBS 3-29=-106/127, 3-28=-892/359, 4-28=0/1157, 9-26=-146/1124, 10-26=0/1767, 11-24=-2364/478, 12-24=-2446/649, 12-23=0/1128, 13-23=-454/20, 13-21=0/789, 13-20=-1472/913, 14-20=-1145/602, 14-19=-308/388, 15-19=-262/186, 15-18=-134/200, 10-25=-3346/112, 11-25=-208/3240, 5-30=-2665/255, 8-30=-2884/347, 6-30=-56/181, 7-30=-64/435

NOTES
1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 12 - 2 rows at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 9-10, 5-30, 8-30; Wall dead load (5.0psf) on member(s).4-28, 9-26
7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 26-28
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 347 lb uplift at joint 2, 888 lb uplift at joint 24, 1310 lb uplift at joint 20 and 326 lb uplift at joint 16.
9) Girder carries hip end with 7-0-0 right side setback, 34-0-0 left side setback, and 7-0-0 end setback.
10) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 336 lb up at 49-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Continued on page 2

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T09	ROOF TRUSS	1	3	Job Reference (optional)

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-54, 4-5=-64, 5-6=-54, 6-7=-54, 7-8=-54, 8-10=-64, 10-31=-54, 14-31=-112(F=-58), 14-17=-54, 2-28=-30, 26-28=-110, 26-32=-30, 19-32=-171(F=-141), 16-19=-30, 5-8=-10

Drag: 4-28=-10, 9-26=-10

Concentrated Loads (lb)

Vert: 19=-539(F)

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

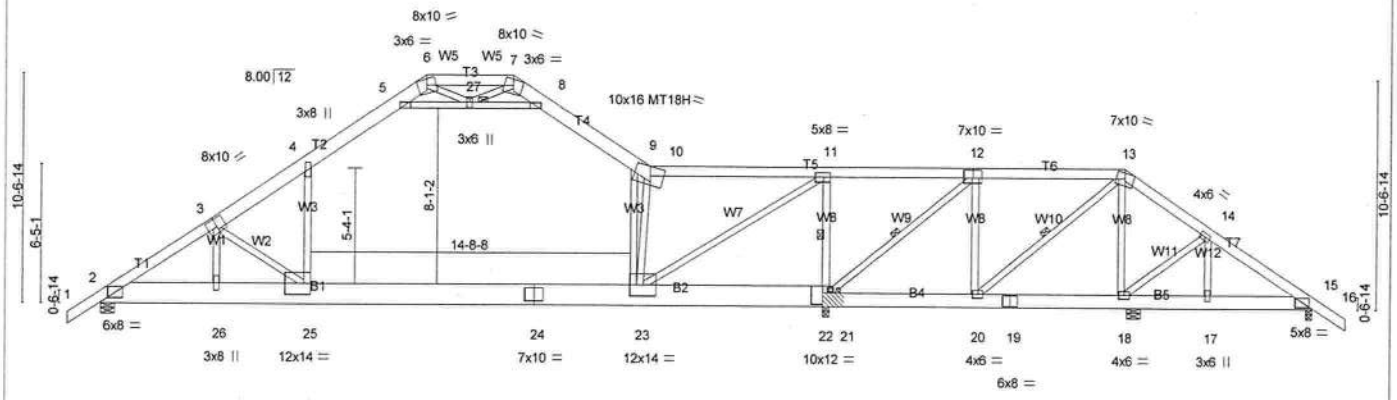
Dwg.#0223071031

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T10	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 12:23:42 2007 Page 1

-1-6-0	5-3-0	9-6-0	15-0-0	19-0-0	24-6-0	25-2-12	33-5-12	40-4-4	47-2-12	51-2-2	56-0-0	57-6-0
1-6-0	5-3-0	4-3-0	5-6-0	4-0-0	5-6-0	0-8-12	8-3-0	6-10-8	6-10-8	3-11-6	4-9-14	1-6-0

Scale = 1:100.6



51-2-2

5-3-0	9-6-0	24-6-0	33-5-12	40-4-4	47-2-12	47-8-0	56-0-0
5-3-0	4-3-0	15-0-0	8-11-12	6-10-8	6-10-8	0-5-43-6-2	4-9-14

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [10:0-8-0,0-2-10], [11:0-3-8,0-2-8], [12:0-5-0,0-4-8], [23:0-3-8,0-6-0], [25:0-3-8,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.55	23-25	>724	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.96	Vert(TL)	-0.91	23-25	>436	240	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.02	22	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.19	23-25	>999	240	Weight: 527 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except*
T2 2 X 8 SYP No.1D, T4 2 X 8 SYP No.1D
BOT CHORD 2 X 8 SYP No.1D *Except*
B1 2 X 12 SYP No.2, B2 2 X 12 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W7 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-5-5 oc purlins, except 2-0-0 oc purlins (5-6-14 max.): 6-7, 10-13.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 11-22, 12-22, 13-20
JOINTS 1 Brace at Jt(s): 27

REACTIONS (lb/size) 2=1931/0-8-0, 22=3501/0-4-2 (0-3-8 + bearing block), 18=793/0-8-0, 15=126/0-3-8
Max Horz 2=353(load case 4)
Max Uplift2=-347(load case 5), 22=-565(load case 6), 18=-405(load case 6), 15=-323(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-2902/299, 3-4=-2391/143, 4-5=-1597/266, 5-6=0/725, 6-7=0/925, 7-8=0/912, 8-9=-1709/297, 9-10=-2356/280, 10-11=-1867/226, 11-12=-56/1624, 12-13=0/749, 13-14=0/652, 14-15=0/374, 15-16=0/46
BOT CHORD 2-26=-306/2408, 25-26=-308/2411, 24-25=-134/1666, 23-24=-134/1666, 22-23=-1690/403, 21-22=-746/201, 20-21=-757/201, 19-20=-491/223, 18-19=-491/223, 17-18=-279/2, 15-17=-279/2
WEBS 3-26=-98/157, 3-25=-965/361, 4-25=0/1206, 9-23=-248/1214, 10-23=-1915/396, 11-22=-2805/381, 11-23=-162/4146, 12-22=-1134/251, 12-20=0/431, 13-20=-339/0, 13-18=-427/237, 14-18=-293/280, 14-17=-151/133, 5-27=-2592/262, 8-27=-2977/366, 6-27=-67/142, 7-27=-79/520

NOTES
1) 2 X 8 SYP No.1D bearing block 12" long at jt. 22 attached to front face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners. Bearing is assumed to be SYP.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 9-10, 5-27, 8-27; Wall dead load (5.0psf) on member(s).4-25, 9-23
7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 23-25
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 347 lb uplift at joint 2, 565 lb uplift at joint 22, 405 lb uplift at joint 18 and 323 lb uplift at joint 15.
9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

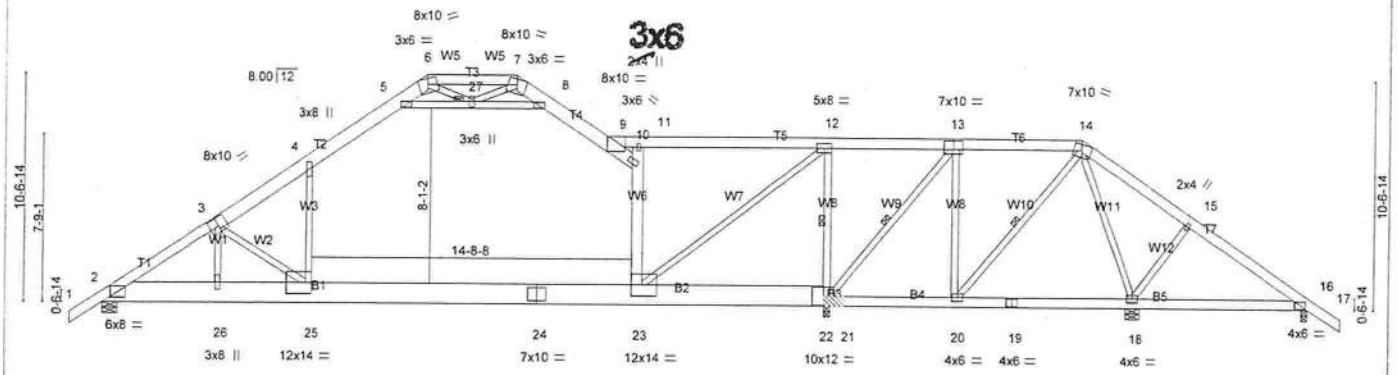
Dwg.#0223071032

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T11	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 12:36:17 2007 Page 1

-1-6-0	5-3-0	9-6-0	15-0-0	19-0-0	23-2-11	24-6-0	33-5-12	39-4-4	45-2-12	50-3-7	56-0-0	57-6-0
1-6-0	5-3-0	4-3-0	5-6-0	4-0-0	4-2-11	1-3-5	8-11-12	5-10-8	5-10-8	5-0-11	5-8-9	1-6-0

Scale = 1:100.8



5-3-0	9-6-0	24-6-0	33-5-12	39-4-4	47-8-0	56-0-0
5-3-0	4-3-0	15-0-0	8-11-12	5-10-8	8-3-12	8-4-0

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [12:0-3-8,0-2-8], [13:0-5-0,0-4-8], [23:0-5-8,0-6-0], [25:0-3-8,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	-0.47	23-25	>836	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.79	23-25	>504	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.03	22	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.17	23-25	>999	240	Weight: 517 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except* T2 2 X 8 SYP No.1D, T4 2 X 8 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D *Except* B1 2 X 12 SYP No.2, B2 2 X 12 SYP No.2
WEBS 2 X 4 SYP No.3 *Except* W6 2 X 6 SYP No.1D, W7 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except 2-0-0 oc purlins (5-9-12 max.): 6-7, 9-10, 11-14.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 12-22, 13-22, 14-20
JOINTS 1 Brace at Jt(s): 10, 27

REACTIONS (lb/size) 2=2035/0-8-0, 22=3246/0-3-13 (0-3-8 + bearing block), 18=808/0-8-0, 16=252/0-3-8
Max Horz 2=352(load case 4)
Max Uplift2=-362(load case 5), 22=-559(load case 6), 18=-409(load case 6), 16=-305(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-3065/323, 3-4=-2637/178, 4-5=-1827/296, 5-6=0/768, 6-7=0/881, 7-8=0/796, 8-9=-1785/316, 9-10=-114/113, 9-11=-2126/275, 11-12=-1918/273, 12-13=-76/958, 13-14=0/382, 14-15=0/443, 15-16=-10/279, 16-17=0/44
BOT CHORD 2-26=-331/2541, 25-26=-333/2542, 24-25=-177/1881, 23-24=-177/1881, 22-23=-1007/382, 21-22=-378/235, 20-21=-389/237, 19-20=-210/200, 18-19=-210/200, 16-18=-174/70
WEBS 3-26=-84/103, 3-25=-860/348, 4-25=0/1233, 10-23=-763/358, 10-11=-503/419, 12-23=-127/3512, 12-22=-2662/422, 13-22=-896/215, 13-20=0/420, 14-20=-274/94, 14-18=-331/144, 15-18=-269/283, 5-27=-2950/313, 8-27=-2983/377, 6-27=-26/270, 7-27=-65/331

NOTES
1) 2 X 6 SYP No.1D bearing block 12" long at jt. 22 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. Bearing is assumed to be SYP.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Provide adequate drainage to prevent water ponding.
5) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 9-11, 5-27, 8-27; Wall dead load (5.0psf) on member(s).4-25, 10-23, 10-11
6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 23-25
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 362 lb uplift at joint 2, 559 lb uplift at joint 22, 409 lb uplift at joint 18 and 305 lb uplift at joint 16.
8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

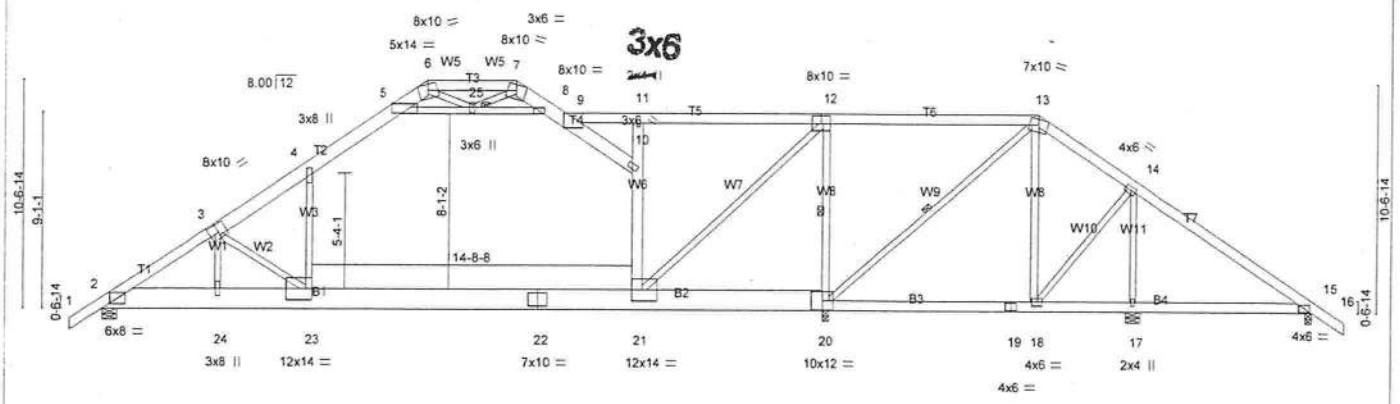
Dwg.#0223071033

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T12	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 16:45:50 2007 Page 1

