

Project Information for:				L216061
Builder:	WOODMAN PARK	Date:	11/2/2006	
Lot:	LOT 10 WILSON PLACE	Start Number:	1043	
Subdivision:	N/A	SEI Ref:	L216061	
County or City:	COLUMBIA COUNTY			
Truss Page Count:	64			

Truss Design Load Information (UNO)				Design Program: MITek	Building Code:	FBC2004
Gravity	42	Wind Standard:	ASCE 7-02			
Roof (psf):		Wind Speed (mph):	110			
Floor (psf):	55					

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

WOOD, WILLIAM G. CBC058182

Address: PO BOX 3535

LAKE CITY,FLORIDA 32056

Designer:

183

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987

Company: Structural Engineering and Inspections, Inc. EB 9196

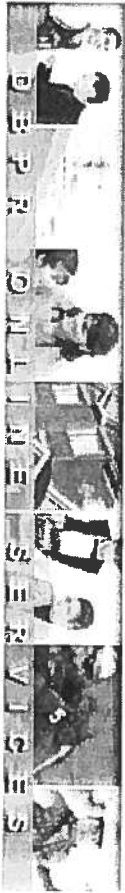
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/ITP1
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for vertical loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitability and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1102061043	11/2/2006	41	T05A	1102061083	11/2/2006
2	CJ11A	1102061044	11/2/2006	42	T06	1102061084	11/2/2006
3	CJ1A	1102061045	11/2/2006	43	T06A	1102061085	11/2/2006
4	CJ1B	1102061046	11/2/2006	44	T06B	1102061086	11/2/2006
5	CJ1C	1102061047	11/2/2006	45	T07	1102061087	11/2/2006
6	CJ1D	1102061048	11/2/2006	46	T08	1102061088	11/2/2006
7	CJ1E	1102061049	11/2/2006	47	T09	1102061089	11/2/2006
8	CJ2	1102061050	11/2/2006	48	T10	1102061090	11/2/2006
9	CJ2A	1102061051	11/2/2006	49	T11	1102061091	11/2/2006
10	CJ2B	1102061052	11/2/2006	50	T12	1102061092	11/2/2006
11	CJ2C	1102061053	11/2/2006	51	T13	1102061093	11/2/2006
12	CJ2D	1102061054	11/2/2006	52	T14	1102061094	11/2/2006
13	CJ2E	1102061055	11/2/2006	53	T15	1102061095	11/2/2006
14	CJ3	1102061056	11/2/2006	54	T16	1102061096	11/2/2006
15	CJ3A	1102061057	11/2/2006	55	T17	1102061097	11/2/2006
16	CJ3B	1102061058	11/2/2006	56	T18	1102061098	11/2/2006
17	CJ3C	1102061059	11/2/2006	57	T19	1102061099	11/2/2006
18	CJ4	1102061060	11/2/2006	58	T19G	1102061100	11/2/2006
19	CJ4A	1102061061	11/2/2006	59	T20	1102061101	11/2/2006
20	CJ4B	1102061062	11/2/2006	60	T20G	1102061102	11/2/2006
21	CJ5	1102061063	11/2/2006	61	T21	1102061103	11/2/2006
22	CJ5A	1102061064	11/2/2006	62	T22	1102061104	11/2/2006
23	CJ6	1102061065	11/2/2006	63	T23	1102061105	11/2/2006
24	CJ6A	1102061066	11/2/2006	64	T24	1102061106	11/2/2006
25	CJ6B	1102061067	11/2/2006				
26	CJ9	1102061068	11/2/2006				
27	EJ4	1102061069	11/2/2006				
28	EJ5	1102061070	11/2/2006				
29	EJ7	1102061071	11/2/2006				
30	EJ7A	1102061072	11/2/2006				
31	HJ11	1102061073	11/2/2006				
32	HJ12	1102061074	11/2/2006				
33	HJ7	1102061075	11/2/2006				
34	HJ7A	1102061076	11/2/2006				
35	HJ8	1102061077	11/2/2006				
36	T01	1102061078	11/2/2006				
37	T02	1102061079	11/2/2006				
38	T03	1102061080	11/2/2006				
39	T04	1102061081	11/2/2006				
40	T05	1102061082	11/2/2006				

NOV 02 2006



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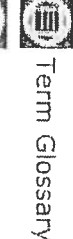
Licensee Details

Licensee Information

Name: **WOOD, WILLIAM G (Primary Name)**
WOODMAN PARK BUILDERS INC (DBA Name)
Main Address: **P.O.BOX 3535**
LAKE CITY Florida 32026
License Mailing:
License Location: **P.O.BOX 3535**
LAKE CITY FL 32026

License Information

License Type: **Certified Building Contractor**
Rank: **Cert Building**
License Number: **CBC058182**
Status: **Current,Active**
Licensure Date: **10/06/1997**
Expires: **08/31/2006**



Term Glossary

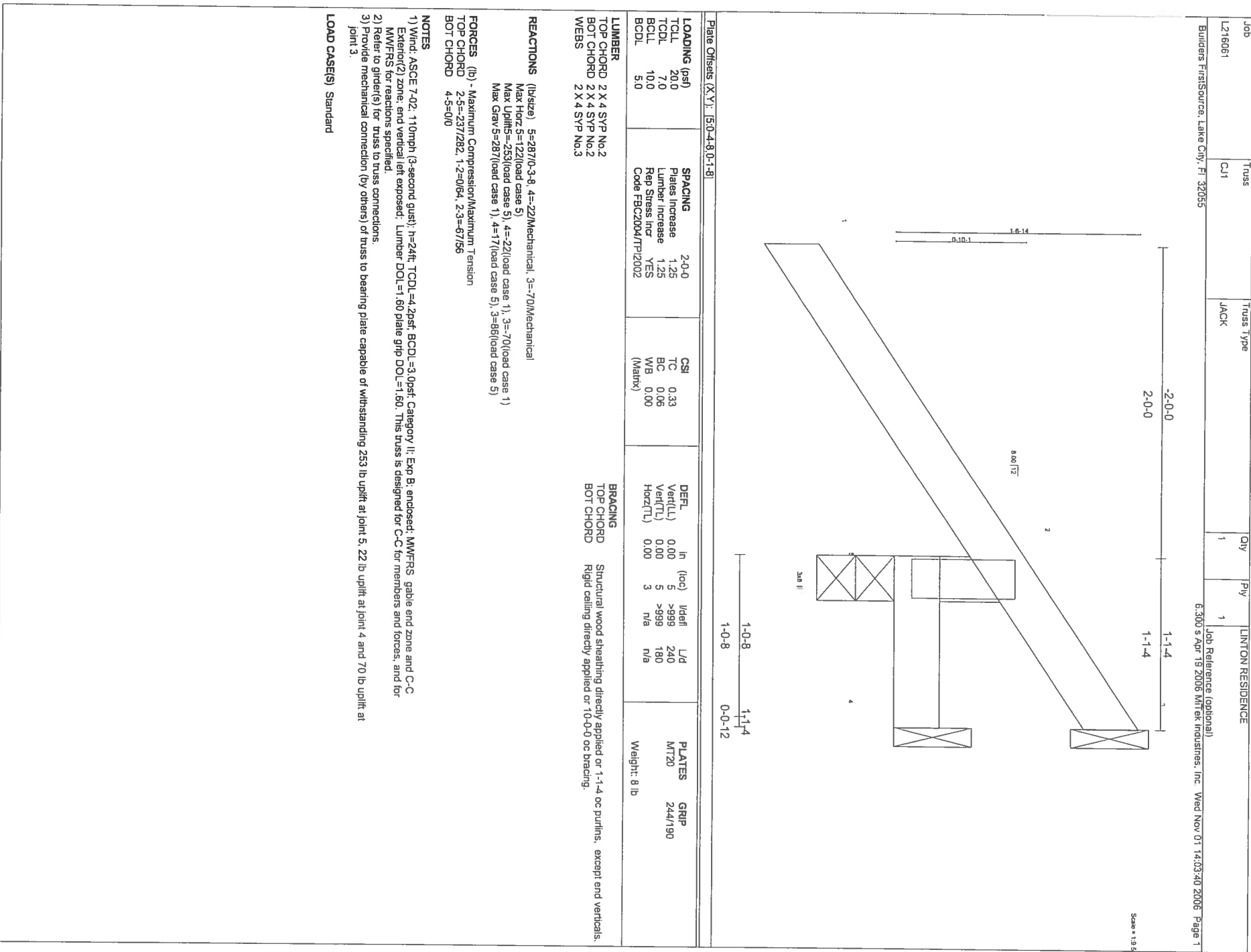


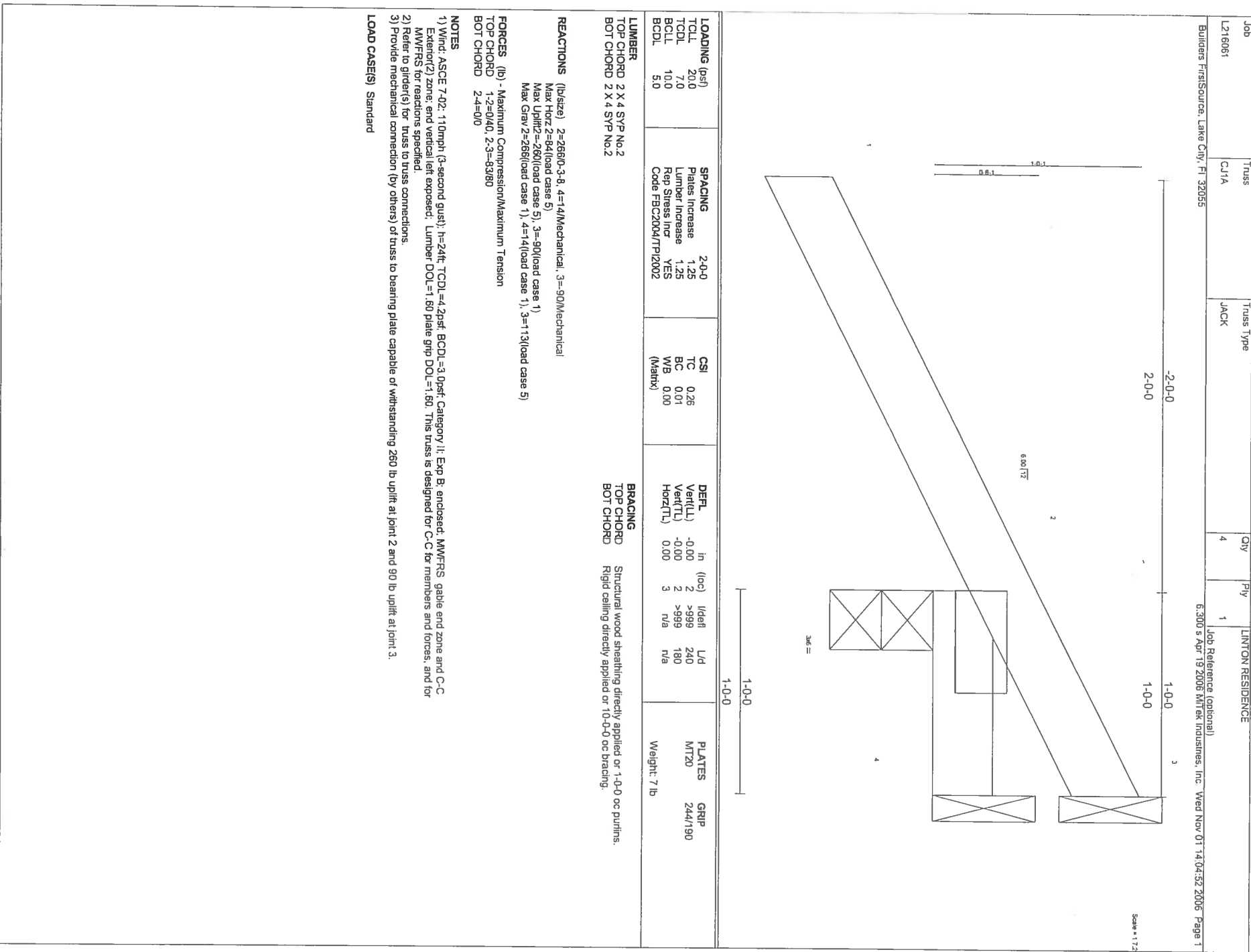
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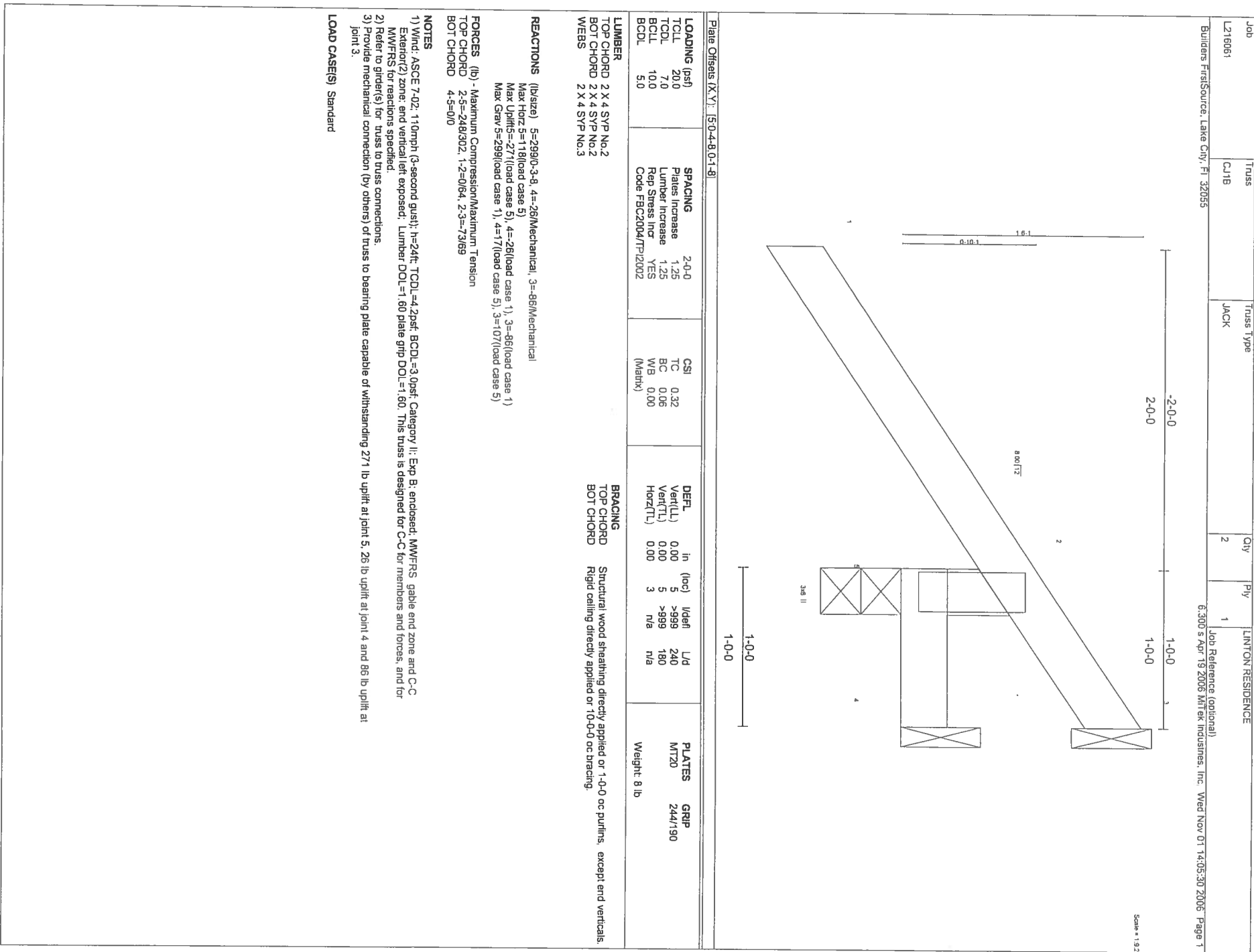


NOVEMBER 02, 2006 TRUSS DESIGN ENGINEER:

THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



Job	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	C1/C	JACK	1	1
Builders FirstSource, Lake City, FL 32055				
6,300 s Apr 19 2005 Mitek Industries, Inc. Wed Nov 01 14:05:54 2005 Page 1				
Job Reference (optional)				

The diagram illustrates a structural truss assembly. Key features include:

- Members:** Labeled with numbers 1 through 5.
- Dimensions:** Horizontal spans are indicated as 2-0-0, 1-1-12, and 1-1-12. Vertical heights are shown as 1-7-3 and 6-10-1.
- Connections:** Various joint types are depicted, including gable end zones and C-C connections.
- Scale:** A scale bar at the bottom right indicates "Scale = 1/8\"/>

Plate Offsets (X,Y) : 5-0-4-8-0-1-8)				
LOADING (psf)	SPACING	CSI	DEFL	PLATES
TOLL 20.0	Plates Increase 2-0-0	TC 0.33	In (oc)	MT20 GRIP
TCOL 7.0	Lumber Increase 1.25	BC 0.07	5 >999	24x190
BCOL 10.0	Rep Stress Incr YES	WB 0.00	>999	
BCDL 5.0	Code FBC2004/PI2002	(Max)	n/a	Weight: 8 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 1-1-12 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

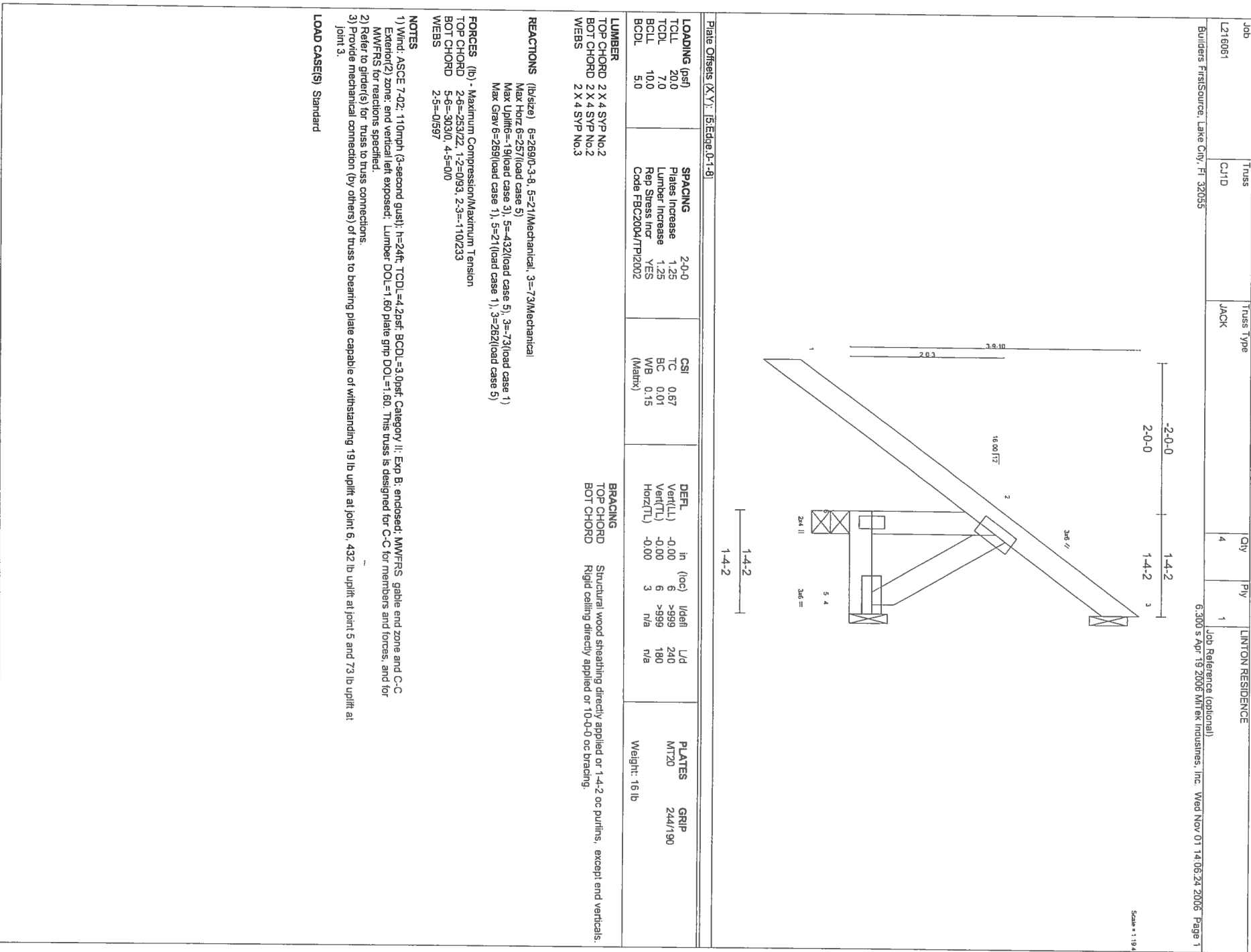
REACTIONS (lb/size) 5=2830/-3-8, 4=20/Mechanical, 3=64/Mechanical
 Max Horiz 5=123(load case 5)
 Max Uplifts=-248(load case 5), 4=-20(load case 1), 3=-64(load case 1)
 Max Grav 5=283(load case 1), 4=17(load case 5), 3=79(load case 5)

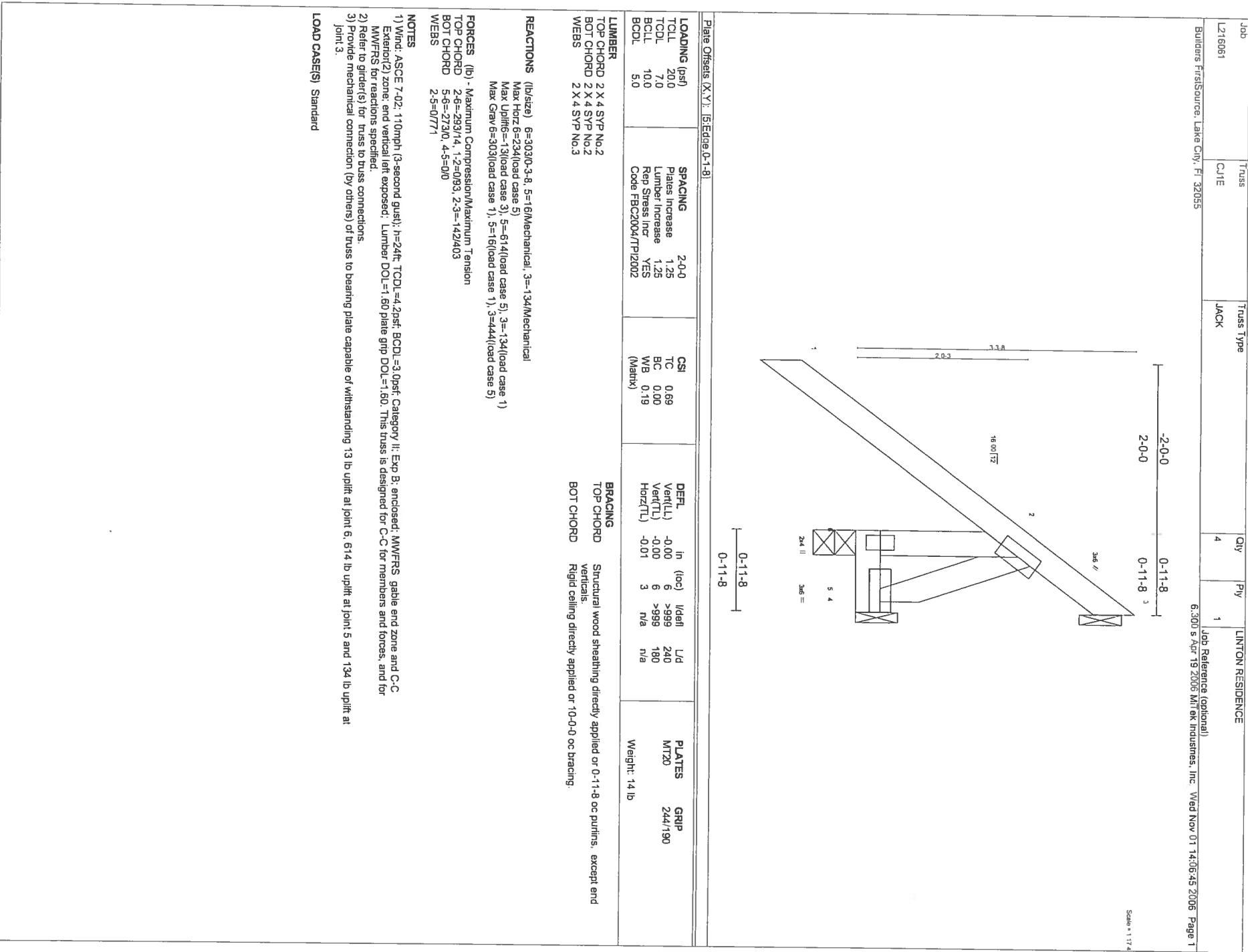
FORCES (lb) - Maximum CompressionMaximum Tension
 TOP CHORD 2-5=-234/276, 1-2=0/64, 2-3=-65/51
 BOT CHORD 4-5=0/0

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=24ft; TCOL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extern(2) zone, end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Refer to girders for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 248 lb uplift at joint 5, 20 lb uplift at joint 4 and 64 lb uplift at joint 3.

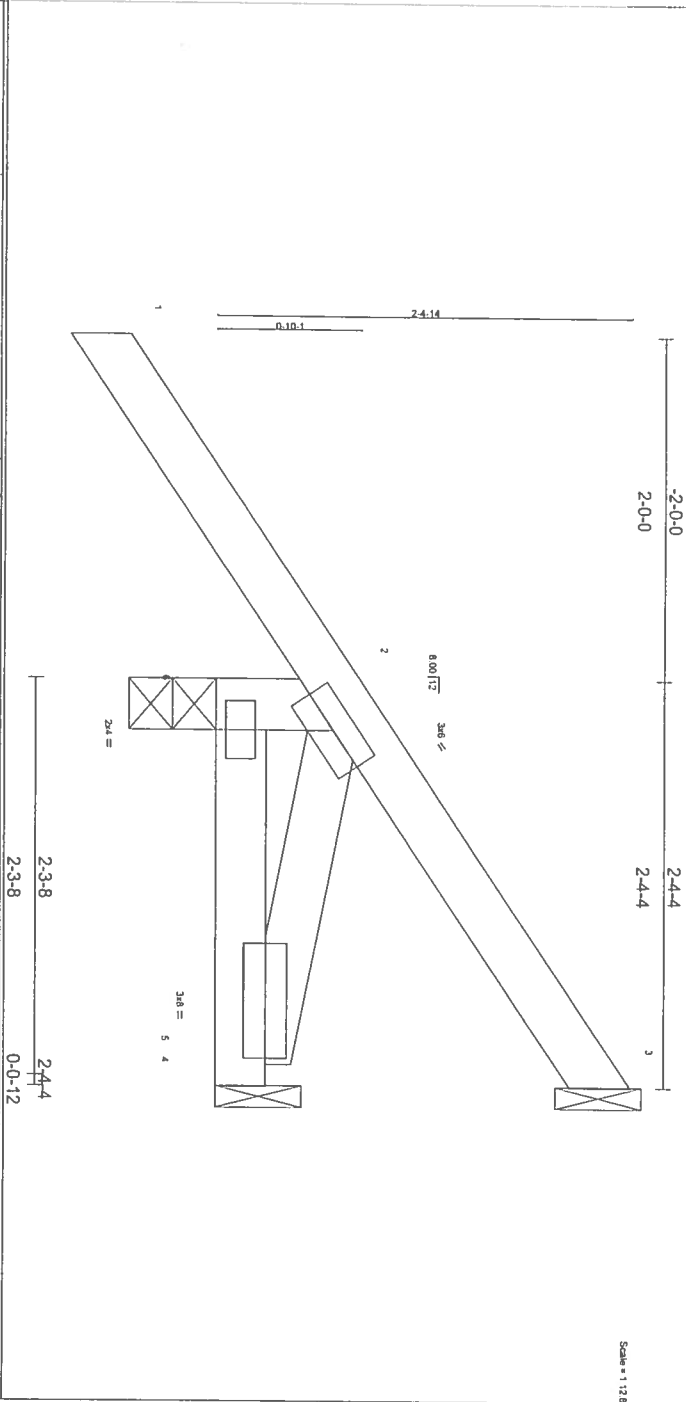
LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	UNIT/RESIDENCE
L216061	C42	JACK	1	1	Job Reference (optional)

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates increase 2.0-0	TC 0.32	in (loc) ldefl	M720	244/190
TCLL 7.0	Lumber increase 1.25	BC 0.03	Vert(L) -0.00 6 >999		
BCLL 10.0	Rep Stress incr YES	WB 0.05	Vert(TL) -0.00 5-6 >999		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)	Horz(TL) -0.00 3 n/a		
				Weight: 15 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP NO.2	TOP CHORD	2 X 4 SYP NO.2
BOT CHORD	2 X 4 SYP NO.2	BOT CHORD	2 X 4 SYP NO.2
WEBS	2 X 4 SYP NO.3		
Structural wood sheathing directly applied or 2-4-4 oc purlins, except end verticals.			
Rigid ceiling directly applied or 10-0-0 oc bracing.			

REACTIONS

(lbs/size) 3=0/Mechanical, 5=36/Mechanical, 6=263/0-3-8
 Max Horiz 6=173(load case 5)
 Max Uplift3=-10(load case 6), 5=-41(load case 5), 6=-175(load case 5)
 Max Grav 3=34(load case 3), 5=36(load case 1), 6=263(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

2-5=0/201

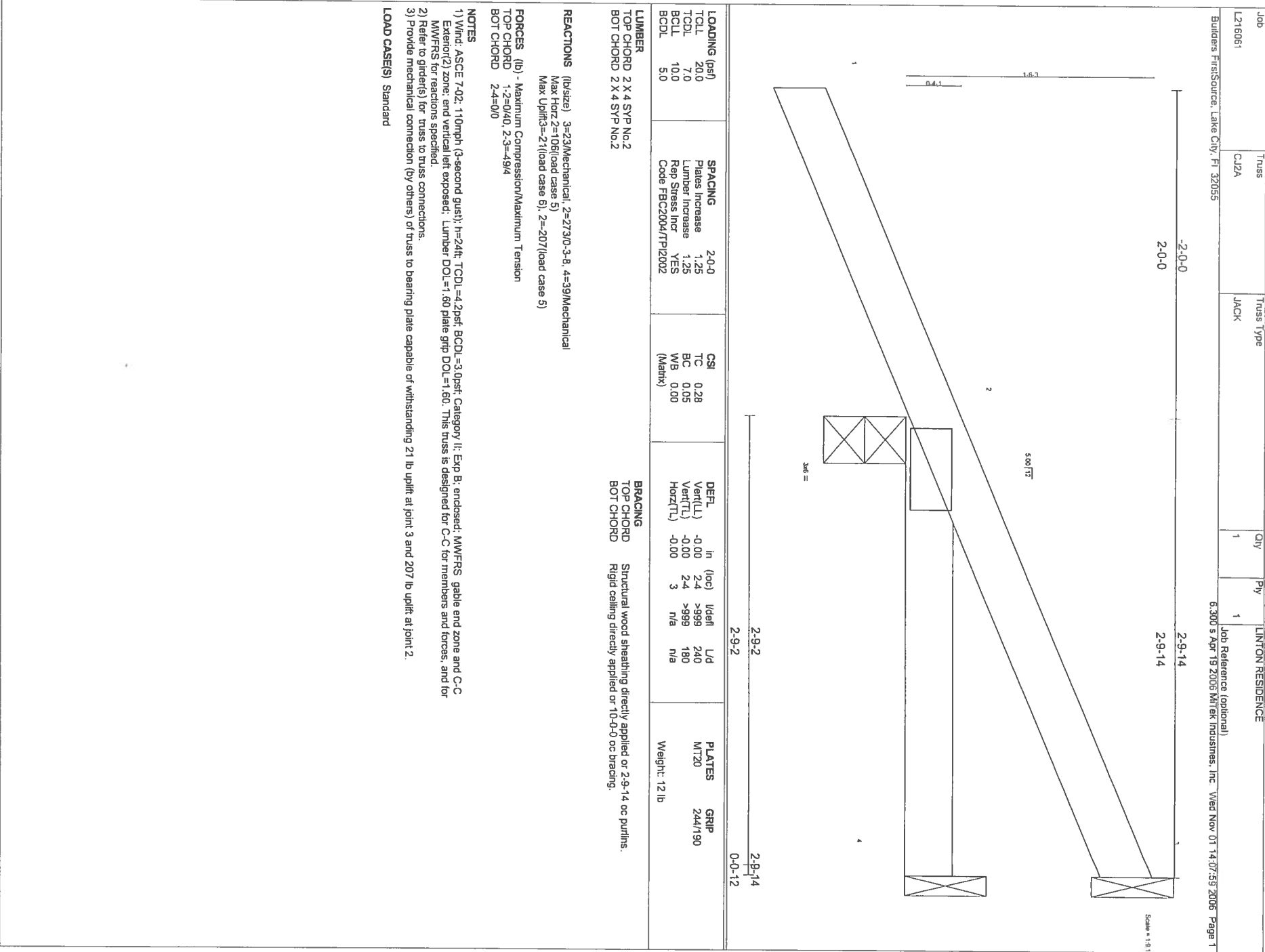
NOTES

- 1) Wind: ASCE 1, -0.2; 110mph (3-second gust); $T=24ft$; $TCDL=2.9psi$; $BCD=3.0psi$; Category II; Exp B; enclosed; MWFFS, gable end zone and C-C (Exterior) zone, and vertical lift exposed; Lumber $DOL=1.60$ plate girth $DOL=1.60$. This truss is designed for C-C for members and for MWFFS for reactions specified.
- 2) Refer to orders for truss to truss connections.
- 3) Bearing at joints/5 considers parallel to grain value using $ANSI/TPI 1$ angle to grain formula. Building designer should verify capacity of bearing surfaces.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 3, 41 lb uplift at joint 5 and 175 lb uplift at joint 6.

LOAD CASE(S) Standard

LOAD CASE(S) Standard

NOVEMBER 02, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	C42B	JACK	1	1	Job Reference (optional)

6.300 s Apr 19 2006 Intek Industries, Inc. Wed Nov 01 14:12:04 2006 Page 1

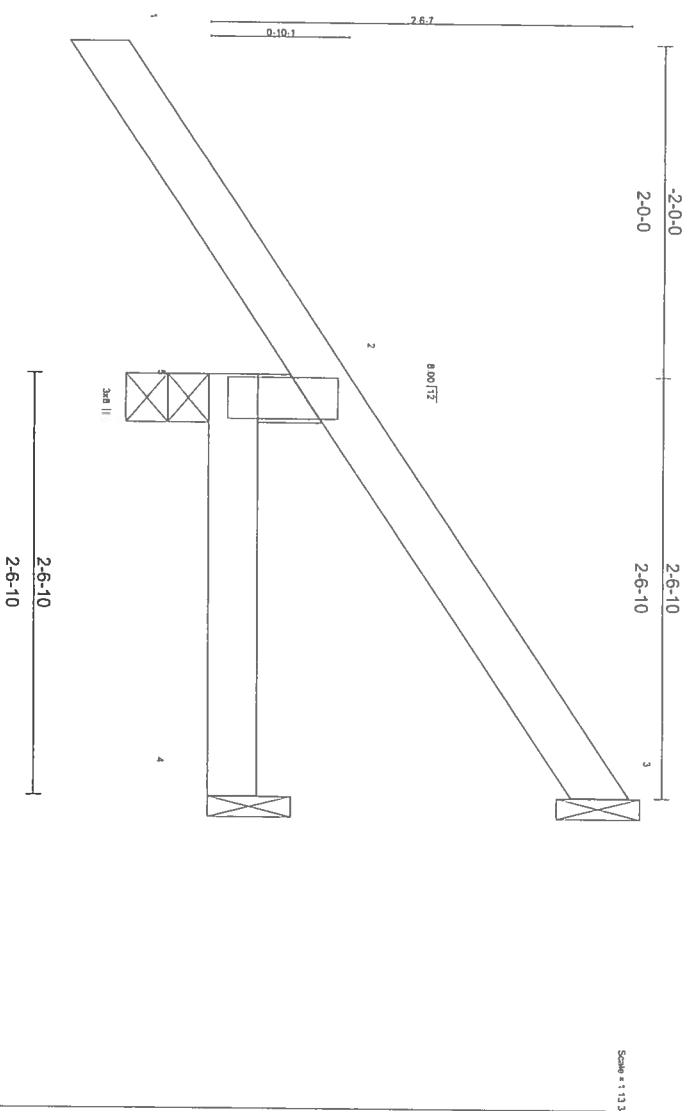


Plate Offsets (X,Y) : [50,-4,-8,0,-1,-8]					
	LOADING (psi)	SPACING	CSI	DEFL	PLATES
TCLL	20.0	Plates Increase	TC 0.38	In (loc)	GRIP
TCDL	7.0	Lumber Increase	BC 0.08	>999	MZD
BCLL	10.0	Rep Stress Incr	WB 0.00	>999	244/190
BCCL	5.0	Code FBC2004/TP12002	(Matrix)	n/a	
				n/a	Weight: 13 lb

TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 2-6-10 cc purlins, except end verticals.
BOT CHORD	2 X 4 SYP No.2		
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 cc bracing.

REACTIONS
(lbs/size) 5=25/70,3-8, 3=28/Mechanical, 4=18/Mechanical
Max Ft/oz 5=17/9(load case 5), 3=34/load case 5)
Max Uplift=172/load case 5), 3=34/load case 6)

FORCES	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	2-5=215/206, 1-2=0/64, 2-3=55/12
BOT CHORD	4-5=0/0

NOTES

(1) Wind: ASCE 7-02: 110mph (3-second gust) $h=24$ ft, TCDF = 4.2%, BCLD = 3.0%, Category II; Exp. B, enclosed; MWFRS, gable end zone and C-C Extension(2) zone; end vertical left exposed; Lumber DOL = 1.60 plate gip DOL = 1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

(2) Refer to girders(s) for truss to truss connections.

