

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
 Florida Engineering Certificate of Authorization Number: 0 278
 Florida Certificate of Product Approval # FL1999
 Page 1 of 2 Document ID: ITKU8228Z0112082331

Truss Fabricator: Anderson Truss Company
 Job Identification: 8-160--OWNER BUILDER Charles Meade -- , **
 Truss Count: 127
 Model Code: Florida Building Code 2004 and 2006 Supplement
 Truss Criteria: ANSI/TPI-2002(STD)/FBC
 Engineering Software: Alpine Software, Versions 7.36, 7.37.
 Structural Engineer of Record: The identity of the structural EOR did not exist as of the seal date per section 61G15-31.003(5a) of the FAC
 Address:
 Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
 Floor - N/A
 Wind - 110 MPH ASCE 7-02 -Closed

[Handwritten signature]

Seal Date: 09/12/2008

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

-Truss Design Engineer-
 James F. Collins Jr.

Florida License Number: 52212
 1950 Marley Drive
 Haines City, FL 33844

Details: TCFILLER-BCFILLER-REPBCEFIL-BRCLBSUB-CNBRGBLK-PIGBACKA-PIGBACKB-

#	Ref	Description	Drawing#	Date
1	71258--T31		08255028	09/11/08
2	71259--T32		08255029	09/11/08
3	71260--T33		08255030	09/11/08
4	71261--T103		08255002	09/11/08
5	71262--T35		08255031	09/11/08
6	71263--T36		08255032	09/11/08
7	71264--T37		08255033	09/11/08
8	71265--T38		08255034	09/11/08
9	71266--T1		08256037	09/12/08
10	71267--T39		08255008	09/11/08
11	71268--T44		08255009	09/11/08
12	71269--T59		08255010	09/11/08
13	71270--T57		08256008	09/12/08
14	71271--T95		08256031	09/12/08
15	71272--T60		08256009	09/12/08
16	71273--T61		08256010	09/12/08
17	71274--T63		08256011	09/12/08
18	71275--T78		08256012	09/12/08
19	71276--T79		08256021	09/12/08
20	71277--T80		08256022	09/12/08
21	71278--T113		08255005	09/11/08
22	71279--T115		08255007	09/11/08
23	71280--T64		08255050	09/11/08
24	71281--T114		08255006	09/11/08
25	71282--T89		08255056	09/11/08
26	71283--T82		08256023	09/12/08
27	71284--T91		08256024	09/12/08
28	71285--T90		08256025	09/12/08
29	71286--T99		08256026	09/12/08
30	71287--T83		08256013	09/12/08
31	71288--T85		08256027	09/12/08
32	71289--T84		08256014	09/12/08
33	71290--T22		08256028	09/12/08
34	71291--T62		08256029	09/12/08
35	71292--T34		08256015	09/12/08
36	71293--T87		08256016	09/12/08

#	Ref	Description	Drawing#	Date
37	71294--T88		08256017	09/12/08
38	71295--T86		08256018	09/12/08
39	71296--T40		08256019	09/12/08
40	71297--T81		08256020	09/12/08
41	71298--T29		08256030	09/12/08
42	71299--T68		08255001	09/11/08
43	71300--T69		08255011	09/11/08
44	71301--T70		08255012	09/11/08
45	71302--T71		08255013	09/11/08
46	71303--T136		08255014	09/11/08
47	71304--T30		08255015	09/11/08
48	71305--T17		08255016	09/11/08
49	71306--T18		08256032	09/12/08
50	71307--T75		08255017	09/11/08
51	71308--T28		08256033	09/12/08
52	71309--T2		08255020	09/11/08
53	71310--T3		08255027	09/11/08
54	71311--T4		08255035	09/11/08
55	71312--T5		08255041	09/11/08
56	71313--T6		08256034	09/12/08
57	71314--T27		08255026	09/11/08
58	71315--T16		08255018	09/11/08
59	71316--T24		08255023	09/11/08
60	71317--T21		08255021	09/11/08
61	71318--T100		08255018	09/11/08
62	71319--T20		08256035	09/12/08
63	71320--T15		08255019	09/11/08
64	71321--T74		08255020	09/11/08
65	71322--T105		08255003	09/11/08
66	71323--T67		08255052	09/11/08
67	71324--T73		08255054	09/11/08
68	71325--T66		08255051	09/11/08
69	71326--T12		08255009	09/11/08
70	71327--T7		08255053	09/11/08
71	71328--T11		08255004	09/11/08
72	71329--T10		08255001	09/11/08



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Page 2 of 2 Document ID:1TKU8228Z0112082331

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Truss Criteria: ANSI/TPI-2002(STD)/FBC

Engineering Software: Alpine Software, Versions 7.36, 7.37.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-02 -Closed

Seal Date: 09/12/2008

-Truss Design Engineer-

James F. Collins Jr.

Florida License Number: 52212

1950 Marley Drive

Haines City, FL 33844

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: TCFILLER-BCFILLER-REPBCEIL-BRCLBSUB-CNBRGBLK-PIGBACKA-PIGBACKB-

#	Ref	Description	Drawing#	Date
73	71330--T9		08255057	09/11/08
74	71331--T8		08255055	09/11/08
75	71332--T13		08255016	09/11/08
76	71333--T19		08255019	09/11/08
77	71334--T14		08255017	09/11/08
78	71335--T25		08255024	09/11/08
79	71336--T56		08255048	09/11/08
80	71337--T122		08255012	09/11/08
81	71338--T123		08255013	09/11/08
82	71339--T124		08255014	09/11/08
83	71340--T26		08255025	09/11/08
84	71341--T125		08255015	09/11/08
85	71342--T54		08255046	09/11/08
86	71343--T51		08255043	09/11/08
87	71344--T58		08255049	09/11/08
88	71345--T50		08255042	09/11/08
89	71346--T121		08255011	09/11/08
90	71347--T23		08255022	09/11/08
91	71348--T46		08255037	09/11/08
92	71349--T119		08255008	09/11/08
93	71350--T49		08255040	09/11/08
94	71351--T48		08255039	09/11/08
95	71352--T47		08255038	09/11/08
96	71353--T120		08255010	09/11/08
97	71354--T55		08255047	09/11/08
98	71355--T52		08255044	09/11/08
99	71356--T53		08255045	09/11/08
100	71357--T45		08255036	09/11/08
101	71358--T146		08255021	09/11/08
102	71359--T141		08256036	09/12/08
103	71360--T140		08255002	09/11/08
104	71361--T111		08255003	09/11/08
105	71362--T130		08255004	09/11/08
106	71363--T147		08255005	09/11/08
107	71364--T72		08255006	09/11/08
108	71365--T131		08255022	09/11/08

#	Ref	Description	Drawing#	Date
109	71366--T133		08255007	09/11/08
110	71367--T65		08255023	09/11/08
111	71368--T92		08256038	09/12/08
112	71369--T76		08255024	09/11/08
113	71370--T41		08255025	09/11/08
114	71371--T42		08256040	09/12/08
115	71372--T43		08256041	09/12/08
116	71373--T93		08255026	09/11/08
117	71374--T94		08256042	09/12/08
118	71375--T77		08255027	09/11/08
119	71376--T96		08256043	09/12/08
120	71377--T97		08255028	09/11/08
121	71378--T98		08256045	09/12/08
122	71379--T101		08255029	09/11/08
123	71380--T102		08256046	09/12/08
124	71381--T108		08255030	09/11/08
125	71382--T107		08256047	09/12/08
126	71383--T106		08255031	09/11/08
127	71384--T104		08256039	09/12/08





110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, iw=1.00 Gcpi (+/-)=0.18

Wind reactions based on MFRS pressures.

Calculated horizontal deflection is 0.20" due to live load and 0.34" due to dead load.

due to dead load.

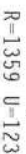
(A) 2x4 #3 or better "T" brace, 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5".min.)nails @ 6" OC.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

Calculated vertical deflection is 0.43" due to live load and 0.73" due to dead load at $x = 24-7-12$.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

QTY:1

FL/-/4/-/-/R/-/

Scale = .125"/Ft.

WARNING: THESE FIBROE EXISTENCE CAN BE FABRICATED, HANDLED, SHIPPING, INSTALLING AND BROACHING REFER TO 8631 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TROSS PAPER INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND NICK (NICK) THROSS CONSULTING OF AMERICA, 62000 ENTERPRISE LANE, MADISON, WI, 53719 FOR SAFETY PRACTICES PRIOR TO THROSS CONSULTING OF AMERICA. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED TOP CEILING.

1. *Journal of the American Medical Association*, 1997; 277: 1039-1043.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH

TYPE: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES,

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MOS (NATIONAL DESIGN SPEC, BY AFSPA) AND IPI. CONNECTOR PLATES ARE MADE OF 3019/AL6CA (2011/AL6CA) WITH 6061 T635 CORRECTION. 11W BCC

CONNECTION MADE OF 20/10/1000 (H, H/33/K) ASIN 6553 GRADE 40/60 (H, K/H, 55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 1604-3

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC 3. A SEAL ON THIS

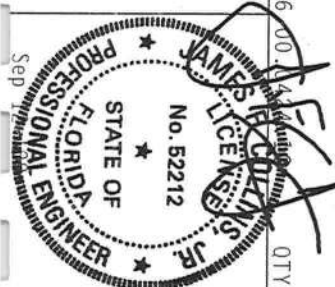
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

BUILDING DESIGNER PER ANSI/FP1 1 SEC. 2.

100

ITW Building Components Group Inc
Haines City, FL 33844
FL COA #0 278



TC LL	20.0 PSF	REF	R8228- 71258
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCU8R8228 08Z50Z8
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	102474
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	UREF-	1TKU8228Z01

Roof overhang supports 2.00 psf soffit load.

(B) 1x4 #3Sb SPF-5 or better "T" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

(A) 2x4 #3 or better "I" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.



PLT TYP. Wave

QTY:1

FL/-/4/-/-/R/-/

Scale = .125" / Ft.

Design Crit: TPI-2002(STD)/FBC

$$\frac{Cq}{RT}=1.00(1.25)/0(0,$$

7.36.00.042 QTY:1

FL/-/4/-/-/R/-/

Scale = .125" / Ft.

WARNING:—FIBERS EXPOSED DURING CASE REMEDIATION, HANDLING, SHIPPING, INSTALLATION AND DEMOLITION OF CASES (CONTAINING COMPARTMENT SAFETY INFORMATION), PUBLISHED BY THE CROSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND MR. GOOD (GOOD TRADING COMPANY OF AMERICA, 62000 ENTERPRISE LANE, MANSION, W 52129) FOR SAFETY PRACTICES AND PRICE TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED BOLTED CEILING.

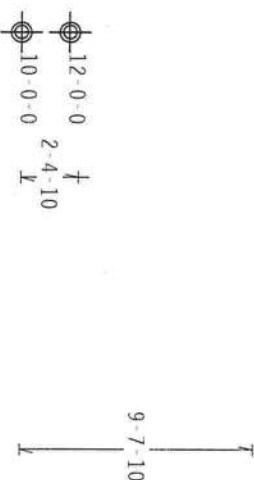
ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0078

Professional Engineer
State of Florida
No. 52212
JAMES E. COLLINS, JR.
LICENSE

TC LL	20.0 PSF	REF	R8228- 71260
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255030
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	102490
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

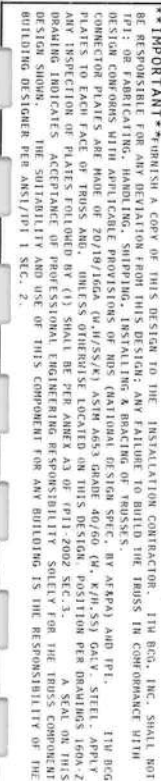
110 mph wind 15.38 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{cpl}(+/-)=0.55$



Scale = .125"/Ft.

Figure 1

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/AP1 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71261
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255002
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	40532
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

WEDS	2X4	SP	#3
Filler	2X4	SP	#2 Dense

See DWGS TCFILLER0207 and BCFILLER0207 for filler details.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

In lieu of structural panels use purllins to brace all flat TC @ 24 OC.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.88 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI(+/-)=0.55

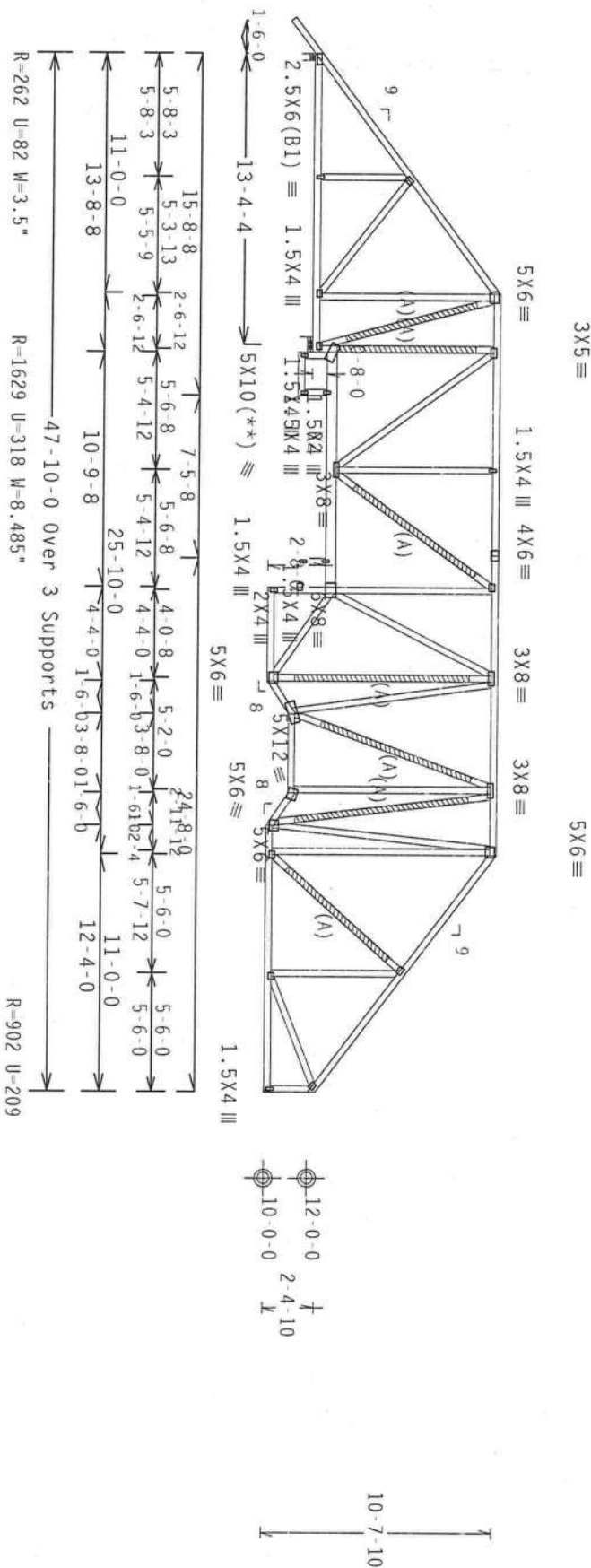
Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Trusses to be spaced at 16.0" OC maximum.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

7.36.00 1048 Quantity QTY: 1

FL/-/4/-/-/R/-/-

Scale = .125" / Ft.

[illegible]

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. 11W BCG, INC. SHALL NOT

IP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES

PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, NOTATION CONNECTION PLATES ARE MADE OF 20/10/100K (W, H/SS/K) ASTM A653 GRADE 40/60 (W, K/H).

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ALPINE

ITW Building Components Group Inc.
 Haines City, FL 33844
 FL COA #0778

[illegible]

TC LL	20.0 PSF	REF	R8228- 71262
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCSR8228 08255031
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SE0N-	102521
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	REF-	ITKU8228201

110 mph wind, 16.38 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.55$

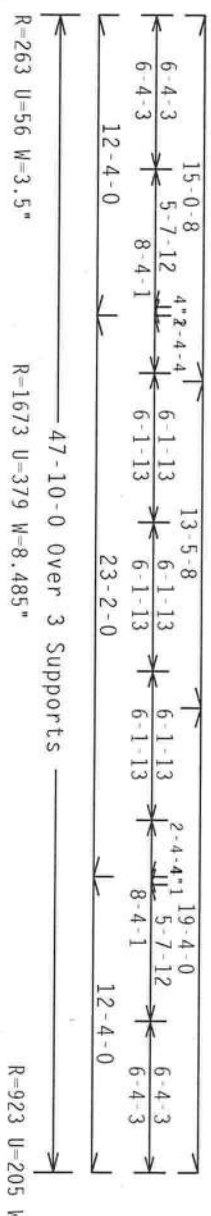
Wind reactions based on MMFRS pressures.

See DWGS ICFILLER0207 and BCFILLER0207 for filler details.

(B) 2x4 #3 or better "T" brace, 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

Trusses to be spaced at 16.0" OC maximum.

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Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$

QTY:1

Scale = .125"/Ft.

6.00
JAMES COLLINS, JR.
RECEIVED
No. 52749
QT



ALPINE

Haines City, FL 33844
FL COA #0278

STATE OF FLORIDA
PROFESSIONAL ENGINEER

No. 52742

SAMUEL J. COLLINS, JR.

OT

Sep 12 '08

TC LL	20.0 PSF	REF	R8228- 71263
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255032
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	102536
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Filler 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

(A) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5".min.)nails @ 6" OC.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Trusses to be spaced at 16.0" OC maximum.

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110 mph wind, 16.88 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCPI (+/-)-0.55

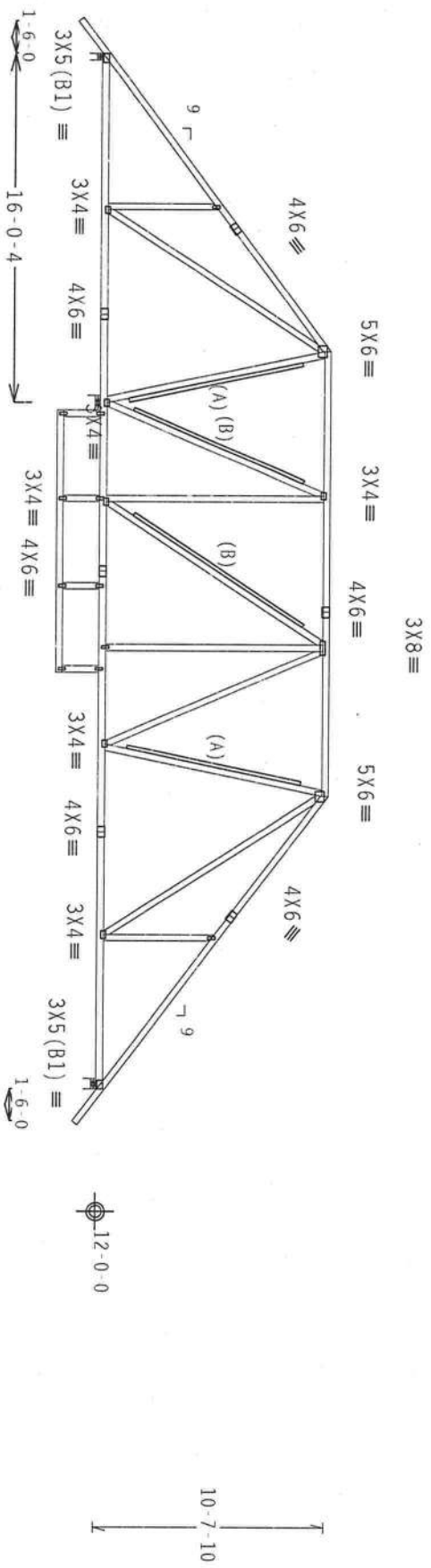
Wind reactions based on MMFRS pressures.

See DWGS TC FILLER0207 and BC FILLER0207 for filler details.

(B) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5".min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



16-4-8 6-3-12 4*1 6-10-0 12-1-8 6-10-0 6-10-0 4*1 19-4-0 7-0-3 7-0-3
7-0-3 9-0-1 4-5-12 6-10-0 4-5-12 9-0-1 7-0-3 13-8-0
13-8-0 20-6-0 13-8-0
47-10-0 Over 3 Supports
R-350 U=77 W=3.5"
R-1621 U=370 W=8.485"
R-888 U=200 W=6"

Note: All Plates Are 1.5X4 Except As Shown.
PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.00

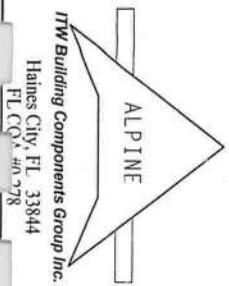
QTY:1 FL/-/4/-/R/-

Scale = .125"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 HUNTER LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304, AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 6300 GREENHURST INDUSTRIAL PARK, SUITE 100, GREENHURST, GA, 30349, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN. ANY FAILURE OF THE TRUSS OR COMPONENTS WITHIN THE DESIGN SHALL BE THE RESPONSIBILITY OF THE DESIGNER. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.

DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.



TC LL	20.0 PSF	REF	R8228- 71264
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255033
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEON-	102548
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	UREF-	1TKU8228201

110 mph wind, 17.38 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCPI (+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWGS TCFILLER0207 and BCFILLER0207 for filler details.

(B) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

In lieu of structural panels use purtins to brace all flat TC @ 24 OC.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



R=862 U=198 W=3.5"

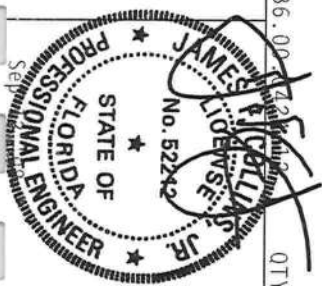
Design Crit: TPI-2002(STD)/FBC

Scale = .125" / Ft.



ALPINE

11W Building Components Group Inc
Haines City, FL 33844
FL COA #0078



TC LL	20.0 PSF	REF	R8228- 71265
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255034
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	102563
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

2 COMPLETE TRIANGLES REQUIRED

Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)

Top chord: 1 row @12.00" o.c.

Webs : 1 Row @ 4" o.c.

Use equal spacing between rows and stagger nails in each row to avoid splitting.

Wind reactions based on MIFRS pressures.

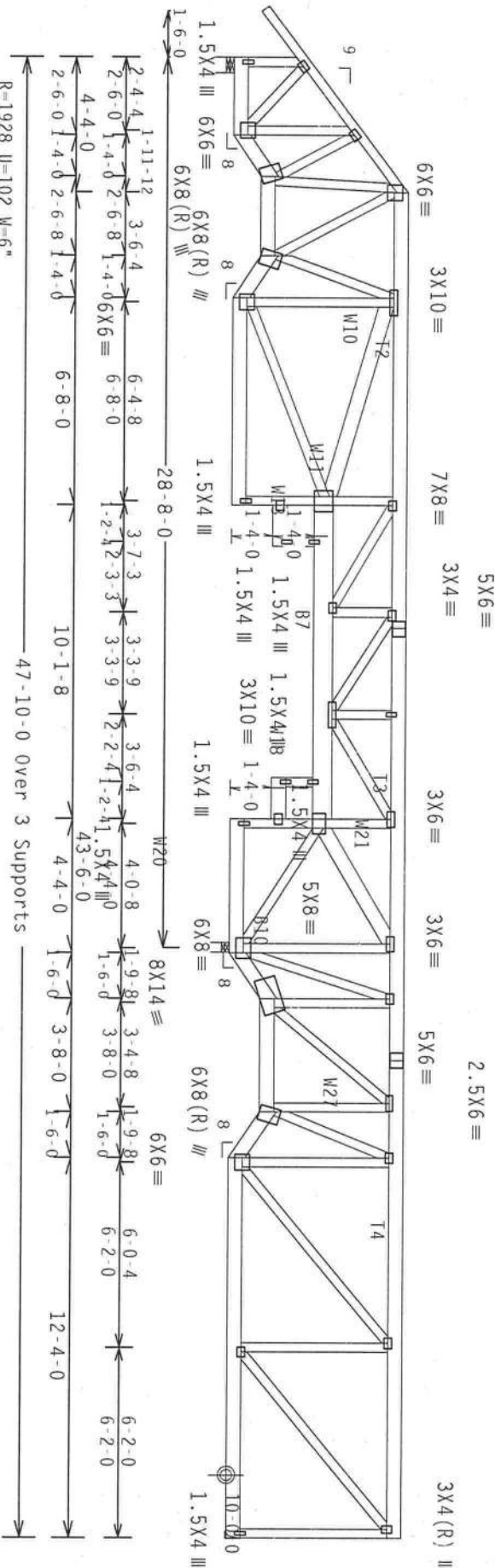
Roof overhang supports 2.00 psf soffit load.

Trusses to be spaced at 16.0" OC maximum.

#1 hip supports 4-4-0 jacks with no webs.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



R=5528 U=218 W=4

R=809 U=24

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

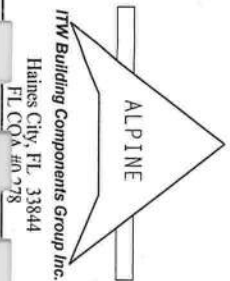
QTY:1 FL/-/4/-/-/R/-

Scale = .1875"/Ft

FOR SALE—FLOORS BUILDING EXTERIOR CASE IN FABRICATION. "MAINTENANCE, SHIPPING, INSTALLING AND BRACING. BEST" (BOLTING) BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY THE (TRUSS PEALE INSTITUTE, 218 NORTH BOSTON, 312, 625-1370). FOR SALE (WOOD TRUSS CONNECTIONS) OF AMERICA, 6150 UNIVERSITY AVENUE, 30353, FOR SAFETY PRACTICES AND PREVENTING THESE CONDITIONS. OTHERWISE IMPLICATED FOR GOOD SHALL HAVE PROBABLY ATTACHED STRUCTURAL PANELS AND BOTTOM GOOD SHALL HAVE A PROBABLY ATTACHED RIGID CEILING.

JAMES
LIT
CEASE
No. 52212
JR.

TC LL	20.0 PSF	REF	R8228- 71266
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCSR0228 08256037
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	103928
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JRF-	1TK8228201

[illegible]

STATE OF
FLORIDA
PROFESSIONAL ENGINEER

BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 103928
DUR.FAC.	1.25	FROM AH
SPACING	SEE ABOVE	JRF- 1TK8228201

110 mph wind, 17.38 ft mean hgt, ASCE 7-02, PART. ENCL. bldg, not located within 5.83 ft from roof edge, CAT 1, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCp1(+/-)=0.55

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.14" due to live load and 0.23" due to dead load.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Trusses to be spaced at 16.0" OC maximum.

Collar tie braced with continuous lateral bracing at 24" OC, or rigid ceiling.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

WARNING: Furnish a copy of this DMG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

 $1.5 \times 4 =$

11-7-10



Design Crit: TPI-2002(STD)/FBC

QTY: 1

FL/-/4/-/-/R/-

Scale = .125" / Ft.

No. 52212

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0778

No. 52212
 STATE OF
 FLORIDA
 PROFESSIONAL ENGINEER
 J.R.

TC LL	20.0 PSF	REF	R8228-71267
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255008
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	104427
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1TKU8228Z01

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

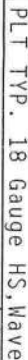
DEFLECTION MEETS L/360 LIVE AND L/240 TOTAL LOAD.

110 mph wind, 17.38 ft mean hgt, ASCE 7-02, PART_ENC, bldg, not
located within 5.83 ft from roof edge, CAT 1, EXP B, wind TC DL=5.0
psf, wind BC DL=5.0 psf, $I_w=1.00$ GCp1(+/-)=0.55

(A) Continuous lateral bracing equally spaced on member.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 14-2-0 to 38-3-0.

Calculated vertical deflection is 0.38" due to live load and 0.76" due to dead load at $X = 33-10-0$.


$$Cq/RT=1.00(1.25)/0(0)$$

WARNING: THESE REQUIREMENTS ARE THE RESULT OF A RECENT COURT DECISION IN AN OSHA COMPLAINT AGAINST ONE OF OUR CLIENTS. THIS COMPLAINT WAS SETTLED OUT OF COURT AND WE HAVE BEEN ADVISED THAT THE COMPLAINT WILL BE FILED WITH THE DISTRICT COURT IN NEW YORK CITY. THE COMPLAINT ALLEGES THAT THE CLIENT'S EMPLOYEES WERE NOT PROPERLY TRAINED IN THE USE OF LIFELINES AND FALL PROTECTION EQUIPMENT. THE COMPLAINT ALSO ALLEGES THAT THE CLIENT'S EMPLOYEES WERE NOT PROPERLY TRAINED IN THE USE OF LIFELINES AND FALL PROTECTION EQUIPMENT. THE COMPLAINT ALSO ALLEGES THAT THE CLIENT'S EMPLOYEES WERE NOT PROPERLY TRAINED IN THE USE OF LIFELINES AND FALL PROTECTION EQUIPMENT.

QTY:1

FL/-/4/-/-/R/-/

Scale = .125"/Ft.

ICLL 20.0

REF R8228- 71268

TC DL	10.0
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DATE 09/11/08

BC	DL	10.0
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DRW HC11SR8228 082550

BC 11 00

ENC DEF/DEF

	6
TOTAL	100

1994-1995

40.0	101.50
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SEQUENCE - 104439

DUR. FAC. 1.25

FROM AH



ALPINE

ITW Building Components Group Inc.
 Haines City, FL 33844
 FL COA #11278

0422
JAMES R. COLEMAN
STATE OF FLORIDA
PROFESSIONAL ENGINEER
No. E2212
QTY

FL/-4/-/R/-		Scale = .125"/ft.
TC LL	20.0 PSF	REF R8228- 71268
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUSR8228 08255009
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 104459
DUR.FAC.	1.25	FROM AH
SPACING	16.0"	JREF- 1TK8228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #1 Dense :B1, B7 2x6 SP #2: :B3 2x6 SP SS:
B4, B6 2x4 SP #2 Dense:
Webs 2x4 SP #3 :W2, W4, W5, W10 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.21" due to live load and 0.42" due to dead load.

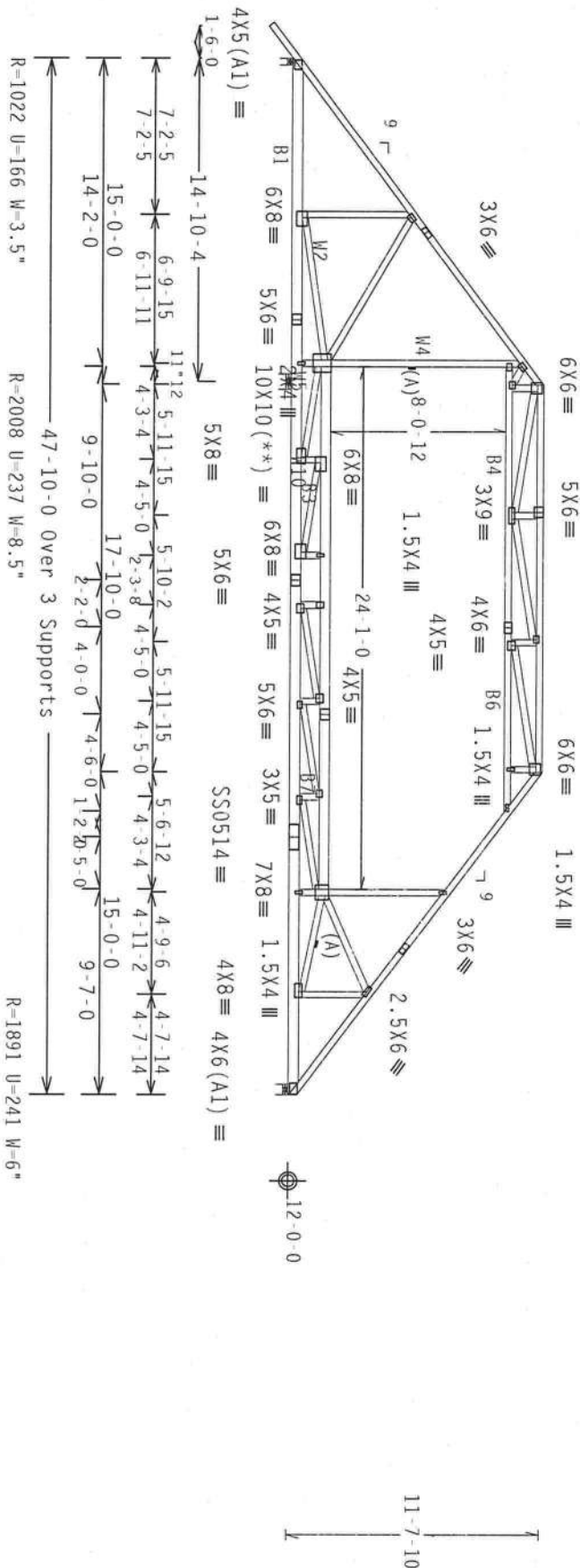
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.
110 mph wind, 17.38 ft mean hgt, ASCE 7-02, PART-ENC. bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCpl(+/-)=0.55
Wind reactions based on MWFRS pressures.
(A) Continuous lateral bracing equally spaced on member.
Trusses to be spaced at 16.0" OC maximum.
BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 14-2-0 to 38-3-0.
Calculated vertical deflection is 0.40" due to live load and 0.83" due to dead load at X = 29-8-8.



Roof overhang supports 2.00 psf soffit load.
Calculated horizontal deflection is 0.17" due to live load and 0.36" due to dead load.

Trusses to be spaced at 16.0" OC maximum.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

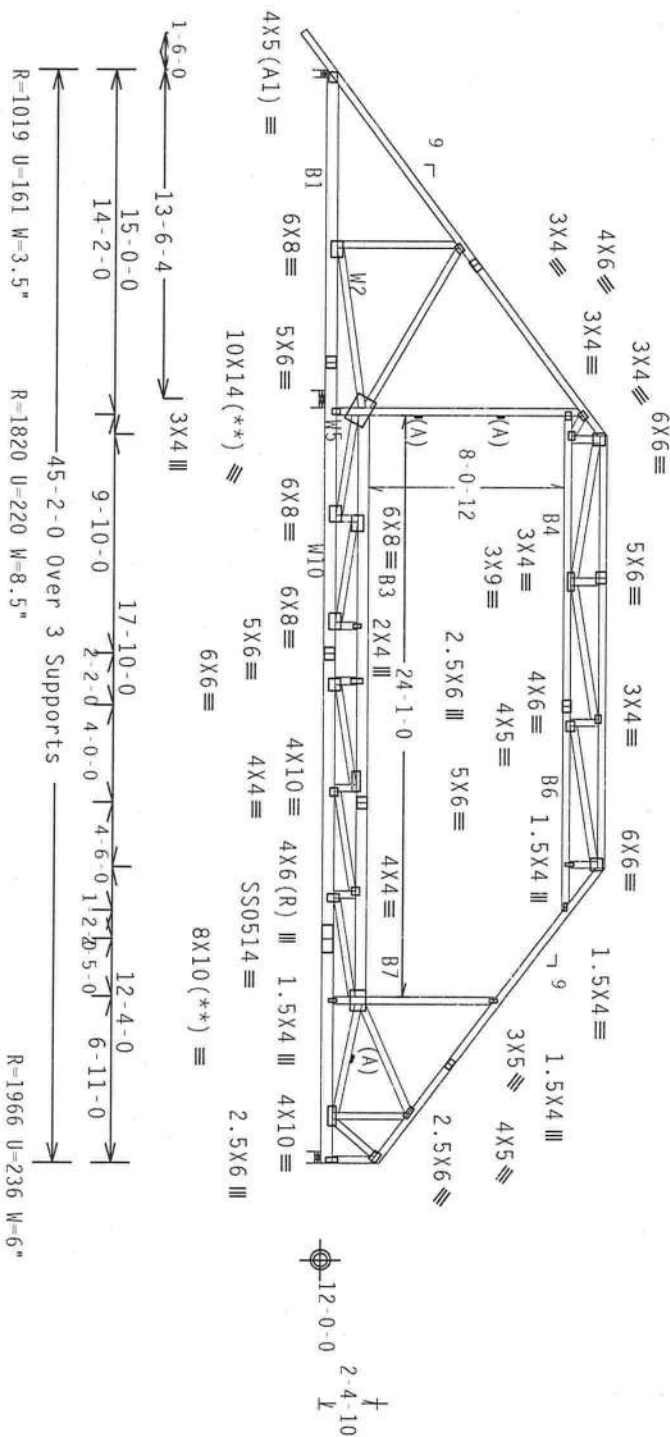
110 mph wind, 17.38 ft mean hgt., ASCE 7-02, PART 1 ENC. bldg. mod located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCP(+/-)=0.55

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 14-2-0 to 38-3-0.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$

7.36.00.042 QTY:1

QTY:1

FL/-/4/-/-/R/-/

Scale = .125"/Ft.

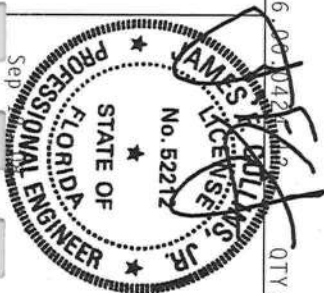
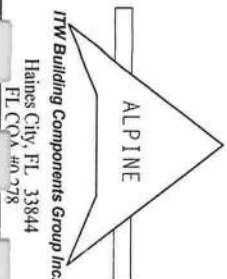
"WARNING"—THESE BUILDING COMPONENTS ARE IN FACTORIAL, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO SC-1 (BUILDING EXISTENCE SAFETY INFORMATION), PUBLISHED BY TPI (TROSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NRC (NATIONAL TRUSS COMPANY OF AMERICA, 6500 ENTERPRISE LANE, MARIETTA, GA 30159). FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED FIELD CELLING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIAA) AND TPI CONNECTOR PLATES ARE MADE OF 304/316L STAINLESS STEEL.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY F

BUILDING DESIGNER PER ANSI/THI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 71270
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUSR8228 08256008
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	104471
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.18" due to live load and 0.36" due to dead load.

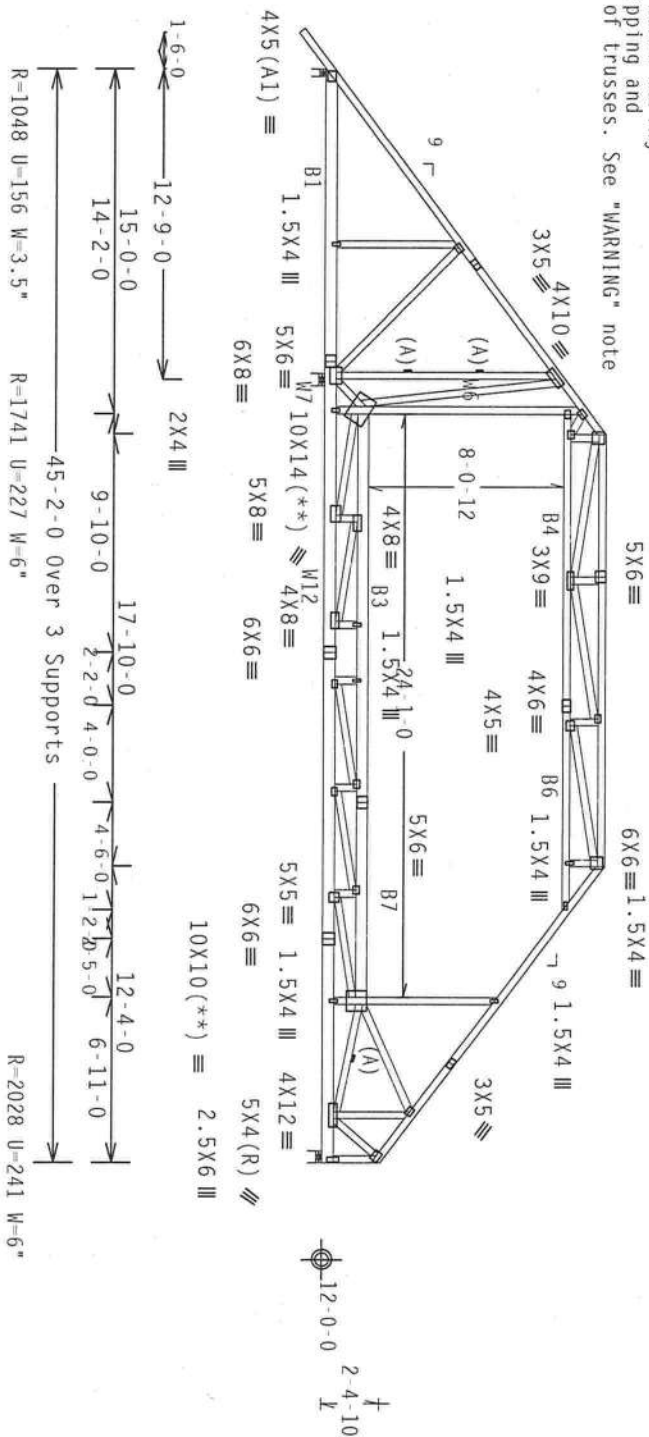
(A) Continuous lateral bracing equally spaced on member.

Trusses to be spaced at 16.0" OC maximum.

Collar tie braced with continuous lateral bracing at 24" OC. on rigid ceiling.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$

7.36

QTY: 1

FL/-/4/-/-/R/-/

Scale = .125"/Ft.

WARNING: THESE ARE HIGHLY EXTENSIVE CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY IN OPERATION), NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND NICK GOOD THRESHING COUNCIL OF AMERICA, 65000 ENTERPRISE LANE, MALDEN, MA 02148 FOR SAFETY PRACTICES PRIOR TO TRANSPORTING THESE FURNACES. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

No. 52212

4.

20

10.0 per cent

DATE _____

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(**) 2 place(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purins to brace all flat TC @ 24" OC.

BC attic room floor loading: LL = 40.00 psf, DL = 10.00 psf; from 14-2.0 to 38.3-0.

Calculated vertical deflection is 0.35" due to live load and 0.72" due to dead load at $X = 29.5$ -0.



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0778



TC LL	20.0 PSF	REF	R8228 - 71272
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCSR8228 08256009
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	104497
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

Roof overhang supports 2.00 psf soffit load.
Calculated horizontal deflection is 0.21" due to live load and 0.44" due to dead load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 17.38 ft mean hgt, ASCE 7-02, PART-ENC. bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $1w=1.00 Gcp(1+/-)=0.55$

Wind reactions based on MMFRS pressures.

