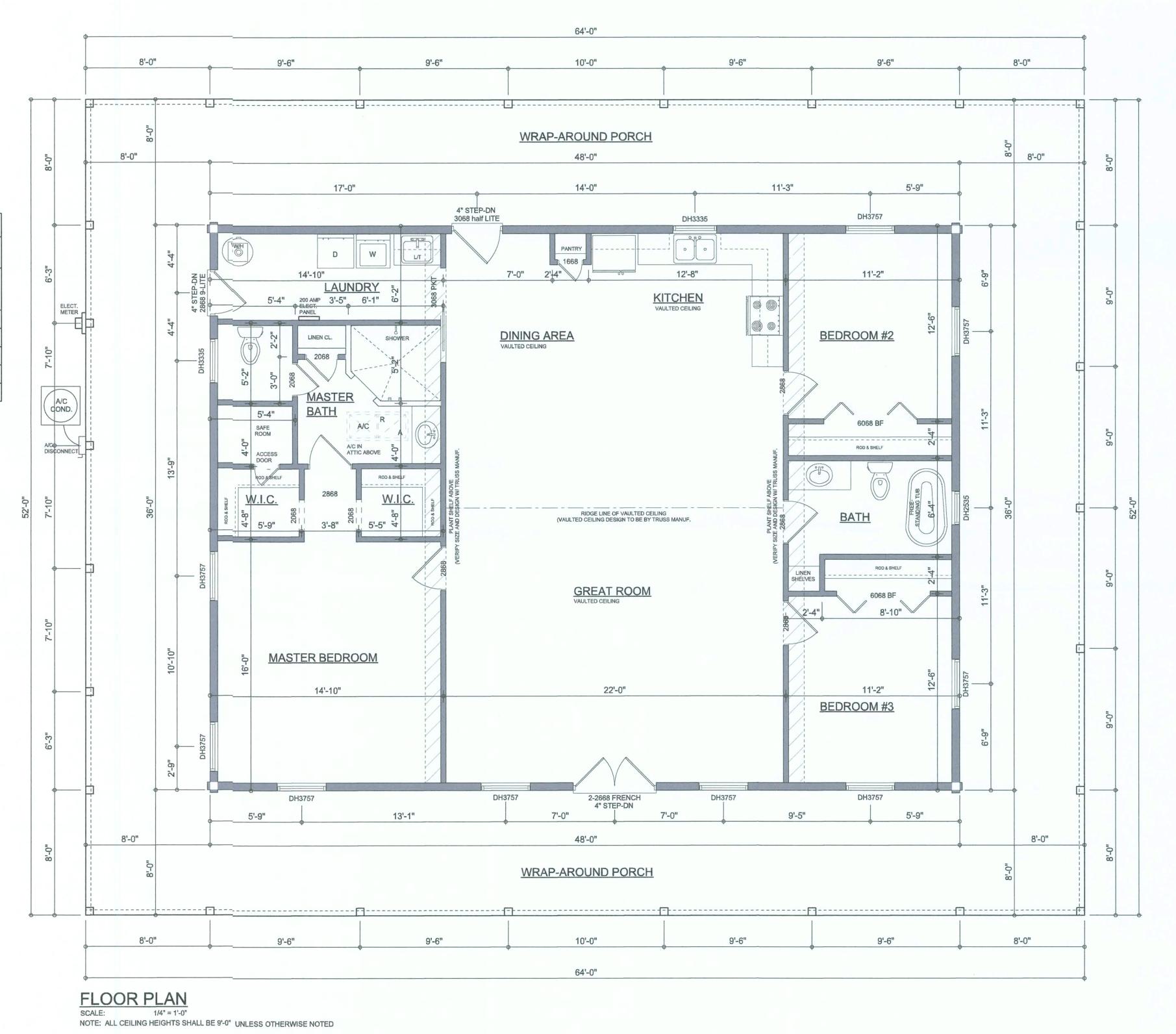


WIDTH HEIGHT HEIGHT	WIDTH HEIGHT
DH3757 3'-1" 4'-9" 3'-1 3/4" 4'-9 3/4"	3/4" 3'-4 3/4" 5'-0 3/4" WOOD INTERIOR PELLA (EGRESS WINDO
DH3335 2'-9" 2'-11" 2'-9 3/4" 2'-1! 3/4"	3/4" 3'-0 3/4" 3'-2 3/4" WOOD INTERIOR PELLA (TEMPERED)
DH2335 2'-I" 2'-II" 2'-I 3/4" 2'-II 3/4"	3/4" 2'-4 3/4" 3'-2 3/4" WOOD INTERIOR PELLA (TEMPERED)

D	OC	R	SC	HEDULE				
D	OOR					REMARKS	SWING	STYLE
	SIZE	=		LOG WALL	OPENING			
DOOR NUMBER	PAIR	WIDTH	НЕІСЯНТ	WIDTH	HEIGHT	-		
1	PAIR	5'-0"	6'-8"	5'-3"	6'-9 1/2"	FRENCH DOOR EXTERIOR FIBERGLAS; THERMOTRU OR EQUAL		
2		3'-0"	6'-8"	3'-3"	6'-9 1/2"	EXTERIOR FIBERGLASS THERMOTRU OR EQUAL		
3		2'-8"	6'-8"	2'-11"	6'-9 1/2"	EXTERIOR FIBERGLAS: THERMOTRU OR EQUAL		
4		2'-8"	6'-8"	N/A	N/A	INTERIOR DOOR		
5		2'-0"	6'-8"	N/A	N/A	INTERIOR DOOR		
6		6'-0"	6'-8"	N/A	N/A	FOLDING CLOSET DOCR		
7		4'-6"	6'-8"	N/A	N/A	FOLDING CLOSET DOCR		
8		3'-0"	6'-8"	N/A	N/A	FOLDING CLOSET DOCR		
9		1'-6"	6'-8"	N/A	N/A	FOLDING CLOSET DOCR		
10		2'-6"	6'-8"	N/A	N/A	INTERIOR POCKET DOOR		
11		2'-0"	6'-8"	N/A	N/A	INTERIOR POCKET DOOR		

LOG WALL OPENINGS ARE TO BE CUT 3" LARGER THAN DOOR ROUGH OPENING JAM)
LOG WALL OPENINGS ARE TO BE CUT 1 1/2" LARGER THEN DOOR ROUGH OPENING (HEAD)



AREA SUMMARY

FLOOR AREA 1,728 S.F. COVERED PORCH AREA 1,600 S.F. TOTAL AREA 3,328 S.F.

REVISIONS

LOG PACKAGE SUPPLIER:

A NEW LOG HOME FOR:

A CUSTOM LOG HOME FOR:

JIM BUDZINSKI

PROJECT ADDRESS: 850 SW Rock Way, Fort White, MAILING ADDRESS:

DESIGNED BY:

WM DESIGN
& ASSOCIATES, INC.
426 SW Commerce Dr. Ste 130
Lake City, Florida 32025
Phone: 386-758-8406
will@willmyers.net

PRINTED DATE:

JOB NUMBER: 20190222 DRAWING NUMBER

A.2

LOG STYLE: 6x 8 CYPRESS LOG

June 20, 2019

FINALS DATE:

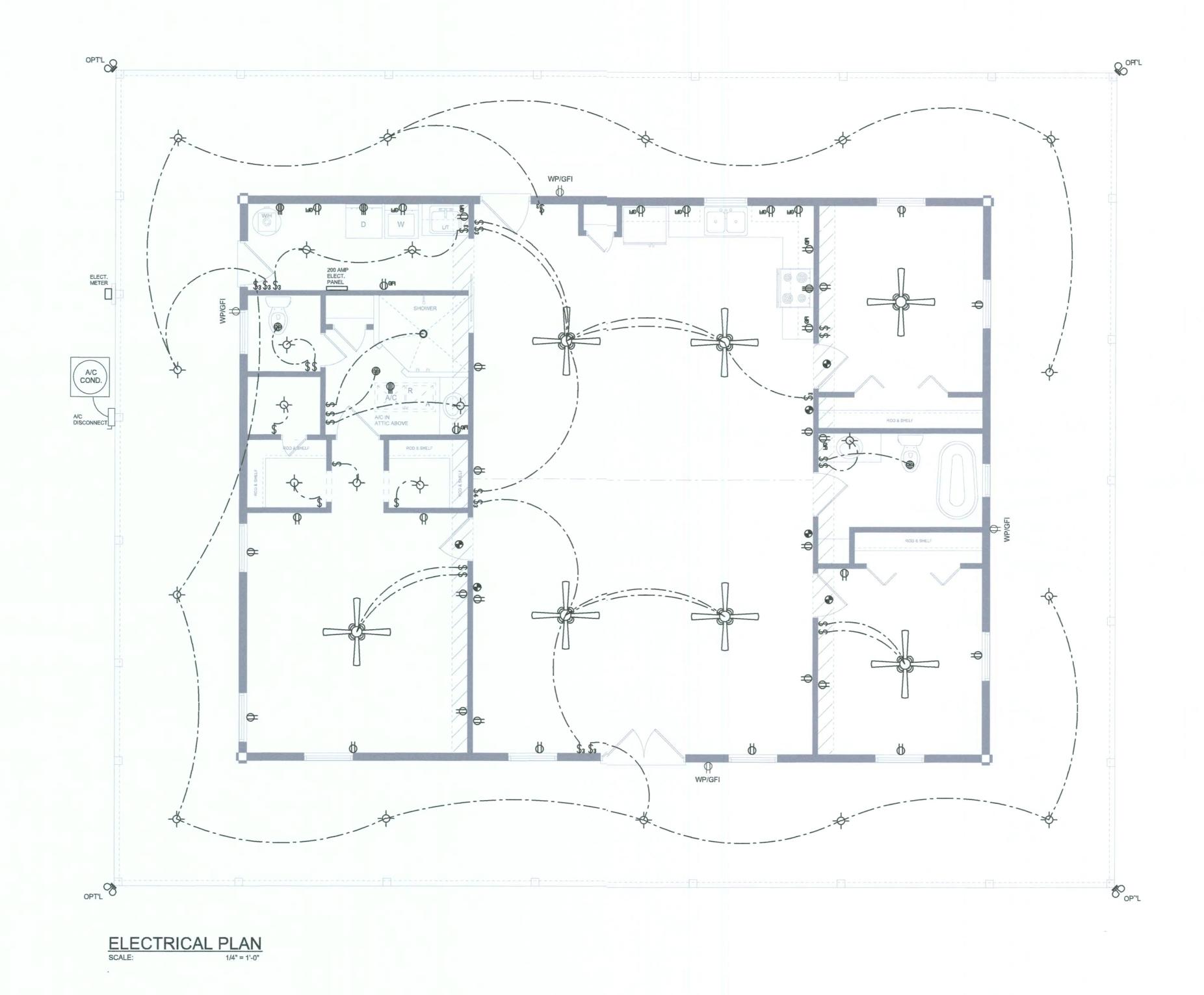
	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
QP	DOUBLE SECURITY LIGHT
0	RECESSED CAN LIGHT
₩	BATH EXHAUST FAN
-\$-	LIGHT FIXTURE
Ф	DUPLEX OUTLET (AFCI & TAMPER RESISTANT)
₩	220v OUTLET
∯GFI	GFI DUPLEX OUTLET (PER NEC 406.8)
†	TELEVISION JACK
PH	TELEPHONE JACK
•	SMOKE / CARBON MONOXIDE DETECTOR (see note below
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
₩P/GFI	WATER PROOF GFI OUTLET
48" FLOUR.	2 OR 4 TUB FLUORESCENT FIXTURE