-1-6-0	5-3-0	9-6-0	15-0-0	19-0-0	21-2-12	24-6-0	33-5-12	43-2-12	47-8-0	56-0-0	57-6-0
1-6-0	5-3-0	4-3-0	5-6-0	4-0-0	2-2-12	3-3-4	8-11-12	9-9-0	4-5-4	8-4-0	1-6-0

Scale = 1:100.8



5-3-0	9-6-0	24-6-0	33-5-12	43-2-12	47-8-0	56-0-0
5-3-0	4-3-0	15-0-0	8-11-12	9-9-0	4-5-4	8-4-0

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [5:Edge,0-5-5], [12:0-5-0,0-4-8], [21:0-5-8,0-6-0], [23:0-3-8,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	-0.44	21-23	>904	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.73	21-23	>545	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.04	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.15	21-23	>999	240	Weight: 521 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except*
T2 2 X 8 SYP No.1D, T4 2 X 8 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D *Except*
B1 2 X 12 SYP No.2, B2 2 X 12 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W6 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except 2-0-0 oc purlins (5-5-2 max.): 6-7, 9-10, 11-13.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-2-6 oc bracing: 21-23
5-0-0 oc bracing: 20-21.
WEBS 1 Row at midpt 12-20, 13-20
JOINTS 1 Brace at Jt(s): 10, 25

REACTIONS (lb/size) 2=2245/0-8-0, 20=2725/0-3-8, 17=870/0-8-0, 15=514/0-3-8
Max Horz 2=352(load case 4)
Max Uplift2=-390(load case 5), 20=-531(load case 4), 17=-401(load case 6), 15=-336(load case 6)
Max Grav 2=2245(load case 1), 20=2794(load case 11), 17=870(load case 1), 15=514(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-3414/371, 3-4=-3051/231, 4-5=-2233/351, 5-6=0/661, 6-7=0/571, 7-8=0/417, 8-9=-1901/338, 9-10=-78/52, 9-11=-2210/330, 11-12=-2229/352, 12-13=-369/357, 13-14=-468/314, 14-15=-342/191, 15-16=0/44
BOT CHORD 2-24=-394/2830, 23-24=-395/2829, 22-23=-258/2237, 21-22=-258/2237, 20-21=-152/413, 19-20=-146/338, 18-19=-146/335, 17-18=-78/184, 15-17=-78/184
WEBS 3-24=-122/85, 3-23=-778/339, 4-23=0/1250, 10-21=-582/348, 10-11=-430/339, 12-21=-148/2889, 12-20=-2542/562, 13-20=-402/218, 13-18=-5/169, 14-18=-111/239, 14-17=-632/258, 5-25=-3246/338, 8-25=-2544/290, 6-25=-49/657, 7-25=-206/121

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 9-11, 5-25, 8-25; Wall dead load (5.0psf) on member(s).4-23, 10-21, 10-11
5) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 21-23
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 390 lb uplift at joint 2, 531 lb uplift at joint 20, 401 lb uplift at joint 17 and 336 lb uplift at joint 15.
7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

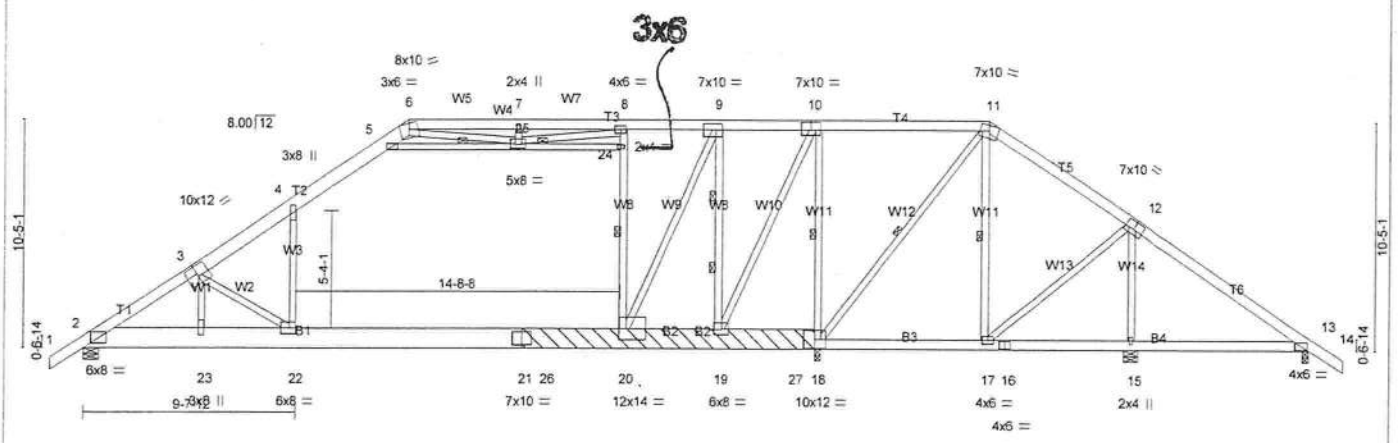
Dwg.#0223071034

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T13	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 12:44:18 2007 Page 1

-1-6-0	5-3-0	9-6-0	14-9-4	19-9-4	24-6-0	33-5-12	41-2-12	47-8-0	56-0-0	57-6-0
1-6-0	5-3-0	4-3-0	5-3-4	5-0-0	4-8-12	8-11-12	7-9-0	6-5-4	8-4-0	1-6-0

Scale = 1:99.3



5-3-0	9-6-0	24-6-0	33-5-12	41-2-12	47-8-0	56-0-0
5-3-0	4-3-0	15-0-0	8-11-12	7-9-0	6-5-4	8-4-0

Plate Offsets (X,Y): [3:0-6-0,0-6-4], [9:0-3-8,0-3-8], [10:0-5-0,0-4-8], [12:0-5-0,0-4-8], [19:0-3-8,0-3-0], [20:0-3-8,0-6-0], [22:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.39	20-22	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.66	20-22	>602	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.94	Horz(TL)	0.05	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.16	20-22	>999	240	Weight: 611 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D *Except*
T2 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 12 SYP No.2 *Except*
B4 2 X 6 SYP No.1D, B3 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W13 2 X 4 SYP No.2, W4 2 X 4 SYP No.2
LBR SCAB 18-21 2 X 12 SYP No.2 one side

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-2-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-11 oc bracing.
WEBS 1 Row at midpt 20-24, 10-18, 11-18, 11-17
2 Rows at 1/3 pts 5-24, 9-19

REACTIONS (lb/size) 2=2382/0-8-0, 18=2329/0-3-8, 15=1010/0-8-0, 13=629/0-3-8
Max Horz 2=348(load case 4)
Max Uplift2=-427(load case 5), 18=-675(load case 3), 15=-430(load case 6), 13=-375(load case 6)
Max Grav 2=2384(load case 10), 18=2456(load case 11), 15=1010(load case 1), 13=629(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-3702/442, 3-4=-3248/280, 4-5=-2527/417, 5-6=-150/482, 6-7=-1160/693, 7-8=-1160/693, 8-9=-2416/448, 9-10=-1334/460, 10-11=-598/470, 11-12=-787/406, 12-13=-560/242, 13-14=0/44
BOT CHORD 2-23=-543/3081, 22-23=-547/3085, 21-22=-406/2420, 21-26=-406/2420, 20-26=-406/2420, 19-20=-368/1334, 19-27=-297/643, 18-27=-298/640, 17-18=-203/570, 16-17=-122/343, 15-16=-122/343, 13-15=-121/370
WEBS 3-23=-92/120, 3-22=-806/359, 4-22=0/1056, 20-24=-322/381, 8-24=-223/385, 10-19=-175/1724, 10-18=-2024/518, 11-18=-163/290, 11-17=-33/139, 12-17=-107/298, 12-15=-730/325, 5-25=-2666/208, 24-25=-41/15, 7-25=-58/160, 8-25=-1245/64, 6-25=-232/1418, 9-19=-2503/172, 9-20=-88/2649

NOTES

1) Attached 13-4-0 scab 18 to 21, front face(s) 2 X 12 SYP No.2 with 2 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except : starting at 11-4-0 from end at joint 18, nail 2 row(s) at 7 o.c. for 2-0-0; starting at 7-10-0 from end at joint 18, nail 2 row(s) at 4 o.c. for 2-0-0.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Provide adequate drainage to prevent water ponding.
5) Ceiling dead load (5.0 psf) on member(s). 4-5, 5-25, 24-25; Wall dead load (5.0psf) on member(s).4-22, 20-24
6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 20-22
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 427 lb uplift at joint 2, 675 lb uplift at joint 18, 430 lb uplift at joint 15 and 375 lb uplift at joint 13.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE, STE E, LUTZ, FL 33549

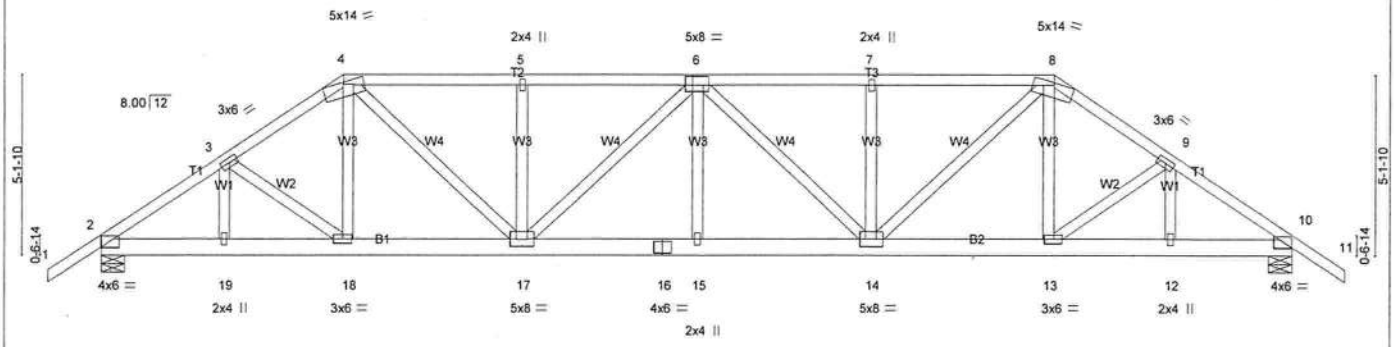
Dwg.#0223071035

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T14	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 12:45:25 2007 Page 1

-1-6-0	3-5-11	6-10-2	11-11-15	17-0-0	22-0-1	27-1-14	30-6-5	34-0-0	35-6-0
1-6-0	3-5-11	3-4-7	5-1-13	5-0-1	5-0-1	5-1-13	3-4-7	3-5-11	1-6-0

Scale = 1:62.2



3-5-11	6-10-2	7-0-0	11-11-15	17-0-0	22-0-1	27-0-0	27-1-14	30-6-5	34-0-0
3-5-11	3-4-7	0-1-14	4-11-15	5-0-1	5-0-1	4-11-15	0-1-14	3-4-7	3-5-11

Plate Offsets (X,Y): [2:0-0-2,0-0-1], [6:0-2-8,0-3-0], [10:0-0-2,0-0-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.16	15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.29	15	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.35	Horz(TL)	0.07	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 458 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=3303/0-8-0, 10=3303/0-8-0
Max Horz 2=-169(load case 2)
Max Uplift2=-1413(load case 3), 10=-1413(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-4849/2137, 3-4=-4912/2260, 4-5=-5684/2682, 5-6=-5683/2683, 6-7=-5683/2683, 7-8=-5684/2682, 8-9=-4912/2261, 9-10=-4849/2139, 10-11=0/41
BOT CHORD 2-19=-1772/3751, 18-19=-1772/3751, 17-18=-1943/4100, 16-17=-2910/6211, 15-16=-2910/6211, 14-15=-2910/6211, 13-14=-1795/4100, 12-13=-1605/3751, 10-12=-1605/3751
WEBS 3-19=-85/87, 3-18=-350/475, 4-18=-234/824, 4-17=-1115/2195, 5-17=-570/641, 6-17=-761/407, 6-15=0/444, 6-14=-761/406, 7-14=-570/641, 8-14=-1115/2195, 8-13=-234/824, 9-13=-352/475, 9-12=-85/86

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1413 lb uplift at joint 2 and 1413 lb uplift at joint 10.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 667 lb down and 333 lb up at 27-0-0, and 667 lb down and 333 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-8=-112(F=-58), 8-11=-54, 2-18=-30, 13-18=-84(F=-54), 10-13=-30
Concentrated Loads (lb)
Vert: 18=-667(F) 13=-667(F)

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

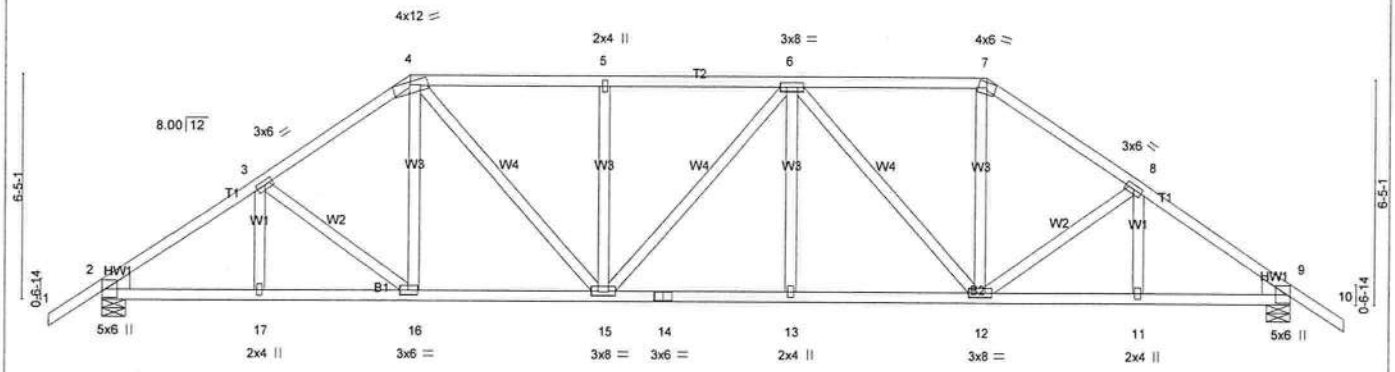
Dwg.#0223071036

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T15	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 12:46:44 2007 Page 1

-1-6-0	4-5-4	8-9-4	14-3-11	19-8-5	25-2-12	29-6-12	34-0-0	35-6-0
1-6-0	4-5-4	4-4-0	5-6-6	5-4-10	5-6-6	4-4-0	4-5-4	1-6-0

Scale = 1:62.2



4-5-4	8-9-4	14-3-11	19-8-5	25-2-12	29-6-12	34-0-0
4-5-4	4-4-0	5-6-6	5-4-10	5-6-6	4-4-0	4-5-4

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.13	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.21	13-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(TL)	0.09	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 209 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 6 SYP No.1D, Right: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-8-6 oc bracing.

