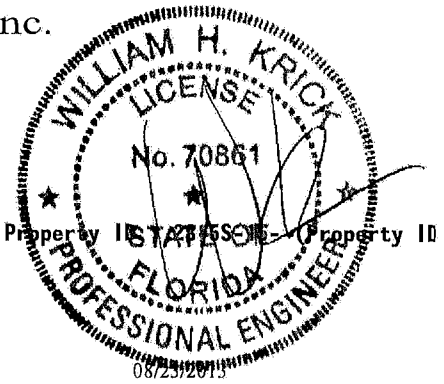


ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1UZ1487-Z0123140332



Truss Fabricator **Anderson Truss Company**
Job Identification **13-209A--Erkinger Home Builders Williamson Residence -- Property ID 13235001-- Property ID 39**
Truss Count **39**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Com/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 12.03.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of the seal date per section 61G15-31.003(5a) of the FAC**
Address **Roof - 37.0 PSF @ 1.25 Duration**
Minimum Design Loads **Floor - N/A**
Wind - 120 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

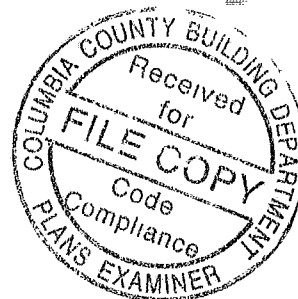
William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-CNNAILSP-

#	Ref	Description	Drawing#	Date
1	29238-A1	49'2" Stepdow	13235028	08/23/13
2	29239-A11	49'2" Specia	13235011	08/23/13
3	29240-A2	49'2" Stepdow	13235029	08/23/13
4	29241-A3	49'2" Stepdow	13235016	08/23/13
5	29242-A4	49'2" Stepdow	13235001	08/23/13
6	29243-A5	49'2" Special	13235022	08/23/13
7	29244-A6	49'2" Special	13235023	08/23/13
8	29245-A7	49'2" Special	13235024	08/23/13
9	29246-A8	49'2" Special	13235021	08/23/13
10	29247-A9	49'2" Special	13235012	08/23/13
11	29248-A10	49'2" Specia	13235013	08/23/13
12	29249--CJ1	1' Jack	13235017	08/23/13
13	29250--CJ3	3' Jack	13235007	08/23/13
14	29251--CJ5	5' Jack	13235003	08/23/13
15	29252--CJ5A	5' Jack	13235014	08/23/13
16	29253-EJ3	2'10" End Ja	13235015	08/23/13
17	29254-EJ36	2'10" End J	13235016	08/23/13
18	29255--EJ7	7' End Jack	13235026	08/23/13
19	29256-H11	49'2" Stepdo	13235006	08/23/13
20	29257-H13	49'2" Stepdo	13235008	08/23/13
21	29258-H15	49'2" Stepdo	13235009	08/23/13
22	29259-H17	49'2" Stepdo	13235010	08/23/13
23	29260-H19	49'2" Stepdo	13235011	08/23/13
24	29261-H21	49'2" Stepdo	13235012	08/23/13
25	29262-H7	49'2" Stepdow	13235020	08/23/13
26	29263-H7A	36'9" Stepdo	13235015	08/23/13
27	29264-H8A	36'9" Stepdo	13235013	08/23/13
28	29265-H9	49'2" Stepdow	13235005	08/23/13
29	29266-HJ3	4'0"1 Hip Ja	13235017	08/23/13
30	29267-HJ7	9'10"13 Hip	13235030	08/23/13
31	29268-HJ7A	9'10"13 Hip	13235018	08/23/13
32	29269-PBA1	9'9"15 Comm	13235027	08/23/13
33	29270-PBA2	9'9"15 Step	13235004	08/23/13
34	29271-PBA4	15'1"14 Ste	13235014	08/23/13
35	29272-PBA5	15'1"14 Ste	13235025	08/23/13
36	29273-PBA6	15'1"14 Ste	13235018	08/23/13

#	Ref	Description	Drawing#	Date
37	29274-PBA7	15'1"14 Spe	13235019	08/23/13
38	29275-PBA8	15'5"10 Spe	13235002	08/23/13
39	29276-PBA9	6'3"14 Step	13235031	08/23/13



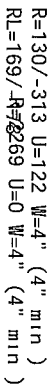
Negative reaction(s) of -312# MAX (See below) from a non-wind load case requires uplift connection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3.5 psf, wind BC DL=5 0 psf GCPI(+/-)=0 18

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit. FBC2010Com/TP1-2007(STD)

$$FT/RT=10\%(0\%)/0(0)$$

JTW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

WILLIAM H. KRICK
JAN 10 1963
U.S. DEPT. OF JUSTICE

QTY:9 FL/-/3/-/-/R/-

Scale = .125"/Ft.

TC LL	20.0 PSF	REF R487-- 29238
TC DL	7.0 PSF	DATE 08/23/13
BC DL	10.0 PSF	DRW HCUSR487 13235028
BC LL	0 0 PSF	HC-ENG WHK/MMHK
TOT. LD	37.0 PSF	SEON- 311389
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1UZ1487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR
(stepdown Hip)

Negative reaction(s) of -303# MAX (See below) from a non-wind load case requires uplift connection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GcP (+/-)=0 18

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



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Design Crit	FBC2010Com/TP1-2007(STD) FT/RT=10%(0%)/0(0)
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Scale = .125"/Ft.

STATE OF

****IMPORTANT**** **WARNING** **READ AND FOLLOW ALL NOTES ON THIS SHEET**
PRIORITY THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trustees require extreme care in fabricating and installing piping, installing and bracing to follow the latest edition of OSHA Building Component Safety Information on by TPI and WTCO. 1. All bracing shall be prior to performing these functions. Installers shall provide temporary bracing and bracing shall have a property attached of 1/4" x 1/4" x 1/4" properly attached structural members and bracing shall have bracing installed per BS21 sections B3, B7 or B10 as applicable.

ITW Build-Right Components Group, Inc. (ITWBCG) shall not be responsible for any deviation from any failure to build the structure. The structure shall be built in accordance with the design of the bracing of trustees. Apply places to each piece of trustee and position as shown on the drawing. Details unless noted otherwise. Refer to draws per 160A-Z for standard place positions. A drawing on or cover page 1 at the drawing and draws acceptance of professional engineer responsible by the design shop. The submittal and use of this design for any responsibility by of the building design per 160A/171 1 Sec 2. For more information see general notes page 171-B03. www.itwbuild.com www.tpi.com www.wtco.com www.sciindustry.com

ITW Build-Right Components Group, Inc. (ITWBCG) shall not be responsible for any deviation from any failure to build the structure. The structure shall be built in accordance with the design of the bracing of trustees. Apply places to each piece of trustee and position as shown on the drawing. Details unless noted otherwise. Refer to draws per 160A-Z for standard place positions. A drawing on or cover page 1 at the drawing and draws acceptance of professional engineer responsible by the design shop. The submittal and use of this design for any responsibility by of the building design per 160A/171 1 Sec 2. For more information see general notes page 171-B03. www.itwbuild.com www.tpi.com www.wtco.com www.sciindustry.com

TC LL	20.0 PSF	REF	R487-- 29240
TC DL	7 0 PSF	DATE	08/23/13
BC DL	10.0 PSF	DRW	HCSUR487 13235029
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37 0 PSF	SEQN-	317390
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	1UZ1487_Z01

(13-209A--Erkinger Home Builders Williamson Residence -- Property ID 23-55-15- - A3 49 2' (Seepdown Hip)

Top chord 2x4 SP #1_12A
Bot chord 2x4 SP #1_12A
Webs 2x4 SP #3_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

MMFRS loads based on trusses located at least 15.00 ft from roof edge

Negative reaction(s) of -321# MAX (See below) from a non-wind load case requires uplift connection

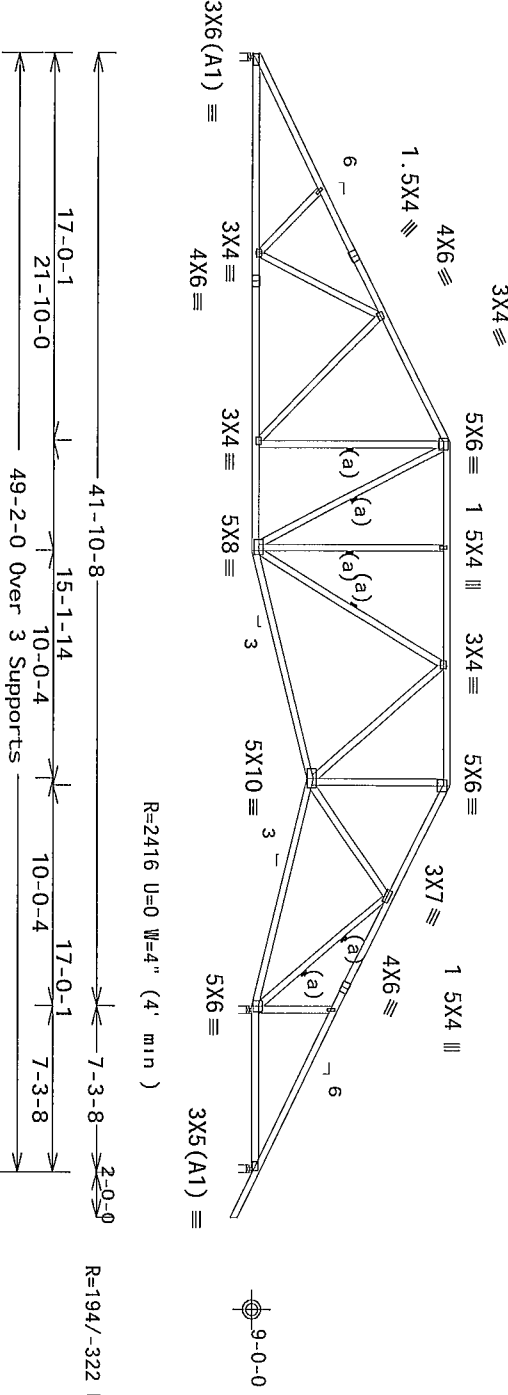
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf GCP1(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below



PLT TYP. Wave

Design Crit. FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

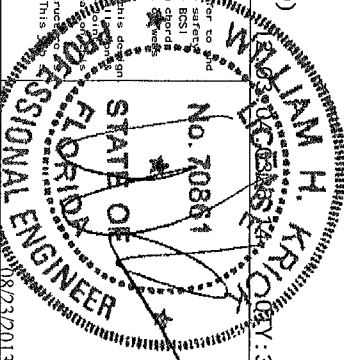
ALPINE

ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0278

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety) Information on by TPI and WCA. Fabricate trusses prior to performing these functions. Installers shall provide temporary bracing per BCSI. Trusses shall have a properly attached r/gid ceiling. Locations shown for permanent lateral bracing shall have bracing installed per BCSI sections B3, B7 or B10 as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any delay arising from this design or any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installing, bracing of trusses. Apply plates to each face of truss and position as shown above and on the drawing. Details unless noted otherwise. Refer to drawings 180A-Z for standard plate positions. A section drawing of cover plate listing this drawing. No cases acceptance of process shall be made. This drawing is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2. For more information see the general notes page. ITW-BCG www.tbog.com TPI www.tpi.org WCA www.wca-industry.com ICC www.iccsafe.org



FL/-/3/-/-/R/-	Scale = .125"/Ft.
TC LL 20.0 PSF	REF R487-- 29241
TC DL 7.0 PSF	DATE 08/23/13
BC DL 10.0 PSF	DRW HCUSR487 13235016
BC LL 0.0 PSF	HC-ENG WHK/WHK
TOT LD. 37.0 PSF	SEON- 311391
DUR.FAC. 1.25	
SPACING 24.0"	JREF- 1U21487_201

TC LL	20.0 PSF	REF	R487-- 29242
TC DL	7 0 PSF	DATE	08/23/13
BC DL	10 0 PSF	DRW	HCSR487 13235001
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37.0 PSF	SEQN-	317392
DUR.FAC.	1 25		
SPACING	24 0"	JREF-	1U21487_Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

MMFRS loads based on trusses located at least 15.00 ft from roof edge

Negative reaction(s) of -354# MAX (See below) from a non-wind load case requires uplift connection

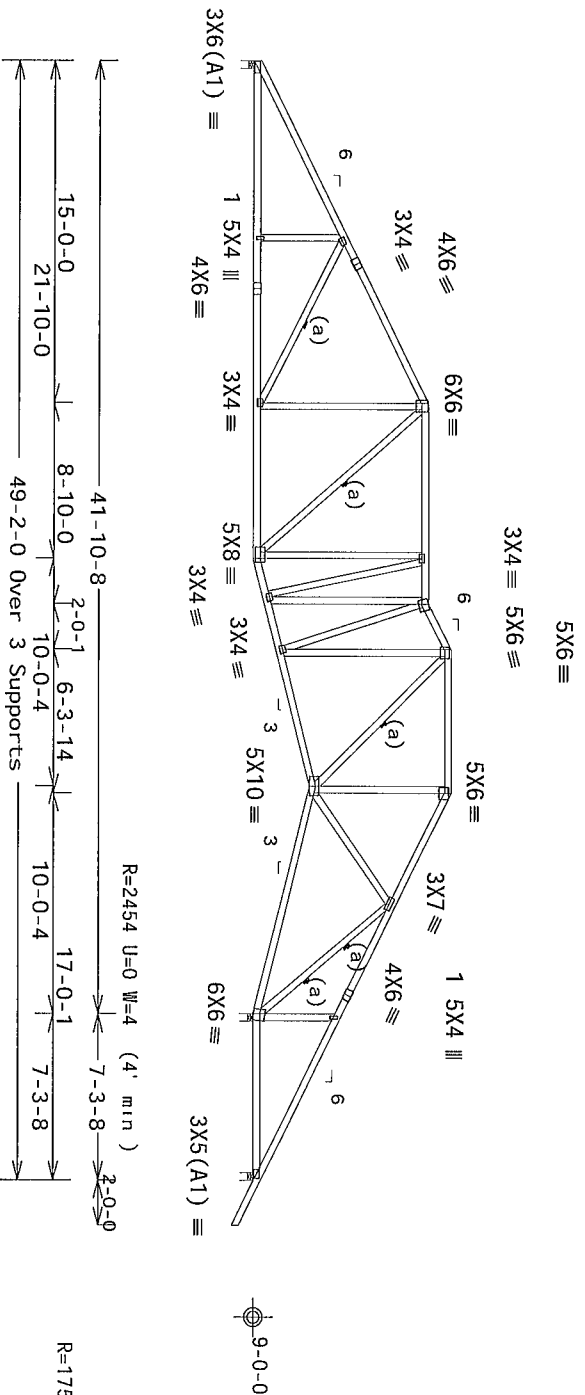
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf GCPI(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below



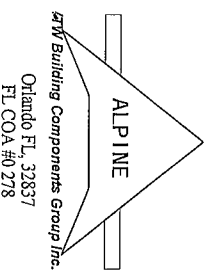
R=1503 U=0 W=4" (4' min)
RL=156/-148

PLT TYP. Wave

Design Crit FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

William H. Kricorian
Professional Engineer
No. 70864
FLORIDA
08/23/2013

FL/-/3/-/-/R/- Scale = .125"/Ft.