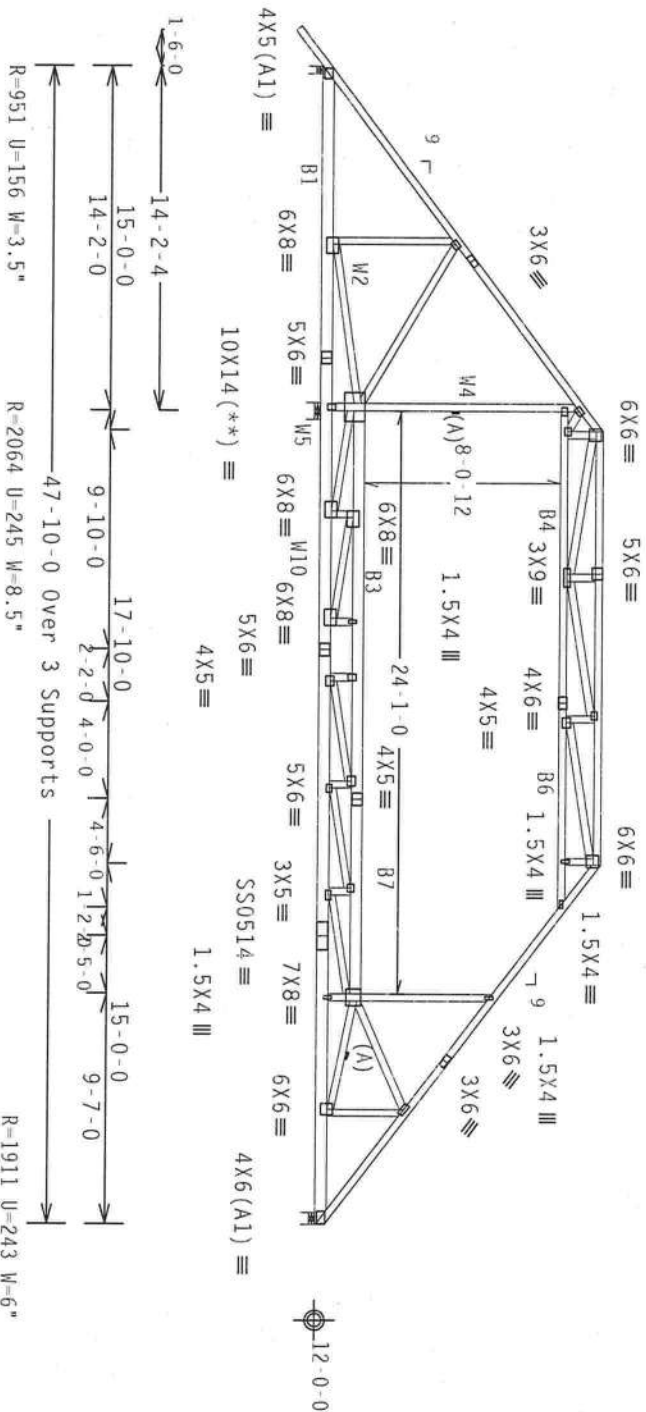
(A) Continuous lateral bracing equally spaced on member.

Trusses to be spaced at 16.0" OC maximum.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 14-2-0 to 38-3-0.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Calculated vertical deflection is 0.42" due to live load and 0.86" due to dead load at X = 29-8-8.



Note: All Plates Are 3X4 Except As Shown.

PLT TYP. 18 Gauge HS, Wave

Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$
~~7.36.00~~

QTY:1

FL/-/4/-/-/R/-/

Scale = .125"/ft.

[illegible]

5.00
JAMES E. COLLINS JR.
LIBRARIAN
No. 52217
OT



ALPINE

ITW Building Components Group Inc.
 Haines City, FL 33844
 FL COA #0 378

STATE OF
FLORIDA
PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R8228- 71273
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRN	HCSR8228 08256010
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SECN-	104512
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Collar tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: $TPI-2002(STD)/FBC$

PLT TYP. 18 Gauge HS, Wave

7.36.00

QTY:1

FL/-/4/-/-/R/-/-

Scale = .125"/Ft.

WARNING: THESE PRODUCTS EXPERIENCE CASE IN FABRICATION, HANDLING, SHIPPING, UNLOADING AND BRACKETING. REFER TO BCS1 (BUILDING CONSTRUCTION SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314 AND THE (800) 460-0078 TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MOBILE, AL 36619 FOR SAFETY PRECAUTIONS TO PREVENT THE MOST FATAL ACCIDENTS. UNLESS OTHERWISE INDICATED, THE ABOVE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

IMPORTANT TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

TYPE: ON FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN POSITION PER DRAWINGS 1604-2 CONNECTION PLATES MADE OF 20/10/1900 (H, H/35/R) 451H 4053 GRADE 40/50 (H, R/H/35) GALV. STEEL. APPLY

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/API 1 SEC. 2.

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11W Building Components Group Inc
Haines City, FL 33844
FL COA #0778

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 17.38 ft mean hgt, ASCE 7-02, PART 5, ENC. bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 Gcp1(+/-)=0.55

Wind reactions based on MFRS pressures.

(A) continuous lateral bracing equally spaced on member.

Trusses to be spaced at 16.0" OC maximum.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 14-2-0 to 38-3-0.

Calculated vertical deflection is 0.40" due to live load and 0.80" due to dead load at $X = 29-8-8$.



TC LL	20.0 PSF	REF	R8228- 71274
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCSR8228 08256011
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	104526
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TK8228Z01

Wind reactions based on MNFRS pressures.

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.27" due to live load and 0.46" due to dead load.

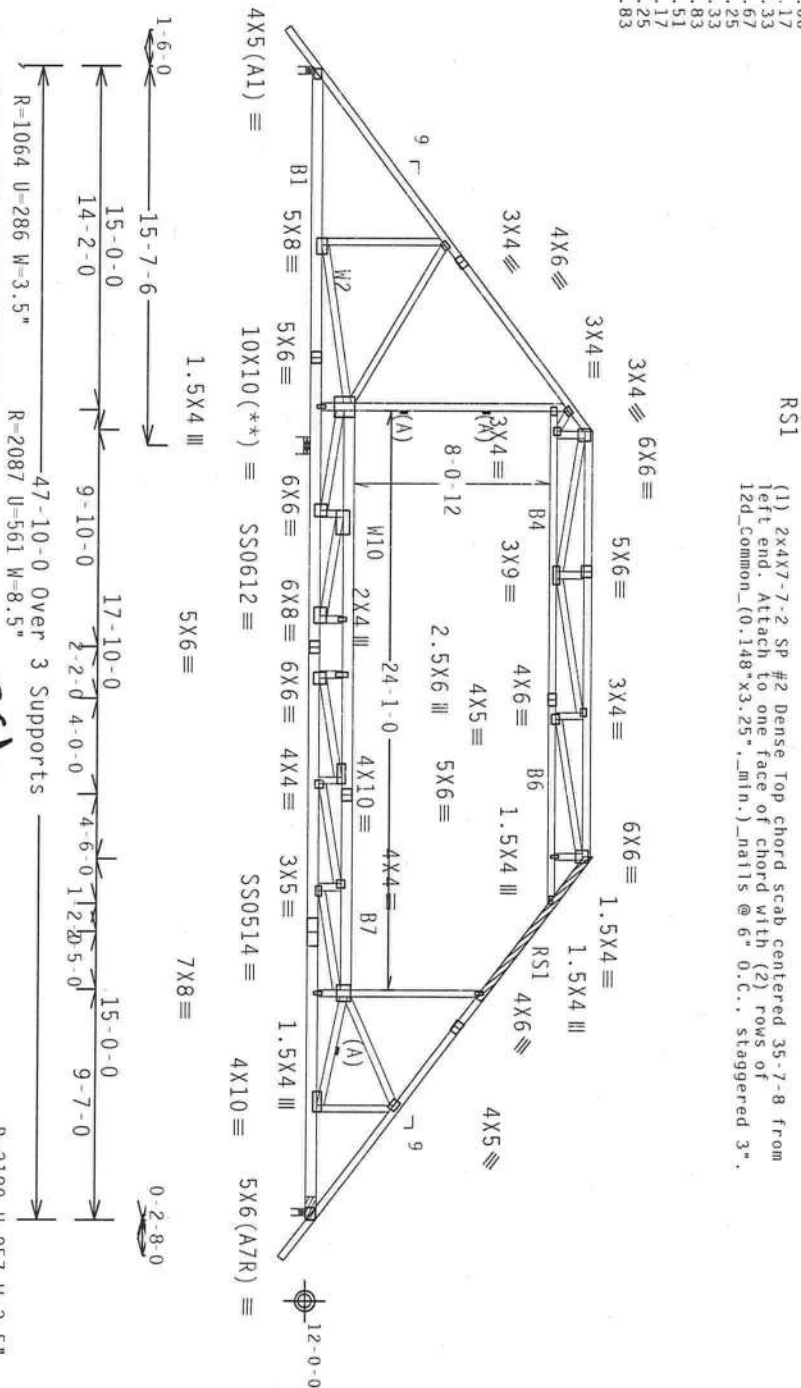
(A) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purllins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

SPECIAL LOADS

TC	From	47 PLF at 1.67 to	47 PLF at 0.00 to
TC	From	43 PLF at 0.00 to	43 PLF at 14.17 to
TC	From	57 PLF at 14.17 to	57 PLF at 34.33 to
TC	From	43 PLF at 34.33 to	43 PLF at 34.67 to
TC	From	60 PLF at 34.67 to	60 PLF at 38.25 to
TC	From	43 PLF at 38.25 to	43 PLF at 47.83 to
TC	From	123 PLF at 39.33 to	233 PLF at 49.51 to
TC	From	47 PLF at 47.83 to	47 PLF at 14.17 to
BC	From	13 PLF at 0.00 to	13 PLF at 14.17 to
BC	From	80 PLF at 14.17 to	80 PLF at 38.25 to
BC	From	13 PLF at 38.25 to	13 PLF at 47.83 to
TC	229 LB Conc.	Load at 14.17	Load at 47.83
TC	108 LB Conc.	Load at 14.17	Load at 47.83
BC	72 LB Conc.	Load at 38.25	Load at 38.25



PLT TYP. 18 Gauge HS, Wave

Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0$$
$$7.36\overline{00}$$

QTY:1

FL/-/4/-/-/R/-/

Scale = .125"/ft.

R=3189 U=857 W=3.5"

WARNING: THESE RESISTED EXTREME CASE IN FABRICATION, HANDLOAD, SHIPPING, INSTALLING AND BRACING REFER TO NCST (CONCRETE COMPOSITE) SAFETY INFORMATION, PUBLISHED BY THE (FIBERS PAPER INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND NRC (NUCLEAR REGULATORY COMMISSION), 6000 ROCKY HILLS DRIVE, SUITE 312, ALEXANDRIA, VA, 22314 FOR SAFETY PRACTICES PERTAINING TO PERFORMANCE OF AMERICA, UNLESS OTHERWISE INDICATED FOR OTHER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED SIDING CELLING.

James Perkins, Jr.
No. 52212
L.I.C.E.N.S.E.

ALPINE

ITW Building Components Group Inc

Haines City, FL 33844

Sep

45

JREF- 1T

201

SPECIAL LOADS

Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.29" due to live load and 0.49" due to dead load.

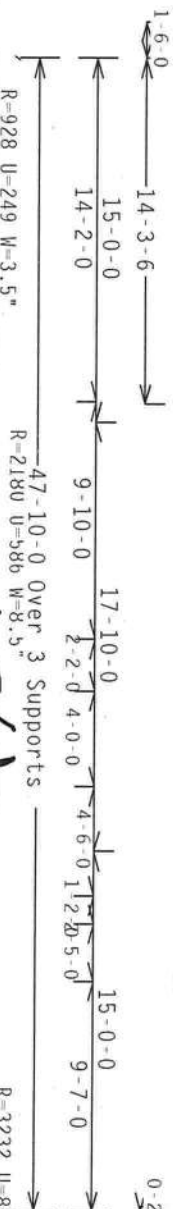
(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Trusses to be spaced at 16.0" OC maximum.

Collar-tie braced with continuous lateral bracing at 24" OC, or rigid ceiling.

Calculated vertical deflection is 0.56" due to live load and 0.95" due to dead load at $X = 33\text{-}10\text{-}0$.

Design Crit: $TPI-2002(STD)/FBC$

PLT TYP. 18 Gauge HS, Wave

7.36.00.

QTY:1

FL/-/4/-/-/R/-/

Scale = .125"/Ft.

WARNING: FIRE'S ROUTINE EXISTENCE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO SC-1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE FIRE LOSS PREVENTION INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND THE 40000 TRUSS COUNCIL OF AMERICA, 65000 INTERSTATE LANE, SUITE 510, FAYETTEVILLE, AR, 72731 FOR FURTHER PRACTICES AND PRECAUTIONS TO PREVENTING FIRE FUNCTIONS. UNDESIRABLE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARTS AND BOTTOM CHORD SHALL HAVE, PROPERLY ATTACHED FIELD CEILING.

****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH ICG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE DESIGN OR THE PROJECT.

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF HD5 (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ITW BCG

CONNECTION PLATES ARE MADE OF 20/10/166A (H, H/SS/K) ASTM A553 GRADE 40/60 (H, K/H, SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC 3. A SEAL ON THIS

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.

BUILDING DESIGNER PER ANSI/FP1 1 SEC. 2.

1

(***) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 17.38 ft mean hgt, ASCE 7-02, PART-ENC, bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC D1=5.0 psf, wind BC D1=5.0 psf, $1w=1.00$ GCP1 (+/-)=0.55

WARNING: This truss doesn't meet L/360 LL and L/240 TL relative deflection criteria. Actual is L/15.84 LL and L/266.59 TL. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

(1) 2x4x7-7-2 SP #2 Dense Top chord scab centered 35-7-8 from left end. Attach to one face of chord with (2) rows of 12d-Common (0.148"x3.25"-min.)_nails @ 6" O.C., staggered 3".

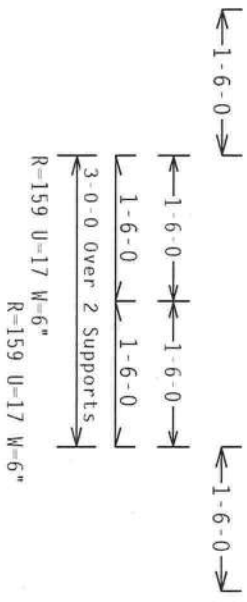
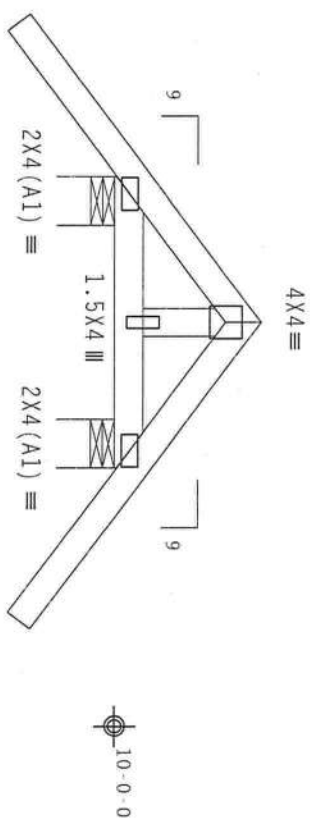
TC LL	20.0 PSF	REF	R8228 - 71277
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUSR8228 08256022
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEON -	104568
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF -	1TKU8228701

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.
Trusses to be spaced at 16.0" OC maximum.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MWFRS pressures.
Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

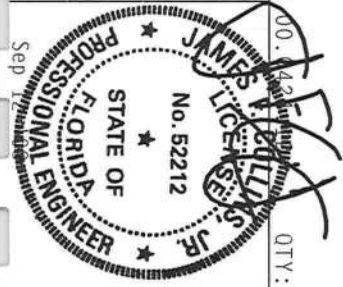


PLT TYP. Wave

Design Crit: TP1-2002 (STD) /FBC
Cq/RT=1.00(1.25)/0(0)

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT SAFETY INFORMATION). PROHIBITED BY TP1 (CROSS PLATE TRISTITUTE, 218 ENTERPRISE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND AREA TRUSS THRUSS CONDUCT OF AMERICA, 6580 ENTERPRISE STREET, SUITE 312, ALEXANDRIA, VA, 22304. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE RCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF TRUSSES IN CONFORMANCE WITH THE RCG, INC. DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE RCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF TRUSSES IN CONFORMANCE WITH THE RCG, INC. DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.



TC LL	20.0 PSF	REF R8228- 71278
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUR8228 08255005
BC LL	0.0 PSF	HC-ENG JB/AP *
TOT. LD.	40.0 PSF	SEON- 102374
DUR. FAC.	1.25	FROM AH
SPACING	16.0"	JREF- 1TKU8228Z01

Scale = .5"/Ft.

Top Chord 2x4 SP #2 Dense
Bot Chord 2x4 SP #2 Dense
Webs 2x4 SP #3

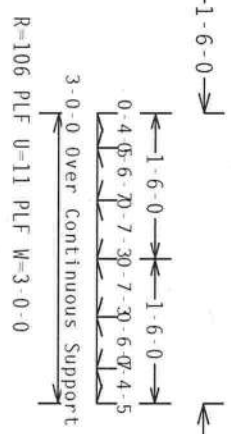
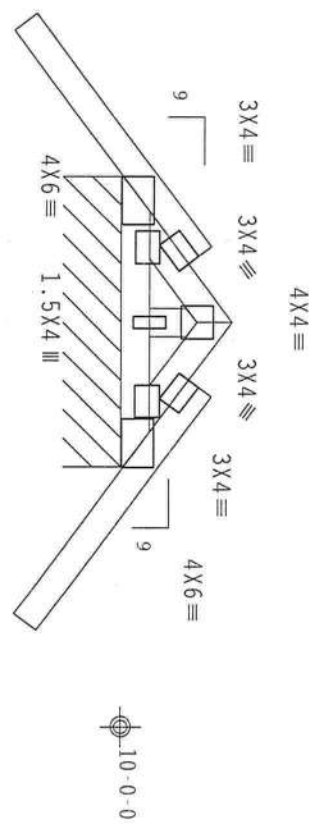
Roof overhang supports 2.00 psf soffit load.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS, DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCPI(+/-)=0.18
Wind reactions based on MWFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.00-173 (10/11/08) ATT.1

FL/-/4/-/-/R/-

Scale = .5"/ft.

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (INCLUDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICK (WOOD TRUSS COUNCIL OF AMERICA), 6300 INTERSTATE LANE, HOUSTON, TX 77057 FOR SAFETY PRACTICES PRIOR TO REPERFORMING THESE FUNCTIONS. UNLESS OTHERWISE SPECIFIED, ALL TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEFLECTION FROM THIS DESIGN. THE FABRICATOR OF THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. BY ACPA) AND TPI. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BMS (NATIONAL DESIGN SPEC. BY ACPA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2017W156A (ALUMINUM) ASTM A653 GRADE 40/50 (4. K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT BUILDING DESIGNER PER ANNEX 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71279
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCU8R8228 08255007
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEGN-	102389
DUR. FAC.	1.25	FROM	AH
SPACING	16.0"	REF-	1TKU8228Z01

110 mph wind, 17.94 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

Wind reactions based on MMFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



1-6-0

R=254 U=35 W=6"

Scale = .5"/Ft.



ALPINE

Haines City, FL 33844
FL COA #0778

FL/-/4/-/-/R/-		Scale = .5"/ft.	
TC LL	20.0 PSF	REF	R8228- 71280
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCSR8228 0825050
BC LL	0.0 PSF	HC-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEQN-	102332
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	UREF-	1TKU8228Z01

JREF-1TKU8228Z01

THIS DOCUMENT PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY IKUSS MFK.

110 mph wind, 17.76 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures.

Trusses to be spaced at 16.0" OC maximum.



1-6-0

Scale = .5" / Ft.

18
300
ESS
AVE

JAMES H. KELLY
LICENSE
No. 52212
JR.



ALPINE

Haines City, FL 33844
FLCOA #0278

OT
FLORIDA
No. 52212
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
★ ★
SEP 17 08

TC LL	20.0 PSF	REF	R8228 - 71281
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCU8R8228 08255006
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	102344
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1TKU8228Z01

JREF - 1TKU8228Z01

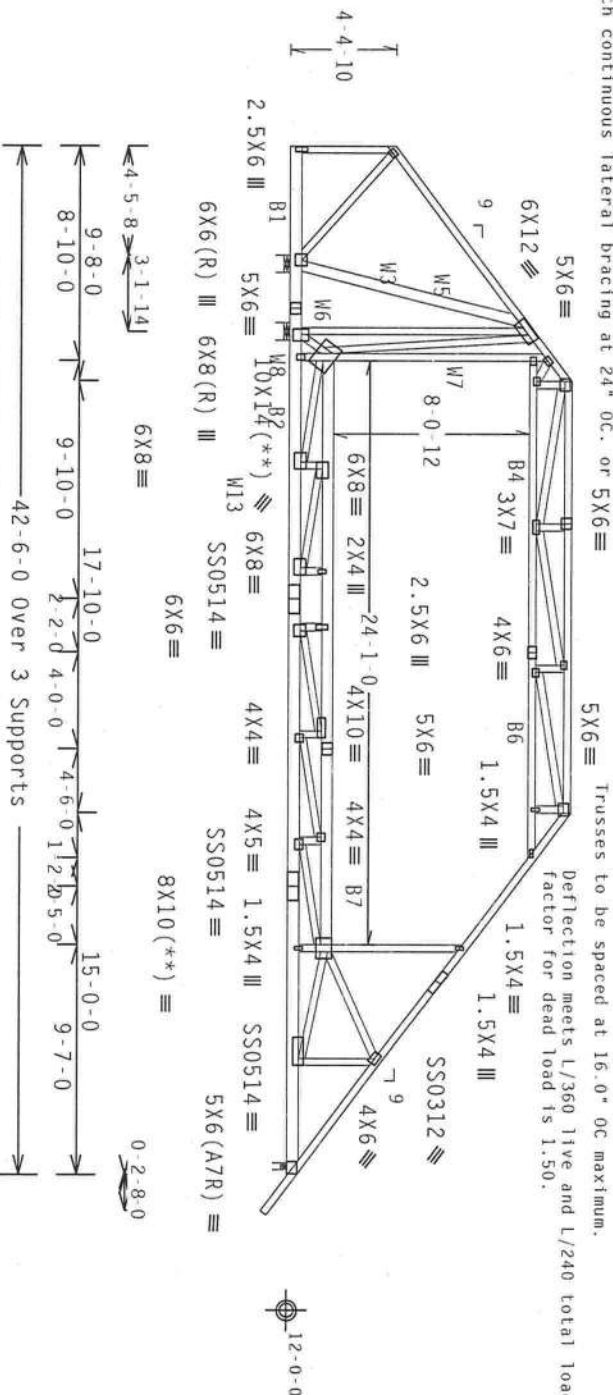
Top Chord 2x4 SP #2 Dense
Bot Chord 2x6 SP #5 : B1, B7 2x6 SP #2 :
B2 2x6 SP #1 Dense : B4, B6 2x4 SP #2 Dense :
Webs 2x4 SP #3 : W3 2x6 SP #2 :
W5, W6, W7, W8, W13 2x4 SP #2 Dense :

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 43 PLF at 0.00 to 43 PLF at 9.67
TC - From 43 PLF at 9.67 to 43 PLF at 15.50
TC - From 43 PLF at 15.50 to 43 PLF at 27.50
TC - From 43 PLF at 27.50 to 43 PLF at 29.33
TC - From 60 PLF at 29.33 to 60 PLF at 32.92
TC - From 43 PLF at 32.92 to 43 PLF at 34.00
TC - From 123 PLF at 34.00 to 131 PLF at 34.58
TC - From 131 PLF at 34.58 to 233 PLF at 42.50
TC - From 43 PLF at 42.50 to 43 PLF at 40.83
TC - From 13 PLF at 40.83 to 13 PLF at 29.00
PLT - From 13 PLF at 29.00 to 13 PLF at 29.00
BC - From 13 PLF at 29.00 to 13 PLF at 8.83
BC - From 80 PLF at 8.83 to 80 PLF at 32.92
BC - From 13 PLF at 32.92 to 13 PLF at 42.50
BC - From 3 PLF at 42.50 to 3 PLF at 44.17
TC - 220 LB Conc. Load at 42.50
BC - 107 LB Conc. Load at 5.83
BC - 72 LB Conc. Load at 32.92

Left end vertical not exposed to wind pressure.

Collar-tie braced with continuous lateral bracing at 24" OC. or 5X6 =



Note: All Plates Are 3/4 Except As Shown.

PLT TYP. 18 Gauge HS.Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/0(0)

7.36-00

FL/-/4/-/-/R/-

Scale = .125"/ft.

WARNING TRUSSES REQUIRING EXTERIOR CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. PROTECT TRUSSES FROM CORROSION BY APPLYING AN ANTI-RUST COATING TO ALL EXPOSED SURFACES. TRUSSES MUST BE STORED ON A LEVEL SURFACE AND NOT EXPOSED TO EXCESSIVE WIND OR OTHER ADVERSE WEATHER CONDITIONS. TRUSSES MUST BE PROTECTED FROM DAMAGE DURING TRANSPORT AND STORAGE. TRUSSES MUST BE PROTECTED FROM DAMAGE DURING TRANSPORT AND STORAGE. TRUSSES MUST BE PROTECTED FROM DAMAGE DURING TRANSPORT AND STORAGE.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0778

2 COMPLETE TRUSSES REQUIRED

Roofing Schedule: (12d Common, 0.148"x3.25", min.) nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Negative reaction(s) of -1110# MAX. (See below) from a non-wind load case requires uplift connection.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 17.38 ft mean hgt, ASCE 7-02, PART-ENC. bldg, not located within 5.83 ft from roof edge, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, W=1.00 Gcpl(+/-)=0.55

Wind reactions based on MWFRS pressures.

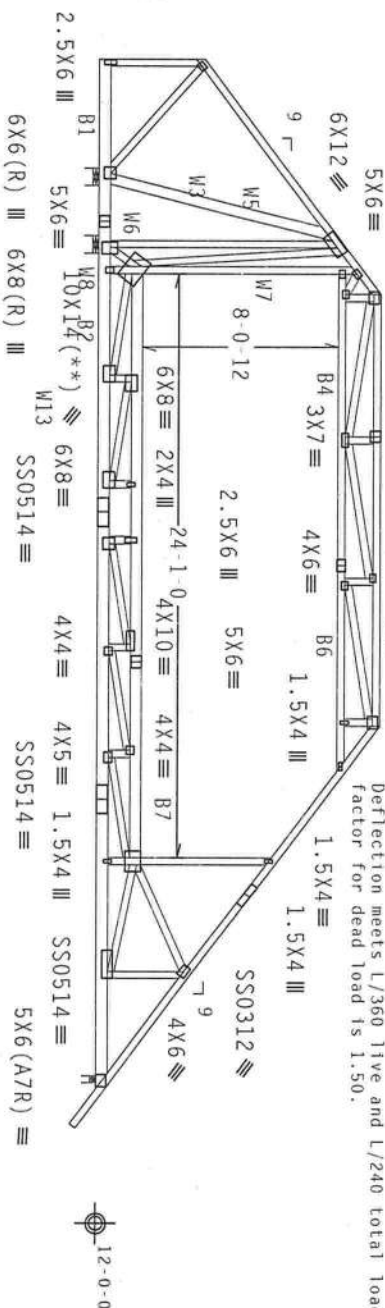
Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.15" due to live load and 0.26" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=3173 U=853 W=8.5"

R=172/-1110 Rw=436 U=511 W=8.5"

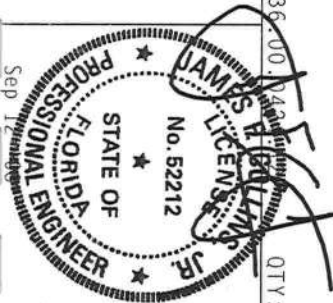
R=3348 U=900 W=3.5"

QTY:1

FL/-/4/-/-/R/-

Scale = .125"/ft.

TC LL	20.0 PSF	REF R8228-71283
TC DL	10.0 PSF	DATE 09/12/08
BC DL	10.0 PSF	DRW HCUR8228 08256023
BC LL	0.0 PSF	HC-ENG JB/AP
TOT. LD.	40.0 PSF	SEON-104581
DUR.FAC.	1.25	FROM AH
SPACING	16.0"	JREF-1TKUR8228201



Bearing blocks: #B11 type: 12d Common (0.148"x3.25", min.) nails
BRG X-LOC #BLOCKS LENGTH/BLK #NAILS/BLK WALL PLATE
2 47.33' 1 12" Rigid Surface
Bearing block to be same size and species as bottom chord.
Refer to drawing CMB68LK0207 for additional information.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 18.01 ft mean hgt., ASCE 7-02, CLOSED bldg, not located within 5.83 ft from roof edge, CAT 1, EXP B, wind TC DL=-0.0 psf, wind BC DL=-5.0 psf. $I_w=1.00$ Gcpl(+/-)=0.18

Calculated horizontal deflection is 0.28" due to live load and 0.48" due to dead load.

Trusses to be spaced at 16.0" OC maximum.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Calculated vertical deflection is 0.69" due to live load and 1.18" due to dead load at $X = 29.5-0$.

(2) 2x4x11.5-11 SP #2 Dense Top chord scabs centered 37.0-2 from

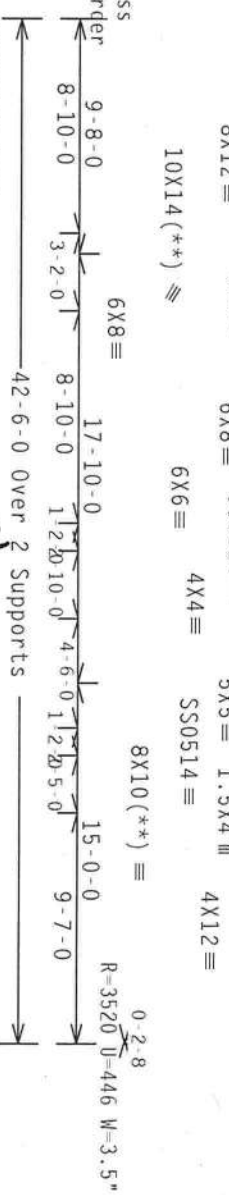
Calculated vertical deflection is 0.69" due to live load and 1.18" due to dead load at $X = 29.5-0$.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Trusses to be spaced at 16.0" OC maximum.

Calculated horizontal deflection is 0.28" due to live load and 0.48" due to dead load.



42-6-0 Over 2 Supports

7.36.00 ~~7.36.00~~ OTY:1 EL: -/4/-/-/R/- Scale = 125"/Ft

TC LL	20.0 PSF	REF	R8228- 712
TC DL	10.0 PSF	DATE	09/12/01

PROFESSIONAL ENGINEER	DUR. FAC. 1.25	FROM AH
-----------------------	----------------	---------

SEP 12 08
SPACING 16.0"
JREF- 1TKU8228Z

FL/-4/-/-R/-		Scale = .125"/ft
TC LL	20.0 PSF	REF R8228- 7122
TC DL	10.0 PSF	DATE 09/12/00
BC DL	10.0 PSF	DRW HCUR8228 0825
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 104615
DUR.FAC.	1.25	FROM AH
SPACING	16.0"	JREF- 1TKU8228Z

```
Scale = .125"/ft
REF  R8228-  712
DATE  09/12/0
DRW  HCUR8228 0825
HC-ENG JB/AP
SEON-  104615
FROM  AH
JREF- 1TKU8228Z
```


Top Chord 2x4 SP #2 Dense
Bot Chord 2x4 SP #2 Dense : B1 2x6 SP #2:
Webs 2x4 SP #3 : W14 2x4 SP #2 Dense:

SPECIAL LOADS
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	122 PLF at -3.26 to 122 PLF at 3.88
TC - From	122 PLF at 3.88 to 122 PLF at 13.26
TC - From	130 PLF at 13.26 to 130 PLF at 21.84
TC - From	122 PLF at 21.84 to 122 PLF at 33.84
TC - From	122 PLF at 33.84 to 122 PLF at 45.09
BC - From	8 PLF at -3.26 to 8 PLF at 0.00
BC - From	40 PLF at 0.00 to 40 PLF at 9.26
BC - From	40 PLF at 9.26 to 40 PLF at 21.26
BC - From	40 PLF at 21.26 to 40 PLF at 33.26
BC - From	40 PLF at 33.26 to 40 PLF at 45.09
BC -	9 LB Conc. Load at 3.50
BC -	4 LB Conc. Load at 3.94
BC -	2 LB Conc. Load at 4.72
BC -	533 LB Conc. Load at 8.69

(8) (2) #3 or better scab braces. Same size & 80% length of web member. Attach one to each face w/10d Box or Gun (0.128"x3".min.)nails @ 6" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d,Common,(0.148"x3.25",-min.)-nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.71 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 8.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, IW=1.00 GCpl(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

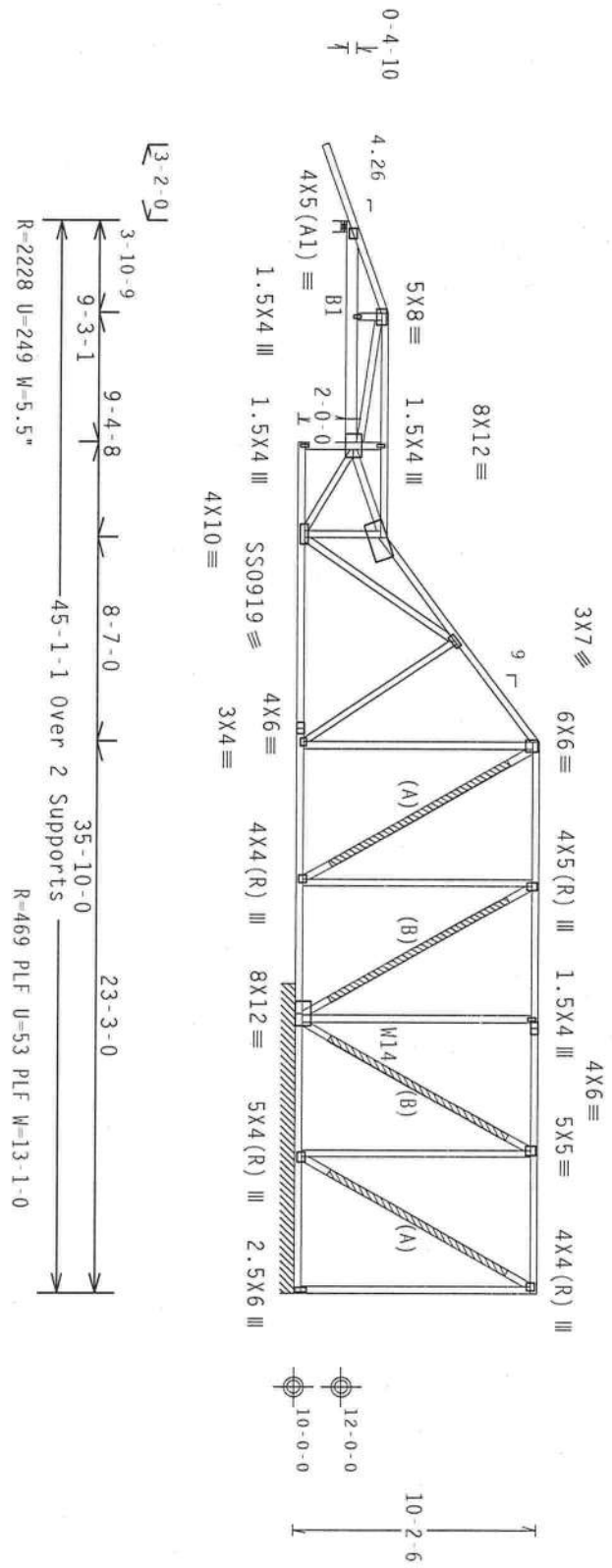
Roof overhang supports 2.00 psf soffit load.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3".min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Trusses to be spaced at 48.0" OC maximum.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



PLT TYP. 18 Gauge HS.Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

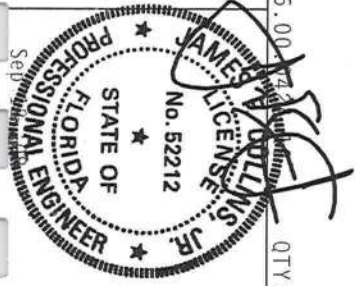
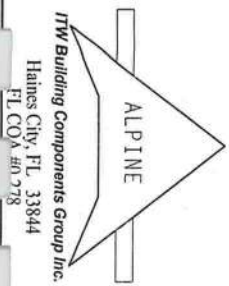
QTY:1 FL/-/4/-/1-/R/-

Scale =.125"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND UNLOADING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PROVIDED BY TPI (TRUSS PLATE INSTITUTE, 2100 ENTERPRISE BLVD, SUITE 100, ALPHARETTA, GA 30004) AND WEA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE BLVD, SUITE 100, ALPHARETTA, GA 30004) FOR PROPER HANDLING AND INSTALLATION. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEK A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71286
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCSUR8228 08256026
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEON-	104774
DUR.FAC.	1.25	FROM	AH
SPACING	48.0"	JREF-	1TKU8228Z01

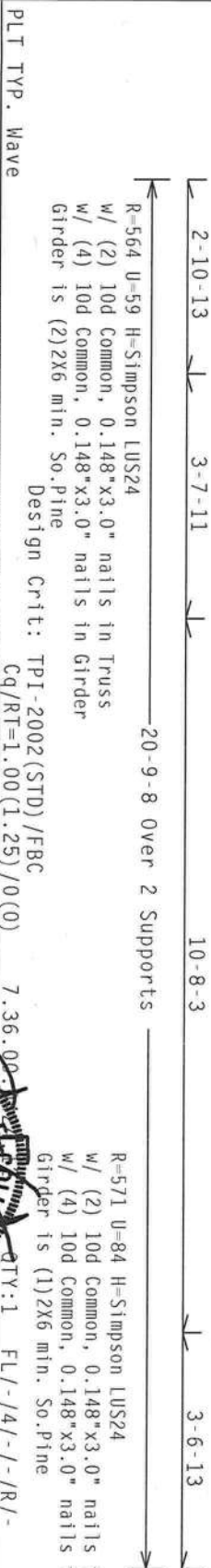
110 mph wind, 16.99 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, IW=1.00 GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Trusses to be spaced at 16.0" OC maximum.
Provide for complete drainage of roof.

2.5X6 //

TP1-2002(STD)/FBC
Cq/RT=1.00(1.25)

7.36.00

QTY:1 FL/-/4/-/-/R/-/

Scale = .375"/Ft.

R=571 U84 H=Simpson LUS24		Qty: 1		FL/-/4/-/-/R/-		Scale	
W/ (2) 10d Common, 0.148"x3.0" nails in Truss		TC LL	20.0	PSF	REF		
W/ (4) 10d Common, 0.148"x3.0" nails in Girder		TC DL	10.0	PSF	DATE		
Girder is (1)2X6 min. So.Pine							

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW DCG, INC., SHALL NOT

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AFSPA) AND TPI.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A) OF TP11-2002 SEC.3. A SEAL ON THIS

DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/APA 1 SEC. 2.

Figure 1. A schematic diagram of the experimental design. The subjects were divided into two groups: the control group and the experimental group. The control group received a standard training program, while the experimental group received a modified training program. The results of the training program were compared between the two groups.

STATE OF FLORIDA
PROFESSIONAL ENGINEER
No. 52212
JAMES M. COLLINS, JR.
CITY

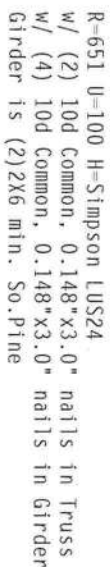
TC LL	20.0 PSF	REF	R8228- 71287
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUSR8228 08256013
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT. LD.	40.0 PSF	SEQN-	104814
DUR. FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

110 mph wind, 18.98 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCp1(+/-)=0.18

Trusses to be spaced at 16.0" OC maximum.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.



Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$

7.36.00

QTY:1

FL/-/4/-/-/R/-/

Scale = .3125"/Ft.

*WARNING: THESE RESIN, EPOXY, EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND PRACTICING. THE FOLLOWING INFORMATION, PUBLISHED BY THE CROSS SALES INSTITUTE, 218
REFER TO DC51 (BUILT-UP COMPONENT SAFETY INFORMATION), 6-0003 COUNCIL OF AMERICA, 6-0003
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, AND WIFE, 6-0003 CROSSING THESE FUNCTIONS,
INTERPRET LATE, MADISON, WI 53703) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS,
UNLESS OTHERWISE INDICATED THAT CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
PROPERLY ATTACHED BRIDGELING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH THIS OR FABBICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TROSSES.

CONNECTOR PLATES ARE MADE OF 2018R16GA (U H.S./V) ASTM A653 GRADE 40/60 (U K/U SCS) CAIN

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF IP11-2002 SEC.3. A SEAL ON THIS

DRIVING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

— 100 —

OTY

JAMES R. LEE, JR.
Professional Engineer
No. 52212
State of Florida
Professional Engineer

Sep 12 08

TC LL	20.0 PSF	REF	R8228- 71288
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUSR8228 08256027
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	104828
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

JREF - 1TKU8228Z01

Na11ng Schedule: (12d_Common_(0.148"x3.25",_min.)_na11s)
 100 Chord: 1 Row @12.00" A

Use equal spacing between rows and stagger nail in each row to avoid splitting.

Wind reactions based on MWFRS pressures.

Max JT VERT DEFL: LL: 0.24" DL: 0.35" recommended camber 5/8"

In lieu of structural panels use purlins to brace TC @ 24" OC.

factor for dead load is 1.50.

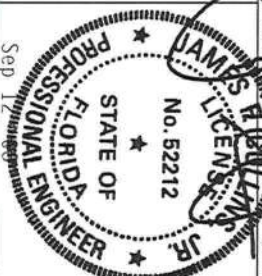
The TC of this truss shall be braced with attached spans at 24'-00" in lieu of structural sheathing.



Scale = .125"/Ft.