REACTIONS (lb/size) 2=1499/0-8-0, 9=1499/0-8-0
Max Horz 2=-216(load case 3)
Max Uplift2=-496(load case 5), 9=-496(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-2057/623, 3-4=-1808/639, 4-5=-1871/720, 5-6=-1871/720, 6-7=-1472/576, 7-8=-1808/639, 8-9=-2057/624, 9-10=0/37
BOT CHORD 2-17=-631/1566, 16-17=-631/1566, 15-16=-583/1461, 14-15=-683/1870, 13-14=-683/1870, 12-13=-683/1870, 11-12=-415/1566, 9-11=-415/1566
WEBS 3-17=0/124, 3-16=-149/152, 4-16=-62/240, 4-15=-382/677, 5-15=-301/263, 6-15=-71/73, 6-13=0/161, 6-12=-668/379, 7-12=-171/690, 8-12=-149/153, 8-11=0/124

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 496 lb uplift at joint 2 and 496 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071037

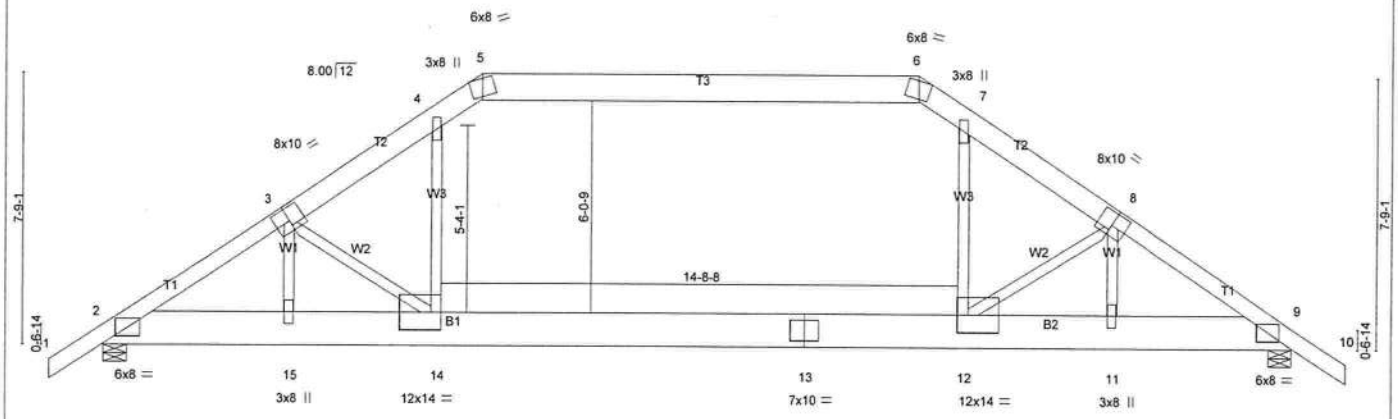
Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T16	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 12:53:01 2007 Page 1

-1-6-0 | 5-3-0 | 9-6-0 | 10-9-4 | 23-2-12 | 24-6-0 | 28-9-0 | 34-0-0 | 35-6-0

1-6-0 | 5-3-0 | 4-3-0 | 1-3-4 | 12-5-7 | 1-3-4 | 4-3-0 | 5-3-0 | 1-6-0

Scale = 1:62.2



5-3-0 | 9-6-0 | 24-6-0 | 28-9-0 | 34-0-0

5-3-0 | 4-3-0 | 15-0-0 | 4-3-0 | 5-3-0

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [8:0-5-0,0-4-8], [12:0-3-8,0-6-0], [14:0-3-8,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.22	12-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.35	12-14	>999	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.06	14	>999	240	Weight: 330 lb	

LUMBER
TOP CHORD 2 X 8 SYP No.1D *Except*
T3 2 X 10 SYP No.2, T1 2 X 6 SYP No.1D, T1 2 X 6 SYP No.1D
BOT CHORD 2 X 12 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-8-5 oc bracing.

REACTIONS (lb/size) 9=2235/0-8-0, 2=2235/0-8-0
Max Horz 2=-246(load case 3)
Max Uplift9=-318(load case 6), 2=-318(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-3302/243, 3-4=-3487/184, 4-5=-2573/291, 5-6=-2771/244, 6-7=-2573/291, 7-8=-3487/184, 8-9=-3302/246, 9-10=0/53
BOT CHORD 2-15=-334/2697, 14-15=-339/2682, 13-14=-151/2771, 12-13=-151/2771, 11-12=-97/2682, 9-11=-92/2697
WEBS 3-15=-624/74, 3-14=-278/384, 4-14=0/1420, 7-12=0/1420, 8-12=-284/384, 8-11=-624/80

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Ceiling dead load (5.0 psf) on member(s). 4-5, 5-6, 6-7; Wall dead load (5.0psf) on member(s).4-14, 7-12
5) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 318 lb uplift at joint 9 and 318 lb uplift at joint 2.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071038

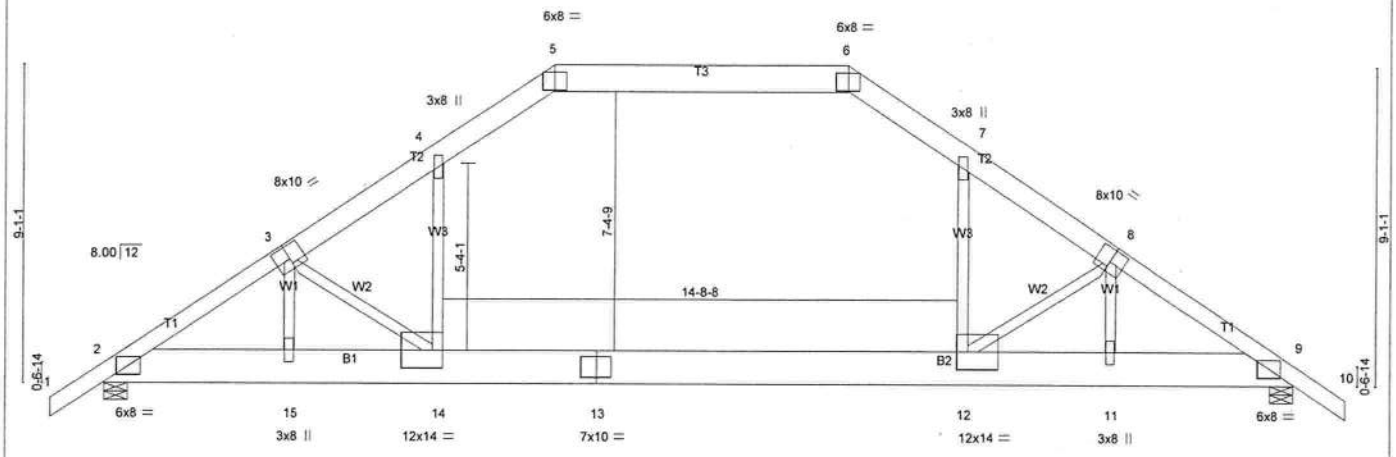
Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T17	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 13:12:58 2007 Page 1

-1-6-0 | 5-3-0 | 9-6-0 | 12-9-4 | 21-2-12 | 24-6-0 | 28-9-0 | 34-0-0 | 35-6-0

1-6-0 | 5-3-0 | 4-3-0 | 3-3-4 | 8-5-7 | 3-3-4 | 4-3-0 | 5-3-0 | 1-6-0

Scale = 1:62.2



5-3-0 | 9-6-0 | 24-6-0 | 28-9-0 | 34-0-0

5-3-0 | 4-3-0 | 15-0-0 | 4-3-0 | 5-3-0

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [5:0-4-0,0-0-1], [6:0-4-0,0-0-1], [8:0-5-0,0-4-8], [12:0-3-8,0-6-0], [14:0-3-8,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.97	Vert(LL)	-0.44	12-14	>904	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.73	12-14	>549	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.59	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.12	12-14	>999	240	Weight: 329 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except*
T3 2 X 10 SYP No.2, T2 2 X 8 SYP No.1D, T2 2 X 8 SYP No.1D
BOT CHORD 2 X 12 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=2235/0-8-0, 9=2235/0-8-0
Max Horz 2=-293(load case 3)
Max Uplift2=-332(load case 5), 9=-332(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-3326/272, 3-4=-3315/178, 4-5=-2298/274, 5-6=-2440/258, 6-7=-2297/274, 7-8=-3315/177, 8-9=-3326/275, 9-10=0/53
BOT CHORD 2-15=-269/2761, 14-15=-275/2748, 13-14=-54/2440, 12-13=-54/2440, 11-12=-72/2748, 9-11=-69/2761
WEBS 3-15=-589/132, 3-14=-496/336, 4-14=0/1612, 7-12=0/1612, 8-12=-496/339, 8-11=-589/138

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 5 and 6.
5) Ceiling dead load (5.0 psf) on member(s). 4-5, 5-6, 6-7; Wall dead load (5.0psf) on member(s).4-14, 7-12
6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 332 lb uplift at joint 2 and 332 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071039

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T18	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 13:16:33 2007 Page 1

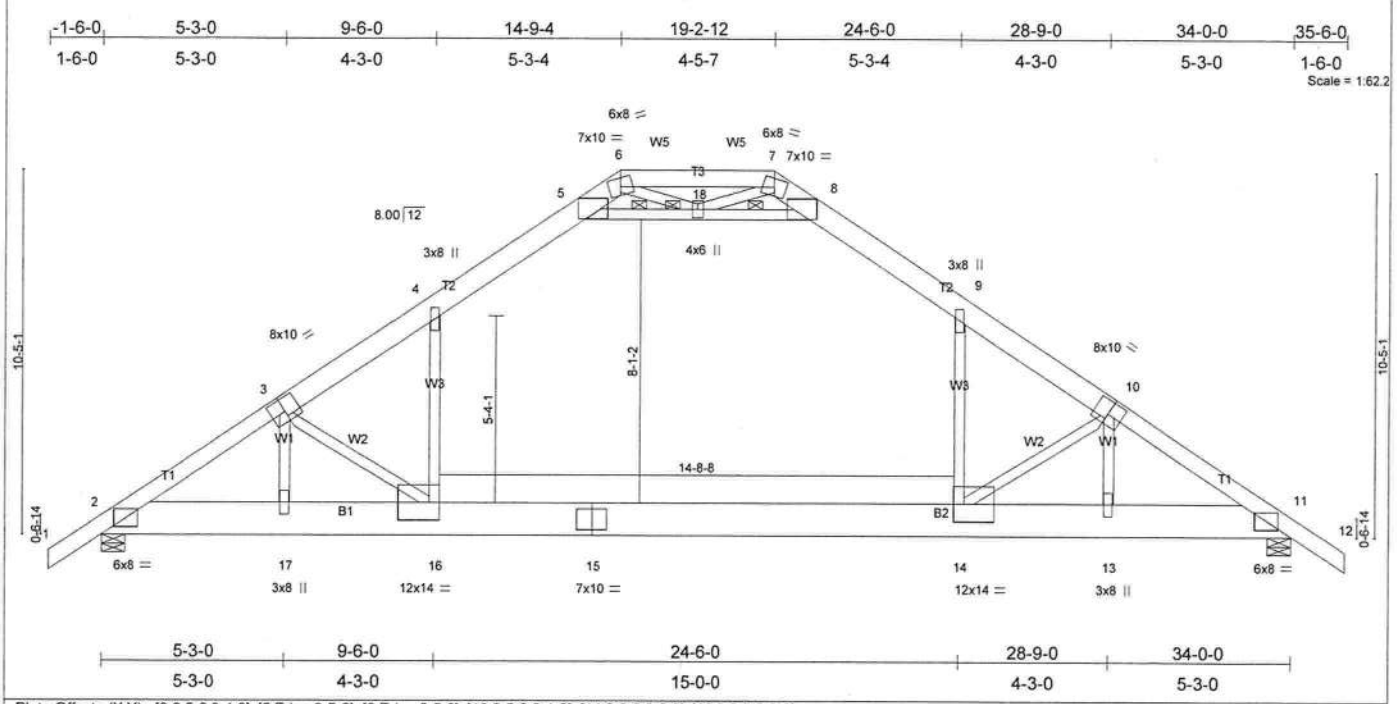


Plate Offsets (X,Y): [3:0-5-0,0-4-8], [5:Edge,0-5-6], [8:Edge,0-5-6], [10:0-5-0,0-4-8], [14:0-3-8,0-6-0], [16:0-3-8,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.74	Vert(LL)	-0.53	14-16	>757	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.88	14-16	>455	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.16	14-16	>999	240	Weight: 337 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except* T2 2 X 8 SYP 2400F 2.0E, T2 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 12 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 8-4-10 oc bracing: 14-16.
WEBS 1 Row at midpt 5-18, 8-18
JOINTS 1 Brace at Jt(s): 18