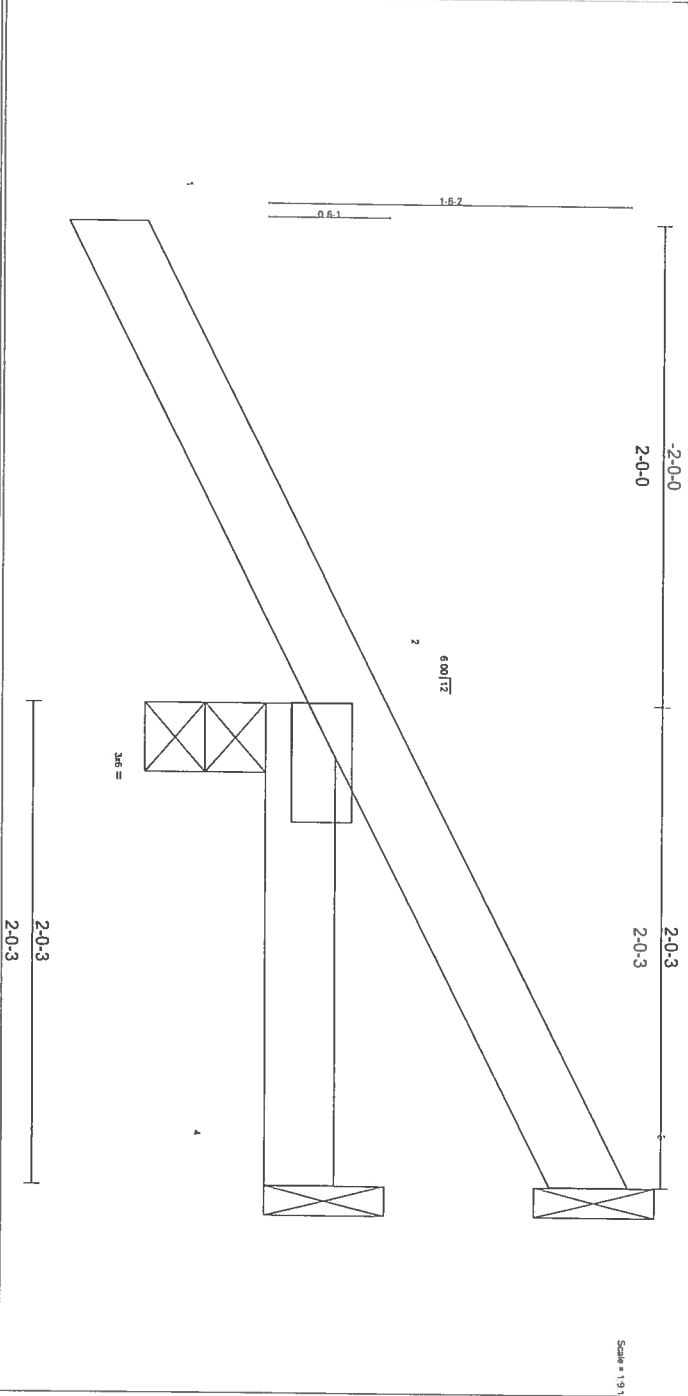
(3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 172 lb uplift at joint 5 and 34 lb uplift at joint 3.

LOAD CASE(S)	Standard
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Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	C2C	JACK	1	1	

Job Reference (optional)

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LOADING (psf)	SPPACING	CSI	DEFL	PLATES	GRIP
TCL	Plates Increase	TC 0.28	in (loc)	L/def	MIT20
TDCL	Lumber Increase	BC 0.02	Vert(TL)	L/d	244/190
BCLL	Rep Stress Incr	WB 0.00	Horz(TL)		
BCDL	Code FBC2004/TP2002	(Matrix)			
5.0				Weight: 10 lb	

TOP CHORD	2 X 4 SYP No.2	BOT CHORD	2 X 4 SYP No.2
TOP CHORD	Structural wood sheathing directly applied or 2-0-3 cc purlins	TOP CHORD	Structural wood sheathing directly applied or 2-0-3 cc purlins
BOT CHORD	Rigid ceiling directly applied or 10-0-0 cc bracing	BOT CHORD	Rigid ceiling directly applied or 10-0-0 cc bracing

REACTIONS (lb/size) 3=20/Mechanical, 2=261/0-3-8, 4=27/Mechanical
(Max Heat =106/used 2000 F)

Max Uplift3=-20(load case 1), 2=-215(load case 5)
Max Grav 3=32(load case 5), 2=261(load case 1), 4=27(load case 1)

TOP CHORD 1-2=0/41, 2-3=64/16
BOT CHORD 2-4=0/0

NOTES

Wind: ASCE 7-02, 1.0 mph (3-second gust); $n=24$ ft
 (Note: ASCE 7-02, 1.0 mph (3-second gust); $n=24$ ft
 Exh10-2) zone, and vertical left exposed. Lumber DOL=1.60 plate girth DOL=1.60. This truss is designed for C-C for members and forces, and for
 MWFRS for reactions specified

2) Refer to girder(s) for truss to truss connections

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 3 and 215 lb uplift at joint 2

-LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	C/LD	JACK	4	1	Job Reference (optional)

6:300 5 Apr '19 2006 Mfrk Industries, Inc. Wed Nov 01 14:14:01 2006 Page 1

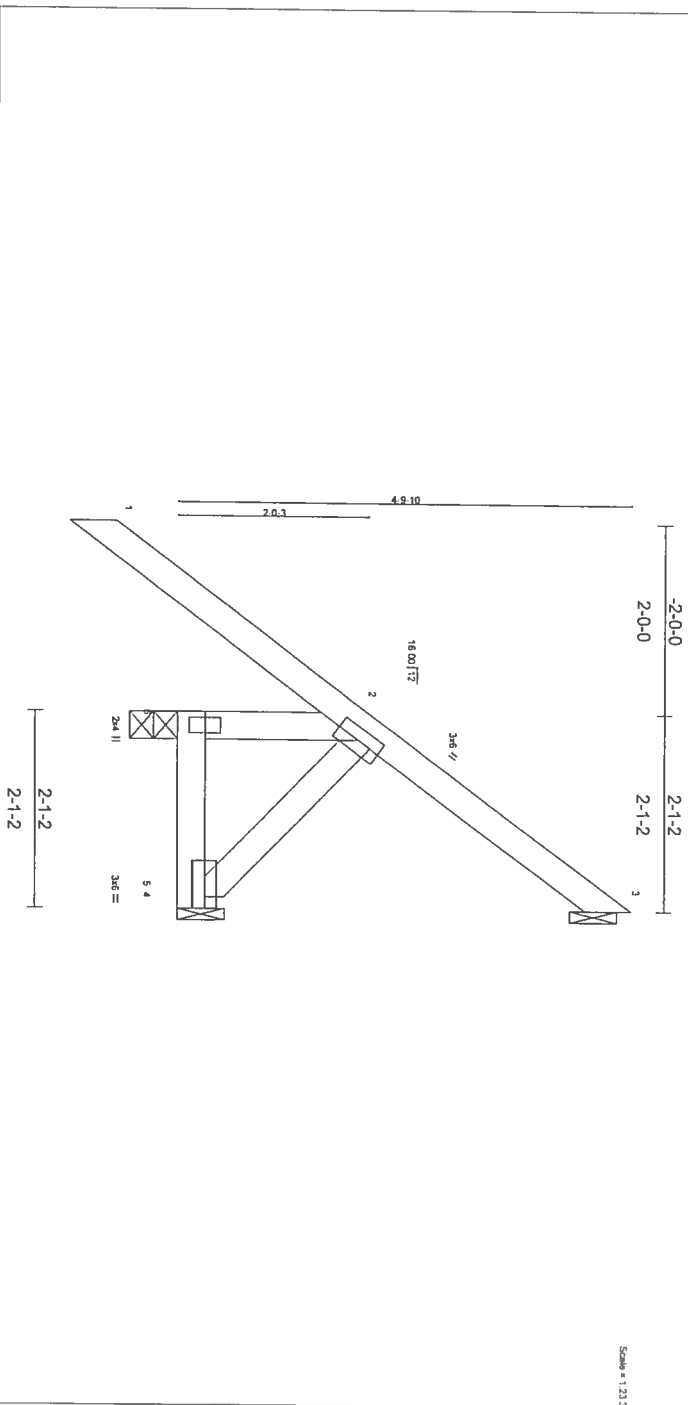


TABLE C-10 TRADE CRUISES IN THE BERING OCEAN					
LOADING (psf)	SPACING	CSI	DEFL.	L/D	PLATES
TOLL 20.0	Plates Increase 1.25	TC 0.65	In (in) 6	>999 240	GRIP
TCDL 7.0	Lumber Increase 1.25	BC 0.02	Ver(1/L) -0.00	>999 180	MT20 244/150
BCLL 10.0	Rap Stress Incr YES	WB 0.13	Horz(TL) -0.00	n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			Weight: 20 lb

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 2'-1'-2" oc purlins, except end verticals.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10'-0'-0" bracing.
WEBS	2 X 4 SYP No.3		

REACTIONS (lb/size) 6=260/0-3-8, 3=15/Mechanical, 5=33/Mechanical

Max Uplift6=21(load case 3), 3=15(load case 1), 5=293(load case 5)
Max Grav6=250(load case 1), 3=96(load case 5), 5=33(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

BOT CHORD 5-6=-357/0, 4-5=0/0
WEBS 2-5=-0/505

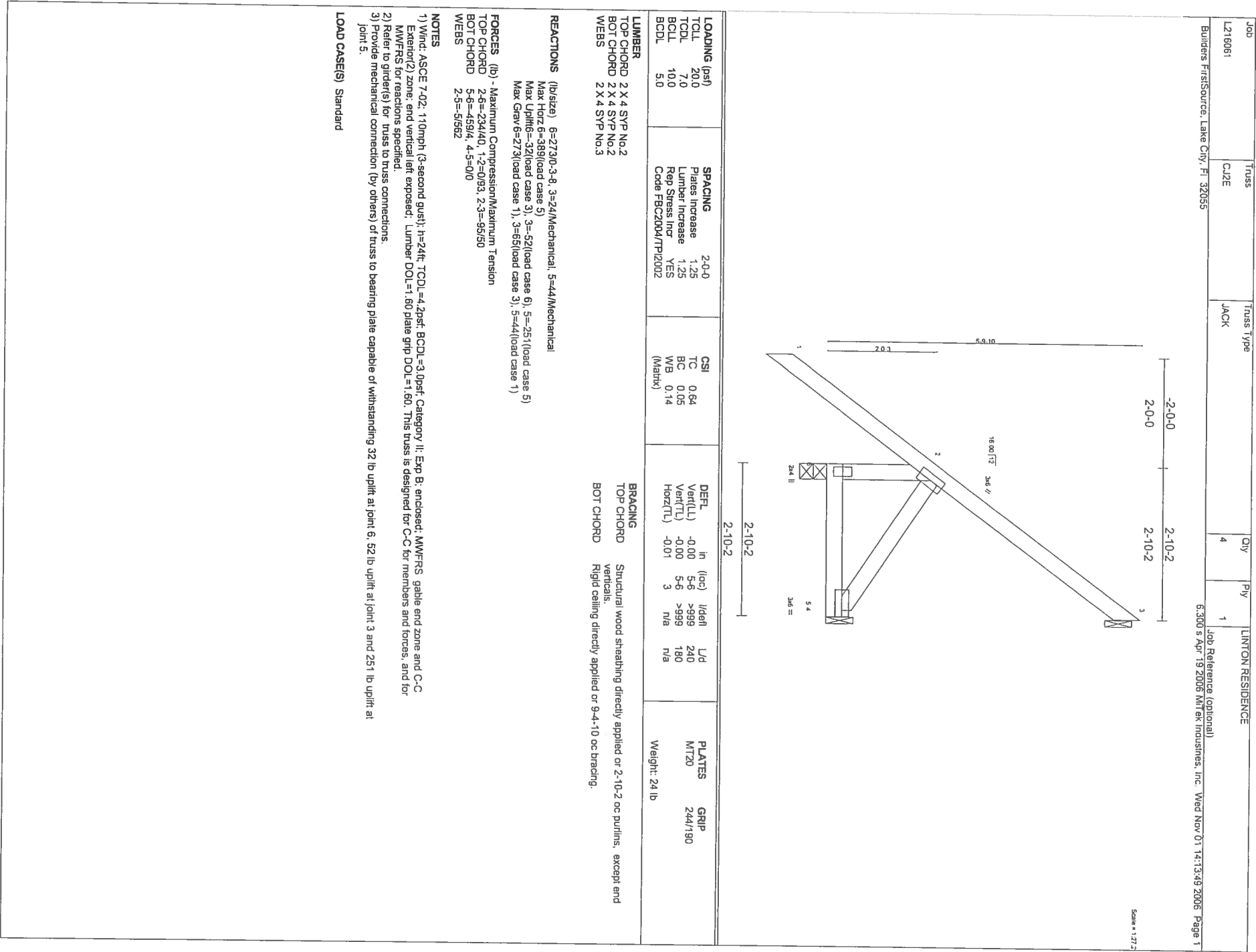
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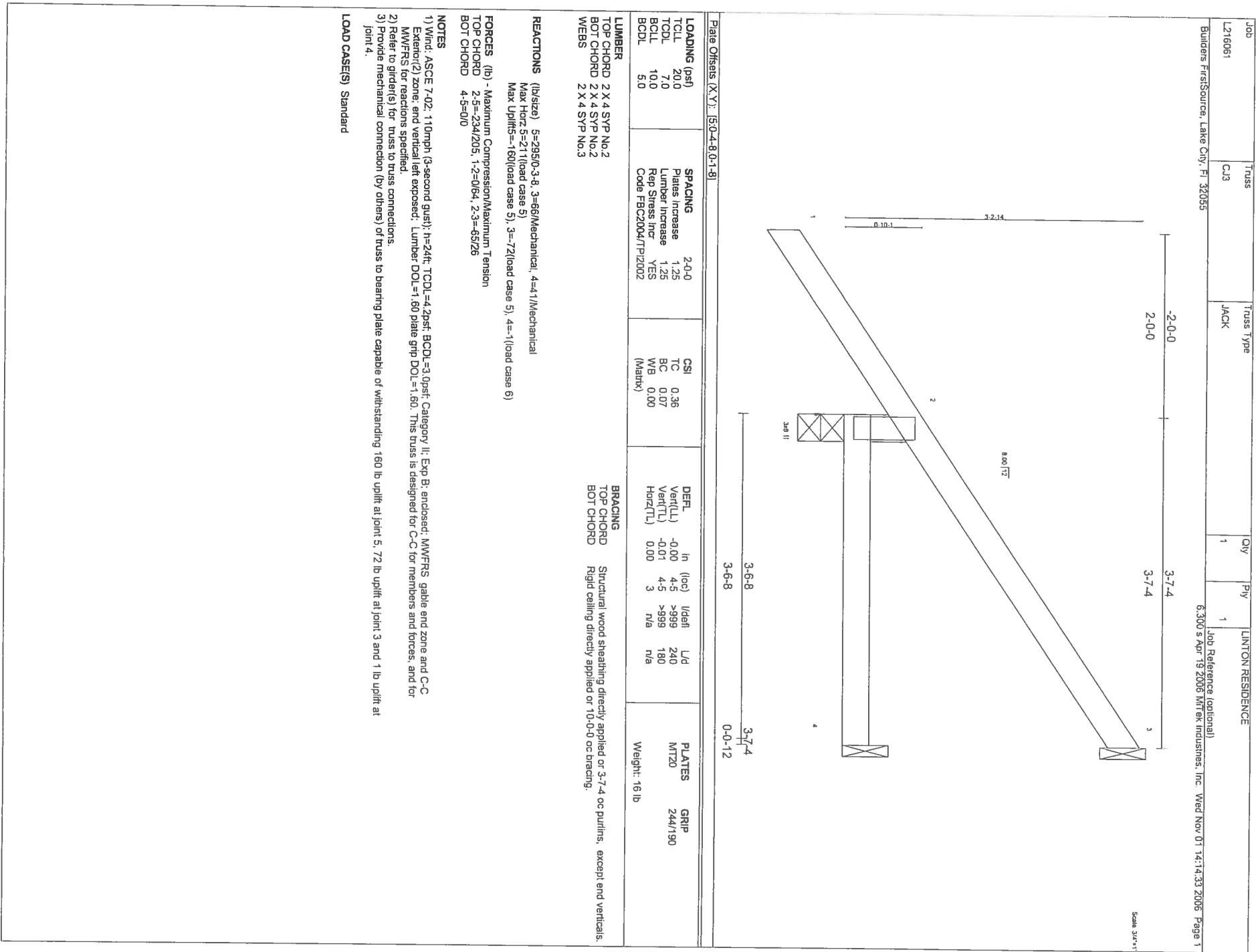
Extension(2) zone; end vertical left exposed. Lumber DOI=1.60 plate grip DOI=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

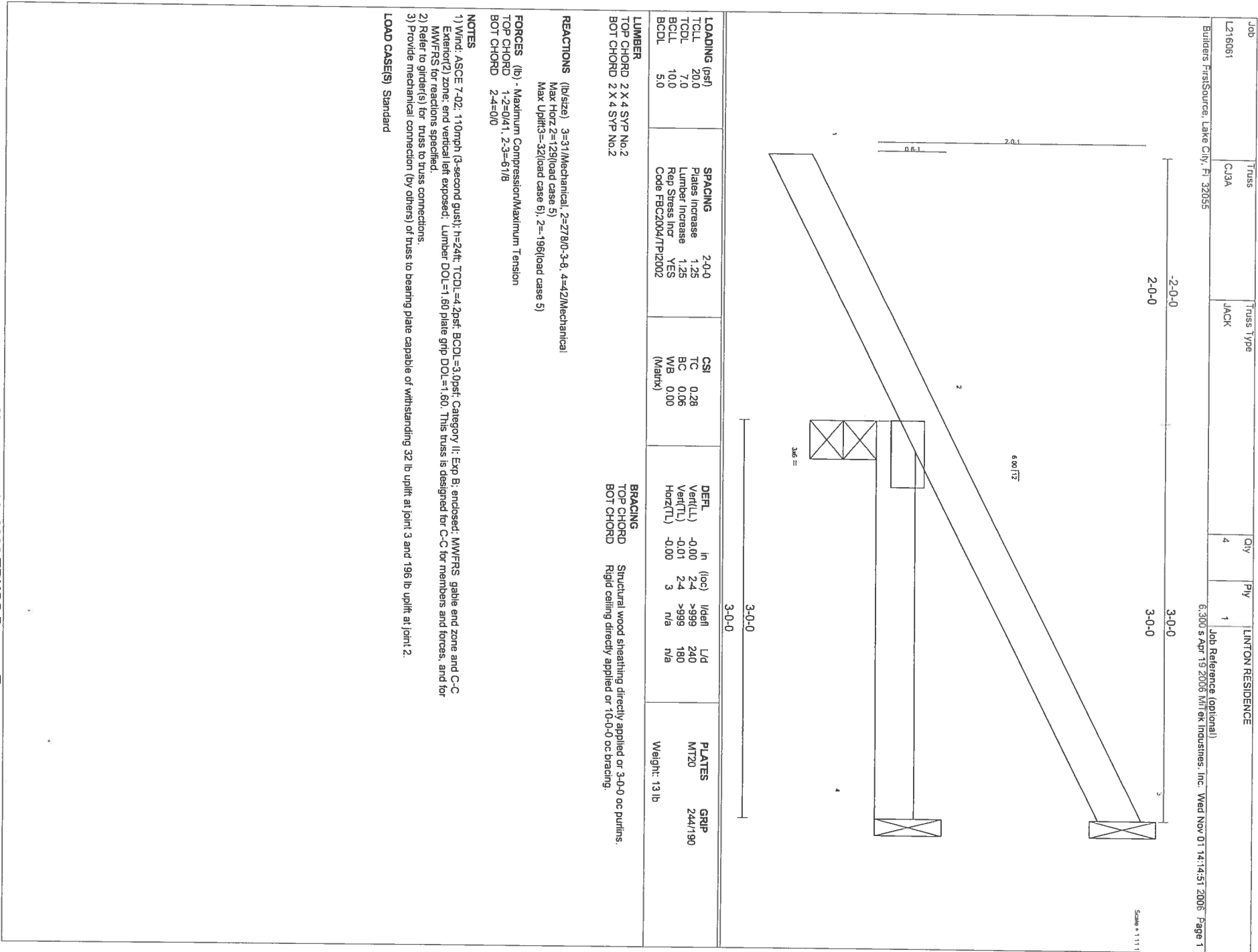
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2.1 lb uplift at joint 6, 15 lb uplift at joint 3 and 293 lb uplift at joint 5.

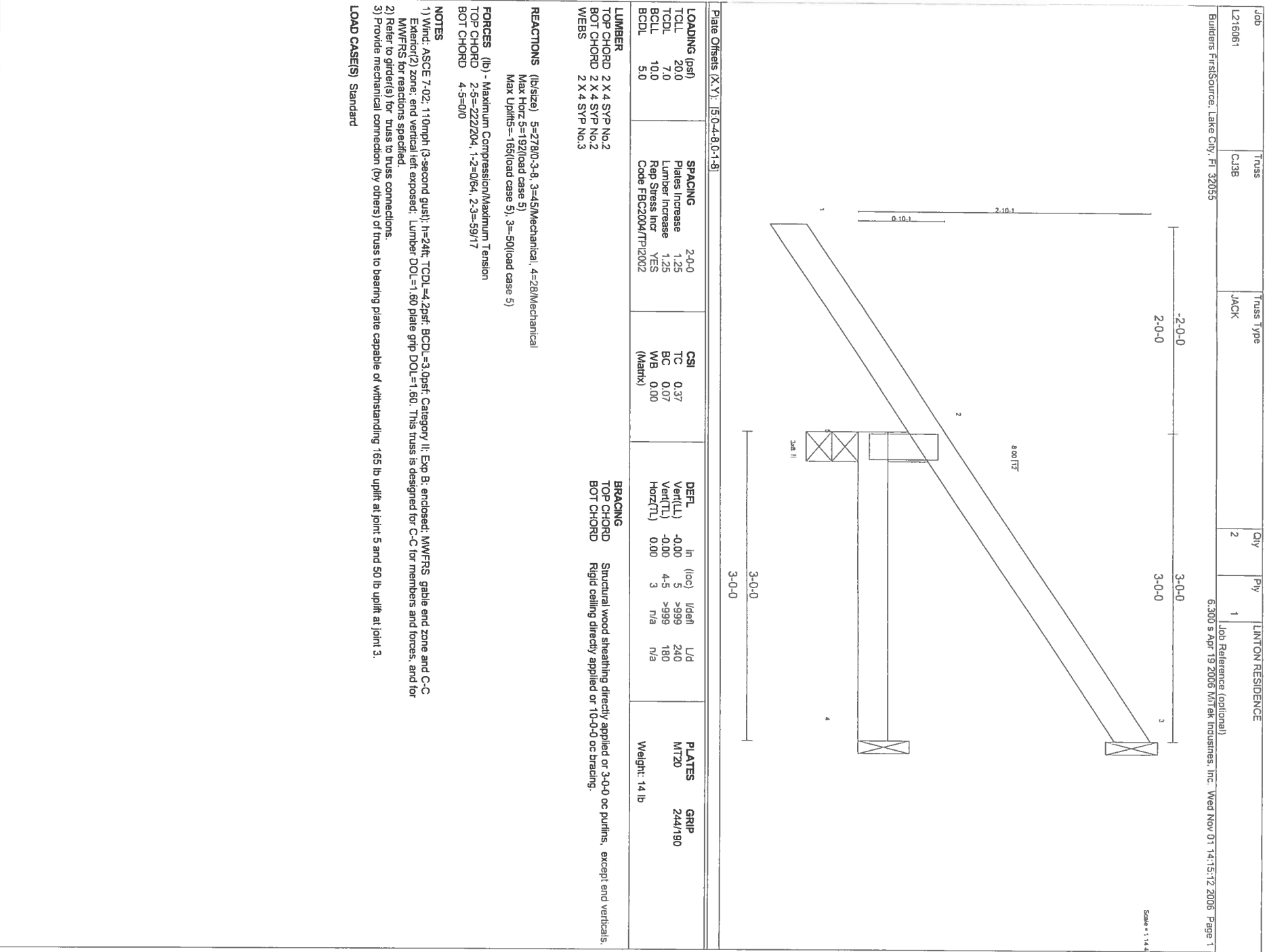
LOAD CASE(S) Standard

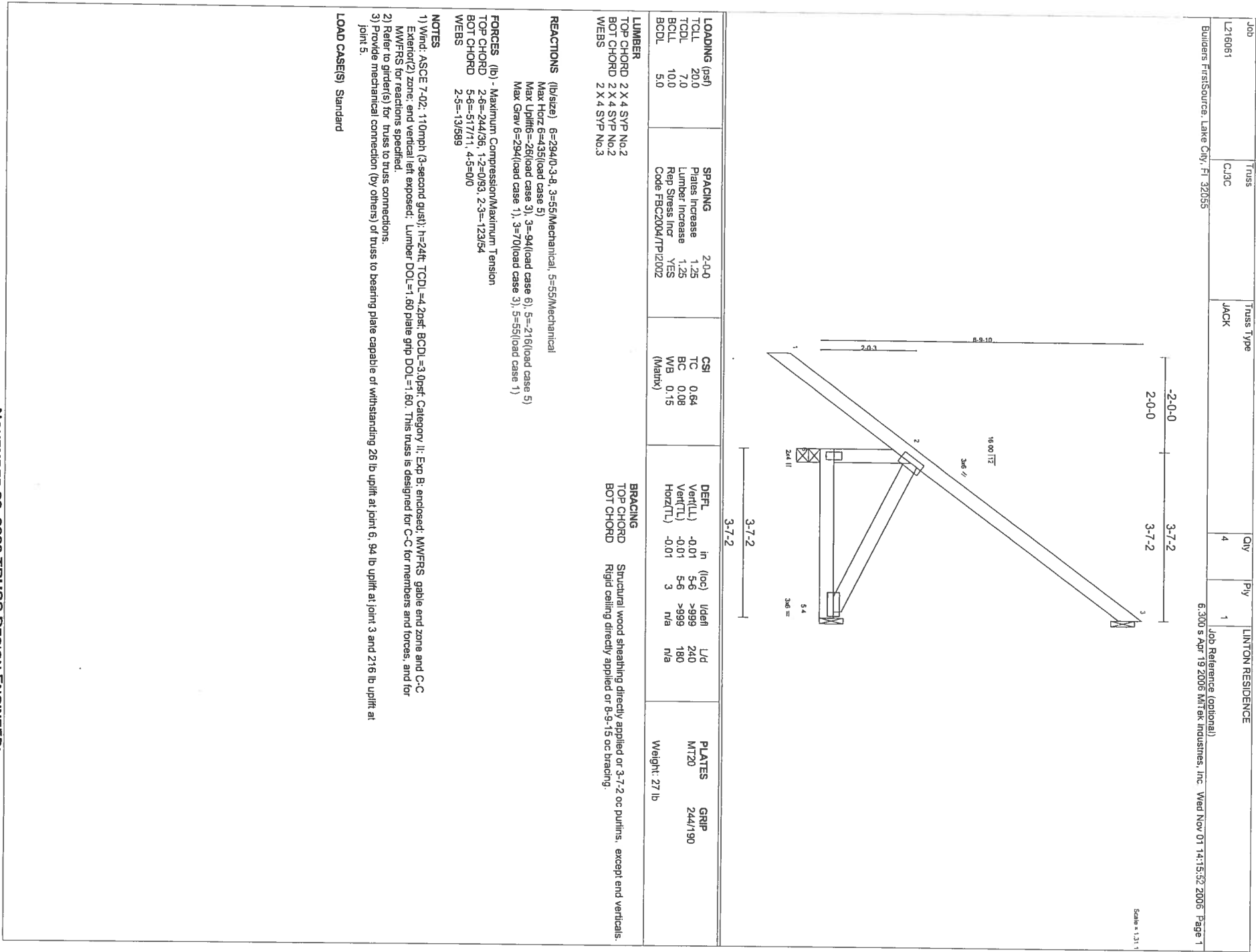
NOVEMBER 02, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 609877
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

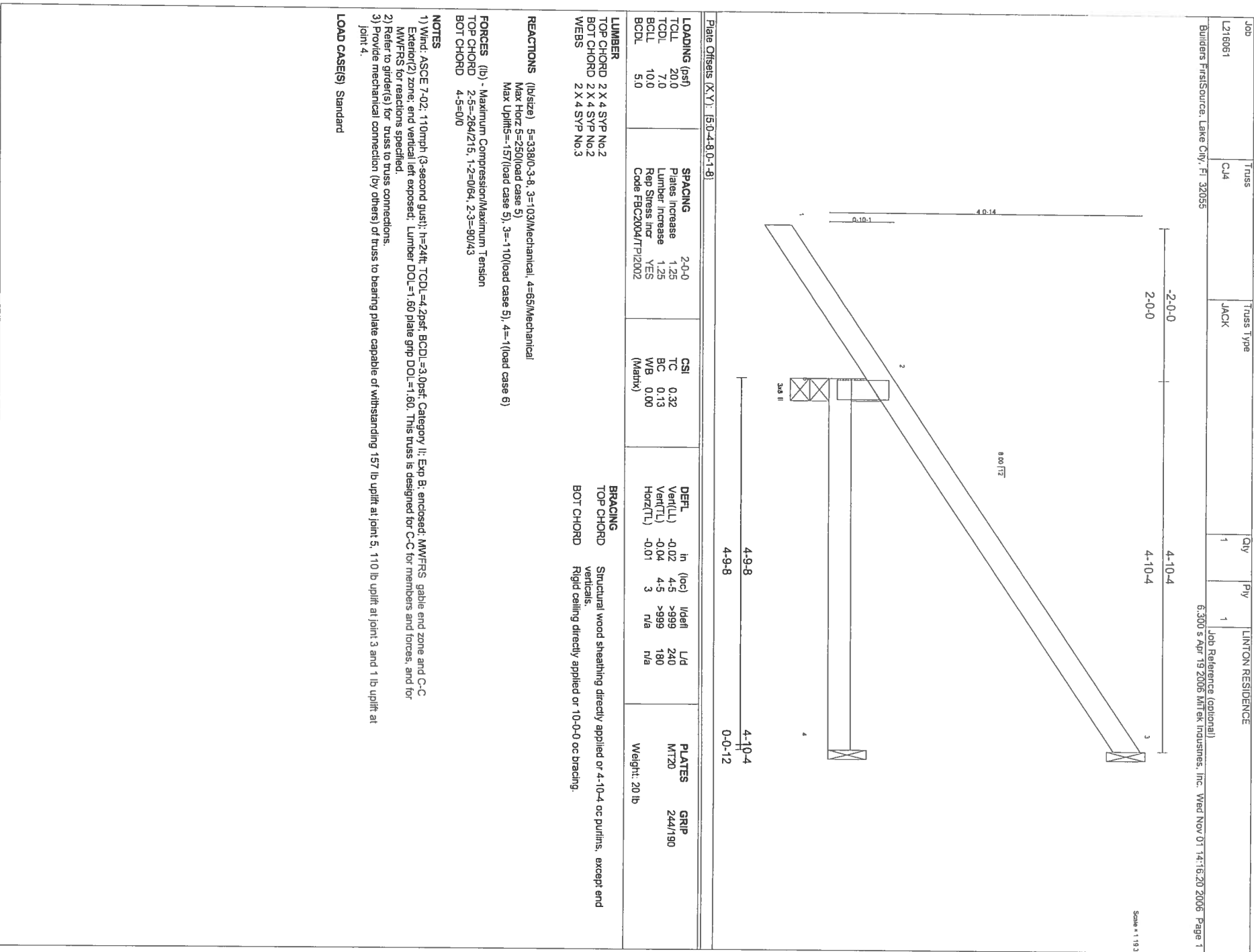






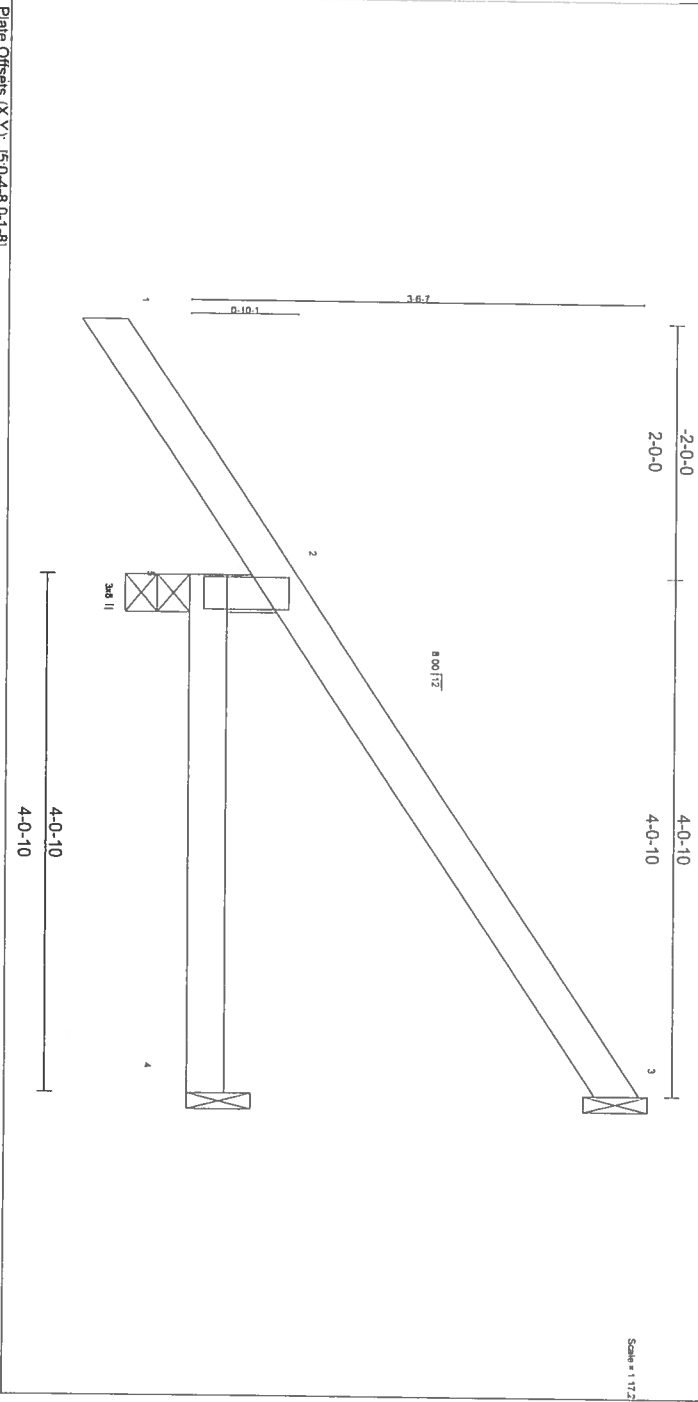






Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	CMA	JACK	1	1	Job Reference (optional)

6:30 s Apr 19 2006 Wm ak Industries, Inc Wed Nov 01 14:16:51 2006 Page 1



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
20.0	2-0-0	TC	In	M120	244/190
7.0	Plates Increase	BC	Vert(LL)		
10.0	Lumber Increase	WB	Vert(TL)		
5.0	Rep Stress Incr		Horz(TL)		
	Code FBC2004/P12002	(Mattn)		Weight: 18 lb	

TOP CHORD	TOP CHORD
2 X 4 SYP No.2	Structural wood sheathing directly applied or 4'-0-10" oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10'-0-0" oc bracing.
2 X 4 SYP No.3	
WEBS	

REACTIONS
(lbs/size) 5=3100-3-8, 3=79/Mechanical, 4=50/Mechanical
Max Horiz 5=225/(load case 5), 4=1/(load case 6)
Max Uplift=158/(load case 5), 3=86/(load case 5), 4=1/(load case 6)

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 2-5=244/208, 1-2=0/64, 2-3=71/33
 BOT CHORD 4-5=0/0

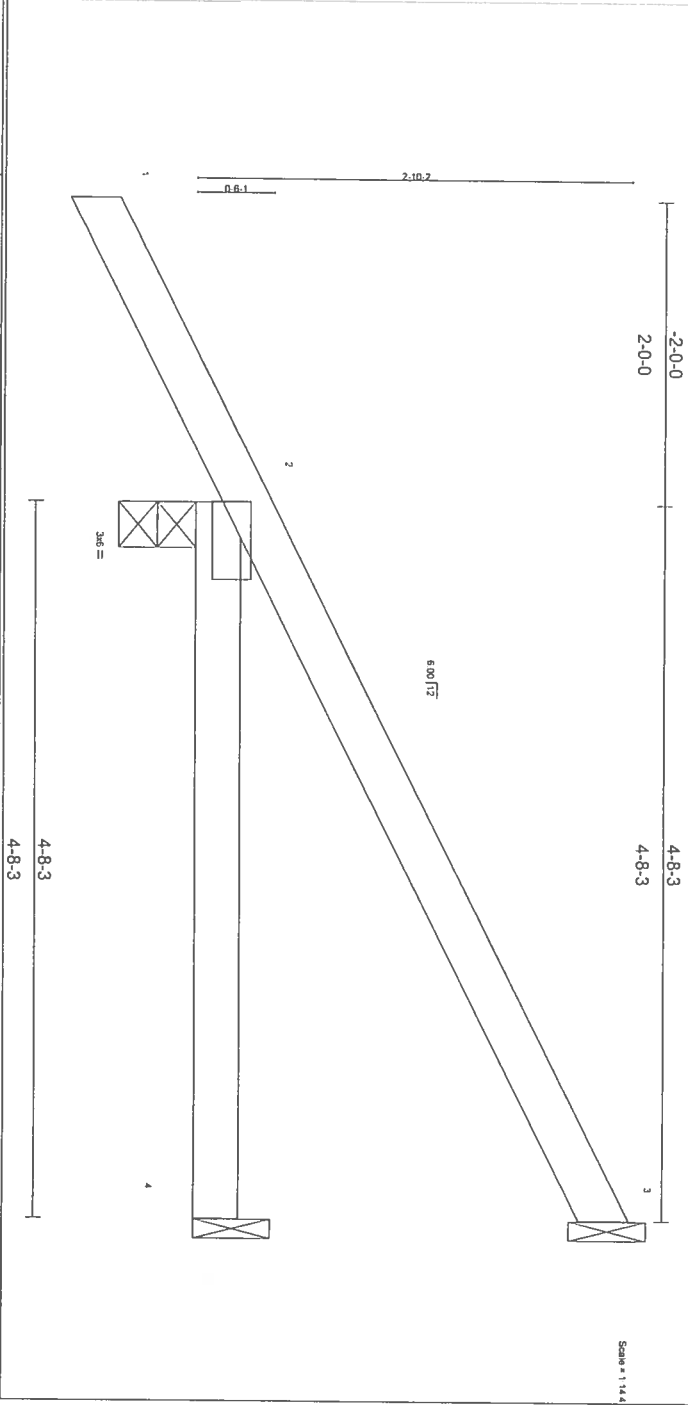
Notes:
(1) Wind: ASCE 7-02, 110 mph (3-second gust); $h=24$ ft, $TCDF=4.2$ psf, Category II; Exp B; enclosed; MWFRS, gable end zone and C-C Extension(2) zone; end vertical left exposed; Lumber $DOL=1.60$ plate grip $DOL=1.60$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
(2) Refer to girders for truss to truss connections.
(3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 5, 86 lb uplift at joint 3 and 1 lb uplift at joint 4.