ALPINE Building Components Group Inc.
Orlando, FL 32837
FL COA #0278

Professional Engineer
No. 70864
FLORIDA
08/23/2013

TC LL	20.0 PSF	REF	R487-- 29243
IC DL	7.0 PSF	DATE	08/23/13
BC DL	10.0 PSF	DRN	HCUSR487 13235022
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	311393
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1U21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MEMBER
(Special)

Negative reaction(s) of -378# MAX (See below) from a non-wind load case requires uplift connection

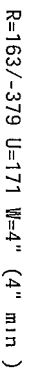
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT I, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf Gcpl(+/-)=0.18

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

CC:

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



R=2483 U=0 W=4" (4 min)

7-3

17-2-0

10-0-4

7-3

Design Crit	FBC2010Com/TP1-2007 (STD)	FT/RT=10%(0%)/0(0)
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Figure 1. FI/-/3/-/-/B/-/-

$$Scale = 125''/E +$$

JTW Building Components Group Inc.

*****WARNING*** READ AND FOLLOW ALL NOTES ON THIS SHEET!**
*****IMPORTANT*** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS**

Trustees require extreme care in fabricating handling, shipping, installing and bracing to follow the latest ed of code of BCS (Building Component Safety Informant on by TPI and WTCO) Unit cases not or performing these functions. Installers shall provide temporary bracing and bracing as needed otherwise as top chord shaft have properly attached structural sheathing and bolts shall have a properly attached set of girders. Locations shown for permanent lateral restraint shall have been braced per detailed per BCSi sections 83, 87 or 810 as applicable.

TPI Building Component Group Inc. (TBMGS) shall not be responsible for any new action for any TPI building to build the trust, in conformance with AS/NZS/TPI 1 or for handling any piping bracing of trustees. Apply plates to each truss and post on as shown above and on drawing or cover plate. Use the same design as shown (BOM) for standard plate posts only. The drawing or cover plate is not to be drawing. The sub title and use of this design go for the responsibility of the building design group per AS/NZS/TPI 1 Sec 2. For more information see general notes page TPI-806 www.tpiinc.com TPI www.tpiinc.org WTCO www.sbcindustry.com www.tpiinc.org www.sbcindustry.com www.tpiinc.org www.sbcindustry.com

No. 70861
 STATE OF
 FLORIDA
 PROFESSIONAL ENGINEER
 This is to certify that the above named person is duly licensed as a Professional Engineer in the State of Florida.
 Witness my hand and seal of office this 1st day of March, 1913.
 J. M. [Signature]
 State Engineer

08/22/2013

TC LL	20.0 PSF	REF	R487--	29244
IC DL	7.0 PSF	DATE	08/23/13	
BC DL	10.0 PSF	DRW	HCUSR487	13235023
BC LL	0 0 PSF	HC-ENG	WHK/WHK	
TOT. LD	37.0 PSF	SEQN-	311394	
DUR. FAC.	1 25			
SPACING	24 0"	JREF-	1UZI487	Z01

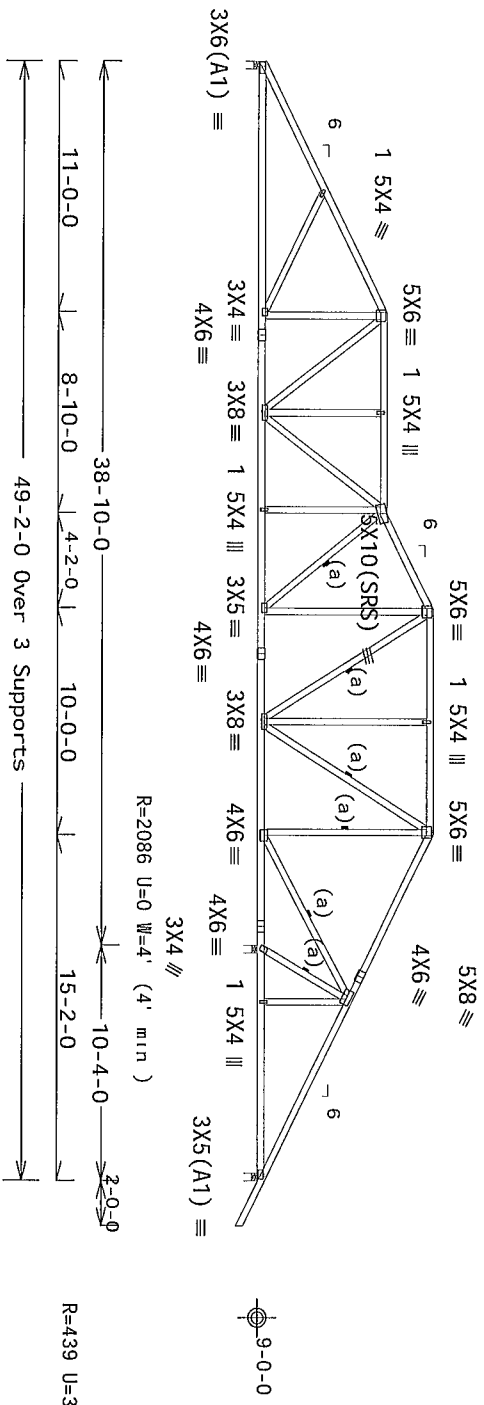
Top chord 2x4 SP #1_12A
 Bot chord 2x4 SP #1_12A
 Webs 2x4 SP #3_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See "WARNING" note below

- (a) Continuous lateral bracing equally spaced on member
- In lieu of structural panels use purlins to brace all flat TC @ 24" OC
- Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50
- MMFRS loads based on trusses located at least 15.00 ft from roof edge

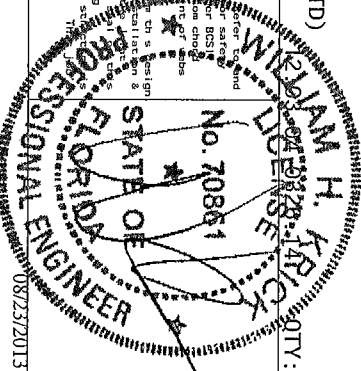
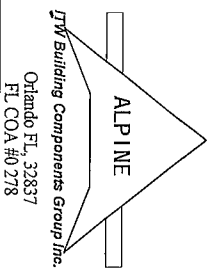


PLT TYP Wave Design Crit FBC2010Com/TP1-2007(STD) FT/RT=10%(0%)/0(0)

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating handling shipping installing and bracing. Follow the latest edition of BCS1 (Building Component Safety Information) by TPI and WDA. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCS1 sections B3.57 or B10 as applicable.

1TW Building Components Group, Inc. (1TWBCG) shall not be responsible for any deviation from this design or any failure of the truss system. The user shall verify the design and use of this design for any specific application. The user shall verify the design and use of this design for any specific application. The user shall verify the design and use of this design for any specific application.



FL/-/3/-/-/R/-	Scale = .125"/Ft.
TC LL 20.0 PSF	REF R487-- 29245
TC DL 7.0 PSF	DATE 08/23/13
BC DL 10.0 PSF	DRW HCUSR487 13235024
BC LL 0.0 PSF	HC-ENG WHK/WHK
TOT.LD. 37.0 PSF	SECON- 311395
DUR.FAC. 1.25	
SPACING 24 0"	JREF- 1U21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MEMBER
(Special)

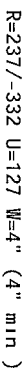
Negative reaction(s) of -332# MAX (See below) from a non-wind load case requires uplift connection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf Gcpi(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Design Crit: FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

FI/-/3/-/-/-/R/-

Scale = 125"/Ft

ITW Building Components Group Inc.

****IMPORTANT**** ****WARNING**** **READ AND FOLLOW ALL NOTES ON THIS SHEET**

Tenuses requ to extreme care in fabricating and erecting all components and bracing
follow the latest edition of BCSI (Bu) design manual and all applicable codes and standards.
practices prior to performing any construction. All installers shall provide temporary bracing
unless noted otherwise. No chord shall have properly attached structural sheathing and bolts
shall have a properly installed end girdling. Locations shown for permanent lateral restraints
shall show bracing method per BCSI sections B3, B7 or B10 as applicable.

ITW Bu Ltd or Components Group Inc. (ITWBGCS) shall not be responsible for any deviation from
any Bu Ltd or build the truss in conformance with ANSI/TPI-1 or for handling shipping ITW
building of trusses. Apply plans to even face of truss and position as shown above and on
drawing or cover plate listing this drawing. The suitability and use of this design for any
responsibility solely for the design shown. The suitability and use of this design for any
general liability of the Build ng Designer per ANSI/TPI-1 Sec 2. For more information on see
technical notes page ITW-BGC www.ctabg.com TPI www.spintec.org WTC www.sbc-industry.com
www.walcraft.org

TC LL	20.0 PSF	REF	R487-- 29246
TC DL	7.0 PSF	DATE	08/23/13
BC DL	10.0 PSF	DRW	HCUSR487 13235021
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37.0 PSF	SEQN-	311396
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1UZI487 Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR (Special)

Negative reaction(s) of -414# MAX (See below) from a non-wind load case requires uplift connection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf Gcpl(+/-)=0 18

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Cr it. FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

Scale = .125"/Ft.

ITW Building Components Group Inc.

[illegible]

08/22/2012

TC LL	20 0 PSF	REF	R487--	29247
TC DL	7 0 PSF	DATE	08/23/13	
BC DL	10.0 PSF	DRW	HCU8R487	13235012
BC LL	0.0 PSF	HC-ENG	WHK/WHK	
TOT LD	37 0 PSF	SEQN-	317271	
DUR.FAC.	1 25			
SPACING	24 0"	JREF-	1UZI487	Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MEMBER (Special)

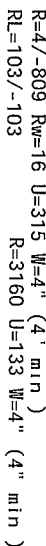
Negative reaction(s) of -809# MAX (See below) from a non-wind load case requires uplift connection

120 mph wind, 15.00 ft mean hgt., ASCE 7-10, CLOSED bldg, not located within 6.50 ft from roof edge, RISK CAT I, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf GCP(+/-)=0.18

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See **WARNING** note below.



R=1348 U=66 W=4" (4' min)

Design Crit. FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

*****IMPORTANT*****
*****WARNING***** READ AND FOLLOW ALL NOTES ON THIS SHEET!
 FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

ITV Building Components Group Inc.

Orlando FL, 3283
FL COA #0278

[illegible]

Professional Engineer Seal for William H. Krivy, No. 70861, State of Florida. The seal is circular with the text "WILLIAM H. KRIVY" around the top, "No. 70861" in the center, and "STATE OF FLORIDA" around the bottom. The words "PROFESSIONAL ENGINEER" are written along the inner edge of the seal.

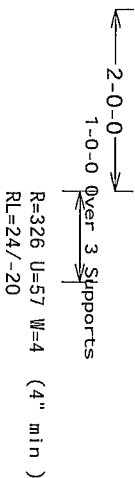
FL/-/3/-/-/R/-		Scale = .125"/Ft.
TC LL	20.0 PSF	REF R487-- 29248
TC DL	7 0 PSF	DATE 08/23/13
BC DL	10.0 PSF	DRW HCURS487 13235013
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT LD	37 0 PSF	SEQN- 317276
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1U21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR
(<)

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC

Wind loads and reactions based on MNFRS with additional C&C member

Deflection factor for dead load	meets L/240	live and L/180	total load	Creep increase
Provide (2) 16d common nails(0 162"x3 5"), toe nailed at Top chord				
Provide (2) 16d common nails(0 162"x3 5"), toe nailed at Bot chord				



Design Crit	FBC2010Com/TP1-2007(Std)	FT/RT=10%(0%)/0(0)

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

No. 70861
 STATE OF
 FLORIDA
 PROFESSIONAL ENGINEER

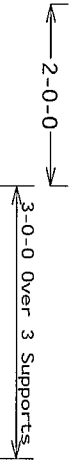
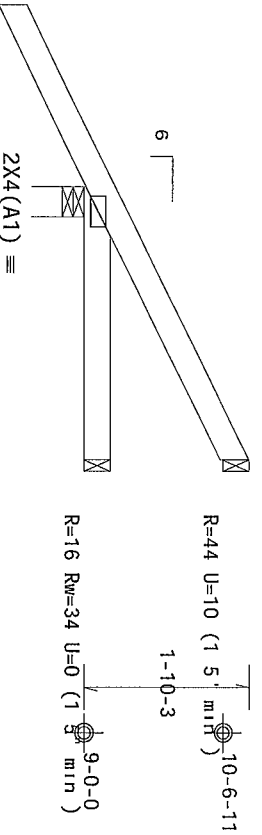
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TC DL	7 0 PSF	DATE	08/23/13
BC DL	10 0 PSF	DRW	HCSUR487 13235017
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	311397
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1U21487 Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A use design values approved
1/5/2012 by ALSC