ALL INTERIOR RECEPTACLES SHALL BE AFCI
(ARC FAULT CIRCUIT INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER

ALL SMOKE DETECTORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR AND SHALL HAVE BATTERY BACKUP POWER AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS. CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR SHALL BE USED AS AN EQUIPMENT GROUND.

IT IS THE LICENSED ELECTRICAL CONTRACTORS RESPONSIBILITY TO INSURE THAT ALL WORK PERFORMED AND EQUIPMENT INSTALLED MEETS OR EXCEEDS THE NFPA70 2014 NATIONAL ELECTRIC CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.



REVISIONS

LOG PACKAGE SUPPLIER:

A NEW LOG HOME FOR:

I BUDZINSKI A CUSTOM I

IM

PROJECT AD

MAILING AD

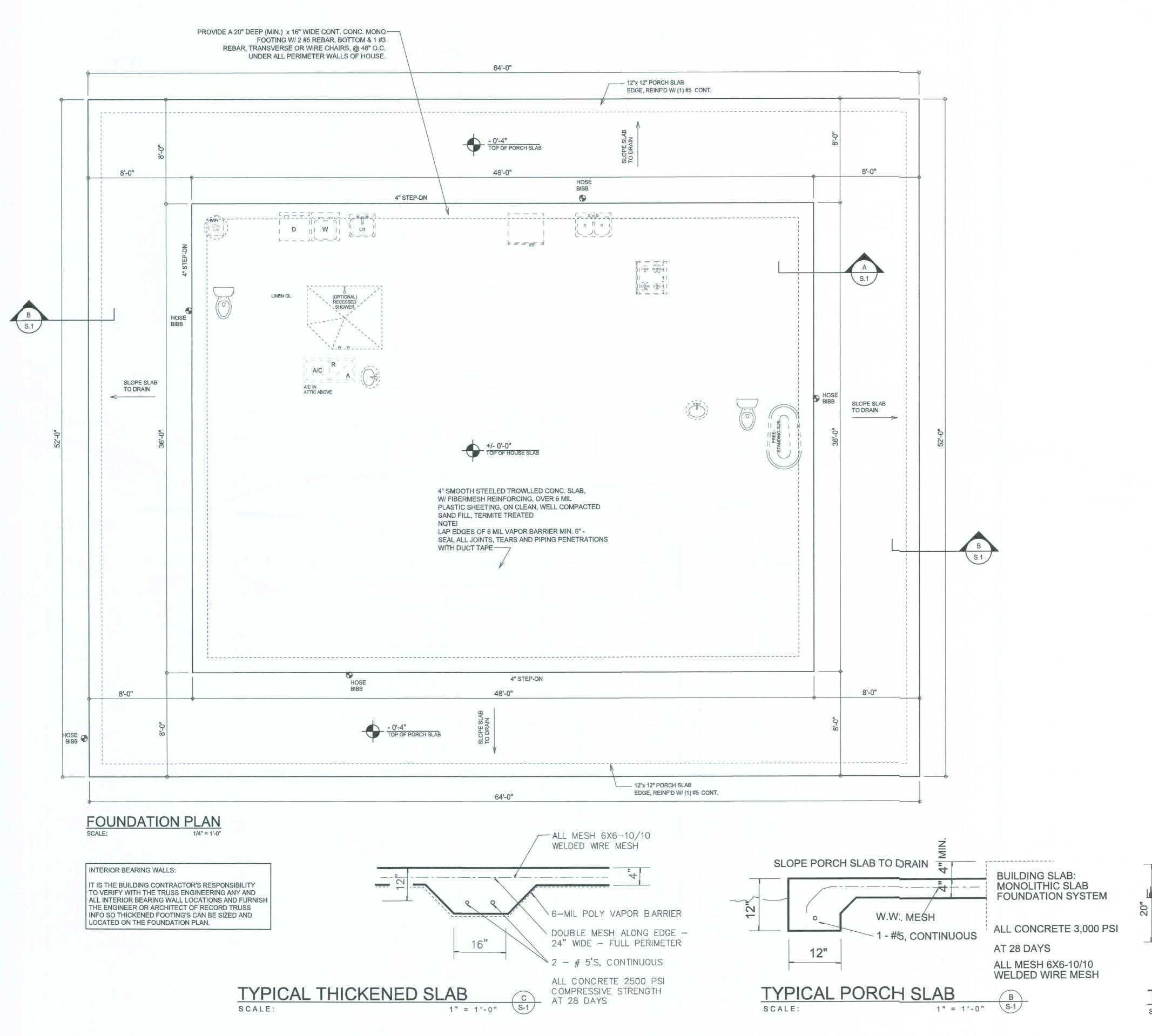
DESIGNED BY: WM DESIGN
& ASSOCIATES, INC.
426 SW Commerce Dr. Ste 130
Lake City, Florida 32025
Phone: 386-758-8406

will@willmyers.net

PRINTED DATE: June 20, 2019

LOG STYLE: 6x 8 CYPRESS LOG

JOB NUMBER: 20190222 DRAWING NUMBER **A.3**



CONCRETE / MASONRY / METALS GENERAL NOTES:

- DESIGN SOIL BEARING PRESSURE 1000 PSF.
- 2. EXPANSIVE SOILS: WHERE DIRECTED BY THE SOILS ENGINEER, SOIL AUGMENTATION PER THE SOILS ENGINEER'S SPECIFICATIONS SHALL BE IMPLEMENTED PRIOR TO PLACING ANY FOUNDATIONS - TESTS AS SPECIFIED SHALL BE PREFORMEDTO DETERMINE THE SUITABILITY OF THE SUB-GRADE TO SUPPORT THE DESIGN LOADS.
- CLEAN SAND FILL OVER STRIPPEDAND COMPACTION SHALL BE NOT LESS THAN 95% ASMEASURED BY A MODIFIED PROCTOR TEST AT THE RATE OF ONE TEST FOR EACH 1500 SF OF BUILDING PAD AREA, OR FRACTION THEREOF.
- 4. REINFORCING STEEL SHALL BE GRADE 60 AND MEET THE REQUIRE-MENTS OF ASTM A615, ALL BENDSSHALL BE MADE COLD.
- WELDED WIRE MESH SLAB REINFORCING SHALL MEET THE REQUIRE-MENTS OF ASTM A185 - MIN. YEILDSTRESS = 85 KSI.
- CONCRETE SHALL BE STANDARD MIX F'c = 3000 PSI FOR ALL FTGS, SLABS, COLUMNS AND BEAMS OR SHALL BE STANDARD PUMP MIX F'c = 3000 PSI. STRENGTH SHALL BE ATTAINED WITHIN 28 DAYS OF PLACE-MENT. MIXING, PLACING AND FINISHING SHALL BE AS PER ACI STANDARDS.
- CONCRETE BLOCK SHALL BE AS PER MANUFACTURER'S PRODUCT GUIDE FOR ASTM C-90 REQUIREMENTS WITH MEDIUM SURFACE FINISH -F'm = 1500 PSI.
- 8. MORTAR SHALL BE TYPE "M" OR "N" FOR ALL MASONRY UNITS.
- STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 STANDARDS FOR STRENGTH, BOLTS SHALL BE ASTM A307 / GRADE 1 OR A325, AS PER PLAN REQUIREMENTS.
- 10. WELDS SHALL BE AS PER "AMERICAN WELDING SOCIETY" STANDARDS FOR STRUCTURAL STEEL APPLICATIONS.
- 11. 2X4 P/T WOOD SILL, CONT., ALL AROUND, W/ 5/8"~ A.B. W/ 3" SQ. X 1/4" PLATE WASHERS VITHIN 6" FROM EACH CORNER, EA. WAY, & WITHIN 6" FROM ALL WALL OPENINGS / ENDS - 1/2"~ A.B. W/ 2" SQ. WASHERS ALONG EACH RUN @ 48" O.C., MAX. - ALL ANCHOR BOLTS SHALL HAVE A MINIMUM OF 8" EMBEDMENT INTO THE CONCRETE.

PRIOR TO THE CONSTRUCTION OF THE FOUNDATION, THE CONTRACTOR SHALL COORDINATE ANY INTERIOR BEARING LOCATION CONDITIONS PER THE TRUSS ENGINEERED SHOP DRAWINGS WITH THE FOUNDATION PLAN. ANY INTERIOR BEARING LOCATIONS OR ANY POINT LOADS OF 4.0 K OR GREATER SHALL BE SUPPORTED VIA A MODIFIED FOUNDATION PLAN TAKING THESE LOADS INTO CONSIDERATION. THE CONTRACTOR SHALL MAKE THE ENGINEERED TRUSS SHOP DRAWINGS AVAILABLE TO THE ARCHITECT FOR THE PURPOSE OF RENDERING SUCH MODIFICATIONS PRIOR TO POURING ANY CONCRETE.

THE DESIGN WIND SPEED FOR THS PROJECT IS 140 MPH PER 2017 PER R301.2.1.1 AND LOCAL JURISDICTION REQUIFEMENTS

ADDED FILL SHALL BE CONPACTED TO 98% DRY COMPACTION PER THE "MODIFIEDPROCTOR" METHOD.