LOAD CASE(S) Standard

NOVEMBER 02, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss Type	Qty	Ply	
L216061	CJ4B	JACK	1	LINTON RESIDENCE
				Job Reference (optional)

6.300 s Apr 19 2006 Mtek Industries, Inc Wed Nov 01 14:17:16 2006 Page 1



LUMBER		SPACING		CSI		DEFL.		PLATES		GRIP	
TOP CHORD	2 X 4 SYP No.2	2-0-0	Plates increase	TC	0.28	In	I/defl	L/d	M20	244/190	
1ST CHORD	2 X 4 SYP No.2	1-25	Lumber increase	BC	0.14	Vert(LL)	>.999	240			
		1.25	Rep Stress Incr	WB	0.00	Vert(TL)	2-4	180			
		YES	Code FBC2004/TP2002	Matrx		Horz(TL)	3	n/a			
Weight: 18 lb											

REACTIONS
(lb/size) 3=93/Mechanical, 2=332/0-3-8, 4=677/Mechanical
Max Horz 2=167(load case 5)
Max Up lift 3=83(load case 5), 2=193(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/41, 2-3=-88/34
 BOT CHORD 2-4=0/0

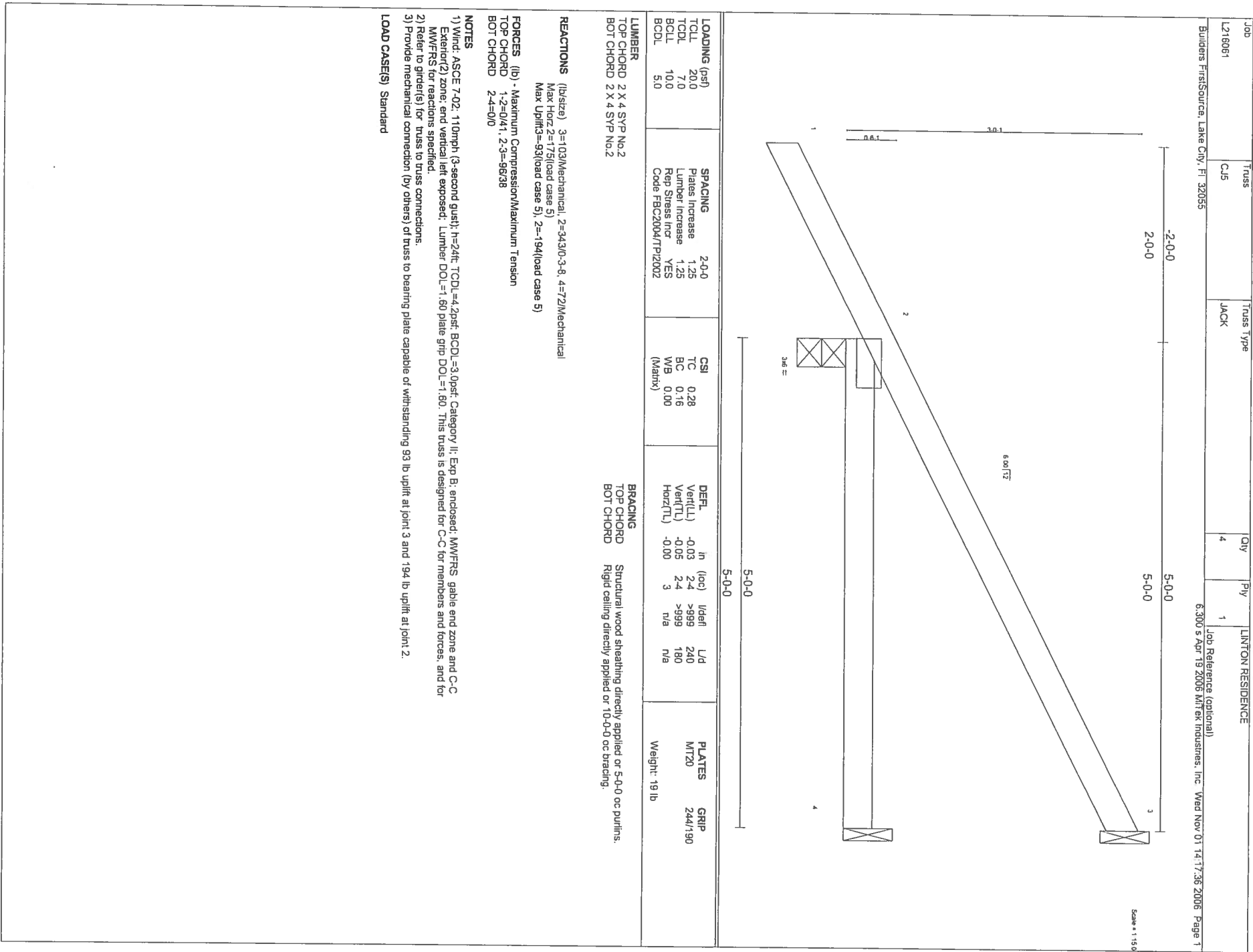
NOTES

2. Refer to girder(s) for truss to bearing plate connections.

3. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at Joint 3 and 193 lb uplift at Joint 2.

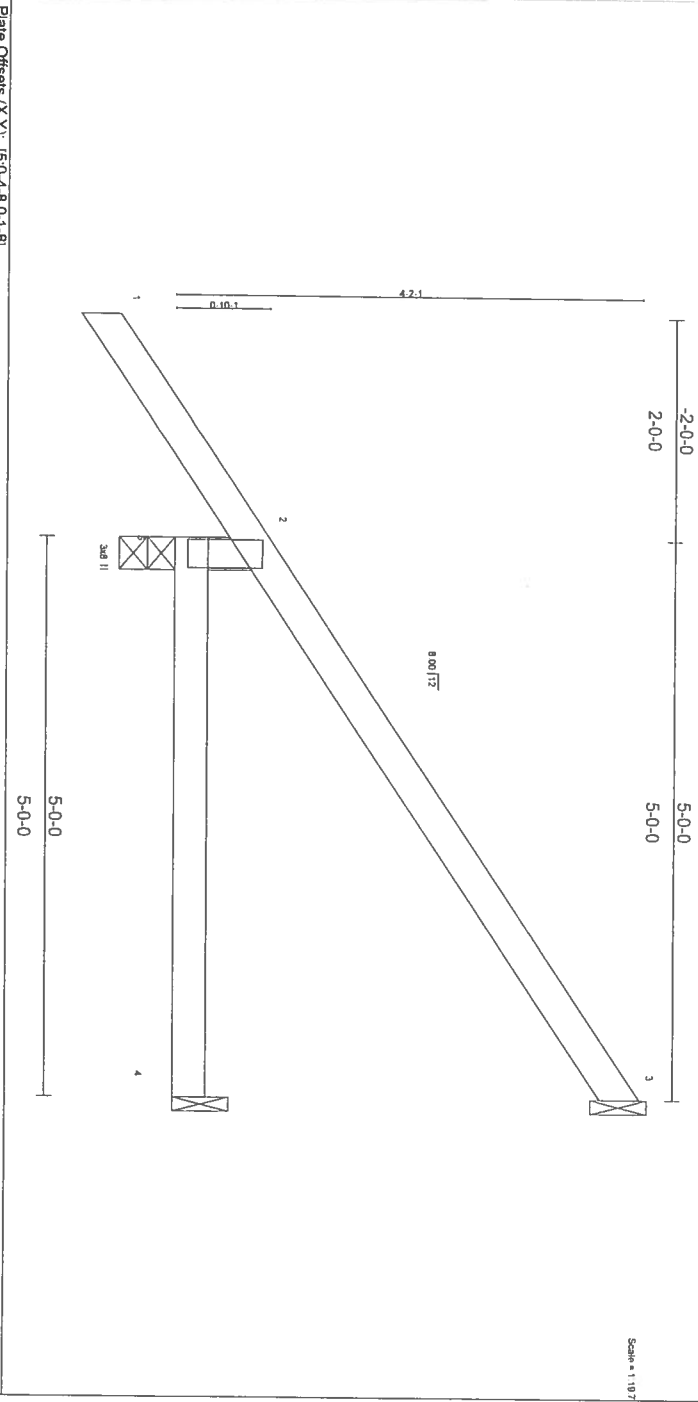
4. Wind: ASCE -02; Vertical load (second gust): $w=24$ ft; TDD = 4 psf; BCDL = 3 psf; Category II; Exp B, enclosed; MWFRS, gable end zone and C-C Eave/Drip Zone, and 170 lb horizontal exposed; Lumber DOCL=1.60 plate gyp DOCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	CJ5A	JACK	2	1	Job Reference (optional)

6.300 s Apr 19 2006 Wmtek Industries, Inc Wed Nov 01 14:16:13 2006 Page 1



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRP
TCLL 20.0	Plates increase 1.25	TC 0.32	in (loc)	M120	244/190
TCDL 7.0	Lumber increase 1.25	BC 0.15	Vert(LL) -0.02		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Vert(TL) -0.04		
BCDL 5.0	Code FBC2004/TP12002	(Matrx)	Horz(TL) -0.02		
			L/d 240		
			n/a		
			n/a		
				Weight 21 lb	

TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 5-0-0 cc purfins, except end verticals.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 cc bracing.
MEERS	2 X 4 SYP No.3		

REACTIONS (lb/size) 5=343/0-3-8, 3=107/Mechanical, 4=68/Mechanical

Max Upift5=-157(load case 5), 3=-115(load case 5), 4=-1(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

30T CHORD 4-5=0/0

NOTES

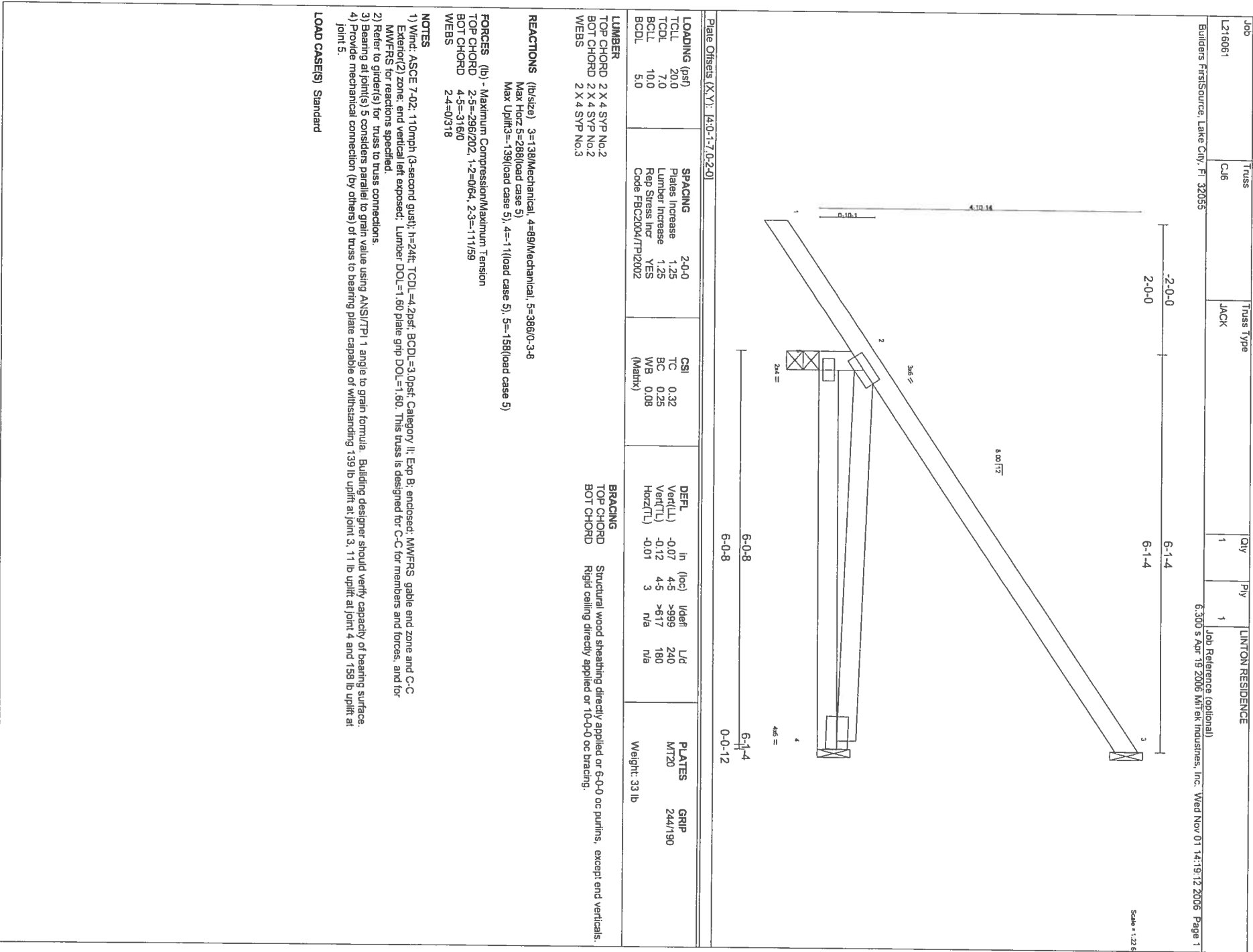
Wind: ASCE 7-02, 110 mph (3-second gust), $h=24$ ft. TOI = 4.25 ft, BCOI = 3.09 ft, Category II. Exp B, enclosed; MWFRS, gable end zone and C-C Eave/roof zone; end vertical left exposed. Lumber DOI = 1.60, plate girth DOI = 1.60. This truss is designed for C-C for members and forces, and for

2) Refer to girder(s) for truss to truss connections

1) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 157 lb uplift at joint 5, 115 lb uplift at joint 3 and 1 lb uplift at joint 4.

Standard

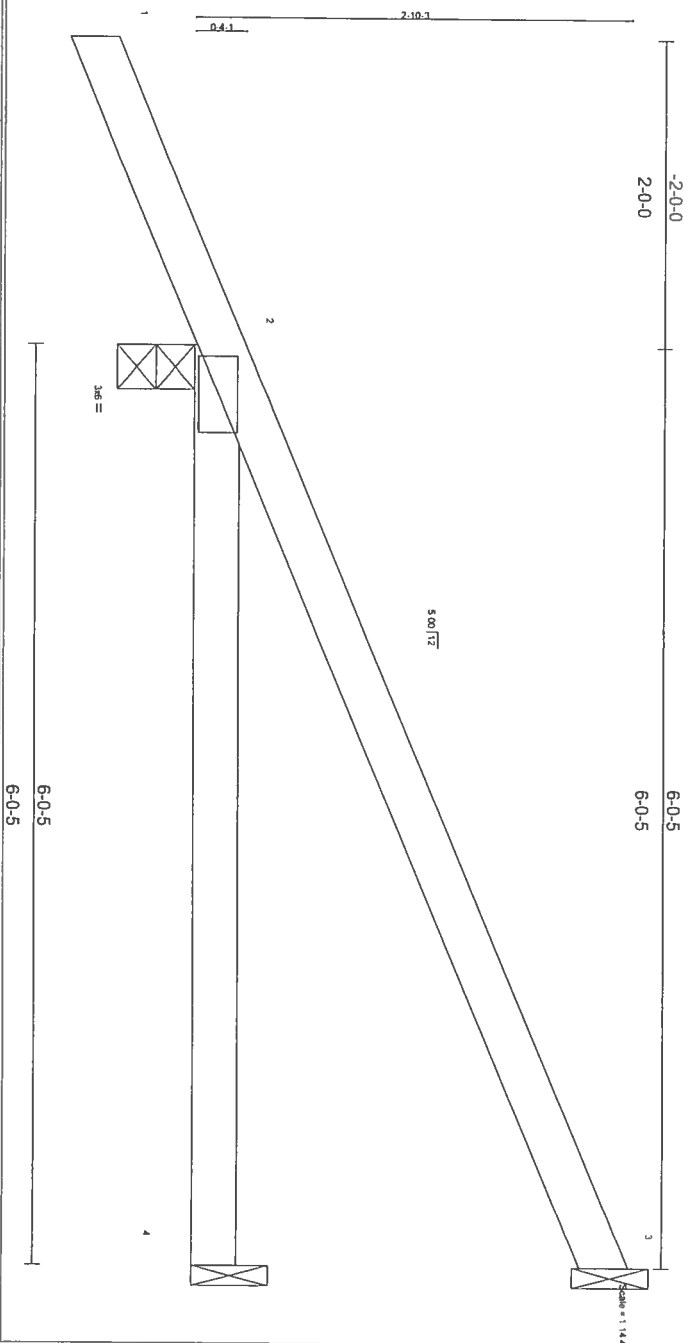
NOVEMBER 22, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
161605 N. FLORIDA AVE. STE B, LUTZ, FL 33549



Job	Truss Type	Qty	Ply	Job Reference (optional)
L216061	CJBA	1	1	LINTON RESIDENCE

Builders FirstSource, Lake City, FL 32035

6,300 s Apr 19 2006 Mfr ek industries, Inc Wed Nov 01 14:20:02 2006 Page 1



LOADING (psf)	SPACING	CSI	DEFL.	in (loc)	l/defl	L/d	PLATES	GRP
TCLL 20.0	Plates increase 1.25	TC 0.31	Vert(L)	-0.06	2.4	>999	MT20	244/190
TCLL 7.0	Lumber increase 1.25	BC 0.24	Vert(T)	-0.11	2.4	>664		
BCLL 10.0	Rep Stress incr YES	WB 0.00	Horz(TL)	-0.00	3	n/a		
BCDL 5.0	Code FBC2004/TP2002	(Manrx)				n/a	Weight: 22 lb	

TOP CHORD	2 X 4 SYP No.2	BOT CHORD	2 X 4 SYP No.2
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	2 X 4 SYP No.2
Structural wood sheathing directly applied or 6-0-0 cc purflins		Structural wood sheathing directly applied or 6-0-0 cc purflins	
Rigid ceiling directly applied or 10-0-0 cc bracing.		Rigid ceiling directly applied or 10-0-0 cc bracing.	

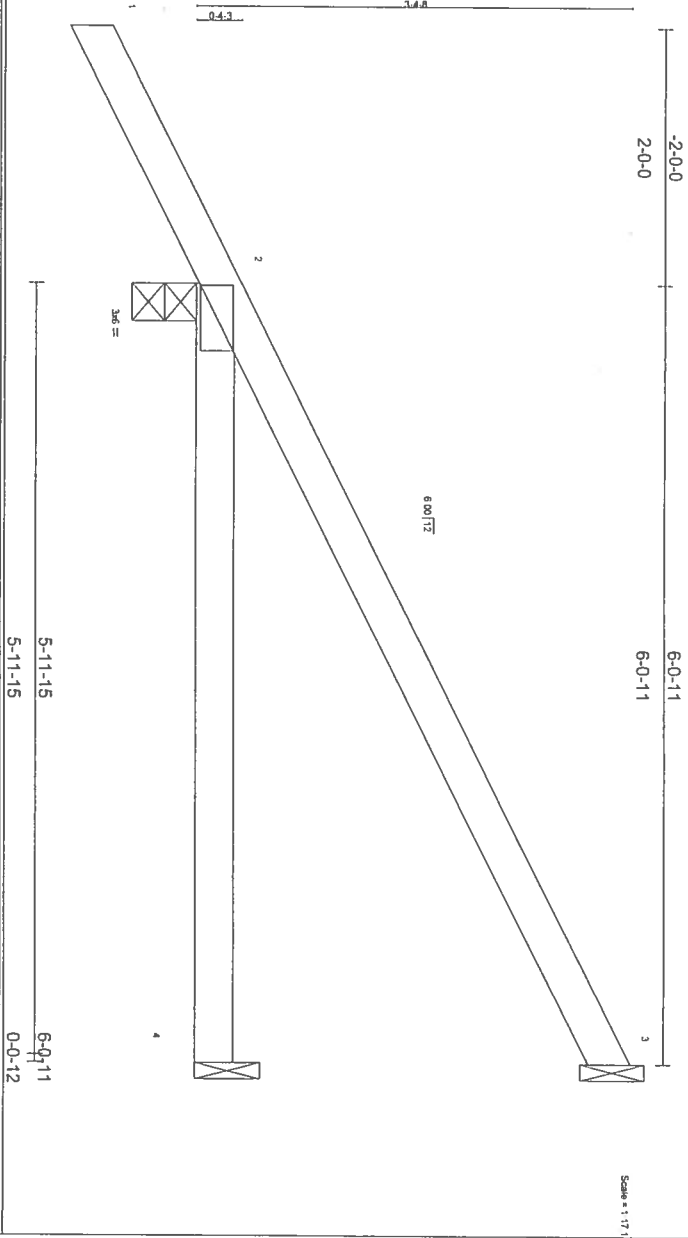
REACTIONS
(lb/size) 3=1.36/Mechanical, 2=382/0-3-8, 4=87/Mechanical
Max Horiz 2=168(load case 5)
Max Uplift3=-109(load case 5), 2=-213(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/40, 2-3=90/41
BOT CHORD	2-4=0/0

NOTES
1) Wind: ASCE 7-02; 1.0mph (3-second gust); $h=24ft$; $TCDL=4.2psf$; $BCDL=3.0psf$; Category II; Exp. B; enclosed; MWFRS, gable end zone and C-C Extension(2) zone; and vertical lift exposed. Lumber DOL=1.60; Plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girders for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 109 lb uplift at joint 3 and 213 lb uplift at joint 2.
LOAD CASE(S) Standard

NOVEMBER 02, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
6105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	CJABR	JACK	4	1	
Buildings FirstSource, Lake City, FL 32055					6,300 s Apr '19 2006 Mfrtek Industries, Inc. Wed Nov 01 14:20:39 2006 Page 1



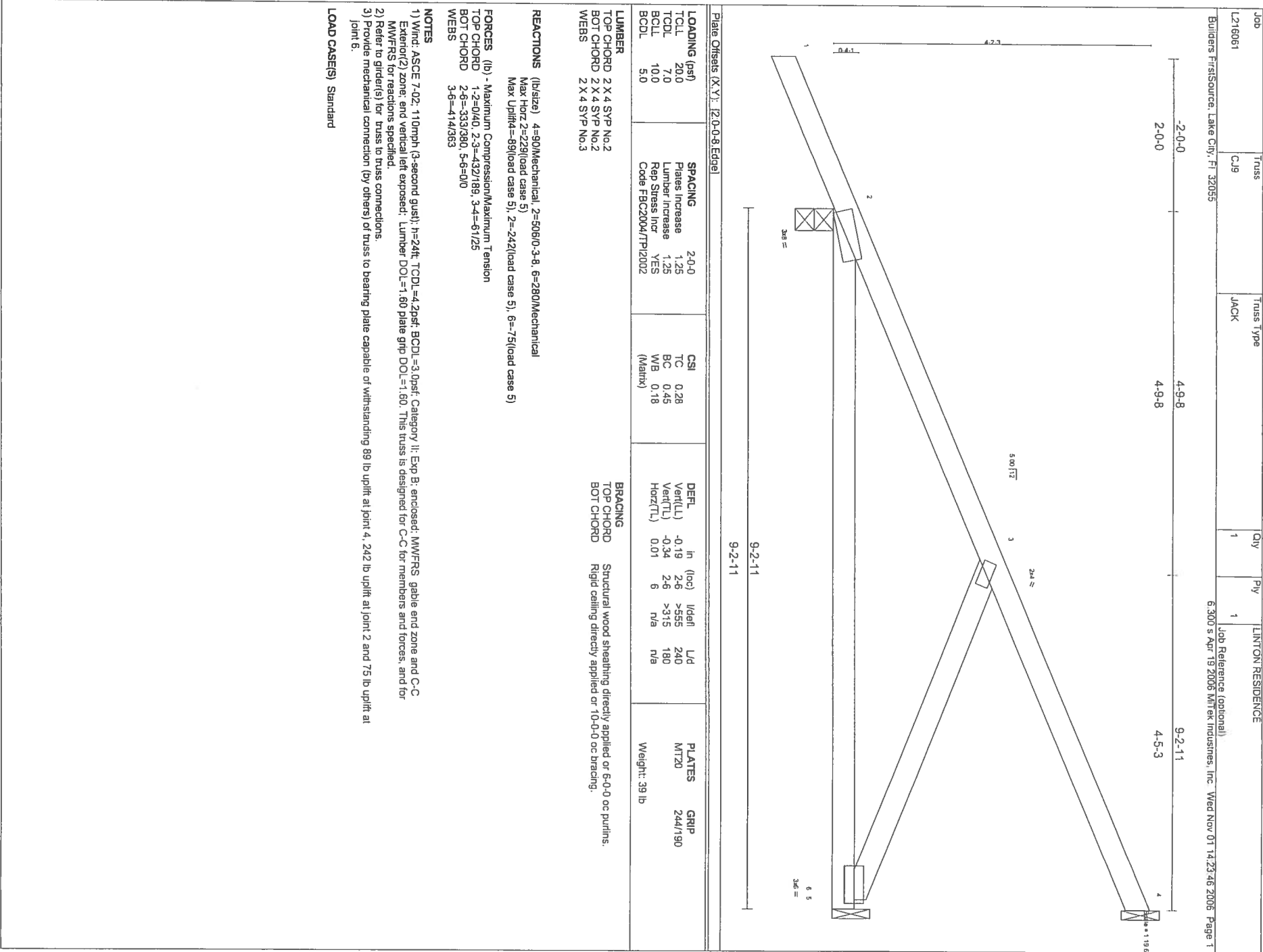
LOADING (psf)	SPACING	CSI	DEFL	PLATES
TCLL 20.0	Plates Increase 2'-0"	TC 0.34	Ver(TL) -0.06	MT20 244/190
TCCL 7.0	Lumber Increase 1.25	BC 0.24	Ver(TL) -0.11	
BCCL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00	
BCDL 5.0	Code FBC2004/TP12002	(Matnx)		Weight: 23 lb

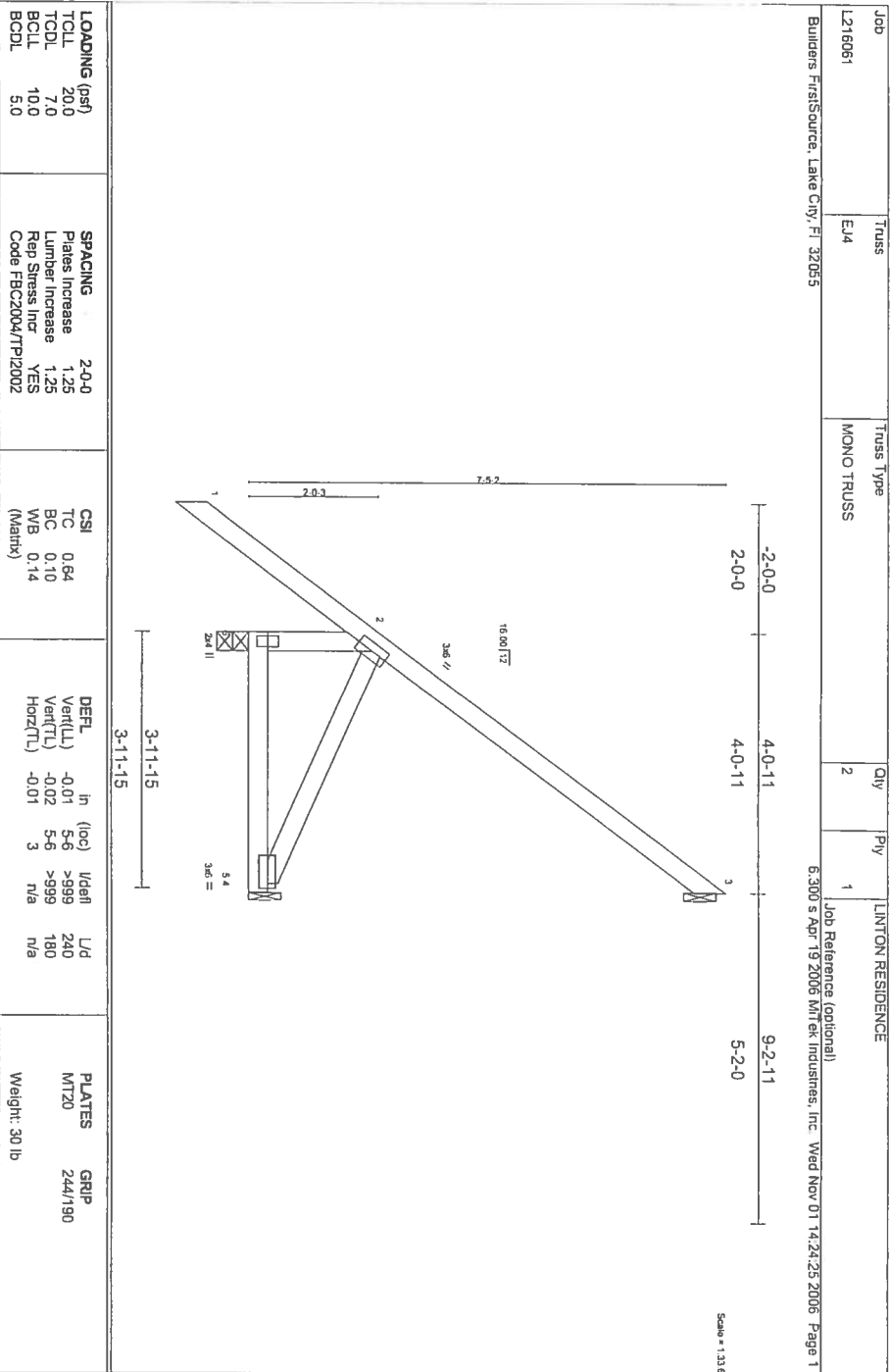
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (lb/size) 3=137(Mechanical), 2=3830-3-8, 4=88(Mechanical)
Max Horiz 2=202(load case 5)
Max Uplift3=-119(load case 5), 2=-204(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-11/148
BOT CHORD 2-4=0/0

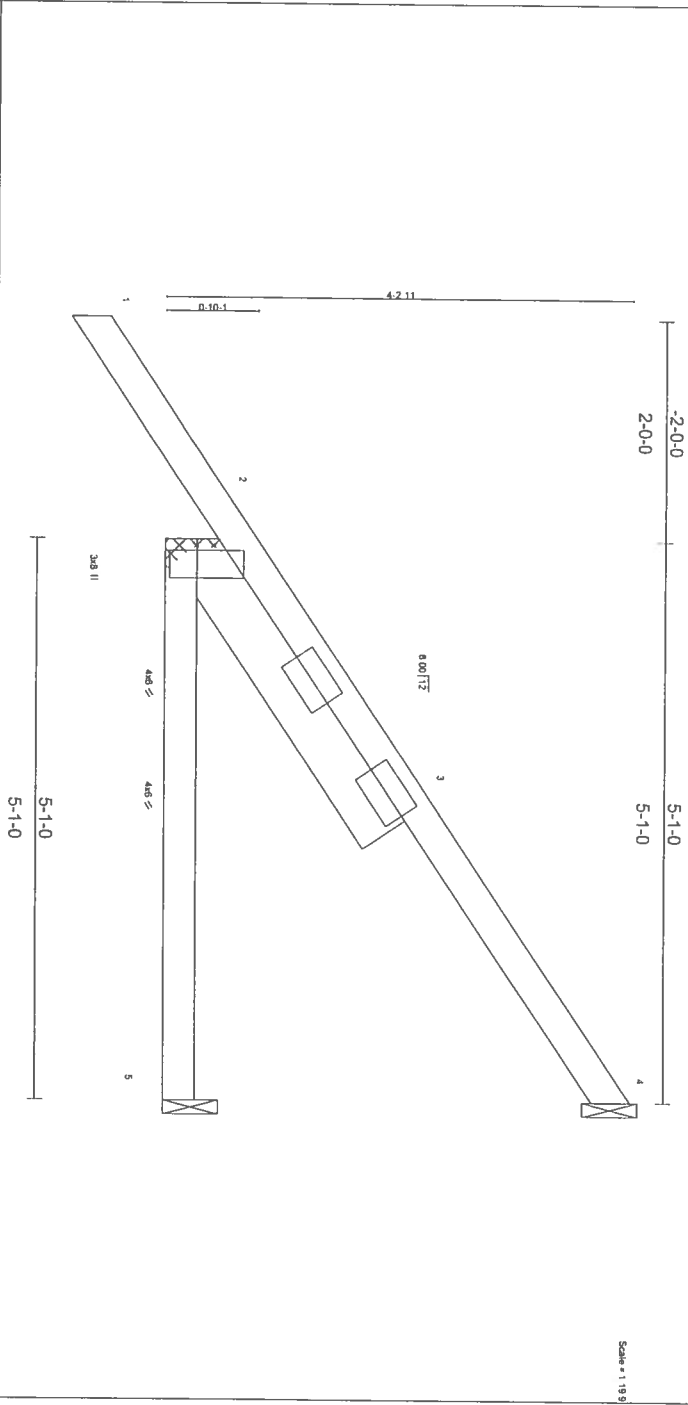
NOTES
1) Wind: ASCE 7-02: 110mph (3-second gust); h=24ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60; plate gnp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 3 and 204 lb uplift at joint 2.
LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	E15	MONO TRUSS	6	1	Job Reference (optional)

6:300 s Apr 19 2006 Mike Industries, Inc. Wed Nov 01 14:25:11 2006 Page 1



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.23	In (loc) Vert(LL) -0.03 Vert(TL) -0.06	L/D 240	244/190
TCLD 7.0	Lumber Increase 1.25	BC 0.18	2-5 >999	180	
BCLL 10.0	Rep Stress Incr YES	WB 0.00	4	n/a	
BCDL 5.0	Code FBC2004/TP/2002	(Matrix)		Weight: 28 lb	

TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 5'-10'-00 pc pultrus.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10'-0-0 oc bracing.
SLIDER	Left 2 X 6 SYP No.1D 3-1-15		

REACTIONS
(lb/size) 4=1.14/Mechanical, 2=340/Mechanical, 5=75/Mechanical
Max Horz 2=231(load case 5)
Max Uplift4=137(load case 5), 2=150(load case 5)

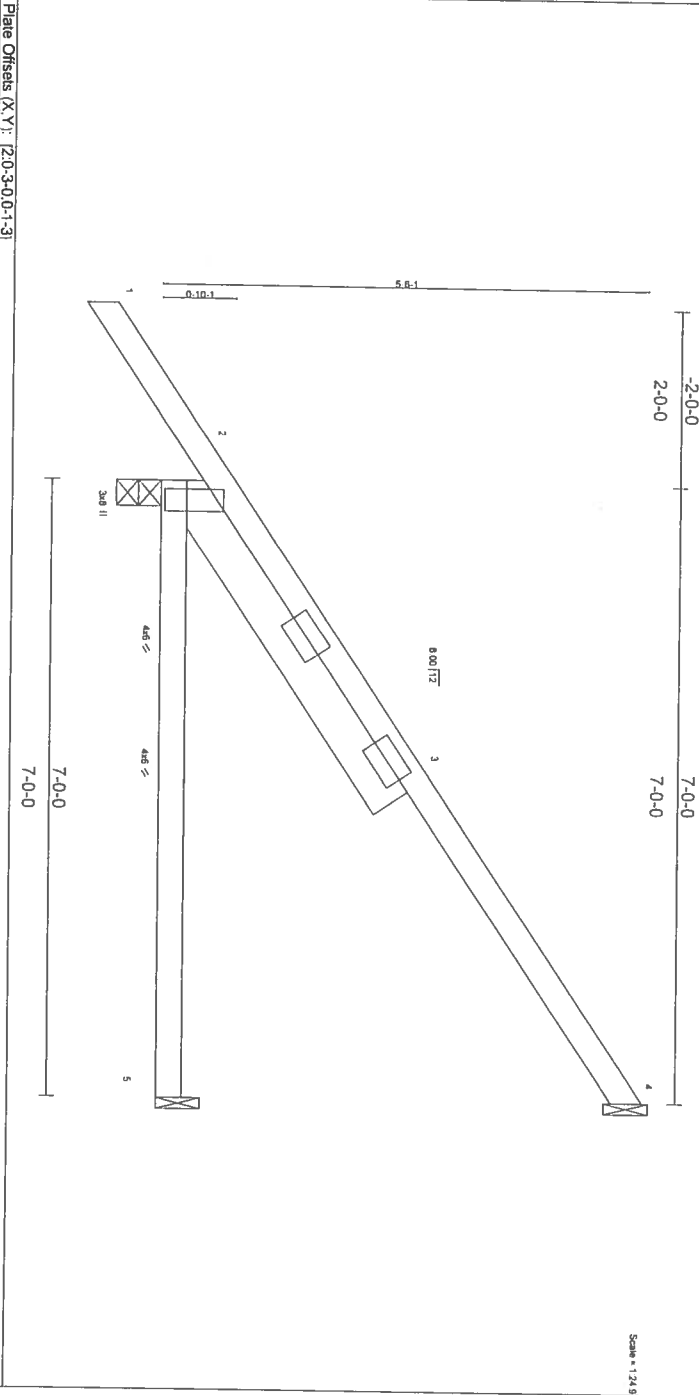
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=104/10, 3-4=87/47
BOT CHORD 2-5=0/0