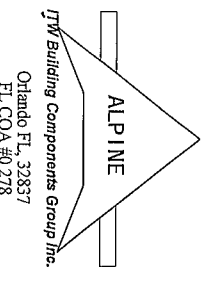
This design is based on lumber values in effect prior to June 1, 2013
and shall only be used on projects designed and permitted prior to
this date unless specifically approved in writing by the building
authority having jurisdiction, the building designer and the project
owner

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC
DL=5.0 psf GCPI(+/-)=0.18
Wind loads and reactions based on MMFRS with additional C&C member
design
Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1.50
Provide (2) 16d common nails(0 162"x3 5'), toe nailed at Top chord
Provide (2) 16d common nails(0 162"x3 5'), toe nailed at Bot chord

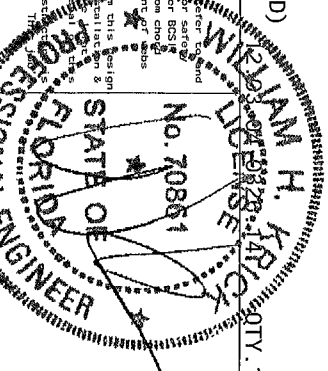


R=288 U=23 W=4 (4 min)
RL=41/-24

PLT TYP Wave
Design Crit FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)



****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCS (Building Components Safety) information on by TPI and WDA for BCS practices prior to performing these functions. Installers shall provide temporary bracing unless noted otherwise. Top chord shall have properly attached structural sheathing and become closed truss. Bottom chord shall have properly attached structural sheathing and become closed truss. All trusses shall have bracing installed per BCS sections B3, B7 or B10 as applicable.
JTW Building Components Group Inc. (JTWBCG) shall not be responsible for any deviation from this design or any failure to build the trusses in conformity with ANSI/TPI 1 or ANSI/TPI 2 for standard plate positions. A truss is, unless noted otherwise, Refer to drawings 160A-2 for standard plate positions. A truss shall be drawn on or cover page 1 of the design shown. The suitability and use of this design for any other purpose is the responsibility of the building designer. For more information see the general notes on page 1 of the design shown. TPI www.tpiinc.org WDA www.wdaindustry.com IBC www.imatec.org



QTY. 12 FL/-/3/-/-/R/-				Scale = .5"/Ft.	
TC LL	20.0 PSF	REF	R487--	29250	
TC DL	7.0 PSF	DATE	08/23/13		
BC DL	10.0 PSF	DRW	HCSR487	13235007	
BC LL	0.0 PSF	HC-ENG	WHK/WHK		
TOT LD.	37.0 PSF	SEON-	311398		
DUR. FAC.	1.25				
SPACING	24 0"	JREF	1U21487_201		

(13-209A--Erkinger Home Builders Williamson Residence -- Property ID 23-5S-15- - CJS 5' Jack)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with '12A" use design values approved
1/5/2012 by ALSC

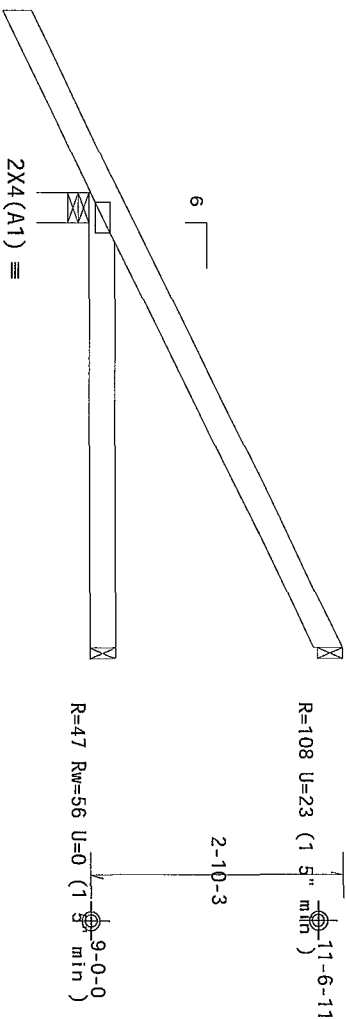
This design is based on lumber values in effect prior to June 1, 2013
and shall only be used on projects designed and permitted prior to
this date unless specifically approved in writing by the building
authority having jurisdiction, the building designer and the project
owner

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf,
wind BC DL=5.0 psf GCPI(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member
design

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1.50

Provide (2) 16d common nails(0 16d"x3 5"), toe nailed at Top chord
Provide (2) 16d common nails(0 16d"x3 5"), toe nailed at Bot chord



2-0-0
5-0-0 Over 3 Supports
R=343 U=17 W=4" (4' min)
RL=57/-28

PLT TYP Wave

Design Crit FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

QTY 11 FL/-/3/-/R/-

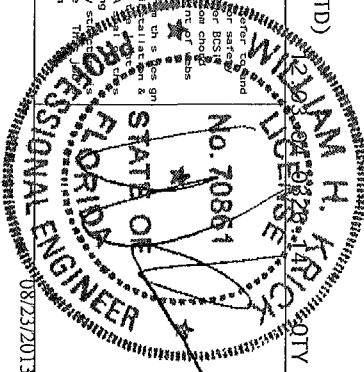
Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL 32837
FL COA #0278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, and bracing to follow the latest edition of BCS (Building Component Safety Information by TPI and WCA) practices prior to performing these functions. Installers shall provide temporary bracing for BCS unless noted otherwise. Top chord shall have properly attached structural sheathing and be braced in accordance with BCS. All bracing shall be installed in accordance with BCS. All bracing shall have bracing installed per BCS section B3, B7 or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any design or fabrication errors on this design. The user of this design shall be responsible for any design or fabrication errors. Details of trusses, apply plates to each face of truss and position as shown above and on drawings unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A drawing or cover page listing this design shows the availability and use of this design for any specific project. The user of this design shall be responsible for any design or fabrication errors. General notes apply to ITWBCG. www.itwbcg.com TPI www.tpi-inc.org WCA www.wcaindustry.com IBC www.ibracke.org



TC LL	20.0 PSF	REF R487-- 29251
TC DL	7.0 PSF	DATE 08/23/13
BC DL	10.0 PSF	DRW HCUSR487 13235003
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT LD	37.0 PSF	SEQN- 311399
DUR. FAC	1.25	
SPACING	24.0"	JREF- 1U21487_Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved
1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013
and shall only be used on projects designed and permitted prior to
this date unless specifically approved in writing by the building
authority having jurisdiction, the building designer and the project
owner

Provide (2) 16d common nails(0 162"x3 5"), toe nailed at Top chord
Provide (2) 16d common nails(0 162"x3 5"), toe nailed at Bot chord

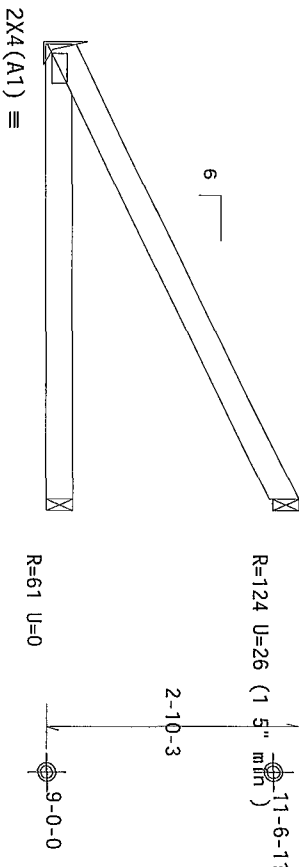
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf,
wind BC DL=5 0 psf GCpl(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member
design

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50

These hangers and support conditions used at bearings indicated

(H1) = Simpson
(H2) = (J) Hanger not calculated
(H3) = (J) Hanger not calculated



5-0-0 Over 3 Supports

R=194 U=0
RL=41
H=H1

PLT TYP. Wave

Design Crit FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

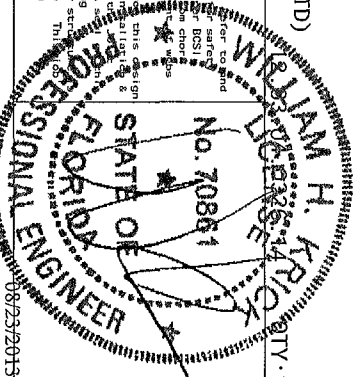
Trusses require extreme care in fabricating handling shipping installing and bracing
follow the latest edition of BCSI (Building Component Safety Information) by TPI and WDOA
practices prior to performing these functions. Installers shall provide temporary bracing
Unless noted otherwise top chord shall have properly attached structural sheathing and blocking
Blocking shall be installed per BCSI Section B3 by or B10 as applicable
Blocking shall have bracing installed per BCSI Section B3 by or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design
any failure or bracing of trusses. Apply plates to each face of truss and post as shown above and on
drawing or cover page 1 stating this drawing indicates acceptance of professional engineering
responsibility solely for the building designer per ANSI/TPI 1 Sec 2 for more information see
the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 for more information see
ITWBCG www.itwbcg.com TPI www.tpi.net.org WDOA www.wdoindustry.com
ITW Building Components Group Inc
Orlando FL 32837
FL COA #0 278

ALPINE

ITW Building Components Group Inc

Orlando FL 32837
FL COA #0 278



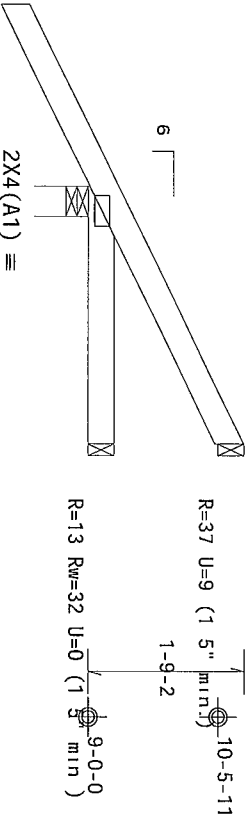
FL/-/3/-/-/R/-	Scale = .5"/Ft.
TC LL 20 0 PSF	REF R487-- 29252
TC DL 7 0 PSF	DATE 08/23/13
BC DL 10.0 PSF	DRW HCUSR487 13235014
BC LL 0.0 PSF	HC-ENG WHK/WHK
TOT LD 37.0 PSF	SEQN- 317279
DUR.FAC. 1.25	
SPACING 24.0"	JREF - 1U21487_201

(13-209A--Erkinger Home Builders Williamson Residence -- Property ID 23-55-15- - E/3 2 10' End Jack)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Lumber grades designated with "12A use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf GCPI(+/-)=0.18
Wind loads and reactions based on MMFERS with additional C&C member design
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50
Provide (2) 16d common nails(0 162"x3 5"), toe nailed at Top chord Provide (2) 16d common nails(0 162"x3 5"), toe nailed at Bot chord



2-0-0' Over 3 Supports

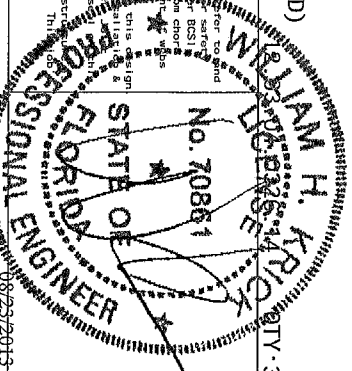
R=285 U=23 W=4 (4' min)
RL=40/-24

PLT TYP. Wave Design Crit: FBC2010Com/TP1-2007(STD) FT/RT=10%(0%)/0(0)

ALPINE

ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating handling shipping and bracing. Follow the latest edition of BCS (Building Component Safety Information by TPI and WTC) practice or to performing these functions. Installers shall provide temporary bracing to the trusses in accordance with the BCS (Building Component Safety Information by TPI and WTC) shall have a properly attached and sealed BCS (Building Component Safety Information by TPI and WTC) shall have bracing installed per BCS sections B3 B7 or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TP1-1 or for handling shipping and on the drawing of cover page 1. See drawing for details. Acceptance of process and engineer's signature shall be the responsibility of the user. The user shall be responsible for any deviation from the general notes page ITW-BCG www.itwbcg.com TPI www.tpi.net WTC www.wtcindustry.com ICC www.icccafe.org



FL/-/3/-/-/R/-	Scale = .5"/Ft.
TC LL	20.0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT. LD	37.0 PSF
DUR. FAC.	1.25
SPACING	24.0"
JREF - 1U21487_Z01	

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

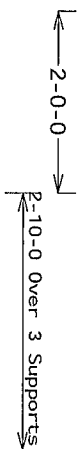
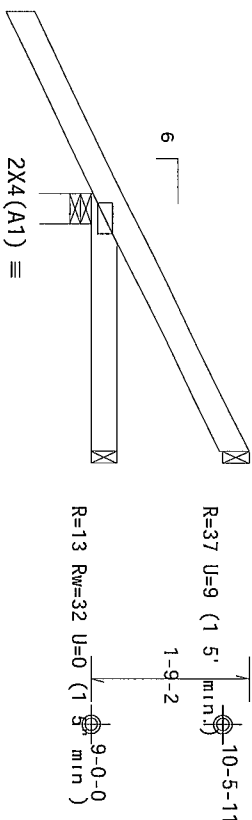
Provide { 2 } 16d common nails(0 162"x3 5'), toe nailed at Top chord Provide { 2 } 16d common nails(0 162"x3 5'), toe nailed at Bot chord

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, Exp B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCp(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MMFRS loads based on trusses located at least 7 50 ft from roof edge



R=285 U=23 W=4" (4' min)
RL=40/-24

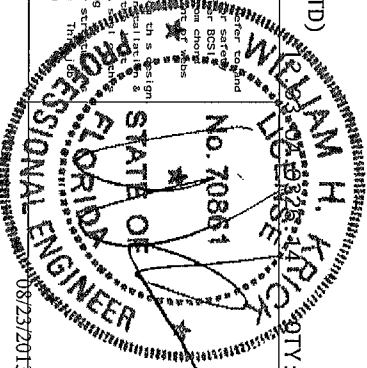
PLT TYP. Wave

Design Crit. FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

ALPINE

Orlando FL, 32837
FL COA #0278

****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating handling shipping installing and bracing follow the latest edition of BCSI (Building Component Safety Information by TPI and WDA) unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI section B3 B7 or B10 as applicable
ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any future building code changes in engineering or construction standards and on the part of the designer or truss manufacturer. Refer to drawings 160A-2 for standard plate positions. A drawing of cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The submittal and use of this design for any structure is the responsibility of the building designer. For more information on see ITWBCG general product literature. ITWBCG www.itwbcg.com TPI www.tpiinc.org WDA www.wdaindustry.com IBC www.icc-interactive.org