H.V.A.C. CONTRACTOR SHALL PREPARE "AS-BUILT" SHOP DRAWINGS INDICATING ALL H.V.A.C. WORK, INCLUDING ALL DUCTWORK LOC., SIZES, LINES, EQUIPMENT SCH. & BALANCING REPORT - CONT'R SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

REINFORCED CONCRETE



INSKI DD 9 A CUSTOM B

PROJECT AD

MALLING AD





JOB NUMBER 20190222

OF 4 SHEETS

SHEET NUMBER

VAPOR (2)-#5'S BARRIER 3,000-PSI CONCRETE AT 28 DAYS

TYPICAL WALL FOOTING I" = 1'-0" S.1

ACTUAL SHEAR WALL CAPACITY (FOR LOG WALLS) IS THE TOTAL CAPACITY OF LOG SCREWS (TOTAL CAPACITY = # OF LOG SCREWS X LOG SCREW SHEAR CAPACITY = # 09 LOG SCREWS LOG SCREW SHEAR CAPACITY = # 09 TO CALCULATE THE TOTAL # 0F LOG SCREWS DIVIDE THE WALL LENGTH BY LOG SCREW SPACING AND ADD LOG SCREWS IN FOR EACH CORNER (ALL LOG WALLS ARE SHEAR WALLS)

ACTUAL SHEAR WALL CAPACITY (FOR FRAMED WALLS) IS THE TOTAL LENGTH OF ALL WALL SEGMENTS WITH FULL HEIGHT SHEATHING AND A WIDTH TO HEIGHT RATIO GREATER THAN 1: 3.5 TIMES ALLOWABLE SHEAR CAPACITY WITH TABLE 3.17E ADJUSTMENT FROM WFCM-2001 (PLUS SPECIAL SHEAR WALL SEGMENTS IF NOTED.) (ALLOWABLE SHEAR CAPACITY= 436 PLF FOR FULLY BLOCKED 7:16° OSB W/ 8d AT 6° OC EDGE, 12° OC FIELD AND UNBLOCKED 1/2° GYP BOARD W/ 5d COOLER NAILS '7 OC EDGE, 10° OC FIELD)
(ALL EXTERIOR FRAMED WALLS ARE TYPE II SHEAR WALLS) REQUIRED SHEAR WALL CAPACITY IS FROM WFCM 2001, TABLE 2.5a & 2.5b

LOG FASTENERS @ 16" OC

(CR EQUIVALENT CALCULATION)

SHEARWALL CALCULATION W01) LOG & FRAMED WALLS

STRUCTURAL PLAN NOTES:

ALL LOAD BEARING LOG HEADERS SN-1 SHALL BE A MINIMUM OF (2) 6x8 OR (1) 6x12 LOG COURSES (U.O.N.)

ALL LOAD BEARING HEADERS IN SN-2 FRAMED WALLS SHALL BE A MINIMUM OF (2) 2X10 SYP#2 (U.O.N.)

7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 4" OC EDGE, 12" OC FIELD (U.N.O.)

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

GENERAL NOTES:

1. BUILDER / OWNER IS TO VERIFY ALL DIMENSIONS BEFOR BEGINNING CONSTRUCTION.

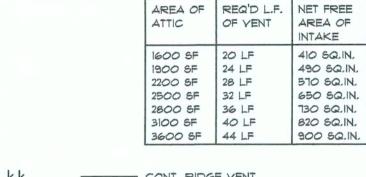
2. ALL FRAMING MEMBERS ARE TO BE SYP#2 U.N.O. 3. ALTHOUGH TRUSSES ARE SHOWN AT 24" OC IT IS THE RESPONSIBLITY OF THE TRUSS MANUFACTURER TO DETERMIN TRUSS SPACING AND PLACEMENT.

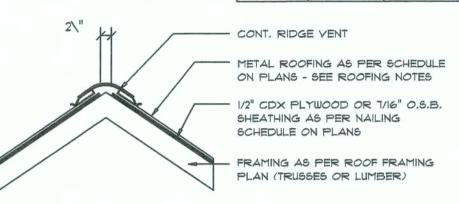
4. R22 = SEE DETAIL # SHOWN IN BOX

7/16" OSBOR 5/8" CDX ROOF SHEATHING ATTACHE) PER EAVE CONNECTION DETAIL PRE-ENGINEERED ROOF TRUSSES AT 24" O.C. MAX. (DESIGNED BY OTHERS) -2x6 SYP#2 LEDGER ROOF SYSTEM (SEE ROOF FRAMING DETAILS) w/ (6) 131" x 3.25" NAILS @ 24" OC TO SCAB / TRUSS FRAMING - AT'ACH RAFTERS TO LEDGER w/ (1) LOG SELECT CONNECTOR PER UPLIFT LOADS OF RAFTER / TRUSS TO WALL FASTERN AND (8) .131" x 3.25" NAILS FROM BACK SIDE OF WALL THROUGH CONNECTION TABLE LEDGER INTO RAFTER ATTACH RAFFERS TO PORCH HEADER W/----(2) LOG BOS! THRU RAFTER TO HEADER OR TOE-SCREWED FROM HEADER INTO RAFTER - OPTIONAL 2x6 SYP #2 RAFTER PLATE W/ LOG FASTENERS INTO LOG WALL (SAME SPACING AS IN LOG WALL) -RODFING AS PER OWNER 15/16" T&G DECKING LOG WALL SYSTEM (SEE LOG DETAIL FOR PROFILE) VALLEY FLASHING PER DETAIL, 'A' w/ 2) 16d EACH RAFTER LOG FASTENERS @ 48° OC MAX STAGGERED EACH COURSE 2x4/6 SCAB w/ IN LOG WALL & INTO RIM JOIST (2) .131" x 3.25" NAILS @ 6" OC AND AT EACH LOG END, SPLICE, AND LOG CUT (SEE LOG CONNECTION DETAILS FOR NUMBER AND SPACING OF - KICK-OUT FLASHING PER FASTENERS) TRUSS HEEL W/ 7/16" CSB OR 5/8" CDX (SEE BUCK DETAIL FOR FASTENER SHEATHING FULLEY BLOCKED PLACMENT BY OPENINGS) w/ 8d @ 6" OC EDGE AND 4" OC FIELD FLOOR SYSTEM (SEE FLOOR FRAMING PLAN) TRUSS TO TOP PLATE (4) .131X3.25" TOE NAIIS OPTIONAL LOG SIDING WALL OR -(FOR SHEAR LOADS) BEAM SELECT TRUSS CONNECTORS POST TO (SEE FLOOR FRAMING PLAN) FROM THE CONNECTOR TABLE HEADER - 2x_ PT SILL PLATE PER UPLIFT LOADS FROM w/ (6) LOG ENGINEERED TRUSSES DRAWINGS TOE-SCREWS - 8" MIN. FOUNDATION WALL

HEAVY TIMBER RAFTERS 8' MAX SPAN

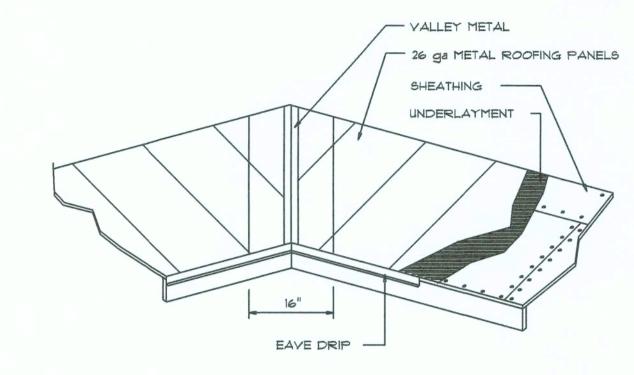
TYP. PORCH @ TRUSS HEEL DETAIL TYP. 1-STORY LOG WALL SECTION ON RIM @ WOOD FLOOR SYSTEM





Ridge Vent DETAIL

SCALE: 3/4" = 1'-0"



VALLEY FLASHING

	TALS for FLASI	HING/ROOF	NG:
MATERIAL	MINIMUM THICKNESS (In)	GAGE	WEIGHT
COPPER			16
ALUMINUM	0.024		
STAINLESS STEEL		28	
GALVANIZED STEEL	er10.0	26 (ZINC COATED G90)	
ZINC ALLOY LEAD PAINTED TERNE	0.027		40 20

Roofing/Flashing DETS.

SCALE: NONE



THE CONTRACTOR SHALL COORDINATE THE TRUSS TO TRUSS ANCHOR REQUIREMENTS WITH THE TRUSS ENGINEERING SHOP DRAWINGS. SOME OF THE TRUSS TO TRUSS CONNECTIONS WILL REQUIRE ANCHOR STRAPS IN ADDITION TO TYPICAL NAILING, ANCHOR DEVICES SHALL BE REQUIRED FOR ALL JOINTS WITH AN UPLIFT OR GRAVITY LOAD OF 100 LBS OR GREATER.

TRUSSES BEARING ON INTERIOR PARTITIONS WHERE UPLIFT LOADS ARE PRESENT SHALL REQUIRE ANCHORS OF EQUAL OR GREATER LOAD CAPACITY THAN THAT INDICATED BY THE TRUSS SHOP DRAWINGS. THE UPLIFT ANCHOR SYSTEM SHALL BE CONTINUOUS TO THE FOUNDATION.

-PRE-ENGINEERED -PRE-ENGINEERED — STANDARD TRUSSES (BY OTHERS) STANDARD TRUSSES (BY OTHERS) ATTACHED AS PER ATTACHED AS PER TRUSS ENGINEERING TRUSS ENGINEERING -PRE-ENGINEERED -'SCISSOR STYLE' TRUSSIES (BY OTHERS) ATTACHED AS PER A/C IN ATTIC ABOVE TRUSS ENGINE ERING TYPICAL PORCH ROOF FRAMING:

PROJECT COORDINATION REQUIREMENTS

THESE PLANS ARE DRAWN FOR AVERAGE SITE CONDITIONS AND COMPLIANCE WITH APPLICABLE CODES AT THE TIME THEY ARE DRAWN, DUE TO YARYING STATE, LOCAL, AND NATIONAL CODES RULES AND REGULATIONS, N.P.GEISLER, ARCHITCT CANNOT WARRANT COMPLIANCE WITH ALL APPLICABLE STATE, LOCAL, AND NATIONAL CODES IN YOUR AREA OR WITH YOUR PARTICULAR SITE CONDITIONS. IT IS THE RESPONSIBILITY OF THE PURCHASER AND/OR BUILDER TO SEE THAT THE STRUCTURE IS BUILT IN STRICT COMPLIANCE WITH ALL GOVERNING MUNICIPAL CODES (CITY, COUNTY, STATE, AND FEDERAL). IF YOUR CITY OR STATE REQUIRES AN ENGINEER'S SEAL FOR THE SITE/CIVIL PORTIONS OF THE WORK,, YOU WILL NEED TO HAVE THAT DONE LOCALLY BY A QUALIFIED, LICENCED PROFESSIONAL ENGINEER.