036-00-142
UAMC
No. 52212
J.R.
QTY

Haines City, FL 3384
FL COA #0278



TC LL	20.0 PSF	REF	R8228- 71290
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUSR8228 08256028
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	105023
DUR.FAC.	1.25	FROM	AH
SPACING	48.0"	JREF-	1TKU8228201

4 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d-Common-(0.148"x3.25"-min.))-na11s)

1

Bot Chord: 1 Row @12.00" o.c.

Weds @ 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid spilling.

IN ADDITION TO EXISTING NAILING, APPLY SIMPSON (1/4" X 6.0") SDS SCREWS @ 24" OC ON LOADED FACE IN TOP & BOTTOM CHORDS.

Negative reaction(s) of -78# MAX. (See below) from a non-wind load case requires uplift connection.

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

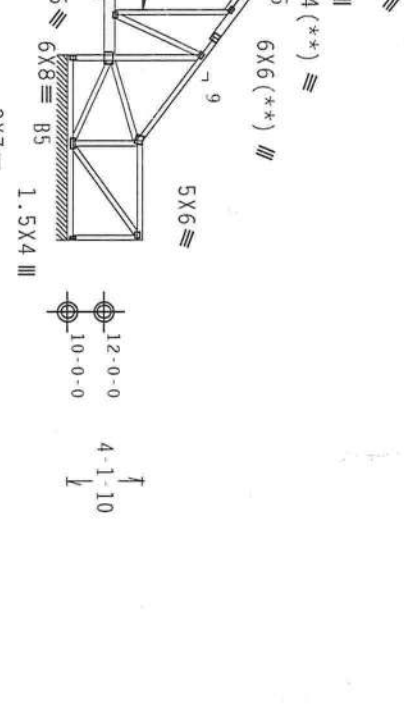
110 mph wind, 17.88 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 5.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCpl(+/-)=0.18

End verticals not exposed to wind pressure.

Collar-tie braced with continuous lateral bracing at 24" OC. Calculated vertical deflection is 0.64" due to live load and

WARNING: Furnish a copy of this DWG to the installation

and installation of trusses. See "WARNING" note below.

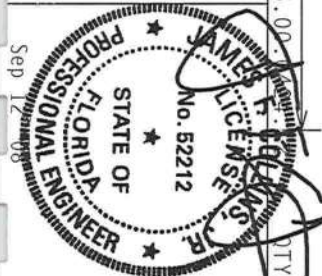
10-3-0
8-11[illegible]

JAMES A. LIC. No.

STATION

FLC PROFESSIONAL

Sep 12



FL/-/4/-/-/R/-		Scale = .09375"/ft.
TC LL	20.0 PSF	REF R8228- 71291
TC DL	10.0 PSF	DATE 09/12/08
BC DL	10.0 PSF	DRW HCUR8228 08256029
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEON- 105006
DUR.FAC.	1.25	FROM AH
SPACING	30.0"	JREF- 1TKU8228Z01

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 17.48 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC

DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI (+/-)=0.18

Wind reactions based on MWFRS pressures.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require

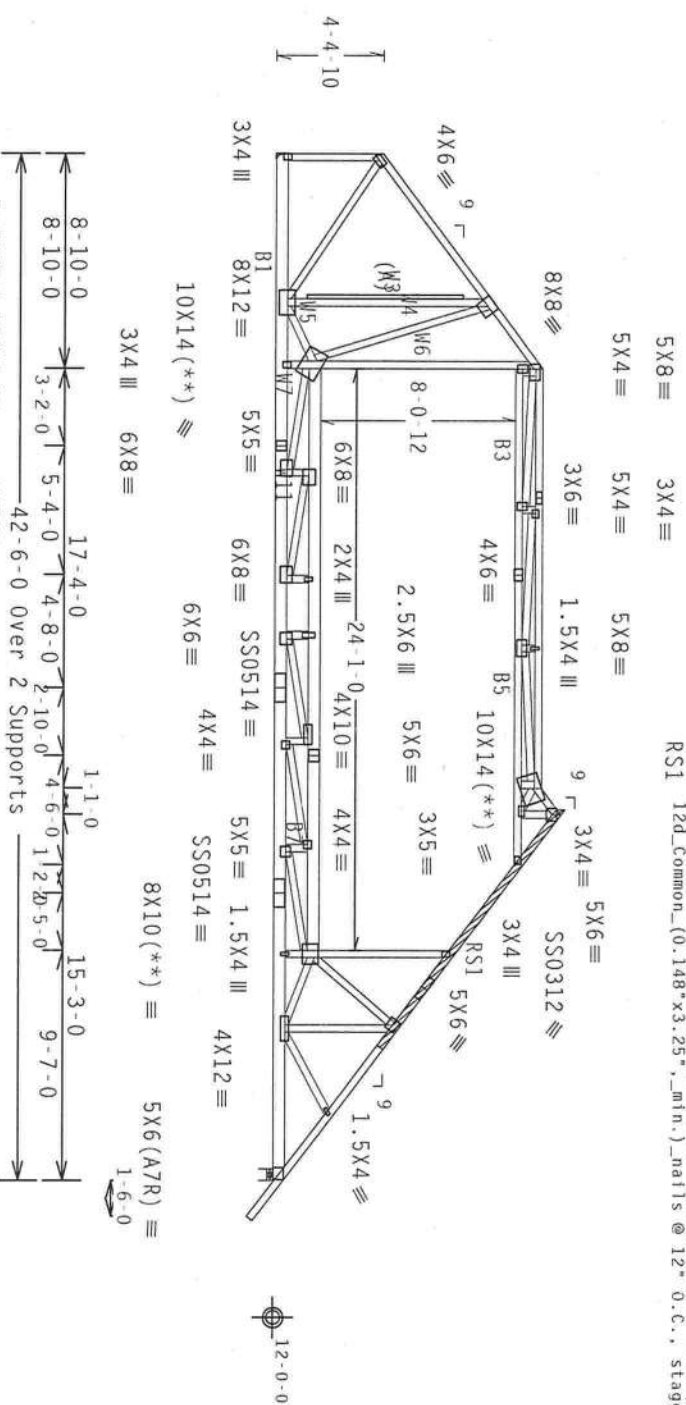
different connections than indicated. Refer to manufacturer publication for additional information.

Trusses to be spaced at 16.0" OC maximum.

Deflection moete 1/360 line and 1/240 total load rigid ceiling.

factor for dead load is 1.50.

(2) 2X4X125-12 SP #2 Dense top chord scabs centered 3/-3-6 from left end. Attach one to each outer face of chord with (2) rows of



R=2278 U=122 H=Simpson HUS26
w/ (4) 16d, 0.162"x2.5" nails in Truss
w/ (14) 16d, 0.162"x2.5" nails in Girder
Girder is (2) 2X8 min. So, Pine

Design Crit: TPI-2002(STD)/FBC

PLT TYP. 18 Gauge HS. Wave

QTY:1

FL/-/4/-/1/-/R/-/

Scale = .125"/Ft.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSESS.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIAA) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 2018/1664 (U.H./S) ALUM. 6063 GRADE 40/40 (U. H./S) GALV. STEEL. APPLY

PLATES TO EACH OF THOSE AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEC A3 OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROVISIONAL ELECTRICAL RESPONSIBILITY. COLLECT FOR THE THREE COMPONENTS

DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/AP1 1 SEC. 2.



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FLCOA #01278

TC LL	20.0 PSF	REF	R8228- 71292
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCSUR8228 08256015
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	104852
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Left end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Trusses to be spaced at 16.0" OC maximum.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 20-11-0 to 32-11-0.

$$5 \times 6 \equiv 6 \times 6 \equiv$$
$$3 \times 4 \begin{pmatrix} * & * \\ * & * \end{pmatrix} \equiv$$

If = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Wind reactions based on MMFRS pressures.

2 COMPLETE TRUSSES REQUIRED

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common (0.148"x3.25", min.)_nails)

top chord:	1 Row	@12.00" o.c.
Bot Chord:	1 Row	@12.00" o.c.
Walls	1 Row	@ 4" o.c.

Use equal spacing between rows and stagger nails in each row to avoid splitting.

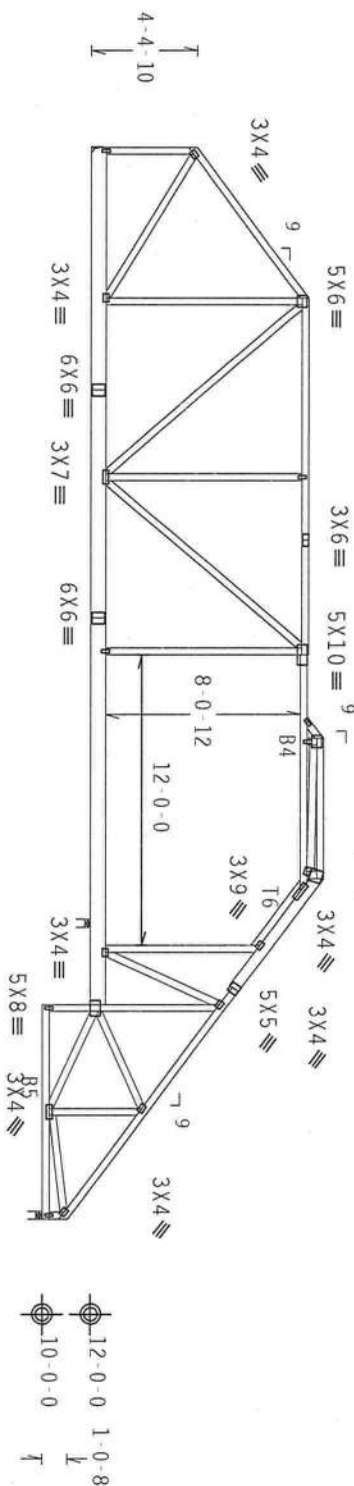


Diagram illustrating the layout of a 3x7 beam with dimensions and supports:

- Overall length: 44-3-8
- Over 3 Supports: 32-0-0
- Dimensions from left end:
 - 6-2-0
 - 20-11-0
 - 14-9-0
 - 3-5-0
 - 5-10-0
 - 9-9-5
 - 4-8-11
 - 14-1-8
 - 8-10-8

R=1587 U=106 H=Simpson HUS28-2

w/ (6) 10d Common, 0.148"x3.0" nails in Truss

Girder is (2) 2X8 min. So. Pine

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

$$Cq/RT=1.00(1.25)/0(0)$$

7.36.00

QTY:1

FL/-/4/-/-/R/-/-

Scale = .125"/Ft.

"WARNING—" PRICES BEHIND THE EXTREME CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND MAINTAINING OF A BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY THE (FIBROS PLAST INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WERE GOOD THINGS CONCERNING OF AMERICA, ENTERPRISE LAW, MADISON, WI 53719) FOR SAFETY PRACTICES RELATE TO THESE FUNCTIONS. UNLESS OTHERWISE INDICATED THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CELLING.

****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE FOLLOWING SPECIFICATIONS, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIAA) AND TPI. CONNECTOR PLATES ARE MADE OF 20/18/16GA (40/55/60) ASTM A653 GRADE 40/60 (40/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF JOISTS AND BRICES OUTSIDE LOCATED ON TOP OF BEAM. POSITION JOIST BRACES 16GA.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A. OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TROUS COMPONENT

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 No. 52212
 JAMES E. JOHNSON, JR.
 CIVIL ENGINEER
 Sep 12 2008

FL/4/-/-/R/-		Scale = .125"/Ft.
TC LL	20.0 PSF	REF R8228 - 71293
TC DL	10.0 PSF	DATE 09/12/08
BC DL	10.0 PSF	DRW HCUR8228 08256016
BC LL	0.0 PSF	HC-ENG JB/AP
TOT. LD.	40.0 PSF	SEQN- 104948
DUR. FAC.	1.25	FROM AH
SPACING	16.0"	JREF- ITKU8228Z01

FL/-4/-/-R/-		Scale = .125"/Ft.
TC LL	20.0 PSF	REF R8228- 71294
TC DL	10.0 PSF	DATE 09/12/08
BC DL	10.0 PSF	DRW HCUR8228 08256017
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 104964
DUR.FAC.	1.25	FROM AH
SPACING	16.0"	JREF- 1TKU8228Z01

TC LL	20.0 PSF	REF	R8228- 71295
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUSR8228 08256018
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	104943
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

Top chord 2x4 SP #2 Dense; IT5 2x6 SP #2:
Bot chord 2x8 SP #1 Dense; B4, B5 2x4 SP #2 Dense;
Webs 2x4 SP #3

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 16.93 ft mean ht, ASCE 7-02, CLOSED bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, IW=1.00 GCP(+/-)-0.18

End verticals not exposed to wind pressure.

Max JT VERT DEFL: LL: 0.54" DL: 0.96" recommended camber 1 5/8"

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Trusses to be spaced at 16.0" OC maximum.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 20-11-0 to 32-11-0.

Calculated vertical deflection is 0.43" due to live load and 0.96" due to dead load at X = 20-7-8.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

3 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common, 0.148"x3.25", min.)_nails)

Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 12.00" o.c.
Webs: 1 Row @ 4" o.c.

Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Wind reactions based on MMFRS pressures.

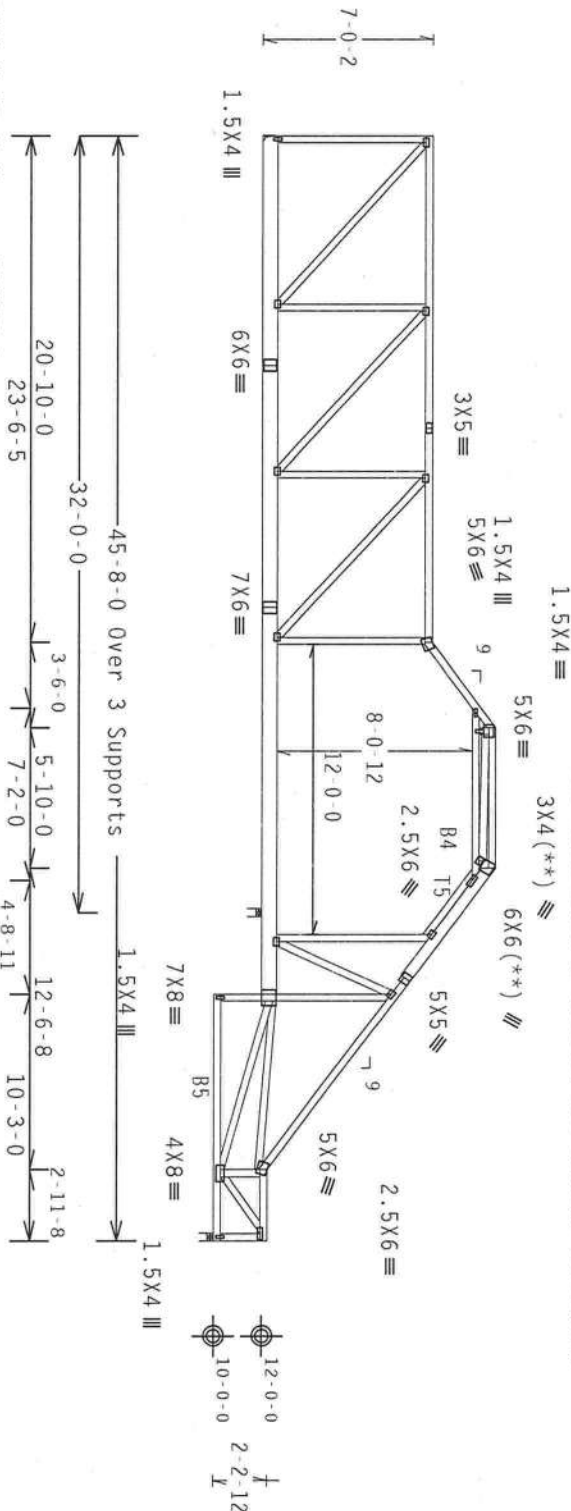
Calculated horizontal deflection is 0.14" due to live load and 0.30" due to dead load.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Collar-tie braced with continuous lateral bracing at 24" OC, or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Note: All Plates Are 3x4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

QTY: 1

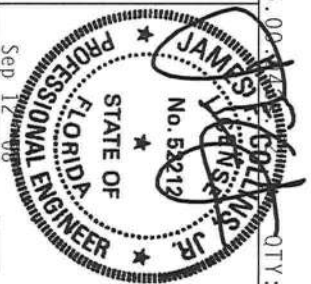
FL/-/4/-/-/R/-

Scale = .125"/Ft.

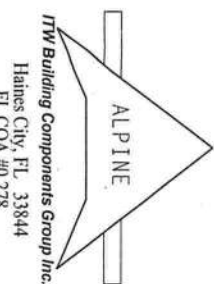
****WARNING**** BRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304, AND WICA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING.

DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF 2005 NATIONAL DESIGN SPEC. BY AIA/PAI AND TPI. DESIGNER'S PLATES ARE MADE OF 2010/101604 (ALUMINUM) AS PER AIA/PAI 40/60 (ALUMINUM) GALV. STEEL. APPLY PROPERLY ATTACHED RIGID CEILING. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS OF TPI-2002, SECTION 10.1.1. BRACING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71296
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUSR8228 08256019
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEON-	104977
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	UREF-	1TKU8228Z01



3 COMPLETE TRUSSES REQUIRED

```

Nailing Schedule: (12d,Common,(0.148"x3.25",min.),_nails)
Top Chord: 1 Row @12.00" 0.C.
Bot Chord: 1 Row @12.00" 0.C.
Shebs : 1 Row @ 4" 0 C

```

Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Wind reactions based on MMFRS pressures.

Calculated horizontal deflection is 0.11" due to live load and 0.24" due to dead load.

It is recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping, and installation of trusses. See "WARNING" note below.

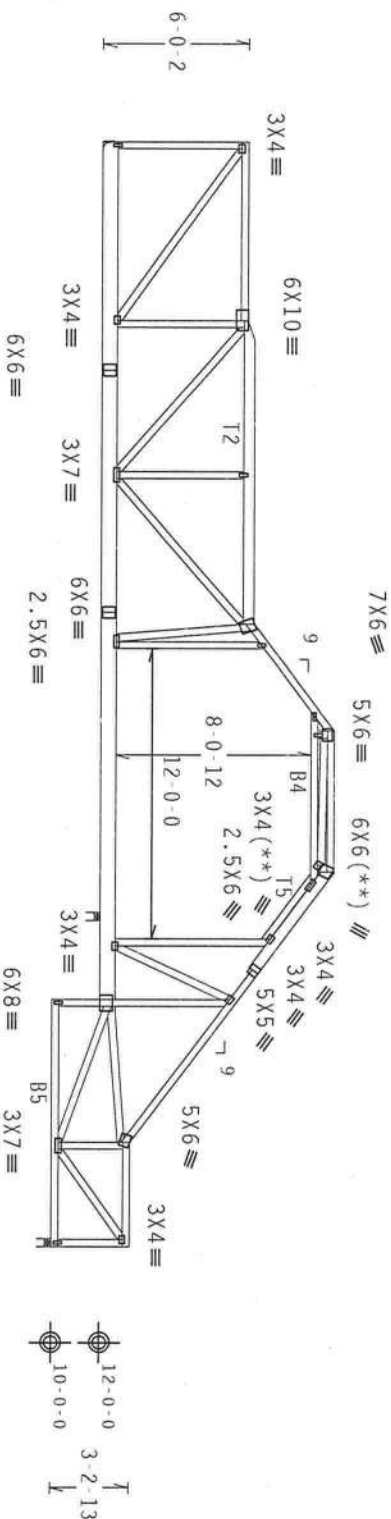


Diagram of a truss structure with dimensions and material specifications:

- Overall length: 45'-8" over 3 Supports
- Span between supports: 32'-0"
- Truss members and dimensions:
 - Top chord: 19'-10"-10"
 - Bottom chord: 23'-6"-4"
 - Vertical height: 4'-5"-5"
 - Internal vertical member: 5'-10"-0"
 - Internal vertical member: 7'-2"-1"
 - Internal vertical member: 4'-8"-11"
 - Internal vertical member: 11'-2"-8"
 - Internal vertical member: 10'-3"-0"
 - Internal vertical member: 4'-3"-9"
- Material specifications:
 - R=1232 U=110 H=Simpson HGU526-3
 - W/ (8) 10d Common, 0.148"x3.0" nails in Truss
 - W/ (20) 10d Common, 0.148"x3.0" nails in Girder
 - Girder is (2)2X8 min. So. Pine
 - R=1760 U=101 W=4"
 - R=729 U=52 W=4"

Design Crit: $TPI-2002(STD)/FBC$
$$Cq/RT=1.00(1.25)/0(0)$$

7.36.00

QTY:1

Scale = .125"/Ft.

WARNING: THESE PRODUCTS EXPERIMENT CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO DESI (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TROSS PATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WFLA (WOOD BROS. COMPANY OF AMERICA, 65000 ENTERPRISE LANE, MOBILE, AL 36619) FOR SAFETY PRACTICES AND WAYS TO IMPROVING THESE FUNCTIONS. UNLESS INDICATED FOR OTHER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED FIELD CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN OR ANY FAILURE TO ROLL THE TRUSS IN CONFORMANCE WITH THE RECOMMENDED FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

CONNECTIONS WITH APPLICABLE PROVISIONS OF MOS (NATIONAL DESIGN SPEC., BY AISC) AND TPI-1 (HRC).

STEEL PLATES ARE MADE OF 20/LR166GA USFSA (STEEL GRADE 40/40 55% CARBON STEEL).

PLATES TO EACH FACE OF THUSOS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITIONED FOR INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMXX AS OF 1711-2002 SEC.3. A SEAL ON THIS

DRAWING INDICATE ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE CROSS CORPORATE DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AIA/1P1 1 SEC. 2.

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FL/-/4/-/-/R/-		Scale = .125"/ft.	
TC LL	20.0 PSF	REF	R8228 - 71297
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUSR8228 08256020
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	104992
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-8-2 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-8-2 overhang. Right side jacks have 0-0-0 setback with 0-0-0 cant and 0-0-0 overhang.


$$Cq/RT=1.00(1.25)/0(0)$$

FL/-/4/-/-/R/-/

Scale = .1875"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF

REF	R8228- 71298
DATE	09/12/08

TC LL	20.0 PSF	REF R8228- 71298
TC DL	10.0 PSF	DATE 09/12/08
BC DL	10.0 PSF	DRW HCU\$R8228 08256030
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEON- 104219
DUR.FAC.	1.25	FROM AH
SPACING	16.0"	JREF - 1TKU8228701

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAI II, EXP B, wind TC DL-5.0 psf, Wind BC DL=5.0 psf, Iw=1.00 Gcpi (+/-)=0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



7.36.00

QTY:1

FL/-/4/-/-/R/-/

Scale = .1875"/Ft.

STOLLINS, JR.
LICENSE
No. 53279

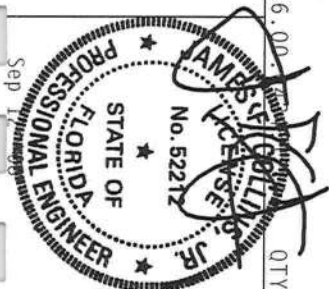
TC LL	20.0 PSF
TC DL	10.0 PSF

REF	R8228 - 71299
DATE	09/11/08



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0078



TC LL	20.0 PSF	REF	R8228- 71299
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCU8R8228 0825500
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	104224
DUR.FAC.	1.25	FROM	AH
SPACING.	16.0"	JREF-	ITK8228Z01

Top Chord 2x4 SP #2 Dense
Bot Chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Lt Bearing Leg 2x4 SP #3:

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Trusses to be spaced at 16.0" OC maximum.

Provide for complete drainage of roof.

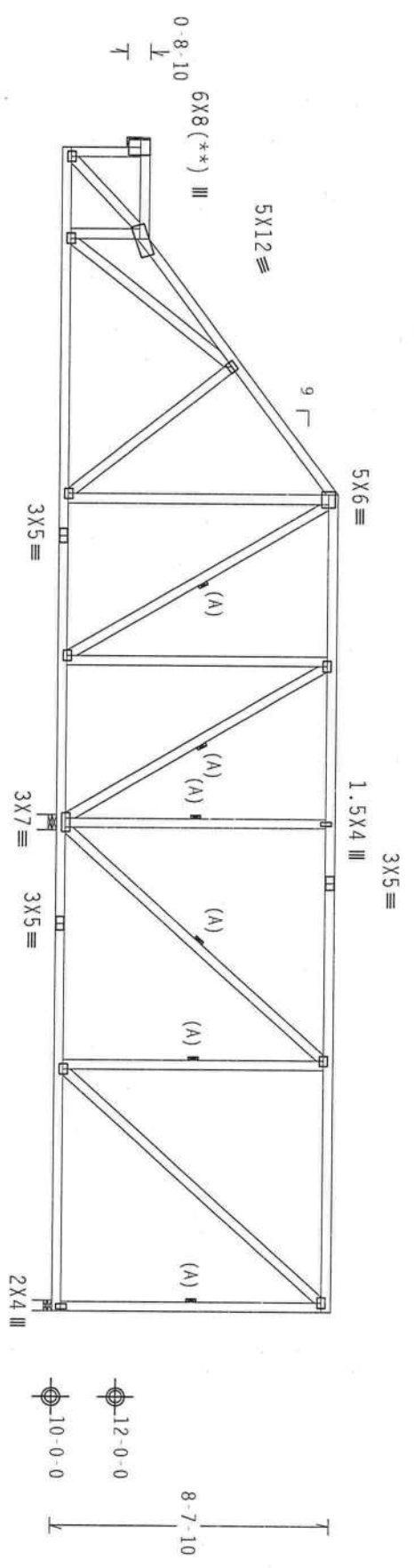
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.68 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 Gcpi(+/-)=0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



2'-9-13 3'-11-5 5'-1-14 5'-0-2 7'-6-2 7'-7-14
2'-9-13 7'-10-11 5'-1-14 5'-0-2 7'-6-2 7'-7-14
2'-9-13 7'-10-11 5'-1-14 5'-0-2 7'-6-2 7'-7-14
25'-4-0
R=500 U=34
36'-4-0 Over 3 Supports
R=1265 U=137 W=5.657"
R=290 U=42 W=4"

Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

WARNING THUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. NORTH LEE STREET, SUITE 112, AUSTIN, TEXAS 78701. (512) 441-1111. TRUSS COMPANY OF AMERICA, 6200 ENTERPRISE LANE, MOBILE, AL 36619. FOR SAFETY FACTORS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

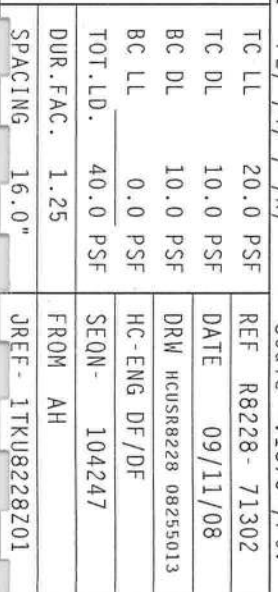


FL/-/4/-/1-/R/-		Scale = .1875"/ft.	
TC LL	20.0 PSF	REF	R8228- 71301
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255012
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON-	104239
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

110 mph wind, 16.51 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCPI (+/-) 0.18



BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Top chord 2x6 SP #2 :T2, T3 2x6 SP #1 Dense:
Bot chord 2x8 SP #1 Dense :B3 2x4 SP #2 Dense:
Meds 2x4 SP #3

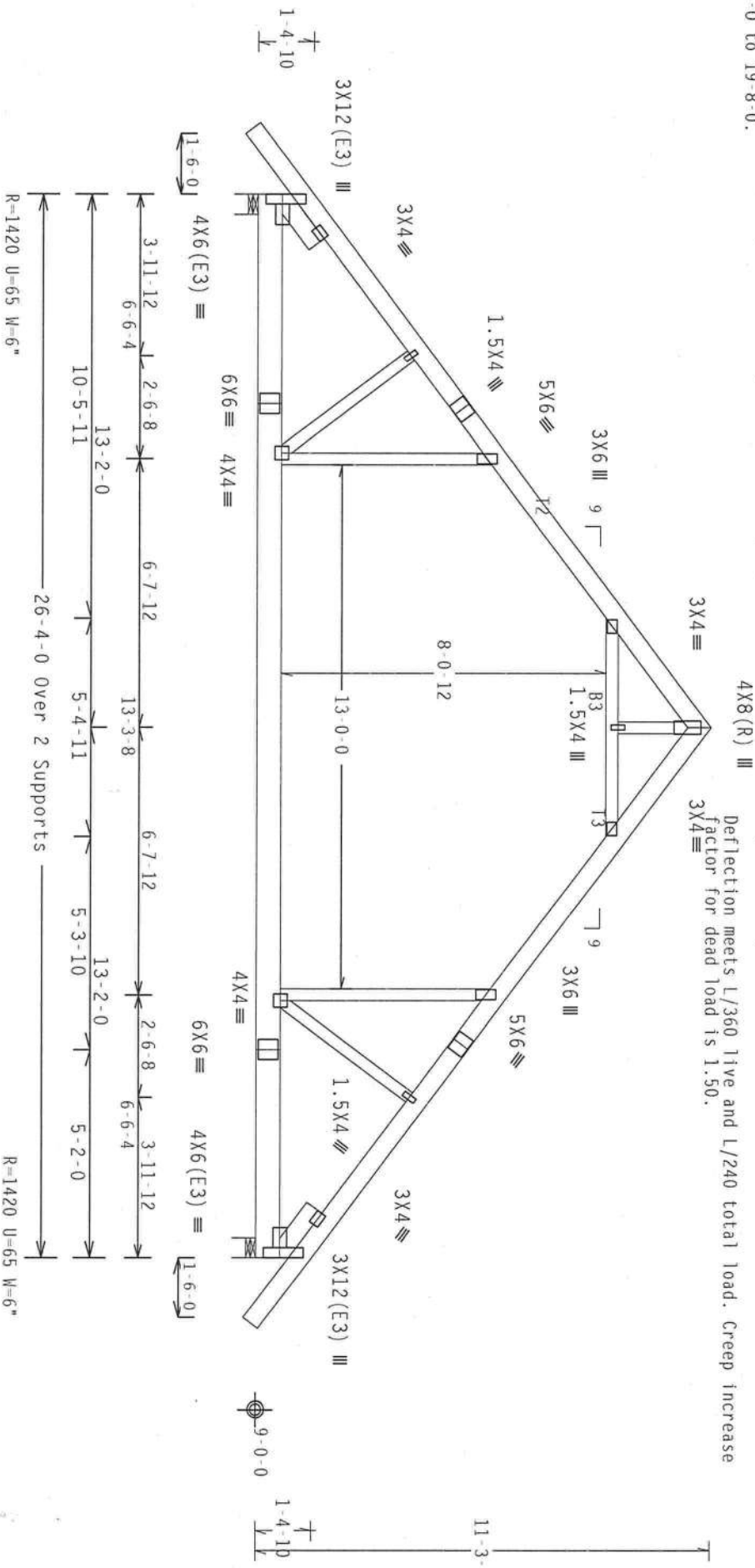
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'
:Rt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'

Roof overhang supports 2.00 psf soffit load.

Trusses to be spaced at 16.0" OC maximum.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from
6-8-0 to 19-8-0.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf,
wind BC DL=5.0 psf, lw=1.00 GCPI(+/-)=0.18
Wind reactions based on MWFRS pressures.
Calculated horizontal deflection is 0.16" due to live load and 0.31"
due to dead load.
Collar-tie braced with continuous lateral bracing at 24" OC. or
rigid ceiling.
Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TP1-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BEST PRACTICES FOR TRUSS FABRICATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 210
N. STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WEA (WOOD TRUSS COUNCIL OF AMERICA, 6300
HARRISON AVENUE, SUITE 100, FORT WORTH, TEXAS 76116) FOR TRUSS FABRICATION PRACTICES. UNLESS
OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

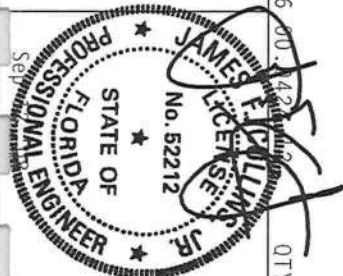
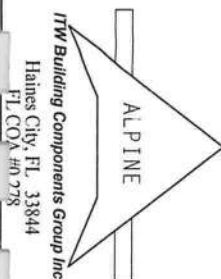
IMPORTANT PROVIDE A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE RCG, INC. SHALL NOT
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF THE TRUSS IN CONFORMANCE WITH
THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. THE RCG, INC. SHALL NOT BE RESPONSIBLE FOR
TP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE RCG, INC. SHALL NOT
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. (BY ACPA) AND TP1. THE RCG,
CONTRACTOR PLATES ARE MADE OF 20/18/1664 (4X/5/5/4) ASH ASSG GRADE 40/60 (4, 4X/5/5/4) GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER AS OF TP11-2002 SEC. 3. A SEAL ON THIS
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT
DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE
BUILDING DESIGNER PER ANSI/TP1 1 SEC. 2.

QTY:1

FL/-/4/-/-/R/-

Scale =.25"/Ft.

TC LL	20.0 PSF	REF R8228- 71303
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUSR8228 08255014
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEQN- 104060
DUR. FAC.	1.25	FROM AH
SPACING	16.0"	JREF- 1TKU8228Z01



Top chord 2x4 SP #2 Dense
Bot chord 2x8 SP #1 Dense
Webs 2x4 SP #3

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 43 PLF at 0.00 to 43 PLF at 6.42
BC - From 13 PLF at 0.00 to 13 PLF at 6.61
PLB- 750 LB Conc. Load at 1.33, 2.67, 4.00, 5.33
300 LB Conc. Load at (6.42,9.04)

Wind reactions based on MWFRS pressures.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS. TRUSS ENGINEER & FABRICATOR ARE NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.

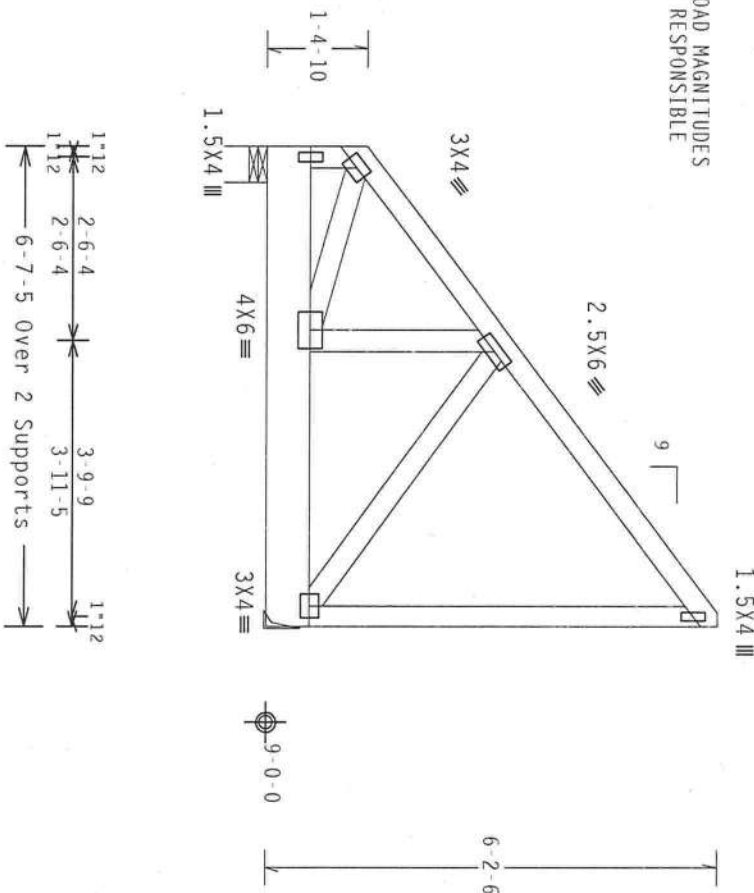
2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common (0.148"x3.25", min.)_nails)

Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @3.75" o.c.
Webs : 1 Row @4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCPI(+/-)=0.18

Right end vertical not exposed to wind pressure.



R=1683 U=181 W=6"

R=1983 U=213

PLT TYP. Wave

Design Crit: TP1-2002(STD)/FBC

Cq/RT=1.00(1.25)/0(0)

7.36.00

QTY:1

FL/-/4/-/-/R/-

Scale =.375"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (INCLUDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TP1 (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6100 WOODBURN AVENUE, SUITE 100, WILSON, NJ, 07094) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. THE TRUSS IN CONFORMANCE WITH TP1: ON FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG CONTRACTOR PLATES ARE MADE OF 2019/1664 (W/N/S/S) ASTM A653 GRADE 40/50 (W, K/H, S/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP1-2002 SEC.3. A SEAL ON THIS BRACING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TP1 1 SEC. 2.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844



SPACING	SEE ABOVE	DATE	REF
TC LL	20.0 PSF	09/11/08	R8228-71304
BC DL	10.0 PSF		
BC DL	10.0 PSF		
BC LL	0.0 PSF		
TOT. LD.	40.0 PSF		
DUR. FAC.	1.25		
FROM	AH		
JREF -	1TKU8228Z01		

Nailing Schedule: (12d Common (0.148"x3.25", min.)_nails)

Webbs : 1 Row @ 4" O.C.

Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ Gcpl (+/-)=0.18

Wind reactions based on MMFRS pressures.

Roof overhang supports 2.00 psf soffit load.

Repeat nailing as each layer is applied. Use

between rows and stagger nails in each row to avoid splitting.
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf,
wind BC DL-5.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18
Wind reactions based on MWFRS pressures.
Roof overhang supports 2.00 psf soffit load.

4X8(R) III



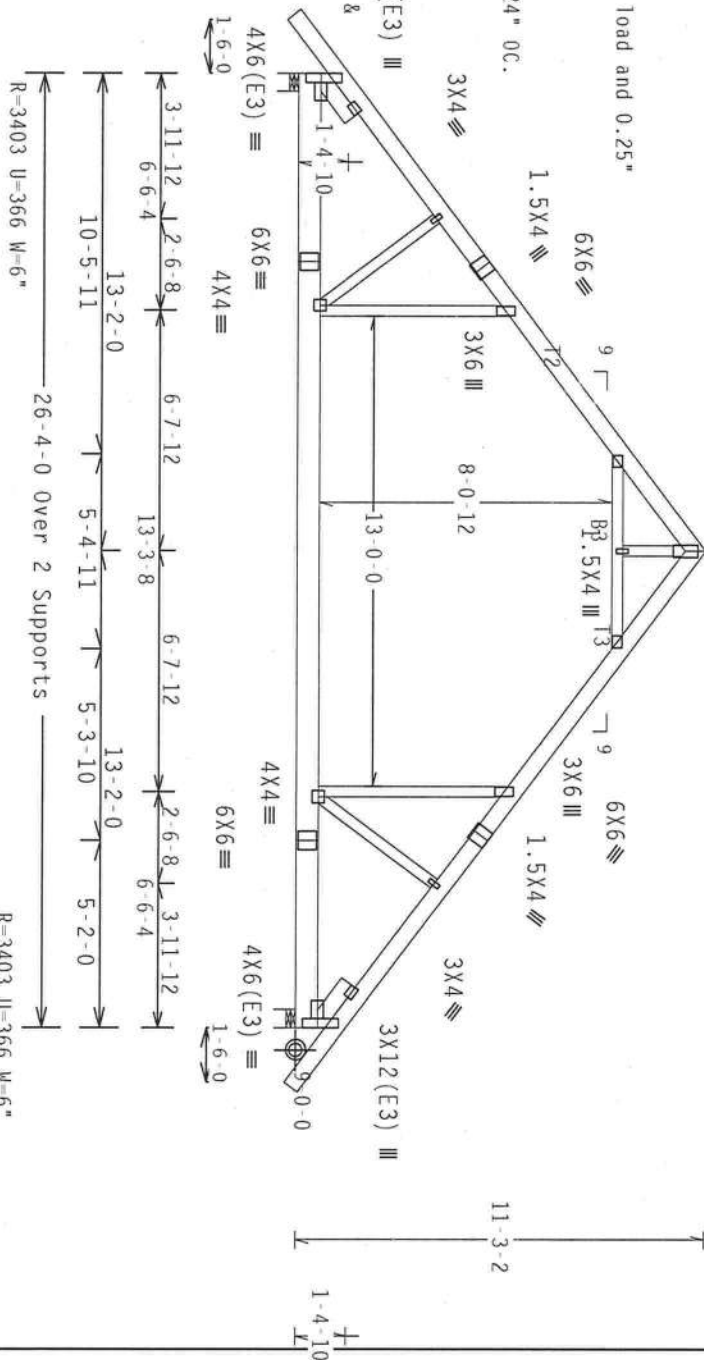
3X4 ≡



3X4 ≡



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 2. $\frac{1}{3}$
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Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

QTY:1

FL/-/4/-/-/R/-

Scale = .1875"/Ft.