REACTIONS (lb/size) 2=2235/0-8-0, 11=2235/0-8-0
Max Horz 2=-341(load case 3)
Max Uplift2=-344(load case 5), 11=-344(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-3334/294, 3-4=-3256/206, 4-5=-2225/290, 5-6=0/1517, 6-7=0/1684, 7-8=0/1518, 8-9=-2225/290, 9-10=-3256/206, 10-11=-3334/298, 11-12=0/53
BOT CHORD 2-17=-266/2771, 16-17=-269/2759, 15-16=0/2353, 14-15=0/2353, 13-14=-92/2759, 11-13=-89/2771
WEBS 3-17=-563/153, 3-16=-630/360, 4-16=0/1637, 9-14=0/1637, 10-14=-630/364, 10-13=-563/161, 5-18=-4447/347, 8-18=-4447/346, 6-18=-66/518, 7-18=-66/519

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 5-18, 8-18; Wall dead load (5.0psf) on member(s).4-16, 9-14
5) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 14-16
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 344 lb uplift at joint 2 and 344 lb uplift at joint 11.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071040

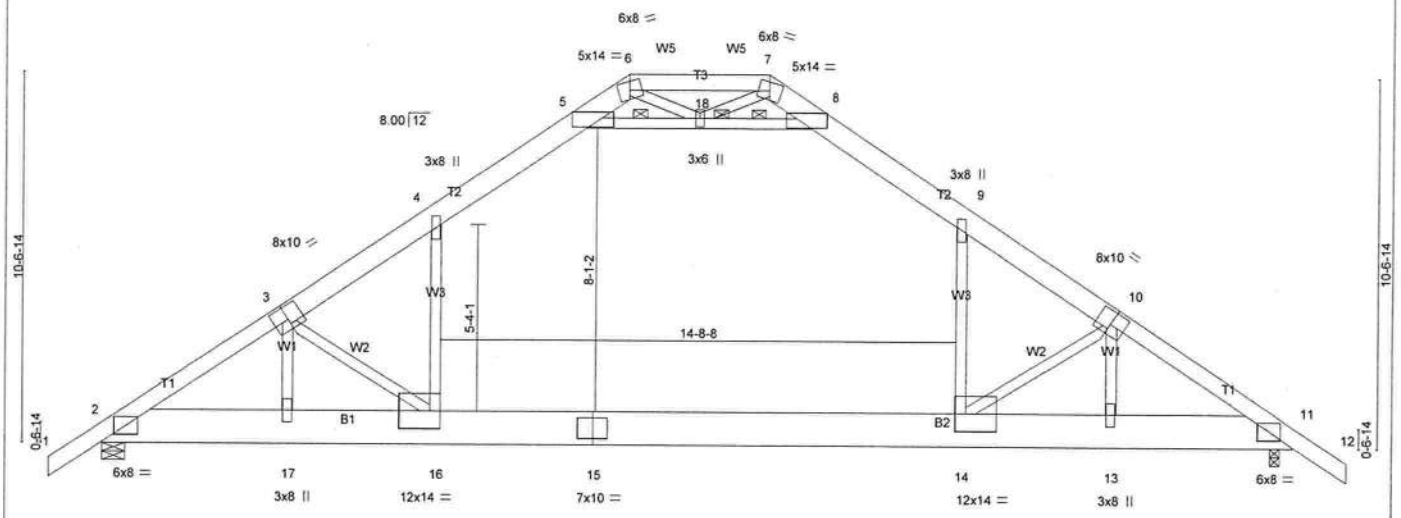
Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T19	ROOF TRUSS	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 13:20:04 2007 Page 1

-1-6-0 | 5-3-0 | 9-6-0 | 15-0-0 | 19-0-0 | 24-6-0 | 28-9-0 | 34-0-0 | 35-6-0

1-6-0 | 5-3-0 | 4-3-0 | 5-6-0 | 4-0-0 | 5-6-0 | 4-3-0 | 5-3-0 | 1-6-0

Scale = 1:62.2



5-3-0 | 9-6-0 | 24-6-0 | 28-9-0 | 33-5-12 | 34-0-0

5-3-0 | 4-3-0 | 15-0-0 | 4-3-0 | 4-8-12 | 0-6-4

Plate Offsets (X,Y): [3:0-5-0,0-4-8], [5:Edge,0-5-5], [8:Edge,0-5-5], [10:0-5-0,0-4-8], [14:0-3-8,0-6-0], [16:0-3-8,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.75	Vert(LL)	-0.53	14-16	>755	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.89	14-16	>454	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.16	14-16	>999	240	Weight: 337 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except* T2 2 X 8 SYP 2400F 2.0E, T2 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 12 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins, except 2-0-0 oc purlins (10-0-0 max.): 6-7.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except: 8-4-10 oc bracing: 14-16.
WEBS 1 Row at midpt 5-18, 8-18
JOINTS 1 Brace at Jt(s): 18

REACTIONS (lb/size) 2=2247/0-8-0, 11=2228/0-3-8
Max Horz 2=-347(load case 3)
Max Uplift2=-346(load case 5), 11=-339(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-3351/298, 3-4=-3294/216, 4-5=-2253/298, 5-6=0/1363, 6-7=0/1572, 7-8=0/1358, 8-9=-2250/296, 9-10=-3300/214, 10-11=-3430/319, 11-12=0/52
BOT CHORD 2-17=-271/2785, 16-17=-274/2773, 15-16=0/2386, 14-15=0/2386, 13-14=-112/2868, 11-13=-109/2879
WEBS 3-17=-587/153, 3-16=-613/359, 4-16=0/1655, 9-14=0/1670, 10-14=-723/383, 10-13=-543/163, 5-18=-4268/356, 8-18=-4258/350, 6-18=-77/431, 7-18=-77/420

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 5-18, 8-18; Wall dead load (5.0psf) on member(s).4-16, 9-14
5) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 14-16
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 346 lb uplift at joint 2 and 339 lb uplift at joint 11.
7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071041

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T20	ROOF TRUSS	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 13:23:16 2007 Page 1

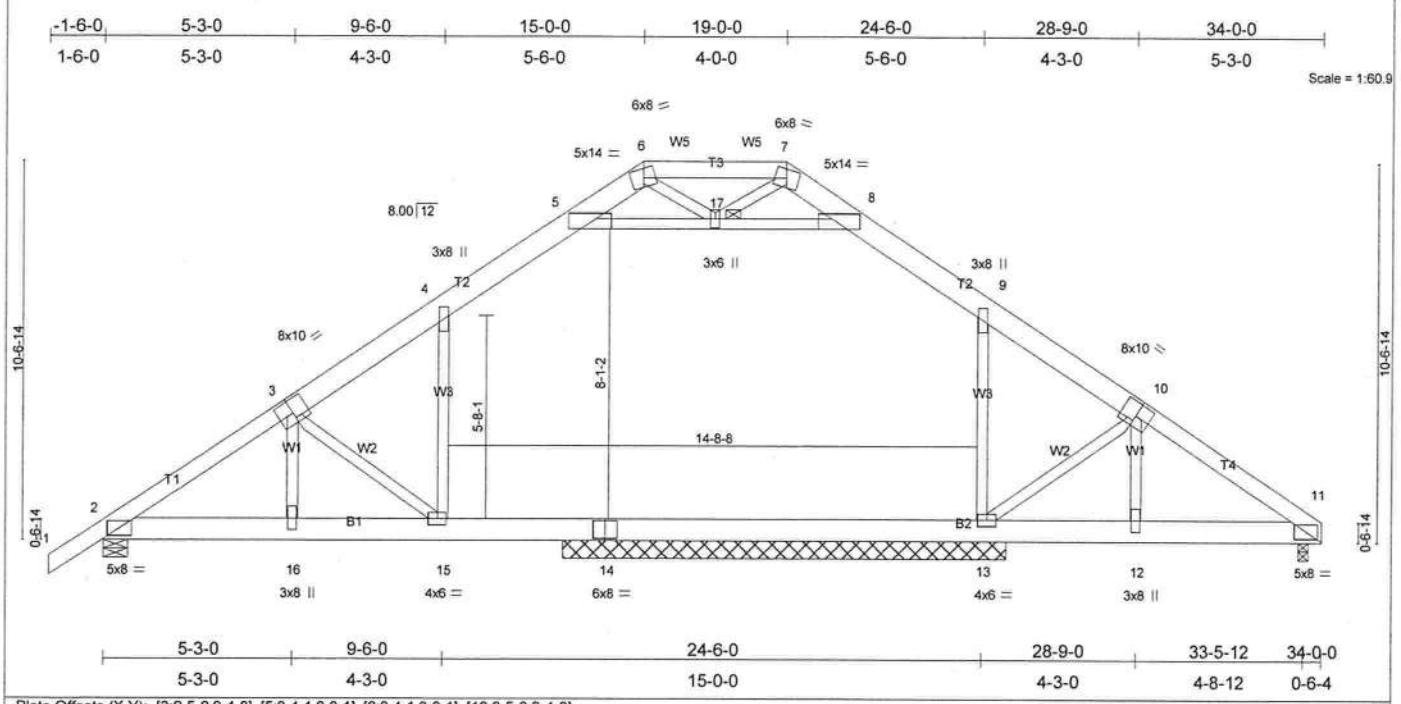


Plate Offsets (X,Y): [3:0-5-0,0-4-8], [5:0-4-1,0-0-1], [8:0-4-1,0-0-1], [10:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-4-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.69	Vert(LL)	-0.39	13-15	>742	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.63	13-15	>458	240		
BCLL	10.0	Rep Stress Incr	NO	WB	0.31	Horz(TL)	0.03	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.11	13-15	>999	240	Weight: 560 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except* T2 2 X 8 SYP No.1D, T2 2 X 8 SYP No.1D
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.) (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 6, 7, 17

REACTIONS (lb/size) 2=2330/0-8-0, 13=1063/12-4-0, 11=1729/0-3-8
Max Horz 2=431(load case 3)
Max Uplift2=-430(load case 4), 13=-268(load case 2), 11=-261(load case 4)
Max Grav 2=2330(load case 1), 13=1268(load case 10), 11=1729(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/56, 2-3=-3456/409, 3-4=-3168/258, 4-5=-2097/364, 5-6=0/624, 6-7=0/831, 7-8=0/800, 8-9=-2238/398, 9-10=-2901/341, 10-11=-2534/426
BOT CHORD 2-16=-427/2827, 15-16=-429/2822, 14-15=-239/2230, 13-14=-239/2230, 12-13=-281/2052, 11-12=-280/2074
WEBS 3-16=-257/19, 3-15=-775/400, 4-15=0/1673, 9-13=-227/1224, 10-13=-18/408, 10-12=-877/0, 5-17=-2944/364, 8-17=-3441/463, 6-17=-130/126, 7-17=-70/546

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 5-17, 8-17; Wall dead load (5.0psf) on member(s).4-15, 9-13
7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 430 lb uplift at joint 2, 268 lb uplift at joint 13 and 261 lb uplift at joint 11.
9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0223071042

Job	Truss	Truss Type	Qty	Ply	WAGONER RES. DON REED
L227913	T21	ROOF TRUSS	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 21 13:24:11 2007 Page 1

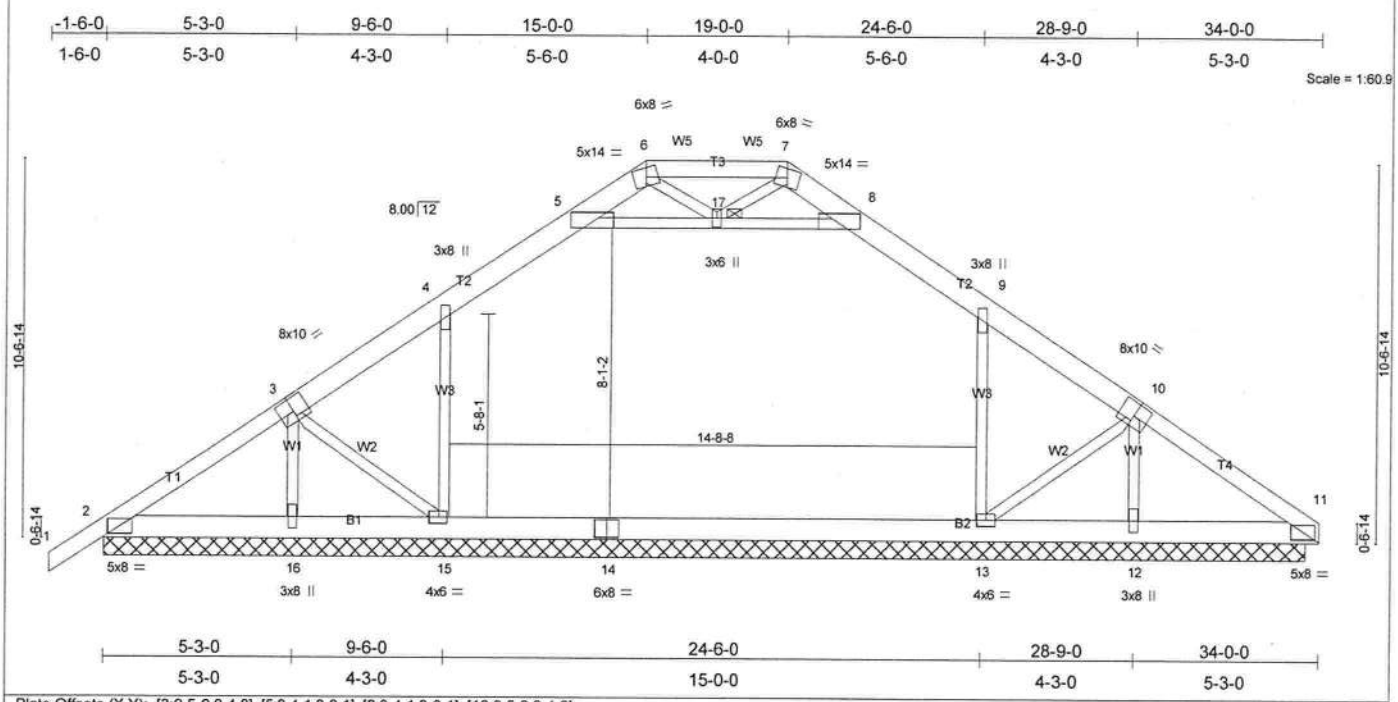


Plate Offsets (X,Y): [3:0-5-0,0-4-8], [5:0-4-1,0-0-1], [8:0-4-1,0-0-1], [10:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-4-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.04	Vert(LL)	0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	0.00	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.05	Horz(TL)	0.00	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 560 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except* T2 2 X 8 SYP No.1D, T2 2 X 8 SYP No.1D
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.) (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 6, 7, 17