FL/-/3/-/-/R/-	Scale = .5"/Ft.
TC LL 20.0 PSF	REF R487-- 29254
TC DL 7.0 PSF	DATE 08/23/13
BC DL 10.0 PSF	DRW HCUSR487 13235016
BC LL 0 0 PSF	HC-ENG WHK/WHK
TOT. LD. 37.0 PSF	SEON- 3172277
DUR. FAC 1.25	
SPACING 24 0"	JREF - 1U21487_201

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

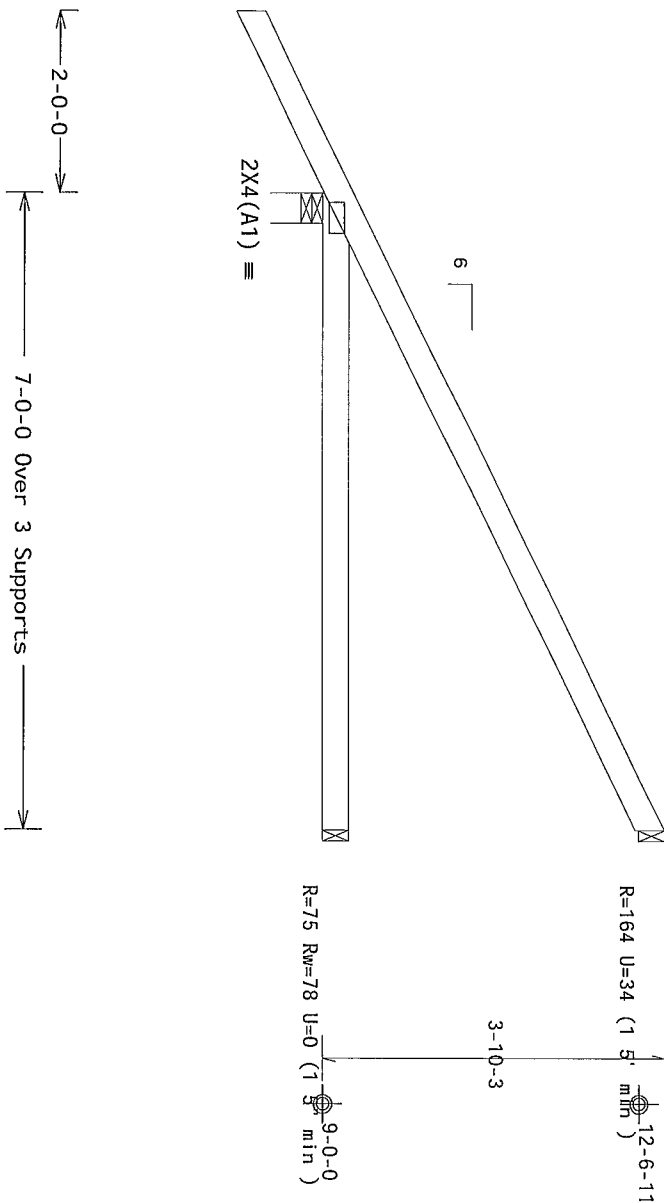
Provide (2) 16d common nails(0 162"x3 5"), toe nailed at Top chord
Provide (2) 16d common nails(0 162"x3 5"), toe nailed at Bot chord

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50

MMFRS loads based on trusses located at least 7 50 ft from roof edge



PLT TYP. Wave

Design Crit: FBC2010Com/TP1-2007(STD)

$$FT/RT = 10\%(0\%)/0(0)$$

****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

תאריך: 10.12.2019

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

WILLIAM H. KRICK
JAN 28 1968
OTW

QTY:47 FL/-/3/-/-/R/-

Scale = .5"/Ft.

TC LL	20.0 PSF	REF	R487-- 29255
TC DL	7.0 PSF	DATE	08/23/13
BC DL	10.0 PSF	DRW	HCUSR487 13235026
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT.LD	37 0 PSF	SEQN-	311400
DUR.FAC.	1 25		
SPACING	24 0"	JREF-	1UZI487_Z01

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50

MINFRS loads based on trusses located at least 7 50 ft from roof edge

Negative reaction(s) of -193# MAX (See below) from a non-wind load case requires uplift connection

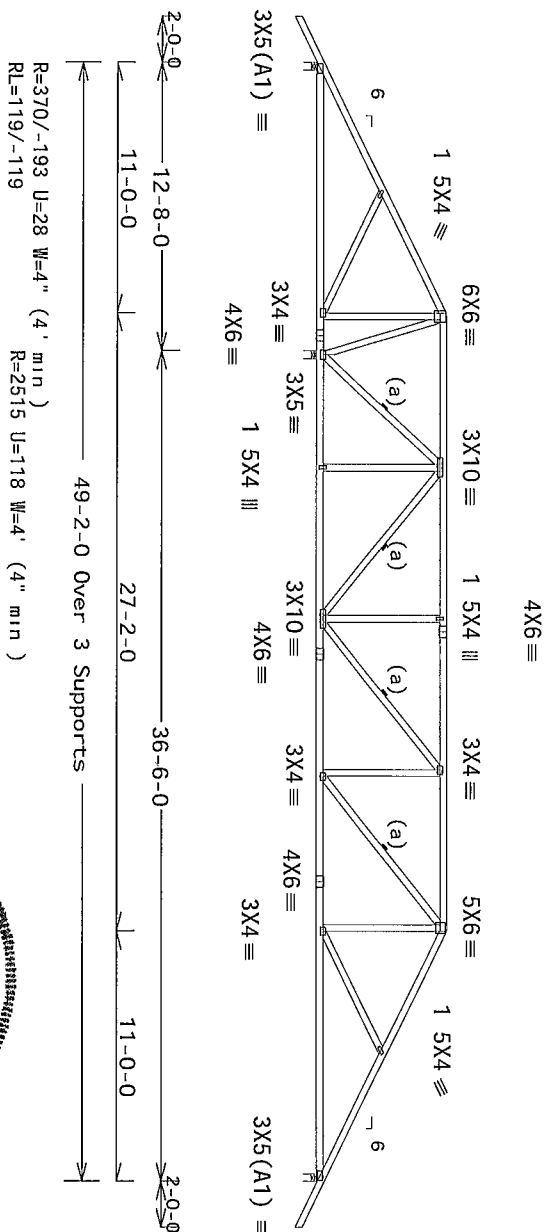
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCP(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below



R=1340 U=60 W=4" (4" min)

PLT TYP. Wave

Design Crit: FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

Q1:1

FL/-/3/-/-/R/-

Scale = .125"/Ft.

****IMPORTANT****
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

•

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

****IMPORTANT**** **READ AND FOLLOW ALL NOTES ON THIS SHEET!**
****WARNING**** **FURNISH THIS DECLET TO ALL CONTRACTORS INCLUDING INSTALLERS**

Trussess require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Build Up Component Steepest Information by TPI and WTC) practices prior to performing these functions. Installers shall provide temporary bracing unless noted otherwise. Contractors shall have properly attached structural sheathing and bracing shall have a properly attached field celling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3, B7 or B10 as applicable.

Any failure of any Component Group Line (TMBCS) shall not be responsible for any damage on the roof. Do not build the truss in conformance with ANSI/APA 1 or for handling any steel on the roof. Do not install unless noted otherwise. Refer to drawings 1604-2 for foundation details. Do not draw or cover panel 1187 on this drawing. Indicates acceptance of professional seal near responsibility solely for the design shown. The sub title and use of this drawing for the responsibility of the Build Up Designer per ANSI/APA 1 Sec 2. For more information on steel general notes panel 1187-BCSI www.tblong.com TPI www.tpi.net.org WTC www.stcindustry.com

ICC www.iccsafe.org

08/23/2013

TC LL	20.0 PSF	REF	R487-- 29256
TC DL	7.0 PSF	DATE	08/23/13
BC DL	10.0 PSF	DRW	HOURS487 13235006
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37.0 PSF	SEQN-	311403
DUR.FAC.	1.25		
SPACING	24 0"	JREF-	1UZ1487_Z01

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL-3 5 psf, wind BC DL=5 0 psf Gcpi(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

(a) Continuous lateral bracing equally spaced on member
In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

MMFRS loads based on trusses located at least 15.00 ft. from roof edge



R=2385 U=0 W=4" (4 min)

Design Crit: FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

W. L. D. 1897

REF R487-- 29257

Orlando FL, 32837
FL COA #0278

No. 70861
 STATE OF
 FLORIDA
 PROFESSIONAL
 ENGINEER

TC DL	7 0 PSF	DATE	08/23/13
BC DL	10.0 PSF	DRW	HCSR487 13235000
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD	37 0 PSF	SEQN-	311438
DUR. FAC.	1.25		
SPACING	24 0"	JREF-	1U71487_Z0

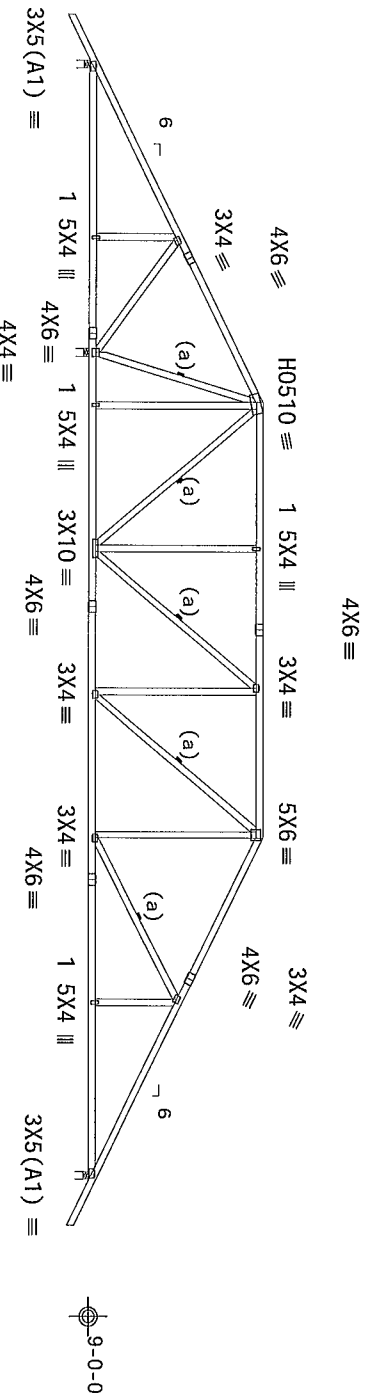
Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf GCP(+/-)=0.18
Wind loads and reactions based on MMFRS with additional C&C member design
(a) Continuous lateral bracing equally spaced on member
In lieu of structural panels use purlins to brace all flat TC @ 24" OC
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50
MMFRS loads based on trusses located at least 15.00 ft from roof edge



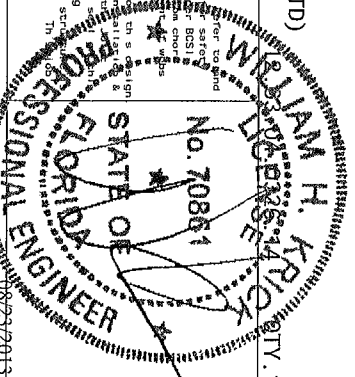
12'-0" 15'-0" 19'-2" 36'-6" 15'-0" 2'-0"
49'-2" Over 3 Supports
R=441 U=8 W=4" (4" min)
R=152/-152 R=2279 U=0 W=4" (4" min)
R=1398 U=0 W=4" (4" min)

PLT TYP. 20 Gauge HS, Wave Design Crit FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

ALPINE

ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0278

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety) Information on by TPI and WDA. Unless noted otherwise, top chord shall have proper y attached structural sheathing and bracing shall have bracing installed per BCSI section 8.7 or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure in the truss system. The user of this design shall be responsible for the design and use of this design for any structure. The user shall be responsible for the design and use of this design for any structure. The user shall be responsible for the design and use of this design for any structure.
Decal its unless noted otherwise. Refer to drawings 180A-2 for standard plate post-tension. A responsibility solely for the design shown. The user shall be responsible for the design and use of this design for any structure. The user shall be responsible for the design and use of this design for any structure. The user shall be responsible for the design and use of this design for any structure.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure in the truss system. The user of this design shall be responsible for the design and use of this design for any structure. The user shall be responsible for the design and use of this design for any structure. The user shall be responsible for the design and use of this design for any structure.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure in the truss system. The user of this design shall be responsible for the design and use of this design for any structure. The user shall be responsible for the design and use of this design for any structure. The user shall be responsible for the design and use of this design for any structure.