WARNING: THESE RECORDS EXISTENCE CAUSE INFORMATION, HANDLING, SHIPPING, INSTALLING AND MAINTAINING OF THESE RECORDS TO BE A HAZARDOUS ACTIVITY. THESE RECORDS SHOULD BE HANDLED, SHIPPED, INSTALLED AND MAINTAINED IN ACCORDANCE WITH THE FOLLOWING SAFETY PRACTICES: (1) WEAR PROTECTIVE EQUIPMENT (EYE PROTECTION, GLOVES, RESPIRATOR, ETC.) WHEN HANDLING THESE RECORDS; (2) DO NOT INHALE DUST OR FUMES FROM THESE RECORDS; (3) DO NOT DRINK OR EAT FROM CONTAINERS THAT HAVE BEEN IN CONTACT WITH THESE RECORDS; (4) DO NOT DISPOSE OF THESE RECORDS IN A MANNER THAT COULD CAUSE ENVIRONMENTAL CONTAMINATION; (5) DO NOT BURN THESE RECORDS; (6) DO NOT SMOKE OR DRINK ALCOHOL WHILE HANDLING THESE RECORDS; (7) DO NOT ALLOW CHILDREN OR PETS TO BE IN CONTACT WITH THESE RECORDS; (8) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH FOOD OR DRINK; (9) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH OPEN FLAMES OR HEAT SOURCES; (10) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH WATER OR OTHER LIQUIDS; (11) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH OILS OR GREASES; (12) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH ACIDIC OR ALKALINE SUBSTANCES; (13) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH SOLVENTS OR OTHER CORROSIVE SUBSTANCES; (14) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH ELECTRICAL EQUIPMENT OR POWER SOURCES; (15) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH MOVING PARTS OF MACHINERY; (16) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH SHARP OBJECTS; (17) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH HOT SURFACES; (18) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH RADIATION; (19) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH ULTRASONIC WAVES; (20) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH VIBRATION; (21) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH STATIC ELECTRICITY; (22) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH MAGNETIC FIELDS; (23) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH ELECTROMAGNETIC INTERFERENCE; (24) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH OTHER HAZARDOUS SUBSTANCES; (25) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH OTHER HAZARDOUS ACTIVITIES. THESE RECORDS SHOULD BE HANDLED, SHIPPED, INSTALLED AND MAINTAINED IN ACCORDANCE WITH THE FOLLOWING SAFETY PRACTICES: (1) WEAR PROTECTIVE EQUIPMENT (EYE PROTECTION, GLOVES, RESPIRATOR, ETC.) WHEN HANDLING THESE RECORDS; (2) DO NOT INHALE DUST OR FUMES FROM THESE RECORDS; (3) DO NOT DRINK OR EAT FROM CONTAINERS THAT HAVE BEEN IN CONTACT WITH THESE RECORDS; (4) DO NOT DISPOSE OF THESE RECORDS IN A MANNER THAT COULD CAUSE ENVIRONMENTAL CONTAMINATION; (5) DO NOT BURN THESE RECORDS; (6) DO NOT SMOKE OR DRINK ALCOHOL WHILE HANDLING THESE RECORDS; (7) DO NOT ALLOW CHILDREN OR PETS TO BE IN CONTACT WITH THESE RECORDS; (8) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH FOOD OR DRINK; (9) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH OPEN FLAMES OR HEAT SOURCES; (10) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH WATER OR OTHER LIQUIDS; (11) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH OILS OR GREASES; (12) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH ACIDIC OR ALKALINE SUBSTANCES; (13) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH SOLVENTS OR OTHER CORROSIVE SUBSTANCES; (14) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH ELECTRICAL EQUIPMENT OR POWER SOURCES; (15) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH MOVING PARTS OF MACHINERY; (16) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH SHARP OBJECTS; (17) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH HOT SURFACES; (18) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH RADIATION; (19) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH ULTRASONIC WAVES; (20) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH VIBRATION; (21) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH STATIC ELECTRICITY; (22) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH MAGNETIC FIELDS; (23) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH ELECTROMAGNETIC INTERFERENCE; (24) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH OTHER HAZARDOUS SUBSTANCES; (25) DO NOT ALLOW THESE RECORDS TO COME IN CONTACT WITH OTHER HAZARDOUS ACTIVITIES.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH IT11, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & DRAGING OF TRUSSES.

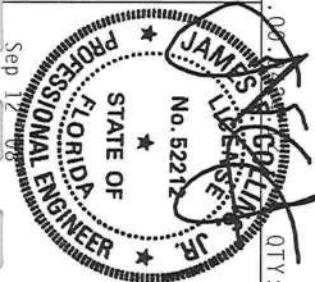
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A.3 OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/HFI 1 SEC. 2.



ALPINE

ITW Building Components Group Inc.
 Haines City, FL 33844
 FL COA #0778



TC LL	20.0 PSF	REF	R8228- 71305
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255016
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	104114
DUR.FAC.	1.25	FROM	AH
SPACING SEE ABOVE		DRFF-	1TKU8228Z01

Top Chord 2x6 SP #2 : T2 2x6 SP #1 Dense:

: T3 2x6 SP SS:
Bot Chord 2x8 SP #1 Dense : B3 2x4 SP #2 Dense:

Webbs 2x4 SP #3
: Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'

: Rt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'

Calculated horizontal deflection is 0.22" due to live load and 0.33" due to dead load.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Trusses to be spaced at 41.0" OC maximum.

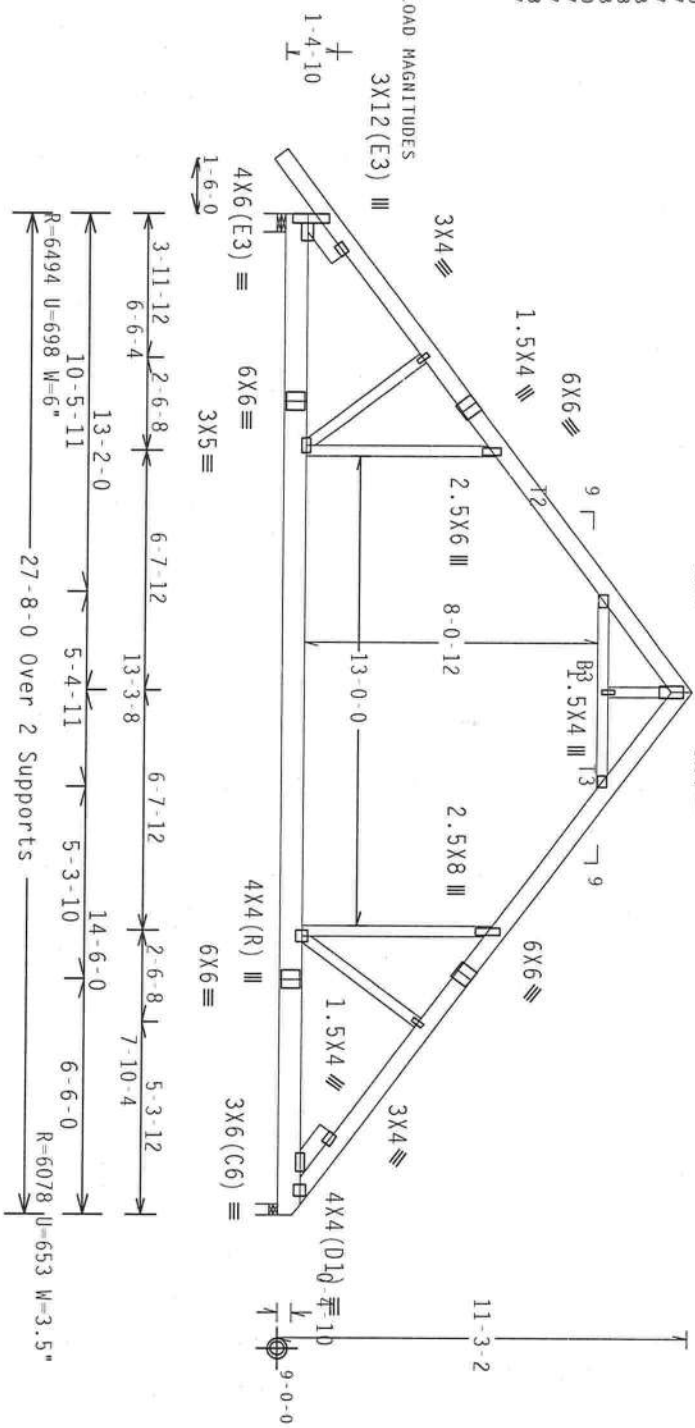
Collar-tie braced with continuous lateral bracing at 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

SPECIAL LOADS

TC - From	111 PLF at -1.25 to 111 PLF at 0.00
TC - From	348 PLF at 0.00 to 280 PLF at 5.17
TC - From	280 PLF at 5.17 to 281 PLF at 6.67
TC - From	303 PLF at 6.67 to 281 PLF at 8.33
TC - From	154 PLF at 8.33 to 111 PLF at 10.47
TC - From	111 PLF at 10.47 to 111 PLF at 13.17
TC - From	111 PLF at 13.17 to 111 PLF at 15.86
TC - From	154 PLF at 15.86 to 154 PLF at 18.00
TC - From	281 PLF at 18.00 to 303 PLF at 19.67
TC - From	260 PLF at 19.67 to 280 PLF at 21.17
TC - From	111 PLF at 21.17 to 348 PLF at 26.33
TC - From	34 PLF at 26.33 to 111 PLF at 27.38
PLT - From	34 PLF at 10.81 to 9 PLF at 15.53
BC - From	9 PLF at -1.78 to 101 PLF at 0.00
BC - From	101 PLF at 0.00 to 272 PLF at 6.67
BC - From	272 PLF at 6.67 to 272 PLF at 19.67
BC - From	101 PLF at 19.67 to 135 PLF at 27.38
TC - From	220 LB Conc. Load at 0.00, 135 PLF at 27.38
BC -	178 LB Conc. Load at 6.67, 19.67

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS. TRUSS ENGINEER & FABRICATOR ARE NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/0(0)

7.36.00

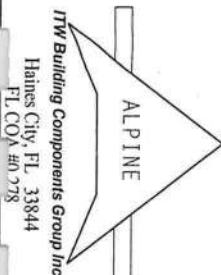
UTT:1

FL/-/4/-/R/-

Scale = .1875"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE COMPONENT SAFETY INFORMATION. PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WEA GOOD TRUSS CONSTRUCTION OF AMERICA, 6300 OTHERS, KNOXVILLE, TENNESSEE, 37919 FOR SAFETY PRACTICES PRIOR TO REPAIRING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS AND SPACING MUST BE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN. THE DESIGNER IS NOT RESPONSIBLE FOR THE TRUSS IN COMPLIANCE WITH THE FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE TRUSS IN COMPLIANCE WITH THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ALKALAI AND TPI). THE BCG CONNECTOR PLATES ARE MADE OF 2018/1664 (44/55/2) ASTM A563 GRADE 40/50 (44/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



4 COMPLETE TRUSSES REQUIRED

Roofing Schedule: (124 Common (0.148"x3.25", min.) nails)

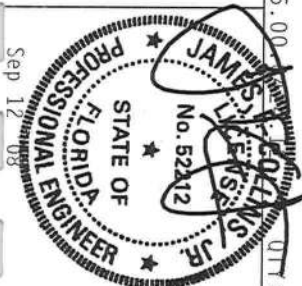
Top Chord: 1 Row @ 7.00" o.c.
Bot Chord: 1 Row @ 7.00" o.c.
Webbs : 1 Row @ 4" o.c.

Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting. In addition apply (1) 1/2" bolt at each 2x6 and 2x8 chord joint location.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 3.92 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GcPf(+/-)=0.18

Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.



TC LL	20.0 PSF	REF R8228- 71307
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUR8228 08255017
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEON- 104319
DUR.FAC.	1.25	FROM AH
SPACING	SEE ABOVE	JREF- 1TKU8228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCPI(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

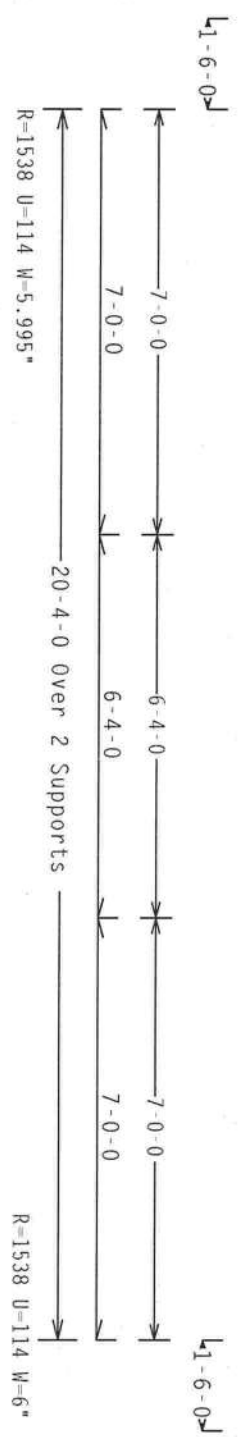
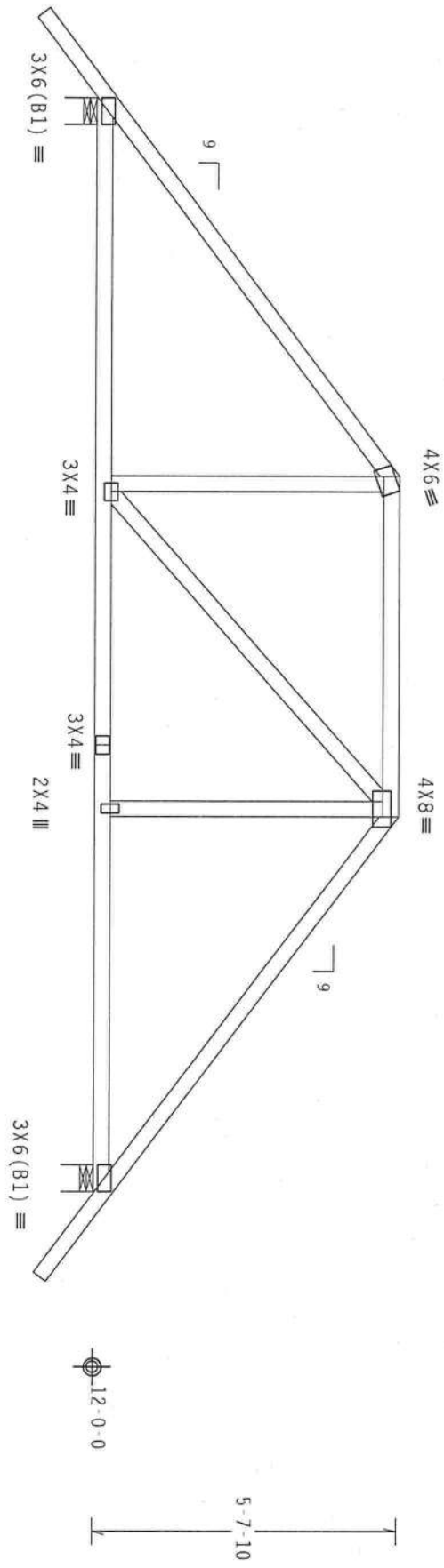
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Trusses to be spaced at 16.0" OC maximum.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-8-2 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-8-2 overhang. Right side jacks have 7-0-0 setback with 0-0-0 cant and 1-8-2 overhang.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TP1-2002(STD)/FBC

Cq/RT=1.00(1.25)/0(0)

7.36.00

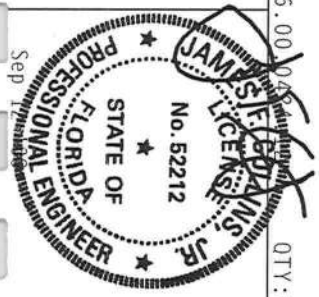
QTY:1

FL/-/4/-/-/R/-

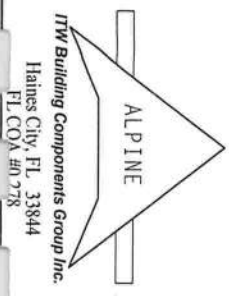
Scale = .3125"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (INCLUDING COMPONENT SAFETY INFORMATION). PROHIBITED BY TP1 (TRUSS PLATE INSTITUTE, 218 E. 10TH ST., SUITE 300, ALBUQUERQUE, NM, 87102) AND AISC (6000 TRUSS, CONDUCT OF AMERICA, 6200 ENTERPRISE LANE, HOUSTON, TX 77036) TO THE EXTENT OF CONFLICT. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. ENG. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, BY A/R/P/A, AND TP1. THE REG. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AWS (NATIONAL DESIGN SPEC., BY A/R/P/A, AND TP1. THE REG. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. (AISC) OR THIS DRAWING INDICATES. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AWS/TP1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71309
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCSR8228 08255020
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEQN-	101470
DUR. FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	UREF-	1TKU8228Z01



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #00778

Top Chord 2x4 SP #2 Dense
Bot Chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

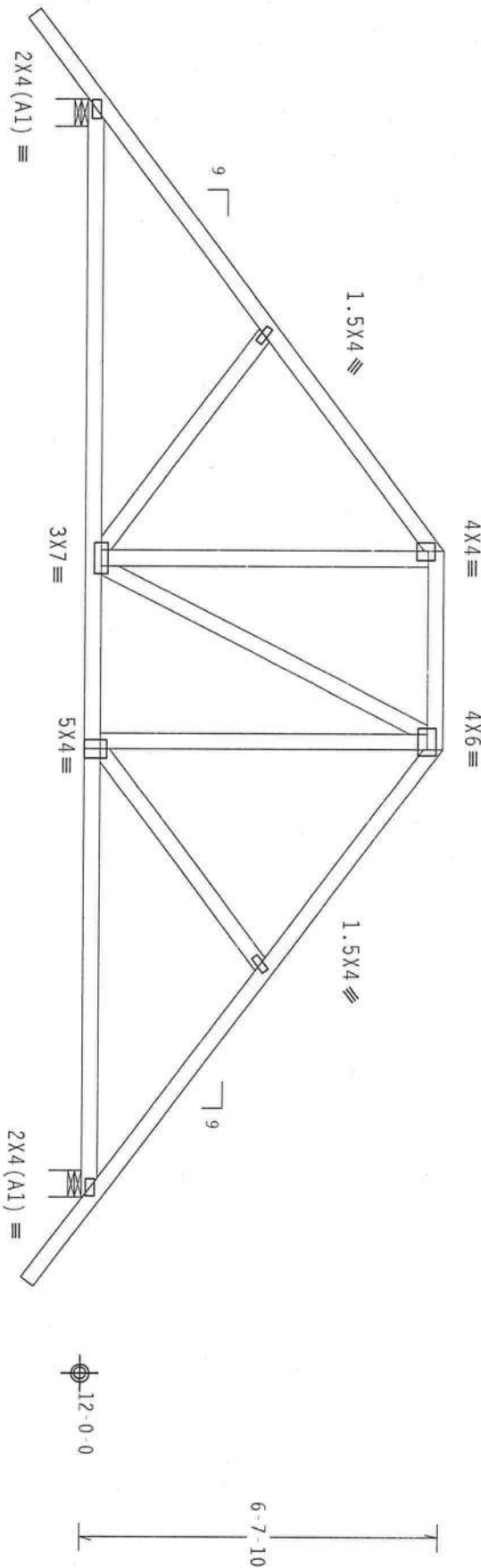
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI(+/-)-0.18

Wind reactions based on MMFRS pressures.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



4-4-3 8-4-0 3-11-13 3-8-0 3-8-0 3-11-13 8-4-0 4-4-3
8-4-0 8-4-0 3-8-0 3-8-0 8-4-0
R-650 U=55 W=5.996"

PLT TYP. Wave

Design Crit: TPI-2002 (STD) /FBC
Cq/RT=1.00(1.25)/0(0)

7.36.00

QTY:1

FL/-/4/-/-/R/-

Scale = .3125"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (QUALITY) COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 ENTERPRISE LANE, MOHAWK, MI 48719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED OR COMBINED SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS IN CONFORMANCE WITH THE DESIGN SPECIFICATIONS. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS IN CONFORMANCE WITH THE DESIGN SPECIFICATIONS. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS IN CONFORMANCE WITH THE DESIGN SPECIFICATIONS.

THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS IN CONFORMANCE WITH THE DESIGN SPECIFICATIONS. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS IN CONFORMANCE WITH THE DESIGN SPECIFICATIONS. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS IN CONFORMANCE WITH THE DESIGN SPECIFICATIONS.

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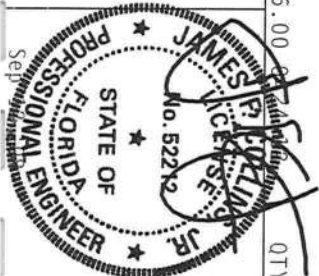
THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS IN CONFORMANCE WITH THE DESIGN SPECIFICATIONS. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS IN CONFORMANCE WITH THE DESIGN SPECIFICATIONS. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS IN CONFORMANCE WITH THE DESIGN SPECIFICATIONS.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA 00778



TC LL	20.0 PSF	REF	R8228- 71310
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255027
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT. LD.	40.0 PSF	SEON-	101475
DUR. FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

110 mph wind, 15.38 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures.

Trusses to be spaced at 16.0" OC maximum.

Detection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.


$$Cq/RT=1.00(1.25)/0(0)$$

QTY:1

FL/-/4/-/-/R/-

Scale = .3125"/Ft.

AMERICAN COLLEGE NEWS

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

TC LL	20.0 PSF	REF	R8228- 71311
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255035
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT.LD.	40.0 PSF	SEON-	101480
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	UREF -	1TKU8228201

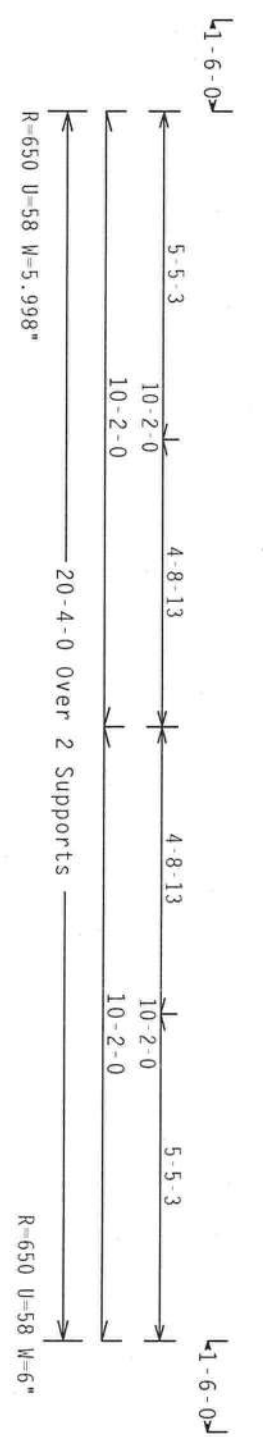
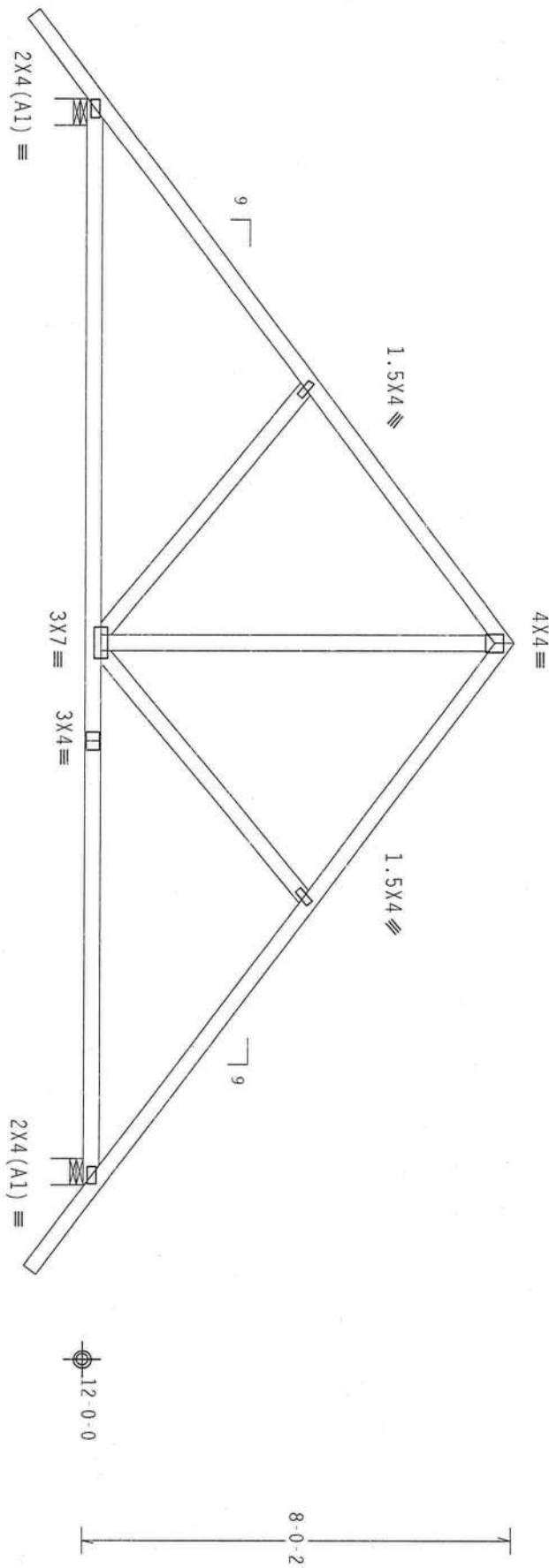
Top Chord 2x4 SP #2 Dense
Bot Chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.57 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, IW=1.00 GCPI(+/-)-0.18

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MMFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002 (STD) /FBC

Cq/RT=1.00(1.25)/0(0)

7.36.00.00

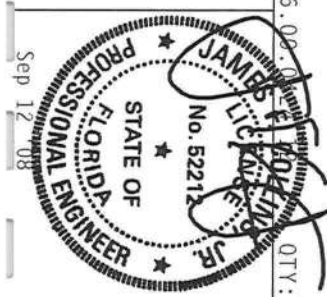
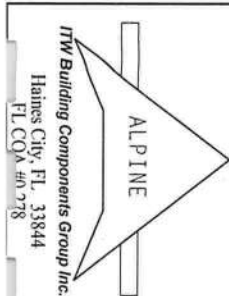
QTY:1

FL/-/4/-/-/R/-

Scale = .3125"/ft.

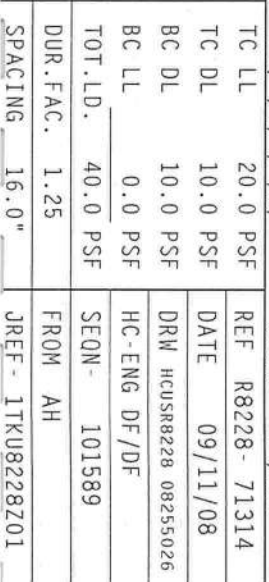
****WARNING**** TRUSSES REQUIR EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, HOUSTON, TX 77037) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE SPECIFIED, ALL DIMENSIONS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** THROUGH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING, FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. THE BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BCS (NATIONAL DESIGN SPEC., BY ACPA) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 2019/166A (W/H/S/S) ASTM A551 GRADE 40/50 (W, K/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS BRACING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71312
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255041
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEON-	101487
DUR. FAC.	1.25	FROM	AH
SPACING	16.0"	UREF-	1TKU8228Z01

SPECIAL LOADS			
----- (LUMBER DUR. FAC. = 1.25 / PLATE DUR. FAC. = 1.25)			
TC	From	43 PLF at 0.00 to	43 PLF at 1.63
TC	From	42 PLF at 1.63 to	42 PLF at 6.38
TC	From	42 PLF at 6.38 to	42 PLF at 8.29
TC	From	42 PLF at 8.29 to	42 PLF at 13.04
TC	From	43 PLF at 13.04 to	43 PLF at 14.67
BC	From	13 PLF at 0.00 to	13 PLF at 14.67
TC		32 LB Conc. Load at	1.63, 13.04
TC		36 LB Conc. Load at	2.17, 12.49
TC		63 LB Conc. Load at	4.06, 10.61
TC		89 LB Conc. Load at	5.95, 8.72
TC		238 LB Conc. Load at	6.38, 8.29
TC		97 LB Conc. Load at	6.89
BC		13 LB Conc. Load at	1.63, 13.04
BC		14 LB Conc. Load at	2.17, 12.49
BC		26 LB Conc. Load at	4.06, 10.61
BC		37 LB Conc. Load at	5.95, 8.72
BC		94 LB Conc. Load at	6.38, 8.29
BC		41 LB Conc. Load at	6.89



110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



Scale = .5"/Ft.

James A. Gilliam, Jr.
No. 52212
LICENSE

TC LL	20.0 PSF	REF R8228- 71315
TC DL	10.0 PSF	DATE 09/11/08



ALPINE

Haines City, FL 33844
FL COA #0078

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL=5.0 psf, Iw=1.00 Gcpi (+/-)=0.18

Wind reactions based on MFRS pressures.

Wind reactions based on MFRS pressures.



7.36.00

QTY:1

FL/-/4/-/-/R/-

Scale = .375" / Ft.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0078



TC LL	20.0 PSF	REF	R8228- 71316
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCU8R8228 08255023
BC LL	0.0 PSF	HC-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEQN-	101597
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

SPECIAL LOADS

Trusses to be spaced at 16.0" OC maximum.

TRUSS MAY NOT BE INSTALLED END FOR END.

Nailing Schedule: (12d Common (0.148"x3.25", min.)_nails)

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gc(+)=-0.18

Wind reactions based on MWFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$
~~7.36.00.~~

QTY:1

FL/-/4/-/-/R/-/

Scale = .375" / Ft.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT

[illegible]

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/FP1 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71317
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255021
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	40719
DUR.FAC.	1.25	FROM	AH
SPACING SEE ABOVE		JREF-	1TKU8228Z01

Top Chord 2x4 SP #2 Dense
Bot Chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

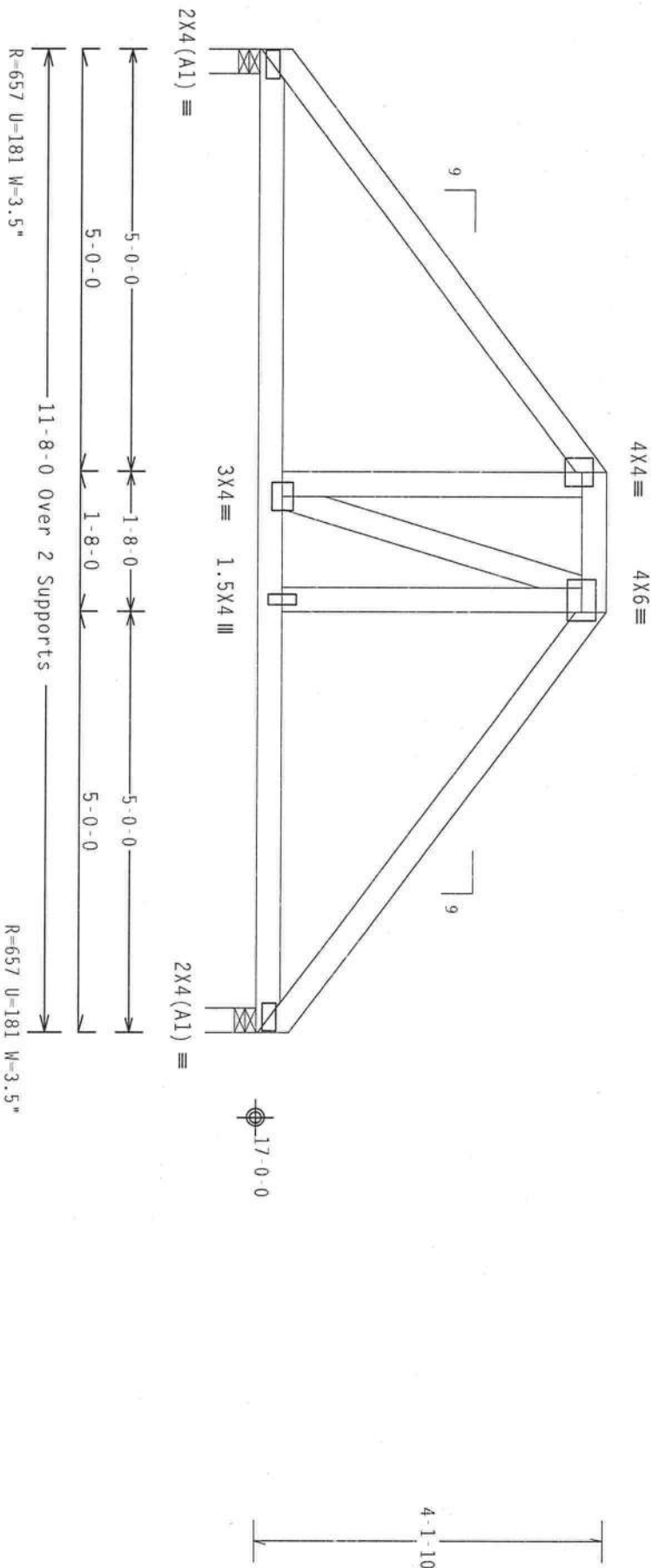
#1 hip supports 5-0-0 jacks with no webs.

110 mph wind, 19.26 ft mean hgt, ASCE 7-02, PART-ENC. bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $GCFI(+/-)=0.55$

Wind reactions based on MMFRS pressures.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.00

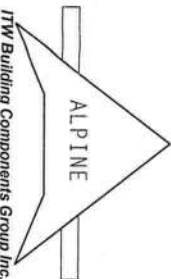
QTY:1

FL/-/4/-/-/R/-

Scale = .5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, PROTON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

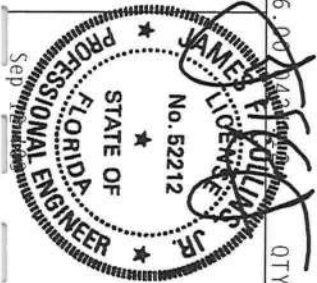
****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING OR TO THE PERSONS OR PROPERTY CAUSED BY THE FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SPECIFICATIONS AND THE INSTALLATION INSTRUCTIONS. THE TRUSS IS TO BE USED IN CONFORMANCE WITH THE DESIGN SPECIFICATIONS AND THE INSTALLATION INSTRUCTIONS. THE TRUSS IS TO BE USED IN CONFORMANCE WITH THE DESIGN SPECIFICATIONS AND THE INSTALLATION INSTRUCTIONS.



ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #H278



SPACING	SEE ABOVE	JREF - 1TKU8228Z01
DUR. FAC.	1.25	FROM AH
TOT. LD.	40.0 PSF	SEON- 104273
BC LL	0.0 PSF	HC-ENG DF/DF
BC DL	10.0 PSF	DRW HCUR8228 08255018
TC DL	10.0 PSF	DATE 09/11/08
TC LL	20.0 PSF	REF R8228- 71318

110 mph wind, 18.76 ft mean hgt, ASCE 7-02, PART ENC. bldg, located anywhere in roof, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $1w=1.00$ GCP(+/-)=0.55

Wind reactions based on MMFRS pressures.

Trusses to be spaced at 16.0" OC maximum.

Left side jacks have 3-8-0 setback with 1-6-0 cant and 0-0-0 overhang. End jacks have 3-8-0 setback with 1-6-0 cant and 0-0-0 overhang. Right side jacks have 3-8-0 setback with 1-6-0 cant and 0-0-0 overhang.


$$Cq/RT=1.00(1.25)/0(0)$$

QTY: 1

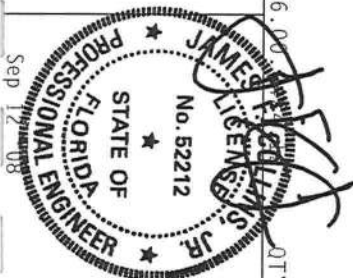
Scale = .5" / Ft.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITU REG. INC. SHALL NOT



ALPINE

Haines City, FL 33844
FL COA #0078



TC LL	20.0 PSF	REF	R8228- 71319
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUSR8228 08256035
BC LL	0.0 PSF	HC- ENG	AP/AP
TOT.LD.	40.0 PSF	SEQN-	104277
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

110 mph wind, 19.26 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.55$

Wind reactions based on MWFRS pressures.

Trusses to be spaced at 16.0" OC maximum.

 $Cq/RT=1.00(1.25)/0(0)$

QTY:1

Scale = .5"/Ft.

****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL NOT

ITW Building Components Group Inc.

Haines City, FL 33844

TC LL	20.0 PSF	REF	R8228- 71320
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCHSR0228 08255019
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON-	104281
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

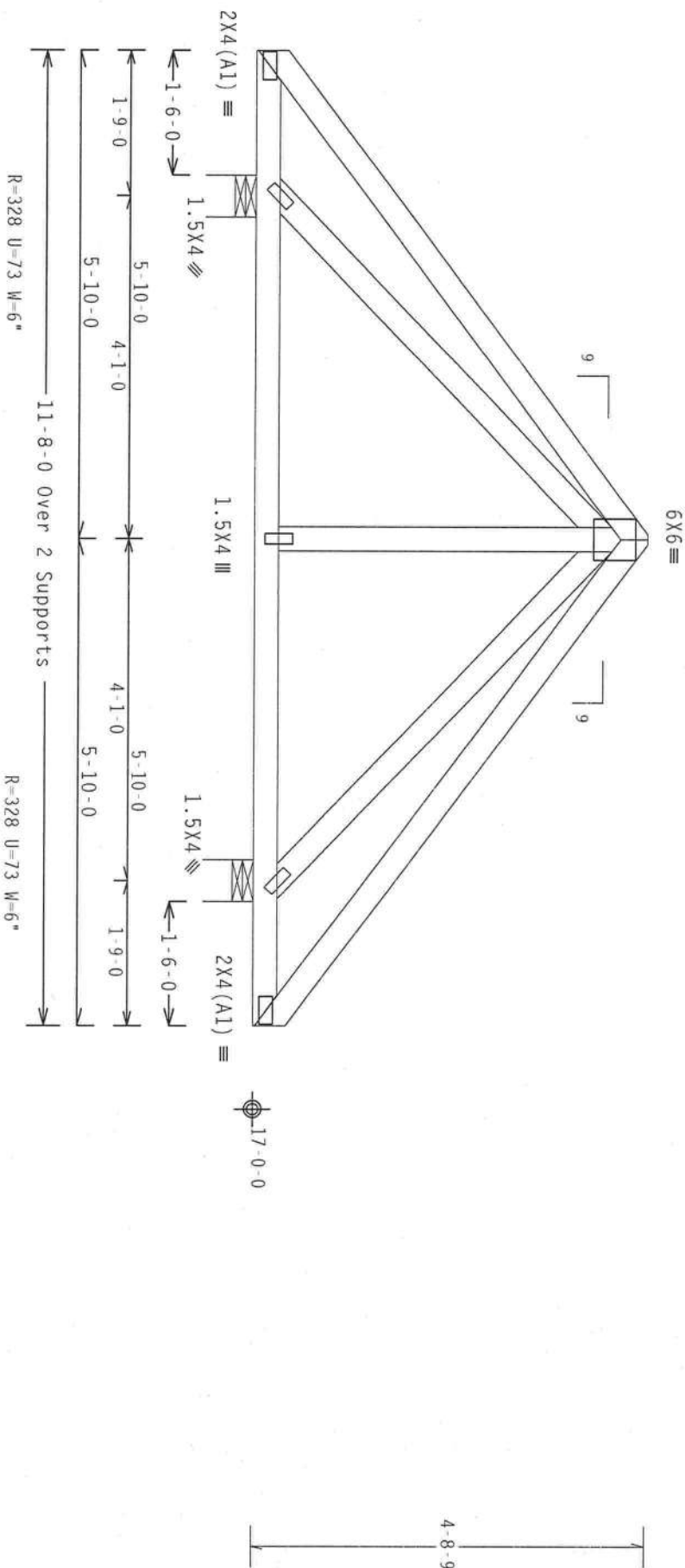
Top Chord 2x4 SP #2 Dense
Bot Chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 19.55 ft mean hgt, ASCE 7-02, PART-ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCPI(+/-)=0.55

Wind reactions based on MMFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36-00

QTY:1

FL/-/4/-/-/R/-

Scale = .5"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC&I (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, PROUDSON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF THIS TRUSS. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF THIS TRUSS. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF THIS TRUSS. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF THIS TRUSS. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF THIS TRUSS. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

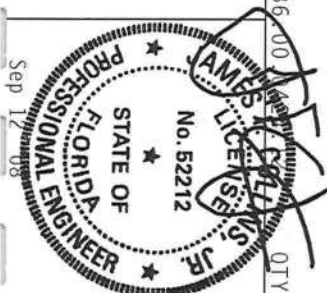
THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF THIS TRUSS. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.



NTW Building Components Group Inc.

Haines City, FL 33844

FL COA no. 278

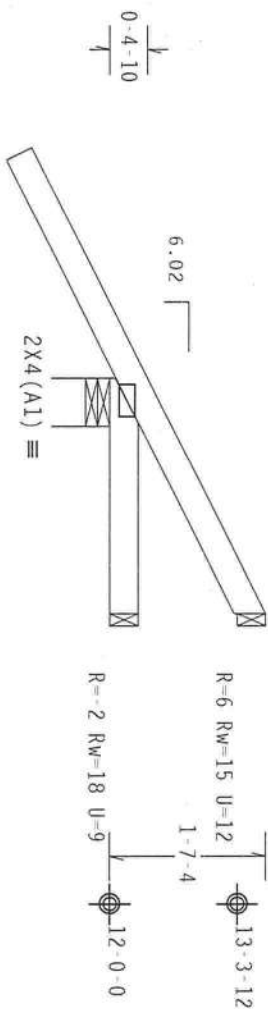


TC LL	20.0 PSF	REF	R8228- 71321
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255020
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEON-	104285
DUR. FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 Gcpl (+/-)=0.18

Wind reactions based on MMFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



2-2-15
2-5-3 Over 3 Supports
R=232 U=36 W=6"

Design Crit: $TP1-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

$$Cq/RT=1.00(1.25)/0(0)$$

7.36.00

QTY:1

FL/-/4/-/-/R/-/-

Scale = .5"/Ft.

WARNING: FIRE RESISTING CONCRETE REINFORCED CEMENT FIBER CEMENT BOARD SHALL BE USED IN FABRICATION, HANDLING, SHIPPING, INSTALLING, AND BROCKING REFER TO ACSI (BUILDING CONCRETE INSTITUTE), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, (800) 368-6000 TRUSS COMPANY, INC., 65000 ENTERPRISE LANE, MOUNTAIN VIEW, TX 75755 FOR SAFETY PRACTICES PRIOR TO REMOVING THE FIBER CEMENT BOARD. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN, OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MSD (NATIONAL DESIGN SPEC., BY AISC) AND THE CONNECTOR PLATES ARE MADE OF 20/19/166A (H, M/55/K) ASTM A553 GRADE 40/60 (H, K/H, 55) GALV., STEEL. PLATES TO EACH FACE OF BRISSES AND UNLESS OTHERWISE NOTED ON THIS DESIGN, POSITION ARE DRAWINGS 15GA-2

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN OF THE 1994-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENTS.