NOTES
 1) Wind: ASCE 7-02, 110mph (3-second gust); $h=24\text{ft}$; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp. B, enclosed; MWFRS, gable end zone and C-C interior (1) zone, end vertical (left) exposed. Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

LOAD CASE(S) Standard

THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60988
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	E17	MONO TRUSS	24	1	
					Job Reference (optional)
					6 300 S Apr 19 2006 KDKK Inc Wood New York 44 75 25 0000 5



LOADING (psi)	SPACING	CSI	DEFL.	in (top)	l/def	L/d	PLATES	GRIP
TCL 20.0	Plates increase 1.25	TC 0.53	Vert(L)	-0.11	2.5	>771	240	M720
7.0	Lumber increase 1.25	BC 0.30	Vert(TL)	-0.18	2.5	>465	180	244/190
10.0	Rep Stress incr YES	WB 0.00	Horz(TL)	0.03	4	n/a	n/a	
5.0	Code FBC2004/TP2002	(Matrix)						Weight: 37 lb

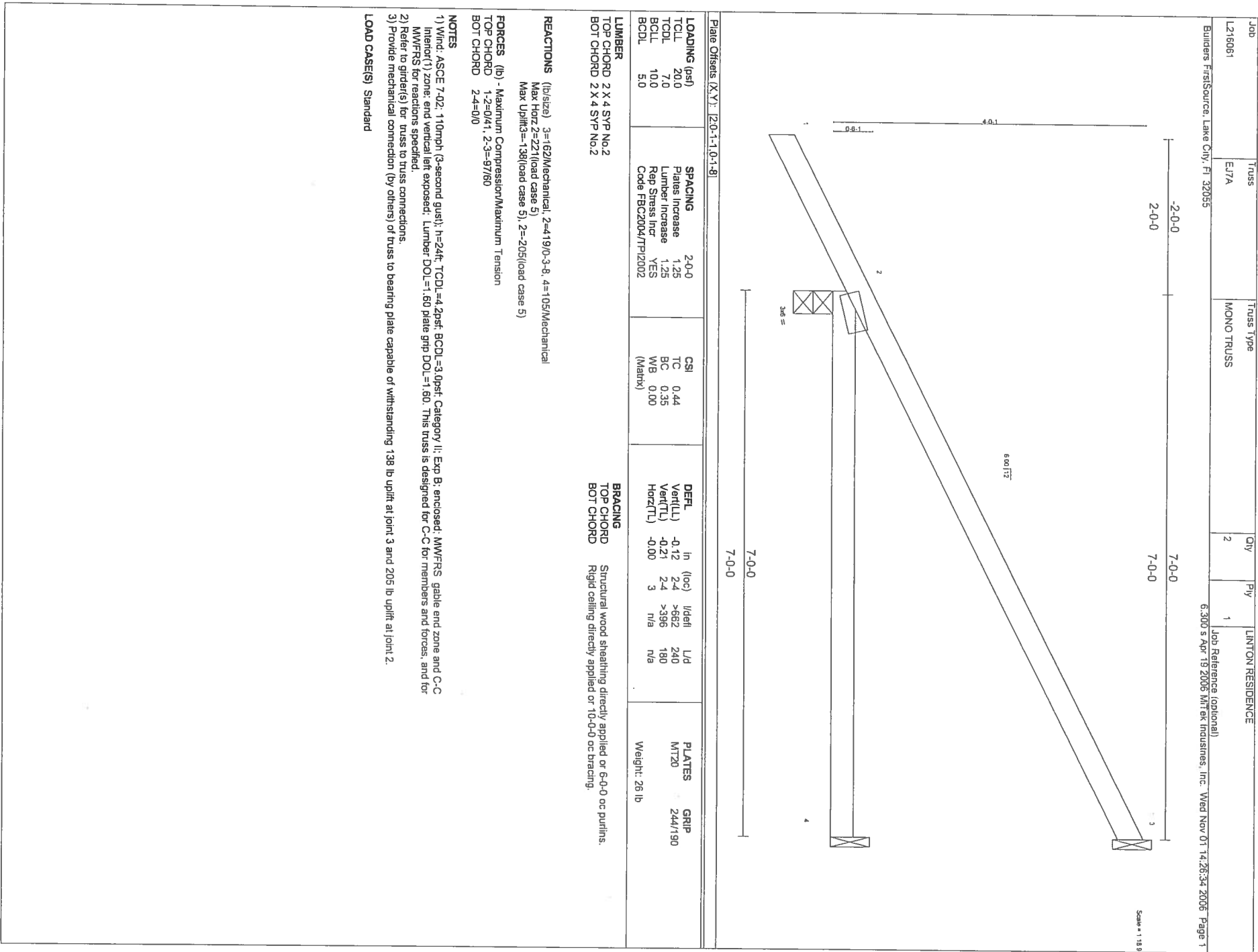
TOP CHORD	BRACING
2 X 4 SYP No. 2	Structural wood sheathing directly applied or 6-0-0 oc putins.
NOT CHORD	
2 X 4 SYP No. 2	
Left 2 X 6 SYP No. 10 4-3-12	Rigid ceiling directly applied or 18-0-0 oc bracing.
SLIDER	

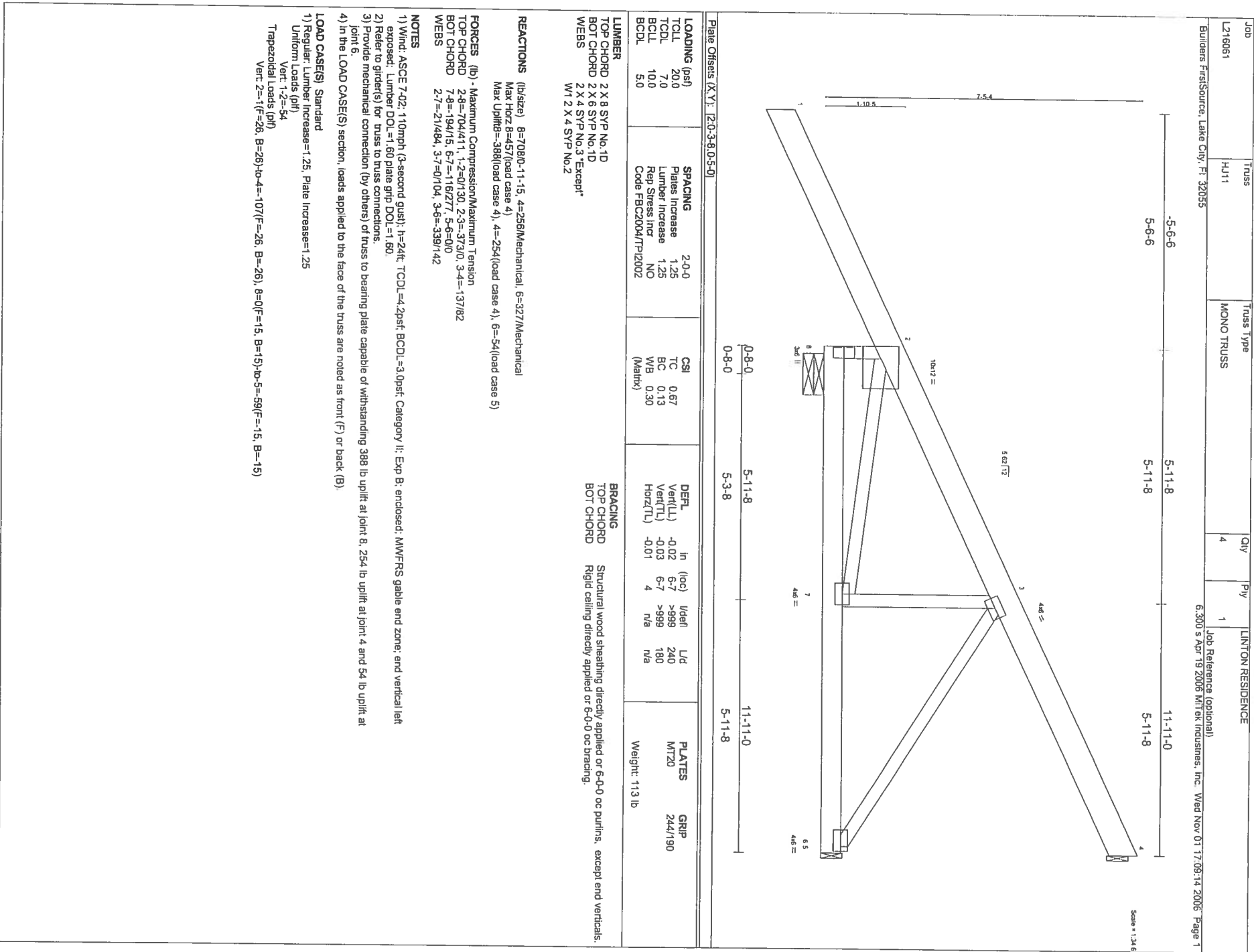
REACTIONS
(b) (size) 4=178/Mechanical 2=4=150/3-8, 5=98/Mechanical
Max Horz 2=290/(load case 5)
Max Uprift=178/(load case 5), 2=154/(load case 5), 5=1/(load case 5)

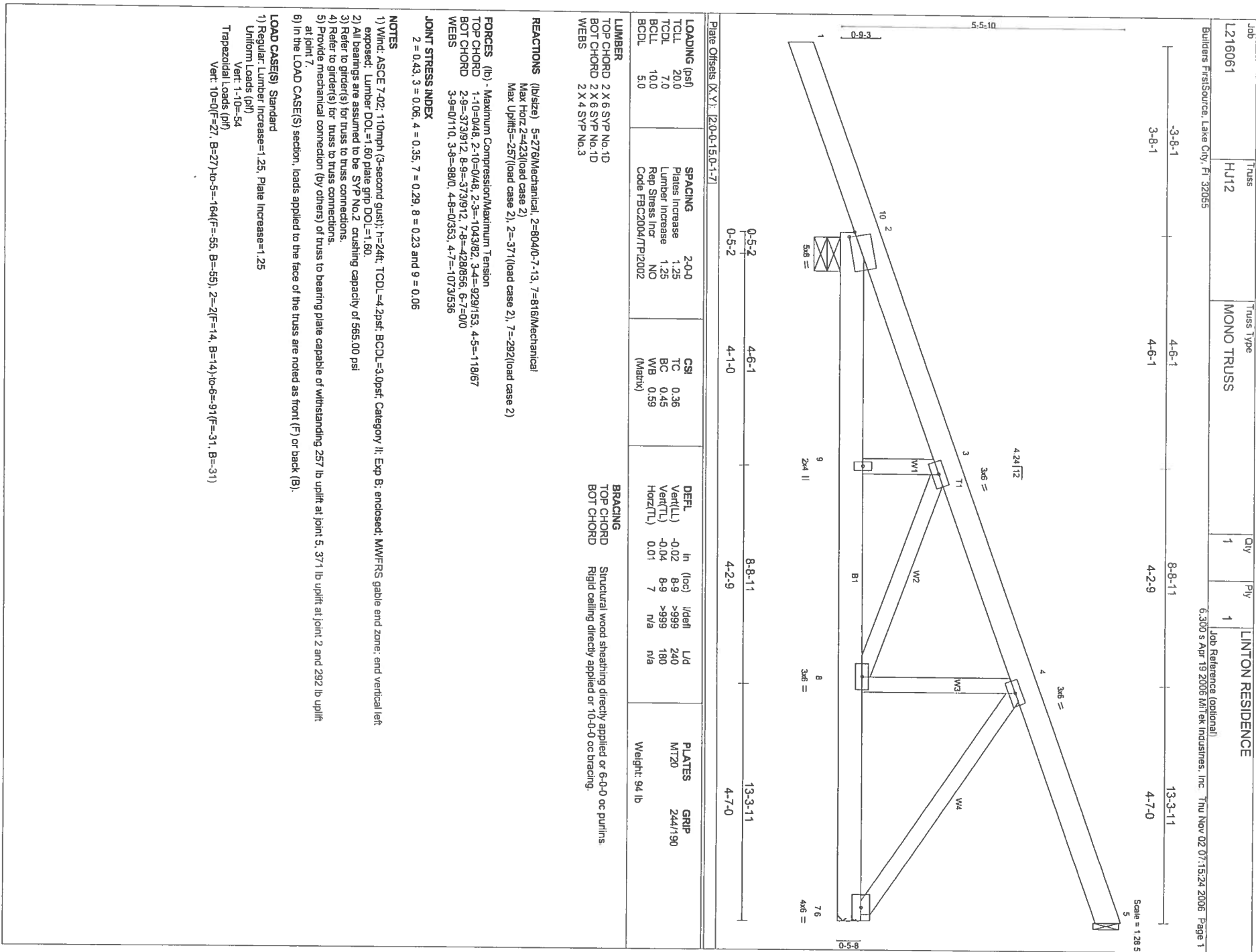
(b) - maximum compression
 OP CHORD 1-2=0/41, 2-3=-130/12, 3-4=-113/77
 LOT CHORD 2-5=0/0

NOTES
 Wind: ASCE 7-02, 110mph (3-second gust); $h=24ft$; $TCOL=4ft$; $BCOL=3ft$; Category II; Exp B; enclosed; MWFRS: gable end zone and C-C Interior (1) zone; and vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 Refer to girders(s) for truss to truss connections.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 4, 154 lb uplift at joint 2 and 1 lb uplift at joint 5.

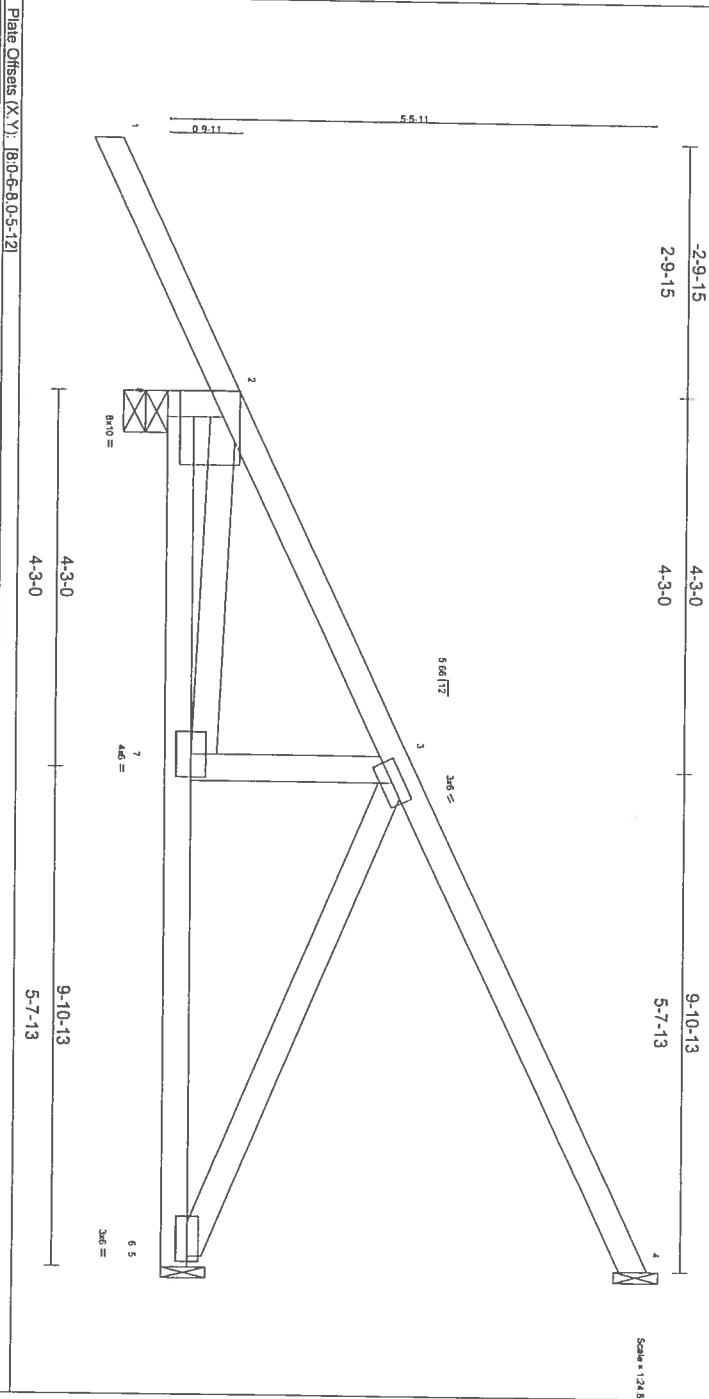
LOAD CASE(S) Standard







Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	HJ7	JACK	1	1	Job Reference (optional) 6,300 S. Ave. 19 2006 Mikael Properties, Inc. Mod. Nov. 11 15:03:35 9006 Pines



LOADING (psi)	SPACING	CSI	DEFL.	in	(loc)	ldefl	Ld	PLATES	GRP
TCLL 20.0	Plates increase 2-0.0	TC 0.95	Vert(L)	-0.06	6-7	>999	240	M120	244/190
TCDL 7.0	Lumber increase 1.25	BC 0.41	Vert(TL)	-0.11	6-7	>999	180		
BCLL 10.0	Rep Stress Incr NO	WB 0.39	Horz(TL)	-0.01	4	n/a	n/a		
BCDL 5.0	Code FBC2004/TP2002	(Matrx)						Weight: 54 lb	

LUMBER		BRACING	
TOP CHORD	BOT CHORD	TOP CHORD	BOT CHORD
2 X 4 SYP No.2	2 X 4 SYP No.2	Structural wood sheathing directly applied or 6-0-0 oc purlins, except: end verticals.	
2 X 4 SYP No.3	2 X 4 SYP No.3	Rigid ceiling directly applied or 6-0-0 oc bracing	

REACTIONS
(lb/size) 8=524/0-5-1, 4=263/Mechanical, 6=396/Mechanical
Max Horz=380(load case 4)
Max Uplift=211(load case 4), 4=264(load case 4), 6=103(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-8=527/205, 1-2=0/68, 2-3=582/12, 3-4=148/61
BOT CHORD 7-8=358/161, 6-7=293/529, 5-6=0/0
WEBS 3-7=0/113, 3-6=581/322, 2-7=458/895

NOTES

- 1) **WIND:** ASCE - V-2; 11 mph (3-second gust); $w_z=24$ ft; TCDL = 4.2 psf; BCDL = 3.0 psf; Category II; Exp B; enclosed; MWFRS gable end and zone; end vertical left exposed; Lumber DOL = 1.60 plate gfr DOL = 1.60.
- 2) Refer to girders(s) for truss to truss connections.
- 3) Bearing at joint(s) 8 considers parallel to grain value using ANS/APP 1 angle in grain formula. Building designer should verify capacity of bearing surface.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 211 to uplift at joint 8, 264 to uplift at joint 4 and 103 to uplift at joint 6.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber increase=1.25, Plate increase=1.25

Vert. 1-2=-5

Trapezoidal Loads (plf)