FL/-/3/-/-/R/-	Scale = .125"/Ft.
TC LL 20 0 PSF	REF R487-- 29258
TC DL 7 0 PSF	DATE 08/23/13
BC DL 10 0 PSF	DRW HCUSR487 13235009
BC LL 0.0 PSF	HC-ENG WHK/WHK
TOT LD 37 0 PSF	SEON- 311405
DUR.FAC 1 25	
SPACING 24 0"	JREF- 1U21487_201

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf GCPI(+/-)=0.18

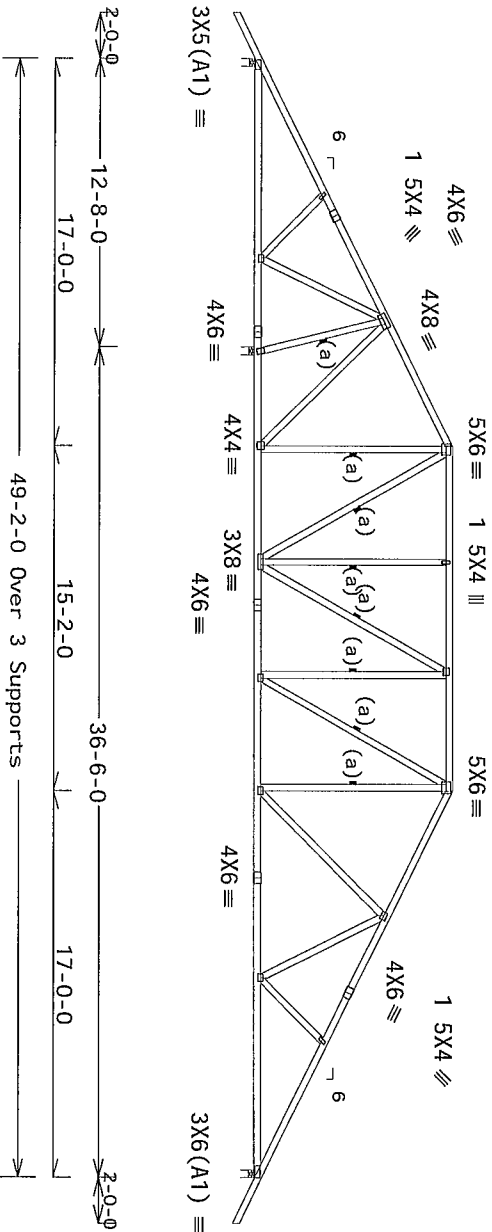
Wind loads and reactions based on MMFRS with additional C&C member design

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

MMFRS loads based on trusses located at least 15.00 ft from roof edge



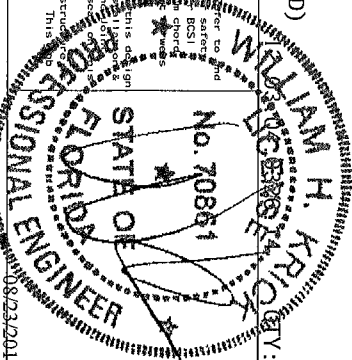
Note All Plates Are 3X4 Except As Shown
Design Crit FBC2010Com/TP1-2007(STD)
PLT TYP Wave

ALPINE

ITW Building Components Group Inc.

Orlando FL 32837
FL COA #02178

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCS1 (Building Component Safety) information on by TPI and WDOA. The contractor shall be responsible for ensuring that the trusses are properly braced and supported during installation. Unless noted otherwise, the top chord shall have properly attached structural sheathing and bottom chords shall have a properly attached 5/8" x 8" ceiling. Locations shown for permanent lateral bracing shall have bracing installed per BCS1 sections B3, B7 or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installing or bracing the truss. The contractor shall be responsible for ensuring that the trusses are properly braced and supported during installation. Drawings or cover page 1 stating this design and use of this design for any structure shall be the responsibility of the building designer per ANSI/TPI 1 Sec 2. For more information see the responsible party of the building designer per ANSI/TPI 1 Sec 2. This is not a design for any structure. General notes page ITW BCG www.itwbcg.com TPI www.tpinet.org WDOA www.structure.com IBC www.lobate.org



TC LL	20 0 PSF	REF R487-- 29259
TC DL	7.0 PSF	DATE 08/23/13
BC DL	10 0 PSF	DRW HCUSR487 13235010
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT LD	37 0 PSF	SEON- 311406
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1U21487_201

Scale = .125"/Ft.

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR
(Stepdown Hip)

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

(a) Continuous lateral bracing equally spaced on member
In lieu of structural panels use purlins to brace all flat TC @ 24
OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50



Scale = .125"/Ft.

REF R487-- 29261

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

DATE	08/23/13
DRW#	HCUSR487 1323501
HC-ENG	MMH/MMH
SEQN-	311407
JREF-	1UZ1487_Z0

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MEFFRE
(Stepdown Hip)

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCP1 (+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral bracing equally spaced on member
In lieu of structural panels use purlins to brace all flat TC @ 24
0C

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

MMF/RS loads based on trusses located at least 15.00 ft. from roof edge



R=2083 U=0 W=4 (4 min)

Design Crit FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

Copy:

Scale = 125"/Ft

14

drawing is unless noted otherwise. Refer to drawing AISC 160A-2 for standard plate positions. AISC 160A-2 also provides information on the design of cover plates. It is the responsibility of the designer to ensure that the cover plates are designed in accordance with the requirements of the AISC 160A-2. For more information on any of the topics mentioned in this document, please contact the American Institute of Steel Construction, Inc. at 100 W. Madison St., Suite 1700, Chicago, IL 60602. Tel: 312.670.3400. Fax: 312.670.3401. Email: info@aisc.org. Website: www.aisc.org.

9
sea
struc
Th s

DUR.FAC.	1.25	JREF- 1U21487 Z01
SPACING	24 0"	

Top chord 2x4 SP_2850F-2 3E T1 T5 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A B3 2x4 SP_2850F-2 3E
Weds 2x4 SP_#3_12A W5 2x6 SP_#2_12A
Rt Splice Block 2x4 SP_#3_12A
Lumber grades designated with 12A use design values approved
1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1 2013
and shall only be used on projects designed and permitted prior to
this date unless specifically approved in writing by the building
authority having jurisdiction the building designer and the project
owner

Brig blocks 0 131 x3 min nails
x-loc #blocks length/bik #nails/bik wall plate
2 12 667 1 23 16 Rigid Surface
Brig block to be same size and species as chord
Refer to drawing CUNA1LSP0109 for more information

Negative reaction(s) of -668# MAX (See below) from a non-wind load
case requires uplift connection

120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg not located
within 9 00 ft from roof edge, RISK CAT II EXP B wind TC DL=3 5 psf
wind BC DL=5 0 psf 6Cpl(+/-)=0 18

In lieu of structural panels use purlins to brace all flat TC @ 24
OC

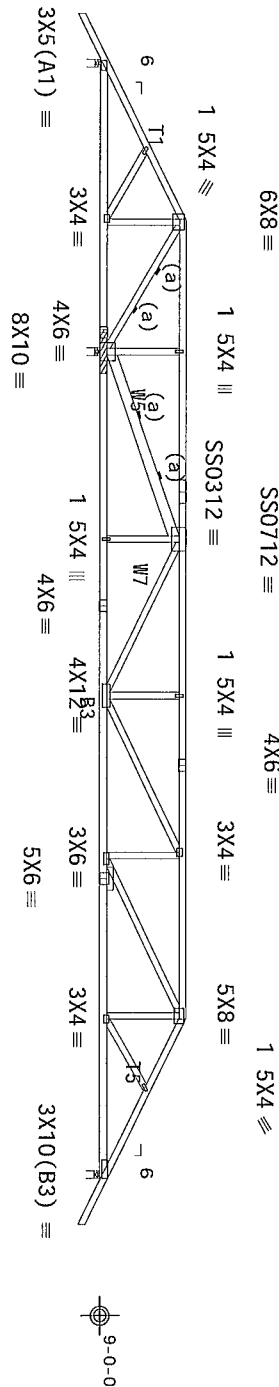
Special loads

TC-From	Dur Fac =1 25 / Plate Dur Fac =1 25)	TC-From	Dur Fac =1 25 / Plate Dur Fac =1 25)
TC-From	56 pif at 7 00 to 56 pif at 7 00	TC-From	56 pif at 7 00 to 56 pif at 7 00
TC-From	28 pif at 7 00 to 28 pif at 19 00	TC-From	28 pif at 7 00 to 28 pif at 19 00
TC-From	28 pif at 19 00 to 28 pif at 31 00	TC-From	28 pif at 19 00 to 28 pif at 31 00
TC-From	56 pif at 31 00 to 56 pif at 42 17	TC-From	56 pif at 31 00 to 56 pif at 42 17
TC-From	56 pif at 42 17 to 56 pif at 51 00	TC-From	56 pif at 42 17 to 56 pif at 51 00
BC-From	20 pif at 0 00 to 20 pif at 7 03	BC-From	20 pif at 0 00 to 20 pif at 7 03
BC-From	10 pif at 7 03 to 10 pif at 12 00	BC-From	10 pif at 7 03 to 10 pif at 12 00
BC-From	10 pif at 12 00 to 10 pif at 24 00	BC-From	10 pif at 12 00 to 10 pif at 24 00
BC-From	10 pif at 24 00 to 10 pif at 36 00	BC-From	10 pif at 24 00 to 10 pif at 36 00
BC-From	20 pif at 36 00 to 20 pif at 42 14	BC-From	20 pif at 36 00 to 20 pif at 42 14
BC-From	4 pif at 42 14 to 4 pif at 49 17	BC-From	4 pif at 42 14 to 4 pif at 49 17
TC-239 08 1b Conc Load at 7 03 42 14		TC-239 08 1b Conc Load at 7 03 42 14	
TC-163 57 1b Conc Load at 9 06 11 06 13 06 15 06		TC-163 57 1b Conc Load at 9 06 11 06 13 06 15 06	
17 06 19 06 21 06 23 06 24 58 26 10 28 10 30 10 32 10		17 06 19 06 21 06 23 06 24 58 26 10 28 10 30 10 32 10	
BC-247 55 1b Conc Load at 7 03 42 14		BC-247 55 1b Conc Load at 7 03 42 14	
17 06 19 06 21 06 23 06 24 58 26 10 28 10 30 10 32 10		17 06 19 06 21 06 23 06 24 58 26 10 28 10 30 10 32 10	

Wind loads and reactions based on MMFRS

(a) Continuous lateral bracing equally spaced on member
Deflection meets L/240 live and L/180 total load Creep increase
Factor for dead load is 1 50

WARNING Furnish a copy of this DWG to the installation contractor
Special care must be taken during handling, shipping and installation
of trusses See WARNING note below



2'-0" 12'-8" 7'-0" 35'-2" 36'-6" 7'-0" 2'-0"
R=669 Rw=14 U=0 W=4" (4" min)
R=5671 U=399 W=4 (4" min)
49'-2" Over 3 Supports
R=2367 U=178 W=4" (4" min)

PLT TYP. 18 Gauge HS, Wave

Design Crit FBC2010Com/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

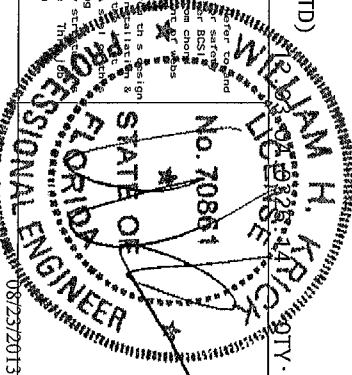
FL/-/3/-/R/- Scale = .125"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0 278

IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Tosses new to extreme care in their use and handling and shipping and installing and bracing
prior to the latest edition of BCS (Building Components Group Inc.) and ITW (ITW Building Components Group Inc.)
practices prior to performing these functions. Installers shall provide temporary bracing
unless noted otherwise. Top chord shall have properly attached structural sheathing and beam chord
shall have a properly attached rigid ceiling. Locate on shown for permanent lateral restraint.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design
any time to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installing, or
detaching the truss from the building. ITWBCG shall not be responsible for any deviation from this design
drawing or cover page listing this design shown. The suitability and use of this design for any structure
the responsibility of the Building Designer per ANSI/TPI 1 Sec 2. For more information see
general notes page ITW-BGS www.itwbcg.com TPI www.tpiinst.org WTCA www.steelindustry.com
ITC www.icecave.org



TC LL	20.0 PSF	REF R487-- 29262
TC DL	7.0 PSF	DATE 08/23/13
BC DL	10.0 PSF	DRW HOURS487 13235020
BC LL	0 0 PSF	HC-ENG WHK/WHK
TOT. LD.	37.0 PSF	SEON- 311433
DUR. FAC.	1.25	
SPACING	24 0"	JREF - 1U71487_201

Webs 2x4 SP_#3_12A W2, W8 2x4 SP_#1_12A

2012 by ALSC

1

- This plat

BC DL=5 0 p

loads and r

or for dead

Wave

ALPINE

FL COA #0278

FL COA #0278

Date: 01/25/

[illegible]

at $X = 18-4-8$

Wave

ALPINE

Building Components

Orlando FL, 328

FL COA #0 277

tepdawn Hip)

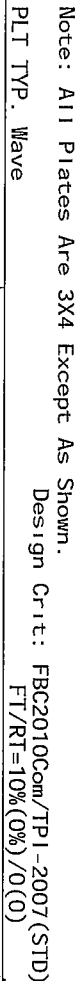
Negative reaction(s) of -267# MAX (See below) from a non-wind load case requires uplift connection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 6 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf,
wind BC DL=5 0 psf Gcpi (+/-)=0 18

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC


WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Scale = .125"/Ft

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling and bracing. Follow the latest edition of BCSI (Building Component Safety Information) by TPI and WTCA. Practices prior to performing these functions. Installers shall provide temporary bracing and bracing noted otherwise. No top chord shall have properly attached structural sheathing and bracing.