SHOP DWG COORDINATION: THE TRUSS ANCHOR STRAPS AS INDICATED IN THE CONSTRUCTION DOCUMENTS ARE SUGGESTED STRAPS AND THAT THE TRUSS ENGINEERED SHOP DRAWING LOADS TAKE PRECEDENCE OVER THAT

INDICATED IN THE CONSTRUCTION DOCUMENTS. THE UPLIFT LOADS INDICATED FOR EACH TRUSS IN THE ENGINEERED TRUSS SHOP DRAWINGS MAY BE MATCHED TO STANDARD PRODUCT UPLIFT RATINGS FOR COMPARABLE UPLIFT CONNECTORS, AND THAT THE PRODUCTS THAT PROVIDE EQUAL OR GREATER UPLIFT RESISTANCE FOR THE LISTED LOADS MAY BE USED IN LIEU OF THOSE INDICATED IN THE CONSTRUCTION DOCUMENTS OR AS APPROVED BY THE BUILDING OFFICIAL.

STRUCTURAL PLAN **ROOF FRAMING PLAN**

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

- 4x 6 CYP#2 RAFTERS @ 48" O.C.

- 15/16" T&g CYPRESS DECKING (SEE PORCH DETAIL FOR ATTACHMENT)

- 4x 8 CYP#2 HIP RAFTERS

FRAMING

ROOM

ZINSKI

7

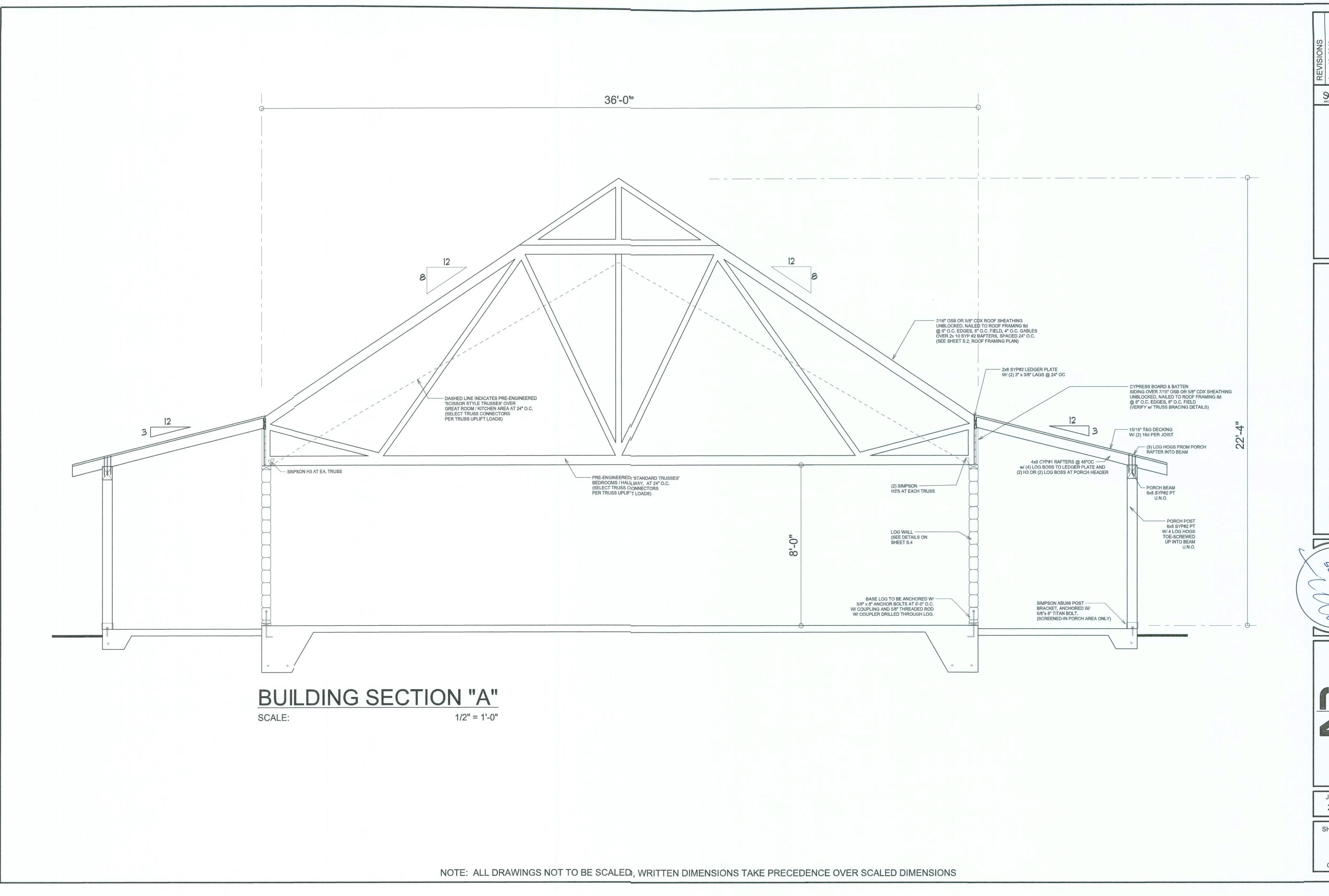
9

PROJECT ADI

JOB NUMBER 20190222

SHEET NUMBER

OF 4 SHEETS



BUILDING (

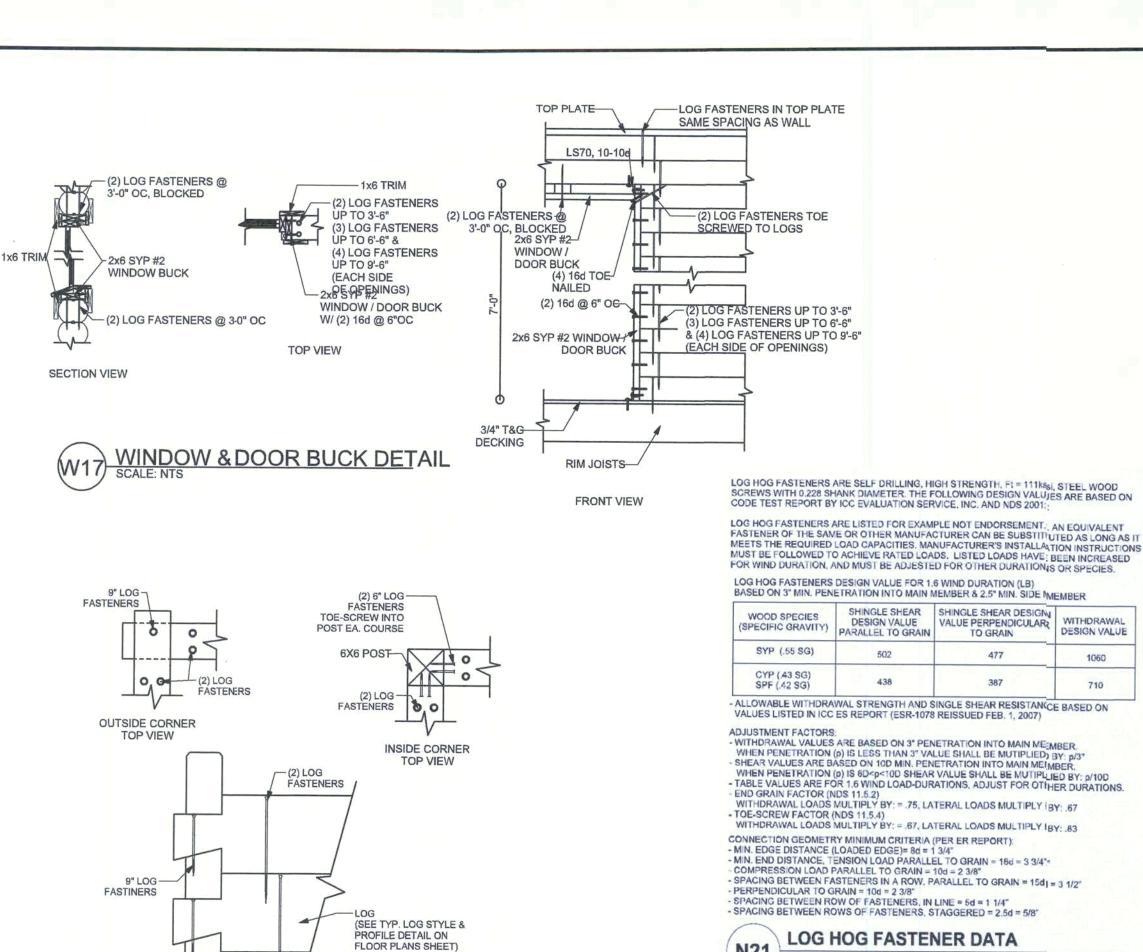
A CUSTOM BUILDING FOR:

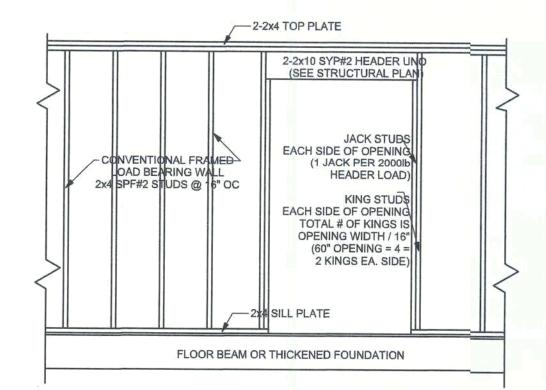
JIM BUDZINSKI

PROJECT ADDRESS: 850 SW Rock Wav Energy
MAILING ADDRESS:

JOB NUMBER 2019<mark>0222</mark>

SHEET NUMBER OF 4 SHEETS

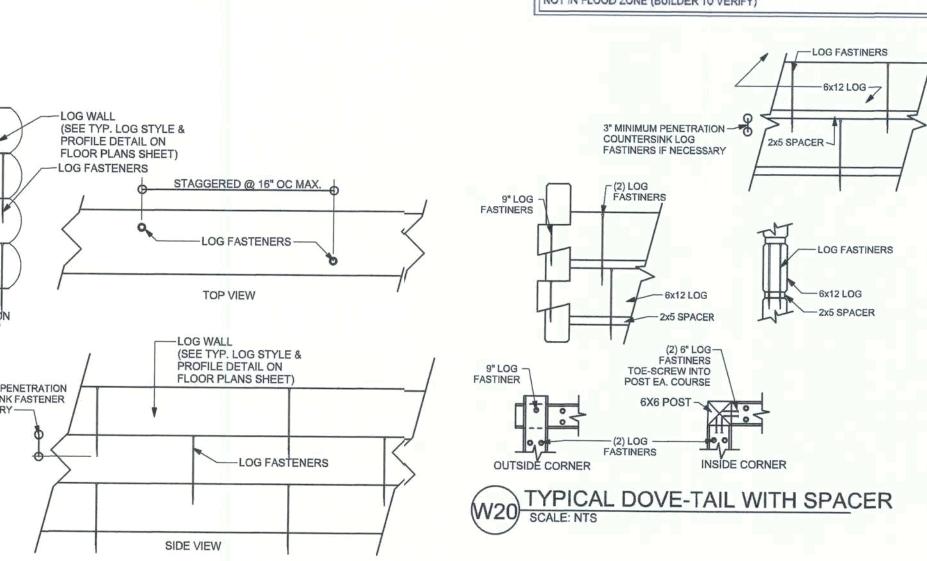




TYPICAL DESIGN VALUES



W24 TYPICAL LOG WALL / FASTENER PLACEMENT



N21 STRUCTURAL DESIGN NOTES

RAFTERS SHALL BE IN ACCORDANCE WITH RAFTER SPAN TABLES.

REQUIRED WHEN RIDGE BEAM IS PROVIDED.

RAFTER OVERHANGS SHALL NOT EXCEED THE LESSER OF 1/3 RAFTER SPAN OR 24" UNO. RAKE OVERHANG OUTLOOKERS ARE 2X4 PURLINS CONNECTED PER GABLE DETAIL UNO. RAKE OVERHANGS SHALL NOT EXCEED THE LESSER OF 1/2 PURLIN LENGTH OR 24".

RAKE OVERHANGS USING 2X4/6 OUTLOOK BLOCKING @ 24" OC SHALL NOT EXCEED 12"

IS 1 1/2" ON WOOD OR METAL AND 3" ON MASONRY OR CONCRETE.
RAFTERS AND CEILING JOISTS HAVING A DEPTH-TO-THICKNESS RATIO EXCEEDING 5:

RAFTER BEARING ON BEAM OR HANGERS. CEILING JOISTS OR RAFTER TIES ARE NOT

RIDGE BOARDS ARE BE PERMITTED PROVIDED: MIN. THICKNESS IS 1" NOMINAL, DEPTH IS

NOT LESS THAN CUT END OF THE RAFTERS, RAFTERS ARE PLACED DIRECTLY OPPOSITE

EACH OTHER, AND WALLS ARE TIED BY, RAFTER TIES, CEILING OR FLOOR SYSTEM.

WHERE THE ROOF PITCH IS LESS THAN 3:12, STRUCTURAL MEMBERS THAT SUPPORT

A CONTINUOUS LOAD PATH SHALL BE PROVIDED TO TRANSMIT THE UPLIFT FORCES

SHALL EXTEND SO THE TOP NAIL IS WITHIN 1" OF THE TOP OF THE RAFTER, OR

CEILING JOISTS SHALL BE IN ACCORDANCE WITH CEILING JOIST SPAN TABLE. WHEN CEILING JOISTS ARE USED TO PROVIDE RESISTANCE TO RAFTER THRUST.

ATTACHED AT EACH END, TO BEARING WALLS, HEADERS, OR RIDGE BEAMS.

1 1/2" THICK T&G FOR RAFTERS @ 48" OC. ATTACH WITH (2) 16d EACH JOIST.

HANGERS ARE REQUIRED FOR ALL HEADER JOISTS OVER 6 FT. WIDE.

WOOD RAFTER FRAMING

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

.) WIND EXPOSURE = C, BUILDER MUST FIELD VERIFY

.) BASIC WIND SPEED = 140 MPH, (3 SEC GUST, 33 FT, EXP. C)

tOPOGRAPHIC FACTOR = 1.0, BUILDER MUST FIELD VERIFY

INTERNAL PRESSURE COEFFICIENT = +/- ,18 (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone Effective Wind Area (ft2)

3 46 -117

5 50 -67

Garage Door

2014 FBCR.

Table R301.2.(4)

9x7 Garage Door 44 -50

16x7 Garage Door 42 -47

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

) BUILDING CATEGORY = II, (MRI = 700 YR)

.) ROOF ANGLE = 7-45 DEGREES

.) MEAN ROOF HEIGHT = <30 FT

DESIGN DATA

MEAN ROOF HEIGHT

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

30 PSF (SLEEPING ROOMS)

16 PSF (4:12 TO <12:12)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 2000PSF

JOISTS. FOR LESS THAN 4 FT. WIDE USE A SINGLE HEADER AND TRIMMER JOIS' FOR OPENINGS MORE THAN 4 FT. WIDE DOUBLE THE TRIMMER AND HEADER JOISTS.

RAFTERS & CEILING JOISTS (RIDGES, HIPS, & VALLEYS) SHALL BE DESIGNED AS BEAMS.

COLLAR TIES OR RIDGE TENSION STRAPS TO RESIST WIND UPLIFT SHALL BE PROVIDED.

FROM RAFTER/TRUSS TIES TO THE FOUNDATION. FOR RAFTER CONSTRUCTION, STRAPS

LAPPED JOISTS MUST BE NAILED TOGETHER OR STRAPPED TOGETHER TO RESIST LOAD

CATHEDRAL CEILINGS WITHOUT CEILING JOISTS OR RAFTER TIES SHALL HAVE RAFTERS

OPENINGS IN ROOF AND CEILING FRAMING SHALL BE FRAMED WITH HEADER & TRIMMER

PROVIDE HANGER OR LEDGER AT HEADER FOR RAFTERS AND JOISTS OVER 12' SPAN

MINIMUM THICKNESS OF STRUCTURAL PANEL ROOF SHEATHING: 16" OC RAFTERS USE 7/16" FOR UPTO 130 MPH; 24" OC RAFTER USE 7/16" UPTO 110 MPH, 5/8" UPTO 140 MPH

0.113 INCH NOMINAL SHANK DIAMETER / RING DIAMETER OF 0.012" OVER SHANK DIA.

16 TO 20 RINGS PER INCH / 0.280" FULL ROUND HEAD DIAMETER / 2" NAIL LENGTH

WIND LOADS PER 5th EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017) R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS:

PROVIDE DIAPHRAGM BLOCKING AT PANEL EDGES IN THE FIRST 2 BAYS, 48" OC MAX.

PROVIDE EDGE SUPPORT BLOCKING OR EDGE CLIPS FOR 7/16" SHEATHING w/ 24" SPANS

ATTACH STRUCTURAL SHEATHING W/8d RING-SHANK @ 6"OC EDGE & FIELD, 4"OC GABLE

UMBER ROOF SHEATHING SHALL BE A MIN. OF 5/8" THICK FOR 24" OC RAFTER SPACING

WRAPPED AROUND TOP OF THE RAFTER WITH ONE NAIL MIN. ON THE OPPOSITE SIDE.

SHALL HAVE LATERAL SUPPORT AT POINTS OF BEARING TO PREVENT ROTATION.

RIDGE, HIP, AND VALLEY BEAMS SHALL BE INSTALLED PER FRAMING PLANS, WITH

RAFTERS AND CEILING JOISTS SHALL BEARING DIRECTLY ON BEAMS, GIRDERS, LEDGERS

OR BEARING WALLS OR BE SUPPORTED BY JOIST HANGERS. MINIMUM BEARING LENGTH

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS. ALL CONNECTIONS EXPOSED DIRECTLY TO THE WEATHER SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

LOG WALLS: ALL LOG WALLS ARE MILLED LOGS WITH FLAT STACKING SURFACES. EACH COURSE IS ATTACHED TO THE COURSE BELOW WITH LOG FASTENERS. FASTENER SPACING IS BASED ON REQUIRED PULLOUT STRENGTH FOR WIND UPLIFT AND REQUIRED SHEAR STRENGHT FOR LATERAL WIND LOADS.

INTERIOR STUD WALLS: ALL INTERIOR STUD WALLS ARE NON-LOAD BEARING; UNO. ROOF LOADS TO BE CARRIED ON LOG WALLS OR ROOF BEAMS WITH INTERIOR SUPPORT COLUMNS; UNO. BEARING WALL STUDS TO BE SPF#2; UNO. NON-LOAD BEARING WALL STUDS MAY BE SPF STUD GRADE. ALL PLATES NOT PROTECTED FROM MOSTURE TO BE SYP#2 PT.

EXTERIOR STUD WALLS: ALL EXTERIOR STUD WALLS ARE LOAD BEARING SHEAR WALLS WITH SPF#2 STUDS, SYP#2 PT BOTTOM PLATE, SPF#2 DOUBLE TOP PLATE WITH 10-16d NAILS PER LAP SPLICE: SP4, 6-10d "U" STRAP TOP AND BOTTOM AT 48" OC UNO; 7/16" OSB OR 5/8" CDX SHEATHING, WITH PANEL EDGES FULLY BLOCKED, FASTENED WITH 8d COMMOM NAILS, SPACING 6" OC PANEL EDGES, 12" OC INTERMEDIATE FRAMING MEMBERS; UNO.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING ROOF SEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB OR 5/8" CDX SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH

PANEL EDGES, STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES

6"OC INTERMEDIATE MEMBERS, 4" OC GABLE ENDS AND DIAPHRAGM BOUNDARY; UNO.

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED AND SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE. REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO

ROOF VENTILATION: ROOF VENTILATION IS TO MEET OR EXCEED FLORIDA BUILDING CODE RES.

WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE.

FLASHING: BUILDER IS TO PROVIDE FLASHING TO MEET LOCAL CODE REQUIRMENTS AND INSTALLED IN A WORKMANLIKE MANNER TO PREVENT ANY POSSIBILITY OF MOISTURE DAMAGE, TOXIC MOLD, OR ANY OTHER DETRIMENTAL EFFECT. ALSO, FOLLOW FLASHING MANUFACTURER'S DATA SHEET AND SMACNA LITERATURE AND STANDARDS.

N22 SITE / FOUNDATION NOTES

SITE PREPERATION: SITE ANALYSIS AND PREPERATION INFORMATION IS NOT PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE OWNER. ALL FOUNDATIONS AND FOOTINGS ARE DESIGNED FOR STABLE SOIL CONDITIONS WITH 2000 PSF BEARING CAPACITY. SITE INSPECTION OF SOIL CONDITIONS SHALL DETERMINE IF THERE IS ANY EVIDENCE OF UNSUITABLE BEARING MATERIALS. QUESTIONABLE MATERIALS PRESENT SHOULD CALL FOR SOILS TEST AND ANALYSIS BY GEOTECHNICAL ENGINEER TO ASSURE THAT EXPANDING CLAYS AND OTHER PROBLEMATIC SOILS CONDITIONS DO NOT EXIST, OR TO ALLOW MITIGATION SHOULD THEY EXIST. ALL FILL UNDER STRUCTURAL ELEMENTS SHALL BE CLEAN SAND/SOIL FILL, FREE FROM DEBRIS AND ORGANIC MATERIALS COMPACTED IN LIFTS OF NOT MORE THAN 6 IN., LOOSE MEASURE. IT IS THE OWNER'S / BUILDER'S RESPONSIBILITY TO VERIFY EXISTING SOIL AND CLEAN FILL ARE COMPACTED TO 95% OF MAX. DRY DENSITY PER THE MODIFIED PROCTOR TEST AND PROVIDE 2000 PSF MIN. BEARING CAPACITY OR REQUEST FOUNDATION DESIGN BASED ON ACTUAL SITE CONDITIONS.

FOUNDATION: THE OWNER HAS NOT YET PROVIDED A GEOTECHNICAL REPORT TO THE ENGINEER. ASSUMED SAFE BEARING CAPACITY OF 2000 PSF SHALL BE CONFIRMED IN THE FIELD BY REGISTERED GEOTECHNICAL ENGINEER OR SHALL BE APPROVED BY THE OWNER. FOOTING AND SLABS ARE TO BEAR ON FIRM UNDISTURBED EARTH OR CLEAN SAND / SOIL FILL, FREE FROM DEBRIS, AND ORGANIC MATERIALS COMPACTED IN LIFTS OF NOT MORE THAN 6 IN., LOOSE MEASURE. WHERE UNACCEPTABLE MATERIAL OCCURS, EXCAVATE AND REPLACE WITH ENGINEERED FILL. NO FOUNDATION CONCRETE SHALL BE INSTALLED UNTIL ALL FOUNDATION WORK HAS BEEN COORDINATED WITH UNDERGROUND UTILITIES. FOOTING SHALL BE LOWERED WHERE REQUIRED TO AVOID UTILITIES. TO MINIMZE WEATHERING, THE LAST 6" OF EXCAVATION FOR ALL FOOTINGS SHALL BE MADE IMMEDIATELY PRIOR TO

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATER. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF ENGINEER, OWNER OR HIS

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 23".

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

CONCRETE BLOCK: ASTM C-90 WITH MEDIUM SURFACE FINISH, F'm = 1500 PSI. MORTAR: TYPE M OR N FOR ALL MASONRY UNITS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU. WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