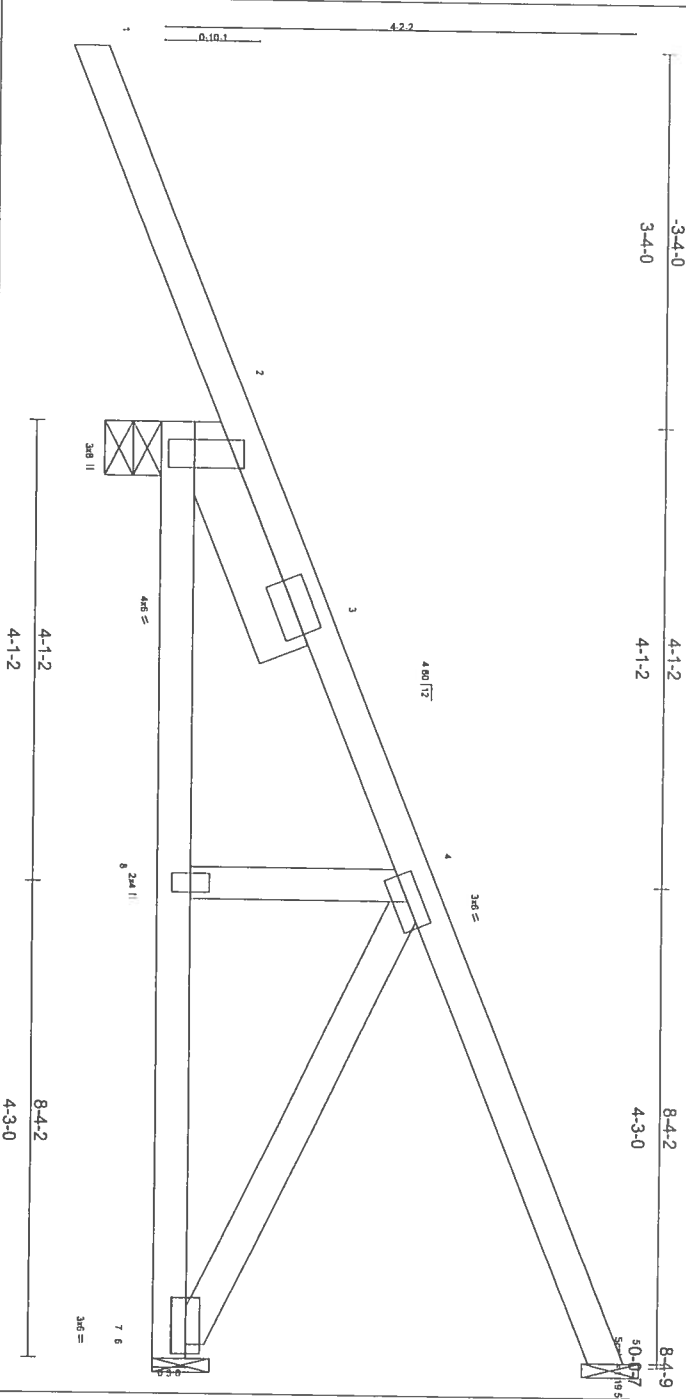
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Verf Z=-2(F=26, B=26)-10-4=-134(F=-40, B=-40), B=0(F=15, B=15)-10-5=-74(F=-22, B=-22)
```

10

Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	HJB	MONO TRUSS	1	1	Job Reference (optional)

6:300 s Act 19 2006 Mitek Industries Inc. Word Nov 01 15:06:16 2006 5000 1

Builder's FirstSource, Lake City, Fl 32055



LOADING (psf)	SPACING	CSI	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.63	in (top) l/d	M120	244/190
TCDL 7.0	Plates increase 1.25	BC 0.25	Ver(T/L) -0.03		
BCLL 10.0	Lumber increase 1.25	WB 0.14	Ver(T/L) -0.04		
BCDL 5.0	Rep Stress Incr NO		Horz(T/L) -0.01		
	Code FBC2004/TP2002	(Main)			
				Weight: 46 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2 X 4 SYP No.3		
SLIDER	Left 2 X 6 SYP No.1D 2-2-9		

REACTIONS (lb/size) 5=184(Mechanical, 2-4=56/0-5-13, 7=272)Mechanical
Max Horz 2-2=15(load case 4)
Max Uplift=+168(load case 4), 2-=250(load case 4), 7-=47(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/42, 2-3=389/0, 3-4=365/0, 4-5=83/51
 BOT CHORD 2-8=116/307, 7-8=116/307, 6-7=0/0
 WEBS 4-8=0/118, 4-7=348/131

NOTES

- 1) Wind: ASCE 7-02, 110mph (3-second gust); $h=24$ ft; TCDF = 4; 2psf. Category II; Exp B, enclosed; MMFRS gable end zone; end vertical left exposed. Lumber: DOI = 60 Plate girth BOL-1-60.
- 2) Refer to girders for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 5, 250 lb uplift at joint 2 and 47 lb uplift at joint 7.
- 4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular. Lumber Increase=1.25, Plate Increase=1.25

Trapezoidal Loads (psi)
Vert. 2=0(F=27, B=27)-to-5=-113(F=-30, B=-30), 2=0(F=15, B=15)-to-6=-63(F=-16, B=-16)

NOVEMBER 02, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	101	MONO HIP	1	2	
Builder: FirstSource, Lake City, FL 32055					6:30:05 s Apr 19 2006 Mill Creek Industries, Inc. Wed Nov 01 16:34:19 2006 Page 1
Job Reference (optional)					
2-0-0					42-0-0
6-8-11					29-7-3
5-8-2					35-10-3
5-10-5					6-1-13
5-5-11					
6-3-0					
6-1-13					
2-0-0					
6-8-11					
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5-10-5					
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Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	T02	HIP	1	1	
Builder: FirstSource, Lake City, FL 32055					6,300 S Apr 19 2006 Mitek Industries, Inc. Wed Nov 07 16:16:34 2006 Page 1

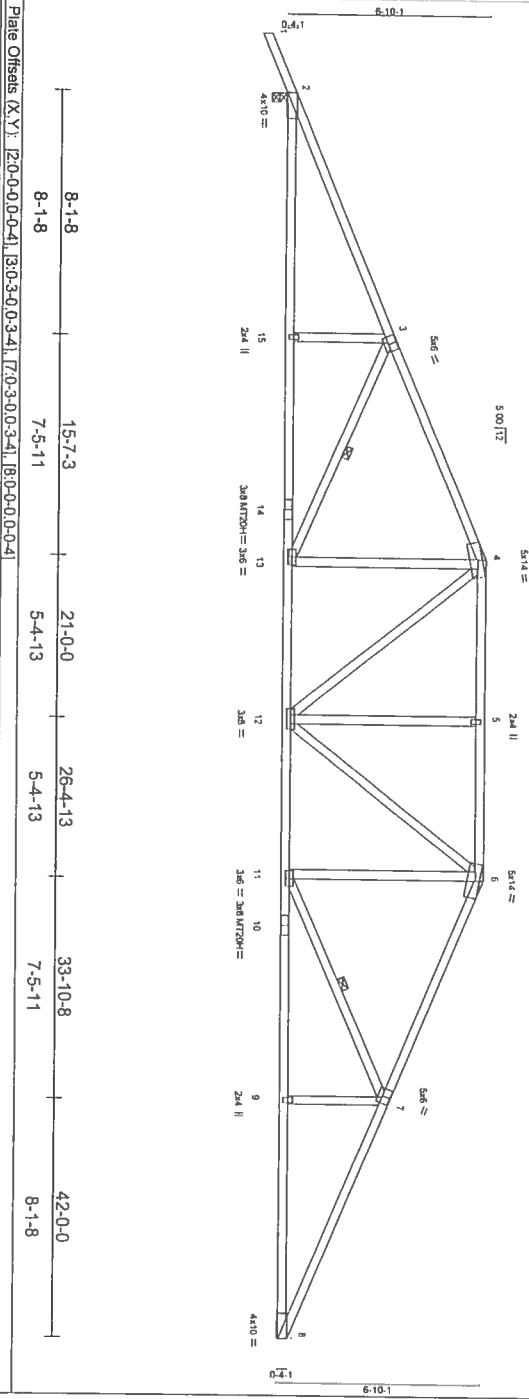


Plate Offsets (X,Y): [20-0-0,0-0-4], [30-3-0,0-3-4], [70-3-0,0-3-4], [80-0-0,0-0-4]									
LOADING (psf)		SPACING		CSI		DEF.		PLATES	
TCL	20.0	Plates Increase	2-0-0	TC	0.87	Vert(1)	-0.37	MT20	GRIP
TCCL	7.0	Lumber Increase	1.25	BC	0.97	Vert(1)	-0.60	MT20	244/190
BCL	10.0	Rep Stress Incr	YES	WB	0.32	Horz(1)	0.22	MT20H	187/143
BCDL	5.0	Code FBC2004/FP12002		(Manly)					Weight: 217 lb

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	2 X 4 SYP No.3	WEBS	1 Row at midpt: 3-13, 7-11

REACTIONS (lb/size) 2=1874/0-3-8, 8=1752/Mechanical
Max Horiz 2=143(load case 5)
Max Uplift2=663(load case 5), 8=539(load case 6)

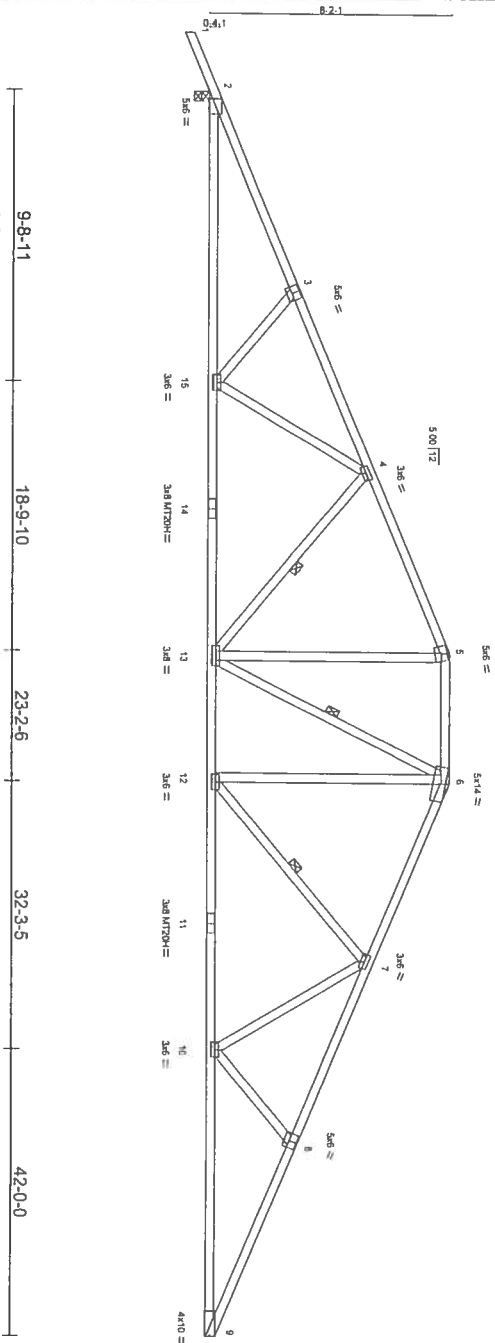
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-3795/1087, 3-4=-2942/888, 4-5=-2813/908, 5-6=-2813/908, 6-7=-2952/878, 7-8=-3852/1140
BOT CHORD 2-15=-1001/3418, 14-15=-1001/3418, 13-14=-1001/3418, 12-13=-8422/851, 11-12=-6332/860, 10-11=-964/3478, 9-10=-964/3478, 8-9=-964/3478
WEBS 3-15=0/264, 3-13=-861/398, 4-13=-1235/49, 4-12=-119/431, 5-12=-277/187, 6-12=-112/419, 6-11=-137/567, 7-11=-917/1445, 7-9=0/280

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02, 110mph (3-second gust); h=24ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS, gable end zone and C-C interior(1) zone. Lumber DOL=1.80 plate grp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 663 lb uplift at joint 2 and 539 lb uplift at joint 8.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	UNITON RESIDENCE
L216061	T03	HIP	1	1	
Builders FirstSource, Lake City, Fl 32055					
6.300 5 Apr 19 2006 MITek Industries, Inc Wed Nov 01 16:17:15 2006 Page 1					
Job Reference (optional)					
1-2-0-0	6-8-6	12-9-0	18-9-10	23-2-6	29-3-0
2-0-0	6-8-6	6-0-10	6-0-10	4-4-13	6-0-10
					35-3-10
					42-0-0
					6-8-6

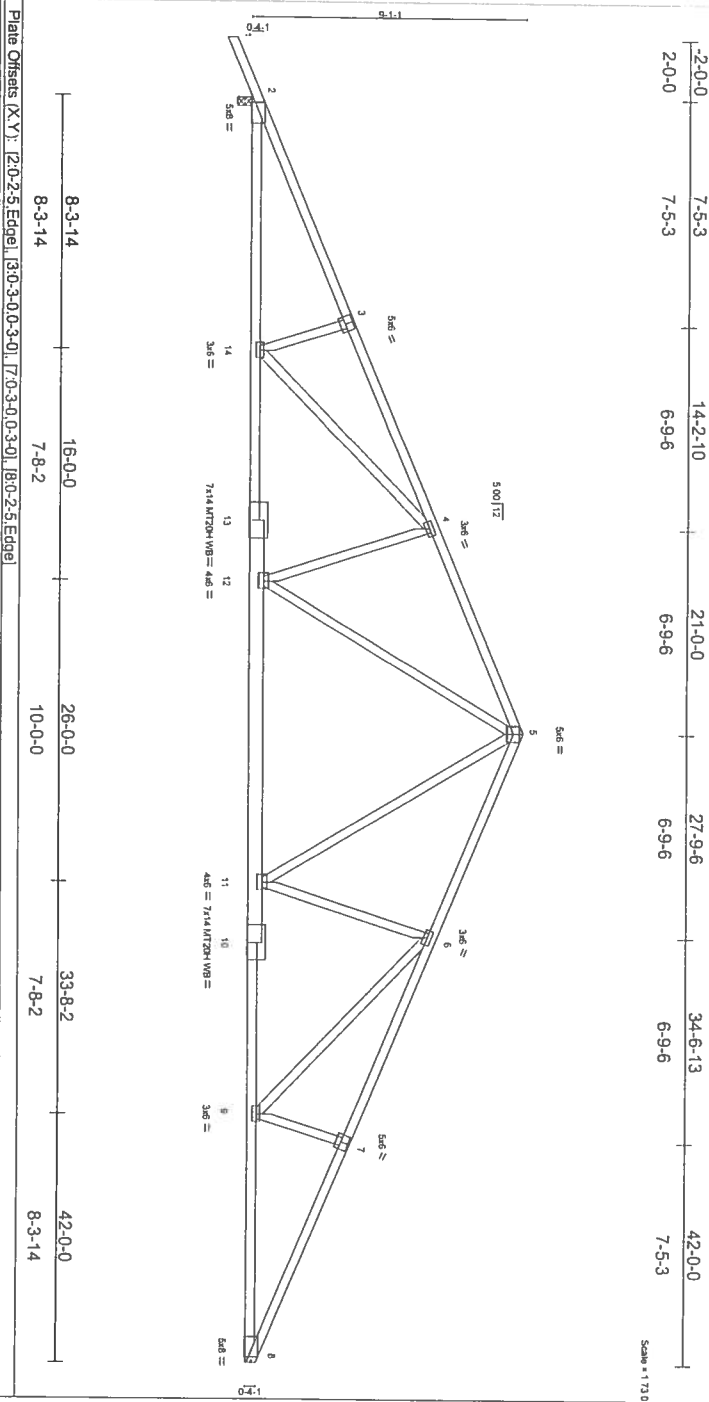
Scale = 1/4" = 1'-0"



Job	Trust Type	Qty	Ply	LINTON RESIDENCE
L216061	T04	2	1	Job Reference (optional)

Builders: FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	CSI	DEFL	PLATES
			in (loc)	GRIP
TCLL 200	Plates increase 1.25	TC 0.82	Ver(TL) -0.61 11-12	M720 244/190
TCLL 7.0	Lumber increase 1.25	BC 0.84	Ver(TL) -0.98 11-12	M720H 187/143
BCLL 100	Rep Stress Incr YES	WB 0.92	Horz(TL) 0.20 8	
BCLL 5.0	Code FBC2004/TP12002	(Matrix)		Weight: 225 lb

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD	2 X 4 SYP No.1D *Except*	BOT CHORD	Rigid ceiling directly applied or 5-8-14 oc bracing.
B22 X 6 SYP No.1D			
WEBS	2 X 4 SYP No.3		

REACTIONS
(ilvsize) 2=212/0-3, 8=1999/Mechanical
Max Horiz 2=175(load case 5)
Max Upriz2=794(load case 5), 8=669(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

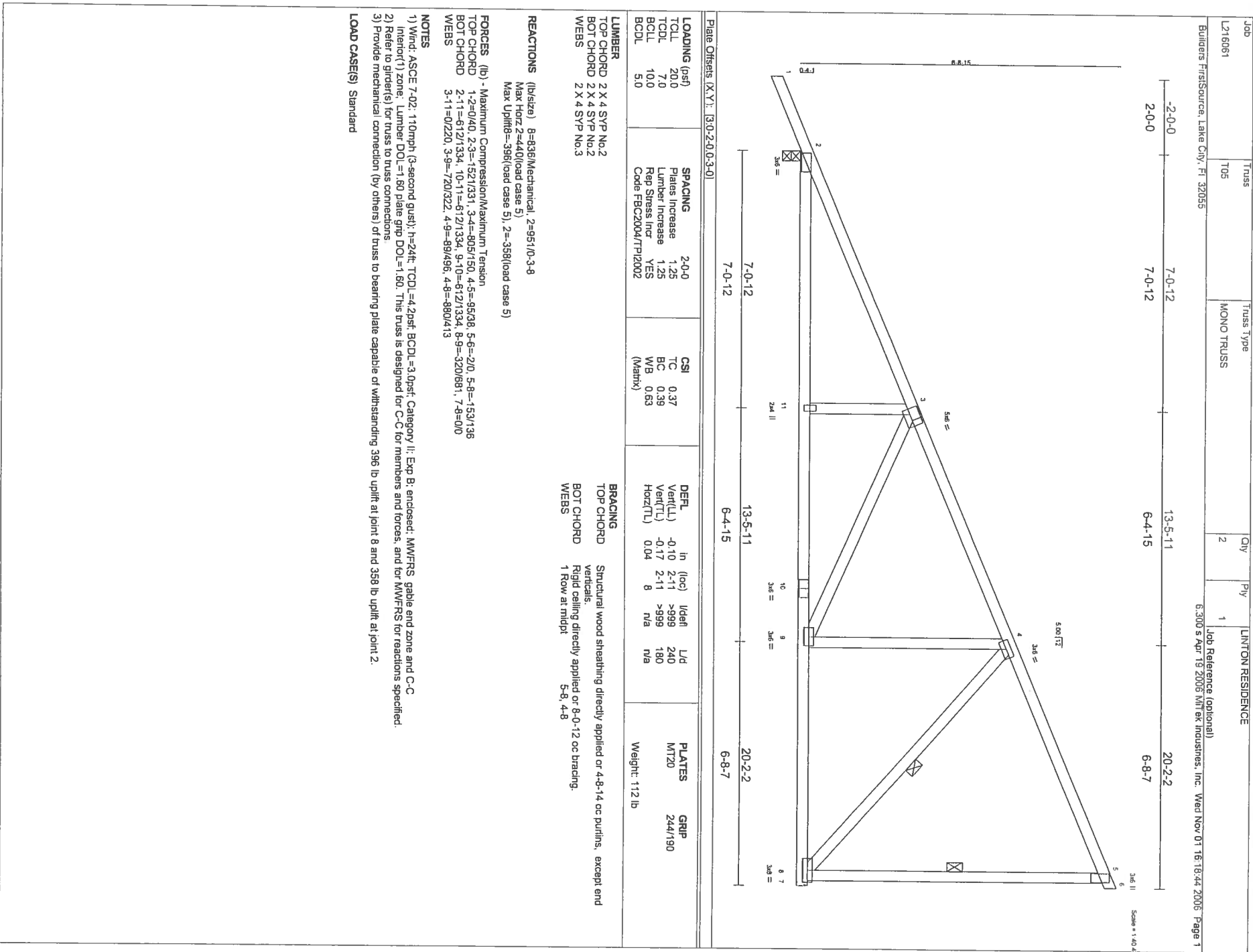
TOP CHORD	1-2=-440, 2-3=-4394/1421, 3-4=-4274/1472, 4-5=-3568/1278, 5-6=-3570/1287, 6-7=-4308/1521, 7-8=-4411/1468
BOT CHORD	1-2=413/5914, 13-14=-1068/3402, 12-13=-1067/3401, 11-12=-696/2567, 10-11=-956/3412, 9-10=-957/3408, 8-9=-1270/4008
WEEBS	3-14=-333/282, 4-14=-27/669, 4-12-6-14/400, 5-12=-5/41/346, 5-11=-520/1352, 6-11=-624/413, 6-9=-322/725, 7-9=-344/296

NOTES

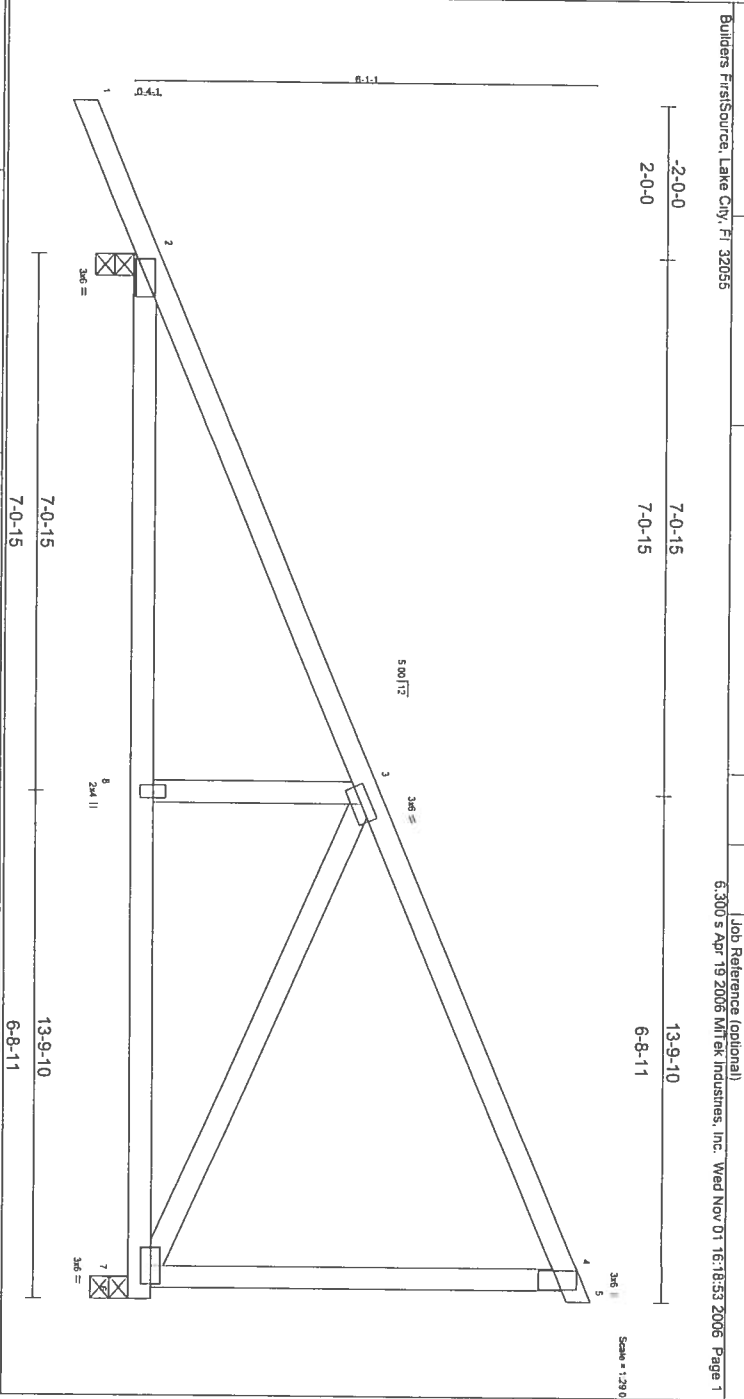
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02: 110 mph (3.60 dead gust); $h=244'$; TCDF = .205; BCLD = 3.0 psf; Category II; Exp. B: enclosed; MWFRS: gable end zone and C-C interior(1) zone; Lumber DOL = 1.60 plate grip DOL = 1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) All plates are $\frac{1}{4}$ " thick unless otherwise indicated.
- 4) Refer to girders(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 794 lb uplift at joint 2 and 669 lb uplift at joint 8.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (q/f)
Vert. 1-5=-54, 5-8=-54, 2-12=-30, 11-12=-80(f=-50), 8-11=-30

NOVEMBER 02, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	T05A	MONO TRUSS	5	1	
Builder's FirstSource, Lake City, FL 32055					6,300 s Apr 19 2006 Mitek Industries, Inc. Wed Nov 01 16:18:53 2006 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEF.	in	(loc)	l/def	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.38	Vert(LL)	-0.08	2-8	>999	240	MT20	244/190
TCCL 7.0	Lumber Increase	1.25	BC 0.31	Vert(TL)	-0.13	2-8	>999	180		
BCCL 10.0	Rep Stress Incr	YES	WB 0.68	Horz(TL)	0.02	7	n/a	n/a		
BCCL 5.0	Code FBC2004/TP12002		(Mathy)							
LUMBER										Weight: 68 lb
TOP CHORD 2 X 4 SYP No.2										
BOT CHORD 2 X 4 SYP No.2										
WEBS 2 X 4 SYP No.3										
BRACING										
TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins, except end verticals.										
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.										

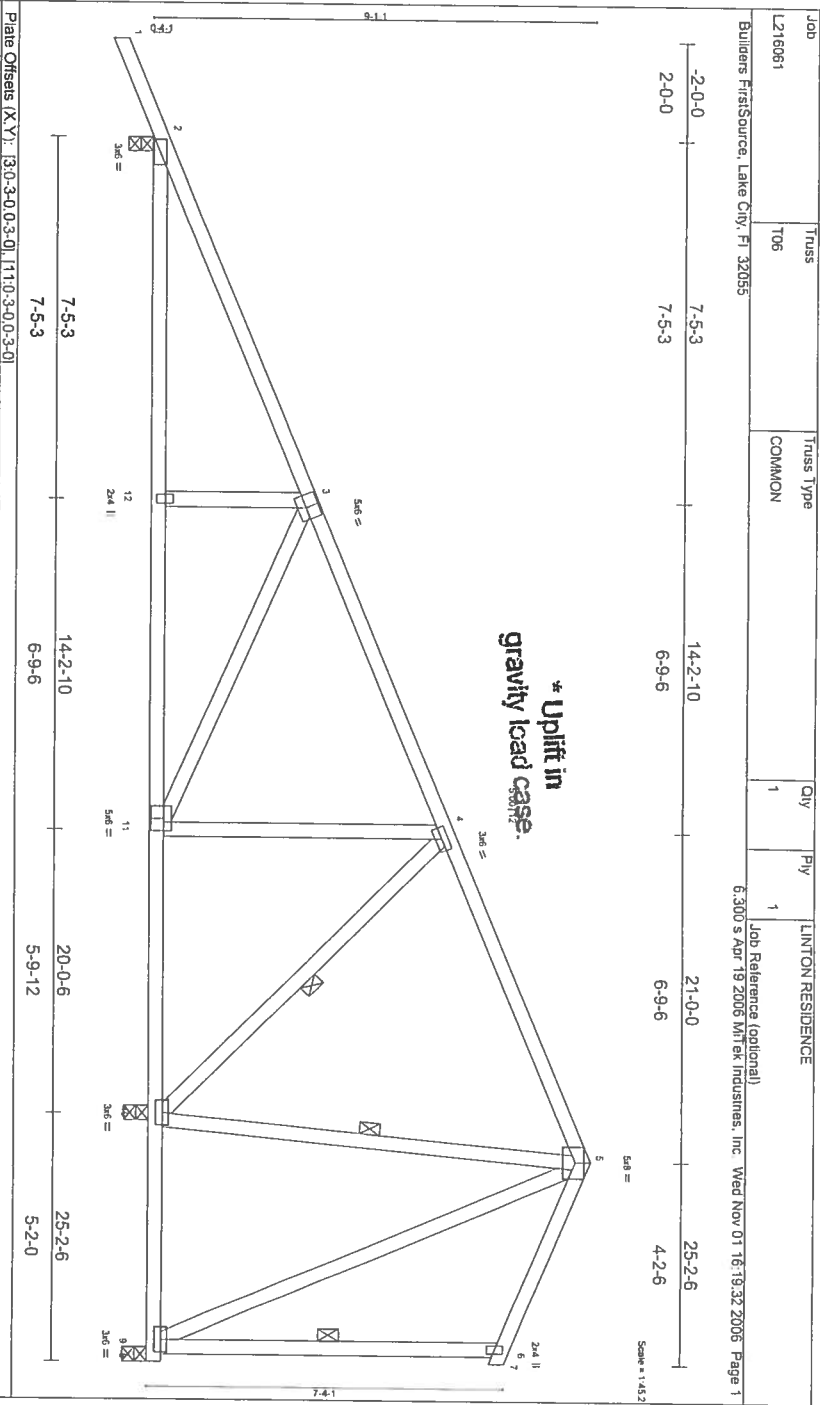
REACTIONS (lb/size) 7=555(0-3-8, 2=888(0-3-8
Max Horiz 2=318(load case 5)
Max Uplift 7=-265(load case 5), 2=-286(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=864/154, 3-4=-108/26, 4-5=-2/0, 4-7=-144/134
BOT CHORD 1-2=-328/733, 7-8=-328/733, 6-7=0/0

WEBS 3-8=0/228, 3-7=-7/4344

NOTES
1) Wind, ASCE 7-02, 110mph (3-second gust), h=24ft, TCCL=4.2psf, BCCL=3.0psf, Category II, Exp B, enclosed, MWFRS, gable end zone and C-C interior(1) zone, Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 265 lb uplift at joint 7 and 286 lb uplift at joint 2.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	PLATES	GRP
TCLL 20.0	Plates increase 1.25	MT20	244/190
TCCL 7.0	Lumber increase 1.25		
BCCL 10.0	Rep Stress Incr YES		
BCCL 5.0	Code FBC2004/TP2002		

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

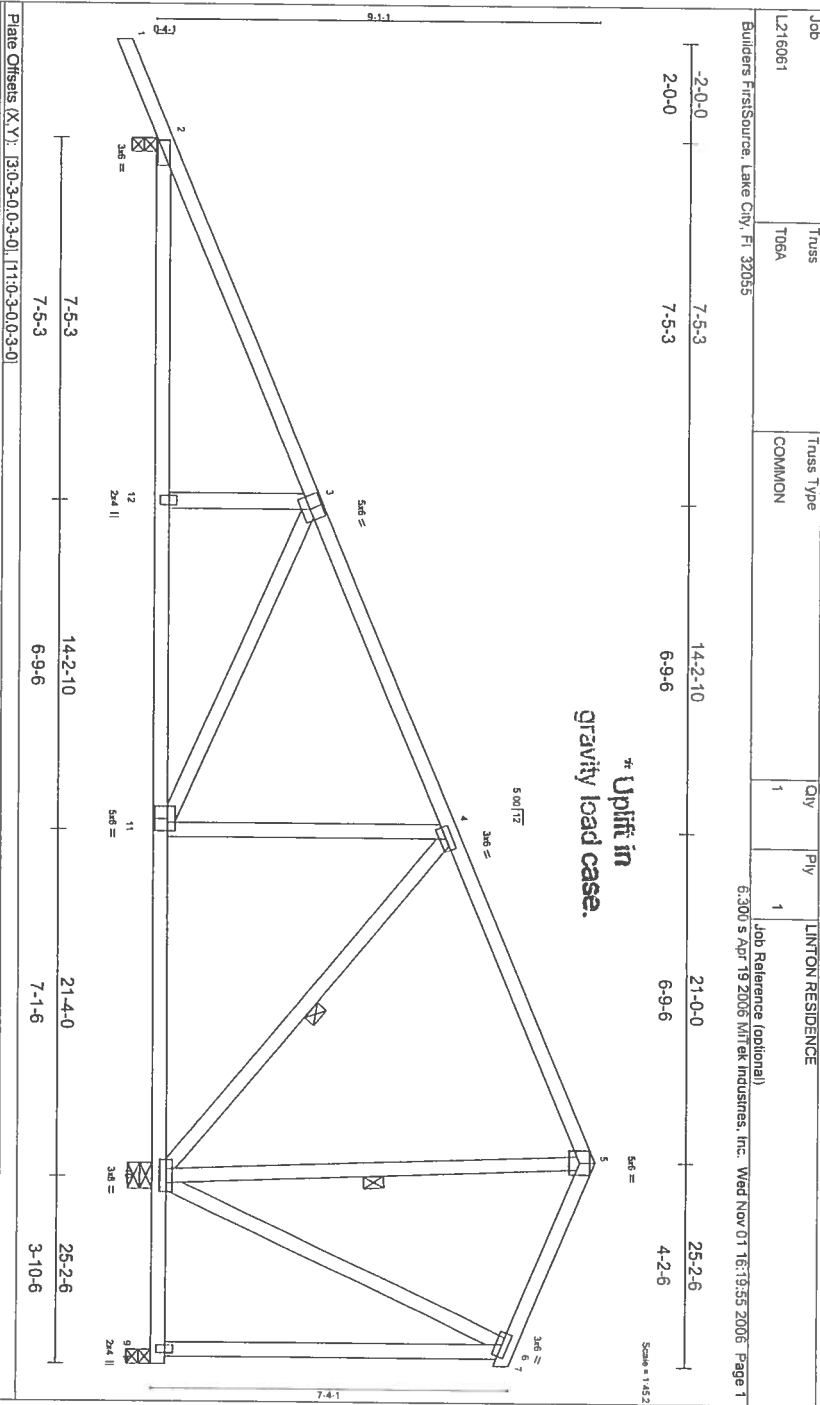
BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt: 4-10, 5-10, 6-9

REACTIONS (lb/size) 2=846/0-3-8, 10=1614/0-3-8, 9=264/0-3-8
Max Horiz 2=404(load case 5)
Max Uplift 2=340(load case 5), 10=615(load case 5), 9=373(load case 9)
Max Grav 2=846(load case 1), 10=614(load case 1), 9=104(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=1233/275, 3-4=457/82, 4-5=1173/54, 5-6=44/54, 6-7=0/0, 6-9=97/66
BOT CHORD 2-12=521/1066, 11-12=522/1059, 10-11=207/356, 9-10=185/51, 8-9=5/0
WEBS 3-12=0/242, 3-11=778/348, 4-11=109/511, 4-10=879/347, 5-9=120/450

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02, 110mph (3-second gust); h=24ft, TCCL=4.2psf, BCCL=3.0psf, Category II; Exp B; enclosed, MWFRS, gable end zone and C-C interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 340 lb uplift at joint 2, 615 lb uplift at joint 10 and 373 lb uplift at joint 9.

LOAD CASE(S) Standard



LOADING (psf)		SPACING		2-0-0		CSI		DEFL.		PLATES	
TCCL	200	Plates Increase	1.25	TC	0.37	Vert(L)	-0.12	In (loc)	1/defl	L/d	GRIP
TCCL	7.0	Lumber Increase	1.25	BC	0.47	Vert(L)	-0.20	2-12	>999	240	MIT20
BCCL	10.0	Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.03	10	n/a	180	244/190
BCCL	5.0	Code FBC2004/TP12002		(Manly)						n/a	Weight: 152 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt: 4-10, 5-10

REACTIONS (lb/size) 2=888/0-3-8, 9=556/0-3-8, 10=1894/0-6-7
Max Horiz2=404(load case 5)
Max Uplift2=356(load case 5), 9=650(load case 9), 10=722(load case 5)
Max Grav2=889(load case 9), 9=227(load case 9), 10=1894(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1539/316, 3-4=-573/122, 4-5=-120/345, 5-6=-89/356, 6-7=0/0, 6-9=-219/655
BOT CHORD 2-12=-558/1164, 11-12=-559/1157, 10-11=-244/463, 9-10=-5/4, 8-9=0/0
WEBS 3-12=0/236, 3-11=-/69/349, 4-11=-105/532, 4-10=-936/436, 5-10=-549/276, 6-10=-616/222

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=24ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS; gable end zone and C-C interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 356 lb uplift at joint 2, 650 lb uplift at joint 9 and 722 lb uplift at joint 10.

LOAD CASE(S) Standard

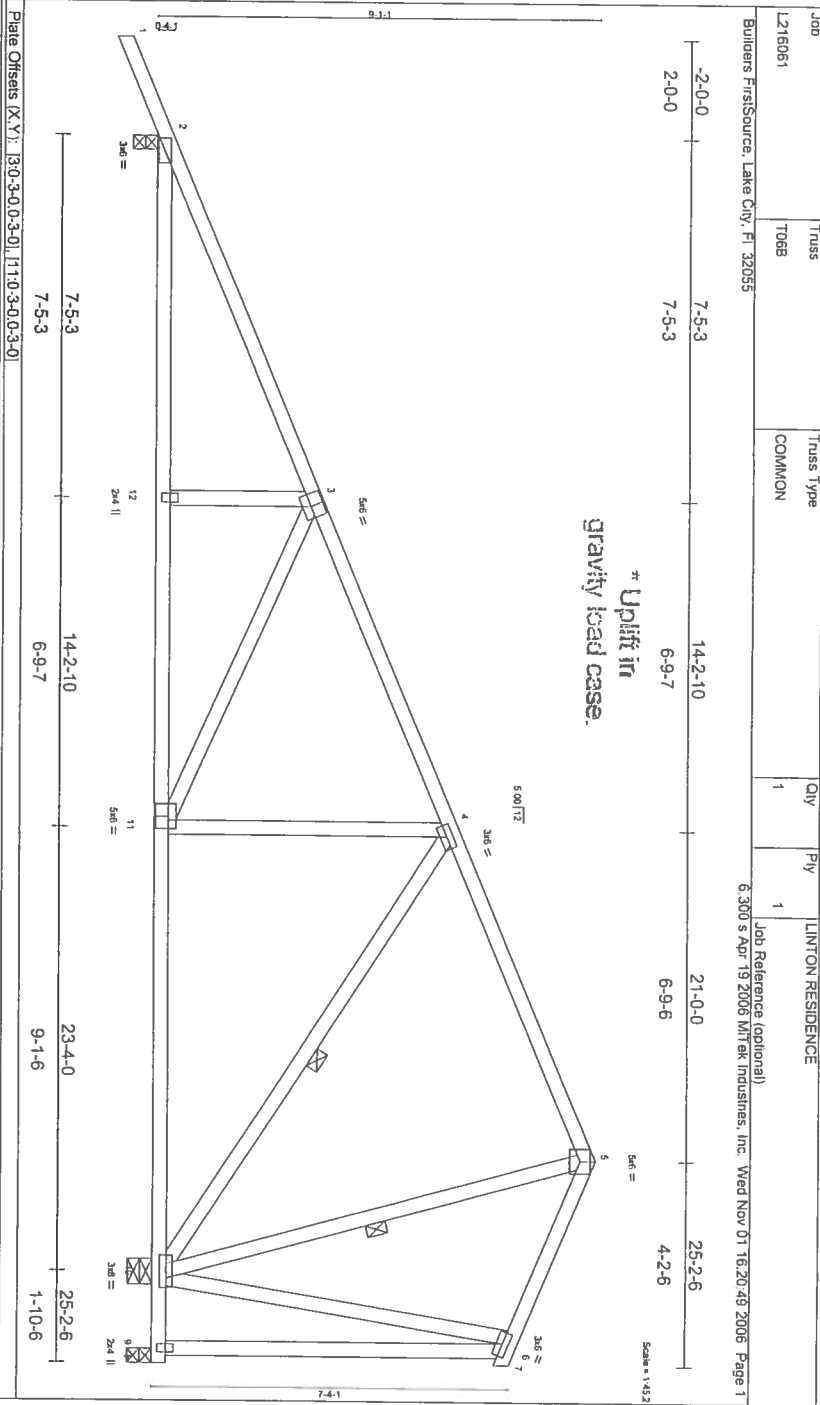


Plate Offsets (X,Y): (30-3-0-3-0), (110-3-0-3-0)									
LOADING (psf)									
TCDL	20.0	SPACING	2'-0"						
TCOL	7.0	Plates Increase	1.25						
BCLL	10.0	Lumber Increase	1.25						
BCDL	5.0	Rep Stress Incr	YES						
		Code FBC2004/PT2002							
CSJ									
TC	0.35								
BC	0.56								
WB	0.73								
(Wahy)									
DEFL									
Vert(L)	-0.14	in (cc)							
Vert(R)	-0.24	10-11							
Horz(L)	0.05	10							
Horz(R)		n/a							
PLATES									
MT20									
GRIP									
2x4/190									
Weight: 155 lb									

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-10, 5-10

REACTIONS (lb/size) 2=1054/0-3-8, 9=562/0-3-8, 10=1716/0-6-7
Max Horiz 2=401(load case 5)
Max Uplift 2=425(load case 5), 9=609(load case 9), 10=582(load case 5)
Max Grav 2=1054(load case 1), 9=153(load case 5), 10=1716(load case 1)

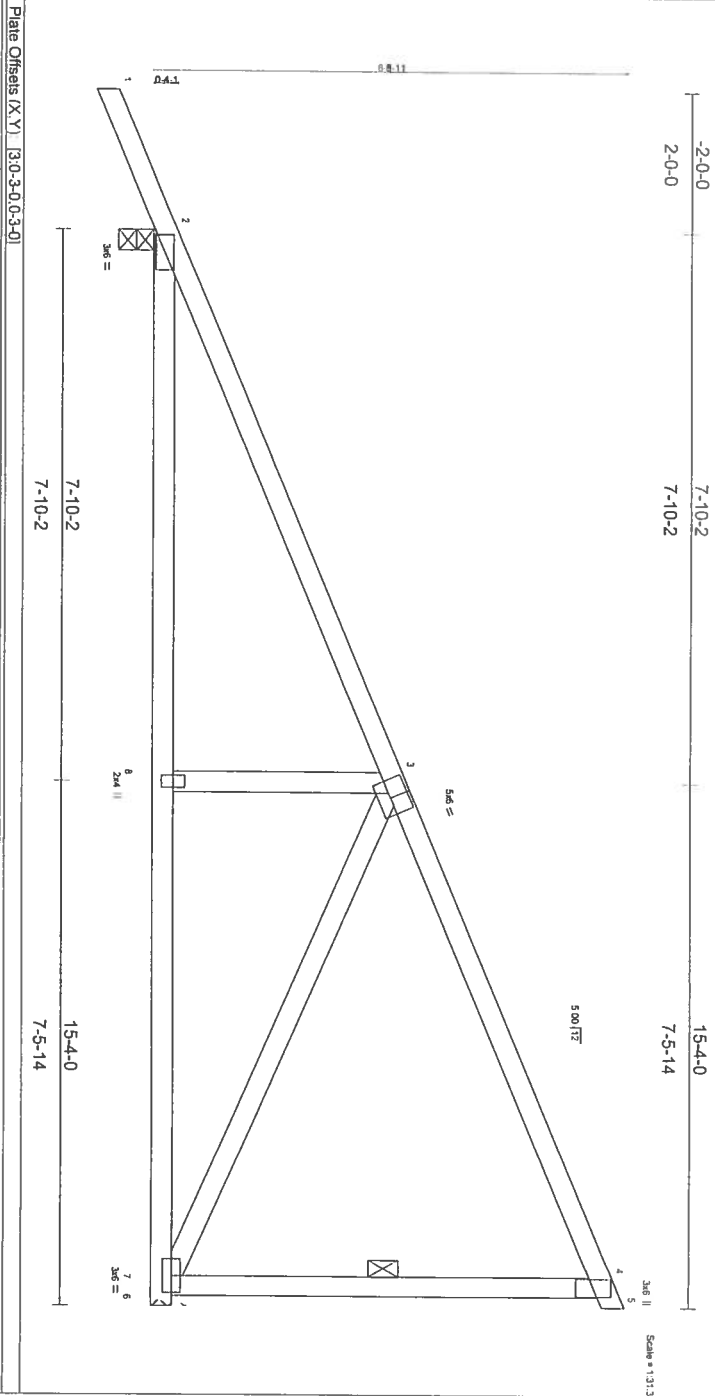
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=1752/491, 3-4=1003/296, 4-5=718/8, 5-6=291/69, 6-7=0/2, 6-9=164/473
BOT CHORD 2-12=715/1544, 11-12=716/1537, 10-11=402/850, 9-10=10/1, 8-9=0/0
WEBS 3-12=0/224, 3-11=750/348, 4-11=98/356, 4-10=1033/460, 5-10=387/211, 6-10=465/180

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02, 110mph (3-second gust); h=24ft, TCDL=4.2psf, BCCL=3.0psf, Category II, Exp B; enclosed, MWFRS, gable end zone and C-C interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 425 lb uplift at joint 2, 609 lb uplift at joint 9 and 582 lb uplift at joint 10.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	107	MONO TRUSS	5	1	

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCL	Plates increase	TC	In (loc)	L/def	M/20
TCDL	Lumber increase	BC	Vert(II)	240	244/190
BCL	Rep Stress incr	WB	Vert(TL)	160	
BCL	Code FBC2004/PI2002	0.96	Horz(TL)	n/a	
5.0		(Mainx)			Weight: 76 lb

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 5-9-14 oc purlins, except and verticals.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2 X 4 SYP No.3	WEBS	1 Row at midpt 4-7

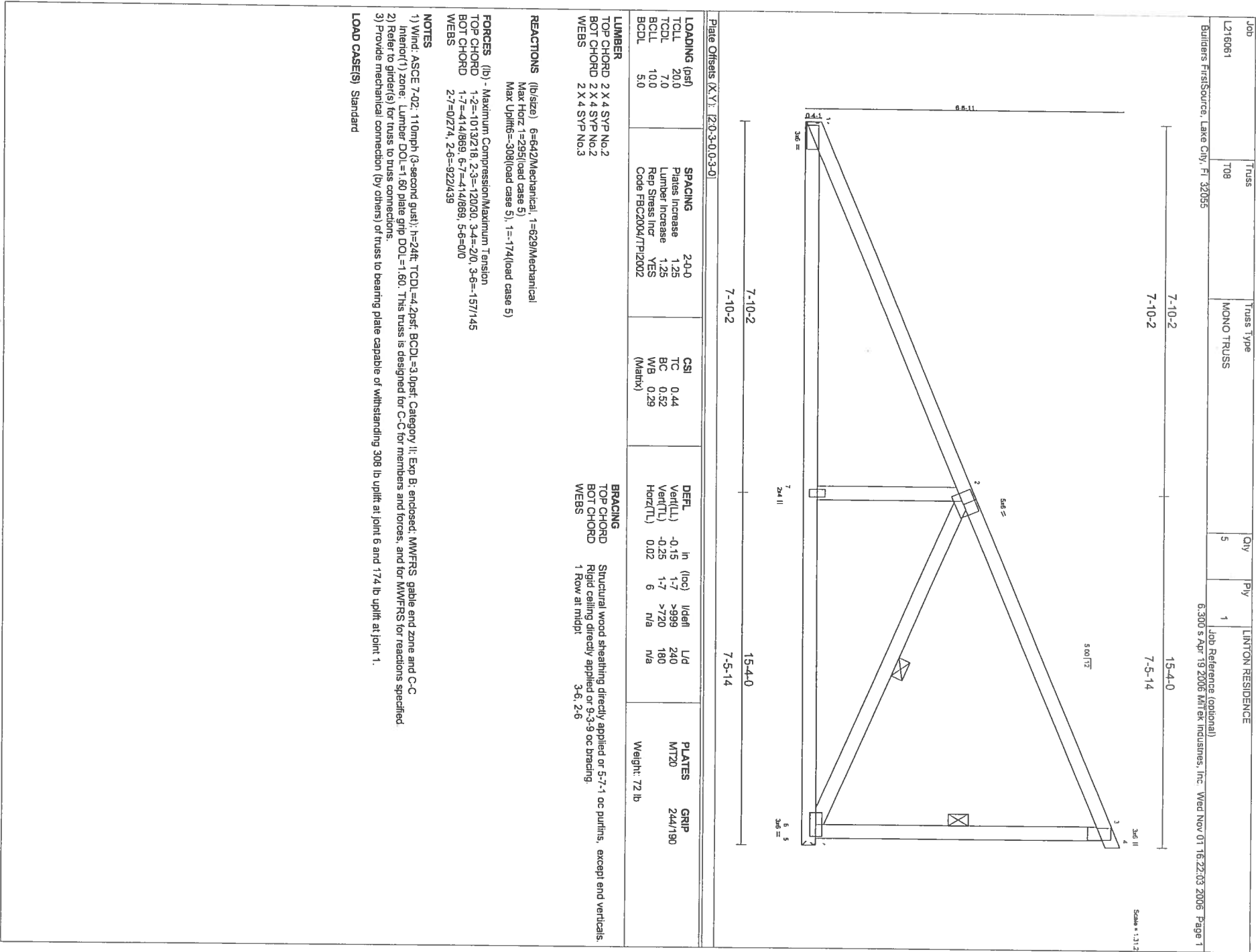
REACTIONS
(lb/size) 7=630/Mechanical, 2=750/0-3-8
Max Horiz 2=347(load case 5)
Max Uplift 7=297(load case 5), 2=303(load case 5)

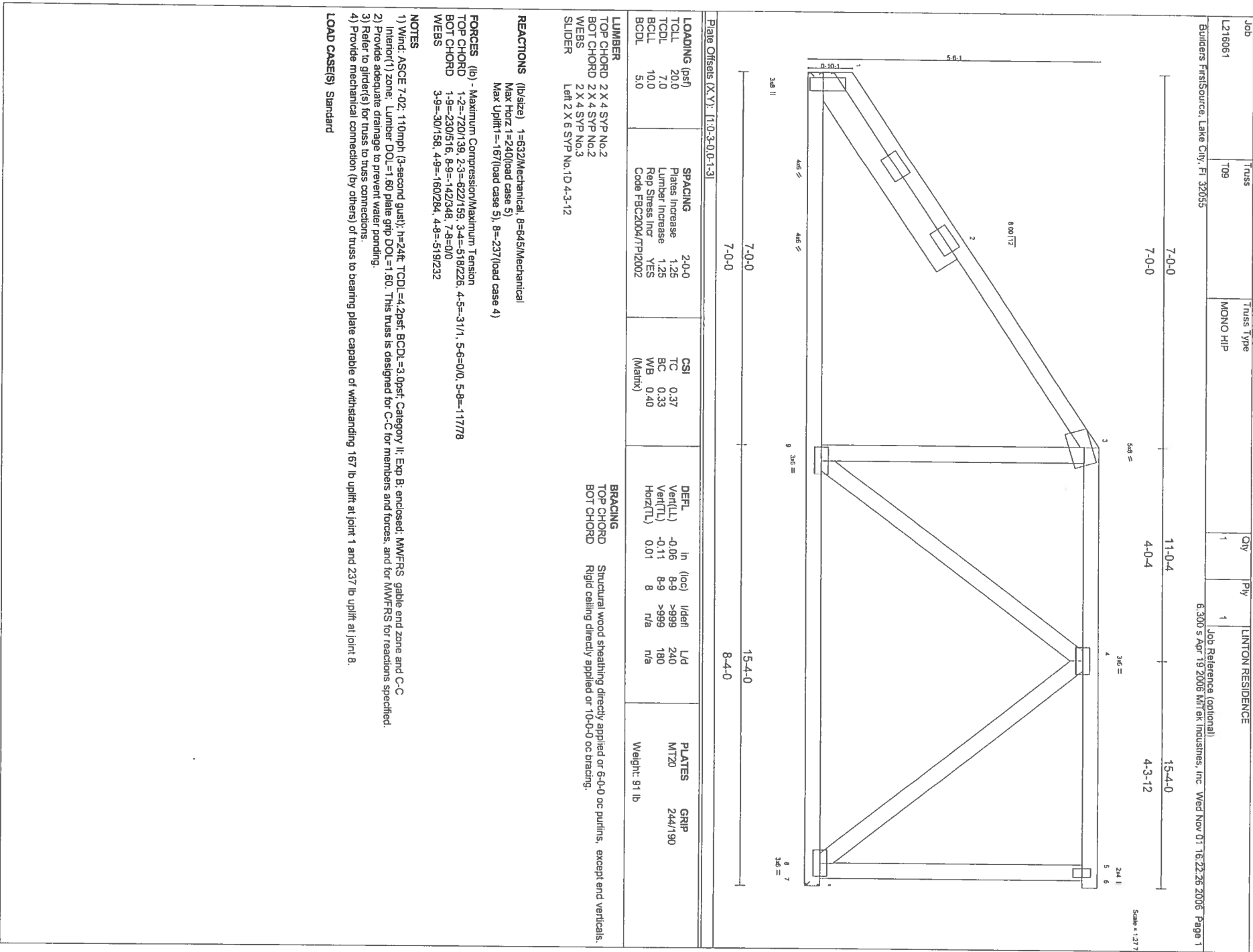
FORCES (lb) - Maximum Compression/Maximum Tension
 STOP CHORD 1-2=0/40, 2-3=977/182, 3-4=121/29, 4-5=-20, 4-7=-160/148
 BOT CHORD 2-8=377/831, 7-8=377/831, 6-7=0/0
 WEBS 3-8=0/258, 3-7=-877/395

NOTES

- (1) Wind ASCE 7-02: 110m (3-second gust), $h=24t$, TCDF = 0.25%, BCDL = 0.05% Category II; Exp. B, enclosed, MWFRS gable end zone and C-C interior (1) zone; Lumber DOL = 1.60 plate grip DOL = 1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified in (2).
- (2) Refer to girders (s) for truss to truss connections.
- (3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 7 and 303 lb uplift at joint 2.

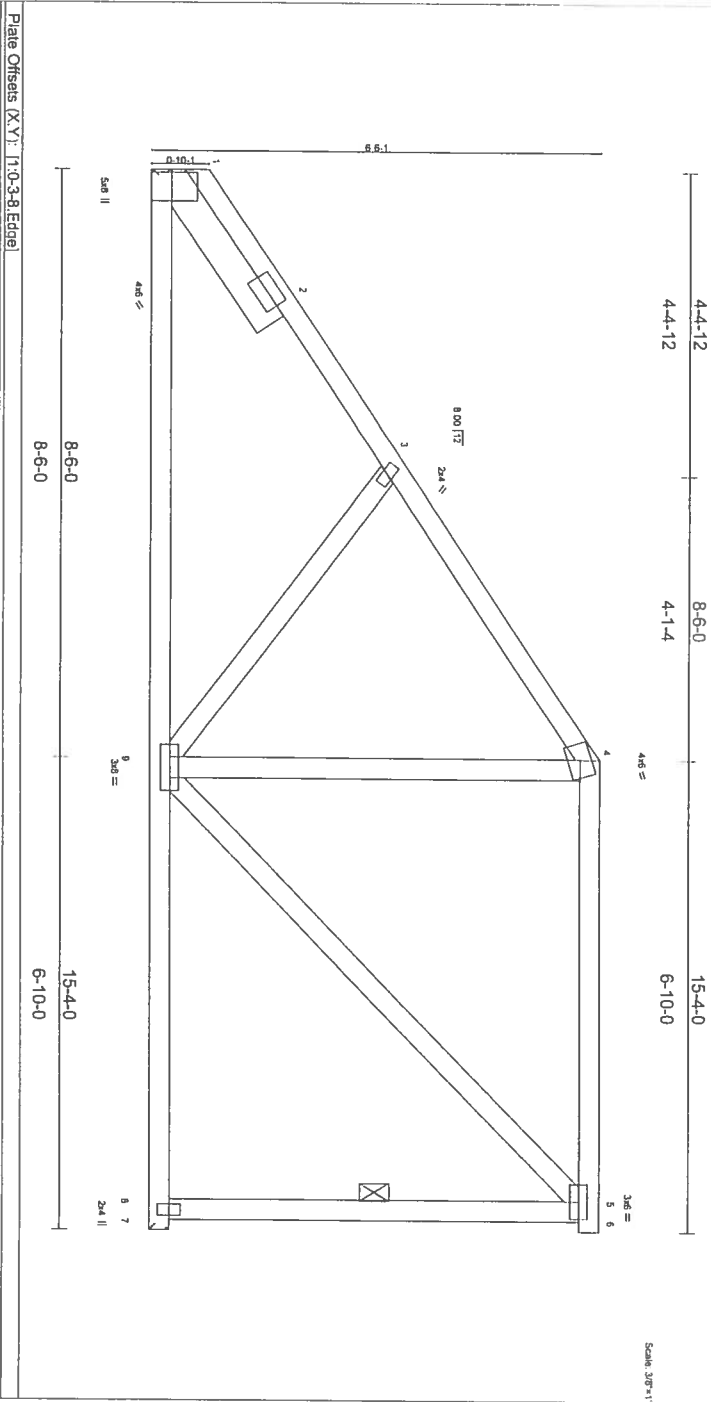
LOAD CASE(S) Standard





Job	Truss Type	Qty	Ply	Job Reference (optional)
L216061	MONO HIP	1	1	

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LOADING (psf)	SPACING	2'-0"	CSI	DEFL	In (in)	Udefl	L/d	PLATES	GRIP
TCLL	Plates increase	1.25	BC	Ver(T/L)	-0.09	1-9	240	MT20	244/190
TCLL	Lumber increase	1.25	TC	Ver(T/L)	-0.15	-999	180		
BCLL	Rep Stress Incr	YES	WB	Horz(T/L)	0.01	8	n/a		
BCLD	Code FBC2004/TP2002		(Matrx)					Weight: 33 lb	
LUMBER									
TOP CHORD	2 X 4 SYP No.2			BRACING					
BOT CHORD	2 X 4 SYP No.2			Structural wood sheathing directly applied or 6'-0" oc purlins, except end verticals.					
WEBS	2 X 4 SYP No.3			Rigid ceiling directly applied or 10'-0" oc bracing.					
WEBS	Left 2 X 6 SYP No.1D 2'-8"-1			1 Row at midpt					
				5-8					

REACTIONS (lb/size) 1=532/Mechanical, 8=645/Mechanical
Max Horz 1=286(load case 5)
Max Uplift1=-159(load case 5), 8=-232(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-147/1185, 2-3=-688/199, 3-4=-557/151, 4-5=-423/183, 5-6=0/0, 5-9=553/245
BOT CHORD 1-9=-353/555, 8-9=-19/30, 7-8=0/0
WEBS 3-9=-167/212, 4-9=-44/136, 5-9=-245/546

NOTES

1) Wind: ASCE 7-02, 110mph (3-second gust), $h=24ft$, $TCFD=4.2psf$, $BCFD=3.0psf$, Category II, Exp B; encased; MMWFRS gable end zone and C-C interior (1) zone. Lumber DOL=1.60, plate gip DOL=1.60. This truss is designed for C-C for members and forces, and for MMWFRS for reactions specified

2) Provide adequate drainage to prevent water ponding.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 159 lb uplift at joint 1 and 532 lb uplift at joint B.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	T11	MONO HIP	1	1	
Builders FirstSource, Lake City, FL 32055					6,300 S Apr 19 2006 Mittek Industries, Inc. Wed Nov 01 16:23:12 2006 Page 1

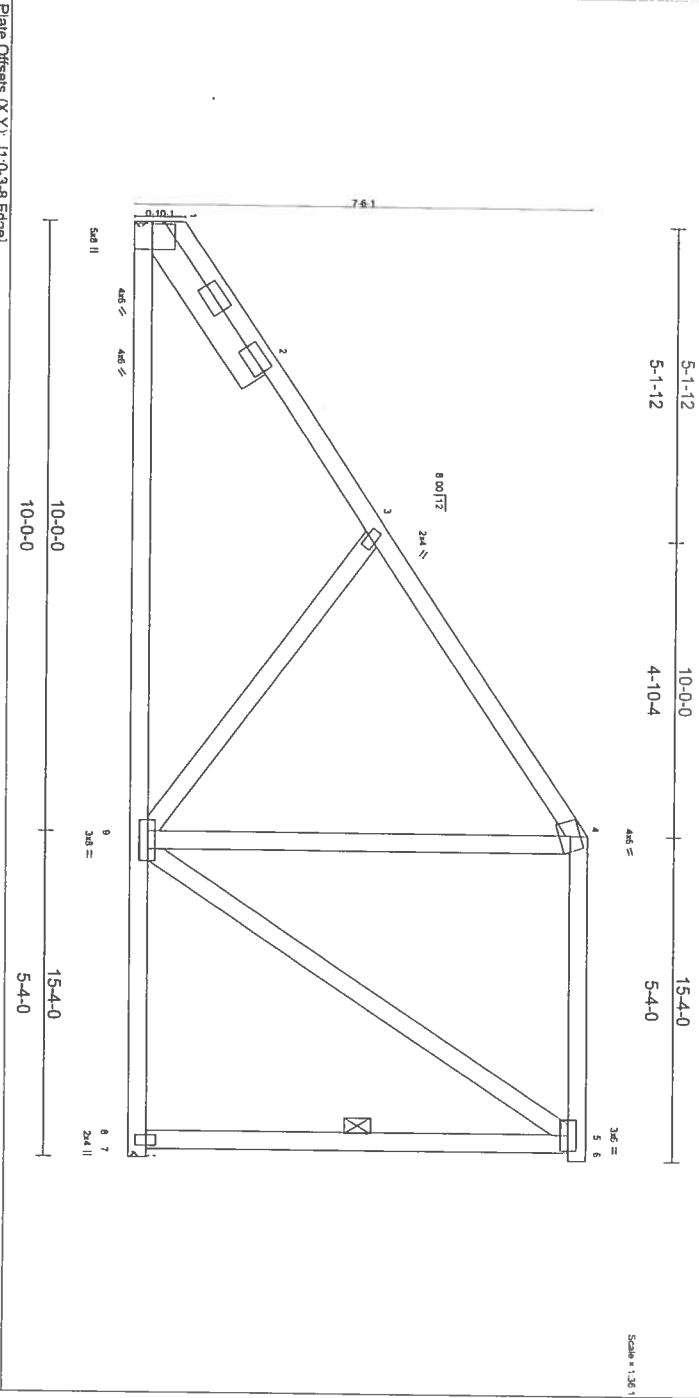


Plate Offsets (X,Y) [10-3-8 Edge]									
LOADING (psf)		SPACING		CSI		DEF.		PLATES	
TCU	20.0	Plates Increase	2-0-0	TC	0.22	Vert(L)	-0.17	M120	GRIP
TCU	7.0	Lumber Increase	1.25	BC	0.44	Vert(T)	-0.30	M120	244/190
BCU	10.0	Rep Stress Incr	YES	WB	0.35	Horz(T)	0.01		
BCU	5.0	Code FBC2004/FP12002		(Matth)					Weight: 98 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 3-1-8