REACTIONS (lb/size) 2=585/33-7-8, 16=452/33-7-8, 15=724/33-7-8, 13=719/33-7-8, 12=473/33-7-8, 11=474/33-7-8
Max Horz 2=431(load case 3)
Max Uplift2=-189(load case 4), 16=-211(load case 4), 15=-276(load case 4), 13=-278(load case 5), 12=-215(load case 5), 11=-93(load case 4)
Max Grav 2=585(load case 1), 16=452(load case 1), 15=791(load case 9), 13=786(load case 10), 12=473(load case 1), 11=474(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-530/281, 3-4=-612/316, 4-5=-637/342, 5-6=-443/283, 6-7=-371/229, 7-8=-443/268, 8-9=-637/334, 9-10=-613/287, 10-11=-529/191
BOT CHORD 2-16=-268/372, 15-16=-264/361, 14-15=-250/450, 13-14=-250/450, 12-13=-153/363, 11-12=-154/374
WEBS 3-16=-425/202, 3-15=-53/121, 4-15=-361/314, 9-13=-359/302, 10-13=-123/129, 10-12=-429/181, 5-17=-100/228, 8-17=-82/211, 6-17=-91/92, 7-17=-89/91

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 189 lb uplift at joint 2, 211 lb uplift at joint 16, 276 lb uplift at joint 15, 278 lb uplift at joint 13, 215 lb uplift at joint 12 and 93 lb uplift at joint 11.
7) Non Standard bearing condition. Review required.
8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

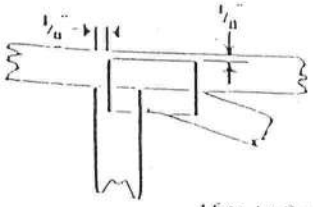
LOAD CASE(S) Standard

FEBRUARY 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

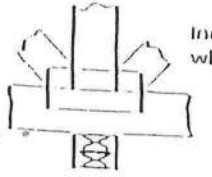
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

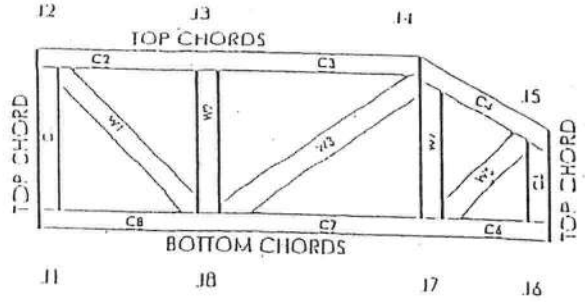
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

Don Reed Const. Fax 755-7272

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001
ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS LISTED ARE SUBJECT TO CHANGE

EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INCLUDE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 SECTION 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND TWO-FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: U.S. HIGHWAY 41 FROM COLUMBIA COUNTYS NORTHERN BOUNDARY TO THE INTERSECTION OF MYRTIS ROAD, FOLLOW MYRTIS EAST TO THE INTERSECTION OF C.R. 245, FOLLOW C.R. 245 SOUTH TO THE SOUTHERN BOUNDARY OF COLUMBIA COUNTY.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ---------- 100 MPH
   ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------- 110 MPH
2. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

GENERAL REQUIREMENTS: Two (2) complete set of plans containing the following:

Applicant	Plans Examiner	
✓	☐	All drawings must be clear, concise and drawn to scale("Optional"details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans
✓	☐	Designer's name and signature on document(FBC 104.2.1) If licensed architect or engineer, official seal shall be affixed
✓	☐	**Site Plan including:**
		a) Dimensions of lot
		b) Dimensions of building setbacks
		c) Location of all other buildings on lot , well and septic tank if applicable, and all utility easements.
		d) Provide a full legal description of property
✓	☐	**Wind-load Engineering Summary, calculations and any details required**
		a) Plans or specifications must state compliance with FBC Section 1606
		b) The following information must be shown as per secition 1606 1.7 FBC
		a. Basic wind speed (MPH)
		b. Wind importance factor (I) and building category
		c. Wind exposure - if more than one wind expore is used , the wind exposure and applicable wind dirceion shall be indicated
		d. The applicable internal pressure coefficient
		e. Components and Cladding. The design wind pressure in terms of psf (kN/m^2), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional
		Elevations including:
✓	☐	a) All Sides
✓	☐	b) Roof pitch
✓	☐	c) Overhang dimensions and detail with attic ventilation

d) Location, size and height above roof of chimneys
e) Location and size of skylights
d) Building height
e) Number of stories

Floor Plan including:

a) Rooms labeled and dimensioned
b) Shear walls
c) Windows and Doors(including garage doors) showing size, mfg, approval listing and attachmenspecs.(FBC1707)and safety glazing where needed (egress windows in bedrooms to be shown)
d) Fireplaces (gas appliance(vented or non-vented) or wood burning with hearth
e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
f) Must show and identify accessability requirements (accessible bathroom)

Foundation Plan including:

a) Location of all load bearing walls with required footings indicated as standard or monolithic and their dimensions and reinforcing
b) All posts and/or column footing including size and reinforcing
c) Any special support required by soil analysis such as piling
d) Location of any vertical steel

Roof System

a) Truss package including:
   1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
   2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

b) Conventional Framing Layout including
   1. Rafter size, species and spacing
   2. Attachment to wall and uplift
   3. Ridge Beam sized and valley framing and support details
   4. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

a) Masonry wall
   1. All materials making up wall
   2. Block size and motar type with size and spacing of reinforcement
   3. Lintel, tie-beam sizes and reinenforcement
   4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
   5. All required connectors with uplift rating and required number and size of fasteners for continous tie from roof to foundation
   6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
   7. Fire resistant construction (if required)
   8. Fireproofing requirements
   9. Show type of termite treatment (termiticide or alternative method)
   10. Slab on grade
      a. Vapor retarder (6 mil. polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or

welded wire fabric reinfrocement and supports
11. Indicate where pressuretreated wood will be placed
12. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

b) Wood Frame wall
1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required connectors with uplift rating and required number and size of fasteners for continous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fasting requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if required)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6 mil polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinenforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

c) Metal Frame wall and roof (Designed, signed and sealed by Fl. Reg. Prof. Engineer or Architect)

Floor Framing System

a) Floor truss package including layout and details signed and sealed by Fl. Reg. P.E.
b) Floor joist size and spacing
c) Girder size and spacing
d) Attachment of joist to girder
e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
b) Ceiling fans
c) Smoke detectors
d) Service panel and sub-panel size and location(s)
e) Meter location with type of service entrance (overhead or underground)
f) Appliances and HVAC equipment

HVAC information

a) Manual J sizing equipment or equivalent computation
b) Exhaust fans in bathrooms

☑ ☐ **Energy Calculations** (dimensions shall match plans)

☑ ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders — only if ~~owner~~ owner is building

Notice of Commencement

Private Potable Water

a) Size of pump motor

b) Size of pressure tank

c) Cycle Stop Valve if used

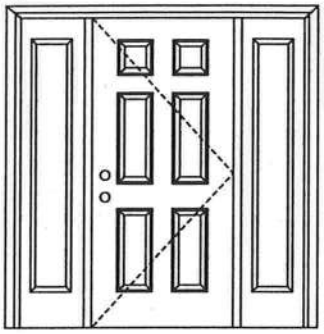
OXO

Opaque Outswing Unit

COP-WL-MA0124-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Single Door with 2 Sidelites
Maximum unit size = 5'4" x 6'8"

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED on opaque panel, but is required on glazed panels.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0014-02 or MAD-WL-MA0017-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft™ Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Masonite®

OXO

Opaque Outswing Unit

COP-WL-MA0124-02

FIBERGLASS DOORS

APPROVED SIDELITE STYLES:


129 Series

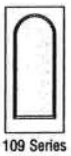

200 Series

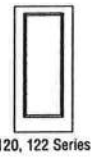

12R, 12L, 23R, 23L, 24R, 24L Series


450 Series


152 Series


149 Series


109 Series


120, 122 Series


300 Series

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panel glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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Masonite®

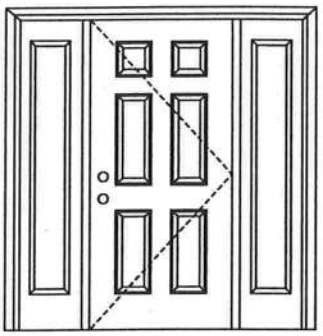
OXO

Opaque Inswing Unit

COP-WL-MA0104-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



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Single Door with 2 Sidelites
Maximum unit size = 5'4" x 6'8"

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limited water unless special threshold design is used.

Large Missile Impact Resistance

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MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

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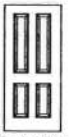
APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft™ Wood-Grain & Textured FIBERGLASS ENTRY DOORS

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March 10, 2003
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Masonite®

OXO
Opaque Inswing Unit

COP-WL-MA0104-02

FIBERGLASS DOORS

APPROVED SIDELITE STYLES:

129 Series | 200 Series | 12R, 12L, 23R, 23L, 24R, 24L Series | 450 Series | 152 Series | 149 Series | 109 Series | 120, 122 Series | 300 Series

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panel glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
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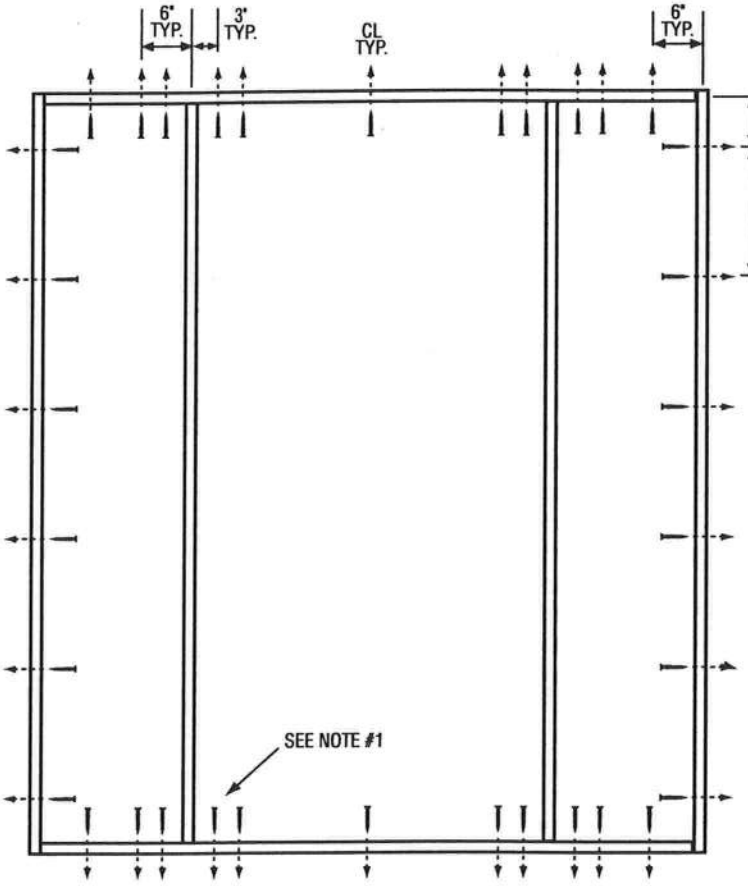
Masonite®

OXO
Unit

MID-WL-MA0004-02

SINGLE DOOR WITH 2 SIDELITES



Minimum Fastener Count

- 6 per vertical framing member
- 11 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0249*, 0269*, 3244*, 3249, 3264* or 3269**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Mrach 10, 2003

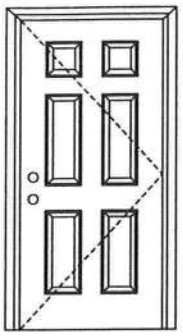
Masonite®

X
Opaque Inswing Unit

COP-WL-MA0101-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etisemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+76.0/-76.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

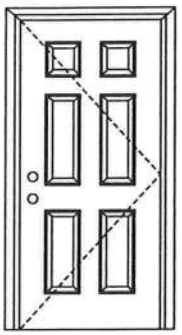
Masonite®

X
Opaque Outswing Unit

COP-WL-MA0121-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure

+76.0/-76.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0011-02.