STRAPS & TIES	UPLIFT	UPLIFT	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER	TO PLATE
H5	455	265	115	-	100			4-8d x 1 1/
НЗ	415	290	125		105	140		4-8d x 1 1/
H2.5A	480	480	110	110	110	110	10/10/20/20 10/200	5-8d x 1 1/
H6	950	820					8-8d	8-8d
H8	745	565		1			5-10d x 1 1/2"	5-10d x 1 1
H10	990	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/
H10-2	760	655	455	395	390	340	6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1
LTS12 - LTS20	1000	620					6-10d x 1 1/2"	6-10d x 1 1
MTS12 - MTS30	1000	860					7-10d x 1 1/2"	7-10d x 1 1
HTS16-HTS30	1450	1245					12-10d x 1 1/2"	12-10d x 1
STUD STRAP CONNECTOR	UPLIFT	UPLIFT					TO STUDS	TO PLATE
SSP (2-PLATES)	435	435	-	+-		_	4 -10d	3-10d
SSP (1-PLATE)	455	420		\vdash		_	4 -10d	1-10d
DSP (2-PLATES)	825	825	-	-		_	8 -10d	6 -10d
DSP (1-PLATE)	825	600		-			8 -10d	2 -10d
SP1	585	535		-			6 -10d	4 -10d
SP2	1065	605	-	-		-	6 -10d	6 -10d
SP4	885	760					6-10d x 1 1/2"	0-100
SPH4	1240	1065	-	\vdash		_	10-10d x 1 1/2"	-
SP6	885	760	-	-		_	6-10d x 1 1/2"	
SPH6	1240	1065		-			10-10d x 1 1/2"	
LSTA18	1235	1110	-	-			14-10d	
LSTA21	1235	1235		-		_	16-10d	
CS20	1030	1030		\vdash			14-10d	
CS16	1705	1705			-	-	22-10d	
HEAVY GIRDER TIEDOWNS		UPLIFT	F1 SYP	F2 SVP	F1 SPF	F2	TO GIRPER	TO BEAM O
LGT2	2050	1785	700	170	700		14-16d	14-16d
LGT3-SDS2.5	3685	2655	795	410		410	12-SDS 1/4" x 2 1/2"	26-16dS
MGT	3965	3330	700	410	790	410	THE SERVICE STREET, ASSESSMENT OF THE SERVICE STREET, STREET, STREET, STREET, STREET, STREET, STREET, STREET,	
HGT-2		10070002	_			_	22 -10d	Control of the Contro
HGT-2	10980	6485					16 -10d	2-5/8" ANCH
HGT-3	10980 10530	6485 9035	F1	F2			16 -10d 16 -10d	2-5/8" ANCH 2-5/8" ANCH
	10980 10530	6485	F1 SYP	F2 SYP			16 -10d 16 -10d TO BEAM OR POST	2-5/8" ANCH
HGT-3 EMBEDDED ANCHORS	10980 10530 UPLIFT SYP	6485 9035 UPLIFT SYP					16 -10d 16 -10d TO BEAM	2-5/8" ANCH 2-5/8" ANCH TO FOUNDATIO
HGT-3 EMBEDDED ANCHORS IETA12 - META40 (2) META (CMU)	10980 10530 UPLIFT SYP 1-PLY	9035 UPLIFT SYP 2-PLY	340	SYP			16 - 10d 16 - 10d TO BEAM OR POST 7-10d x 1 1/2"	2-5/8" ANCH 2-5/8" ANCH TO FOUNDATION
HGT-3 EMBEDDED ANCHORS JETA12 - META40 (2) META	10980 10530 UPLIFT SYP 1-PLY	9035 UPLIFT SYP 2-PLY	340 1210	725			16 -10d 16 -10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY)	2-5/8" ANCH 2-5/8" ANCH TO FOUNDATION EMBEDDE
HGT-3 EMBEDDED ANCHORS JETA12 - META40 (2) META (CMU) (2) META (CONCRETE)	10980 10530 UPLIFT SYP 1-PLY 1450	6485 9035 UPLIFT SYP 2-PLY 1450 1900	340 1210	725 1160			16 -10d 16 -10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 10-10d x 1 1/2" (1-PLY)	2-5/8" ANCH TO FOUNDATIO EMBEDDE EMBEDDE
HGT-3 EMBEDDED ANCHORS JETA12 - META40 (2) META (CMU) (2) META (CONCRETE)	10980 10530 UPLIFT SYP 1-PLY 1450 1985	6485 9035 UPLIFT SYP 2-PLY 1450 1900 2565	340 1210 1210 340	725 1160 1160			16 -10d 16 -10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 9-10d x 1 1/2"	2-5/8" ANCH TO FOUNDATIO EMBEDDE EMBEDDE EMBEDDE EMBEDDE
HGT-3 EMBEDDED ANCHORS ETA12 - META40 (2) META (CMU) (2) META (CONCRETE) ETA16 - HETA40 (2) HETA	10980 10530 UPLIFT SYP 1-PLY 1450 1985 1985	9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2500	340 1210 1210 340 1225	725 1160 1160 725			16 -10d 16 -10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 14-16d (2-PLY) 9-10d x 1 1/2" or 8-16d 10-10d x 1 1/2" or 8-16d	2-5/8" ANCH 2-5/8" ANCH TO FOUNDATIO EMBEDDE EMBEDDE EMBEDDE EMBEDDE
HGT-3 EMBEDDED ANCHORS DETA12 - META40 (2) META (CMU) (2) META (CONCRETE) ETA16 - HETA40 (2) HETA (CMU) (2) HETA	10980 10530 UPLIFT SYP 1-PLY 1450 1985 1985 1810 2035	9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2500	340 1210 1210 340 1225	725 1160 1160 725 1520			16 -10d 16 -10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 10-10d x 1 1/2" or 8-16d 10-10d x 1 1/2" or 8-16d 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY)	2-5/8" ANCH TO FOUNDATIO EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE
HGT-3 EMBEDDED ANCHORS META12 - META40 (2) META (CMU) (2) META (CONCRETE) META16 - HETA40 (2) HETA (CMU) (2) HETA (CMU) (2) HETA (CONCRETE) HHETA16 -	10980 10530 UPLIFT SYP 1-PLY 1450 1985 1985 1810 2035	6485 9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2500 2700	340 1210 1210 340 1225 1225 340	725 1160 1160 725 1520			16 -10d 16 -10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 9-10d x 1 1/2" or 8-16d 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY)	2-5/8" ANCH 2-5/8" ANCH TO FOUNDATIO EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE
HGT-3 EMBEDDED ANCHORS META12 - META40 (2) META (CMU) (2) META (CONCRETE) META16 - HETA40 (2) HETA (CMU) (2) HETA (CONCRETE) HHETA16 - HHETA40 (2) HHETA	10980 10530 UPLIFT SYP 1-PLY 1450 1985 1985 1810 2035 2035	9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2500 2700 2235 2500	340 1210 1210 340 1225 1225 340 1225	725 1160 1160 725 1520 1520 815			16 - 10d 16 - 10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 14-16d (2-PLY) 9-10d x 1 1/2" or 8-16d 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 10-10d x 1 1/2" or 9-16d	2-5/8" ANCH 2-5/8" ANCH TO FOUNDATIO EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE
HGT-3 EMBEDDED ANCHORS IETA12 - META40 (2) META (CMU) (2) META (CONCRETE) IETA16 - HETA40 (2) HETA (CMU) (2) HETA (CONCRETE) HHETA16 - HHETA40 (2) HHETA (CONCRETE) HHETA40 (2) HHETA (CMU) (2) HHETA (CMU) (2) HHETA (CMU)	10980 10530 UPLIFT SYP 1-PLY 1450 1985 1985 1810 2035 2035 2235	9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2500 2700 2235 2500 3350	340 1210 1210 340 1225 1225 340 1225	725 1160 1160 725 1520 1520 815 1520			16 -10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 9-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) TO BEAM	2-5/8" ANCH 2-5/8" ANCH TO FOUNDATIO EMBEDDE TO
HGT-3 EMBEDDED ANCHORS ETA12 - META40 (2) META (CMU) (2) META (CONCRETE) ETA16 - HETA40 (2) HETA (CMU) (2) HETA (CONCRETE) HHETA16 - HHETA40 (2) HHETA (CMU) (2) HHETA (CMU) (2) HHETA (CMU) (2) HHETA (CMU) (3) HHETA	10980 10530 UPLIFT SYP 1-PLY 1450 1985 1985 1810 2035 2035 2235 2035 UPLIFT	6485 9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2700 2295 2500 3350 UPLIFT	340 1210 1210 340 1225 1225 340 1225	725 1160 1160 725 1520 1520 815 1520			16 -10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 9-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) TO BEAM OR POST	2-5/8" ANCH 2-5/8" ANCH TO FOUNDATIO EMBEDDE FOUNDATIO
HGT-3 EMBEDDED ANCHORS JETA12 - META40 (2) META (CMU) (2) META (CONCRETE) JETA16 - HETA40 (2) HETA (CMU) (2) HETA (CONCRETE) HHETA16 - HHETA40 (2) HHETA (CONCRETE) HHETA40 (2) HHETA (CMU) (2) HHETA (CMU) (2) HHETA (CMU) (2) HHETA (CONCRETE) HOLDOWNS / POST BASE	10980 10530 UPLIFT SYP 1-PLY 1450 1985 1985 1810 2035 2035 2235 2035 2035 UPLIFT SYP	6485 9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2500 2700 2235 2500 3350 UPLIFT SPF	340 1210 1210 340 1225 1225 340 1225	725 1160 1160 725 1520 1520 815 1520			16 -10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 9-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) TO BEAM	2-5/8" ANCH 2-5/8" ANCH TO FOUNDATION EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE EMBEDDE
HGT-3 EMBEDDED ANCHORS DETA12 - META40 (2) META (CMU) (2) META (CONCRETE) ETA16 - HETA40 (2) HETA (CMU) (2) HETA (CONCRETE) HHETA16 - HHETA40 (2) HHETA (CMU) (2) HHETA (CONCRETE) HOLDOWNS / POST BASE LTT19	10980 10530 UPLIFT SYP 1-PLY 1450 1985 1985 1810 2035 2035 2035 2035 2035 UPLIFT SYP 1350	9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2500 2700 2235 2500 3350 UPLIFT SPF 1305	340 1210 1210 340 1225 1225 340 1225	725 1160 1160 725 1520 1520 815 1520			16 -10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 9-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) TO BEAM OR POST 8-16d	2-5/8" ANCH 2-5/8" ANCH TO FOUNDATIO EMBEDDE TO
HGT-3 EMBEDDED ANCHORS DETA12 - META40 (2) META (CMU) (2) META (CONCRETE) ETA16 - HETA40 (2) HETA (CMU) (2) HETA (CONCRETE) HHETA16 - HHETA40 (2) HHETA (CONCRETE) HHETA40 (2) HHETA (CMU) (2) HHETA (CMU) (2) HHETA (CMU) (2) HHETA (CMU) (2) HHETA (CONCRETE) HOLDOWNS / POST BASE LTT19 LTT131	10980 10530 UPLIFT SYP 1-PLY 1450 1985 1985 1810 2035 2035 2035 2035 2035 UPLIFT SYP 1350 2310	9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2500 2700 2235 2500 3350 UPLIFT SPF 1305 2310	340 1210 1210 340 1225 1225 340 1225	725 1160 1160 725 1520 1520 815 1520			16 -10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 19-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) TO BEAM OR POST 8-16d 18-10d x 1 1/2" 18-16d	2-5/8" ANCHO 2-5/8" ANCHO POUNDATION EMBEDDE 5/8" ANCHO 5/8" ANCHO 5/8" ANCHO 5/8" ANCHO
HGT-3 EMBEDDED ANCHORS META12 - META40 (2) META (CMU) (2) META (CONCRETE) META16 - HETA40 (2) HETA (CMU) (2) HETA (CONCRETE) HHETA16 - HHETA40 (2) HHETA (CONCRETE) HHETA40 (2) HHETA (CONCRETE) HOLDOWNS / POST BASE LTT19 LTT131 HTT16	10980 10530 UPLIFT SYP 1-PLY 1450 1985 1985 1810 2035 2035 2035 2035 2035 UPLIFT SYP 1350 2310 4175	9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2500 2700 2235 2500 3350 UPLIFT SPF 1305 2310 3695 5250	340 1210 1210 340 1225 1225 340 1225	725 1160 1160 725 1520 1520 815 1520			16 -10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) TO BEAM OR POST 8-16d 18-10d x 1 1/2" 18-16d 32-16d	2-5/8" ANCHO 2-5/8" ANCHO POUNDATION EMBEDDE 5/8" ANCHO 5/8" ANCHO 5/8" ANCHO 5/8" ANCHO 5/8" ANCHO 5/8" ANCHO
HGT-3 EMBEDDED ANCHORS JETA12 - META40 (2) META (CMU) (2) META (CONCRETE) JETA16 - HETA40 (2) HETA (CMU) (2) HETA (CONCRETE) HHETA16 - HHETA40 (2) HHETA (CONCRETE) HHETA40 (2) HHETA (CONCRETE) HHETA40 (2) HHETA (CONCRETE) HOLDOWNS / POST BASE LTT19 LTT131 HTT16 HTT122	10980 10530 UPLIFT SYP 1-PLY 1450 1985 1985 2035 2035 2035 2035 2035 2035 2035 203	9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2500 2700 2235 2500 3350 UPLIFT SPF 1305 2310 3695 5250 2215	340 1210 1210 340 1225 1225 340 1225	725 1160 1160 725 1520 1520 815 1520			16 -10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 11-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 11-16d (2-PLY)	2-5/8" ANCHO 2-5/8" ANCHO TO FOUNDATIO EMBEDDE 5/8" ANCHO
HGT-3 EMBEDDED ANCHORS META12 - META40 (2) META (CMU) (2) META (CONCRETE) META16 - HETA40 (2) HETA (CMU) (2) HETA (CONCRETE) HHETA16 - HHETA40 (2) HHETA (CMU) (2) HHETA (CMU) (2) HHETA (CMU) (2) HHETA (CMU) (1) HHETA (CMU) (2) HHETA (CMU) (2) HHETA (CMU) (3) HHETA (CMU) (4) HHETA (CMU) (5) HHETA (CMU) (6) HHETA (CMU) (7) HHETA (CMU) (8) HHETA (CMU) (9) HHETA (CMU) (1) HHETA (CMU) (1) HHETA (CMU) (2) HHETA (CMU) (3) HHETA (CMU) (4) HHETA (CMU) (5) HHETA (CMU) (6) HHETA (CMU) (7) HHETA (CMU) (8) HHETA (CMU) (9) HHETA (CMU) (1) HHETA (CMU) (1) HHETA (CMU) (2) HHETA (CMU) (3) HHETA (CMU) (4) HHETA (CMU) (5) HHETA (CMU) (6) HHETA (CMU) (7) HHETA (CMU) (8) HHETA (CMU) (9) HHETA (CMU) (1) HHETA (CMU) (1) HHETA (CMU) (2) HHETA (CMU) (3) HHETA (CMU) (4) HHETA (CMU) (5) HHETA (CMU) (6) HHETA (CMU) (7) HHETA (CMU) (8) HHETA (CMU) (9) HHETA (CMU) (10) HHETA (CMU) (11) HHETA (CMU) (12) HHETA (CMU) (13) HHETA (CMU) (14) HHETA (CMU) (15) HHETA (CMU) (16) HHETA (CMU) (17) HHETA (CMU) (18) HHETA (CMU) (19) HHETA (CMU) (19) HHETA (CMU) (20) HHETA (CMU) (10) HHETA (10980 10530 UPLIFT SYP 1-PLY 1450 1985 1810 2035 2035 2035 2035 2035 2035 2035 203	9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2500 2700 2235 2500 3350 UPLIFT SPF 1305 2310 3695 5250 2215 3285	340 1210 1210 340 1225 1225 340 1225	725 1160 1160 725 1520 1520 815 1520			16 - 10d 16 - 10d 16 - 10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 10-10d x 1 1/2" or 8-16d 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 10-10d x 1 1/2" (1-PLY)	2-5/8" ANCHO 2-5/8" ANCHO TO FOUNDATIO EMBEDDE 5/8" ANCHO
HGT-3 EMBEDDED ANCHORS JETA12 - META40 (2) META (CMU) (2) META (CONCRETE) JETA16 - HETA40 (2) HETA (CMU) (2) HETA (CONCRETE) HHETA16 - HHETA40 (2) HHETA (CMU) (3) HHETA (CMU) (4) HHETA (CMU) (5) HHETA (CMU) (6) HHETA (CMU) (7) HHETA (CONCRETE) HOLDOWNS / POST BASE LTT19 LTT131 HTT16 HTT22 HDU2-SDS2.5 HDU4-SDS2.5 HDU4-SDS2.5	10980 10530 UPLIFT SYP 1-PLY 1450 1985 1985 2035 2035 2035 2035 2035 2035 2035 203	9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2500 2700 2235 2500 3350 UPLIFT SPF 1305 2310 3695 5250 2215 3285 4065	340 1210 1210 340 1225 1225 340 1225	725 1160 1160 725 1520 1520 815 1520			16 - 10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 10-10d x 1 1/2" or 8-16d 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) TO BEAM OR POST 8-16d 18-10d x 1 1/2" 18-16d 32-16d 6-SDS 1/4" x 2 1/2" 10-SDS 1/4" x 2 1/2" 14-SDS 1/4" x 2 1/2"	2-5/8" ANCHO 2-5/8" ANCHO TO FOUNDATIO EMBEDDE 5/8" ANCHO
HGT-3 EMBEDDED ANCHORS JETA12 - META40 (2) META (CMU) (2) META (CONCRETE) JETA16 - HETA40 (2) HETA (CONCRETE) HHETA16 - HHETA40 (2) HHETA (CMU) (3) HHETA (CMU) (4) HHETA (CMU) (5) HHETA (CMU) (6) HHETA (CMU) (7) HHETA (CMU) (8) HHETA (CMU) (9) HHETA (CMU) (10) HHETA (CMU) (11) HHETA (CMU) (12) HHETA (CMU) (2) HHETA (CMU) (3) HHETA (CMU) (4) HHETA (CMU) (5) HHETA (CMU) (6) HHETA (CMU) (7) HHETA (CMU) (8) HHETA (CMU) (9) HHETA (CMU) (10) HHETA (CMU) (11) HHETA (CMU) (12) HHETA (CMU) (13) HHETA (CMU) (14) HHETA (CMU) (15) HHETA (CMU) (16) HHETA (CMU) (17) HHETA (CMU) (18)	10980 10530 UPLIFT SYP 1-PLY 1450 1985 1810 2035 2035 2035 2035 2035 2035 2035 203	9035 UPLIFT SYP 2-PLY 1450 1900 2565 1810 2500 2700 2235 2500 3350 UPLIFT SPF 1305 2310 3695 5250 2215 3285	340 1210 1210 340 1225 1225 340 1225	725 1160 1160 725 1520 1520 815 1520			16 - 10d 16 - 10d 16 - 10d TO BEAM OR POST 7-10d x 1 1/2" or 6-16d 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 10-10d x 1 1/2" or 8-16d 10-10d x 1 1/2" (1-PLY) 12-16d (2-PLY) 10-10d x 1 1/2" (1-PLY) 14-16d (2-PLY) 10-10d x 1 1/2" (1-PLY)	2-5/8" ANCHO 2-5/8" ANCHO TO FOUNDATIO EMBEDDE 5/8" ANCHO