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-8

REACTIONS (lb/size) 1=632/Mechanical, 8=645/Mechanical
Max Horiz 1=332(load case 5)
Max Uplift 1=-147(load case 5), 8=-263(load case 5)

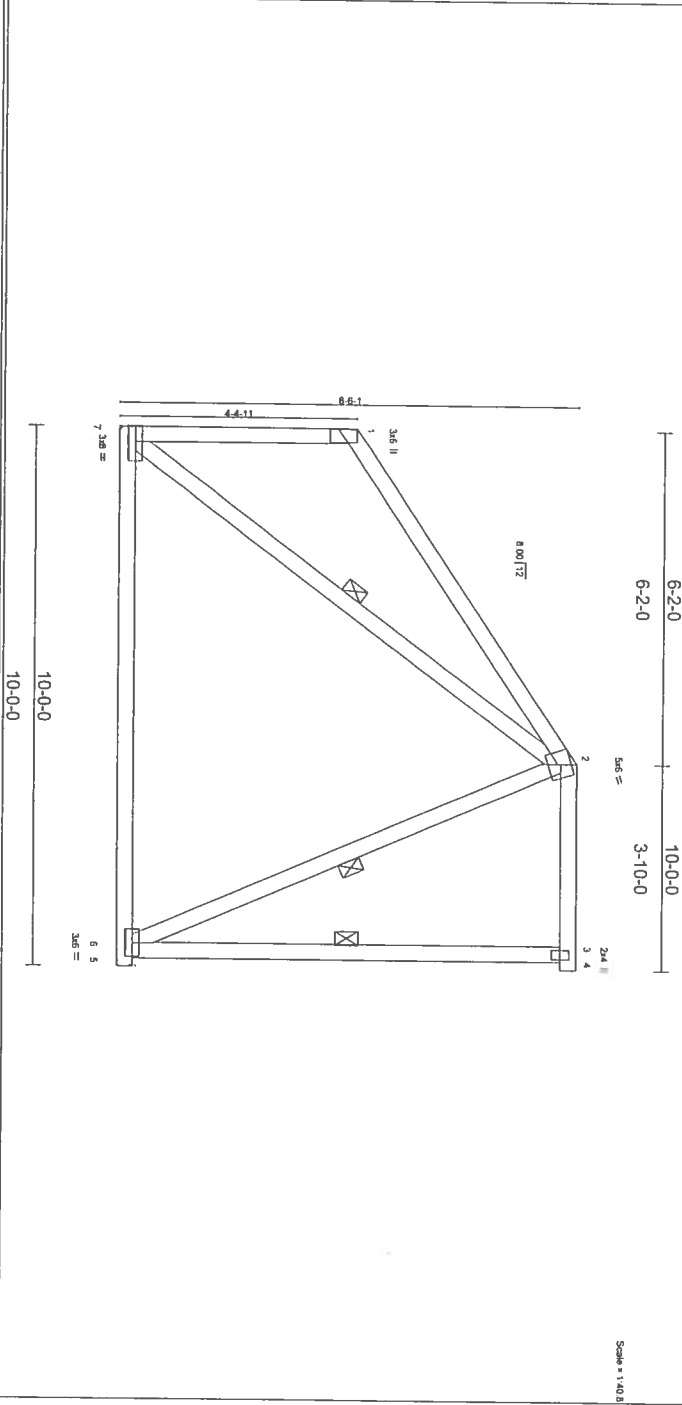
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=714/158, 2-3=836/174, 3-4=474/109, 4-5=-331/153, 5-6=-00, 5-8=-599/270
BOT CHORD 1-9=-369/636, 6-9=-109, 7-9=00
WEBS 3-9=-260/266, 4-9=-65/117, 5-9=-268/573

NOTES
1) Wind: ASCE 7-02, 110mph (3-second gust); n=24ft, TCUL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS, gable end zone and C-C Interior(1) zone; Lumber DOL=1.60, plate grp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 147 lb uplift at joint 1 and 263 lb uplift at joint 8.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	T12	MONO HIP	1	1	Job Reference (optional)

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCL	2.0-0	TC	in (loc)	M120	244/190
TCL	Plates increase	BC	Vert(LL)		
TCL	Lumber increase	WB	Vert(TL)		
BCL	Rep Stress incr	0.08	Horz(TL)		
BCL	Code FBC2004/TP12002	(Matrix)		Weight: 77 lb	

LONGSPAN		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	except end verticals.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	
WEBS	2 X 4 SYP No.3	WEBS	
		Structural wood sheathing directly applied or 6'-0" oc purlins,	
		Rigid ceiling directly applied or 10'-0" oc bracing.	
		1 Row at midpt	
		3-6, 2-6, 2-7	

REACTIONS

(lb/size) 6=415/Mechanical, 7=401/Mechanical
Max Horiz 7=186(load case 5)
Max Uplift6=-235(load case 5), 7=-22(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-194/146, 2-3=-23/0, 3-4=0/0, 3-6=-18/91, 1-1=-222/181

2-6--110/100, 3-6=0/0
2-6--210/295, 2-7--164/68

NOTES

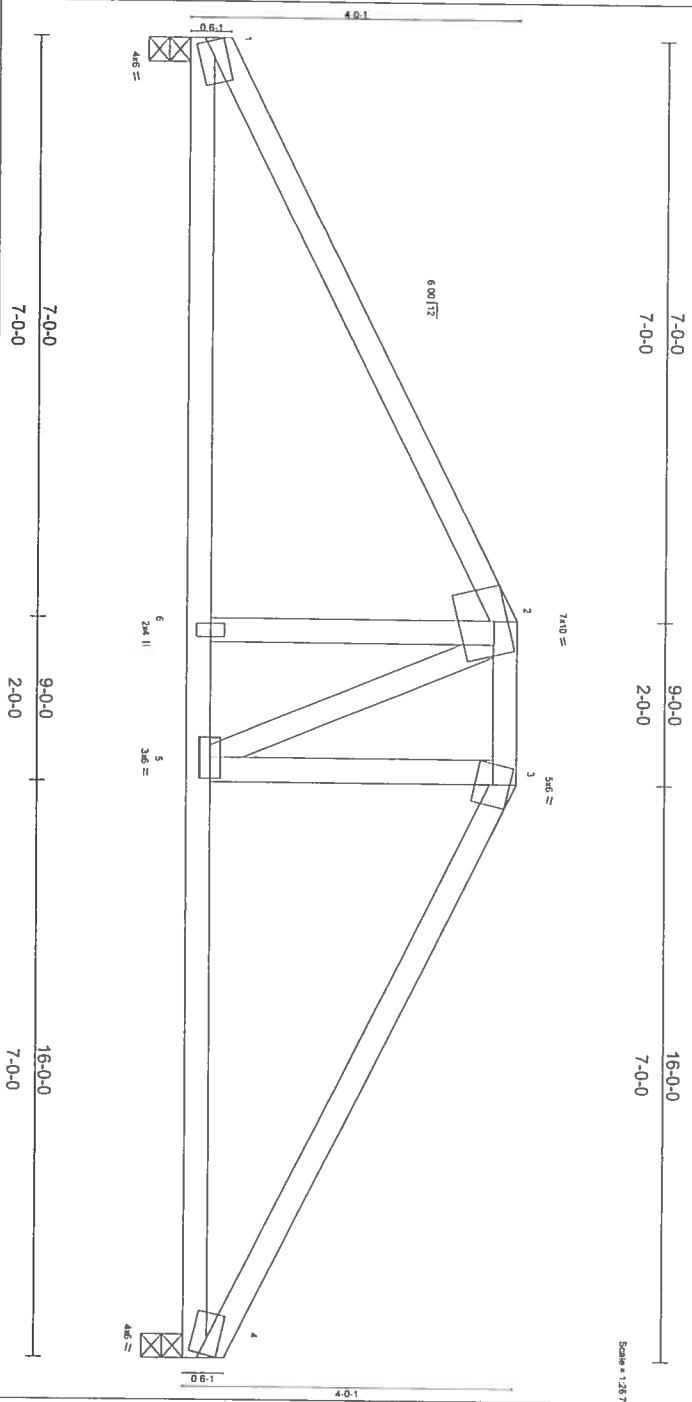
- 1) Wind: ASCE 7 zone: 110m (3rd dense gust) $h=24'$, $TCDL=4.20st$, $BCDI=3.00st$, Category II, Exp B, enclosed, MWFRS cable end zone and C-C Interior(1) zone, Lumber DOL = 1.60 plate girth DOL = 1.60. This truss is designed for C-C for members and bracing, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) Refer to girders for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 235 lb uplift at joint 6 and 22 lb uplift at joint 7.

LOAD CASE(S) Standard

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	T13	HIP	1	1	Job Reference (optional)

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	LOADING (ksf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL	20.0	Plates increase 1.25	0.49	In (in) -0.14	L/D	M72D
TCDL	7.0	Lumber increase 1.25	BC	Vert(TL) -0.22	240	Z44/190
BCLL	10.0	Rep Stress inc NO	WB	Horz(TL) 4	180	
BCOL	5.0	Code FBC2004/TTP2002	(Matrix)	n/a	n/a	Weight: 66 lb

TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 3-8-6 oc pultrus.
BOT. CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 6-9-9 oc bracing.
WEBS	2 X 4 SYP No.3		

REACTIONS (lb/size) 1=1292/0-3-8, 4=1292/0-3-8

Max Horiz 1=54(load case 3)
Max Uplift1=-527(load case 4), 4=-527(load case 5)

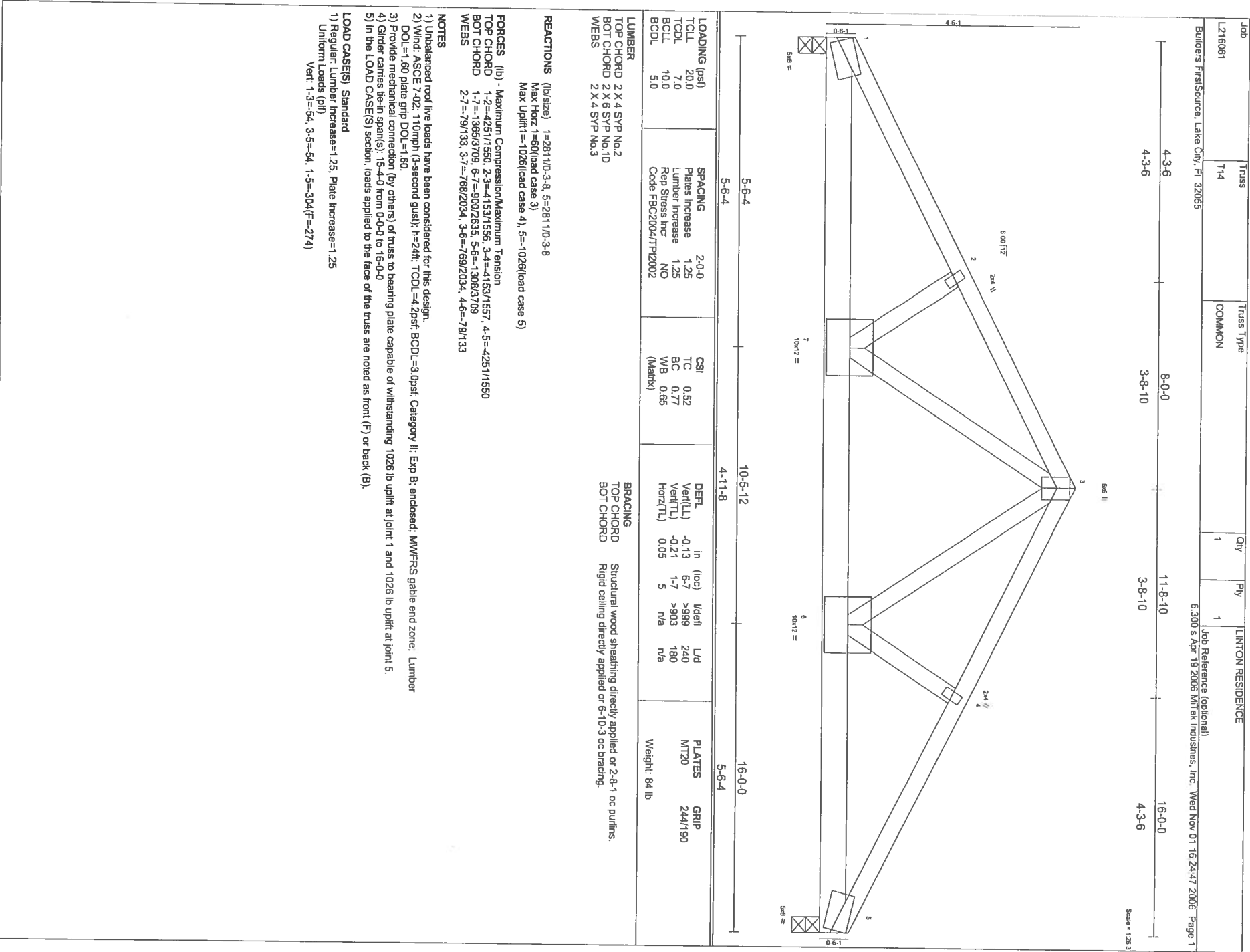
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=2287/947, 2-3=1977/918, 3-4=2292/948
BOT CHORD 1-6=790/1944, 5-6=800/1972, 4-5=753/1948
WEBS 2-6=2647/42, 2-5=172/192, 3-5=312/849

NOTES

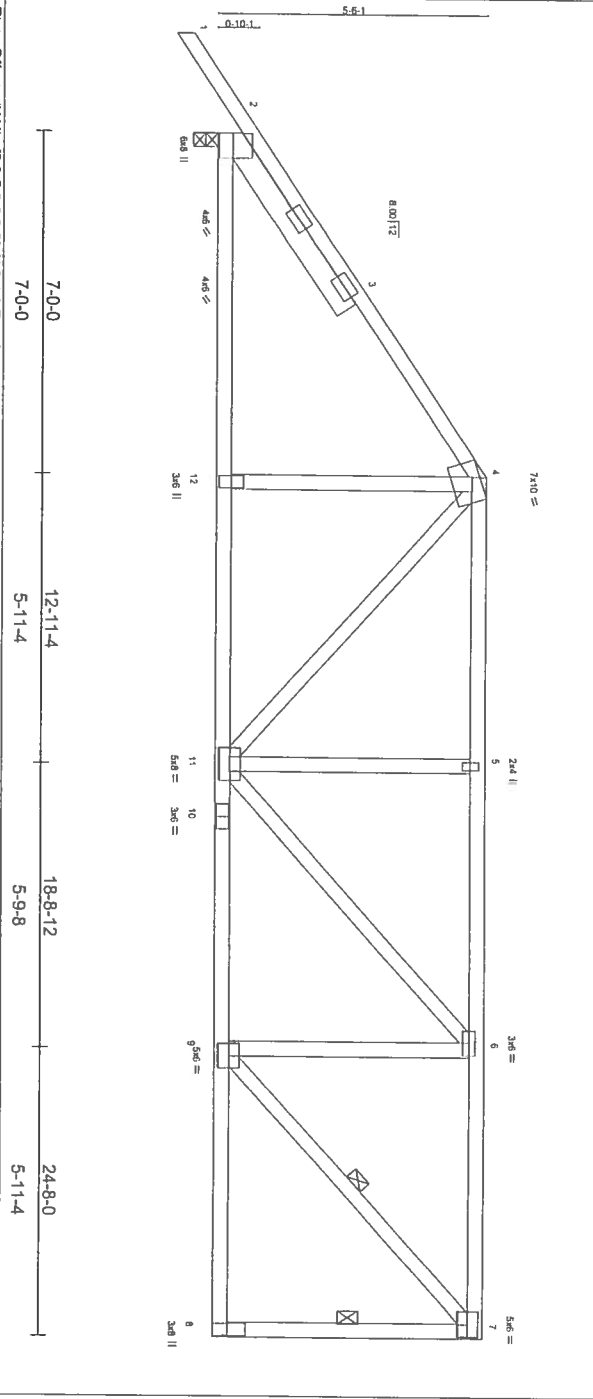
- (1) Unbalanced roof live loads (3-second gust) for this design.
- (2) Wind: ASCE -02; 1.0 mph (3-second gust); $h=24ft$; $TCDL=4.2psf$; $BCDL=3.0psf$; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber $DOE=1.60$; plate gage $BOE=1.60$.
- (3) Provide adequate drainage to prevent water ponding.
- (4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 527 lb uplift at joint 1 and 527 lb uplift at joint 4.
- (5) Girders carries full end with 740-lb and setback.
- (6) Hanger(s) and other connection device(s) shall be provided sufficient to support concentrated loads(s) 539 lb down and 268 lb up at 9'-0" O.C. and 539 lb down and 268 lb up at 7'-0" on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- (7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (pl)
Vert: 1-2=54, 2-3=118(F=64), 3-4=54, 1-6=30, 5-6=65(F=35), 4-5=30
Concentrated Loads (lb)
Vert 5=539(F) 5=539(F)



JOB	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	T1S	MONO HIP	1	1	
Builder's FirstSource, Lake City, Fl 32055					6,300 S Apr 19 2006 Mfrak Industries, Inc Wed Nov 01 16:26:19 2006 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	PLATES	GRIP
TCCL	Plates Increase	1.25	TC	In (cc)	MT20	244/190
TCCL	Lumber Increase	1.25	BC	11-12		
TCCL	Rep Stress Incr	NO	WB	0.07		
BCCL	Code FBC2004/TF12002		(Math)	n/a		
BCCL						Weight: 152 lb

LUMBER	TOP CHORD	2 X 4 SYP No.1D *Except*	BRACING	TOP CHORD	Structural wood sheathing directly applied or 2-11-10 oc purlins, except end verticals.
	BOT CHORD	2 X 4 SYP No.2		BOT CHORD	Rigid ceiling directly applied or 5-8-9 oc bracing.
	WEBS	2 X 4 SYP No.3		WEBS	1 Row at midpt. 7-8, 7-9
	SLIDER	Left 2 X 6 SYP No.1D 4-3-12			

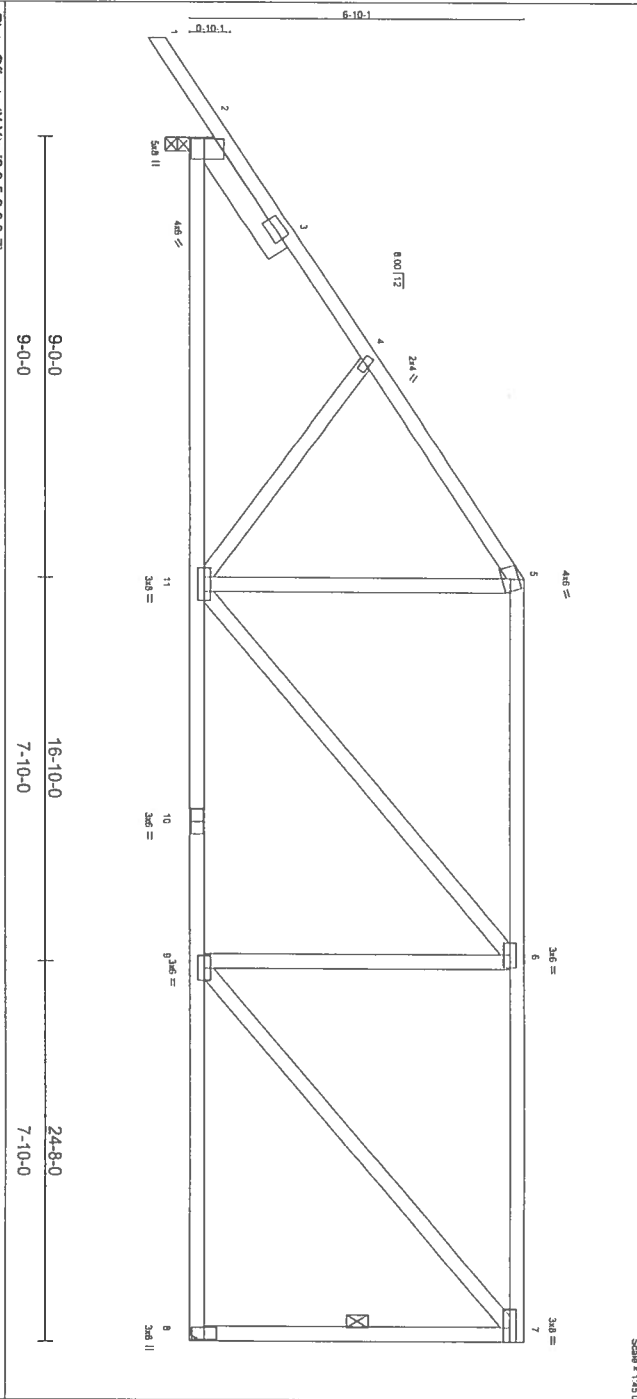
REACTIONS (lb/size) 8=2295/Mechanical, 2=2139/C-3-8
Max Horiz 2=294/load case 4)
Max Uplift 8=-1211(load case 2), 2=-1010(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=3036/1397, 3-4=2921/415, 4-5=2854/1462, 5-6=2853/1463, 6-7=2057/1071, 7-8=2107/1230
BOT CHORD 2-12=1199/2399, 11-12=1206/2382, 10-11=1071/2057, 9-10=1071/2057, 8-9=4173
WEBS 4-12=265/809, 4-11=492/625, 5-11=735/736, 6-11=552/1070, 6-9=1408/1028, 7-9=-1386/2669

- NOTES**
- Wind: ASCE 7-02, 110mph (3-second gust); h=24ft, TCCL=4.2psf, BCCL=3.0psf, Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
 - Provide adequate drainage to prevent water ponding.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1211 lb uplift at joint 8 and 1010 lb uplift at joint 2.
 - Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 332 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (pl)
Vert: 1-4=-54, 4-7=-118(F=-64), 2-12=30, 8-12=65(F=-35)
Concentrated Loads (lb)
Vert: 12=539(F)

Job	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	T16	1	1	Job Reference (optional)



LOADING (psf)	SPACING	CSI	DEFL.	PLATES	GRIP
TCLL 20.0	Plates increase 2.0-0	TC 0.61	in (loc) l/delt	MT20	24x4/190
TCCL 7.0	Lumber increase 1.25	BC 0.48	Ver(T/L) -0.11 2-11 >999		
BCLL 10.0	Rep Stress incr YES	WB 0.73	Ver(T/L) -0.19 2-11 >999		
BCDL 5.0	Code FBC2004/TP/2002	(Matrix)	Horz(T/L) 0.03 8 n/a	Weight 151 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No. 2	TOP CHORD	Structural wood sheathing directly applied or 5-4-9 oc purlins, except end verticals.
BOT CHORD	2 X 4 SYP No. 2	BOT CHORD	Rigid ceiling directly applied or 9'-1-6 oc bracing
WEBS	2 X 4 SYP No. 3	WEBS	1 Row at midpt
SLIDER	Left 2 X 6 STP No. 1D 2-9-14		7-8

REACTIONS
(libsize) 8=1025/Mechanical, 2=11420-3-8
Max Horz 2=354(load case 5)
Max Uplift8=-388(load case 3), 2=-398(load case 5),

FORCES (lb) - Maximum Compression/Maximum Tension
 1-2=0/1, 2-3=1353/244, 3-4=1277/352, 4-5=1162/241, 5-6=948/335, 6-7=994/329, 7-8=913/404
 TOP CHORD
 2-11=480/1019, 10-11=329/894, 9-10=329/894, 8-9=194/2
 BOT CHORD
 4-11=1371/75, 5-11=5263, 6-11=1041/44, 6-9=455/321, 7-9=412/1118
 WEBS

NOTES

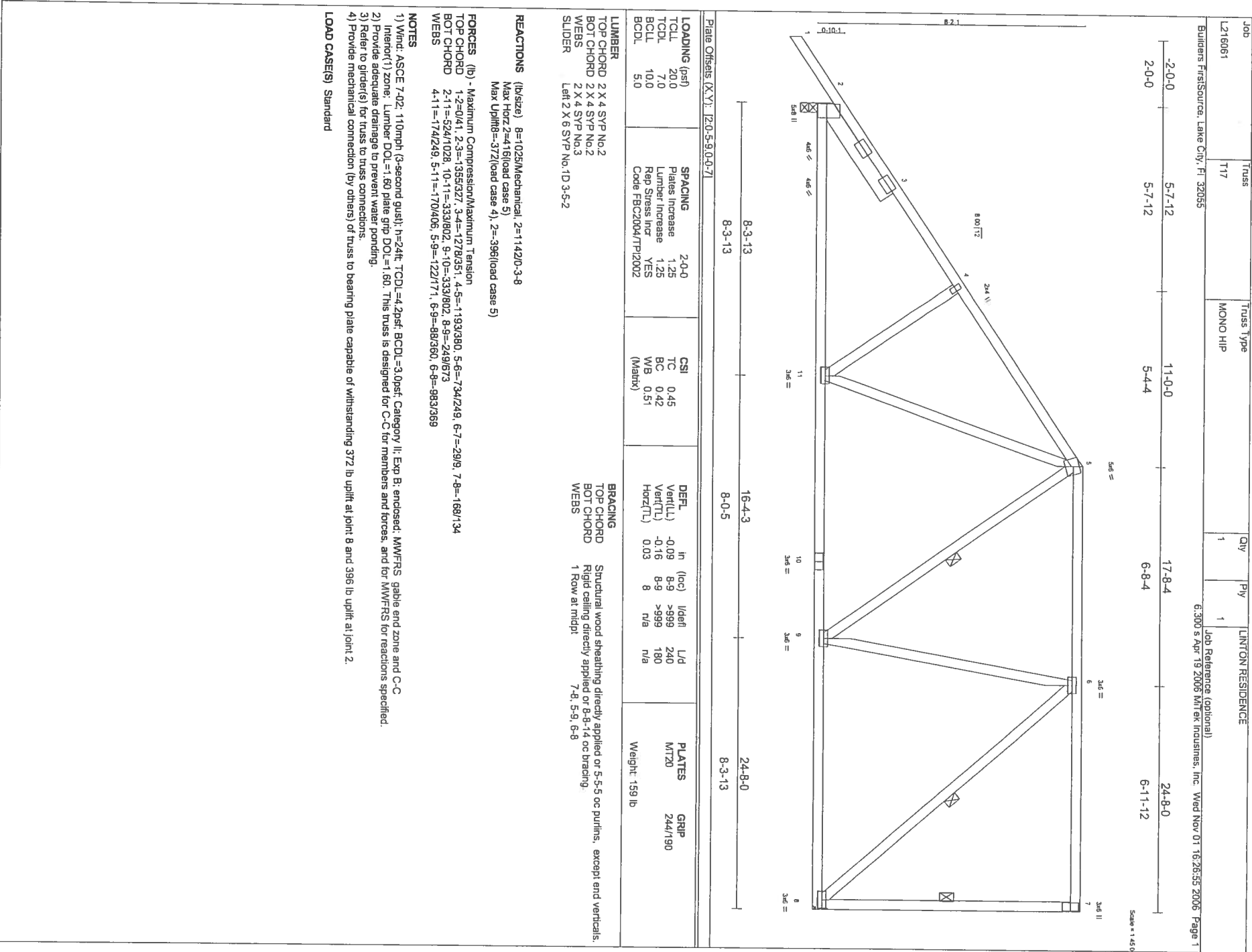
1) Wind: ASCE 7-02, 110mph (3-second gust), $h=24ft$, TCDF=.4, z_{ref} BCD=.3, z_{ref} Cat. I; Exp. B; endoset; MWFRS, gable end zone and C-C Interior(1) zone, Lumber DOL=.60, plate gird DOL=.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 368 lb uplift at joint B and 356 lb uplift at joint 2.

LOAD CASE(S) Standard



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCL	20.0	Plates Increase	1.25	TC	0.45	Ver(1L)	-0.09	B-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Ver(7L)	-0.16	B-9	>999	180		
BCDL	10.0	Rep Stress Incr	YES	WB	0.51	Horz(7L)	0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TP12002		(Mativ)								

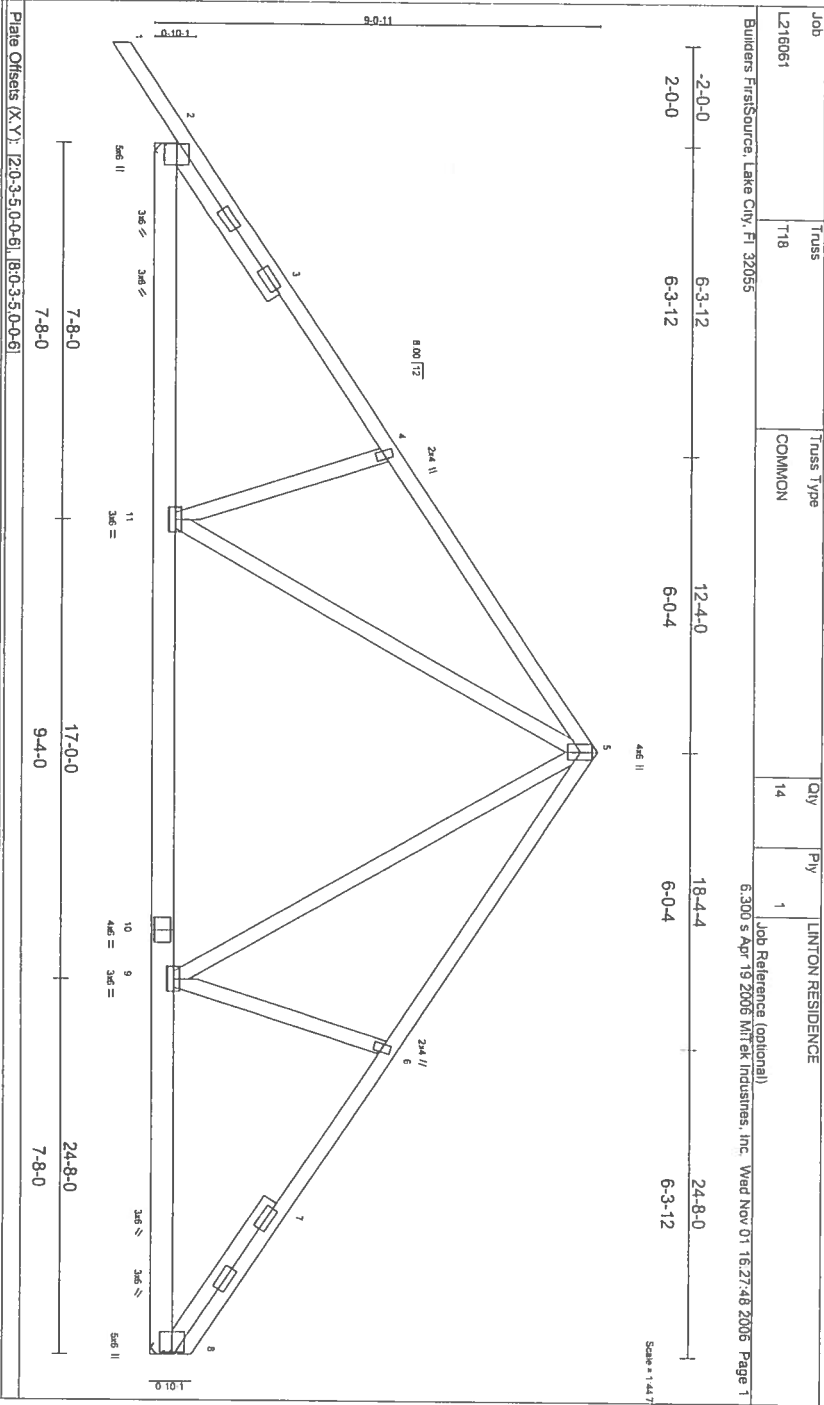
LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 5-5-5 oc purlins, except end verticals.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 8-8-14 oc bracing.
WEBS	2 X 4 SYP No.3	WEBS	1 Row at midpt
SLIDER	Left 2 X 6 SYP No.1D 3-5-2		7-8, 5-9, 6-8

REACTIONS (lb/size) 8=1025/Mechanical 2=11420-3-8
Max Horz 2=416/load case 5)
Max Uplift 8=372/load case 4), 2=-396/load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-1359/327, 3-4=-1278/351, 4-5=-1193/380, 5-6=-734/249, 6-7=-29/9, 7-8=-168/134
BOT CHORD 2-1=-524/1028, 10-11=-333/802, 9-10=-333/802, 8-9=-249/673
WEBS 4-11=-174/249, 5-11=-170/406, 5-9=-122/171, 6-9=-98/360, 6-8=-983/369

NOTES
1) Wind: ASCE 7-02, 110mph (3-second gust); h=24ft, TCDL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS, gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girders(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 372 lb uplift at joint 8 and 396 lb uplift at joint 2.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	2'-0"	CSI	DEFL	in (cc)	l/d	L/d	PLATES	GRP
TCCL 20.0	Plates Increase 1.25		TC 0.30	Vert(L) -0.17	9'-11"	>999	240	MT20	244/190
TCCL 7.0	Lumber Increase 1.25		BC 0.41	Vert(TL) -0.28	9'-11"	>999	180		
BCCL 10.0	Rep Stress Incr YES		WB 0.66	Horz(TL) 0.03	8	n/a	n/a		
BCCL 5.0	Code FBC2004/TP12002		(Matn)					Weight: 158 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.10
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 3-8-13, Right 2 X 4 SYP No.3 3-8-13

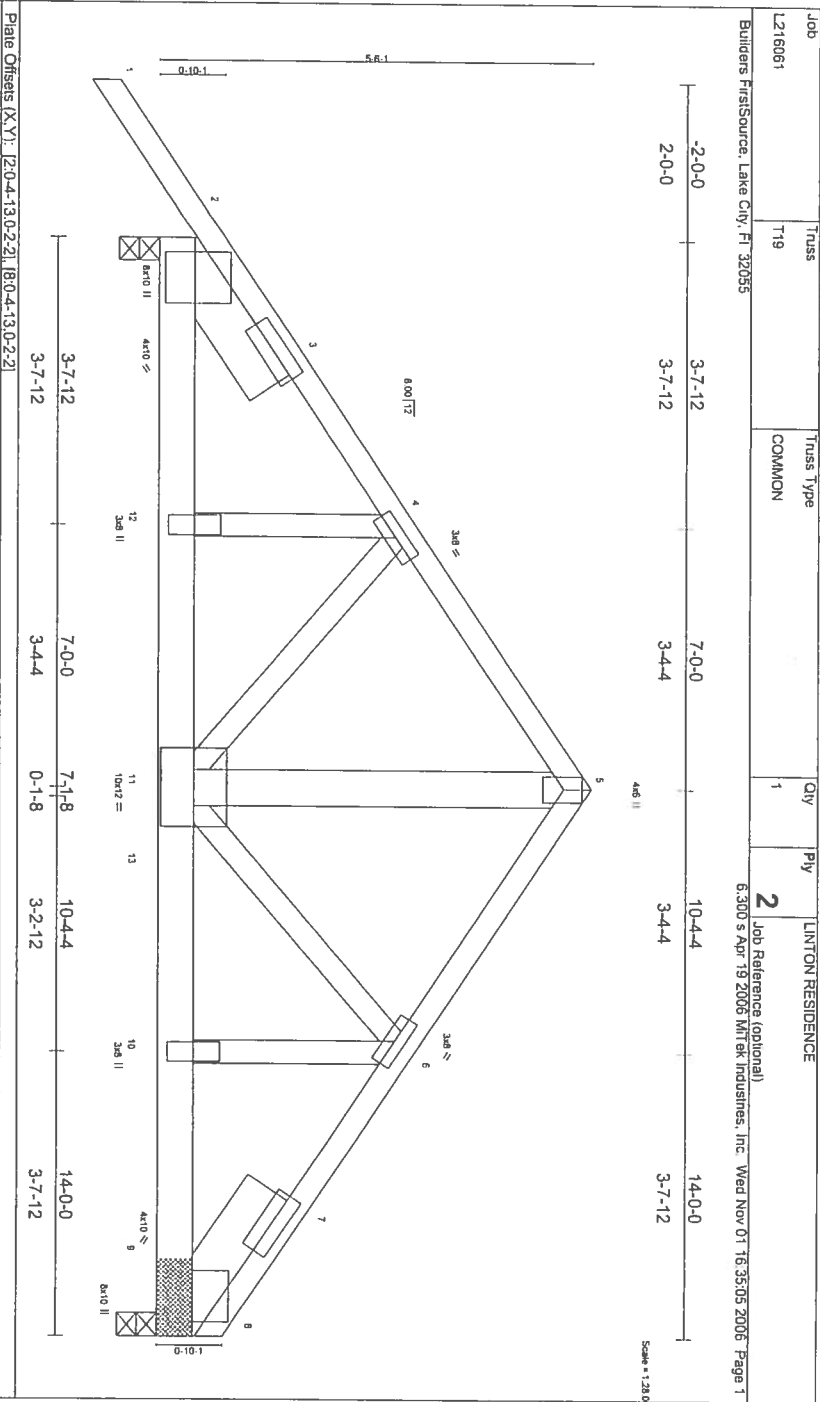
BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=1266/Mechanical, 2=1382/Mechanical
Max Horiz 2=321(load case 4)
Max Uplift 8=413(load case 6), 2=533(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-185/157, 3-4=-1749/584, 4-5=-1707/694, 5-6=-1722/719, 6-7=-1761/607, 7-8=-1842/588
BOT CHORD 2-1=-469/1411, 10-11=-21/973, 9-10=-21/973, 8-9=-378/1425
WEBS 4-11=-218/283, 5-11=-362/837, 5-9=-392/860, 6-9=-226/293

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02, 110mph (3-second gust); h=24ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS, gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Refer to girder(s) for truss to truss connections.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 413 lb uplift at joint 8 and 533 lb uplift at joint 2.
 - 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (psf)
Vert: 1-5=-54, 5-8=-54, 2-11=30, 9-11=80(F=-50), 8-9=-30



LOADING (psf)	SPACING	2'-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRP
TCCL 20.0	Plates increase	1.25	TC 0.77	Vert(L)	-0.07	10-11	>999	MT20	244/190
TCDL 7.0	Lumber increase	1.25	BC 0.65	Vert(TL)	-0.12	10-11	>999		
BCCL 10.0	Rep Stress incr	NO	WB 0.43	Horz(TL)	0.03	8	n/a		
BCDL 5.0	Code FBC2004/FP12002		(Mathty)			n/a	n/a		
								Weight: 220 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *except*
W3 2 X 6 SYP No.1D
SLIDER Left 2 X 8 SYP No.1D 2-1-7, Right 2 X 8 SYP No.1D 2-1-7

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

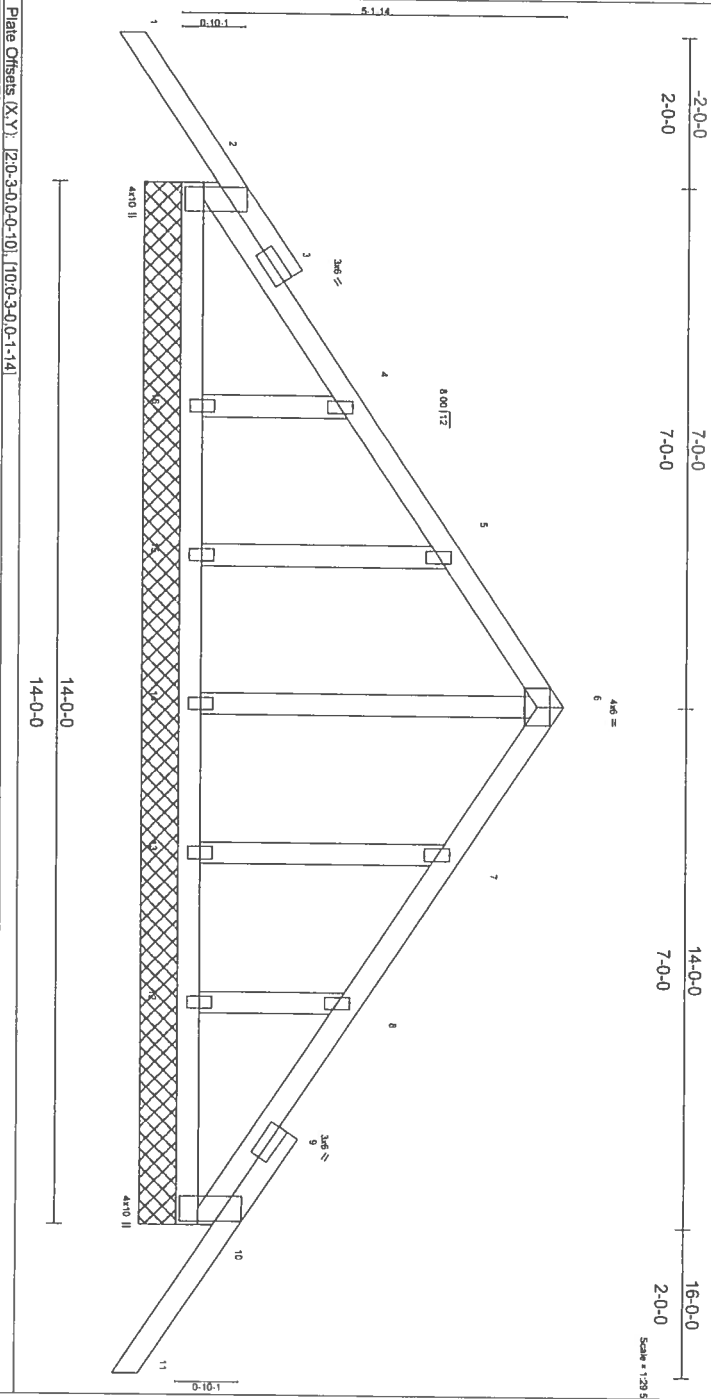
REACTIONS (lb/size) 8=6468/0-3-13 (0-3-8 + bearing block), 2=3733/0-3-8
Max Horz 2=198/lead case 3)
Max Uplift 8=2459/lead case 5), 2=-1466/lead case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=5278/1947, 3-4=5227/1980, 4-5=5577/2155, 5-6=5570/2142, 6-7=-7637/2915, 7-8=-7662/2884
BOT CHORD 2-12=-1528/4160, 11-12=-1528/4160, 11-13=-2270/6144, 10-13=-2270/6144, 9-10=-2270/6144, 8-9=-2270/6144
WEBS 4-12=415/225, 4-11=-309/740, 5-11=-2200/5756, 6-11=-1976/697, 6-10=-1050/2667

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-7-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) 2 X 6 SYP No.1D bearing block 12" long at l. 8 attached to each face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners per block. Bearing is assumed to be SYP.
4) Unbalanced roof live loads have been considered for this design.
5) Wind: ASCE 7-02: 110mph (3-second gust); h=24ft; TCCL=4.2psf; Category II; Exp B, enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate gnp DOL=1.60.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2459 lb uplift at joint 8 and 1466 lb uplift at joint 2.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3915 lb down and 1478 lb up at 7-1-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber increase=1.25, Plate increase=1.25
Uniform Loads (pl)
Vert: 1-5=54, 5-8=54, 2-13=30, 8-13=-864(F=-834)
Concentrated Loads (lb)
Vert: 11=-3915(F)

Job	Truss Type	Qty	Ply	Job Reference (optional)
L216061	T79G	1	1	



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
20.0	Plates increase	0.38	In (loc)	M120	244/190
20.0	Lumber increase	BC 0.06	Vert(L)		
7.0	Rep Stress Incr	WB 0.06	Horz(TL)		
10.0	Code FBC2004/TP12002				
BCDL		(Matnx)		Weight: 80 lb	

TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 cc purfins.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 cc bracing.
OTHERS	2 X 4 SYP No.3		

REACTIONS (lb/size) 2=415/14-0-0, 10=415/14-0-0, 14=196/14-0-0, 15=228/14-0-0, 16=252/14-0-0, 13=228/14-0-0, 12=252/14-0-0

Max Uplink=211(load case 5), 10=233(load case 6), 15=132(load case 5), 16=116(load case 5), 13=137(load case 6), 12=114(load case 6)
Max Grav Z=415(load case 1), 14=196(load case 1), 15=232(load case 9), 16=252(load case 1), 13=232(load case 10), 12=242(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 1-2=3/66, 2-3=14/110, 3-4=112/212, 4-5=87/105, 5-6=85/149, 6-7=85/140, 7-8=87/69, 8-9=4/48, 9-10=141/46, 10-11=3/66
 1-6=221/28, 15-16=221/28, 14-15=221/28, 13-14=221/28, 12-13=221/28, 10-12=221/28
 6-14=135/12, 5-15=175/138, 4-16=183/143, 7-13=175/137, 6-12=183/141

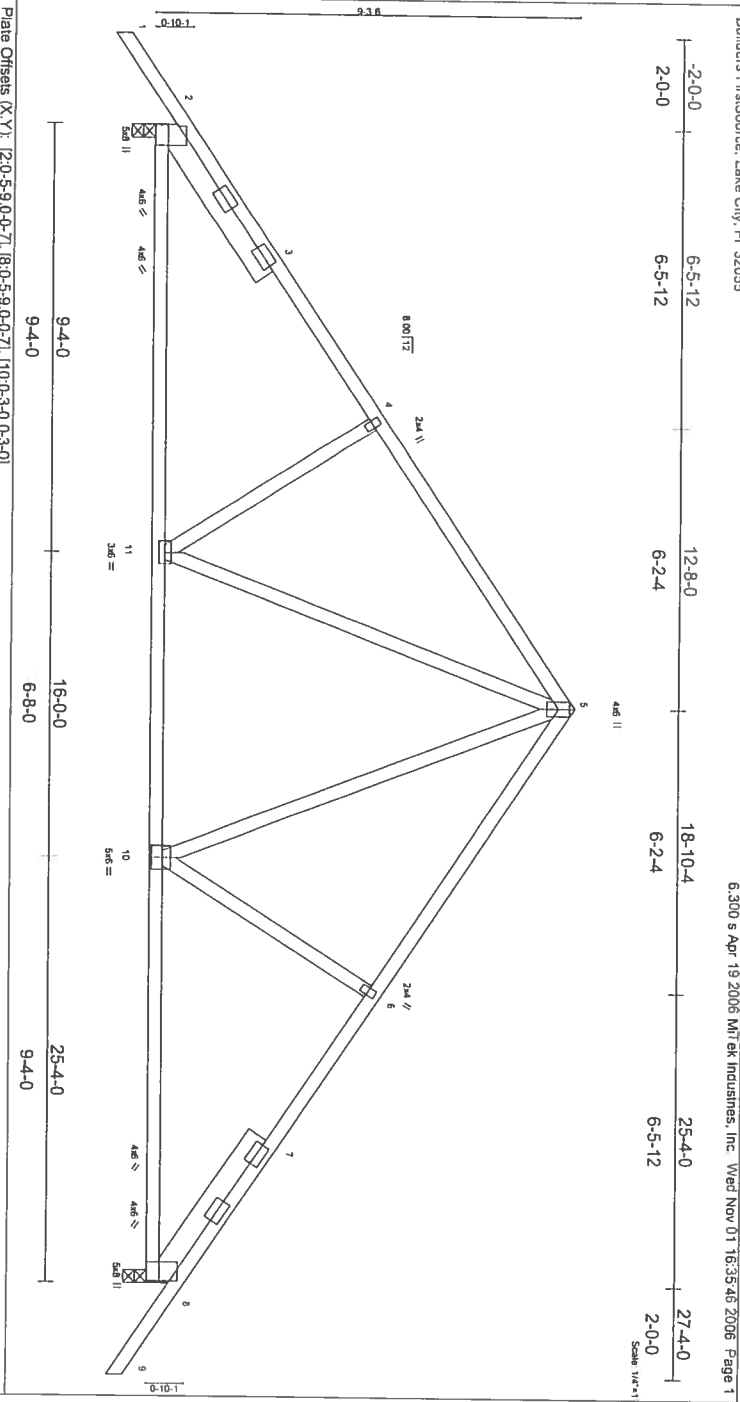
NOTES

- (4) Unanchored rod/tie bars have been considered for this design.
- (5) Wind: ASCE 7-02, 11 (top) C (second gust), $h=24$ ft, $C_{DL}=2.0$, $BCI=3.0$ gust, Category II; Exp B; enclosed; MWFRS; gable end zone and C-C.
- (6) Wind: ASCE 7-02, 11 (top) C (second gust), $h=24$ ft, $C_{DL}=2.0$, $BCI=3.0$ gust, Category II; Exp B; enclosed; MWFRS; gable end zone and C-C.
- (7) Truss designed for wind loads $W=1.60$ plate girth $DOE=1.60$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- (8) Truss designed for wind loads $W=1.60$ plate girth $DOE=1.60$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- (9) All plates are 2x4 M1720 unless noted only. For truss exposed to wind (normal to the face), see Mitrak "Standard Gable End Detail".
- (10) Gable requires continuous bottom chord bearing.
- (11) Gable studs spaced at 24.0-c.
- (12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 211 lb uplift at joint 2, 233 lb uplift at joint 10, 132 lb uplift at joint 15, 116 lb uplift at joint 16, 131 lb uplift at joint 13 and 114 lb uplift at joint 12.

LOAD CASE(S) Standard

- Regular. Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=87, 6-11=87, 2-10=30

Job	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	COMMON	2	1	Job Reference (optional)

[illegible]

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	Structural wood sheathing directly applied or 5-6-11 oc purlins.	
BOT CHORD	2 X 4 SYP No.2	Rigid ceiling directly applied or 10-0-0 oc bracing.	
WEBBS	2 X 4 SYP No.3		
SLIDER	Left: 2 X 6 SYP No.1D 3-11-2. Right: 2 X 6 SYP No.1D 3-11-2.		

REACTIONS

(lb/size) 2=1172/0-3-8, 8=1172/0-3-8
Max Horiz 2=315(load case 3)