ALPINE

Orlando FL, 32837
FL COA #0278

[illegible]

~~08/23/2013~~

TC LL	20 0 PSF	REF	R487-- 29265
TC DL	7.0 PSF	DATE	08/23/13
BC DL	10 0 PSF	DRW	HCSR487 13235005
BC LL	0.0 PSF	HC-ENG	WHK/WMHK
TOT LD	37 0 PSF	SEQN-	311402
DUR FAC.	1 25		
SPACING	24 0"	JREF-	1UZ1487_Z01

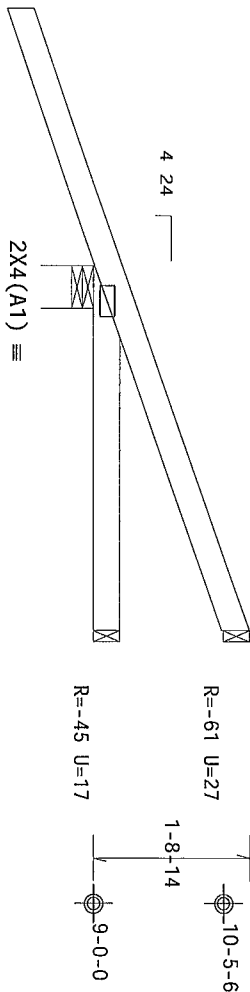
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR
 UP Jack Girder)

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCPI(+/-)=0 18

DL=5.0 psf GCP1 (+/-)=0.18

Wind loads and reactions based on MWFRS

Factor	Deflection meets L/240	live and dead load	Creep increase
Factor for dead load is 1.50			
Provide (2) 16d common nails(0 162' x3 5) ,			toe nailed at Top chord
Provide (2) 16d common nails(0 162' x3 5) ,			toe nailed at Bot chord



2-9-15

← 4-0-1 Over 3 Supports →
R=-55 U=126 W=5 657"

PLT TYP. Wave

Design Crit. FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

ITY:1 FL/-/3/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

These require crane cat in fair cat ng hand ng ship ng install ng and brace ng. Refer to the following latest edition of BCS (Building Component Safety Information) on by TPI and WDO. The safety practice now prior to performing these functions. Installers shall provide temporary brace ng from BCS unless noted otherwise as top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r-d ceiling. Locations shown for permanent lateral restraint shall have brace ng installed per BCS sections 83 B7 or B10 as applicable.

I7W Building Components Group Inc. (IWBDC) shall not be responsible for any delay action resulting from its failure to provide drawings or specifications for the building components, or for any failure to build it under the terms, n conformant w/ the ANSI/TPI 1 or for handling slip printing of trusses. Apply plates to each face of truss and post on as shown above and on the drawing or cover page indicating placement and grades acceptance of professional seal and responsibility by solely for the does gap between the trusses and the steel deck. The responsible b/c ly of the Build ng Does Gap per ANSI/TPI 1 Sec 2 For more information see the general notes page I7W BDC www.iwbdc.org WTCA www.abcdindustry.com
www.icc.org

WILLIAM H. KRICK
LICENSED PROFESSIONAL ENGINEER
STATE OF FLORIDA
No. 70861

08/23/2013

TC LL	20 0 PSF	REF	R487-- 29266
TC DL	7.0 PSF	DATE	08/23/13
BC DL	10.0 PSF	DRW	H05R487 13235017
BC LL	0.0 PSF	HC-ENG	WHK/MMHK
TOT LD	37 0 PSF	SEQN-	317280
DUR.FAC.	1 25		
SPACING	24 0"	JREF-	1U21487_Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

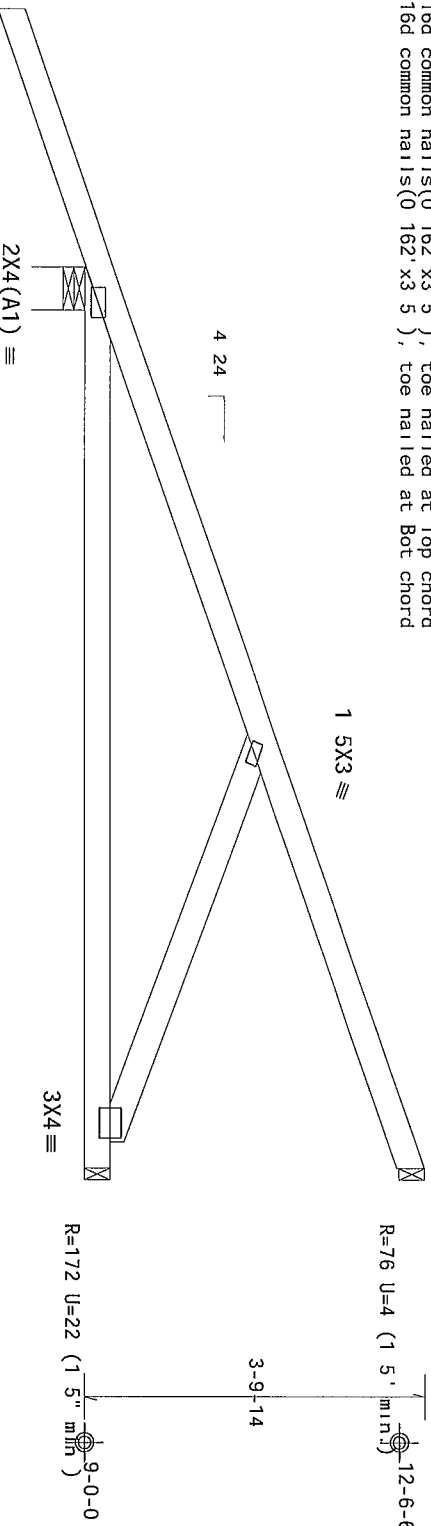
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCp1(+/-)=0 18

Provide { 2 } 16d common nails(0 162 x3 5"), toe nailed at Top chord Provide { 2 } 16d common nails(0 162 x3 5"), toe nailed at Bot chord

Special loads

TC- From	Dur Fac =1 25 /	Plate Dur Fac =1 25)
TC- From	0 pif at -2 83 to	55 pif at 0 00
TC- From	2 pif at 0 00 to	2 pif at 9 90
BC- From	0 pif at -2 83 to	4 pif at 0 00
BC- From	2 pif at 0 00 to	2 pif at 9 90
TC- 198 07 lb Conc	Load at 1 48	
TC- 87 55 lb Conc	Load at 4 31	
TC- 215 58 lb Conc	Load at 7 13	
BC- 61 75 lb Conc	Load at 1 48	
BC- 31 27 lb Conc	Load at 4 31	
BC- 94 56 lb Conc	Load at 7 13	

Wind loads and reactions based on MMFRS
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



2-9-15
9-10-13 Over 3 Supports

R=45 U=163 W=5 656 (5 656 min)

PLT TYP. Wave

Design Crit FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

Scale =.5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL 32837
FL COA #0 278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating and bracing. Refer to the latest edition of BCS (Building Component Safety Information by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCS1 unless noted otherwise. Top chord shall have properly attached structural sheathing and bot chord shall have properly attached right ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCS1 sections B3 B7 or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design and any modifications made to the design. The user shall be responsible for the structural integrity of the building design shown. The user shall verify and use of this design for any structural modification. The user shall verify and use of this design for any structural modification. The user shall verify and use of this design for any structural modification.
ITWBCG www.itwbcg.com TPI www.tpi.net org WCA www.wcaindustry.com

Professional Engineer
No. 70861
STATE OF FLORIDA
WILLIAM H. KRIDER
08/23/2013

TC LL	20.0 PSF	REF R487-- 29267
TC DL	7 0 PSF	DATE 08/23/13
BC DL	10.0 PSF	DRN HCUSR487 13235030
BC LL	0 0 PSF	HC-ENG WHK/WHK
TOT LD	37 0 PSF	SEON- 311428
DUR.FAC	1.25	
SPACING	24.0"	JREF- 1U21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR
3 Hip Jack Girder)

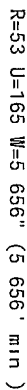
Special loads			

TC- (Lumber	Dur Fac = 1 25 /	Plate Dur Fac = 1 25)	
From 0 pif at -2 83	to 55 pif at 0 00		
TC- 0 00			

BC-From	0 pif at -2 83 to	4 pif at 0 00
BC-From	2 pif at 0 00 to	2 pif at 9 90
BC-From	1 pif at 1 40 to	1 pif at 1 40

TC- 87	55	1b	Conc	Load at	4 31
TC- 231	47	1b	Conc	Load at	7 13
BC- -61	75	1b	Conc	Load at	1 48
BC- 31	27	1b	Conc	Load at	4 31
BC- 107	89	1b	Conc	Load at	7 13

Wind loads and reactions based on MWFRS



Design Crit	FBC2010Com/TP1-2007(STD)	FT/RT=10%(0%)/0(0)

ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0278

08/23/2013

1 FL/-/3/-/-/R/-		Scale = .5"/Ft.	
TC LL	20 0 PSF	REF	R487-- 29268
TC DL	7 0 PSF	DATE	08/23/13
BC DL	10 0 PSF	DRW	HCU8487 13235018
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN-	317283
DUR.FAC.	1.25		
SPACING	24 0"	JREF-	1UJ21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR
5 (Common)

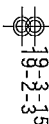
Special loads			
-----Lumber	Dur Fac =1 25 /	Plate	Dur Fac =1 25)
TC- From	56 p1 f at	0 00 to	56 p1 f at
TC- From	56 p1 f at	0 00 to	56 p1 f at

BC- From	4 p1f at	0 00 to	4 p1f at	9 83
----------	----------	---------	----------	------

In lieu of rigid ceiling use purlins to brace BC @ 24" OC

factor for dead load is 1.50

edge



R=1 R_w=9 U=26 W=7 826' (7 826" min)

Design Crit	FBC2010Com/TP1-2007(STD) FT/RT=10%(0%)/0(0)

Scale = .5"/Ft.

ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trussess require extreme care in fabricating handling and installing and bracing
follow the latest code or one of BCIS (Builing Component Safety Information by TPI and WTCOA)
practices prior to performing these functions Installers shall provide temporary bracing
shall have a properly attached rigid or nonrigid bracing system (properly installed) and
any failure to build the trusses in conformance with ANSI/TPI-1 or for handling and
Bracing of trusses Apply plates to each face of truss and post to as shown above and
is unless noted otherwise As Refer to draw ngs 100A-Z for standard plate plans A
and cover the entire length of the truss and ends of members of professional engineer
responsibility for design and construction of the building and the responsibility of the Building Designer per ANSI/TPI-1 Sec 2 For more information on see
general notes page ITW-BOS www.tbwco.com TPI www.tpiinc.org WTCOA www.steelsociety.com
ITC www.itcinc.org

TC LL	20.0 PSF	REF	R487--	29269
TC DL	7.0 PSF	DATE	08/23/13	
BC DL	10.0 PSF	DRW	HCUSR487 13235027	
BC LL	0.0 PSF	HC-ENG	WHK/WHK	
TOT.LD.	37.0 PSF	SEQN-	311419	
DUR.FAC.	1.25			
SPACING	24.0"	JREF-	1UZ1487_Z01	

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3_12A

Lumber grades designated with '12A' use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

Wind loads and reactions based on MMFRS with additional C&C member design

MMFRS loads based on trusses located at least 20 01 ft from roof edge

Refer to DWG PB140100212 for piggyback details

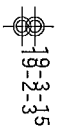
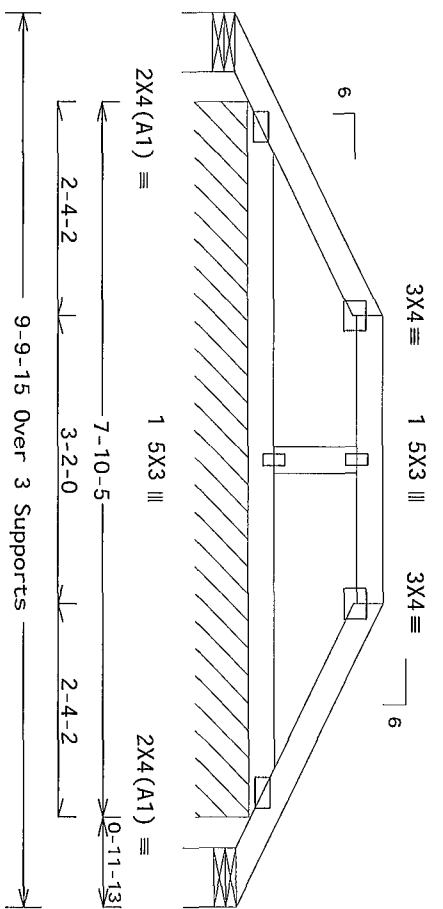
Special loads

TC- From	Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From	56 pif at 0 00 to 56 pif at 3 33
TC- From	56 pif at 3 33 to 56 pif at 6 50
TC- From	56 pif at 6 50 to 56 pif at 9 83
BC- From	4 pif at 0 00 to 4 pif at 9 83

120 mph wind, 20 01 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf 6Cpi(+/-)=0 18

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24' 0C, all BC @ 24' 0C

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



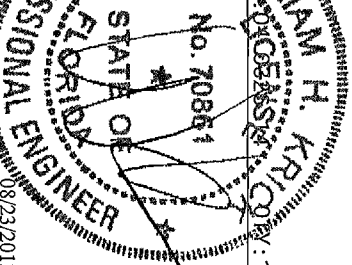
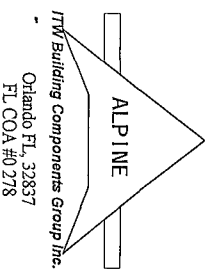
PLT TYP. Wave

Design Crit: FBC2010Com/TP1-2007(STD)
FT/RT=10%(0)/0(0)

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating and handling. Installers shall provide temporary bracing and blocking for all trusses. Trusses shall have bracing installed per BCSI Section B3.7 for B10 as applicable.