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

1

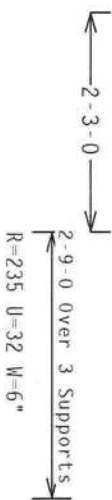
TC LL	20.0 PSF	REF	R8228- 71323
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCHSR8228 08255052
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON-	101384
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JBEF- 1TKU8228Z01	



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0-278

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 gcpl (+/-)=0.18

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Scale = .5" / Ft.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC., SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1, OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIAA) AND IPT. ITW BCS

CONNECTOR PLATES ARE MADE OF 20/10/16GA (W, H/5S/K) ASTM A653 GRADE 40/60 (W, K/H, 5S) GALV. STEEL. APPLY

PLATES TO EACH PAGE OF CROSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION FOR THE DRAWINGS 160A-2

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT AND INSPECTION OF PLATES FOLLOWED BY (1) SMALL REPAIRS AND BY (2) 2002 SEC. 5. A SEAL ON THIS

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE OWNER AND THE ARCHITECT. OF COURSE, COLLECTING MATERIALS FOR THE PROJECTS WITH WHICH WE ARE INVOLVED IS A CHALLENGING TASK. WE ARE PLEASED TO COLLECT FOR THE PROJECTS WITH WHICH WE ARE INVOLVED.

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

100

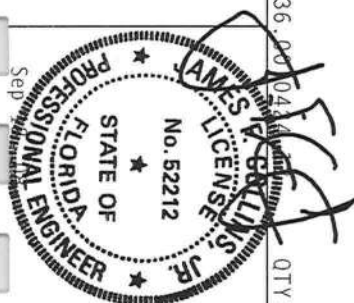


ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

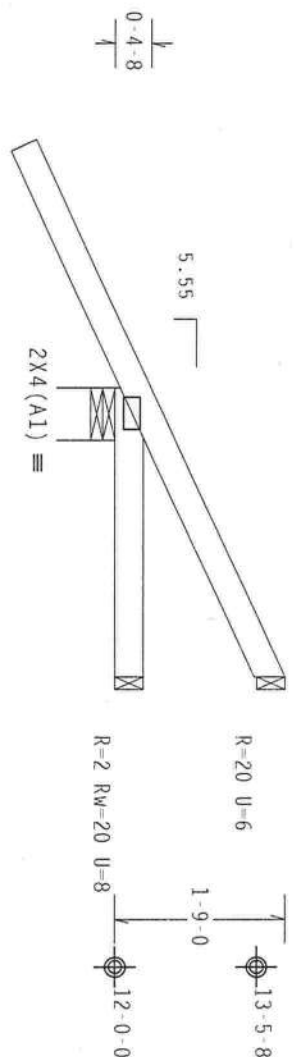


TC LL	20.0 PSF	REF	R8228- 71324
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255054
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	101391
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.
Trusses to be spaced at 16.0" OC maximum.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18
Wind reactions based on MMFRS pressures.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

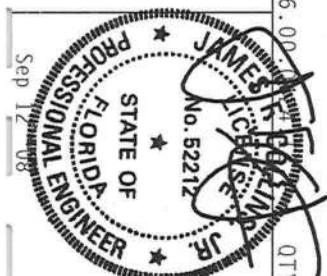
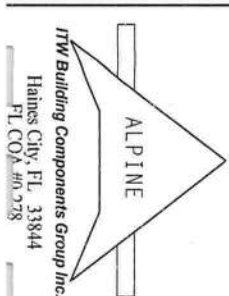


PLT TYP. Wave Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) 7.36.00 QTY:1 FL/-/4/-/-/R/- Scale=.5"/Ft.

WARNING TRUSSES REQUIRING EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NCA (NATIONAL TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, HOUSTON, TX 77036) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR GROUND SHALL HAVE PROPERLY ATTACHED STRUCTURAL PLATES AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

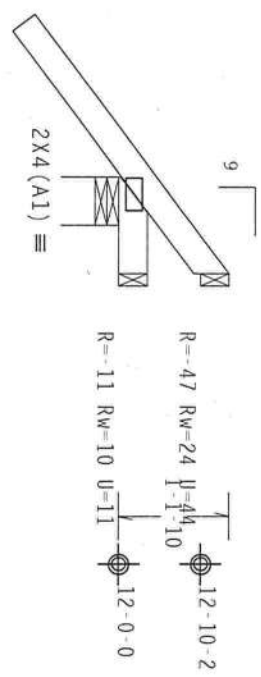


TC LL	20.0 PSF	REF R8228-71325
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUR80228 0825051
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEQN- 101395
DUR. FAC.	1.25	FROM AH
SPACING	16.0"	JREF- 1TKU8228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.
Trusses to be spaced at 16.0" OC maximum.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCPI(+/-)=0.18
Wind reactions based on MWFRS pressures.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



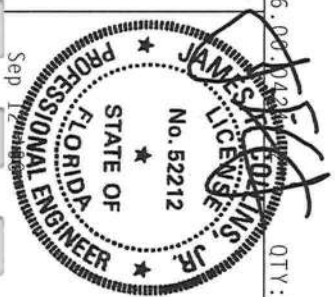
←1-6-0→
1-0-0 Over 3 Supports
R-189 U=36 W=5.996"

PLT TYP. Wave Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) 7.36.00(1.25) QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

WARNING TROSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

ITW Building Components Group Inc.
Haines City, FL 33844
FL CQA 40 278



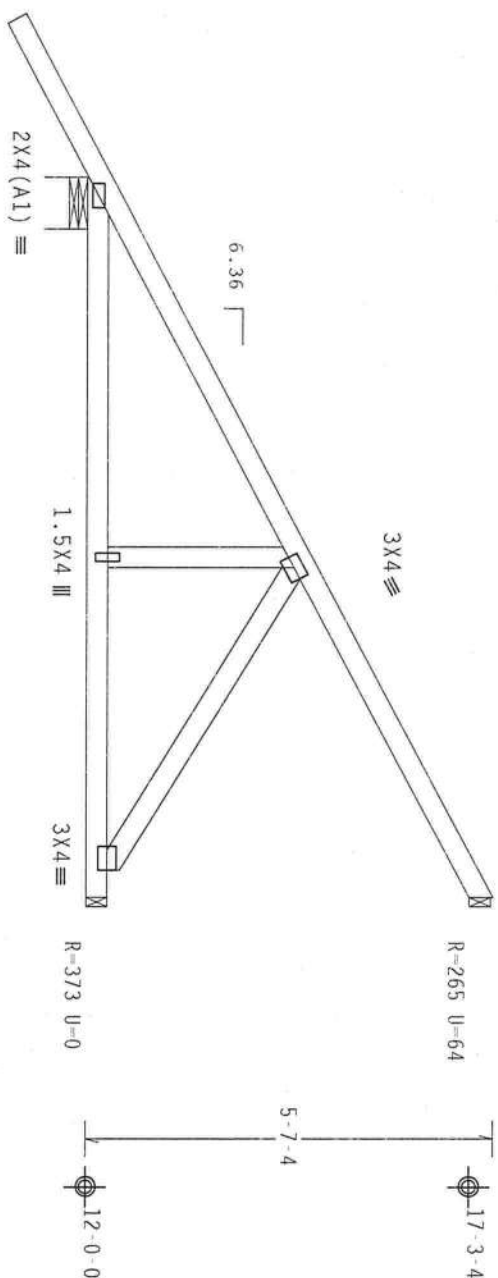
TC LL	20.0 PSF	REF	R8228- 71326
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255009
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEON-	101433
DUR. FAC.	1.25	FROM	AH
SPACING	16.0"	UREF-	1TKU8228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

Hipjack supports 7'-0" setback jacks with no webs.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $GCF(+/-)=0.18$
Wind reactions based on MWFRS pressures.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



2-1-7
5'-2-7
5'-2-7
4'-8-6
9'-10-13 Over 3 Supports
6'-3
R=265 U=64
R=373 U=0
R=496 U=49 W=8.486"

PLT TYP. Wave

Design Crit: TP1-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

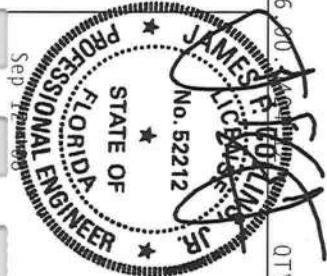
QTY:1

FL/-/4/-/-/R/-

Scale = .375"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 E. 11TH STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND NCA (NATIONAL TRUSS COUNCIL OF AMERICA, 6300 EXTENDED PARKWAY, SUITE 200, FORT WORTH, TEXAS 76116) FOR SAFETY PRACTICES FOR PREVENTING THESE CONDITIONS. OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

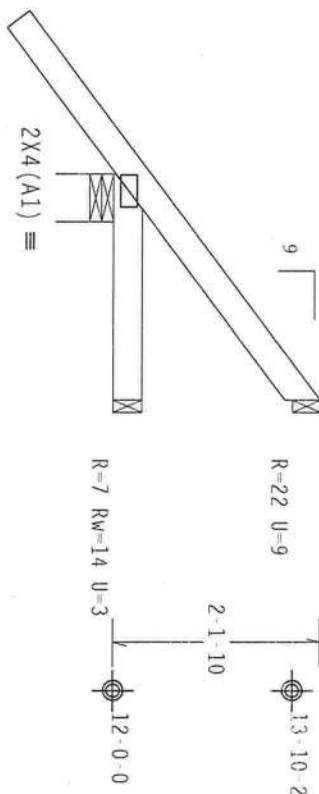


TC LL	20.0 PSF	REF R8228-71327
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUR8228 08255053
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEQN- 101461
DUR. FAC.	1.25	FROM AH
SPACING	SEE ABOVE	JREF- 1TKU8228Z01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof; CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



$\overbrace{1-6-0}^{\text{2-4-0 Over 3 Supports}}$
 $R=176 \quad U=15 \quad W=5.996''$

Design Crit: $TP1-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

7.3600

QTY:1

FL/-/4/-/-/R/-/-

Scale = .5"/ft.

WARNING THESE BUILDING EXISTENCE CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND PROTECTING REFER TO DC-51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FBI CRISIS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, FOR \$2000. BRUCE COUNTELL OF AMERICA, 63000 ENTERPRISE LANE, MIDDLETON, WI, 53170 FOR SAFETY PRACTICES AND WPCO BECOMING THESE STRUCTURES, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PAIRLS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED CLOUD CEILING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

TP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

CONNECTOR PLATES ARE MADE OF 20/18/16GA (H, H/SS/K) ASTM A563 GRADE 40/60 (H, K/H, SS) GALV. STEEL. APPLY

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF IP11-2002 SEC.3. A SEAL ON THIS

DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

10

0.07 — ENT 2572

T0707700VI1-1740

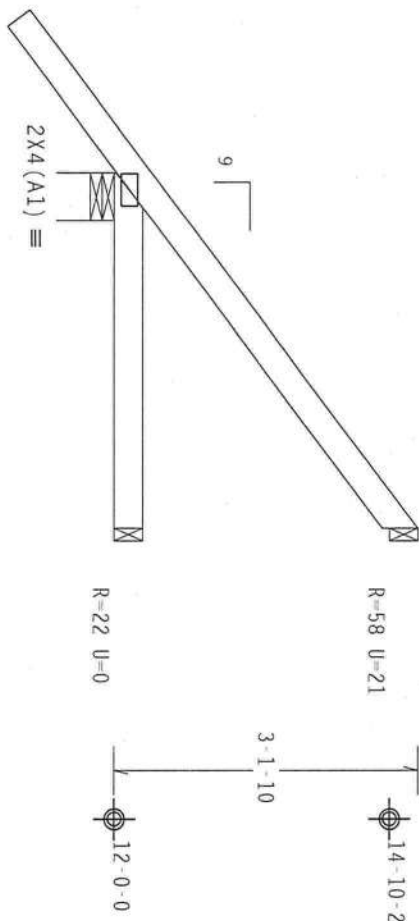
1

THIS WORK PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY KUSS M.K.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 Gcp1(+/-)0.18

Wind reactions based on MMFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



3-8-0 Over 3 Supports

R=202 U=8 W=5.993"

Design Crit: $TP1-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

$$Cq/RT=1.00(1.25)/0(0)$$

7.36.00.04 Quantity QTY:1

QTY: 1

FL/-/4/-/-/R/-/

Scale = .5" / Ft.

WARNING—PROCESSES (INCLUDING EXISTING CASES IN FABRICATION), HANDLING, SHIPPING, INSTALLING AND BROCKING REFERENCE TO GC51 (BUILDING COMPONENT CASE INFORMATION). PUBLISHED BY TPI (TRESS PAPER INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD INSTITUTE OF AMERICA), 63000 UNIVERSITY ENTERPRISE LANE, MIDDLETOWN, NJ, 07095 FOR TECHNICAL PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED CHORD CELLING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH THE TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TROSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/CPA) AND TPI. CONNECTOR PLATES ARE MADE OF 2018/1866A (U.H./SS/X) ASTM A653 GRADE 40/60 (U., K/H.55) GALV., STEEL. APPLY

PLATES TO EACH FEE OF THOSE AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS

ORNING, INDICATE ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE PROFESSIONAL DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

HOLDING DESIGNER PER ANSI/SP1 1 SEC. 2.

STATE OF FLORIDA
PROFESSIONAL ENGINEER
No. 52212
JAMES M. STUBBS, JR.
BOEING CITY

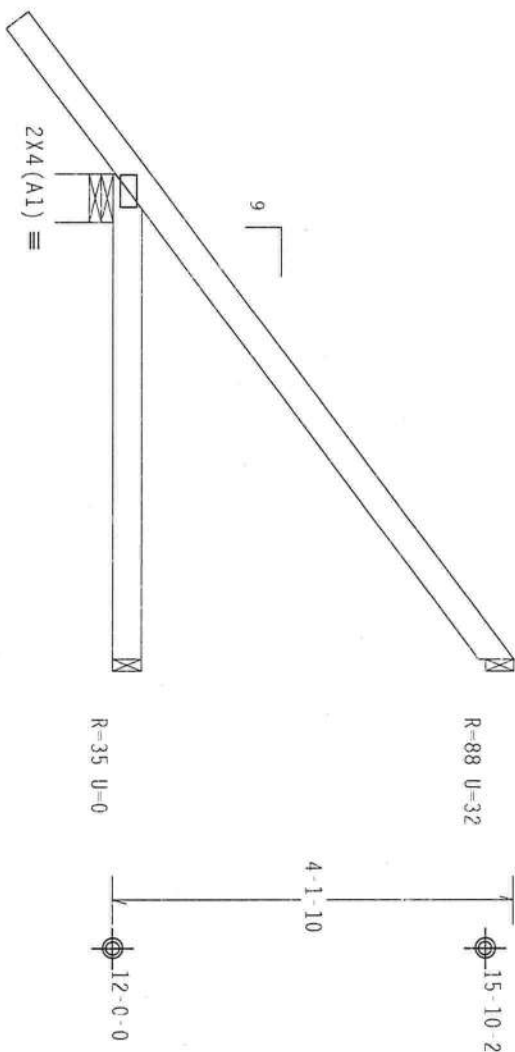
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TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255001
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	101442
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

THIS UMG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY RUSS MFK.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 Gcpi (+/-)=0.18

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

Roof overhang supports 2.00 psf soffit load.
Trusses to be spaced at 16.0" OC maximum.



Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

QTY:1

Scale = .5"/Ft.

****IMPORTANT***TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TIM BCG, INC. SHALL NOT



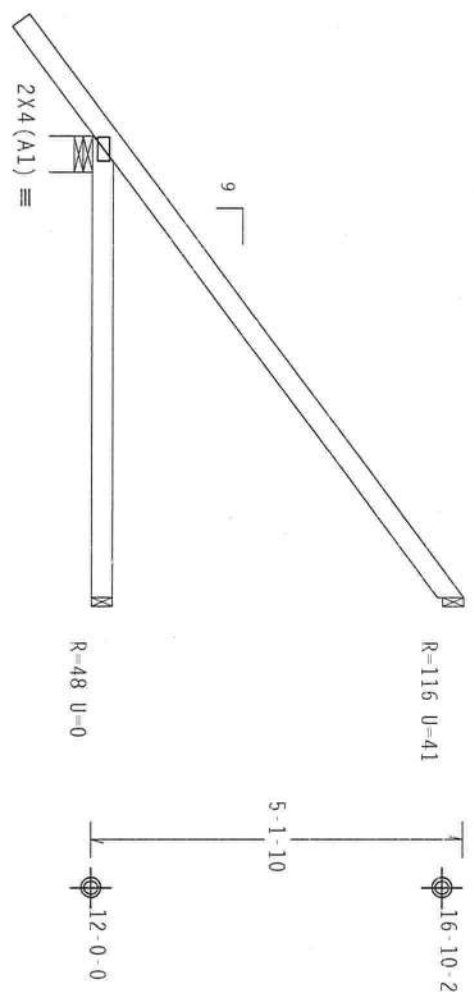
ALPINE

Haines City, FL 33844
FL COA #0278

TC LL	20.0 PSF	REF	R8228- 71330
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255057
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	101446
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Roof overhang supports 2.00 psf soffit load.
Trusses to be spaced at 16.0" OC maximum.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCPI(+/-)-0.18
Wind reactions based on MMFRS pressures.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



6-4-0 Over 3 Supports
R-269 U-0 W-5.995"

PLT TYP. Wave
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)
7.36.00
QTY:1
FL/-/4/-/-/R/-
Scale = .375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NCA (NATIONAL TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, HADISON, NJ 07719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR GROUND SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE TRUSS DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN CONFORMANCE WITH APPLICABLE PROVISIONS OF ANY APPLICABLE DESIGN SPEC. BY ACP/AI AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/10/1004 (40/10/50/7) ASTM A553 GRADE 40/60 (40 KSI/55 KSI) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

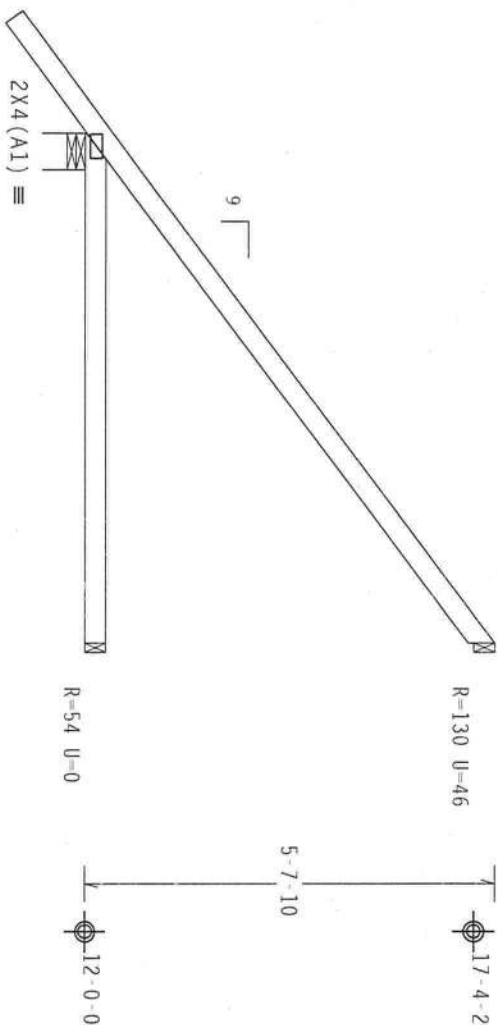
JAMES E. LITZ
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER
SEP 12 2008

TC LL	20.0 PSF	REF R8228- 71331
TC DL	10.0 PSF <td>DATE 09/11/08</td>	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUR8228 08255055
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEQN- 101450
DUR. FAC.	1.25	FROM AH
SPACING	16.0"	JREF- 1TKU8228Z01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi (+/-)=0.18

Wind reactions based on MMFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



1-6-0
7-0-0 Over 3 Supports
R=287 U=0 W=6"

Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

7.36.00.0142 ~~Produktions~~ DTY:1

FL/-/4/-/-/R/-/

Scale = .375" / Ft.

WARNING: THESE RESIN/ETHERE EPOXY CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO 8251 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY THE TIRROS PLASTIC INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD TRUSS COMPANY) OF AMERICA, 65000 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNDESIGNED OR OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUTTHICAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

TP1; ON FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES,

CONNECTION PLATES ARE MADE OF 20/10/10000 (H₂O/35/6) ASIN ABOVE GRADE 40/50 (H₂ K/H₂35) GALV., STEEL, APPLY

DRIVING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SAFELY FOR THE TRUCK COMPONENT

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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ALPINE

ITW Building Components Group Inc.
 Haines City, FL 33844
 FL COA #0278

[illegible]

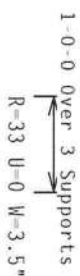
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TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255016
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	101454
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

(8-160--OWNER BUILDER Charles Meade **, ** - T19)

110 mph wind, 17.76 ft mean hgt, ASCE 7-02, PART-ENC. bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCpl(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



Design Crit: $TP1-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

QTY: 1

Scale = .5"/Ft.

James L. Jones
No. 52212

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STATE OF

FLORIDA
SCHOOL

DESIGN ENGINEER

CIVIL

—

—

TC LL	20.0 PSF	REF	R8228- 71333
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255019
BC LL	0.0 PSF	HC-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEQN-	101528
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

110 mph wind, 18.26 ft mean hgt, ASCE 7-02, PART-ENC. bldg, located anywhere in roof, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.55

Wind reactions based on MWFRS pressures.

Wind reactions based on MWFRS pressures.



Scale = .5"/Ft.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT



ALPINE

DESIGN CONFORMS THE APPLICABLE PROVISIONS OF MOST NATIONAL DESIGN SPEC. BY AREA AND IPT. THE REASON FOR THIS IS THAT THE DESIGN OF THE CONCRETE PLATES ARE MADE OF 20/10/64 (P.H/55/2) ASTM A563 GRADE 40/60 (P. W/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF THISS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1.6.6-2.2. AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AREA AS OF IPT-1-2012 SEC. 2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SILENT FOR THE DESIGN SHOW. THE SUITABILITY AND USE OF THIS CONSTRUCTION FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/IPT 1 SEC. 2.

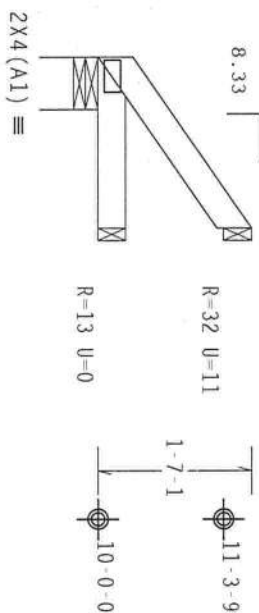


TC LL	20.0 PSF	REF	R8228- 71334
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255017
BC LL	0.0 PSF	HC-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEQN-	101533
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCP1 (+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



1-9-2 Over 3 Supports
 $R=54 \quad U=0 \quad W=6.506''$

PLT TYP. Wave

Design Crit: $TP1-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

7.38.00

QTY:1

FL/-/4/-/-/R/-/-

Scale = .5"/Ft.

WARNING: THESE PANELS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, UNLOADING, UNWRAPPING, INSTALLATION, AND DRIPPING. REFER TO DC51 (BUILDING COMPONENT SAFETY RECOMMENDATION), PUBLISHED BY THE FIBREGLASS PANEL INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND UFGA (GOOD PRACTICES) PUBLISHED BY THE FIBREGLASS PANEL INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 FOR SAFETY PRACTICES TO PREVENT THESE FUNCTIONS. INTERFERENCE LAM, MISSISSAUGA, ONT. L4V 1V7. THESE PANELS ARE NOT TO BE USED FOR STRUCTURAL FUNCTIONS. PANELS INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CHILLING.

No. 52212

4

BC NI 10 0 PCE

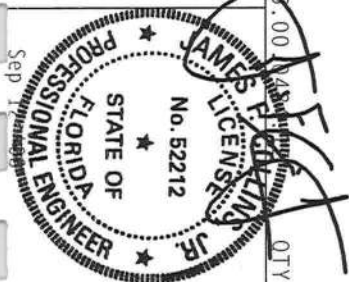
DBM UCUEB0030 0025E02A

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL 005 49378



Set

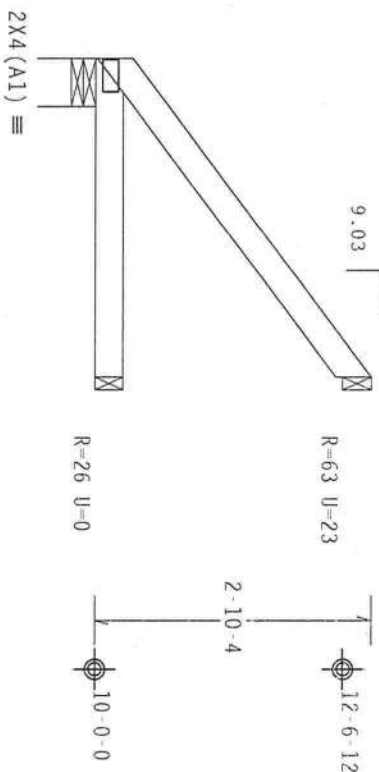
SPACING 16.0"

JREF - 1TKU8228Z01

110 mph wind, 15.00 ft mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 gcpl(+/-)=0.18

Wind reactions based on MMFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



≤ 3 -3-6 Over 3 Supports $\geq$
R=97 U=0 W=6"

PLT TYP. Wave

Design Crit: $TP1-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

~~7.36.00~~

QTY:1

Scale = .5"/ft.

WARNING: "PROTECT YOURSelves FROM THE RISK OF INJURY OR DEATH BY PROPERLY ATTACHED CHORD SHALL HAVE PROPERTY ATTACHED STRUCUTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED RIGID CEILING."

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT

TYPE: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & GRACING OF TRUSSES.

PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z CONNECTION PLATES NOT MORE THAN 20/10/1606 (M, 0/55/5) 451M 4053 GRADE 40/50 (M, 5/11/55) GALV. STEEL, APPLY

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/TP1.1 SEC. 2.



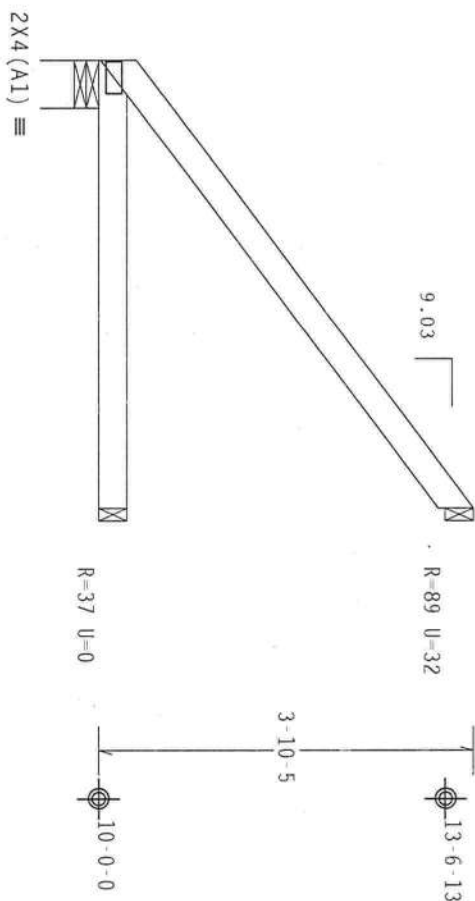
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

TC LL	20.0 PSF	REF	R8228- 71337
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255012
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	101563
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT I, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ Gcpl (+/-)=0.18

Wind reactions based on MMFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



← 4-7-6 Over 3 Supports →
R=135 U=0 W=6"

Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

7.36.00

QTY:1

FL/-/4/-/-/R/-/-

Scale = .5" / Ft.

WARNING—PRIESTS REQUIRE EVIDENT CASE IN FABRICATION...MADRID, SHIPPING, INSTALLING, AND BRACING REFER TO DESI (BIOLOGICAL COMPONENTS) (NUTRITION)...PUBLISHED BY TPI (THOMAS PASTOR INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND MICA (MORON TRAFFIC COUNCIL OF AMERICA), 6500 UNIVERSITY AVE., MADISON, WI 53719 FOR THESE PRACTICES, EACH SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED CHORD CELLING.

****IMPORTANT**** OBTAIN A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCO, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TUBS IN CONFORMANCE WITH THIS OR FABRICATING, MAINTAINING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DE SIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. (NDS) AND THE 1989 EDITION OF THE NATIONAL SPECIFICATION FOR STRUCTURAL STEEL (AISC 360-109). PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITIONED PER DRAWINGS 1600-2/01. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN A3 OF THE 2002 SPEC. 3. A SEAL ON THIS

DRAWING, INVENTIVE ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TROUS COMPONEN
DESIGN SHOULD. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE
BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

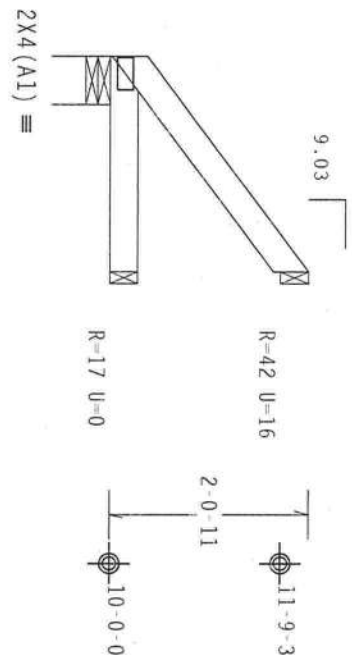
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FL/-4/-1-/R/-		Scale =.5"/Ft.	
TC LL	20.0 PSF	REF	R8228- 71338
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 082550
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	101567
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Trusses to be spaced at 16.0" OC maximum.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCPI(+/-)=0.18
Wind reactions based on MMFRS pressures.



2-2-11 Over 3 Supports
R-67 U-0 W-6"

PLT TYP. Wave
Design Crit: TPI-2002 (STD) /FBC
Cq/RT=1.00(1.25)/0(0)
7.36.00
QTY:1
FL/-/4/-/-/R/-
Scale =.5"/Ft.

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

FLORIDA
STATE OF
PROFESSIONAL ENGINEER
JAMES J. KROSS
No. 52212
Sep 12 2000

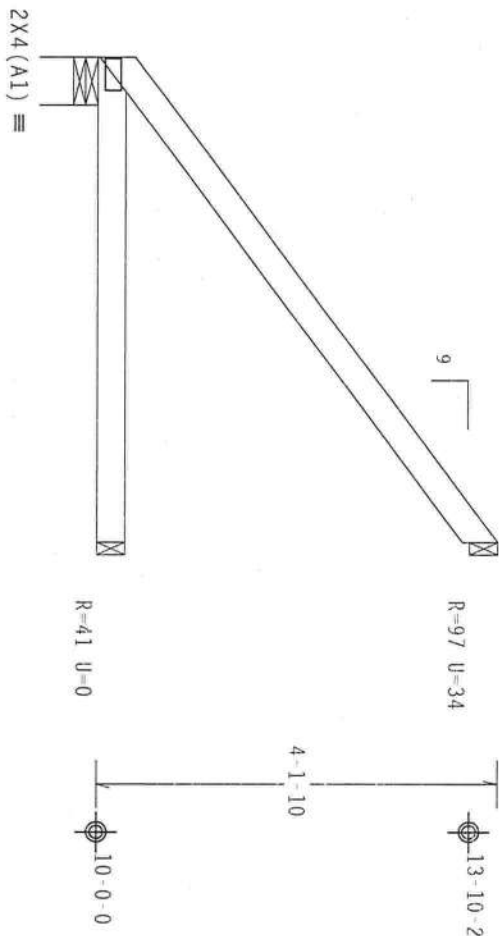
TC LL	20.0 PSF	REF R8228- 71339
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUSR8228 08255014
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEQN- 101581
DUR. FAC.	1.25	FROM AH
SPACING	16.0"	JREF- 1TKU8228Z01

THIS WORK PREPARED FROM COMPUTER INPUT (LOANS & DIMENSIONS) SUBMITTED BY RUSS MRK.

110 mph wind, 15.00 ft mean ht, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ (Cp1 (+/-))=0.18

Wind reactions based on MFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



5-0-0 Over 3 Supports $\rightarrow$
 $R=146$ $U=0$ $W=6"$

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$

7.36.00

QTY:1

FL/-/4/-/-/R/-/

Scale = .5" / Ft.

WARNING:—THIS IS A RIGID EXTRINSIC CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFERRED TO BC31 (BOLLING/CORNER EXTERIOR TIE INFORMATION). PUBLISHED BY THE (TROSS PACE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) FOR THE (TROSS PACE INSTITUTE OF AMERICA, 6,500 RIVERSIDE ENTERPRISE LANE, MADISON, MI, 48176) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CHORDING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM ACG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONDITIONS APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC., BY AIAA) AND TPI. CONNECTOR PLATES ARE MADE OF 2017/81666 (W./H./SS/K) ASTM A953 GRADE 40/60 (W./H./SS) GALV. STEEL. PLATES TO EACH FACE OF BRUSH AND

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE SEAL ON THIS PLATE FOLLOWED BY (1) SHALL BE PER AMERICAN ASSOCIATION OF PROFESSIONAL ENGINEERS (PEER) SECTION 3.3. UNLESS OTHERWISE LOCATED ON THIS SECTION, POSITION PER DRAWINGS TYPICAL TO EACH OF THOSE AND.

DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

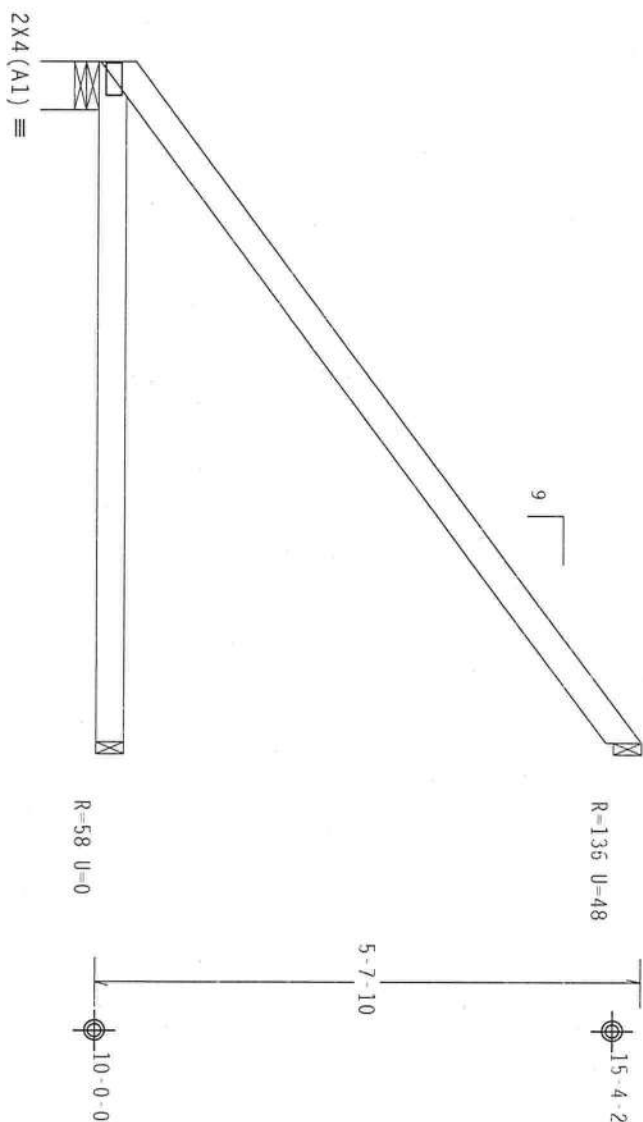
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FL/-/4/-/-R/-		Scale -.5"/ft.
TC LL	20.0 PSF	REF R8228- 71341
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCSRB228 0825015
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 101576
DUR.FAC.	1.25	FROM AH
SPACING	16.0"	JREF- 1TKU8228Z01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge. CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpl(1/4)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



7-0-0 Over 3 Supports $R=203$ $U=0$ $M=6"$

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$

7.36.00

Qty: 1

FL/-/4/-/-/R/-/

Scale = .5"/Ft.

*WARNING-TRUCKS, RECREATIONAL VEHICLES, AND BICYCLES SHOULD NOT BE LOADED ON THE TRAILERS OF THIS TRUCK. TO REDUCE THE RISK OF A FLIPPING COMPONENT SAFETY INFORMATION - PUBLISHED BY THE TRUSS PRACTICE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, AND AMERICAN TRUSS COMPANY OF AMERICA, 60000 INTERSTATE LAKE, MADISON, WI 53707 FOR SAFETY PRACTICES PLEASE REFER TO REFORMING THE STRUCTURES, OVERSEASER INDICATED FOR GROUND SHALL HAVE PROPERLY ATTACHED STRENGTHEN PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED BRIDG CILLING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND IBC. THE BRIDGE CONNECTOR PLATES ARE MADE OF 2018/16GA (W, M/S/S/K) ASTM A653 GRADE 40/50 (W, K/H, SS) GALV. STEEL. APPLY

PLATES TO EACH FACE OF CROSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER Annex A3 OF TPII-2002 SEC.3. A SEAL ON THIS

DRAWING INDICATE, ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENTS. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN.

BUILDING DESIGN PER ANSI/TPI 1 SEC. 2.

0472
 017
 JAMES F. JOHNSON
 No. 52242
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 JR.
 LIC. 52242
 Sep 1

TC LL	20.0 PSF	REF	R8228- 71342
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255046
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	101630
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKUR8228Z01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPi(1/-)-0.18

Wind reactions based on MWFRS pressures.

Shim all supports to solid bearing.


$$Cq/RT=1.00(1.25)/0(0)$$

QTY:1

Scale = .5"/Ft.

JAMES L. BELLS, JR.
N. E. 334
N. E. 334

[illegible]

STATE OF

NOV 19 1964

NGI

33

James L. Wells, Jr.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Exp. 12-31-08

TC LL	20.0 PSF	REF	R8228- 71343
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCSR8228 08255043
BC LL	0.0 PSF	HG-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEGN-	101633
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	UREF-	1TKU8228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

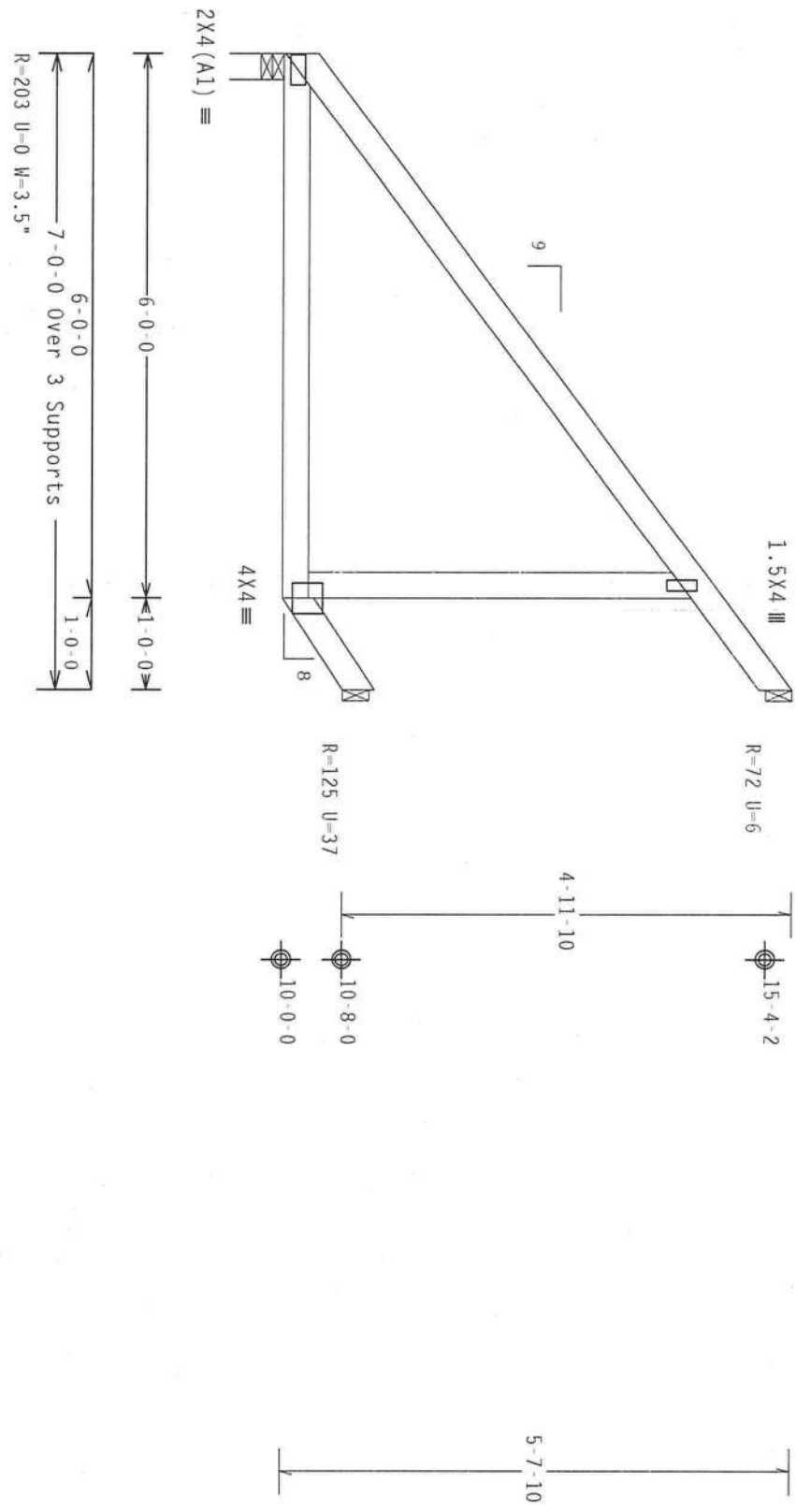
Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

Shim all supports to solid bearing.



PLT TYP. Wave

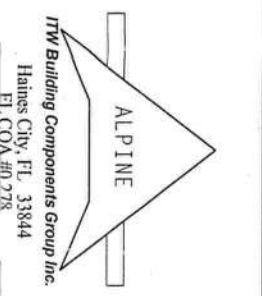
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.00.042

QTY: 1

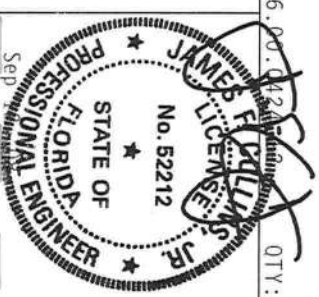
FL/-/4/-/-/R/-

Scale =.5"/Ft.