MINIMUM INSTALLATION DETAIL:

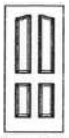
Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

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Masonite®

X
Opaque Outswing Unit

COP-WL-MA0121-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

NCTL 210-1973-1, 2, 3

Certifying Engineer and License Number: Barry Portney, P.E. 16258

CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3025447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft™
Wood-Grain Textured
FIBERGLASS ENTRY DOORS

ARTEK™
Non-Textured Fiberglass Entry Doors

March 10, 2003
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Masonite®

X
Opaque Inswing Unit

COP-WL-MA0101-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

NCTL 210-1973-1, 2, 3

Certifying Engineer and License Number: Barry Portney, P.E. 16258

CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996.

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Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

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March 10, 2003
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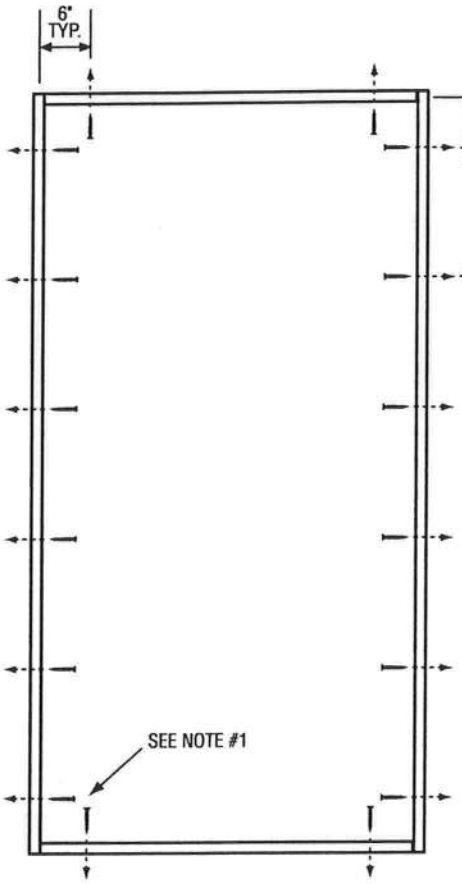
Masonite®

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0246*, 0266*, 3241*, 3246, 3261* or 3266**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 10, 2003

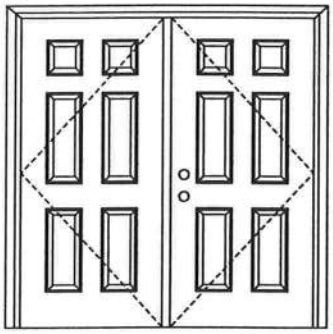
Masonite®

XX
Opaque Outswing Unit

COP-WL-MA0122-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

MINIMUM ASSEMBLY DETAIL:

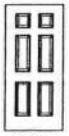
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02.

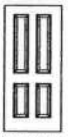
MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft™ Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
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Masonite®

XX
Opaque Outswing Unit

COP-WL-MA0122-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft
Wood-Grain & Textured
FIBERGLASS ENTRY DOORS

ARTEK™
Non-Textured Fiberglass Entry Doors

March 10, 2003
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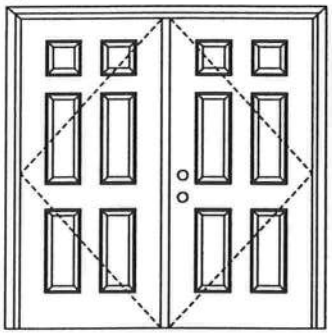
Masonite®

XX
Opaque Inswing Unit

COP-WL-MA0102-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Double Door
Maximum unit size = 6'0" x 6'8"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft™ Wood-Grain Textured FIBERGLASS ENTRY DOORS

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Masonite®

XX
Opaque Inswing Unit

COP-WL-MA0102-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

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Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

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State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

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FIBERGLASS ENTRY DOORS

ARTEK™
Non-Textured Fiberglass Entry Doors

March 10, 2003
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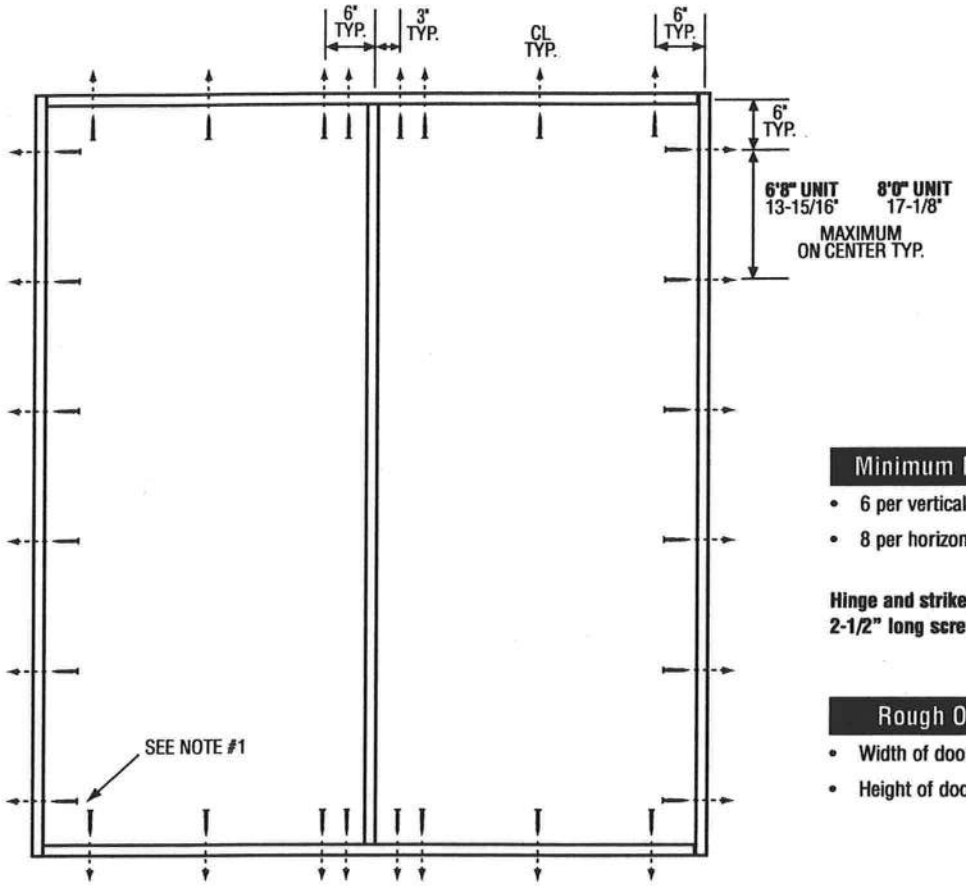
Masonite®

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0247*, 0267*, 3242*, 3247, 3262* or 3267**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 10, 2003

Masonite®

ATN: WEGGIE

Columbia County Building Department
Culvert Waiver

Culvert Waiver No. 000001359

DATE: 04/03/2007 BUILDING PERMIT NO. 25685

APPLICANT KATIE REED PHONE 752-4072

ADDRESS 2230 SE BAYA DRIVE LAKE CITY FL 32025

OWNER CARL & LYNN WAGONER PHONE 407 423-3694

ADDRESS 274 NW EVERETT TERR WHITE SPRING FL 32096

CONTRACTOR DON REED CONSTRUCTION PHONE 752-4072

LOCATION OF PROPERTY 41N, TL ON SUWANNEE VALLEY RD, TR ON EVERETT TERR, 1/4 MILE ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT LEVINGS UNREC 6

PARCEL ID # 20-2S-16-01660-006

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: Katie Reed

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

____ APPROVED ✓ NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS: Needs Culvert

SIGNED: [signature] DATE: 4-6-07

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

RECEIVED
APR 05 2007
By: _____

COLUMBIA COUNTY GATEWAY

CERTIFICATE OF

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 20-2S-16-01660-006 **Building permit No.** 000025685

Use Classification SFD,UTILITY **Fire:** 134.42

Permit Holder DON REED CONSTRUCTION **Waste:** 184.25

Owner of Building CARL & LYNN WAGONER **Total:** 318.67

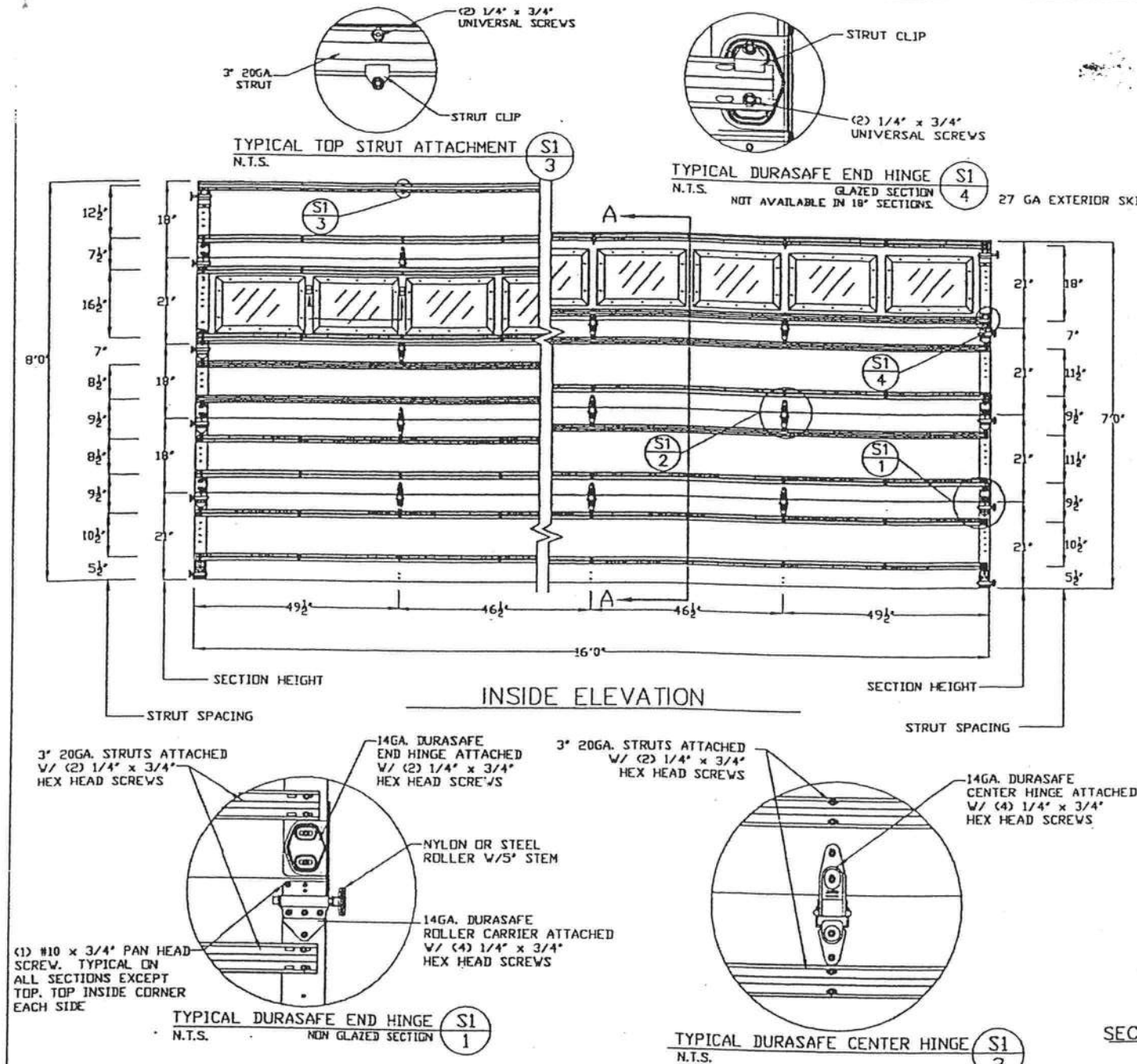
Location: 274 NW EVERETT TERR, WHITE SPRINGS, FL

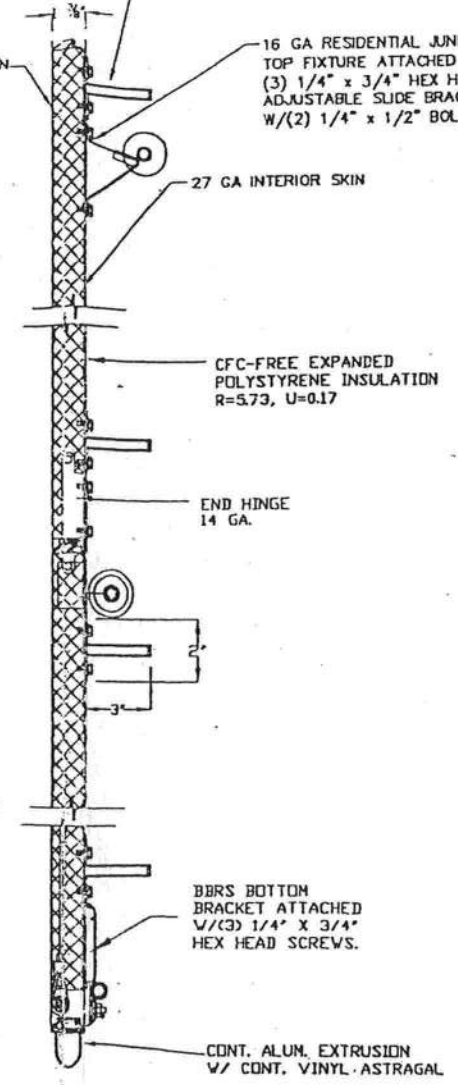
Date: 11/13/2007

Wayne A. Russ

Building Inspector

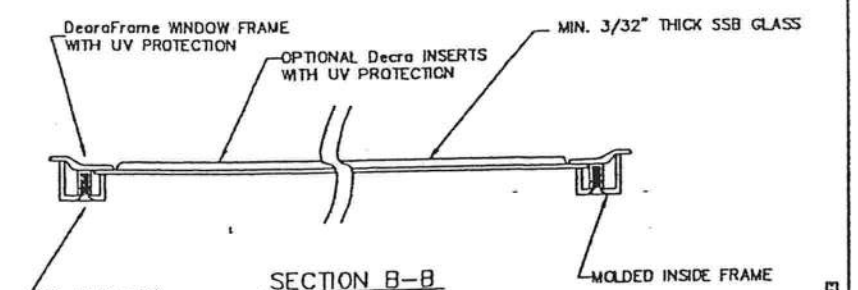
POST IN A CONSPICUOUS PLACE
(Business Places Only)