ITW Building Components Group Inc. Orlando FL 32837 FL COA #0278



FL/-/3/-/-/R/-	Scale = .5"/Ft.
TC LL	20.0 PSF
IC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT. LD.	37.0 PSF
DUR. FAC.	1.25
SPACING	24.0"
JREF	11U21487_201

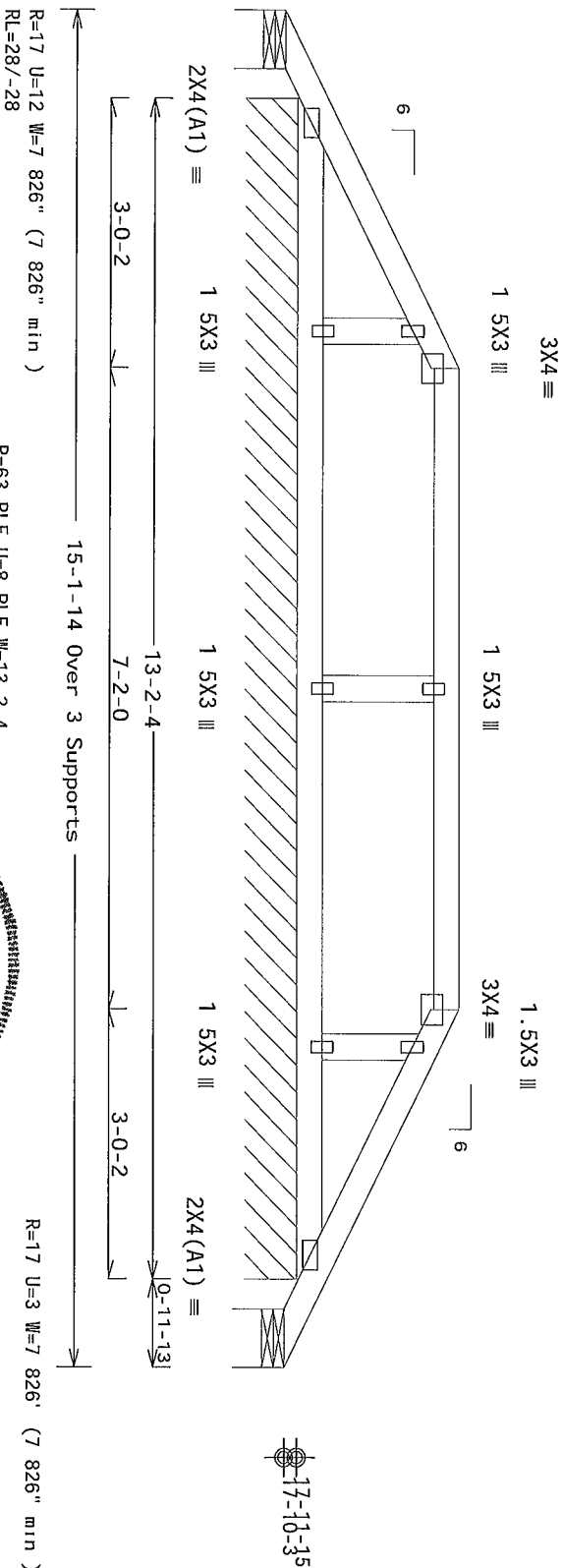
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR
14 Steardown Hip)

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

Wind loads and reactions based on MMFRS with additional C&C member design

MMFRS loads based on trusses located at least 18 85 ft from roof edge

Refer to DWG PB140100212 for piggyback details



PLT TYP. Wave

Design Crit.: FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

FT/RT=10%(0%)/0(0)

FL/-/3/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Twelve (12) inches apart in fair and good weather, and in the presence of the inspector, the contractor shall follow the latest edition of BSI (Building Information Systems) standards for the installation of the BSI practices prior to performing these functions. The contractor shall provide temporary bracing and BSI unless noted otherwise. No chord shall have properly attached structural sheath and bracing. The contractor shall have a property attached (1610 coil) Locations shown for permanent lateral restriction of the chord shall have installed per BSI sections 83, 87 or 810 as applicable.

[illegible]

Special loads	Dur	Fac = 1	25 /	Plate	Dur	Fac = 1	25)		
-----Lumber									
TC - From	56	pif	at	0 00	to	56	pif	at	4 00
TC - From	56	pif	at	4 00	to	56	pif	at	11 18
TC - From	56	pif	at	11 16	to	56	pif	at	15 16
BC - From	4	pif	at	0 00	to	4	pif	at	15 16

120 mph wind, 18.85 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=2.0 psf GCP(+/-)=0.18

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

[illegible]

08/23/2018

TC LL	20.0 PSF	REF	R487-- 29271
IC DL	7.0 PSF	DATE	08/23/13
BC DL	10.0 PSF	DRW	HCHSR487 13235014
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37.0 PSF	SEQN-	311421
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1U71487_201

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR
14 Stepdown Hip)

Special loads

Lumber Dur Fac =1 25 / Plate Dur Fac =1 25
TC- From 56 pif at 0 00 to 56 pif at 2 00

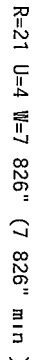
TC-From	56 pif at 13 16 to	56 pif at 15 16
BC-From	4 pif at 0 00 to	4 pif at 15 16

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC

MMFRS loads based on trusses located at least 18 35 ft from roof edge

from roof



Design Crit: FBC2010Com/TP1-2007(STD),
FT/RT=10%(0%)/0(0)

1 FL/-/3/-/-/R/-

Scale = .5"/Ft.

*****WARNING***** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

ALPINE

ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

[illegible]

WILLIAM H. KRICORIAN
No. 70861
STATE OF FLORIDA
MECHANICAL
PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R487-- 29272
IC DL	7.0 PSF	DATE	08/23/13
BC DL	10.0 PSF	DRW	H05R487 13235025
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37 0 PSF	SEQN-	311422
DUR.FAC.	1.25		
SPACING	24 0"	JREF-	1U21487 Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

Wind loads and reactions based on MMFRS with additional C&C member design

MMFRS loads based on trusses located at least 19.35 ft from roof edge

Refer to DWG PB140100212 for piggyback details

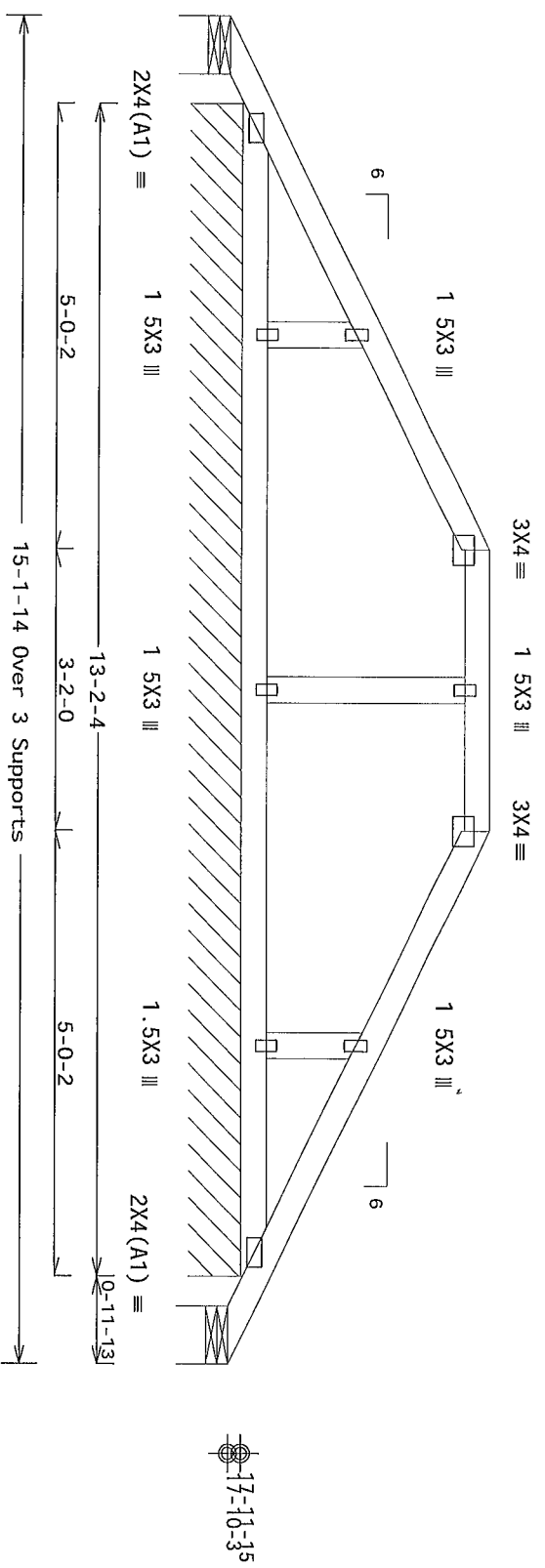
Special loads

	Dur Fac =1.25 / Plate Dur Fac =1.25)
TC- From	56 pif at 0.00 to 56 pif at 6.00
TC- From	56 pif at 6.00 to 56 pif at 9.16
TC- From	56 pif at 9.16 to 56 pif at 15.16
BC- From	4 pif at 0.00 to 4 pif at 15.16

120 mph wind, 19.35 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=2.0 psf GCPI(+/-)=0.18

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24' OC, all BC @ 24' OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

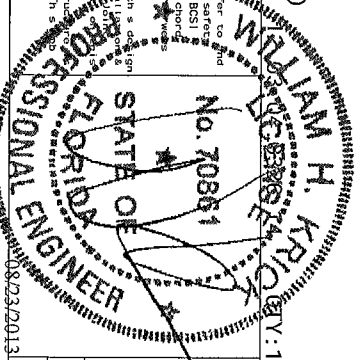
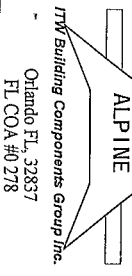


R=20 U=17 W=7 826' (7 826' min) R=63 PLF U=5 PLF W=13-2-4
R=43/-43 R=20 U=2 W=7 826" (7 826" min)

PLT TYP Wave Design Crit FBC2010Com/TP1-2007(STD) FT/RT=10%(0%)/0(0)

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, handling, installing and bracing. Follow the latest edition of BCSI (Building Component Safety) information on by TPI and WTA. Trusses must be erected in accordance with the BCSI information. Trusses must be erected in accordance with the BCSI information. Trusses must be erected in accordance with the BCSI information.



FL/-/3/-/R/-	Scale =.5"/Ft.
TC LL 20.0 PSF	REF R487-- 29273
TC DL 7.0 PSF	DATE 08/23/13
BC DL 10.0 PSF	DRW HCUR487 13235018
BC LL 0.0 PSF	HC-ENG WHK/WHK
TOT.LD 37.0 PSF	SEON- 311423
DUR.FAC 1.25	
SPACING 24.0"	JREF- 1U21487_Z01

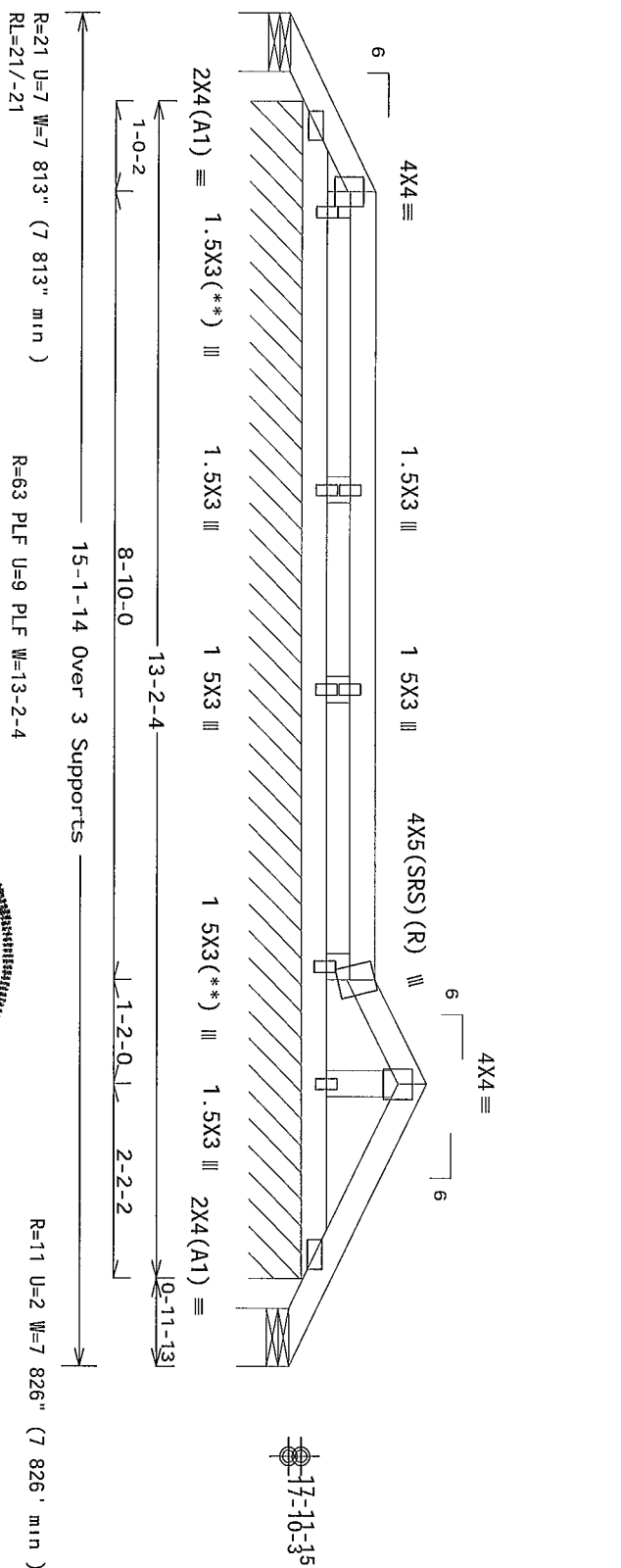
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

Wind loads and reactions based on MNFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increases factor for dead load is 1.50

Refer to DWG PB140100212 for piggyback details



PLT TYP	Wave
---------	------

Design Cr1t: FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

TY.

FL/-/3/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc
Orlando FL, 32837
FL COA #0278

[illegible][illegible]

(**) 2 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements

120 mph wind, 18 64 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCPI (+/-)=0 18

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24' OC, all BC @ 24' OC

edge	loads based on trusses located at least 18 ft from roof edge
------	--

WILLIAM H. KRICK
 LICENSE
 No. 70861
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER

TC LL	20 0 PSF	REF	R487-- 29274
TC DL	7.0 PSF	DATE	08/23/13
BC DL	10.0 PSF	DRW	H05R487 13235019
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD.	37 0 PSF	SEQN-	311424
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1U21487 Z01

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

Wind loads and reactions based on MMFRS with additional C&C member design

Refer to DWG PB140100212 for piggyback details



R=10 RW=11 U=10 W=6 001" (6 001 min)
RL=22/-22

R=72 PLF U=2 PLF W=4-8-0

PLT TYP. Wave

Design Crit	FBC2010Com/TP1-2007(STD) FT/RT=10%(0%)/0(0)
-------------	--

QTY 1 FL/-/3/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trenches requiring care in fabricating, handling, shipping, installing and bracing shall follow the latest edition of BCSI (Building Component Safety) Information by TPI and WTCA. Unless noted or to perform one of those functions, installers shall provide temporary bracing practices per other code. Top chord shall have properly attached structural sheath ng and no shall have a properly attached r g d c t i n g. Bottom chords shown for permanent lateral restraint shall have brace ng installed per BCS sect on B3 BY or B10 as applicable.