****WARNING**** TRUSSES REQUIRING EXTERIOR GABLE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MOULSON, MI 49159) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN COMPANIES WITH APPLICABLE PROVISIONS OF 2003 INTERNATIONAL DESIGN SPEC. BY AIA/PAI AND TPI. DESIGN COMPANIES WITH APPLICABLE PROVISIONS OF 2003 INTERNATIONAL DESIGN SPEC. BY AIA/PAI AND TPI. DESIGN COMPANIES WITH APPLICABLE PROVISIONS OF 2003 INTERNATIONAL DESIGN SPEC. BY AIA/PAI AND TPI. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNE AS OF TPI-2002 SECS. A SEAL ON THIS BRACING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOCIETY FOR THE TRUSS COMPONENT DESIGN GROUP. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71344
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCU8R8228 08255049
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT.LD.	40.0 PSF	SEQN-	101636
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

THIS WORK PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY IKUSS MFK.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 Gcpi(+/-)=0.18

Wind reactions based on MMFRS pressures.


$$Cq/RT=1.00(1.25)/0(0)$$

QTY:1

Scale = .5" / Ft.

****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW AGG, INC. SHALL NOT

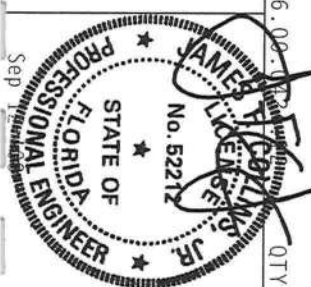
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ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278



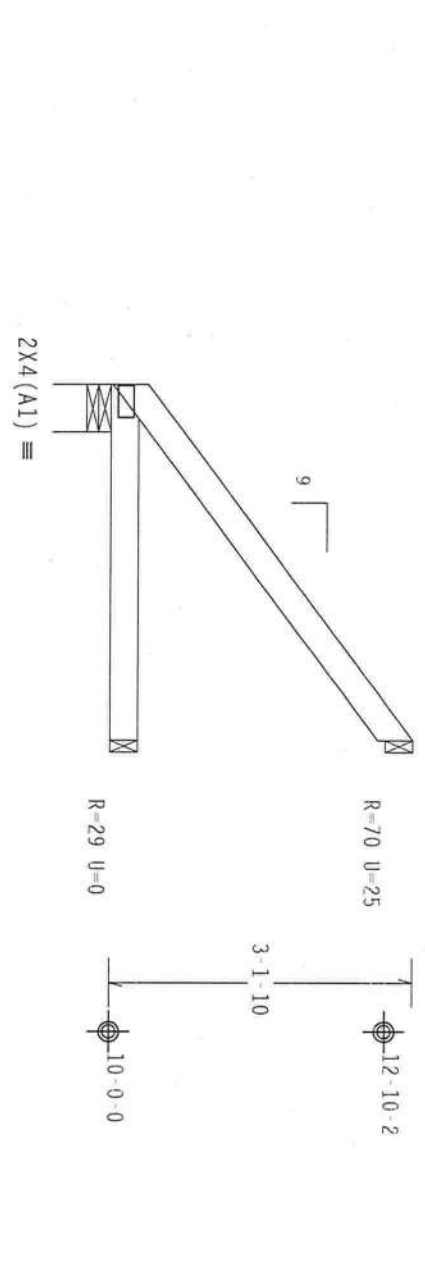
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TC LL	20.0 PSF	REF	R8228- 71346
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255011
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	101642
DUR.FAC.	1.25		
SPACING	16.0"	UREF-	1TKU8228Z01

JREF- 1TKU8228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Trusses to be spaced at 16.0" OC maximum.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18
Wind reactions based on MMFRS pressures.



PLT TYP. Wave
Design Crit: TP1-2002 (STD) /FBC
Cq/RT=1.00(1.25)/0(0)
7.36.00

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

PLT TYP. Wave

Design Crit: TP1-2002 (STD) /FBC

Cq/RT=1.00(1.25)/0(0)

7.36.00

QTY: 1

FL/-/4/-/-/R/-

Scale = .5"/Ft.

JAMES F. MULLINS

PROFESSIONAL ENGINEER

STATE OF FLORIDA

No. 52212

SEP 11 2008

TC LL

20.0 PSF

REF R8228- 71347

TC DL

10.0 PSF

DATE 09/11/08

BC DL

10.0 PSF

DRW HCURSR8228 08255022

BC LL

0.0 PSF

HC-ENG DF/DF

TOT. LD.

40.0 PSF

SEON- 101645

DUR. FAC.

1.25

SPACING

16.0"

JREF- 1TKU8228201

110 mph wind, 15.00 ft mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCP (+/-) = -0.18

Wind reactions based on MMFRS pressures.

Hipjack supports 7-0-0 setback jacks with no webs.

Shim all supports to solid bearing.



NO. 52212

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STATE OF

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CONFIDENTIAL

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TC LL	20.0 PSF	REF	R8228- 71348
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCSR8228 08255037
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	40457
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	REF-	1TK8228201

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCPI (+/-) -0.18

Mind reactions based on MMFRS pressures.

Roof overhang supports 2.00 psf soffit load.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

 $Cq/RT=1.00(1.25)/0(0)$ ~~QTY:1~~

Scale = .5"/Ft.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITG BCG INC SHALL NOT

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AISC) AND IP1. 1TW BCC CONNECTION PLATES ARE MADE OF 3016/16GA. ALL WELDS TO BE MADE TO THE FOLLOWING SPECIFICATIONS:

A SEAL ON THIS

100

Sep

SPACING 16.0"

JREF - 1TKU8228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #2 Dense

Left end vertical exposed to wind pressure. Deflection meets L/240
criteria for brittle and flexible wall coverings.

Calculated horizontal deflection is 0.20" due to live load and 0.09"
due to dead load.

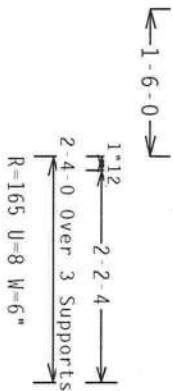
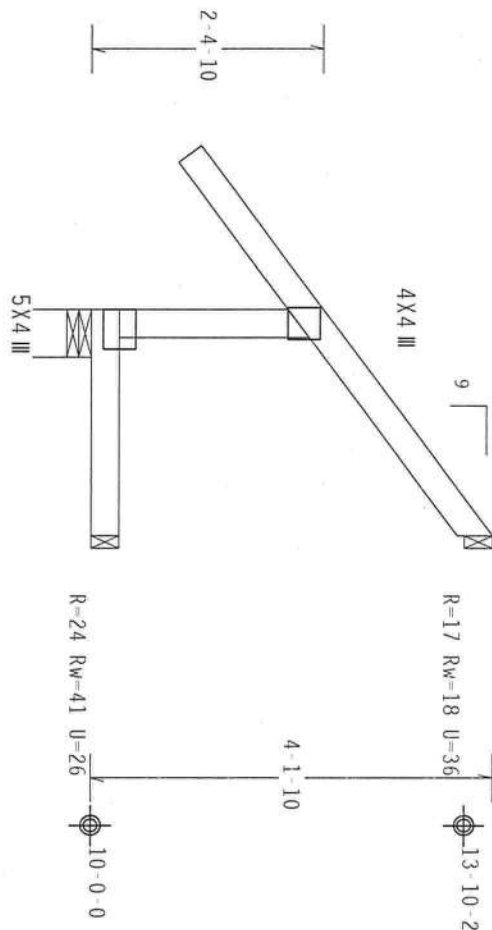
Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf. $I_w=1.00$ $G_{CPI}(+/)=0.18$

Wind reactions based on MMFRS pressures.

Roof overhang supports 2.00 psf soffit load.

Trusses to be spaced at 16.0" OC maximum.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36

QTY:1

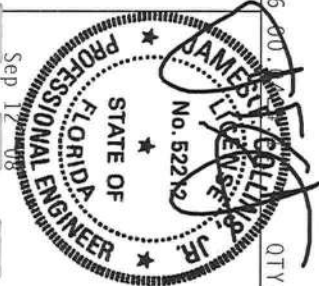
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Scale =.5"/Ft.

WARNING PRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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NTW Building Components Group Inc.
Haines City, FL 33844
FL COA-460-278



TC LL	20.0 PSF	REF R8228- 71351
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUR8228 08255039
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEON- 101658
DUR.FAC.	1.25	*
SPACING	16.0"	

UREF- 1TKU8228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

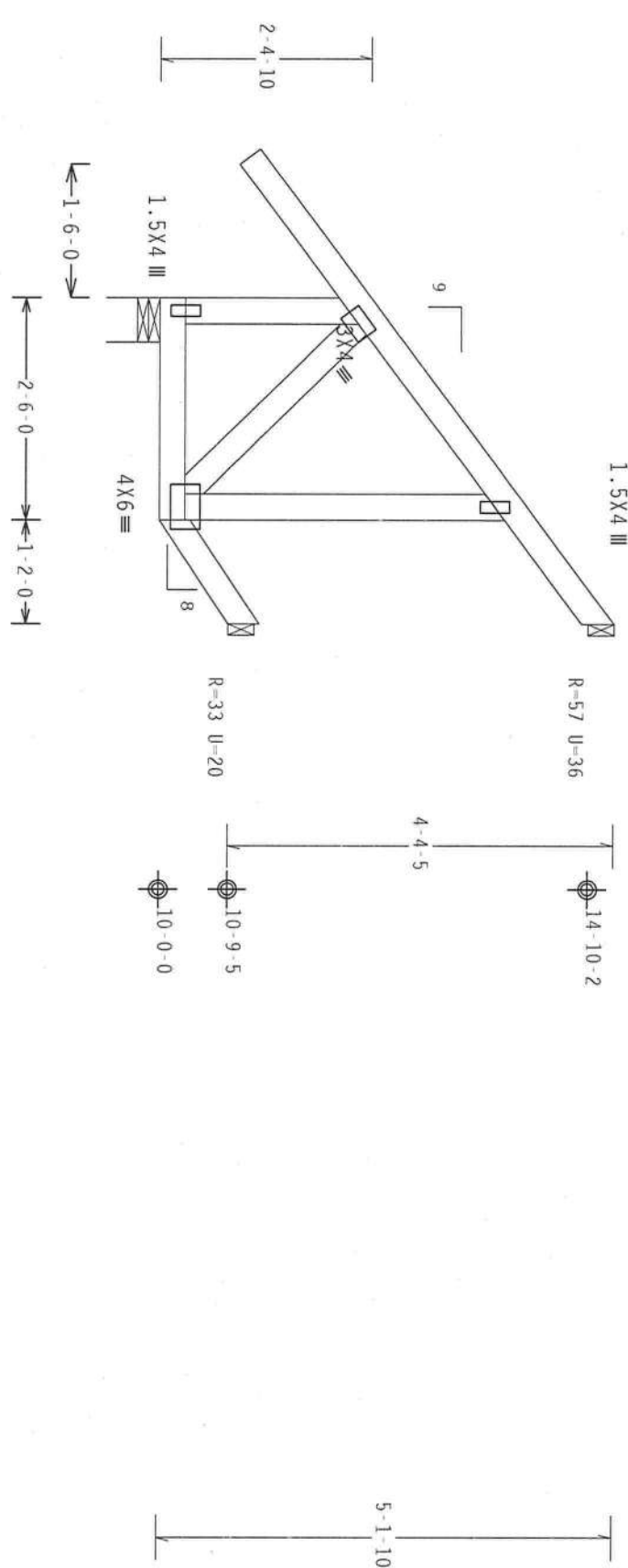
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Trusses to be spaced at 16.0" OC maximum.

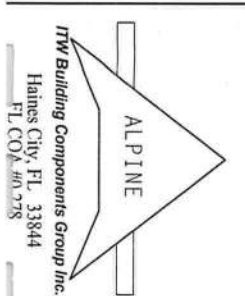
Shim all supports to solid bearing.

Wind reactions based on MMFRS pressures.
Roof overhang supports 2.00 psf soffit load.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



2-6-0
1-2-0
3-8-0 Over 3 Supports
R=195 U=0 W=6"

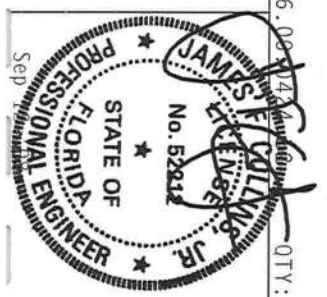
PLT TYP. Wave
Design Crit: TPI-2002 (STD) /FBC
Cq/RT=1.00(1.25)/0(0)
7.36.00



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THESE INSTRUCTIONS, INCLUDING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



TC LL	20.0 PSF	REF R8228- 71352
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUSR8228 08255038
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEON- 101662
DUR. FAC.	1.25	
SPACING	16.0"	

Scale = .5"/Ft.

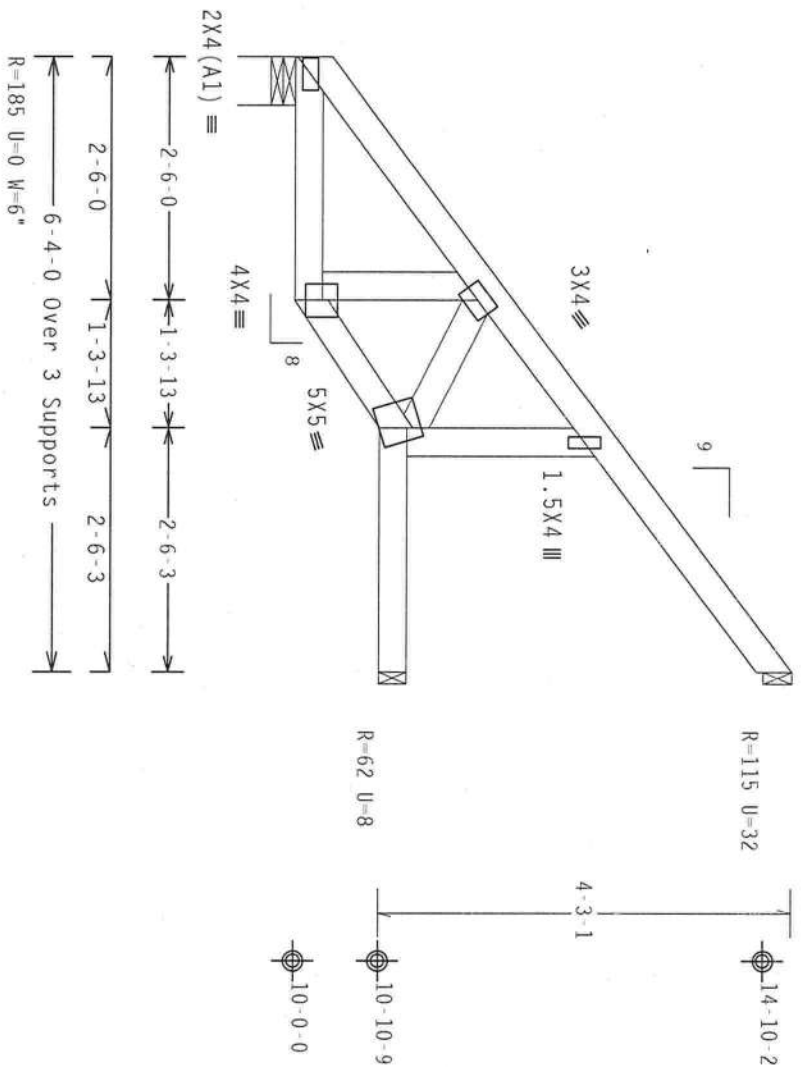
JREF - 1TKU8228201

THIS WORK PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY IRVING MFK.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

Wind reactions based on MFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/$

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QTY:1

FL/-/4/-/-/R/-

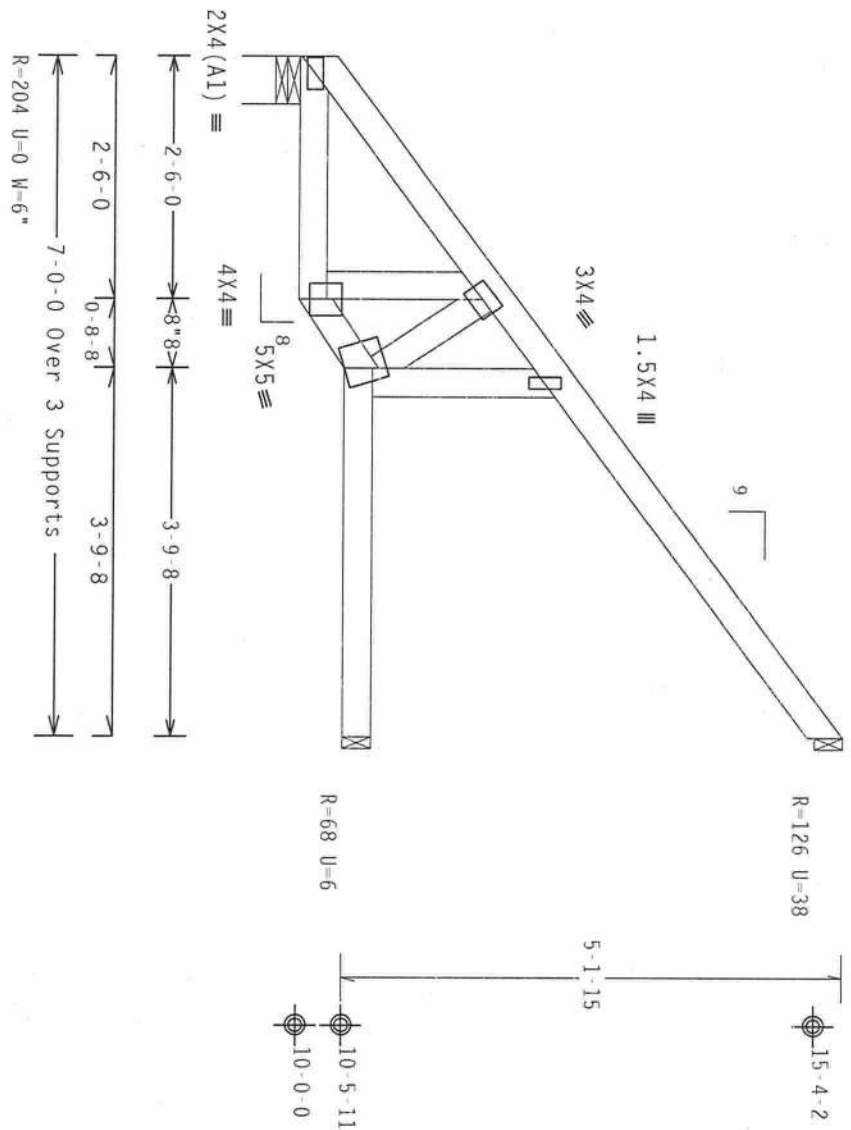
Scale = .5"/Ft.

TC LL	20.0 PSF	REF	R8228- 71353
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCU8R8228 08255010
BC LL	0.0 PSF	HC-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEQN-	101666
DUR.FAC.	1.25		
SPACING	16.0"	JREF -	1TKU8228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ $gC_{pi}(+/-)=0.18$
Wind reactions based on MMFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002 (STD) /FBC
Cq/RT=1.00(1.25)/0(0)

7.36 00 104

QTY:1

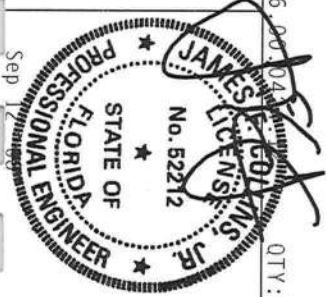
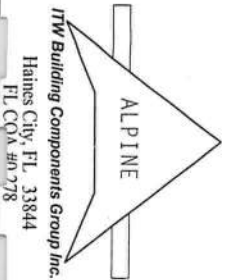
FL/-/4/-/-/R/-

Scale = .5" / Ft.

WARNING TRUSSES ROUTINE EXTREME CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, PROSPECT, MI 48375) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OR THE INSTALLATION OF THIS TRUSS. THE TRUSS IS TO BE CONFORMANT WITH TPI: OR FABRICATING AND HANDLING REQUIREMENTS. UNLESS OTHERWISE INDICATED, THE TRUSS IS TO BE CONFORMANT WITH DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF 2010/1604 (A-H/S/S/Z) ASH A53 GRADE 40/50 (A, K/H/A53) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNE AS OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71354
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255047
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT. LD.	40.0 PSF	SEON-	101670
DUR. FAC.	1.25		
SPACING	16.0"		

JREF- 1TKU8228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

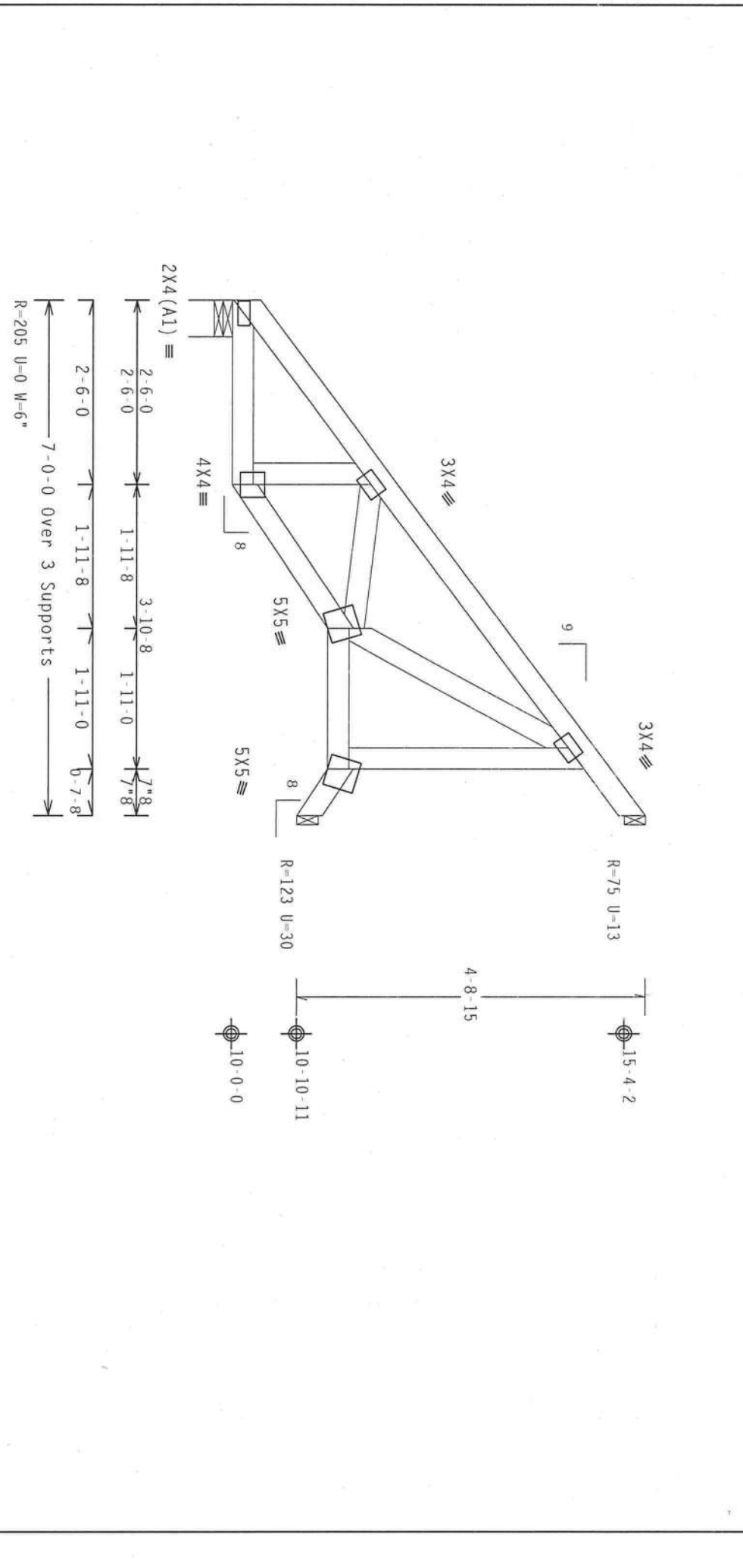
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, IW=1.00 GCPI(+/-)-0.18

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Wind reactions based on MMFRS pressures.

Shim all supports to solid bearing.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0.00

7.36.00

QTY: 1

FL/-/4/-/-/R/-

Scale = .5" / Ft.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL CO# 40 728

STATE OF FLORIDA
PROFESSIONAL ENGINEER
JAMES E. JONES
No. 52212

SPACING	16.0"	JREF- 1TKU8228Z01
TC LL	20.0 PSF	REF R8228- 71355
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUS88228 08255044
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEQN- 101673
DUR. FAC.	1.25	*

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi (+)=0.18

Wind reactions based on MMFRS pressures.

Shim all supports to solid bearing.

 $Cq/RT=1.00(1.25)/0(0)$

QTY:1

Scale = .5"/Ft.

JAMES A. JOELING, JR.
 LICENSE
 No. 52212

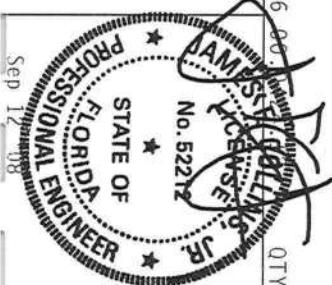
☆

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD TO TP1; OR FABRICATING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0378



Sep 12 '08

JREF- 1TKU8228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

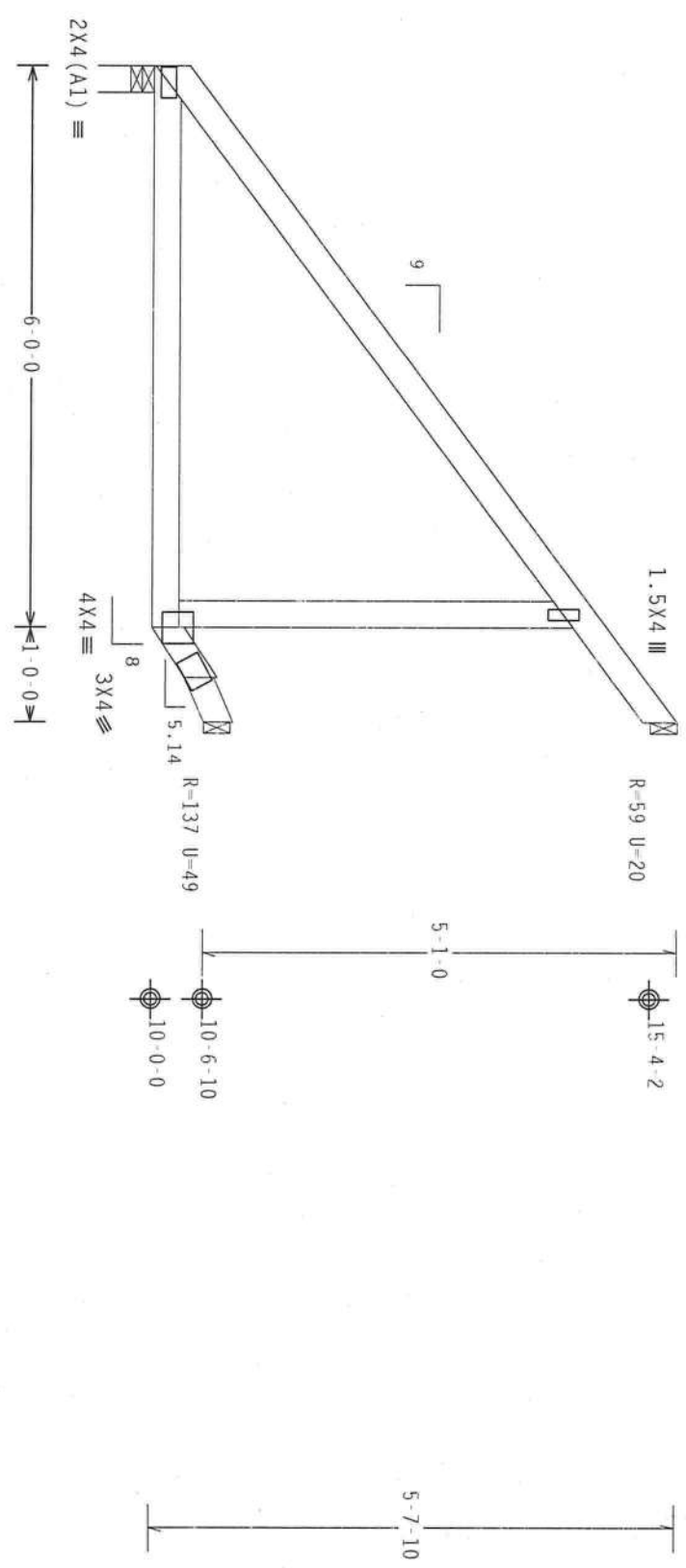
Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. b1dg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCPI(+/-)-0.55

Wind reactions based on MMFRS pressures.

Shim all supports to solid bearing.

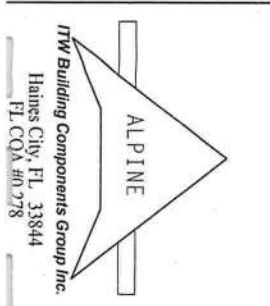


PLT TYP. Wave

Design Crit: TPI-2002 (STD) /FBC
Cq/RT=1.00(1.25)/0(0)

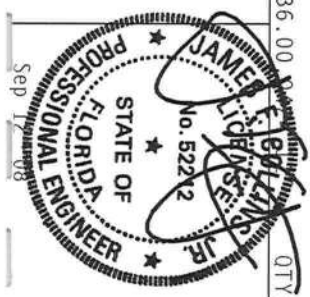
7.36.00 QTY:1

FL/-/4/-/-/R/- Scale =.5"/Ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC&I (BUILDING COMPONENTS INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THE SE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING DURING OR AFTER INSTALLATION. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING DURING OR AFTER INSTALLATION. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING DURING OR AFTER INSTALLATION.



TC LL	20.0 PSF	REF	R8228-71357
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255036
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEON-	102286
DUR. FAC.	1.25	FROM	AH
SPACING	16.0"	UREF-	1TKU8228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

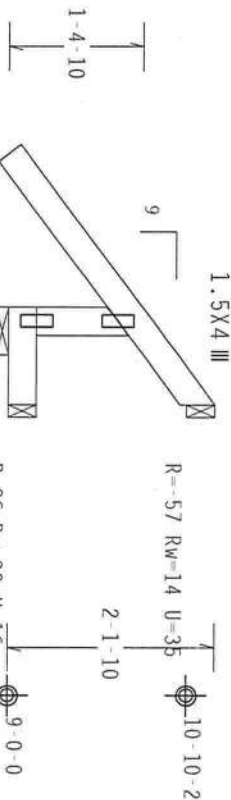
Roof overhang supports 2.00 psf soffit load.

Trusses to be spaced at 16.0" OC maximum.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf. Iw=1.00 GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



1-6-0
1-0-0 Over 3 Supports
R=161 U=7 W=6"

PLT TYP. Wave

Design Crit: TPI-2002 (STD) /FBC
Cq/RT=1.00(1.25)/0(0)

7.36.00

QTY:1

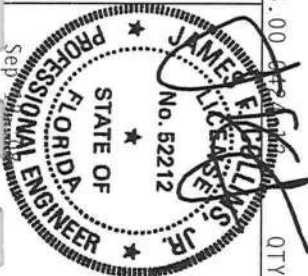
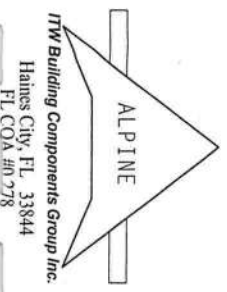
FL/-/4/-/-/R/-

Scale =.5"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORDS, ALL TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE RCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE RCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

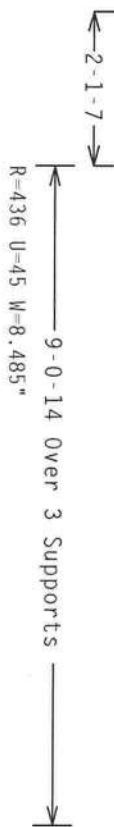


TC LL	20.0 PSF	REF R8228- 71358
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUR8228 08255021
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEON- 104064
DUR. FAC.	1.25	FROM AH
SPACING	16.0"	DATE- 1TKU8228201

110 mph wind, 15.00 ft mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MWFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



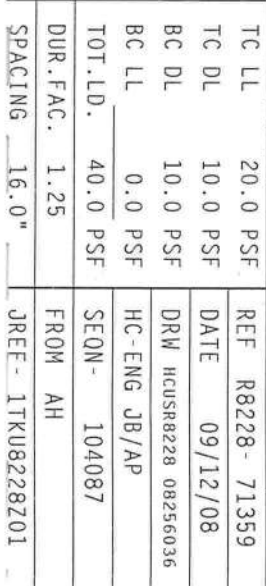
Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

FL/-/4/-/-/R/-/

Scale = .375" / Ft.

1. The first step in the process is to identify the problem or issue that needs to be addressed. This involves gathering information and understanding the context of the problem.

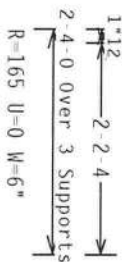
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110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCp(+)=-0.18

Wind reactions based on MMFRS pressures.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

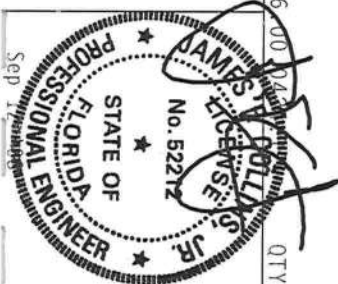


Scale = .5"/Ft.



ALPINE

Haines City, FL 33844
FL COA #0078



TC LL	20.0 PSF	REF	R8228- 71360
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255002
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT.LD.	40.0 PSF	SEQN-	104069
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

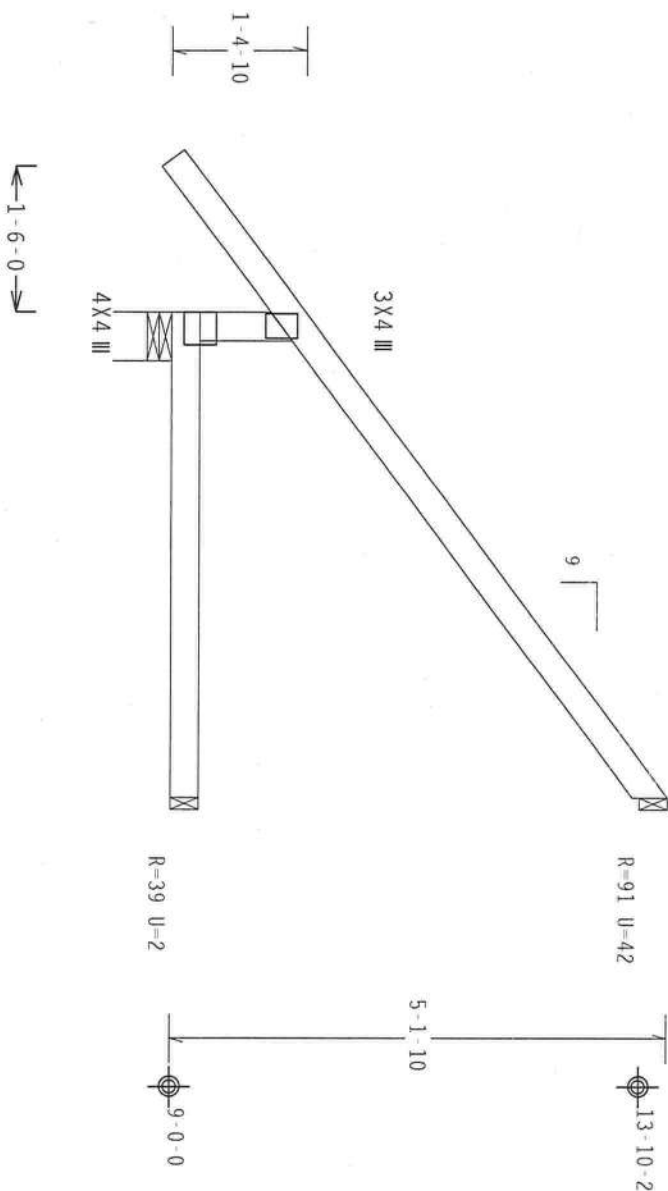
Roof overhang supports 2.00 psf soffit load.

Trusses to be spaced at 16.0" OC maximum.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCPI(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.00

QTY: 1

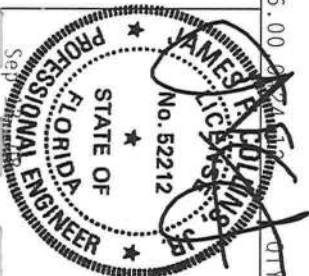
FL/-/4/-/-/R/-

Scale = .5"/Ft.

WARNING TRUSSES BEING EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER ATTACHMENT OF THE TRUSS TO THE BUILDING. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER ATTACHMENT OF THE TRUSS TO THE BUILDING. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER ATTACHMENT OF THE TRUSS TO THE BUILDING.

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA-560778



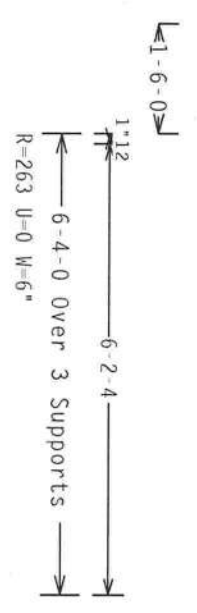
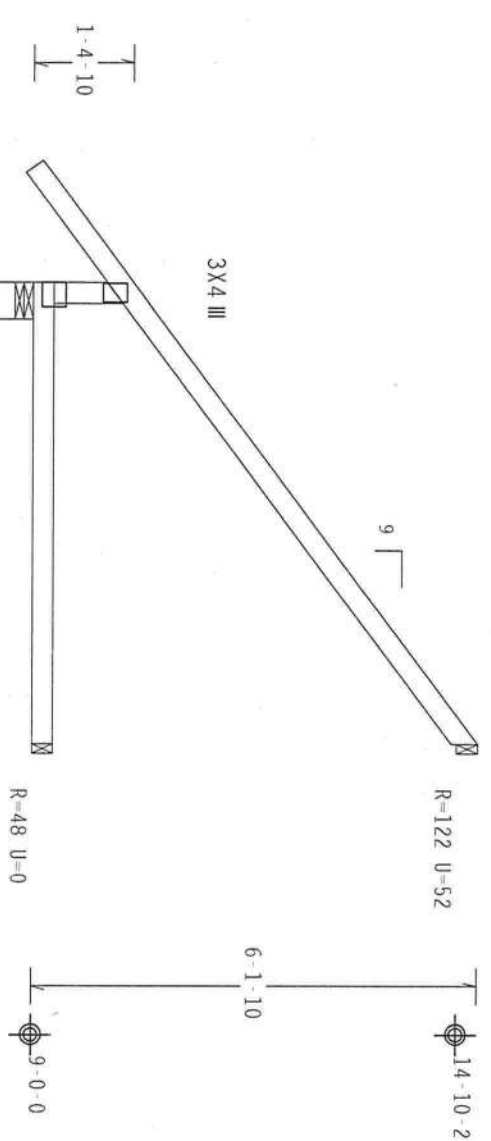
TC LL	20.0 PSF	REF	R8228- 71362
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255004
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT.LD.	40.0 PSF	SEQN-	104077
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

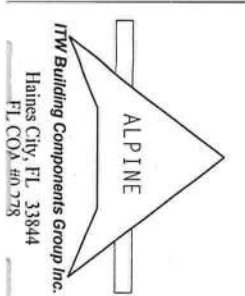
Roof overhang supports 2.00 psf soffit load.

Trusses to be spaced at 16.0" OC maximum.

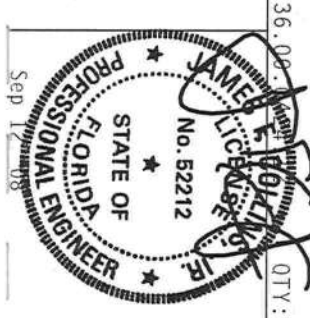
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCPI(+/-)=0.18
Wind reactions based on MMFRS pressures.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA 40078



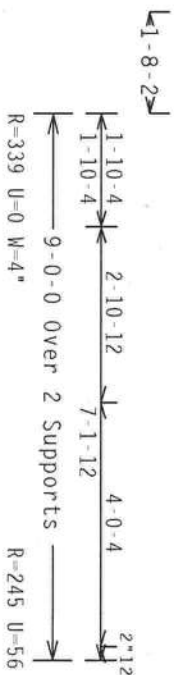
TC LL	20.0 PSF	REF	R8228- 71363
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCSR8228 08255005
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON-	104081
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)
7.36.00
QTY:1
FL/-/4/-/-/R/-
Scale =.375"/Ft.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.



Scale = .3125"/Ft.

A circular library stamp from James H. Mulvaney, Jr. The text "JAMES H. MULVANEY, JR." is arranged in a circle. In the center, the number "52217" is handwritten. The stamp is partially obscured by the binding of the book.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH TP1; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TROSSES.

DESIGN CONDITIONS FOR APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND THE
THE BE
CONNECTION PLATES ARE MADE OF 20/18/1666 (W, M/SS/K) ASTM A653 GRADE 40/60 (Y, K/H, SS) GALV. STEEL.
APPLY
TO EACH EXIST. MEMBER AND
MEMBERS ATTACHED TO THE SECTION
POSITIONS ARE INDICATED BY A

PLATES TO EACH TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER ORIGINATOR'S DRAWING INDICATES. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS CORPUS OF THE BRIDGE SHALL BE PER ANNEX A.3 OF TP11-2002, SEC.3.3. A SEAL ON THIS ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A.3 OF TP11-2002, SEC.3.3.

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/AP1 1 SEC. 2.

100

TC LL	20.0 PSF	REF	R8228- 71364
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255006
BC LL	0.0 PSF	HC-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEQN-	104214
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

110 mph wind, 19.23 ft mean hgt, ASCE 7-02, PART. ENC. bldg, located anywhere in roof; CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.55

Wind reactions based on MWFRS pressures.

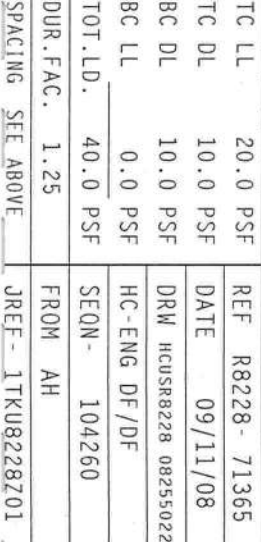
Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



Scale = .5"/Ft.