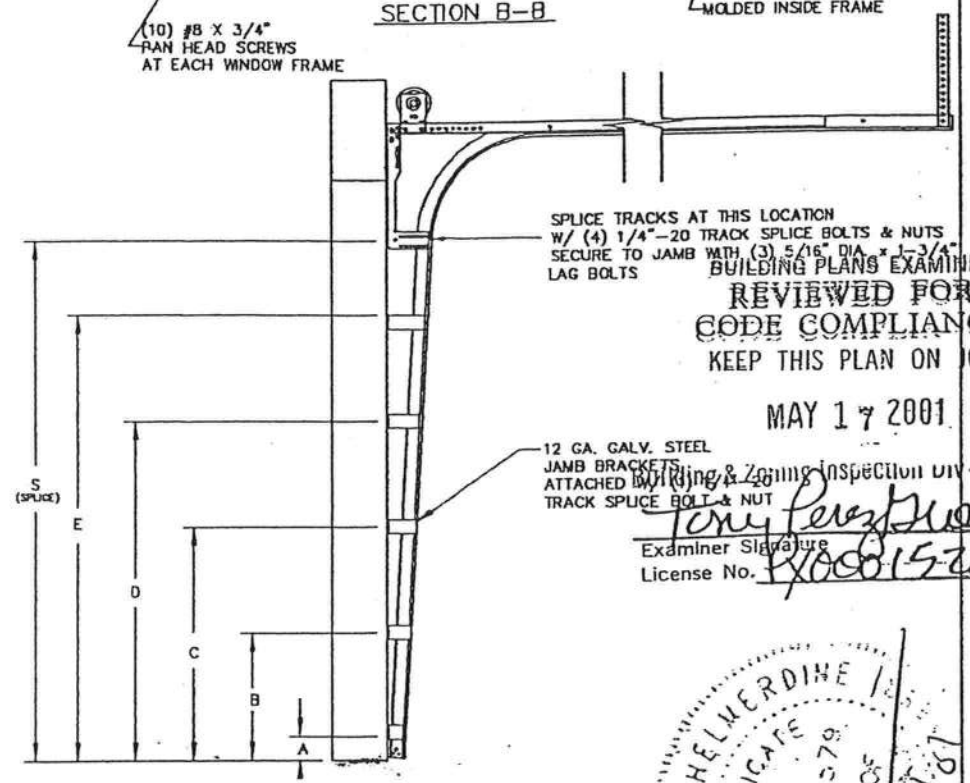




GLAZING OPTION CROSS SECTION

TEST No. SBC-580-020 ON MAY 24, 2000 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +49.5 PSF AND -51.9 PSF. BY COMPARISION, EIGHT (8) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 16' x 7' AND 16' X 8' MODEL 1500-D DOORS.



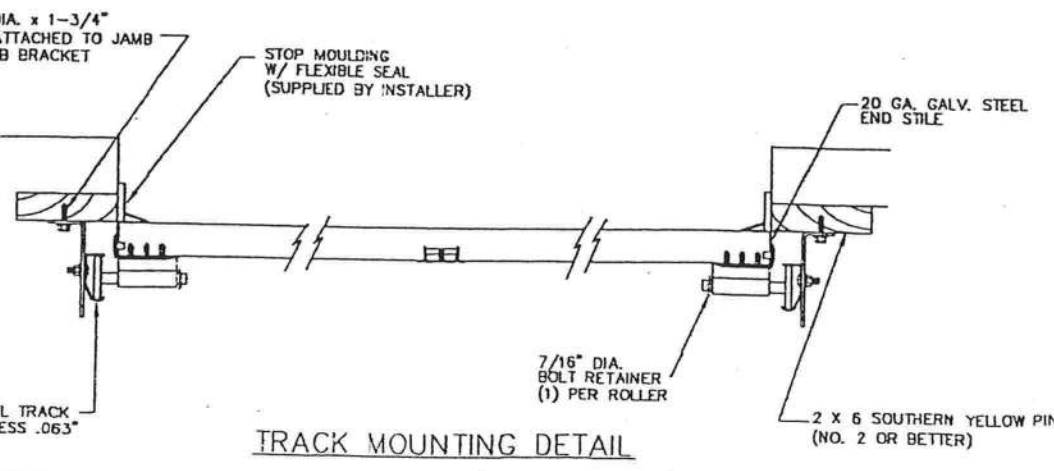


TRACK CONFIGURATION FOR 6'6" UP TO 8' TALL DOORS

JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'-6"	4"	21-1/2"	39"	57"		70"
7'-0"	4"	21-1/2"	42"	63"		76"
7'-6"	4"	18-1/2"	36"	54"	72"	82"
8'-0"	4"	21-1/2"	39"	57"	75"	88"

BUILDING PLANS EXAMINER
REVIEWED FOR CODE COMPLIANCE
KEEP THIS PLAN ON JOB
MAY 17 2001
Building & Zoning Inspection Div - Jax., FL.
Examiner Signature
License No. RA0015720



TRACK MOUNTING DETAIL

WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 110 MPH FASTEST-MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 2,300 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C-90 BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 2-3/4" STARTING 6" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 16" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 27 GA. MIN. (.018") INTERIOR AND EXTERIOR ROLLED FORMED LIGHT COMMERCIAL QUALITY, G-40 GALVANIZATION
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTRED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
   A. BASIC WIND SPEED OF 110 MPH
   B. DOOR CAN BE INSTALLED WITH 5 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   C. 15° MEAN ROOF HEIGHT AT ANY SLOPE
   D. USE FACTOR OF 1.0
   E. EXPOSURE RATING OF C

MAX SIZE 16' x 8'

DESIGN LOADS +29.5 PSF -29.5 PSF

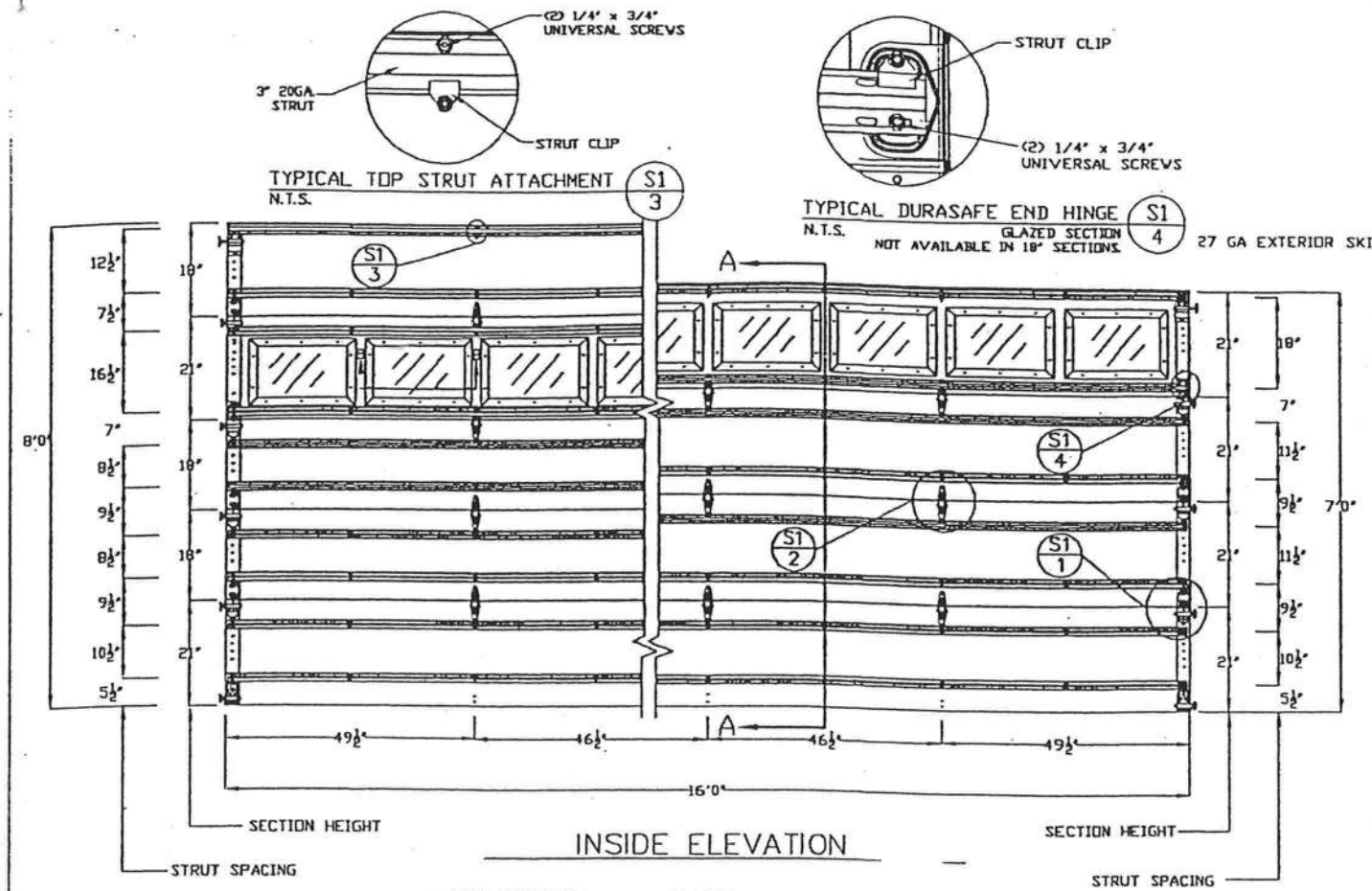
TEST LOADS +45.3 PSF -45.3 PSF

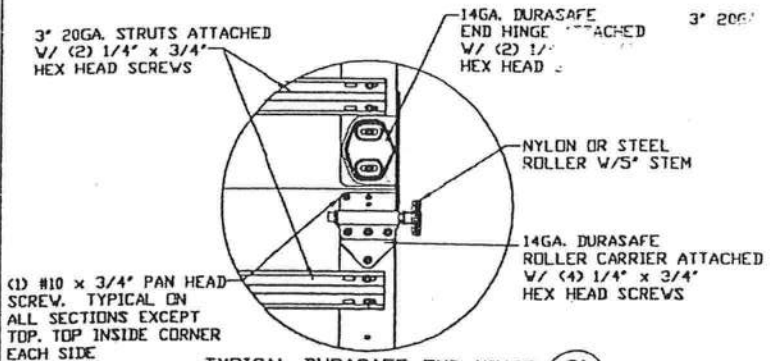
REV	DESCRIPTION OF REVISIONS	DATE	BY

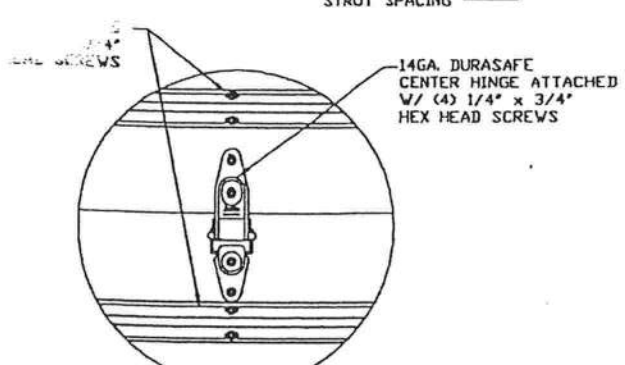
Amarr GARAGE DOORS
5501 GLASSY CREEK BLVD. WINSTON-SALEM, N.C. 27105