ITW Building Components Group Inc. (ITWBGC) shall not be responsible for any delay on production or delivery of products due to any failure to build the truss in accordance with ANSI/TPI-1 or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and post it on as shown above and on drawing. Deck is unless noted otherwise see. Refer to draw gms 100A-Z for standard plate cuts. A sketch drawing or cover page listing the drawings and dates acceptance of process, serial engineering approval and date of construction per ANSI/TPI-1 Sec 2. For more information on the responsibility of the Build group please contact TPI at:

general notes page ITW-BDC www.itwbcg.com TPI www.tpi.net and WTCA www.steelindustry.com

www.cenla.org

Special loads

Species	Threat	Duration	Facility	Plate	Facility	Duration
-----	(Lumber	Dur	Fac = 1	25 /	Plate	Dur Fac = 1 (25)
TC-From	56 pif at	-0 83 to	56 pif at	2 33		
TC-From	56 pif at	2 33 to	56 pif at	5 65		
BC-From	4 pif at	-0 83 to	4 pif at	5 65		

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCPI(+/-)=0 18

In lieu of rigid ceiling use purlins to brace BC @ 24' OC

Deflection meets L/240 live and L/180 total load Creep increases factor for dead load is 1.50

MMWFRS loads based on trusses located at least 15 00 ft from roof edge

TC LL	20 0 PSF	REF	R487-- 29275
TC DL	7.0 PSF	DATE	08/23/13
BC DL	10.0 PSF	DRW	HCSUR487 13235002
BC LL	0 0 PSF	HC-ENG	WHK/MMH
TOT. LD	37 0 PSF	SEQN-	311425
DUR. FAC.	1.25		
SPACING	24 0"	JREF-	1UJ7487_Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3_12A

Lumber grades designated with '12A" use design values approved
1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013
and shall only be used on projects designed and permitted prior to
this date unless specifically approved in writing by the building
authority having jurisdiction, the building designer and the project
owner

Wind loads and reactions based on MMFRS with additional C&C member
design

MMFRS loads based on trusses located at least 18 39 ft from roof
edge

Refer to DWG PB140100212 for piggyback details

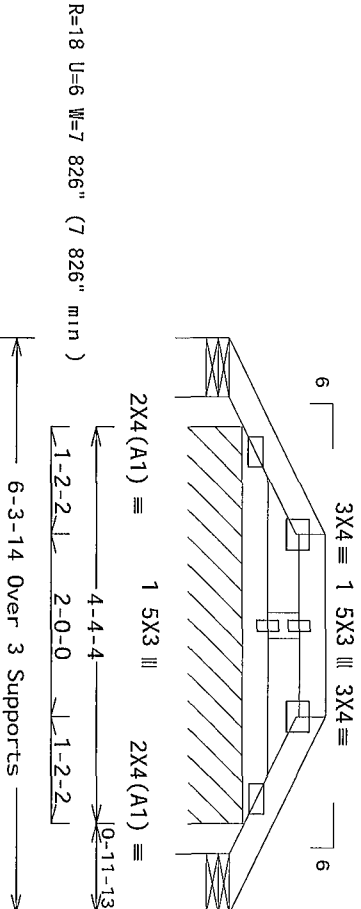
Special loads

Lumber Dur Fac = 1 25 / Plate Dur Fac = 1 25
TC-From 56 pif at 0 00 to 56 pif at 2 16
TC-From 56 pif at 2 16 to 56 pif at 4 16
TC-From 56 pif at 4 16 to 56 pif at 6 33
BC-From 4 pif at 0 00 to 4 pif at 6 33

120 mph wind, 18 39 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC
DL=2 0 psf GCpl(+/-)=0 18

In lieu of structural panels or rigid ceiling use purlins to brace all
Flat TC @ 24" OC, all BC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50



R=18 U=6 W=7 826" (7 826" min)
R=69 PLF U=7 PLF W=4-4-4

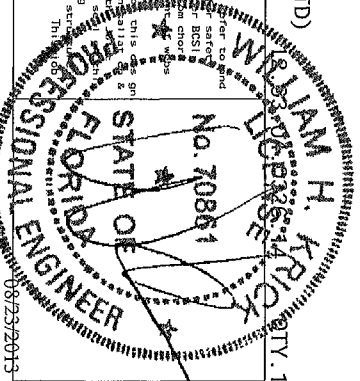
PLT TYP Wave Design Crit FBC2010Com/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing, and bracing. Follow the latest edition of BCSI (Building Components Safety Information by TPI and WDO) practices prior to performing these functions. Installers shall provide temporary bracing unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached ceiling. Lateral bracing shall be provided for permanent lateral restraint. Bracing shall have a proper installed per BCSI section 89 B7 or B10 as applicable.

The Building Components Group Inc. (BCSI) shall not be responsible for any deviation from this design and shall not be responsible for any damage to the building or its contents. The user shall be responsible for the design and use of this design for any other purpose. The user shall be responsible for the design and use of this design for any other purpose. The user shall be responsible for the design and use of this design for any other purpose.

ALPINE
ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0278



TC LL	20 0 PSF	REF	R487-- 29276
TC DL	7 0 PSF	DATE	08/23/13
BC DL	10 0 PSF	DRW	HCUSR487 13235031
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN-	311426
DUR. FAC.	1.25		
SPACING	24 0"	JREF-	1U21487_Z01

C13 WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED

NOTES

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE FOR MINIMUM ALTERNATIVE BRACING RE-RUN DESIGN WITH APPROPRIATE BRACING

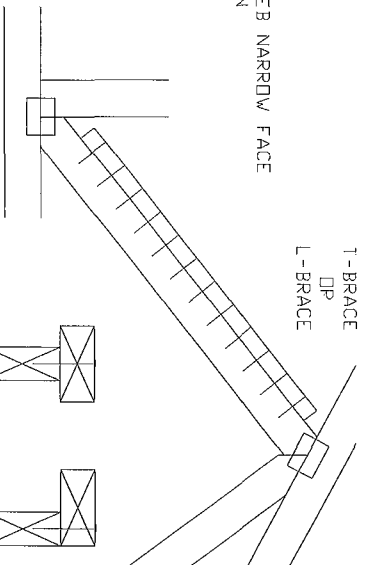
WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 DR 2X4	1 ROW	2X4	1 2X4
2X3 DR 2X4	2 ROWS	2X6	2 2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

1. BRACE L BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN

2. CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB

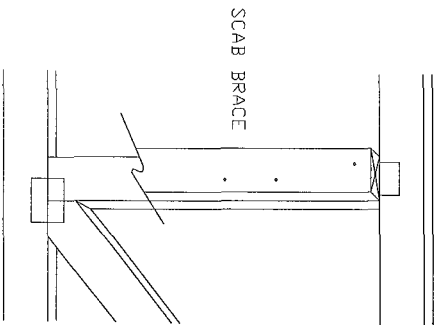
T-BRACING OR L-BRACING

APPLY TO EITHER SIDE OF WEB NARROW FACE ATTACH WITH 10d BOX OR GUN (0128 x 3 MIN) NAILS AT 6' OC BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING

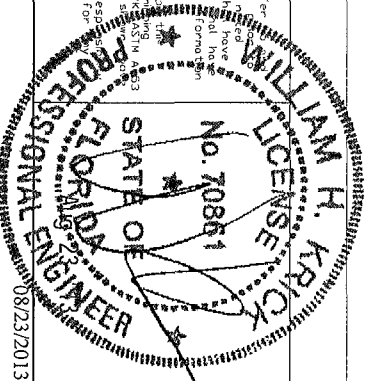
APPLY SCABS TO WIDE FACE OF WEB NO MORE THAN (1) SCAB PER FACE ATTACH WITH 10d BOX OR GUN (0128 x 3 MIN) NAILS AT 6' OC BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



Building Components Group Inc.

Earth City MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
 1. THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED
 2. CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB
 3. BRACE L BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN
 4. APPLY TO EITHER SIDE OF WEB NARROW FACE ATTACH WITH 10d BOX OR GUN (0128 x 3 MIN) NAILS AT 6' OC BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH
 5. APPLY SCABS TO WIDE FACE OF WEB NO MORE THAN (1) SCAB PER FACE ATTACH WITH 10d BOX OR GUN (0128 x 3 MIN) NAILS AT 6' OC BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH
 6. A SEAL ON THIS DRAWING OR COVER PAGE INDICATES ACCEPTANCE AND PROFESSIONAL ENGINEERING RESPONSIBILITY
 7. THE CROSS COMPONENT DESIGN SHOWN IS THE SUGGESTED AND USE OF THIS COMPONENT FOR ANY OTHER APPLICATION IS AT THE USER'S RISK
 8. FOR MORE INFORMATION GO TO THE WEBSITE WWW.BUILDINGCOMPONENTS.COM
 9. BUILDING COMPONENTS GROUP INC. 1111 Bldg. www.bcg.com TEL: 314.221.1111 FAX: 314.221.1112



TC LL	PSF	REF	CLB SUBST
TC DL	PSF	DATE	1/1/09
BC DL	PSF	DRWG	BRCLBSUB0109
BC LL	PSF		
TOT LD	PSF		
DUR FAC			
SPACING			

NAIL FACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS, GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCE THIS DETAIL

LOAD PERPENDICULAR TO GRAIN

A - EDGE DISTANCE (6 NAIL DIAMETERS)

B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)

C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

A - EDGE DISTANCE (6 NAIL DIAMETERS)

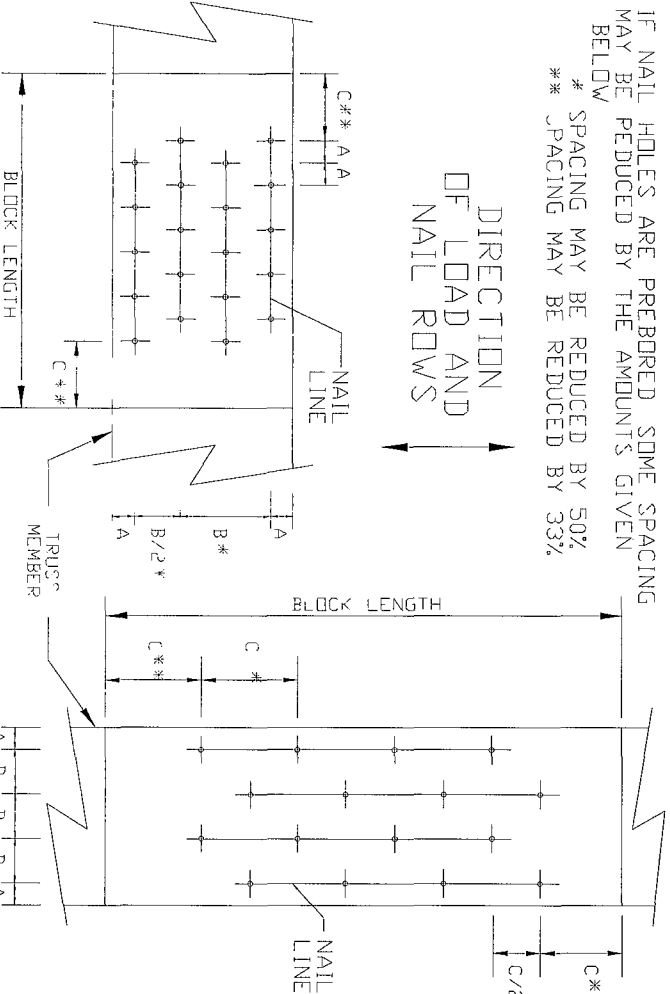
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)

D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW

* SPACING MAY BE REDUCED BY 50%
** SPACING MAY BE REDUCED BY 33%

DIRECTION
OF LOAD AND
NAIL ROWS



MINIMUM NAIL SPACING DISTANCES

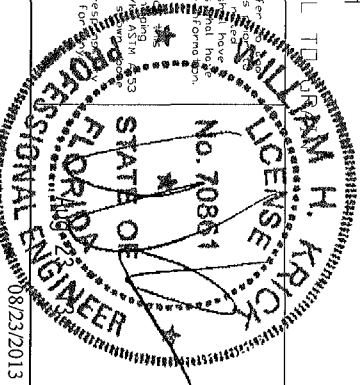
NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BDX (0113 X 2.5 MIN)	3/4	1 3/8	1 3/4	7/8
10d BDX (0128 X 3, MIN)	7/8	1 5/8	2"	1
12d BDX (0128 X 3.25" MIN)	7/8	1 5/8	2	1
16d BDX (0135 X 3.5 MIN)	7/8	1 5/8	2 1/8	1 1/8"
20d BDX (0148 X 4 MIN)	1	1 7/8	2 1/4	1 1/8
8d COMMON (0131 X 2.5 MIN)	7/8	1 5/8	2	1
10d COMMON (0148 X 3.25 MIN)	1	1 7/8	2 1/4	1 1/8
12d COMMON (0148 X 3.25 MIN)	1	1 7/8	2 1/4	1 1/8
16d COMMON (0162 X 3.5 MIN)	1	2	2 1/2	1 1/4
GUN (0120 X 2.5 MIN)	3/4	1 1/2	1 7/8	1"
GUN (0131 X 2.5 MIN)	7/8	1 5/8	2"	1"
GUN (0120 X 3 MIN)	3/4"	1 1/2"	1 7/8	1
GUN (0131 X 3, MIN)	7/8	1 5/8	2	1"



Building Components Group Inc.

Earth City MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET. The engineer is responsible for the design and construction of the structure. The contractor is responsible for the construction of the structure. The engineer is responsible for the design and construction of the structure. The contractor is responsible for the construction of the structure.



REF	NAIL SPACE
DATE	1/1/09
DRWG	CNNAILSP0109