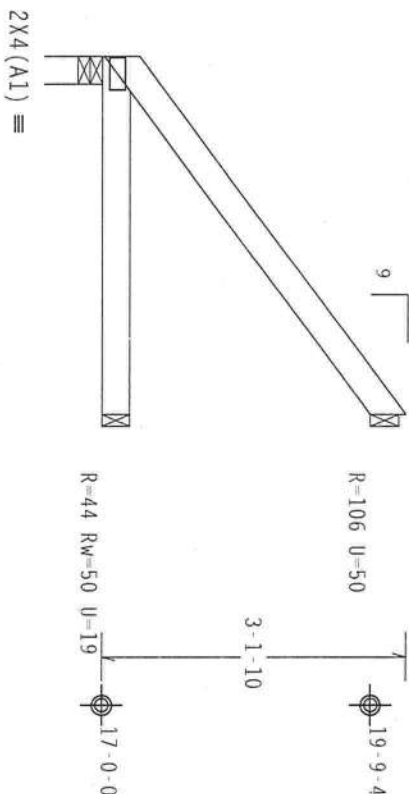
TC LL	20.0 PSF	REF R8228- 71365
TC DL	10.0 PSF	DATE 09/11/08

Haines City, FL 33844
FL COA #0778



110 mph wind, 18.76 ft mean hgt, ASCE 7-02, PART 5, bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind reactions based on MMFRS pressures.



3-8-0 Over 3 Supports $\Rightarrow$
 $R=162$ $U=9$ $W=3.5"$

Design Crit: TPI-2002(STD)
Cq/RT=1.00(1

$$Cq/RT=1.00(1.25)/0(0)$$

7.36.00

QTY:1

FL/-/4/-/-/R/-/

Scale = .5"/Ft.

"MARINERS" ARE THE RIGID EXTERIOR CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING TO BEST PROTECT THE UNDERLYING CONCRETE STRUCTURE. PUBLISHED BY THE TROSS PAPER INSTITUTE, 219 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA., 22314; (800) 678-6000. TRUSS COUNCIL OF AMERICA, INTERESTER LANE, MADISON, WI, 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. INTERESTED INDIVIDUALS FOR CHORD SHALL HAVE PROPERLY ATTACHED STRIBUTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED BLOOD CHALL.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONDITIONS IN APPLICABLE PROVISIONS OF MSD (NATIONAL DESIGN SPEC., BY AIAA) AND TP). CONNECTOR PLATES ARE MADE OF 20/18/1666 (H/M/SS/X) ASTM A653 GRADE 40/60 (U, K/H, SS) GALV. STEEL. PLATES TO EACH EACH OR DRILLS AND UNLESS OTHERWISE SPECIFIED, ALL DRILLING AND REPAIRING SHALL BE IN ACCORDANCE WITH THE

PLATES TO EACH OF 10033 AND, ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX AS OF TPI-2002 SEC.3, DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENTS. A SEAL ON THIS

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

I I I I I I

[illegible]

FL/-/4/-/-/R/-		Scale = .5"/ft.
TC LL	20.0 PSF	REF R8228- 71366
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUSR8228 08255007
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 104269
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TKU8228Z01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GcP1(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



R=533 U=57

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/0(0)$$

7.36.04

QTY:1

FL/-/4/-/-/R/-/

Scale = .5"/Ft.

WARNING: PRIORS RIGID EXTERIOR CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DC-1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATING INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND NIA (WOOD TRUSS COUNCIL OF AMERICA), 6500 MIDWAY ENTERPRISE LANE, MOULTON, WI 53179 FOR SAFETY PRACTICES AND POINTS TO REMEMBER THESE FUNCTIONS, UNDESIRABLE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED MIDRIDGE CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TYPE OR FABRICATING, HANDLING, SHIPPING, INSTALLING OR BRACING OF TRUSSES.

DESIGN CONDITIONS FOR APPLICATION OF NON-ALTERNATE DESIGN SPEC. (BY AIRMA) AND TPI. THE REQUIRED INFORMATION TO BE SUBMITTED BY THE BUILDING DESIGNER SHALL INCLUDE:

CONCRETE PLATES ARE NOTED ON 20/18/16GA (W. 9.0/55/LS) ASYM AGES 16A0E 00/60 (W. 8.0/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1606-2.

AIR INSULATION OF PLATES FOLLOWS BY (1) SHALL BE PER ANNEX A.3 OF TPI-2010 SEC.3.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SIGNED FOR THE TRUSS CORPORATION DESIGN SHOW.

THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AS/NZS/P1 1 SEC. 2.

TC LL	20.0 PSF	REF	R8228- 71367
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255023
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	104255
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1TKU8228Z01

Top Chord 2x4 SP #2
Bot Chord 2x4 SP #2
Webs 2x4 SP #3

(**) 6 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 24.14 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-2.0 psf. lw=1.00 GCFI(+/-)-0.18

Wind reactions based on MMFRS pressures.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

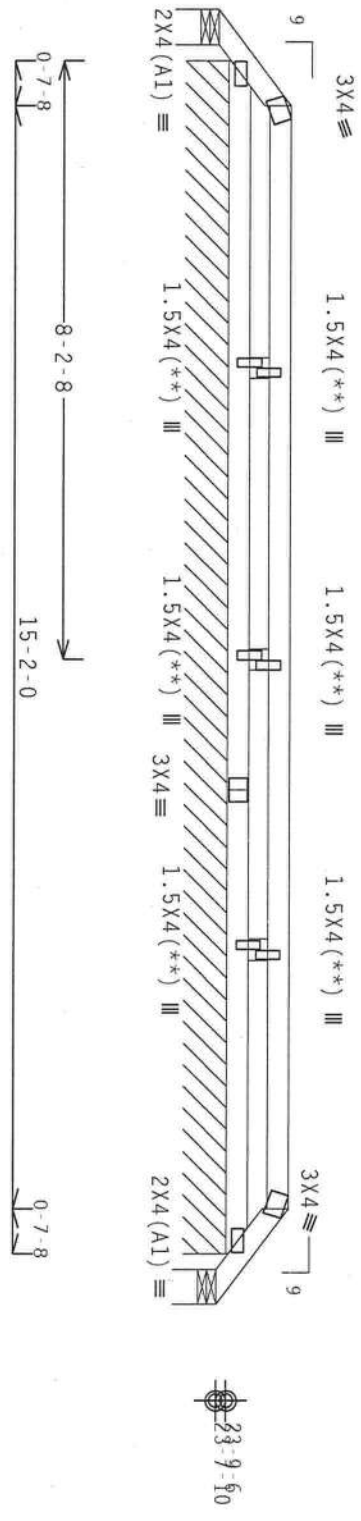
SPECIAL LOADS

TC - From	43 PLF at 0.00 to 43 PLF at 1.33
TC - From	43 PLF at 1.33 to 43 PLF at 16.50
TC - From	43 PLF at 16.50 to 43 PLF at 17.83
BC - From	3 PLF at 0.00 to 3 PLF at 17.83

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=14 Rw=12 U=10 W=5.833"
R=50 PLF U=18 PLF W=16-5-0

R=14 Rw=9 U=5 W=5.833"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.37.00

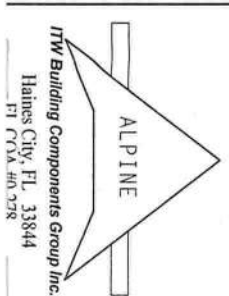
QTY:1 FL/-/4/-/-/R/-

Scale = .375"/ft.

****WARNING**** BRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DISCUSS CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY NDS) AND TPI. ITW BCG CONDUCTS EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1606-2. ALL TRUSSES SHALL BE MANUFACTURED IN ACCORDANCE WITH THE TRUSS MANUFACTURERS ASSOCIATION (TMA) DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228-71368
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUSR8228 08256038
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT. LD.	40.0 PSF	SEON-	14567 REV
DUR. FAC.	1.25	FROM	AH
SPACING	16.0"	JRFF-	1TKU8228201

110 mph wind, 24.64 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

Trusses to be spaced at 16.0" OC maximum.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

	(LUMBER	DUR.FAC.=-1.25	/	PLATE	DUR.FAC.=-1.25)
TC - From	43 PLF at	0.00	to	43 PLF at	2.67
TC - From	43 PLF at	2.67	to	43 PLF at	15.17
TC - From	43 PLF at	15.17	to	43 PLF at	17.83
BC - From	3 PLF at	0.00	to	3 PLF at	17.83

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Design Crit: $TPI-2002(STD)/FBC$
$$Cq/RT=1.00(1.25)/0(0)$$

~~7.36.00.043~~

QTY:1

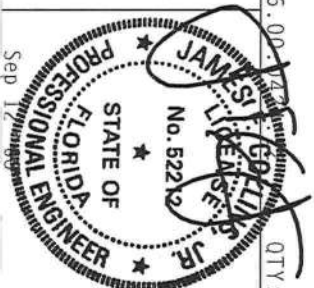
FL/-/4/-/-/R/-/

Scale = .375"/Ft.

WARNING: THESE FIBRE REINFORCED EPOXY GRESIN FABRICATIONS, MANUFACTURING, SHIPMENT, INSTALLATION, AND MAINTENANCE MUST BE DONE IN ACCORDANCE WITH THE FOLLOWING PREPARED SAFETY INFORMATION. PUBLISHED BY THE FIBRE GRESIN INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND VICA (VIRGINIA COMPOSITE INDUSTRIES) OF AMERICA, 65000 15TH AVENUE, MADISON, WI 53719 FOR SAFETY PRECAUTIONS, PRIOR TO READING THESE FIBRE GRESIN, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: BRUSH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE CODE OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONDITIONS ARE APPLICABLE REQUIREMENTS OR NON-APPLICABLE DESIGN SPEC., OF A/R/S AND T/F. THE REASON FOR THIS IS THAT THE CONDUCTOR PLATES ARE MADE OF 20/18/766A (4H/55P) ASTM A663 GRADE 40/60 (4H/35) GALV. STEEL. APPLY PLATES TO EACH FACE OF THUSM AND, NOTWITHSTANDING LOCATED ON POSITIONING PER DRAWINGS 1660-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AREA AS OF T/P1-2002 SEC.3. A SEAL ON THIS DRAINING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TOWNS COMPONENT/RELIABILITY DESIGN SHOW. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AHS/T/P1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71369
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255024
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SECN-	104647
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01

Top Chord 2x4 SP #2 Dense
Bot Chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 25.14 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=-0.18$

Wind reactions based on MMFRS pressures.

Trusses to be spaced at 16.0" OC maximum.

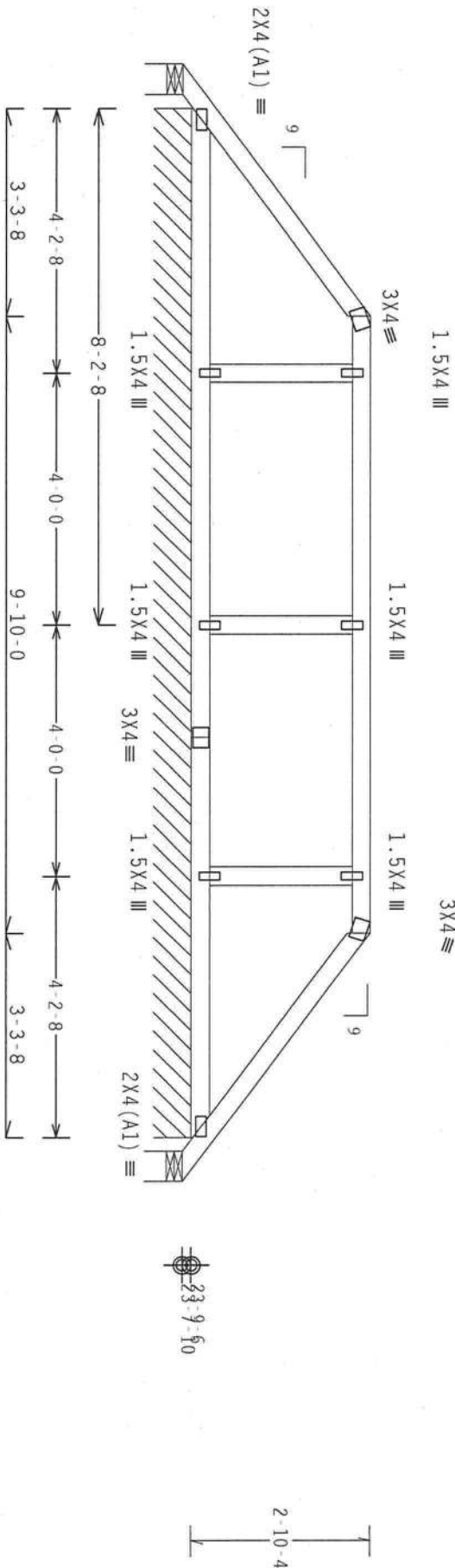
Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 43 PLF at 0.00 to 43 PLF at 4.00
TC - From 43 PLF at 4.00 to 43 PLF at 13.83
TC - From 43 PLF at 13.83 to 43 PLF at 17.83
BC - From 3 PLF at 0.00 to 3 PLF at 17.83

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crtt: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.00

QTY: 1

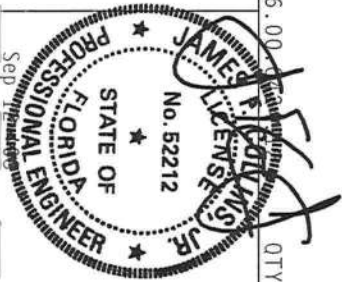
FL/-/4/-/-/R/-

Scale = .375"/Ft.

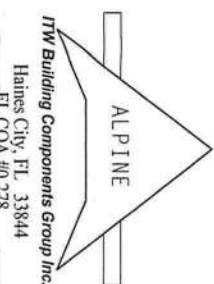
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TIV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ALSEA) AND TPI. TIV BCG CONDUCTS PLATES AND JOINTS OF 20/10/10/10 (W/S/S/S) ASH LAGS (GROUT 50/60 OR 80/100) GARY, STEEL, APPLY REINFORCING BARS TO JOINTS. TIV BCG SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN SHOP. THE STABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71370
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255025
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	104638
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	UREF-	1TKU8228Z01



NTW Building Components Group Inc.
Haimes City, FL 33844
Fl COA #0 378

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC From 43 PLF at 0.00 to 43 PLF at 5.33

TC - From	43 PLF at 12.50 to	43 PLF at 17.83
BC - From	3 PLF at 0.00 to	3 PLF at 17.83

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



JAMES
L. LEASE, JR.
No. 52212

Scale = .375"/Ft.
REF R8228- 7137
DATE 09/12/08

DATE	09/12/08
DDRW	HCSR8228 082566
HHC-ENG	JB/AP
SEQN-	14574
FROM	AH
REF-	1TKU8228Z0

Top Chord 2x4 SP #2
Bot Chord 2x4 SP #2
Webs 2x4 SP #3

110 mph wind, 26.14 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. 1w=1.00 GCPI(+/-)=0.18

Wind reactions based on MWFRS pressures.

Trusses to be spaced at 16.0" OC maximum.

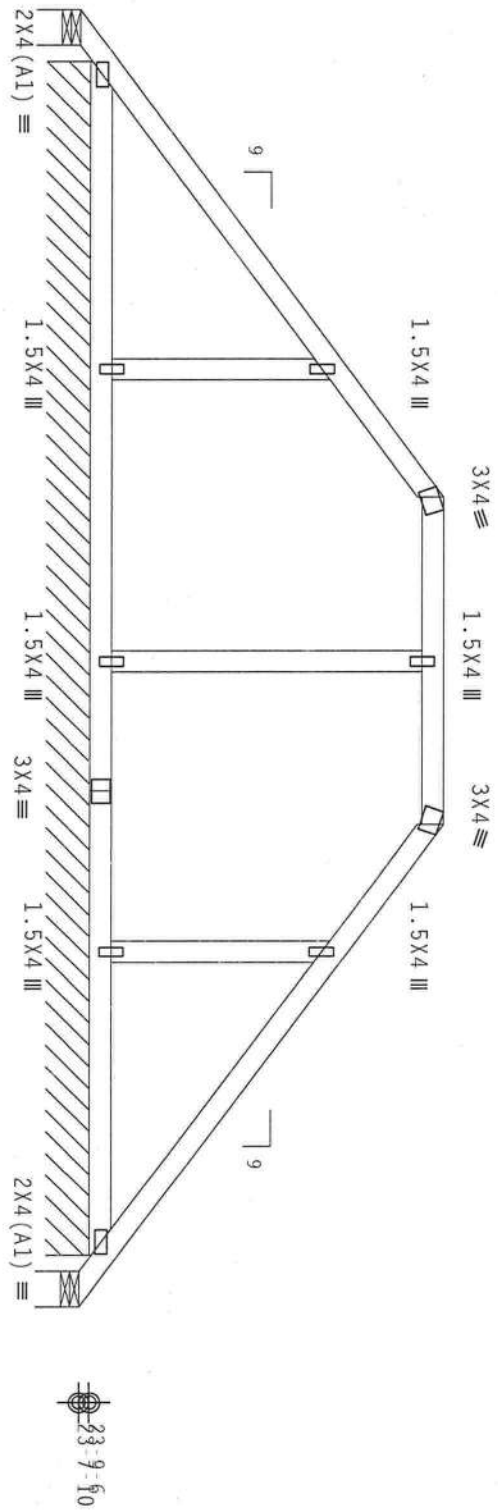
Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

TC - From	43 PLF at 0.00 to	43 PLF at 6.67
TC - From	43 PLF at 6.67 to	43 PLF at 11.17
TC - From	43 PLF at 11.17 to	43 PLF at 17.83
BC - From	3 PLF at 0.00 to	3 PLF at 17.83

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



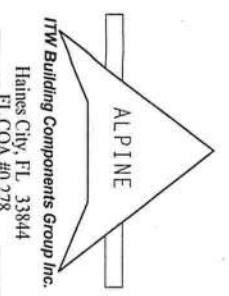
R=-44 Rw=75 U=79 W=5.833"
R=54 PLF U=26 PLF W=16-5-0
R=-44 Rw=55 U=33 W=5.833"

PLT TYP. Wave

Design Crtt: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.37.00.152

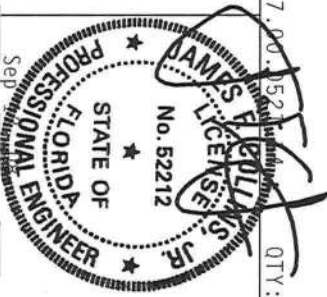
Scale = .375"/Ft.



WARNING TRUSSES BEING THE EXISTING CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING, AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OF FABRICATING, HANDLING, SHIPPING, INSTALLING, AND BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND TPI. THE BCG PLATES TO EACH OF THESE TRUSSES ARE DESIGNED TO BE USED IN CONFORMANCE WITH THE AISC AND TPI SPECIFICATIONS. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED BY A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



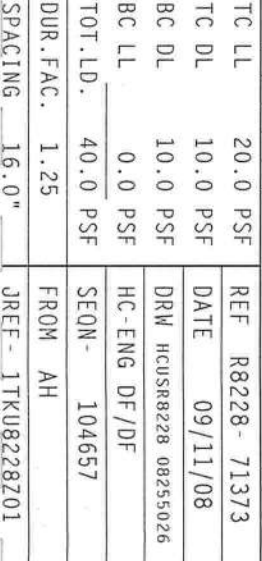
FL/-/4/-/-/R/-	OTY:1
TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	16.0"

REF R8228- 71372	DATE 09/12/08
DRW HCUR8228 08256041	HC-ENG JB/AP
SEQN- 14578	REV
FROM AH	
UREF- 1TKU8228Z01	

SPECIAL LOADS
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 43 PLF at 0.00 to 43 PLF at 8.00
TC - From 43 PLF at 8.00 to 43 PLF at 9.83
TC - From 43 PLF at 9.83 to 43 PLF at 17.83
BC - From 3 PLF at 0.00 to 3 PLF at 17.83

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

110 mph wind, 26.98 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, $I_w=1.00$ $GCFI(+/-)=0.18$

Wind reactions based on MMFRS pressures.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

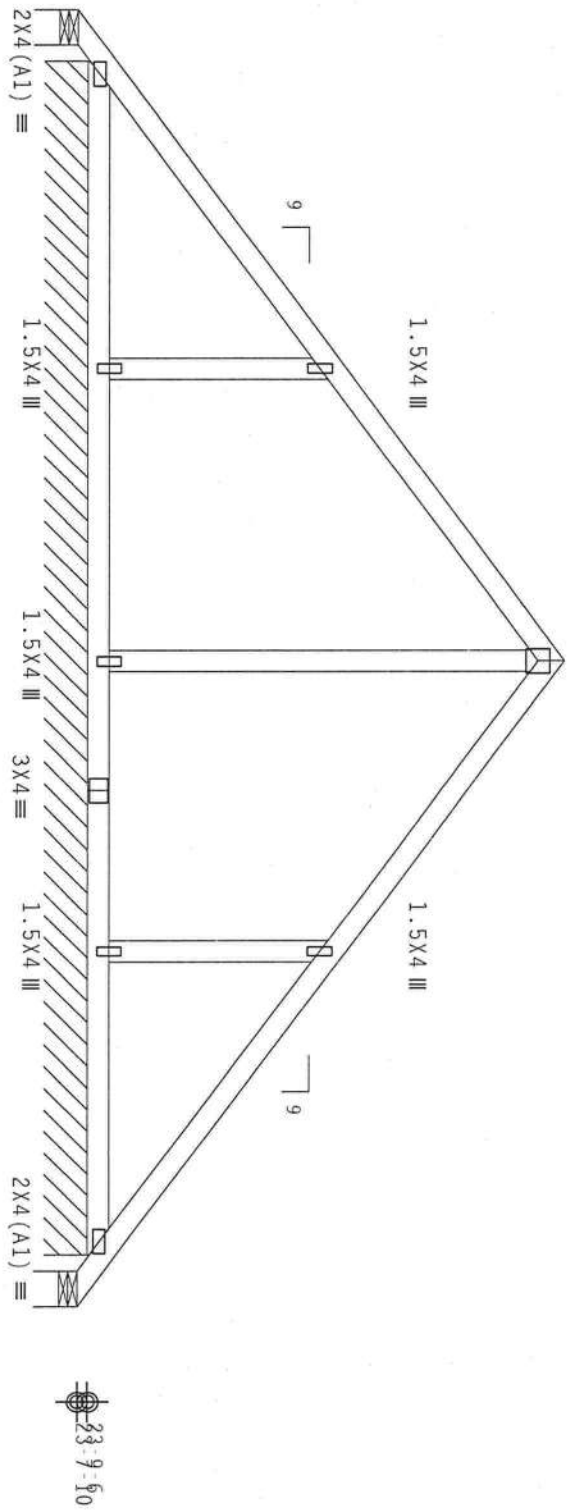
Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 43 PLF at 0.00 to 43 PLF at 8.92
TC - From 43 PLF at 8.92 to 43 PLF at 17.83
BC - From 3 PLF at 0.00 to 3 PLF at 17.83

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Trusses to be spaced at 16.0" OC maximum.



R=40 Rw=90 U=94 W=5.833"
R=53 PLF U=25 PLF W=16-5-0
R=40 Rw=45 U=28 W=5.833"

PLT TYP. Wave

Design Cmt: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.37

QTY: 1

FL/-/4/-/-/R/-

Scale = .375"/Ft.

ALPINE

ntw Building Components Group Inc.

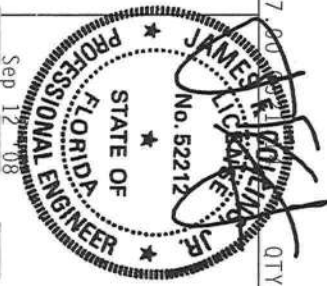
Haines City, FL 33844

FL COA #00378

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304, AND WICA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AISC AND TPI. THE BCG CORRELATION PLATES ARE MADE OF 20/10/100A (40/55/7) ASH 6053 GRADE 40/60 (40, 60/55) GALV. STEEL. APPLY LOCALS TO ALL TRUSSES AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1001-2. ACHORD SECTION OF ALL TRUSSES SHALL BE 16" MIN. UNLESS OTHERWISE SPECIFIED. UNLESS OTHERWISE INDICATED, ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY FOR THE TRUSS COMPONENT DESIGN SHOWN, THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71374
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUSR8228 08256042
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEON-	14581 REV
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF	1TKU8228Z01

SPECIAL LOADS
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC From 43 PLF at 0.00 to 43 PLF at 7.17

	DUR.	FAC. = 1.25	/	PLATE	DUR.	FAC. = 1.25
TC - From	43 PLF at 0.00	to		43 PLF at 7.17		
TC - From	43 PLF at 7.17	to		43 PLF at 10.67		
TC - From	43 PLF at 10.67	to		43 PLF at 17.83		
BC - From	3 PLF at 0.00	to		3 PLF at 17.83		

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

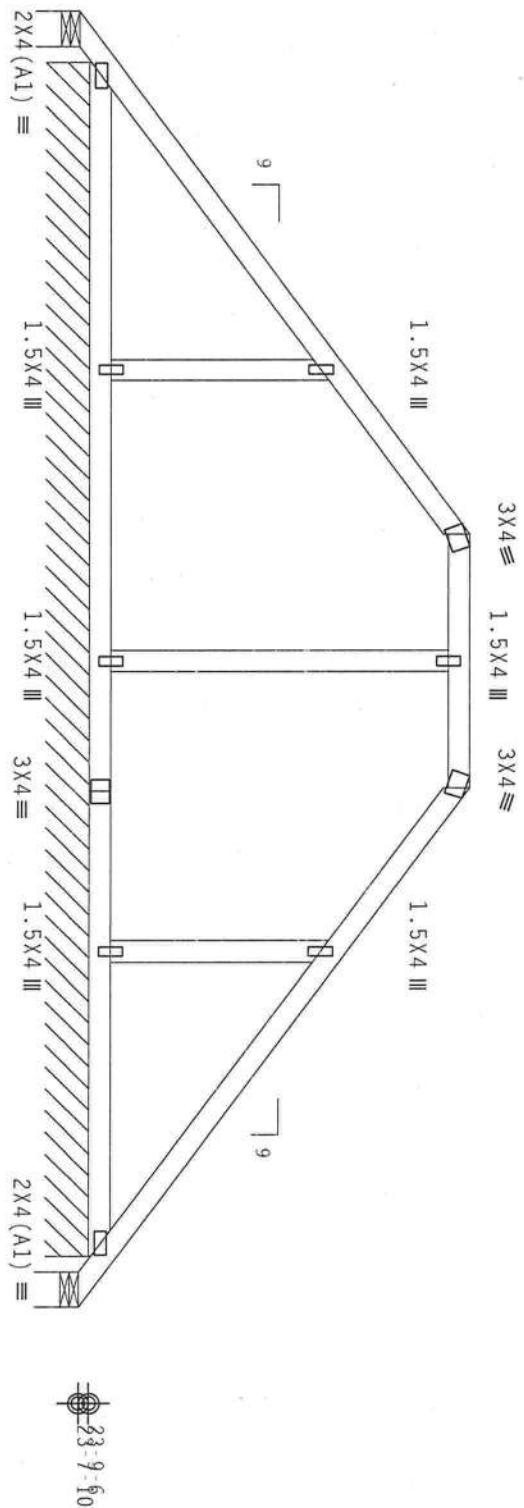


Diagram of a continuous beam with three supports. The beam is divided into four spans: 8'-2", 6'-5", 3'-6", and 6'-5". The total length is 17'-10" over 3 supports. The beam is labeled R=42, R_w=78, U=82, W=5.833" and R=54, P_{LF} U=26, P_{LF} W=16'-5".

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

Scale = .375"/Ft.

WARNING: THESE PRODUCTS REQUIRE PROPER CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND REPAIRING. REFER TO NCST-1 (BUILDING COMPONENT SAFETY CARE IN FABRICATION), NCST-2 (INSTALLATION), NCST-3 (REPAIRING), NCST-4 (HANDLING), NCST-5 (SHIPPING), NCST-6 (INSTALLATION), NCST-7 (HANDLING), NCST-8 (INSTALLATION), NCST-9 (HANDLING), NCST-10 (INSTALLATION), NCST-11 (HANDLING), NCST-12 (INSTALLATION), NCST-13 (HANDLING), NCST-14 (INSTALLATION), NCST-15 (HANDLING), NCST-16 (INSTALLATION), NCST-17 (HANDLING), NCST-18 (INSTALLATION), NCST-19 (HANDLING), NCST-20 (INSTALLATION), NCST-21 (HANDLING), NCST-22 (INSTALLATION), NCST-23 (HANDLING), NCST-24 (INSTALLATION), NCST-25 (HANDLING), NCST-26 (INSTALLATION), NCST-27 (HANDLING), NCST-28 (INSTALLATION), NCST-29 (HANDLING), NCST-30 (INSTALLATION), NCST-31 (HANDLING), NCST-32 (INSTALLATION), NCST-33 (HANDLING), NCST-34 (INSTALLATION), NCST-35 (HANDLING), NCST-36 (INSTALLATION), NCST-37 (HANDLING), NCST-38 (INSTALLATION), NCST-39 (HANDLING), NCST-40 (INSTALLATION), NCST-41 (HANDLING), NCST-42 (INSTALLATION), NCST-43 (HANDLING), NCST-44 (INSTALLATION), NCST-45 (HANDLING), NCST-46 (INSTALLATION), NCST-47 (HANDLING), NCST-48 (INSTALLATION), NCST-49 (HANDLING), NCST-50 (INSTALLATION), NCST-51 (HANDLING), NCST-52 (INSTALLATION), NCST-53 (HANDLING), NCST-54 (INSTALLATION), NCST-55 (HANDLING), NCST-56 (INSTALLATION), NCST-57 (HANDLING), NCST-58 (INSTALLATION), NCST-59 (HANDLING), NCST-60 (INSTALLATION), NCST-61 (HANDLING), NCST-62 (INSTALLATION), NCST-63 (HANDLING), NCST-64 (INSTALLATION), NCST-65 (HANDLING), NCST-66 (INSTALLATION), NCST-67 (HANDLING), NCST-68 (INSTALLATION), NCST-69 (HANDLING), NCST-70 (INSTALLATION), NCST-71 (HANDLING), NCST-72 (INSTALLATION), NCST-73 (HANDLING), NCST-74 (INSTALLATION), NCST-75 (HANDLING), NCST-76 (INSTALLATION), NCST-77 (HANDLING), NCST-78 (INSTALLATION), NCST-79 (HANDLING), NCST-80 (INSTALLATION), NCST-81 (HANDLING), NCST-82 (INSTALLATION), NCST-83 (HANDLING), NCST-84 (INSTALLATION), NCST-85 (HANDLING), NCST-86 (INSTALLATION), NCST-87 (HANDLING), NCST-88 (INSTALLATION), NCST-89 (HANDLING), NCST-90 (INSTALLATION), NCST-91 (HANDLING), NCST-92 (INSTALLATION), NCST-93 (HANDLING), NCST-94 (INSTALLATION), NCST-95 (HANDLING), NCST-96 (INSTALLATION), NCST-97 (HANDLING), NCST-98 (INSTALLATION), NCST-99 (HANDLING), NCST-100 (INSTALLATION).

No. 52212

FL/-/4/-/-/R/-		Scale = .375"/Ft.
TC LL	20.0 PSF	REF R8228 - 71375
TC DL	10.0 PSF	DATE 09/11/08
PC DL	10.0 PSF	



ITW Building Components Group Inc.
Haines City, FL 33844
813 264 4027

1 FL/-/4/-/-/R/-		Scale = .375"/ft.	
TC LL	20.0 PSF	REF	R8228- 71375
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 0825027
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEON-	14584 REV
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228701

SPECIAL LOADS

(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC From 43 PLF at 0.00 to 43 PLF at 7.17

TC - From	43 PLF at 7.17 to	43 PLF at 10.67 to
TC - From	43 PLF at 10.67 to	43 PLF at 17.83 to
BC - From	3 PLF at 0.00 to	3 PLF at 17.83 to

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



7.37

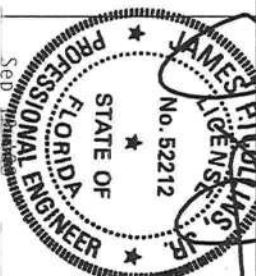
QTY:1 FL/-/4/-/-/R/-

Scale = .375" / Ft.

JAMES H. COLLINS, JR.
LICENSE
No. 52212

ALPINE

ITW Building Components Group Inc

Haines City, FL 33844
FL 33844-2700

TC LL	20.0 PSF	REF	R8228- 71376
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUSR8228 08256043
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	14586 REV
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228701

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Weds 2x4 SP #3

110 mph wind, 25.82 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf,
wind BC DL=2.0 psf. $I_w=1.00$ $GCP(+/)=0.18$

Wind reactions based on MMFRS pressures.

Trusses to be spaced at 16.0" OC maximum.

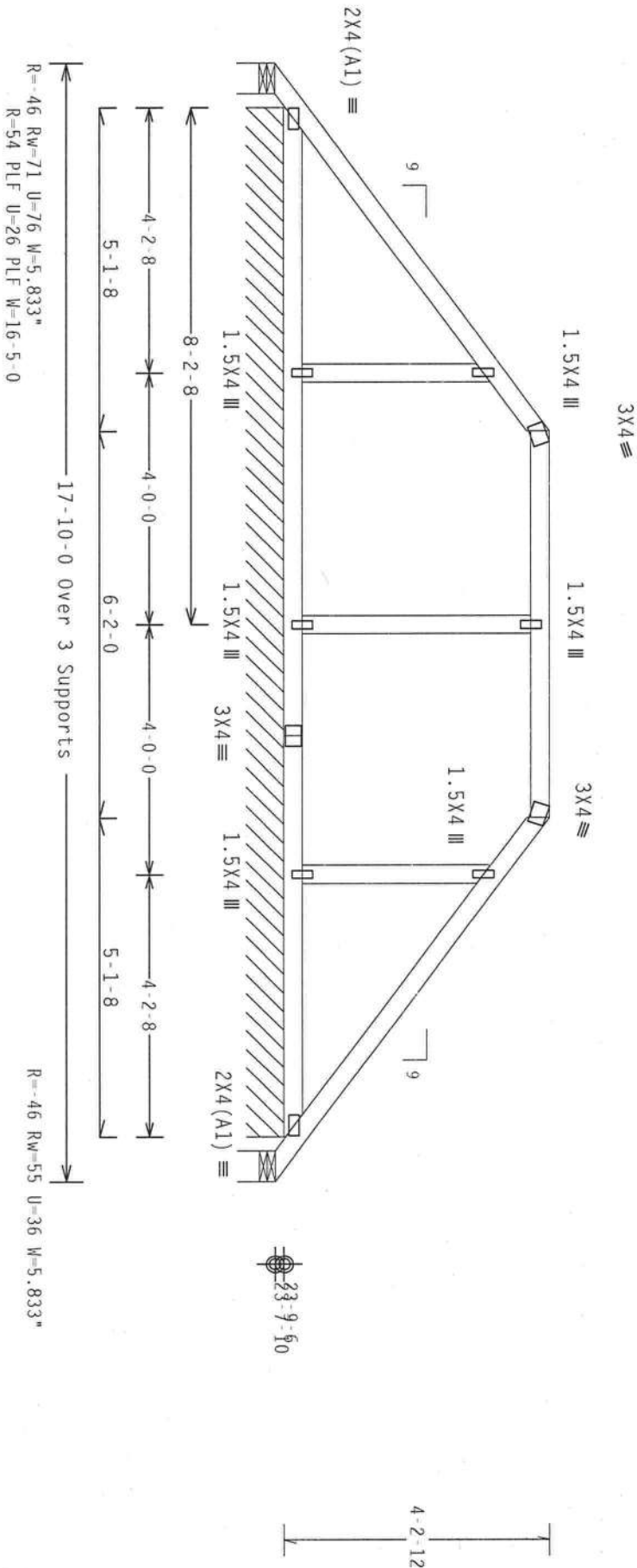
Refer to DWG PIGBACKR0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 43 PLF at 0.00 to 43 PLF at 5.83
TC - From 43 PLF at 5.83 to 43 PLF at 12.00
TC - From 43 PLF at 12.00 to 43 PLF at 17.83
BC - From 3 PLF at 0.00 to 3 PLF at 17.83

In lieu of structural panels or rigid ceiling use purlins to brace
all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/0(0)

7.36

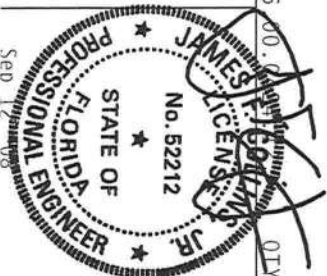
OTY:1

FL/-/4/-/-/R/-

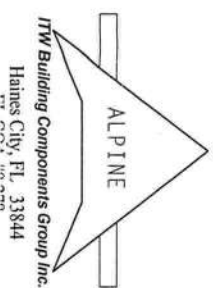
Scale = .375"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO RES1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22319) AND TPCA (WOOD TRUSS COUNCIL OF AMERICA, 6500
BROADVIEW AVENUE, SUITE 100, CHICAGO, IL 60631) FOR ADDITIONAL INFORMATION. UNLESS OTHERWISE
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD IN CONFORMANCE WITH
THIS DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PDA) AND TPI. THE BCG
CONNECTOR PLATES ARE MADE OF 20/18/16GA. (U/H/SS/SR) ASTM A653 GRADE 40/60 (U, K/H/SS) GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT
DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE
BUILDING DESIGNER PER ANSI/TPI 1, SEC. 2.



TC LL	20.0 PSF	REF R8228-71377
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCUSR8228 0825028
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEON- 104678
DUR.FAC.	1.25	FROM AH
SPACING	16.0"	JRRF- 1TKU8228/01



2 COMPLETE TRUSSES REQUIRED $\equiv$
Nailing Schedule: (12d Common (0.148"x3.25", min.)_nails)

2 COMPLETE TRUSSES REQUIRED
Nailing Schedule: (12d Common_ (0.148"x3.25"

Nailing Schedule: (12d Common (0.148"x3.25",min.)_nails)
 Top Chord: 1 Row @12.00" o.c.
 Bot Chord: 1 Row @12.00" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails
 in each row to avoid splitting.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

QTY:1

Scale = .375" / Ft.

U.S. DEPARTMENT OF JUSTICE
FEDERAL BUREAU OF INVESTIGATION
WASHINGTON, D.C. 20535
JAN 11 1964
NO. 52212

REF	R8228- 71374
DATE	09/12/08
DDMM	09/12/08

STATE OF

ITW Building Components Group Inc.