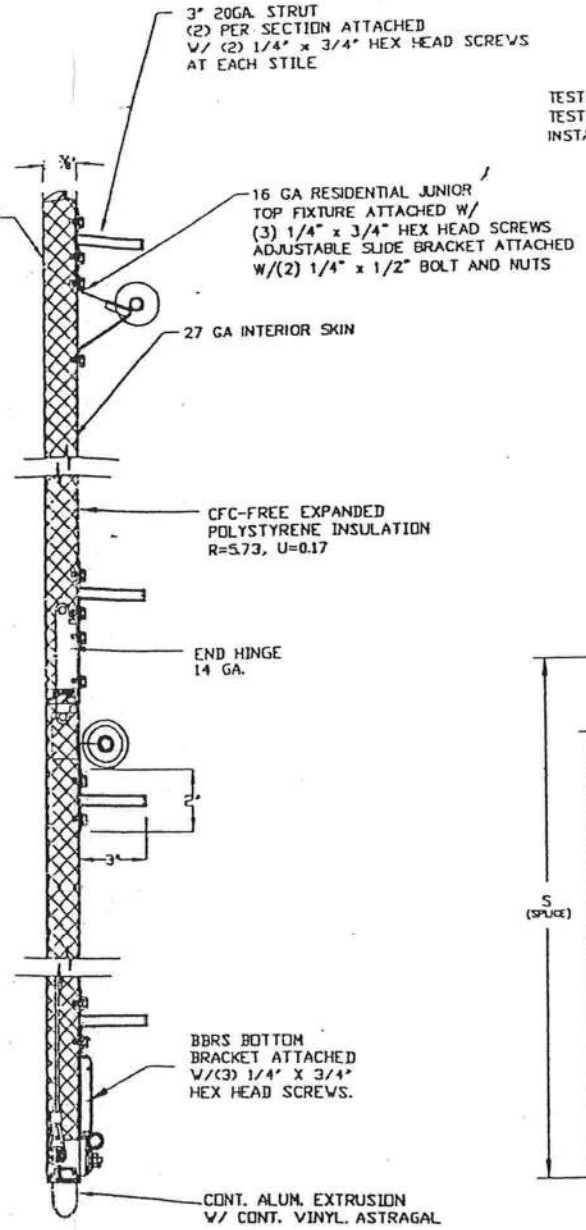
MODEL #1500 WeatherGuard

SIZE	DRAWN BY DJ	DATE 04/18/01	DRAWING NUMBER
B	CHECKED BY	DATE	SBC-580-019-J
SCALE: NOT TO SCALE			SHEET 1 OF 1



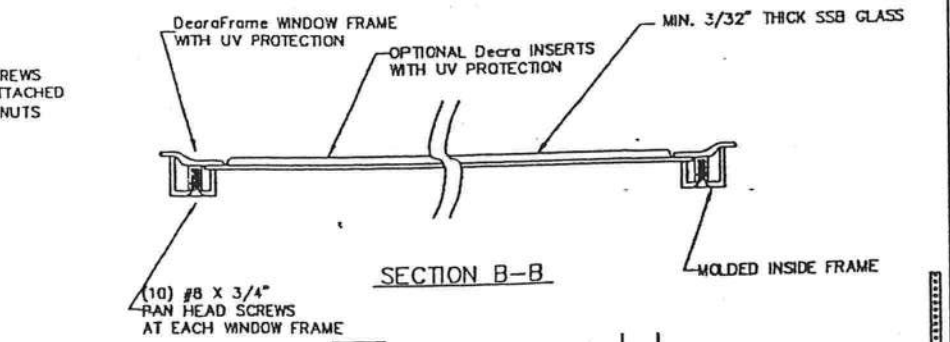


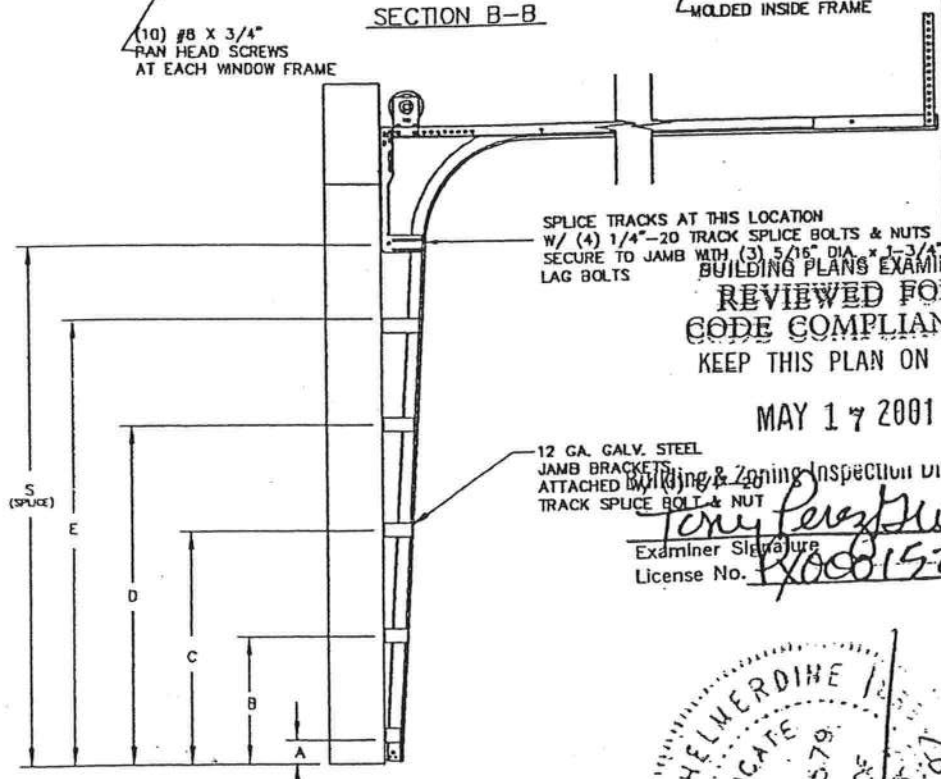




GLAZING OPTION CROSS SECTION

TEST No. SBC-580-020 ON MAY 24, 2000 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +49.5 PSF AND -51.9 PSF. BY COMPARISION, EIGHT (8) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 16' X 7' AND 16' X 8' MODEL 1500-D DOORS.



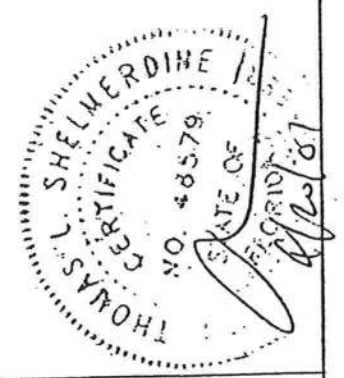


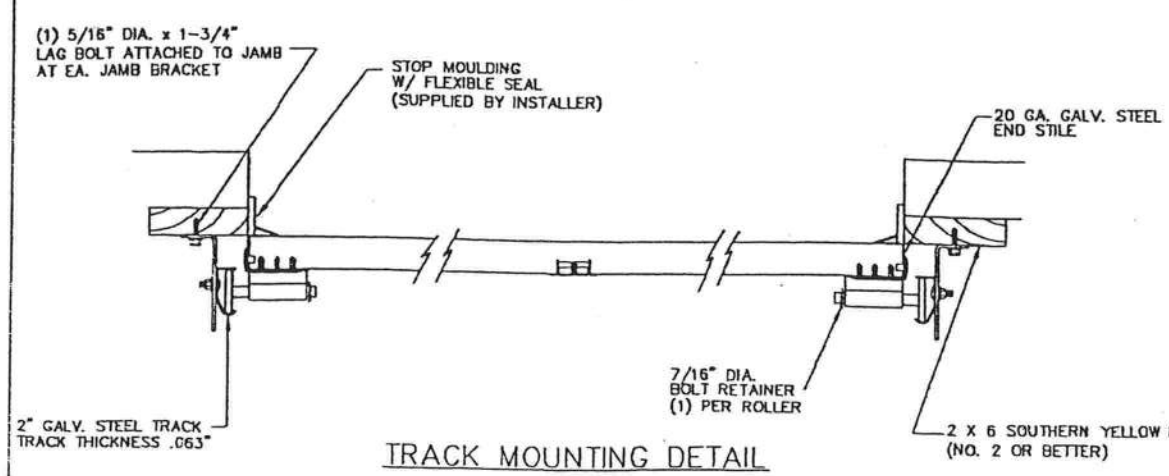
TRACK CONFIGURATION FOR 6'6" UP TO 8' TALL DOORS

JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'-6"	4"	21-1/2"	39"	57"		70"
7'-0"	4"	21-1/2"	42"	63"		76"
7'-6"	4"	18-1/2"	36"	54"	72"	82"
8'-0"	4"	21-1/2"	39"	57"	75"	88"

BUILDING PLANS EXAMINER
REVIEWED FOR CODE COMPLIANCE
KEEP THIS PLAN ON JOB
MAY 17 2001
Building & Zoning Inspection Div. - Jax., FL.
Examiner Signature
License No. RX0015?0





WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 110 MPH FASTEST-MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 2 000 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 8" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 8" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C-90 BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 2-3/4" STARTING 8" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 16" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 27 GA. MIN. (.016") INTERIOR AND EXTERIOR ROLLED FORMED LIGHT COMMERCIAL QUALITY, G-40 GALVANIZATION
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
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   B. DOOR CAN BE INSTALLED WITH 5 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   C. 15° MEAN ROOF HEIGHT AT ANY SLOPE
   D. USE FACTOR OF 1.0
   E. EXPOSURE RATING OF C

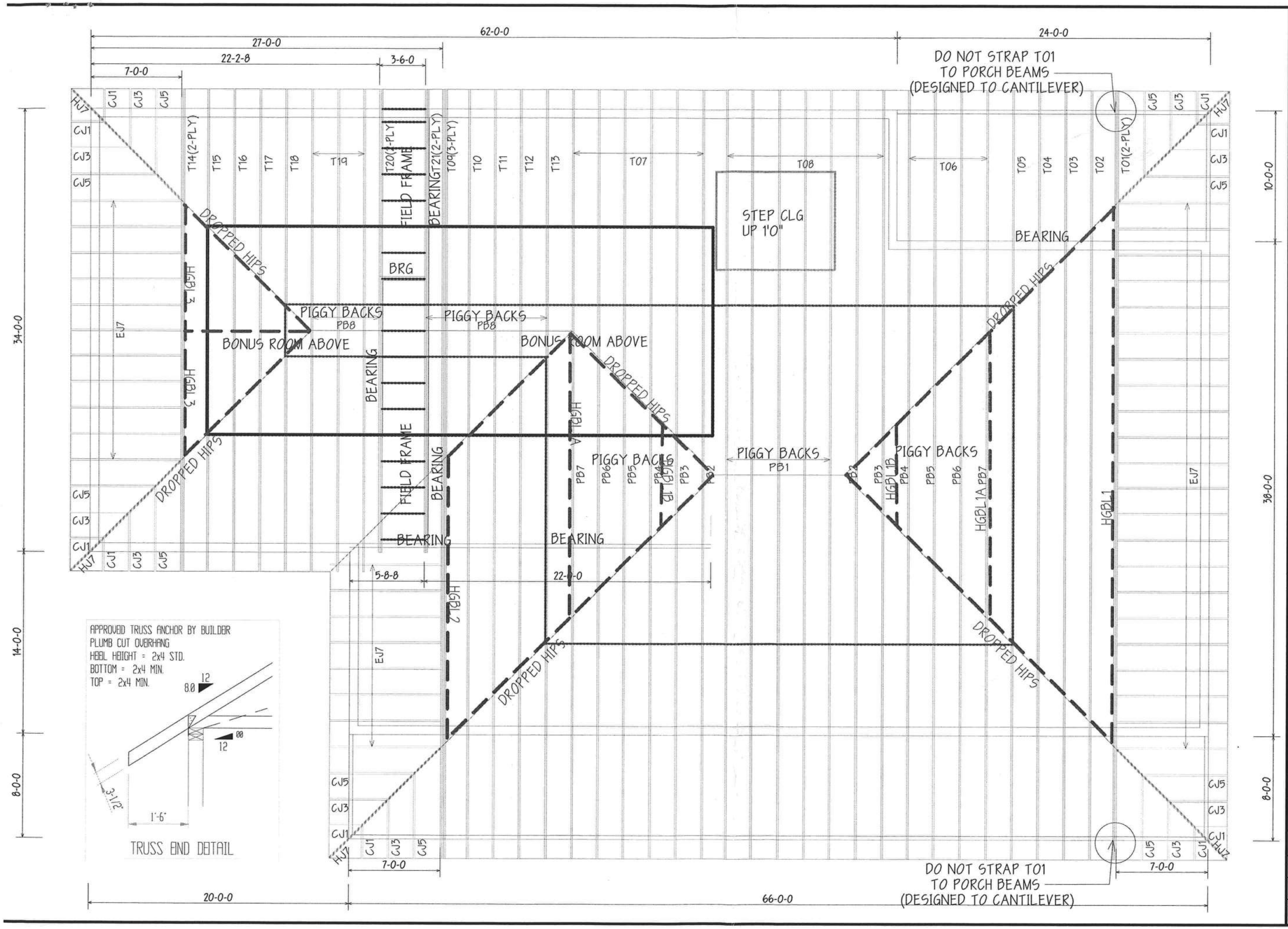
MAX SIZE 16' x 8'
DESIGN LOADS +29.5 PSF -29.5 PSF
TEST LOADS +49.3 PSF -49.3 PSF

REV	DESCRIPTION OF REVISIONS	DATE	BY

Amarr GARAGE DOORS
5923 STATEY CREEK BLVD. WINSTON-SALEM, N.C. 27105

MODEL #1500 WeatherGuard

SIZE	DRAWN BY DJ	DATE 04/16/01	DRAWING NUMBER
B	CHECKED BY	DATE	SBC-580-019-J
SCALE: NOT TO SCALE			SHEET 1 OF 1



APPROVED TRUSS ANCHOR BY BUILDER
PLUMB CUT OVERHANG
HEEL HEIGHT = 2x4 STD.
BOTTOM = 2x4 MIN.
TOP = 2x4 MIN.

TRUSS END DETAIL

BEARING HEIGHT SCHEDULE

9' PLATE

HANGER SCHEDULE

312 - PB PLATES

206 - SEATPLATES

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.
8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: DON REED CONST.		
LEGAL ADDRESS: WAGONER RES.		
MODEL: CUSTOM		KEV/SION: SCALE: NTS
DATE: 2-21-2007	DRAWN BY: G.DORRIS	JOB #: L227913

Notice of Treatment 12472

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: 536 SE Baya Ave
City Lake City FL Phone 752 1703

Site Location: Subdivision Dow Reed Coast
Lot # _____ Block# _____ Permit # 25685
Address 224 NW Everett Ter L.C.

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☐ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling		384	225

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

4-25-07 (Date) 8:30 (Time) T Day (Print Technician's Name)
4-25-07 1:15

Remarks: Cave Creek

Applicator - White Permit File - Canary Permit Holder - Pink
10/05 ©