Max Uplift2=451(load case 5), 8=451(load case 6)

FORCES (lb): Maximum Compression/Maximum Tension
1-2=20/41, 2-3=1369/408, 3-4=1196/477, 5-6=1196/477, 5-6=1207/437, 7-8=1368/408, 8-9=0/41
TOP CHORD
2-1=1368/408, 10-11=98/761, 8-10=225/1066
SOT CHORD
4-11=286/289, 5-11=215/489, 5-10=215/489, 6-10=286/289
MEBS

NOTES

(1) Unbalanced roof live loads have been considered for this design.

(2) Wind: ASCE 7-02, 110 mph (3-second gust), $h=24$ ft, TCDF = 42nd, BCFI = 3.0psf, Category II, Exp B, enclosed, MWFRS, gable end zone and C-C interior(1) zone. Lumber DOL = 60 plate gip DOL = 60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified

(3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 451 lb uplift at joint 2 and 451 lb uplift at joint 6.

LOAD CASE(S) Standard

LOAD CASE(S) Standard

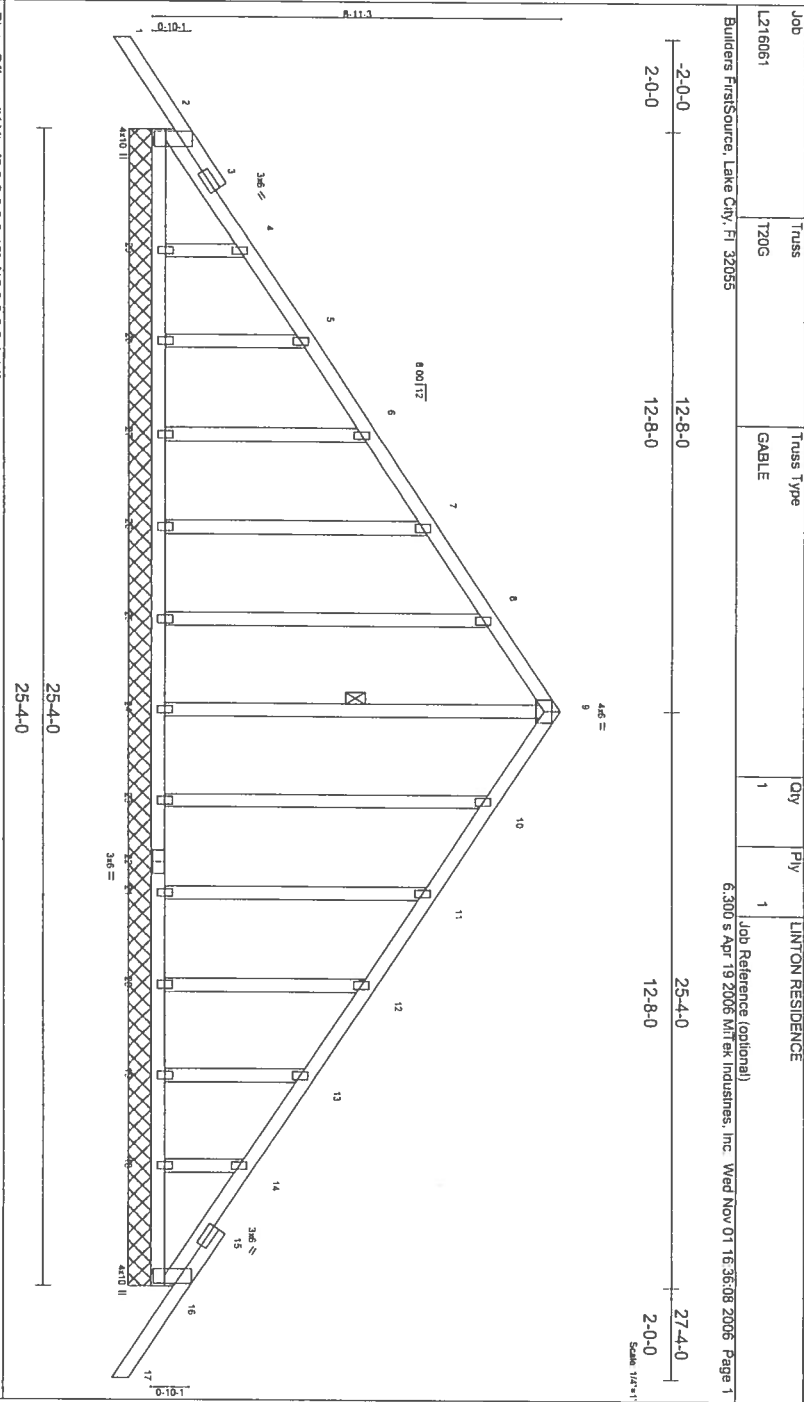


Plate Offsets (X,Y): [2-0-3,0-0-10],[16-0-3,0-0-1-141]									
LOADING (psf)		SPACING		CSI		DEFL		PLATES	
TCCL	20.0	Plates Increase	2-0-0	TC	0.38	Vert(L)	in (loc)	L/d	GRP
TCCL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	17 n/r	120	MIT20
BCCL	10.0	Rep Stress Incr	NO	WB	0.17	Horz(TL)	-0.05 17 n/r	90	244/190
BCCL	5.0	Code FRC2004/FP2002		(Matbty)			0.01 16 n/a	n/a	
LUMBER		BRACING							
TOP CHORD		TOP CHORD							
BOT CHORD		BOT CHORD							
OTHERS		WEBS							
2 X 4 SYP No.2		Structural wood sheathing directly applied or 8-0-0 oc purlins.							
2 X 4 SYP No.3		Rigid ceiling directly applied or 10-0-0 oc bracing.							
		1 Row at midpt 9-24							

REACTIONS (lb/size) 2=403/25-4-0, 16=403/25-4-0, 24=201/25-4-0, 25=233/25-4-0, 26=235/25-4-0, 27=232/25-4-0, 28=240/25-4-0, 29=213/25-4-0, 23=233/25-4-0, 21=235/25-4-0, 20=232/25-4-0, 19=240/25-4-0, 18=213/25-4-0

Max Horiz2=303(load case 3)

Max Uplift2=146(load case 3), 16=167(load case 6), 25=115(load case 5), 28=139(load case 5), 29=101(load case 5), 23=111(load case 6), 21=137(load case 6), 20=125(load case 6), 19=140(load case 6), 18=98(load case 6), 27=232(load case 1), 25=237(load case 9), 26=236(load case 1), 27=233(load case 9), 28=240(load case 1), 29=213(load case 9), 23=237(load case 10), 21=235(load case 1), 20=233(load case 10), 19=240(load case 1), 18=213(load case 10)

Max Grav2=403(load case 1), 16=403(load case 1), 24=201(load case 1), 25=237(load case 9), 26=236(load case 9), 28=240(load case 1), 29=213(load case 9), 23=237(load case 10), 21=235(load case 1), 20=233(load case 10), 19=240(load case 1), 18=213(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

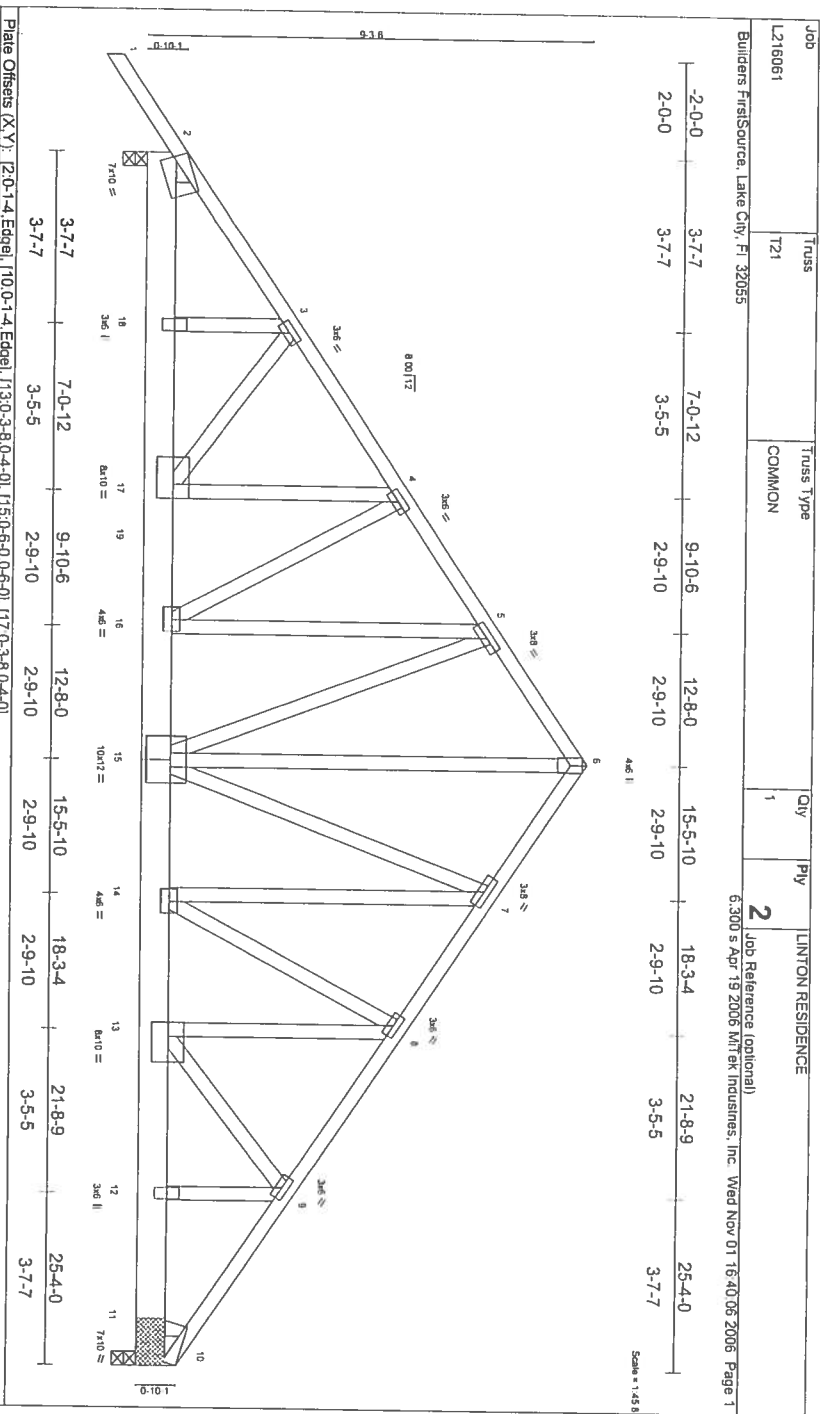
TOP CHORD 1-2=306, 2-3=250/191, 3-4=248/192, 4-5=189/168, 5-6=142/166, 6-7=92/156, 7-8=80/212, 8-9=79/251, 9-10=79/242, 10-11=80/178, 11-12=79/101, 12-13=80/56, 13-14=80/58, 14-15=135/79, 15-16=136/77, 16-17=306

BOT CHORD 2-29=46/214, 28-29=46/214, 27-28=46/214, 26-27=46/214, 25-26=46/214, 24-25=46/214, 23-24=46/214, 22-23=46/214, 21-22=46/214, 20-21=46/214, 19-20=46/214, 18-19=46/214, 17-18=46/214

9-24=165/2, 8-25=177/127, 7-26=175/147, 6-27=173/139, 5-28=179/145, 4-29=156/128, 10-23=177/123, 11-21=175/149, 12-20=173/139, 13-19=179/146, 14-18=156/125

WEBS

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind - ASCE 7-02, 110mph (3-second gust); h=24ft; TCCL=4-20sf; BCCL=3-0psf; Category II; Exp B; enclosed; MWFRS; gable end zone and C-C interior(1) zone; Lumber DOL=1.60 plate grp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Mittek "Standard Gable End Detail"
 - 4) All plates are 2x4 MIT20 unless otherwise indicated.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) Gable studs spaced at 2-0-0 oc.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 2, 167 lb uplift at joint 16, 115 lb uplift at joint 25, 136 lb uplift at joint 26, 126 lb uplift at joint 27, 139 lb uplift at joint 28, 101 lb uplift at joint 29, 111 lb uplift at joint 23, 137 lb uplift at joint 21, 125 lb uplift at joint 20, 140 lb uplift at joint 19 and 96 lb uplift at joint 18.
- LOAD CASE(S)** Standard
- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
- Uniform Loads (plf)
- Vert: 1-9=87, 9-17=87, 2-16=30



LOADING (psf)		SPACING		CSI		DEF.		PLATES	
TCL	20.0	Plates increase	1.25	TC	0.55	Vert(L)	-0.16	L/d	GRIP
BCDL	7.0	Lumber increase	1.25	BC	0.65	Vert(T)	-0.25	L/d	MT20
BCDL	10.0	Rep Stress Incr	NO	WB	0.63	Horz(T)	0.06	n/a	24d/190
BCDL	5.0	Code FBC2004/TP2002		(Many)				n/a	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.10
WEBS 2 X 4 SYP No.3 "Except"
W7 2 X 4 SYP No.2
WEDGE Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

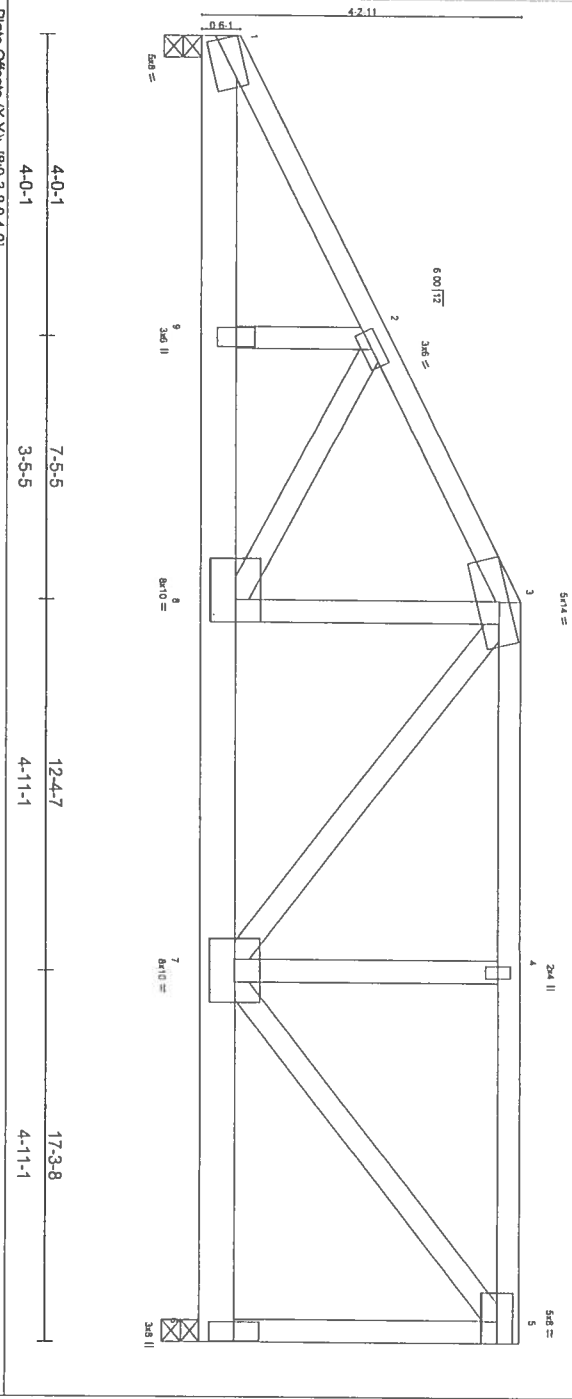
REACTIONS (lb/size) 10-69850-4-2 (0-3-8 + bearing block), 2=56050-3-8
Max Horz 2=328(load case 3)
Max Uplift 10=2646(load case 5), 2=2171(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=8316/3058, 3-4=8451/3202, 4-5=7285/2831, 5-6=8171/2473, 6-7=8170/2465, 7-8=7230/2821, 8-9=8386/3217, 9-10=9575/3620
BOT CHORD 2-18=2431/6451, 17-18=2431/6451, 17-19=2800/7000, 16-19=2800/7000, 15-16=2182/6008, 14-15=2133/5961, 13-14=2522/5947, 12-13=2760/7452, 11-12=2760/7452, 10-11=2760/7452
WEBS 3-18=1341/140, 3-17=329/751, 4-17=835/2252, 4-16=2056/863, 5-16=1049/2705, 5-15=2305/976, 6-15=2577/6485, 7-15=2180/956, 7-14=1028/2561, 8-14=2043/891, 8-13=868/2231, 9-13=673/348, 9-12=572/1542

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated
3) 2 X 8 SYP No. 10 bearing block 12" long at Jt. 10 attached to each face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners per block. Bearing is assumed to be SYP.
4) Unbalanced roof live loads have been considered for this design.
5) Wind: ASCE 7-02; 110mph (3-second gust); h=24ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MMFRS gable end zone; Lumber DOL=1.80 plate grip DOL=1.60.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2646 lb uplift at joint 10 and 2171 lb uplift at joint 2.
7) Girder carries tie-in span(s); 24-8-0 from 8-0-0 to 25-4-0
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2295 lb down and 867 lb up at 7-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber increase=1.25; Plate increase=1.25
Uniform Loads (plf)
Vert: 1-6=54, 6-10=54, 2-19=30, 10-19=500(F=470)
Concentrated Loads (lb)
Vert: 17=2295(F)

Job	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	722	MONO HIP	1	2	Job Reference (optional)
Builders FirstSource, Lake City, Fl 32055					6:30U s Apr 19 2006 Mike Industries, Inc Wed Nov 01 16:41:02 2006 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	0.85	DEF.	In	(oc)	Wdth	L/d	PLATES	GRIP
TCLL 20.0	Plates increase	1.25	TC	0.85	Vert(L)	-0.12	7-8	>999	240	M120	244/190
TCLL 7.0	Lumber increase	1.25	BC	0.67	Vert(TL)	-0.19	7-8	>999	180		
BCCL 10.0	Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.04	6	n/a	n/a		
BCCL 5.0	Code FBC2004/TP12002		(Matth)							Weight: 217 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

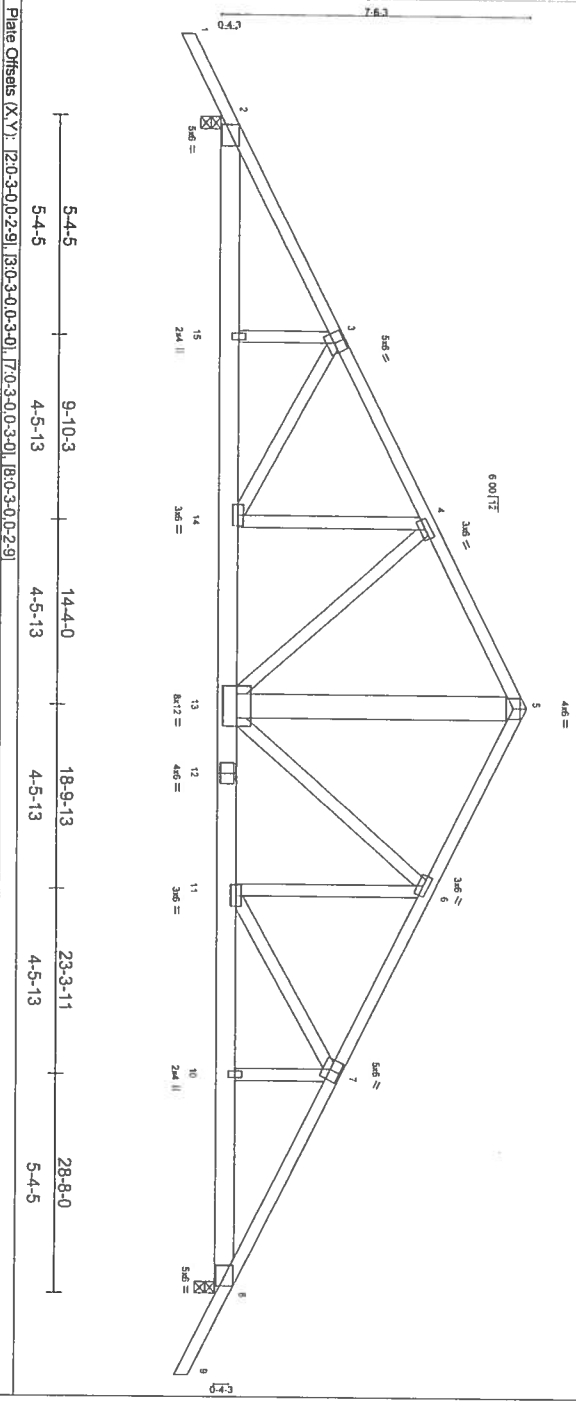
REACTIONS (lb/size) 1=50780;3-8, 6=52770;3-8
Max Horiz 1=178(lload case 4)
Max Uplift1=191(lload case 4)
Max 2056(lload case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8457/3181, 2-3=-667/92553, 3-4=-5020/1952, 4-5=-5019/1952, 5-6=-4164/1687
BOT CHORD 1-9=-2913/7386, 6-9=-2913/7386, 7-8=-2373/6086, 6-7=-68/152
WEBS 2-9=-806/1742, 2-8=-1659/682, 3-8=-1279/3443, 3-7=-1354/587, 4-7=-409/360, 5-7=-2420/6252

- NOTES**
- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 - 2) All loads are considered equally applied to all piles, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - 3) Wind: ASCE 7-02: 110mph (3-second gust); h=24ft; TCLL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate gfp DOL=1.60
 - 4) Provide adequate drainage to prevent water ponding.
 - 5) Provide mechanical connection (by others) of truss to
 - 6) Girdler carries (lb-in span(s)): 24-8-0 from 0-0-0 to 17-3-8
 - 7) Girdler carries hip end with 0-0-0 right side setback, 7-5-5 left side setback, and 5-1-0 end setback.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 373 lb down and 192 lb up at 7-5-5 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber increase=1.25, Plate increase=1.25
Uniform Loads (pl)
Vert: 1-3=-34, 3-5=-92(F=-38), 1-8=-500(F=-470), 6-8=-521(F=-491)
Concentrated Loads (lb)
Vert: 8=-373(F)

JOB	Truss	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	123	COMMON	2	1	
Builder's FirstSource, Lake City, Fl 32055					6,300 S Apr 19 2006 Mitek Industries, Inc. Wed Nov 01 16:41:35 2006 Page 1
					Job Reference (optional)
-2-0-0	5-4-5	9-10-3	14-4-0	18-9-13	23-3-11
2-0-0	5-4-5	4-5-13	4-5-13	4-5-13	28-8-0
					30-8-0
					2-0-0
					Scale = 1/32 = 8



LOADING (psf)	SPACING	2-0-0	CSI	DEF.	in (occ)	l/def	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.33	Vert(L)	-0.16	13	>999	M120	244/190
TCCL 7.0	Lumber Increase	1.25	BC 0.39	Vert(TL)	-0.25	13	>999		
BCCL 10.0	Rep Stress Incr	NO	WB 0.40	Horz(TL)	0.07	8	n/a		
BCCL 5.0	Code FBC2004/TP12002		(Mativ)				n/a		Weight: 195 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.10
WEBS 2 X 4 SYP No.3 Except
W5 2 X 8 SYP No.10

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-1-11 oc bracing.

REACTIONS (lb/size) 2=1834/0-3-8, 8=1834/0-3-8
Max Horiz 2=141(load case 4), 8=783(load case 5)
Max Uplift 2=783(load case 5)

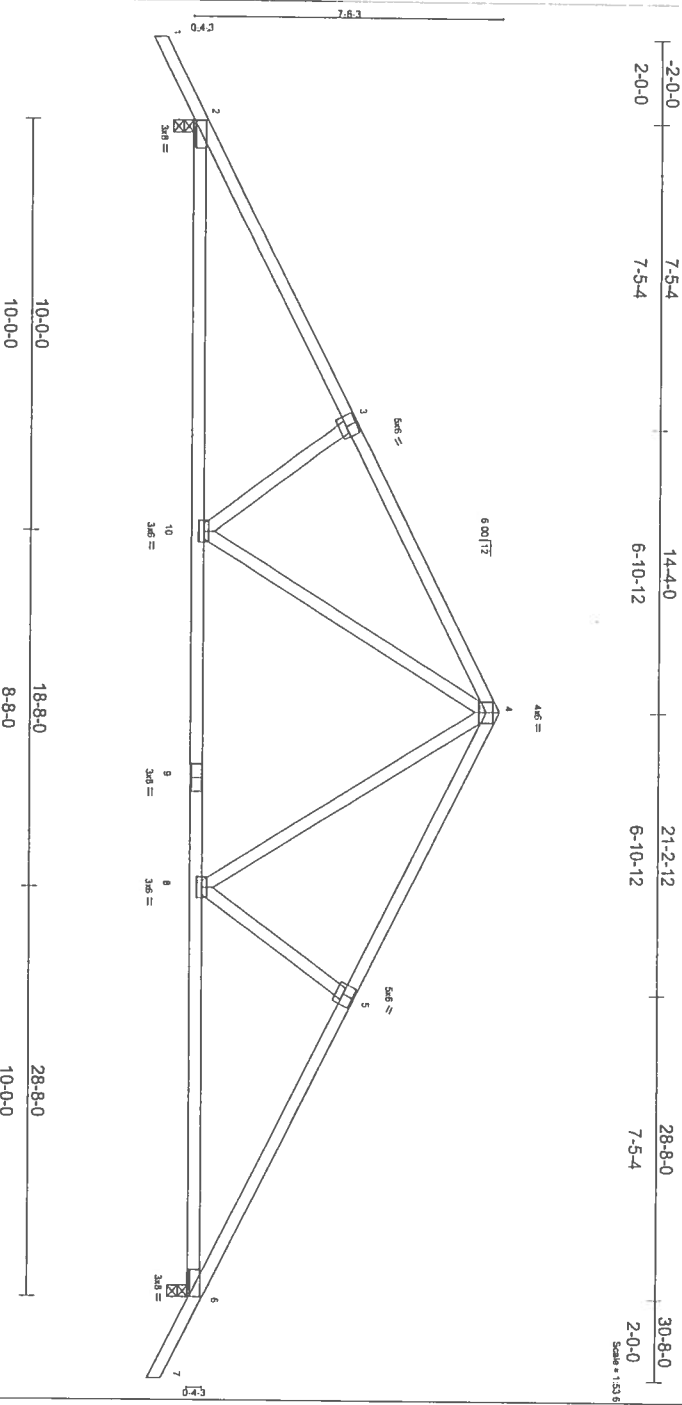
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=3344/1230, 3-4=2862/1186, 4-5=2855/1106, 5-6=2855/1106, 6-7=2862/1186, 7-8=3344/1232, 8-9=0/51
BOT CHORD 2-15=-1097/2924, 14-15=-1099/2921, 13-14=-952/2801, 12-13=873/2601, 11-12=-967/2921, 8-10=-966/2924
WEBS 3-15=0/122, 3-14=-379/173, 4-14=-77343, 4-13=-546/274, 5-13=-832/1984, 6-13=-546/274, 6-11=-77343, 7-11=-379/174, 7-10=0/122

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02, 110mph (3-second gust); h=24ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 783 lb uplift at joint 2 and 783 lb uplift at joint 8.
4) Girder carries hip end with 14-4-0 right side setback, 14-4-0 left side setback, and 4-1-8 and setback.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1054 lb down and 542 lb up at 14-4-0 on bottom chord. The design/section of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (lf)
Vert: F-3=-54, S-9=-54, 2-8=-30
Concentrated Loads (lb)
Vert: 13=-1054(F)

Job	Truss Type	Qty	Ply	LINTON RESIDENCE
L216061	COMMON	10	1	Job Reference (optional)

6,300 s Apr 19 2006 Mtl ex Industries, Inc Wed Nov 01 16:42:08 2006 Page 1



LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	TCCL	Plates Increase	1.25	TC	0.44	In	(loc)	I/del		M/20	244/190
7.0	7.0	Lumber Increase	1.25	BC	0.92	Vert(L)	6-10	>866	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.42	Vert(TL)	>540	180			
BCOL	5.0	Code FPGC2004/TP12002		(Matrix)		Hz(TL)	6	n/a	n/a		
										Weight: 134 lb	

TOP CHORD	TOP CHORD
2 X 4 SYP No.2	Structural wood sheathing directly applied or 3-5-11 oc purlins.
2 X 4 SYP No.2 "except"	Rigid ceiling directly applied or 2-2-0c bracing.
B1 2 X 4 SYP No.1D	
2 X 4 SYP No.3	
WEBS	

REACTIONS
(ib/size) 2=1524/0-3-8, 6=1524/0-3-8
Max Horz 2=139(load case 5)
Max Uplift2=594(load case 5), 6=594(load case 6)

FORCES (b) - Maximum Compression/Maximum Tension
 1-2=0/47, 2-3=2536829, 3-4=23118/0, 4-5=-2208610, 5-6=-2536828, 6-7=0/47
 2-10=7132185, 9-10=-3621500, 8-9=-3621500, 6-8=-5842163
 3-10=3593308, 4-10=-3569959, 4-8=-3359564, 5-8=-3613308
 WEIBS

NOTES

(1) Unbalanced root live loads have been considered for this design.
(2) Wind: MSFC F-02: 110mph (3-second gust), $h \geq 24'$, TCDD-4-20st.
Interact(1) zone: Lumber DOL = 1.60 plate grip DOL = 1.60. This truss is designed for C-C for members and braces, and for WMFRS for reactions specified.
(3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 594 k (uplift at joint 2 and 594 k uplift at joint 6).
In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

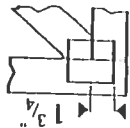
LOAD CASE(S) Standard

1) Regular. Lumber increase=1.25, Plate increase=1.25
Uniform Loads (plf)
Vert: 1-4=54, 4-7=54, 2-10=30, 8-10=80(F=50), 6-8=30

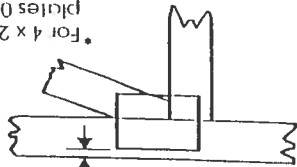
Symbols

PLATE LOCATION AND ORIENTATION

- Center plate on joint unless x, y offsets are indicated.
- Dimensions are in 1/16-in. increments.
- Apply plates to both sides of truss and securely seat.



0-1/16"



- For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.



- This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mittek 20/20 software or upon request.

PLATE SIZE

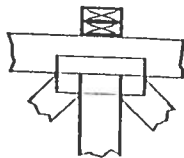
4 X 4

- The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.



- Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



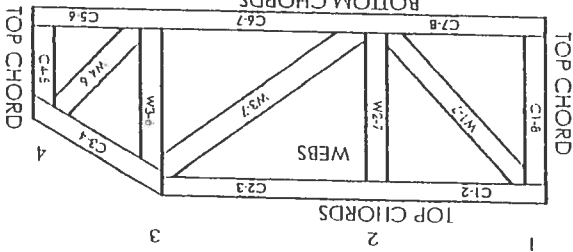
- Indicates location where bearings (supports) occur. Icons vary but location section indicates joint number where bearings occur.

Industry Standards:

- ANSI/FP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
- CSB 89: Design Standard for Bracing.
- BCS11: Building Component Safety Information, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8
dimensions shown in 1/16-in. increments



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.
CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A



Mittek Engineering Reference Sheet: Mill 7473



General Safety Notes

Failure to Follow Could Cause Properly Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.

2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.

3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.

4. Cut members to bear tightly against each other.

5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/FP11.

6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/FP11.

7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.

9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.

10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.

11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.

12. Top chords must be sheathed or purlins provided at spacing shown on design.

13. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted.

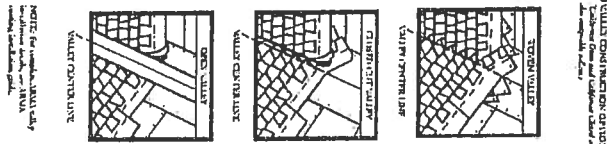
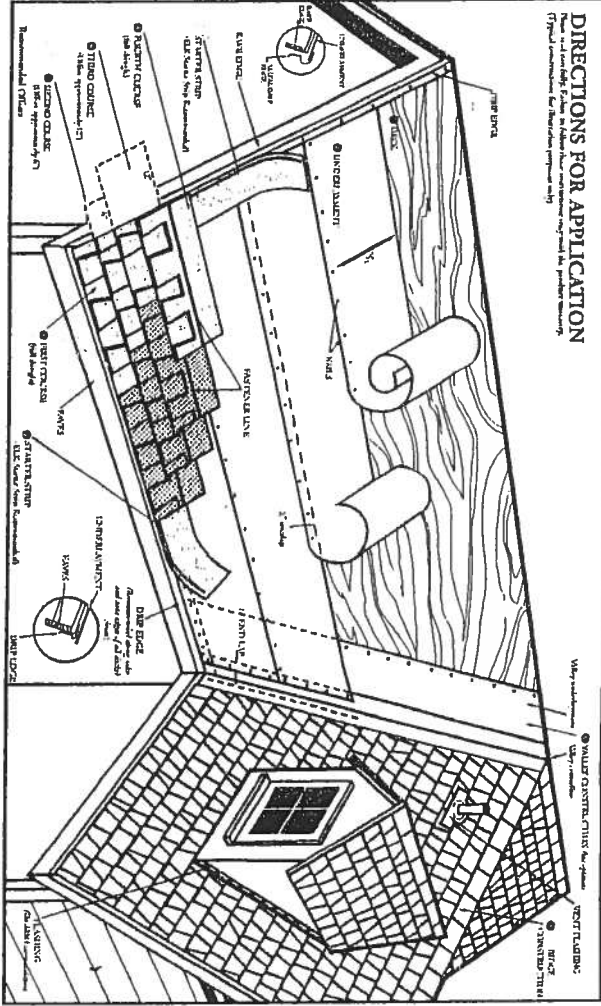
14. Connections not shown are the responsibility of others.

15. Do not cut or alter truss member or plate without prior approval of a professional engineer.

16. Install and load vertically unless indicated otherwise.

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DIRECTIONS FOR APPLICATION
(Typical construction for the following products only)



DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void Elk's product warranty. In some states, the building codes may require additional application methods, so methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All joints should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/4" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt, Elk Versabridge® or self-adhering underlayment is also acceptable. Cover drip edge at eaves only.

For low slope (2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 18". Begin by fastening a 18" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (USE A ROOFING CONTRACTOR, REFER TO APPRA MAINTAIN OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing or no less than 50 pounds over the 18" underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Technical Services Department for application specifications over other decks and other slopes.

STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR THE HEADLAP OF A STRIP SHINGLE WITH THE ADHESIVE STRIP POSITIONED AT THE EAVE EDGE. With at least 7" trimmed from the end of the first shingle, start at the eave edge overlapping the eave and rake edges 1/2" to 3/4" (Reason 2 from the lower edge and 1" from each side).

FIRST COURSE

Start at the eave and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof.

SECOND COURSE

Offset the second course of shingles with respect to the first by approximately 6". Other offsets are approved if greater than 4".

THIRD COURSE

Offset the next course by 6" with respect to the second course, or consistent with the original offset.

FOURTH COURSE

Start at the eave and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES

Repeat application as shown for second, third, and fourth courses. Do not rock shingles sideways up the roof. Offsets may be adjusted around valleys and penetrations.

VALLEY CONSTRUCTION

Open, woven and closed curvalley are acceptable when applied by Asphalt Roofing Manufacturers Association (ARMA) recommended procedures. For metal valleys use 36" wide vertical underlayment prior to applying metal flashing (seam edge with nail). No nails are to be within 6" of valley center.

EDGE CONSTRUCTION

For ridge construction Elk recommends Class "A" Z-Ridge or Seal-A-Ridge® with formula FX® or RidgeCrest® with FX (See ridge package for installation instructions). Vertical RidgeCrest or 3-tab shingles are also approved.

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions. Using the fastener line as a reference, nail or staple the shingle in the double thickness common board area. For shingles without a fastener line, nails or staples must be placed between and/or in the sealant deck.

NAILS: Corrosive resistant 3/16" head, minimum 1 1/2-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for roofs over. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only 3/16" ring shank nails are allowed to be used from the eave's edge to a point up the roof line to past the outside wall line. 1" ring shank nails allowed for re-roof STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 18/16". Note: All properly adjusted staple gun can result in raised staples. This can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less. This product meets the requirements of the IRC 2003 code when fastened with 4 nails.

MAINTAIN APPLICATIONS

Correct fastening is critical to the performance of the roof. For slopes exceeding 60° for 21/12 use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge. The remaining four fasteners equally spaced along the length of the double thickness (lanterned) area. Only fastening methods according to the above instructions are acceptable.

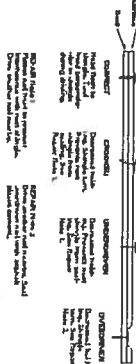
LIMITED WIND WARRANTY

For a Limited Wind Warranty, all Prestique and Raised Profile® shingles must be applied with 4 properly placed fasteners, or in the case of standard applications, 5 properly placed fasteners per shingle.

For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 50 MPH for Prestique I, shingles must be applied with 5 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4" of an inch.

HEEP STOP BLOW-OFFS AND CAIL BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (lanterned) area of the shingle. Nails or staples must be placed along – and through – the fastener line or on products without fastener lines, nail or staple between and in the sealant deck. CAUTION: Do not use fastener line for shingle alignment.



Refer to local codes which in some areas may require specific application techniques and that Elk has specified. All Prestique and Raised Profile® shingles have a UL-60 Wind Resistance Rating when applied in accordance with these instructions using nails or staples as required as well as new construction.

CAUTION TO WHOLESALER: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.



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60RS

Dec. 28, 2001 5:03PM PREMODOR DICKSON 315 445 7200

3085 P. 12/5/2



MIAMI-DADE COUNTY, FLORIDA
METRO-DOME FLAGLER BUILDING

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Premdor Entry Systems
One Premdor Drive
Dickson, TN 37055

BUILDING CODE COMPLIANCE OFFICE
METRO-DOME FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1565
TEL: 375-2901 FAX (005) 375-2908

OFFICE FOR LICENSING SECTION
TEL: 375-52327 FAX (001) 375-2555

CONTRACT FOR ENFORCEMENT DIVISION
TEL: 375-2966 FAX (005) 375-2908

PRODUCT CONTROL DIVISION
TEL: 375-2901 FAX (005) 375-6000

Your application for Notice of Acceptance (NOA) of:
Energy SE Double Door w/sidelite-inswing - 8'0" In a Wood Frame
under Chapter 8 of the Code of Miami-Dade County governing the use of Alternative Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This NOA shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at any time from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined by BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

ACCEPTANCE NO.: 01-1031.06

EXPIRES: 11/05/2006

Renee Buchanan
Chief of Building Code Compliance Division

THIS IS THE COVERSHEET. SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL
CONDITIONS
BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Miami-Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

APPROVED: 12/11/2001

Dec-28-2001 5:04PM PREMDOR DICKSON 6'5 446 7229

#885 P. 13/52

Premdor Entry SystemsACCEPTANCE No.: 01-1031.06APPROVED: December 11, 2001EXPIRES: November 5, 2006NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS

1. SCOPE

- 1.1 This renews Notice of Acceptance (NOA) No. 00-0720.10, which was issued on November 09, 2000. It renews the approval of a residential insulated steel door, as described in Section 2 of this NOA, designed to comply with the South Florida Building Code (SFBC), 1994 Edition for Miami-Dade County, for the locations where the pressure requirements, as determined by SFBC Chapter 23, do not exceed the Design Pressure Rating values indicated in the approved drawings.

2. PRODUCT DESCRIPTION

- 2.1 The Series "Entergy" Inswinging Opaque Double Residential Insulated Steel Doors (Metal Edge) with Sidelites 8' 0" High - Impact Resistant Door Slab Only and its components shall be constructed in strict compliance with the following document: Drawing No 31-1034-EM-1, Sheets 1 through 6 of 6, titled "Premdor (Entergy Metal Edge) Double Door w/ Sidelites in Wood Frame w/ Bumper Threshold - 8' 0" Height (Inswing)", prepared by manufacturer, dated 6/15/98 and revised on 7/27/01, bearing the Miami-Dade County Product Control renewal stamp with the NOA number and expiration date by the Miami-Dade County Product Control Division. This document shall hereinafter be referred to as the approved drawings.

3. LIMITATIONS

- 3.1 This approval applies to single unit applications of pair of doors and single door with sidelites, as shown in approved drawings. Single door units shall include all components described in the active leaf of this approval.

- 3.2 Unit shall be installed only at locations protected by a canopy or overhang such that the angle between the edge of canopy or overhang to sill is less than 45 degrees. Unless unit is installed in non-habitable areas where the unit and the area are designed to accept water infiltration.

4. INSTALLATION

- 4.1 The residential insulated steel door and its components shall be installed in strict compliance with the approved drawings.

4.2 Hurricane protection system (shutters):

Door Slab: The installation of this unit will not require a hurricane protective system.

Sidelites: The installation of these units will require a hurricane protective system.

5. LABELING

- 5.1 Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved".

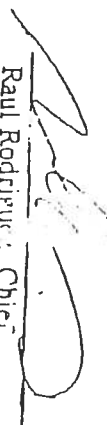
6. BUILDING PERMIT REQUIREMENTS

- 6.1 Application for building permit shall be accompanied by copies of the following:

6.1.1 This Notice of Acceptance.

6.1.2 Duplicate copies of the approved drawings, as identified in Section 2 of this Notice of Acceptance, clearly marked to show the components selected for the proposed installation.

6.1.3 Any other documents required by the Building Official or the South Florida Building Code (SFBC) in order to properly evaluate the installation of this system.


Raul Rodriguez, Chief
Product Control Division

Dec. 28, 2001 5:14PM PREMDOR DICKSON 615 446 7229

+885 2. 14/52

Premdor Entry Systems

ACCEPTANCE No.:

01-1031.06

APPROVED:

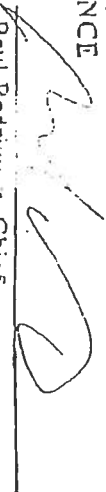
December 11, 2001

EXPIRES:

November 5, 2006

NOTICE OF ACCEPTANCE: STANDARD CONDITIONS

1. Renewal of this Acceptance (approval) shall be considered after a renewal application has been filed and the original submitted documentation, including test supporting data, engineering documents, are no older than eight (8) years.
2. Any and all approved products shall be permanently labeled with the manufacturer's name, city, state, and the following statement: "Miami-Dade County Product Control Approved", or as specifically stated in the specific conditions of this Acceptance.
3. Renewals of Acceptance will not be considered if:
 - a) There has been a change in the South Florida Building Code affecting the evaluation of this product and the product is not in compliance with the code changes;
 - b) The product is no longer the same product (identical) as the one originally approved;
 - c) If the Acceptance holder has not complied with all the requirements of this acceptance, including the correct installation of the product;
 - d) The engineer who originally prepared, signed and sealed the required documentation initially submitted is no longer practicing the engineering profession.
4. Any revision or change in the materials, use, and/or manufacture of the product or process shall automatically be cause for termination of this Acceptance, unless prior written approval has been requested (through the filing of a revision application with appropriate fees) and granted by this office.
5. Any of the following shall also be grounds for removal of this Acceptance:
 - a) Unsatisfactory performance of this product or process.
 - b) Misuse of this Acceptance as an endorsement of any product, for sales, advertising or any other purpose.
6. The Notice of Acceptance number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the Notice of Acceptance is displayed, then it shall be done in its entirety.
7. A copy of this Acceptance as well as approved drawings and other documents, where it applies, shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at all time. The engineer need not resal the copies.
8. Failure to comply with any section of this Acceptance shall be cause for termination and removal of Acceptance.
9. This Notice of Acceptance consists of pages 1, 2 and this last page 3.

END OF THIS ACCEPTANCE
Raul Rodriguez, Chief
Product Control Division

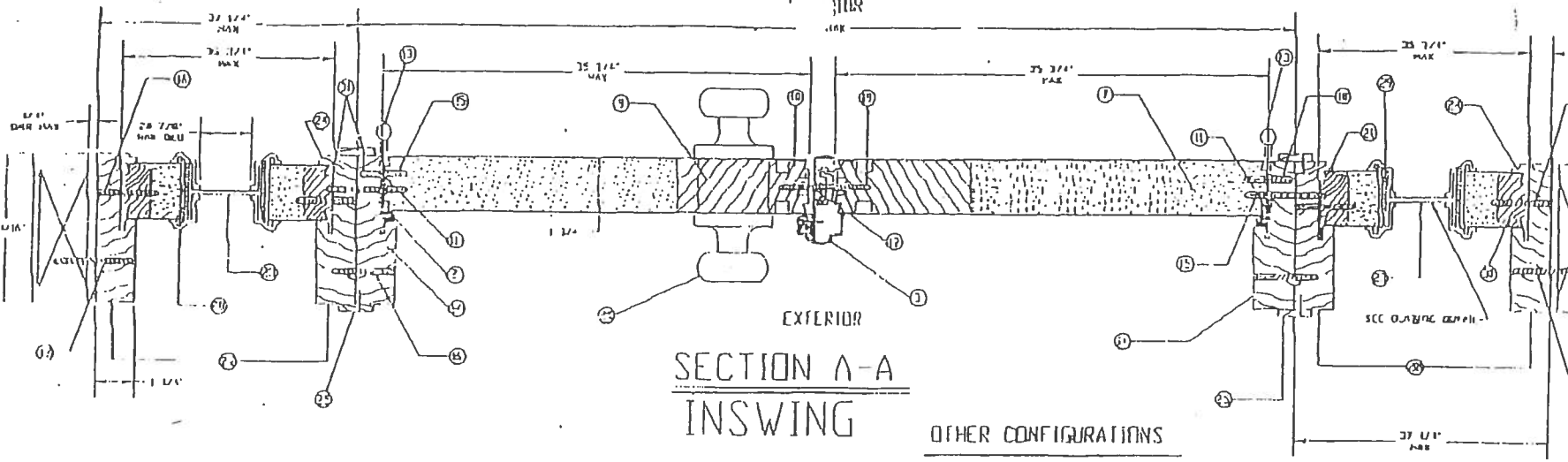
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DOORFAB

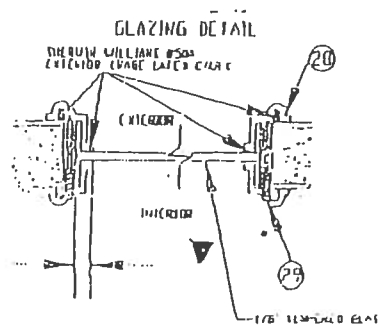
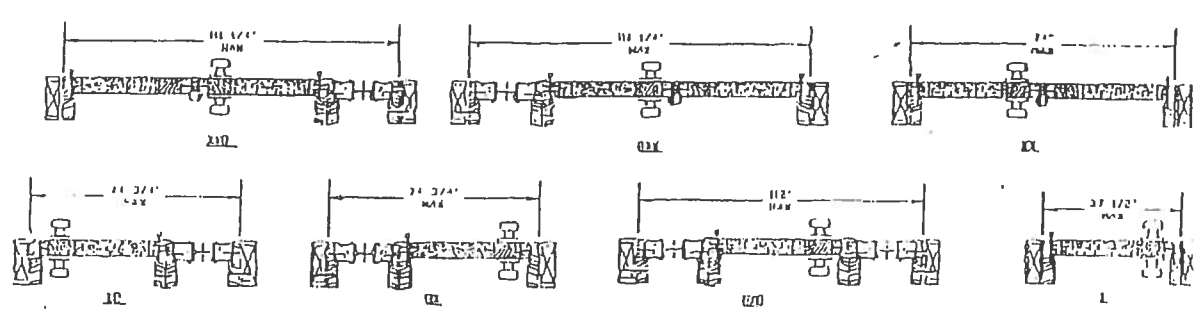
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OTHER CONFIGURATIONS

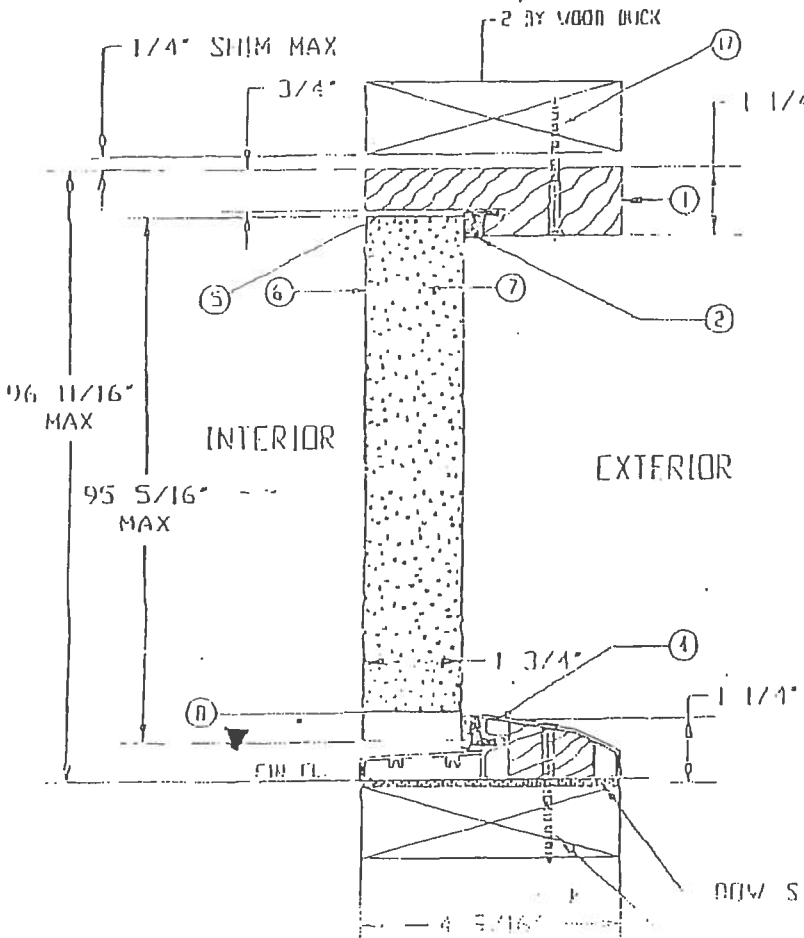


PRODUCT RENEWED

ACCEPTANCE NO. 61-1031-01
EXPIRATION DATE: January 5, 2002
By: [Signature]
REGIONAL CONSULTANT
DOOR ENTRY SYSTEMS

1	DOOR ENTRY SYSTEMS	2	DOOR ENTRY SYSTEMS
3	DOOR ENTRY SYSTEMS	4	DOOR ENTRY SYSTEMS
5	DOOR ENTRY SYSTEMS	6	DOOR ENTRY SYSTEMS
7	DOOR ENTRY SYSTEMS	8	DOOR ENTRY SYSTEMS
9	DOOR ENTRY SYSTEMS	10	DOOR ENTRY SYSTEMS
11	DOOR ENTRY SYSTEMS	12	DOOR ENTRY SYSTEMS
13	DOOR ENTRY SYSTEMS	14	DOOR ENTRY SYSTEMS
15	DOOR ENTRY SYSTEMS	16	DOOR ENTRY SYSTEMS
17	DOOR ENTRY SYSTEMS	18	DOOR ENTRY SYSTEMS
19	DOOR ENTRY SYSTEMS	20	DOOR ENTRY SYSTEMS
21	DOOR ENTRY SYSTEMS	22	DOOR ENTRY SYSTEMS
23	DOOR ENTRY SYSTEMS	24	DOOR ENTRY SYSTEMS
25	DOOR ENTRY SYSTEMS	26	DOOR ENTRY SYSTEMS

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SECTION B-B

DOOR SILICONE 11995

PRODUCT RENEWAL
EXPIRATION DATE 12/31/06
BY: [Signature]
IF YOU DO NOT HAVE A SIGNATURE, THE PRODUCT IS NOT VALID.

PREMOOR ENTRY SYSTEMS
111 E. CHURCH
MILLSBORO, DE 19966

D	DATE OF THIS MODIFICATION	DATE	BY
1	DOOR TEST RESULTS	DATE	BY
2	DOOR TEST RESULTS	DATE	BY
3	DOOR TEST RESULTS	DATE	BY
4	DOOR TEST RESULTS	DATE	BY

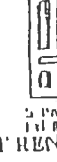
31-1034-EM 1
SHEET 3 OF 3

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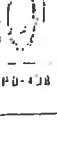
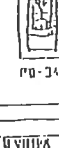
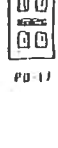
ITEM NO.	DESCRIPTION	PART NUMBER	QUANTITY
1	WOOD HEAD JAMB	EX-14	1 1/4" X 4 9/16" HGL. TO BE PAIR, IN EQUIV. HGL.
2	COMPRESSION WEATHERSTRIP	EX-25	LOCKSEALER BRAND LUTHERAL 9550 (PREMOOR)
3	ALUMINUM ASTROGAL	EX-12	PREMOOR BRAND OR EQUIV. HGL. - 3/16" ALUMINUM ASS'Y
4	ALUMINUM-SUPER THRESHOLD	EX-15	PREMOOR BRAND OR EQUIV. HGL. - 1 1/4" X 4 9/16"
5	HIP CHANNEL	EX-08	PREMOOR BRAND - 1 1/16" - 20 GA ST111
6	STEEL SCREW	EX-01	1/4" X 1/2" HGL. TO BE PAIR, IN EQUIV. HGL.
7	POLYURETHANE FOAM CORE	EX-02	2455 FOAM - DENSITY 20 TO 25 LBS/FT ³
8	ROLLING CHAMFER	EX-07	PREMOOR BRAND - 1 1/16" - 20 GA ST111
9	WOOD LOCK BLOCK	EX-05	1/4" X 1 1/2" HGL. TO BE PAIR, OR EQUIV. HGL.
10	STRIKE STILE	EX-06	PREMOOR BRAND - 1 1/16" - 20 GA ST111
11	HINGE STILE	EX-05	PREMOOR BRAND - 1 1/16" - 20 GA ST111
12	LOCK PLATE FILLER PLATE	EX-13	PREMOOR BRAND - 1/8" THICK - 1 1/4" X 1 1/2" HGL.
13	6"x4" HINGE	EX-15	HINGE BRAND HINGE OR EQUIV. HGL. TO BE PAIR, IN EQUIV. HGL.
14	WOOD HINGE JAMB	EX-17	1 1/4" X 4 9/16" HGL. TO BE PAIR, IN EQUIV. HGL.
15	410-24 X 1/2" F.H.W.S.		(4) SCREWS PER HINGE INTO JAMB
16	410 X 2" F.H.W.S.		(4) SCREWS PER HINGE INTO JAMB
17	410 X 2" F.H.W.S.		(4) SCREWS PER HINGE INTO JAMB
18	410 X 2" F.H.W.S.		(4) SCREWS PER HINGE INTO JAMB
19	410 X 2" F.H.W.S.		(4) SCREWS PER HINGE INTO JAMB
20	LOCKSET		REFER TO ELEVATION VIEW, FOR # OF SCREWS WITH HINGE
21	210 X 1 3/4" F.H.W.S.		(2) SCREWS PER HINGE INTO JAMB
22	WOOD SIDELITE JAMB	EX-10	1 1/4" X 4 9/16" HGL. TO BE PAIR, IN EQUIV. HGL.
23	22" X 00" SINGLE PANEL GLASS	EX-19	TEMPERED GLASS IN POLYURETHANE FRAME - 1/8" GLASS - 1/8" GLASS - 1/8" GLASS
24	STAINLESS STEEL WOOD	EX-20	5/16" X 1/2" HGL. TO BE PAIR, OR EQUIV. HGL.
25	WOOD CASING	EX-21	1/2" X 1" HGL. TO BE PAIR, OR EQUIV. HGL.
26	WOOD SIDELITE HEAD JAMB	EX-22	1 1/4" X 4 9/16" HGL. TO BE PAIR, OR EQUIV. HGL.
27	WOOD SIDELITE BASE	EX-23	1 1/4" X 4 9/16" HGL. TO BE PAIR, OR EQUIV. HGL.
28	POLYPROPYLENE LITE FRAME	EX-1043, OR 2	1/4" Polypropylene by DM
29	26 X 1 1/2" PAN HEAD SCREWS		10 PER FRAME TO BE PAIR, OR EQUIV. HGL.
30	STAINLESS STEEL STILES	EX-26	1/2" X 1 1/2" HGL. TO BE PAIR, OR EQUIV. HGL.
31	PIN NAIL		10 PER FRAME TO BE PAIR, OR EQUIV. HGL.

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5/16
MAX



36"
MAX



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