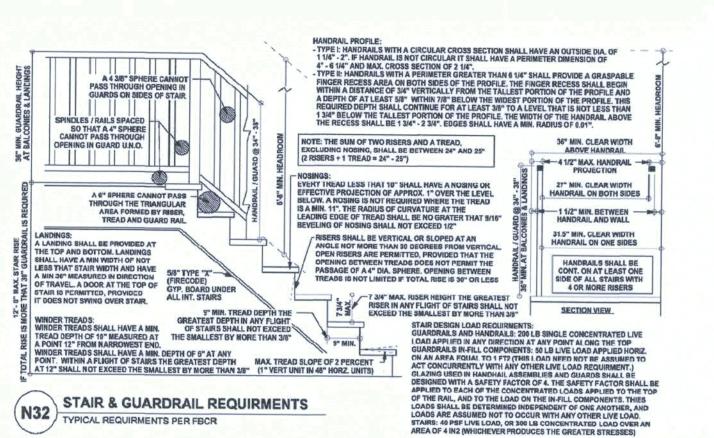
MODEL NO. | ALLOWABLE LOADS LB (WIND 1.6) |

ANCHOR TABLE

GRADE & SPECIES TABLE

	lane	i b (pai)	L (10 psi)
2x8	SYP #2	1200	1.6
2x10	SYP#2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

CYPRESS LOG LOG HOG FASTINERS — GYPRESS WALL LOG



LOG BOSS FASTENERS ARE SELF DRILLING, HIGH STRENGTH, STEEL WOOD SCREWS WITH TRU-KOTE COATING THAT EXCEEDS FM4470 CORROSION STANDARDS MANUFACTURED BY PERMA CHINK SYSTEMS, INC. SHANK DIAMETER = 0.213°, THEAD DIAMETER = 0.260°, THREAD LENGTH = 3.0° LOG BOSS ARE AVAILABLE IN LENGTHSOF 6°, 8°, 10°, 12°, 14° & 16° SCREWS IN LOG WALL ARE TO HAVE A MIN. PENETRATION OF 3° INTO LOWER LOG SELECT A THE FASTENER LENGTH ACCORDINGLY (COUNTER SNK IF NECESSARY)

LOG BOSS FASTENERS ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT FASTENER OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURERS INSTALLATION INSTRUCTIONS MUST DE FOLLOWED TO ACHIEVE RATED LOADS. LISTED LOADS FLAVE BEEN INCREASED FOR WIND DURATION, AND MUST BE ADJESTED FOR OTHER DURATIONS OR SPECIES. LOG BOSS FASTENERS DESIGN VALUE (LB)

WOOD SPECIES (.00SG = SPECIFIC GRAVITY)	SHINGLE SHEAR DESIGN VALUE	WITHDRAWAI DESIGN VALUE
SOUTHER YELLOW PINE .55 SG	296	1248
CYPRESS .43 SG SPRUCE - PINE - FIR .42 SG	231	830

ADJUSTMENT FACTORS:
WITHDRAWAL VALUES ARE BASED ON 3" PENETRATION INTO NAIN MEMBER.
WITHDRAWAL VALUES ARE BASED ON 3" WITHDRAWL VALUE SHALL BE MUTIPLIED BY: p/3"
SHEAR VALUES ARE BASED ON 100 MIN. PENETRATION INTO WAIN MEMBER. - SHERY VALUES ARE BASED ON 100 MIN. PENET RATION INTO MAIN MEMBER.
WHEN PENETRATION IS 60-9-100 SHEAR VALUE SHALL BE MUTPLED BY: g/10D
- FACTOR VALUES FOR LOAD-DURATIONS OTHER THAN 1.8 WIND DURATIONS (SHOWN)
- NOS LAG SCREW ADJUSTMENT FACTORS:
- FOR WITHDRAWAL LOADS IN END GRAIN MUTIPLY BY: .75
- FOR LATERAL LOADS IN END GRAIN MUTIPLY BY: .67
- FOR TOE-SCREWED WITHDRAWAL LOADS MUTIPLY BY: .83
- FOR TOE-SCREWED LATERAL LOADS MUTIPLY BY: .83

LOG BOSS FASTENER DATA N22 TYPICAL DESIGN VALUES

Z

-

 \geq

-

A CL MAJ

JOB NUMBER 20190222

SHEET NUMBER OF 4 SHEETS

-ROOF SYSTEM SEE ROOF PLAN AND ROOF DETAILS RAFTER CONNECTOR CEILING JOISTS SEE FRAMING PLAN (2) H3 TYPICAL U.N.O. OPTIONAL 2x6 SYP #2 RAFTER PLATE 2x6 PT SUB-FASCIA w/ LOG FASTENERS INTO LOG WALL (SAME SPACING AS IN LOG WALL) —TYPICAL LOG WALL SEE DETAILS W17-W24 LOG WALL, AND INTO RIM JOIST SEE DETAIL W24 FOR SPACING -3/4" T&G FLOOR SHEATHING W/ 8d @ 8" OC (TYP.) - OPTIONAL LOG SIDING

-RIM JOIST-SEE FLOOR FRAMING PLAN

PT SILL PLATE - SEE FOUNDATION DETAILS

— (2) 16d TOE- NAILED

TOP VIEW

- 2 LCG FASTENERS TYPICAL @ ALL

LOGJOINTS AND SPLICES

SIDE VIEW

W23 LOG WALL JOINT & SPLICE DETAIL

(2) 16d TOI- NAILED

FOUNDATION - SEE FOUNDATION PLAN & DETAILS

LOG FASTENERS TYPICAL

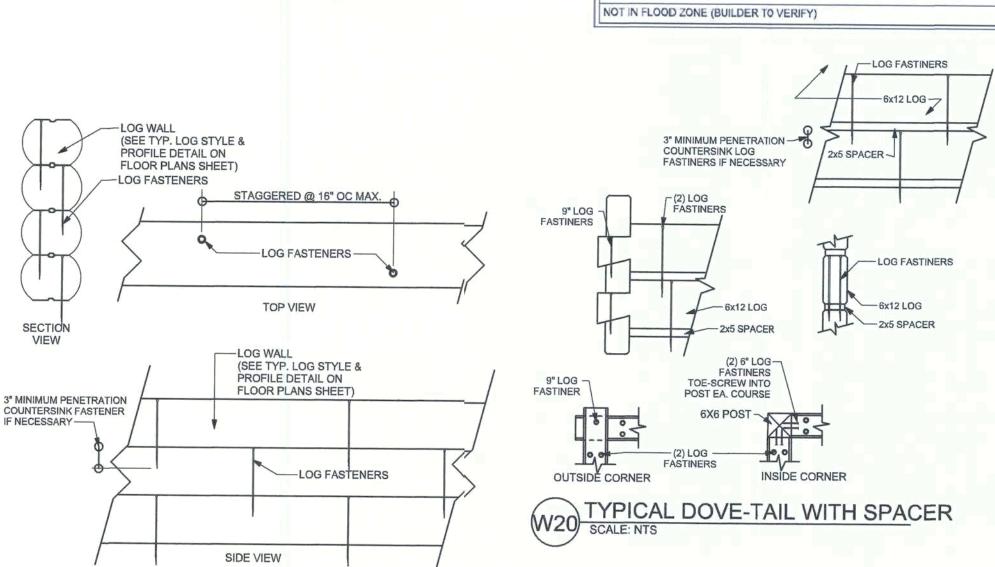
ALL LOG JOINTS AND SPLICES

(SEE TYP. LOG STYLE &

PROFILE DETAIL ON

FLOOR PLANS SHEET

SEE FLOOR FRAMING PLAN



1060