Haines City, FL 33844

*** IMPORTANT *** THE UNDERSIGNED HAS A COPY OF THIS DESIGN DETAILING THE INSTALLATION CONTRACTOR, THE BGC, INC., SHALL BE RESPONSIBLE FOR ANY VIOLATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE THROUS IN CONFORMANCE WITH THE DESIGN COORDINATES WITH APPLICABLE PROVISIONS OF MDX (CONCRETE DESIGN SPEC., BY K.H.-55) GAGE, STEEL, APPROPRIATE CORRECTION PLATES AND MOD. OF 2010/1606 (BY J.H.-55) ASYM. A651, GROUND 40/60, (BY K.H.-55) GAGE, STEEL, APPROPRIATE PLATES TO EACH FACE OF THROUS AND, ONCE THE THROUS ARE LOCATED ON THIS DESIGN, POSITIONING PER DRAWINGS 1650, 1-1, 1-2, 1-3, 1-4, 1-5, 1-6, 1-7, 1-8, 1-9, 1-10, 1-11, 1-12, 1-13, 1-14, 1-15, 1-16, 1-17, 1-18, 1-19, 1-20, 1-21, 1-22, 1-23, 1-24, 1-25, 1-26, 1-27, 1-28, 1-29, 1-30, 1-31, 1-32, 1-33, 1-34, 1-35, 1-36, 1-37, 1-38, 1-39, 1-40, 1-41, 1-42, 1-43, 1-44, 1-45, 1-46, 1-47, 1-48, 1-49, 1-50, 1-51, 1-52, 1-53, 1-54, 1-55, 1-56, 1-57, 1-58, 1-59, 1-60, 1-61, 1-62, 1-63, 1-64, 1-65, 1-66, 1-67, 1-68, 1-69, 1-70, 1-71, 1-72, 1-73, 1-74, 1-75, 1-76, 1-77, 1-78, 1-79, 1-80, 1-81, 1-82, 1-83, 1-84, 1-85, 1-86, 1-87, 1-88, 1-89, 1-90, 1-91, 1-92, 1-93, 1-94, 1-95, 1-96, 1-97, 1-98, 1-99, 1-100, 1-101, 1-102, 1-103, 1-104, 1-105, 1-106, 1-107, 1-108, 1-109, 1-110, 1-111, 1-112, 1-113, 1-114, 1-115, 1-116, 1-117, 1-118, 1-119, 1-120, 1-121, 1-122, 1-123, 1-124, 1-125, 1-126, 1-127, 1-128, 1-129, 1-130, 1-131, 1-132, 1-133, 1-134, 1-135, 1-136, 1-137, 1-138, 1-139, 1-140, 1-141, 1-142, 1-143, 1-144, 1-145, 1-146, 1-147, 1-148, 1-149, 1-150, 1-151, 1-152, 1-153, 1-154, 1-155, 1-156, 1-157, 1-158, 1-159, 1-160, 1-161, 1-162, 1-163, 1-164, 1-165, 1-166, 1-167, 1-168, 1-169, 1-170, 1-171, 1-172, 1-173, 1-174, 1-175, 1-176, 1-177, 1-178, 1-179, 1-180, 1-181, 1-182, 1-183, 1-184, 1-185, 1-186, 1-187, 1-188, 1-189, 1-190, 1-191, 1-192, 1-193, 1-194, 1-195, 1-196, 1-197, 1-198, 1-199, 1-200, 1-201, 1-202, 1-203, 1-204, 1-205, 1-206, 1-207, 1-208, 1-209, 1-210, 1-211, 1-212, 1-213, 1-214, 1-215, 1-216, 1-217, 1-218, 1-219, 1-220, 1-221, 1-222, 1-223, 1-224, 1-225, 1-226, 1-227, 1-228, 1-229, 1-230, 1-231, 1-232, 1-233, 1-234, 1-235, 1-236, 1-237, 1-238, 1-239, 1-240, 1-241, 1-242, 1-243, 1-244, 1-245, 1-246, 1-247, 1-248, 1-249, 1-250, 1-251, 1-252, 1-253, 1-254, 1-255, 1-256, 1-257, 1-258, 1-259, 1-260, 1-261, 1-262, 1-263, 1-264, 1-265, 1-266, 1-267, 1-268, 1-269, 1-270, 1-271, 1-272, 1-273, 1-274, 1-275, 1-276, 1-277, 1-278, 1-279, 1-280, 1-281, 1-282, 1-283, 1-284, 1-285, 1-286, 1-287, 1-288, 1-289, 1-290, 1-291, 1-292, 1-293, 1-294, 1-295, 1-296, 1-297, 1-298, 1-299, 1-300, 1-301, 1-302, 1-303, 1-304, 1-305, 1-306, 1-307, 1-308, 1-309, 1-310, 1-311, 1-312, 1-313, 1-314, 1-315, 1-316, 1-317, 1-318, 1-319, 1-320, 1-321, 1-322, 1-323, 1-324, 1-325, 1-326, 1-327, 1-328, 1-329, 1-330, 1-331, 1-332, 1-333, 1-334, 1-335, 1-336, 1-337, 1-338, 1-339, 1-340, 1-341, 1-342, 1-343, 1-344, 1-345, 1-346, 1-347, 1-348, 1-349, 1-350, 1-351, 1-352, 1-353, 1-354, 1-355, 1-356, 1-357, 1-358, 1-359, 1-360, 1-361, 1-362, 1-363, 1-364, 1-365, 1-366, 1-367, 1-368, 1-369, 1-370, 1-371, 1-372, 1-373, 1-374, 1-375, 1-376, 1-377, 1-378, 1-379, 1-380, 1-381, 1-382, 1-383, 1-384, 1-385, 1-386, 1-387, 1-388, 1-389, 1-390, 1-391, 1-392, 1-393, 1-394, 1-395, 1-396, 1-397, 1-398, 1-399, 1-400, 1-401, 1-402, 1-403, 1-404, 1-405, 1-406, 1-407, 1-408, 1-409, 1-410, 1-411, 1-412, 1-413, 1-414, 1-415, 1-416, 1-417, 1-418, 1-419, 1-420, 1-421, 1-422, 1-423, 1-424, 1-425, 1-426, 1-427, 1-428, 1-429, 1-430, 1-431, 1-432, 1-433, 1-434, 1-435, 1-436, 1-437, 1-438, 1-439, 1-440, 1-441, 1-442, 1-443, 1-444, 1-445, 1-446, 1-447, 1-448, 1-449, 1-450, 1-451, 1-452, 1-453, 1-454, 1-455, 1-456, 1-457, 1-458, 1-459, 1-460, 1-461, 1-462, 1-463, 1-464, 1-465, 1-466, 1-467, 1-468, 1-469, 1-470, 1-471, 1-472, 1-473, 1-474, 1-475, 1-476, 1-477, 1-478, 1-479, 1-480, 1-481, 1-482, 1-483, 1-484, 1-485, 1-486, 1-487, 1-488, 1-489, 1-490, 1-491, 1-492, 1-493, 1-494, 1-495, 1-496, 1-497, 1-498, 1-499, 1-500, 1-501, 1-502, 1-503, 1-504, 1-505, 1-506, 1-507, 1-508, 1-509, 1-510, 1-511, 1-512, 1-513, 1-514, 1-515, 1-516, 1-517, 1-518, 1-519, 1-520, 1-521, 1-522, 1-523, 1-524, 1-525, 1-526, 1-527, 1-528, 1-529, 1-530, 1-531, 1-532, 1-533, 1-534, 1-535, 1-536, 1-537, 1-538, 1-539, 1-540, 1-541, 1-542, 1-543, 1-544, 1-545, 1-546, 1-547, 1-548, 1-549, 1-550, 1-551, 1-552, 1-553, 1-554, 1-555, 1-556, 1-557, 1-558, 1-559, 1-560, 1-561, 1-562, 1-563, 1-564, 1-565, 1-566, 1-567, 1-568, 1-569, 1-570, 1-571, 1-572, 1-57

FL/-/4/-/-/R/-		Scale = .375"/FL	
TC LL	20.0 PSF	REF	R8228- 7137
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUSR8228 08256
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT. LD.	40.0 PSF	SEQN-	14589
DUR. FAC.	1.25	FROM	AH
SPACING	16.0"	REF-	TKU822870

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 24.82 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

Trusses to be spaced at 16.0" OC maximum.

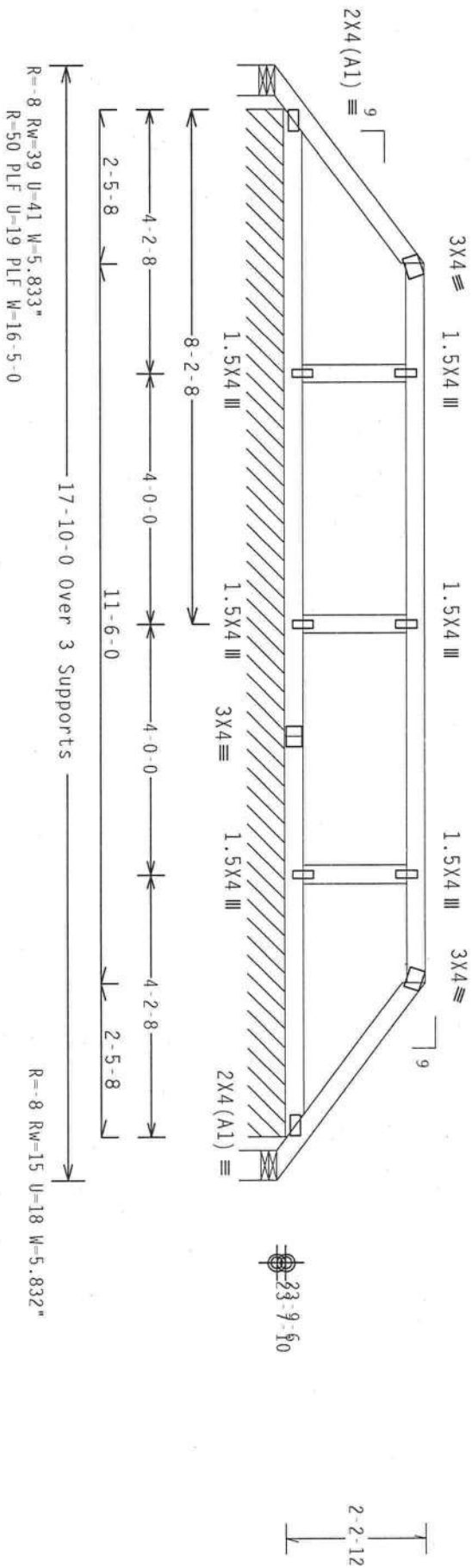
Refer to DWG PIGBACKR0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

----- (LUMBER	DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From	43 PLF at 0.00 to 43 PLF at 3.17
TC - From	43 PLF at 3.17 to 43 PLF at 14.67
TC - From	43 PLF at 14.67 to 43 PLF at 17.83
BC - From	3 PLF at 0.00 to 3 PLF at 17.83

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.00

QTY:1

FL/-/4/-/-/R/-

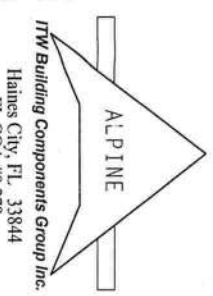
Scale = .375"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, SHIPPING, INSTALLING AND DRACING. REFER TO DESIG. (BUILDING COMPONENT SAFETY INFORMATION - PUBLISHED BY THE TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALABAMA, MO, 62210) FOR GUIDANCE IN THE PROPER HANDLING OF TRUSSES. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITU BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & DRACING OF TRUSSES. THE BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AOS NATIONAL DESIGN SPEC. BY AIA/PA AND TPI. THE BCG CONECTOR PLATES ARE MADE OF 2017/1666 (U.H/55/K) ASIN A653 GRADE 40/60 (H, K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71379
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUR8228 0825029
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEON-	104688
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKU8228Z01



Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

110 mph wind, 24.32 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, IW=1.00 GCP1(+/-)=0.18

Wind reactions based on MMFRS pressures.

Trusses to be spaced at 16.0" OC maximum.

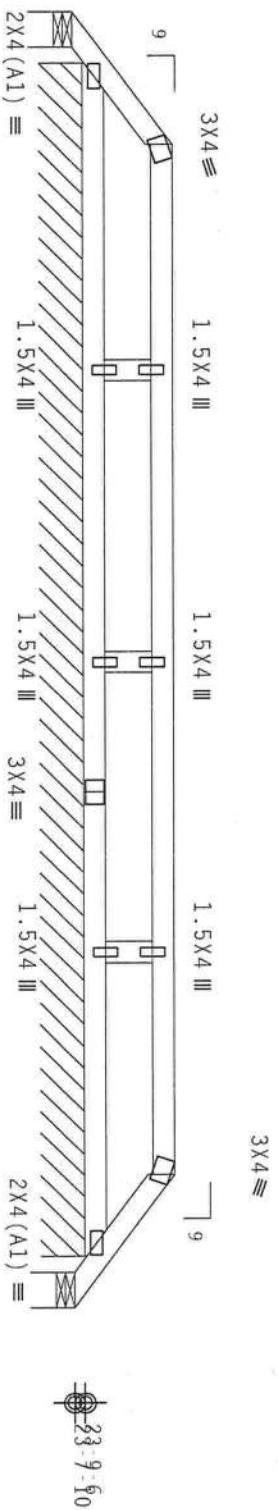
Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

TC - From	43 PLF at 0.00 to 43 PLF at 1.83
TC - From	43 PLF at 1.83 to 43 PLF at 16.00
TC - From	43 PLF at 16.00 to 43 PLF at 17.83
BC - From	3 PLF at 0.00 to 3 PLF at 17.83

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



1'-1'-8" 8'-2'-8" 14'-2'-0" 1'-1'-8"

R=0 Rw=15 U=17 W=5.833"

R=49 PLF U=18 PLF W=16.5-0

R=0 Rw=2 U=3 W=5.833"

PLT TYP. Wave

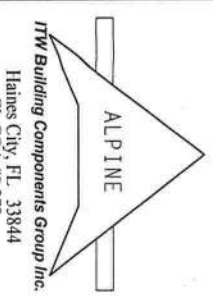
Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/0(0)

7.37.00

QTY:1 FL/-/4/-/-/R/-

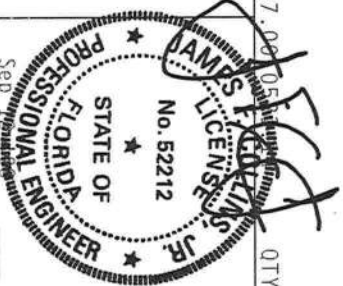
Scale =.375"/Ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DESI (BUILDING COMPONENT SAFETY INFORMATION) - PROVIDED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 500 N. MEEKS STREET, PITTSBURGH, PA, 15222) FOR SAFETY PRACTICES. TRUSSES SHALL BE PROPERLY ATTACHED TO THE STRUCTURE AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** SUPPLIER A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF TRUSSES IN COMPLIANCE WITH THE DESIGN CRITERIA OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 500 N. MEEKS STREET, PITTSBURGH, PA, 15222) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 500 N. MEEKS STREET, PITTSBURGH, PA, 15222) FOR SAFETY PRACTICES. TRUSSES SHALL BE PROPERLY ATTACHED TO THE STRUCTURE AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. (AISC) OR THIS DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. (AISC) OR THIS DESIGN SHOWN.



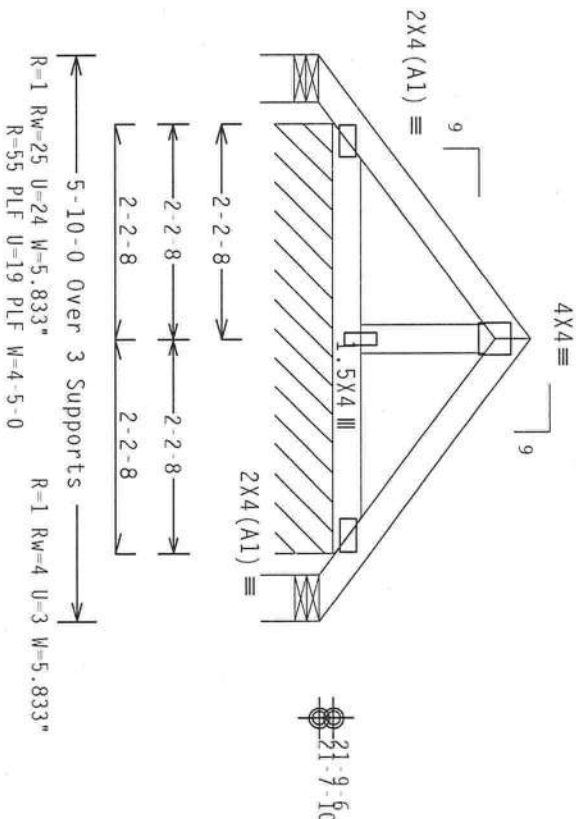
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TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUR8228 08256046
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEON-	14592 REV
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	ITKUR8228201

SPECIAL LOADS			
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25) -----			
TC - From	43 PLF at 0.00 to	43 PLF at	2.92
TC - From	43 PLF at 2.92 to	43 PLF at	5.83
BC - From	3 PLF at 0.00 to	3 PLF at	5.83

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.



2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common, (0.148"x3.25", min.)_nails)

Top Chord: 1 Row @12.00" o.c.
 Bot Chord: 1 Row @12.00" o.c.
 Webs : 1 Row @ 4" o.c.

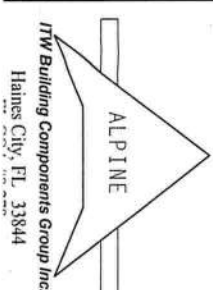
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 22.73 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 Gcpi(+/-)-0.18

Trusses to be spaced at 16.0" OC maximum.

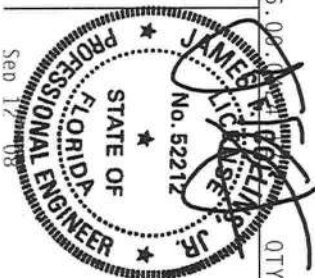
FL/-4/-/-/R/-		Scale=.5"/ft.
TC LL	20.0 PSF	REF R8228- 71381
TC DL	10.0 PSF	DATE 09/11/08
BC DL	10.0 PSF	DRW HCSUR8228 08255030
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 104739
DUR.FAC.	1.25	FROM AH
SPACING	16.0"	JREF- 1TKU8228Z01

PLT TYP. Wave



WARNING—TRUSSES BEING ERECTER CAME IN INFORMATION, HANDLING, UNLOADING, INSTALLING AND DERECTING REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE STEEL TRUSS PRACTICE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA, ONELESS ENTERPRISE PLACE, MADISON, WI 53713) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. OTHERWISE, INDICATED TO CORROSION SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

IMPORTANT—TURNISH A COPY OF THIS DECISION TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DECISION. A FAILURE TO FOLLOW THE TRUSSES IN CORRESPONDANCE WITH THE DECISIONS FOR THE TRUSSES WILL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE TRUSSES WITH APPLICABLE PROVISIONS OF HIS NATIONAL DECISION SPEC. BY AREA AND TPI. THE BCG CONNECTION PLATES ARE MADE OF 201/10/1604 (48/15/25)25 ASYD A563 GR40D 40/40 (48/15/25) GR40D. STEEL APPLY RELATES TO EACH FACE OF TRUSSES AND, UNLESS OTHERWISE NOTICED ON THIS DECISION, POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AREA AS OF TPI-1202 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY FOR THE TRUSSES COMPONENTS OF THE DECISION SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DECISION SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AREA/TPI 1 SEC. 2.



Sep 12 08

Top chord 2x4 SP #2
Bot chord 2x4 SP #2
Webs 2x4 SP #3

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 43 PLF at 0.00 to 43 PLF at 2.92
TC - From 43 PLF at 2.92 to 43 PLF at 5.83
BC - From 3 PLF at 0.00 to 3 PLF at 5.83

Wind reactions based on MMFRS pressures.

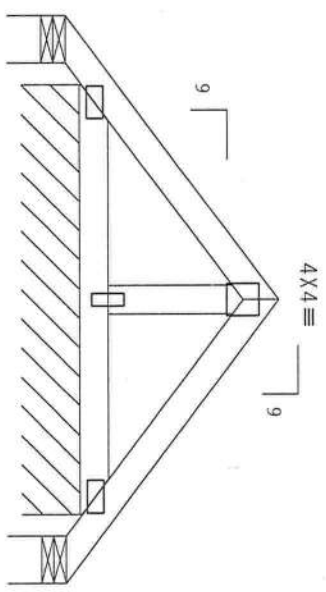
In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

3 COMPLETE TRUSSES REQUIRED

Nailling Schedule: (12d Common (0.148"x3.25", min.)_naills)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailling as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.
110 mph wind, 22.73 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)-0.18
Trusses to be spaced at 16.0" OC maximum.



21-9-60

2X4 (A1) = 1.5X4 III 2X4 (A1) =
2'-2-8 2'-2-8
5-10-0 Over 3 Supports
R=1 Rw=2 U=24 W=5.833"
R=55 PLF U=19 PLF W=4-5-0 R=1 Rw=4 W=5.833"

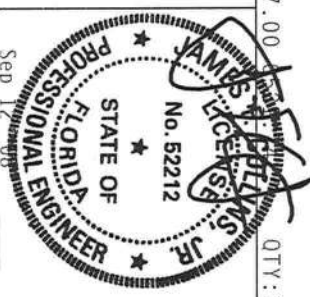
Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DESI (BOLTING COMPONENT SAFETY) FOR OPERATIONAL, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 2100 NORTH ST. AUSTIN, TX 78701. SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT *TURN IN A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (U/L/S/S/X) ASH 6653 GRADE 40/60 (U, K/20/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATS FOLLOWED BY (1) SHALL BE PER AMES AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMST/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 71382
TC DL	10.0 PSF	DATE 09/12/08
BC DL	10.0 PSF	DRW HCURR8228 08256047
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEON- 14595 REV
DUR.FAC.	1.25	FROM AH
SPACING	16.0"	JREF- 1TKU8228201

SPECIAL LOADS			
-----	(LUMBER	DUR.FAC.=1.25	/ PLATE DUR.FAC.=1.25)
TC -	From	43 PLF at 0.00	to 43 PLF at 2.92
TC -	From	43 PLF at 2.92	to 43 PLF at 5.83
BC -	From	3 PLF at 0.00	to 3 PLF at 5.83

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

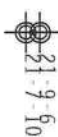
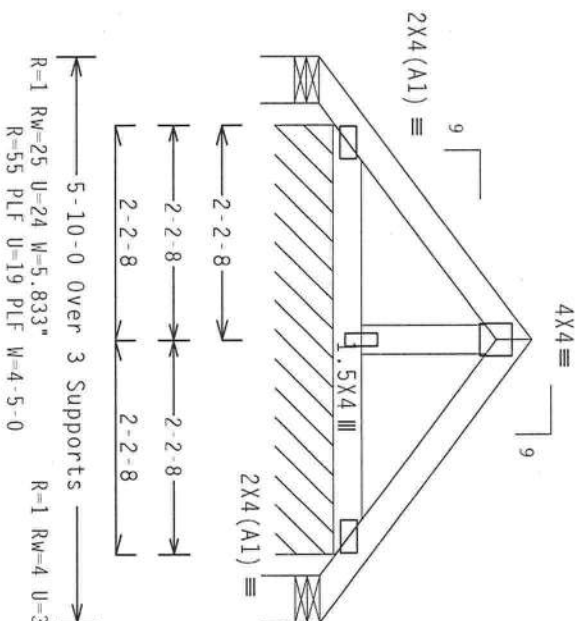
Refer to DWG PIGBACKB0207 for piggyback details
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

Top chord:	1 Row	@ 12.00" o.c.
Bot Chord:	1 Row	@ 12.00" o.c.
Web:	1 Row	@ 4" o.c.

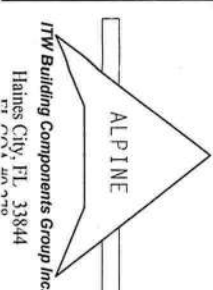
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 22.73 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ Gcpl (+/-)=0.18

Trusses to be spaced at 16.0" OC maximum.



PLT TYP. Wave



***WARNING:** Fumes, fire, explosion, electrical, gas, in contamination, handling, shipping, installing, and breaking refer to dusts (BUILDING COMPONENTS EXCEPT GLASS INSULATION). PUBLISHED BY THE FIBERS PASTE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 and WICA (GOOD TRUSS COMPANY) OF AMERICA, 6300 ENTERPRISE LAKE, MDISON, MI 48131 FOR SAFETY PRACTICES, PLEASE TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED FOR GOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM GOOD SHALL HAVE PROPERLY ATTACHED GRID CELLING.

Design Crit: $TPI-2002(STD)/FBC$
$$Cq/RT=1.00(1.25)/0(0)$$

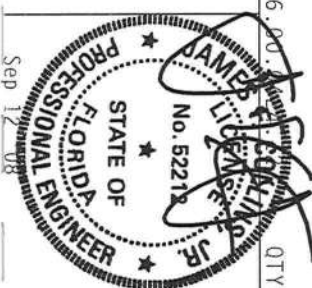
7.36.00

QTY: 1

FL/-/4/-/-/R/-

Scale = .5"/Ft.

* * * IMPORTANT * * * HANDSH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN COORDINATES WITH APPLICABLE PROVISIONS OF AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION) AND THE DESIGN COORDINATES WITH APPLICABLE PROVISIONS OF AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION) PLATES AND BOLTS OF 2010/10/16/64 (A/IN/ST/ST) ASH A655, GRADE 50/60 (F_y 58/66) G.A.V. STEEL. APPLY TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED IN THIS DESIGN, POSITION PER DRAWINGS 1606-2 DRAWING INDICATES. ACCEPTANCE OR PROFESSIONAL ENGINEERING SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/PTP 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71383
TC DL	10.0 PSF	DATE	09/11/08
BC DL	10.0 PSF	DRW	HCUSR8228 08255031
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	104807
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TKUR228Z01

Top Chord 2x4 SP #2
Bot Chord 2x4 SP #2
Webs 2x4 SP #3

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 43 PLF at 0.00 to 43 PLF at 1.83
TC - From 43 PLF at 1.83 to 43 PLF at 4.00
TC - From 43 PLF at 4.00 to 43 PLF at 5.83
BC - From 3 PLF at 0.00 to 3 PLF at 5.83

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

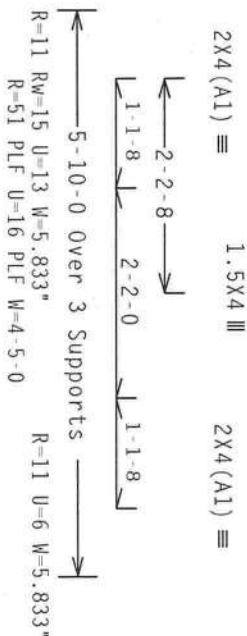
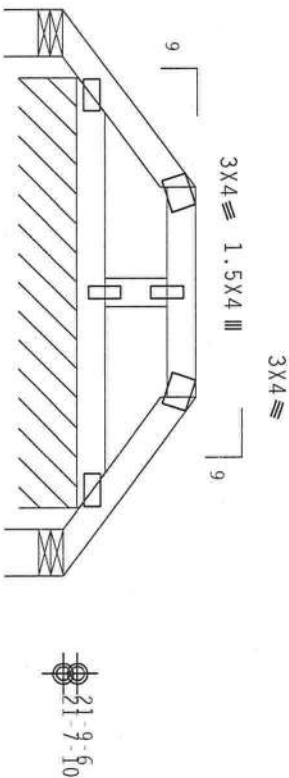
2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common (0.148"x3.25", min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 22.32 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. lw=1.00 GCpi(+/-)-0.18

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

QTY: 1

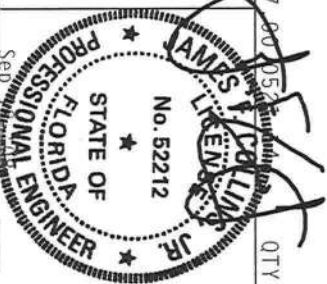
FL/-/4/-/-/R/-

Scale =.5"/Ft.

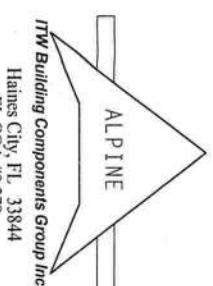
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 317, ALEXANDRIA, VA 22304) FOR TRUSS BRACING REQUIREMENTS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ALAPY) AND TPI. THE BCS CORRELATOR PLATES ARE MADE OF 2017/16/64 (W/45/5) ASTM A563 GRADE 40/60 (W/45/5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE STABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER PART 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 71384
TC DL	10.0 PSF	DATE	09/12/08
BC DL	10.0 PSF	DRW	HCUR8228 08256039
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEON-	14570 REV
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JRFE-	1TKUR228Z01



+ 2X4 CONTINUOUS LATERAL BRACING AT 24 O.C.
MAXIMUM SPACING. ATTACH TO EACH TOP CHORD WITH
(2) 16d COMMON (0.162"X 3.5", MIN) NAILS.
BRACING MATERIAL TO BE SUPPLIED AND ATTACHED
AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.

++ 2X4 SO. PINE #2 N OR SPF #1/#2 FILLER TOP CHORD.

+++ 2X4 SO. PINE #3 OR SPF #1/#2 VERTICAL WEBS SPACED
48" OC MAXIMUM.

* 8/12 MAXIMUM PITCH.

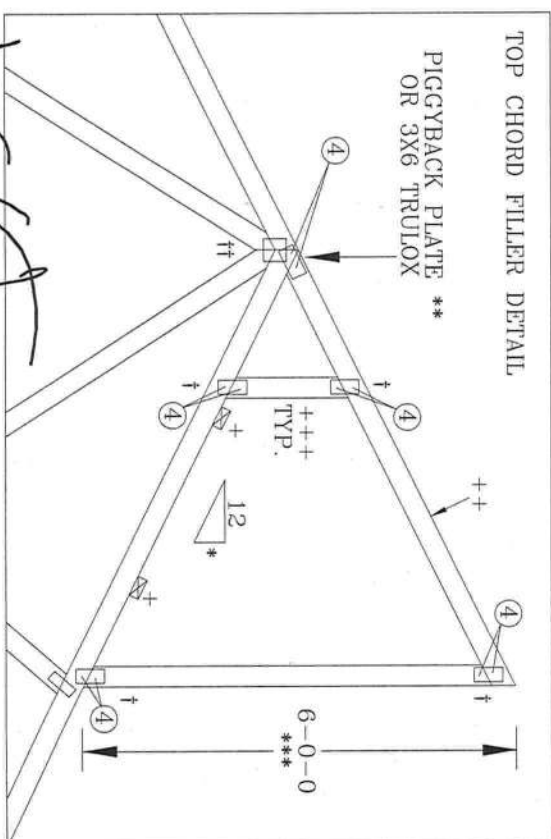
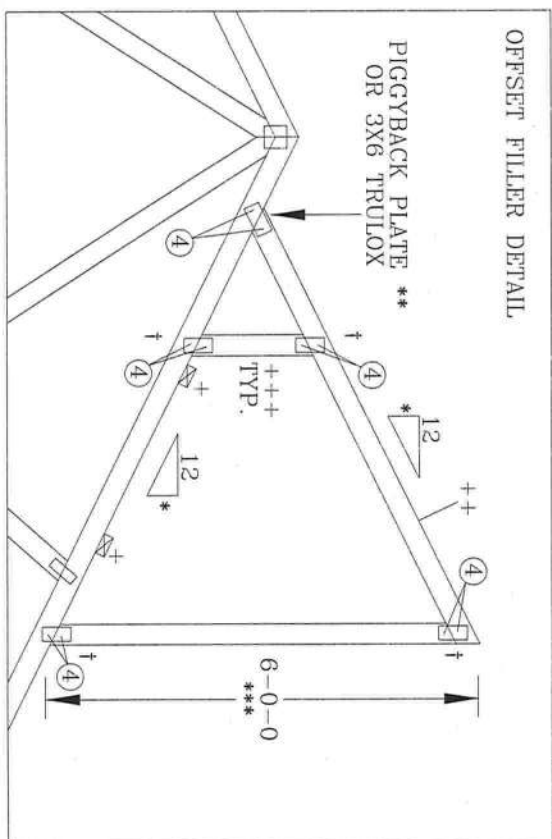
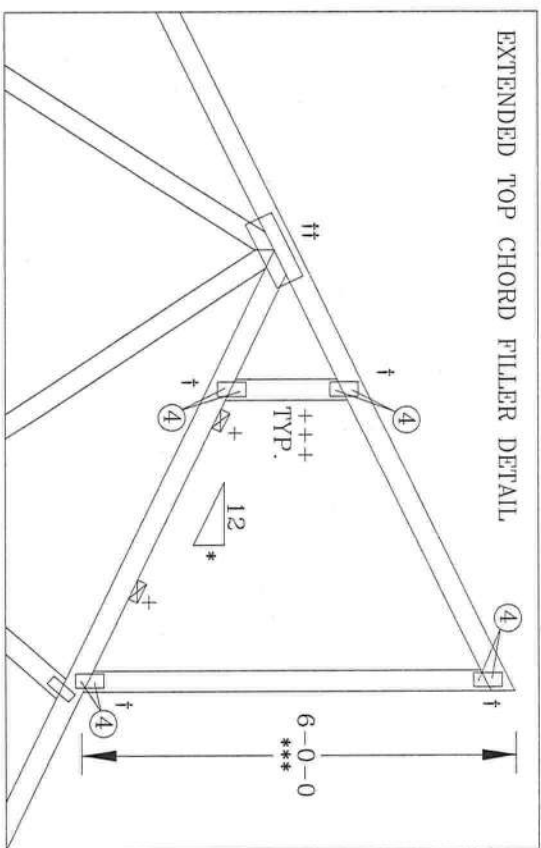
** 2X8.25 PIGGYBACK SPECIAL PLATE. SEE DRAWING PIGBACKB0699
FOR PIGGYBACK SPECIAL PLATE INFORMATION.

*** 6'0" MAXIMUM HEIGHT.

† W2X4 OR 3X6 TRULOX.

‡ REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS
DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT
SHOWN.

0.120"X 1.375" NAILS REQUIRED
FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED
IN CIRCLES MUST BE APPLIED TO EACH FACE OF EACH TRUSS PLY.
SEE DWG. 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS



ALPINE

TRUSS BUILDING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA

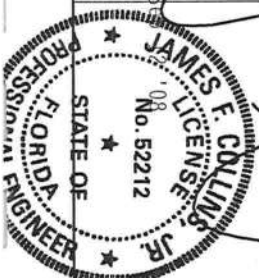
WARNING TRUSSES REQUIRING EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA 22314) AND VITCA (VICTAL COUNCIL OF ARCHITECTS, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. (TYP. BCG, INC.) SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE, OR FABRICATING, HANDLING, SHIPPING, INSTALLING, OR BRACING OF TRUSSES. DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY NDS®) AND TYP. BCG CONNECTOR PLATES ARE MADE OF 2018/18624 C/A/H/S/ASTM A533 GRADE 40 AND C/A/H/S/20 DESIGNATION PER DRAWING 1604-2. HANDLING OF PLATES FOLLOWED BY CD SHALL BE PER ANNEX A3 OF TPI 1-2006 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN, THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ANSI/TPI 1 SEC. 2.

TC LL	MAX 30 PSF	REF	TC-FILLER
TC DL	MAX 15 PSF	DATE	2/23/07
BC DL	MAX 10 PSF	DRWG	TCFILLER0207
BC LL	0 PSF	-ENG	SJP/KAR
TOT. LD.	MAX 55 PSF		
DUR. FAC.	1.15 OR 1.33		
SPACING	24.0"		



ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA



BOTTOM CHORD FILLER DETAIL

* OPTIONAL INTERIOR OR CANTILEVER BEARING. MINIMUM PLATE SIZES (1X3 WAVE) MAY BE USED IF BEARING IS OMITTED. WEDGE OR VERTICAL MEMBER MUST COINCIDE WITH BEARING LOCATION.

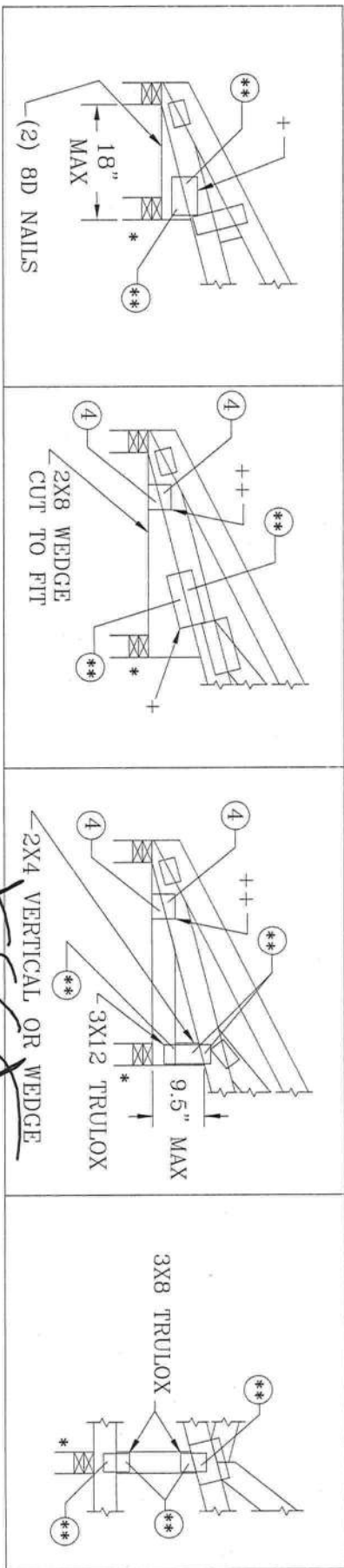
0.120" X 1.375", NAILS, REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF THE TRUSS. SEE DWG. 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS

+ 3X4 WAVE OR 4X8 TRULOX
++ 2X4 WAVE OR 3X6 TRULOX

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

ALL TRULOX PLATES SHOWN ARE MINIMUMS. LARGER PLATES MAY BE REQUIRED TO ACCOMMODATE REQUIRED NAILS (**)

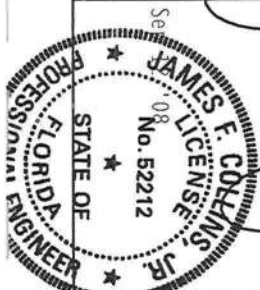
FILLER BOTTOM CHORD OR WEDGE SPECIES	MAXIMUM REACTION		MINIMUM BEARING AREA	** REQUIRED NAILS PER FACE WITH TRULOX PLATES					
	DOWNWARD	UPLIFT		1.00 D.O.L.	1.15 D.O.L.	1.25 D.O.L.	1.33 D.O.L.	1.60 D.O.L.	
DOUGLAS FIR-LARCH	3281#	1656#	1.5" X 3.5"	12	11	10	9	8	
HEM-FIR	2126#	1095#	1.5" X 3.5"	9	8	7	7	6	
SPRUCE-PINE-FIR	2231#	1192#	1.5" X 3.5"	10	9	8	8	6	
SOUTHERN PINE DENSE	3465#	1791#	1.5" X 3.5"	12	11	10	9	8	
SOUTHERN PINE	2966#	1492#	1.5" X 3.5"	10	9	8	8	7	
SOUTHERN PINE NON-DENSE	2520#	1343#	1.5" X 3.5"	9	8	7	7	6	



TRUSS BUILDING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA 22304 AND WCTA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, HANSDEN, VI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ITV, BCG CONNECTOR PLATES ARE MADE OF 2018/1664 (A/4/55)X40 ASTM A653 GRADE 40/60 (A/4/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160-2. ANY INSPECTION OF PLATES FOLLOWED BY CD SHALL BE PER ENGINEER'S DESIGN. ENGINEER'S DESIGN SHALL BE THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER AISC/TPI 1 SEC. 2.



THIS DRAWING REPLACES DRAWINGS A115 A115/R & 884.132			
TC LL	—	PSF	REF BC FILLER
TC DL	—	PSF	DATE 2/23/07
BC DL	10.0	PSF	DRWG BCFILLER0207
BC LL	—	PSF	—ENG DLJ/KAR
TOT. LD.	—	PSF	
DUR. FAC. 1.0/1.15/1.25/1.33			
SPACING 24.0"			

BOTTOM CHORD FILLER REPAIR

RECOMMENDED REPAIR PROCEDURE

1. MEASURE DISTANCE FOR NEW LENGTH OF FILLER.
2. APPLY NEW 2X4 STUD GRADE OR BETTER VERTICAL SCAB TO BOTTOM CHORD AND FILLER WITH (3) NAILS 0.131" DIA. x 3.0" OR LARGER, (I.E. 10d OR 16d COMMON, SINKER, GUN, OR 16d BOX NAILS) TO EACH END OF VERTICAL.
3. CAREFULLY REMOVE EFFECTED CONNECTOR PLATES. USE CARE NOT TO DAMAGE THE REMAINING CONNECTOR PLATES OR LUMBER IN ANY WAY.
4. TRIM FILLER TO LENGTH, AT EDGE OF NEW VERTICAL SCAB.

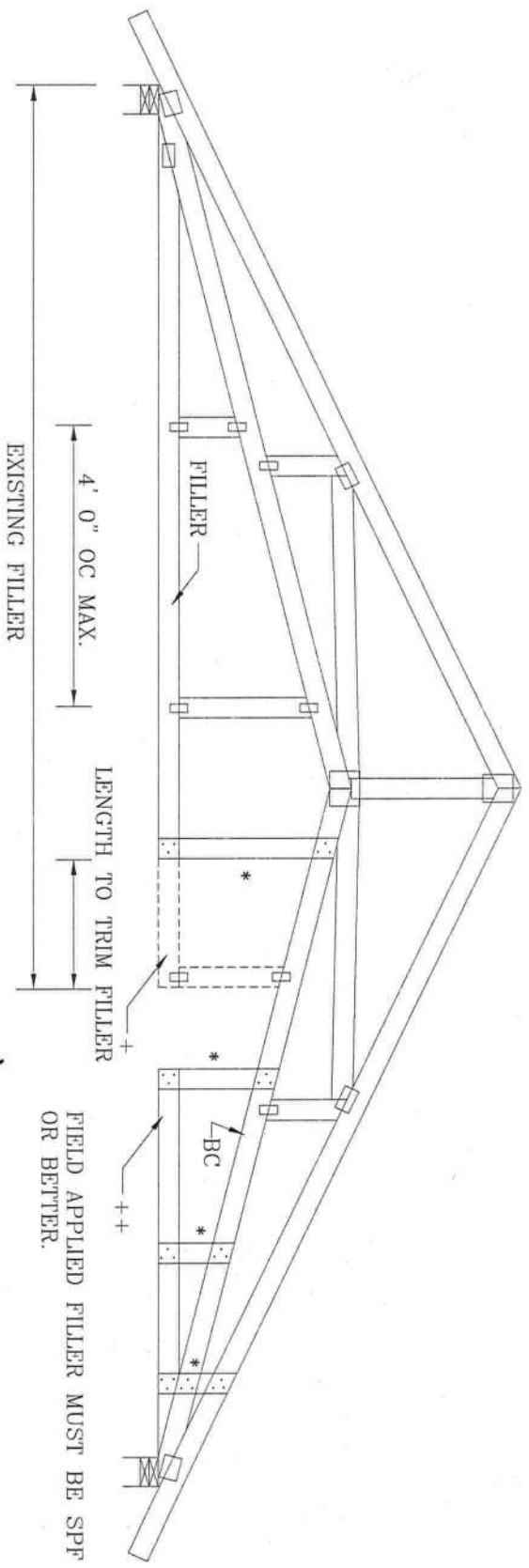
MAXIMUM BOTTOM CHORD LOAD IS 10 PSF.

+ BOTTOM CHORD FILLER TO BE REMOVED. SEE NOTE #3.

++ FIELD APPLIED FILLER.

* 2X4 STUD GRADE OR BETTER VERTICAL SCAB. ATTACH TO BOTTOM CHORD AND FILLER WITH (3) NAILS WITH A MIN. 0.131" DIA. X 3.0" LENGTH.

REFER TO ENGINEER'S SEALED DESIGN REFERENCE THIS DETAIL, FOR ALLOWABLE FILLER DIMENSIONS, PLACEMENT, AND WEBBING.

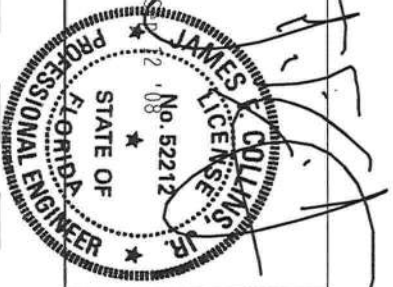


FIELD APPLIED FILLER MUST BE SPF #3 OR BETTER.

THIS DRAWING REPLACES DRAWING 962.767



ITV BUILDING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA



REF	BC FILLER REP.
DATE	2/23/07
DRWG	REPBCTLO207
-ENG	MLH/KAR

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING, AND BRACING. REFER TO BCSI BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 210 NORTH LEE ST., SUITE 312, ALEXANDRIA, VA. 22314, AND VITA CORD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITV, BCG CONNECTOR PLATES ARE MADE OF 2018/1664 (W/H/SS/X) ASTM A653 GRADE 40/60 (W/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, ALL TRUSS MEMBERS 16d OR 18d. UNLESS OTHERWISE LOCATED BY OTHER DESIGN, PER DESIGNER'S INSTRUCTIONS. ANY TRUSS DESIGNING AND/OR MANUFACTURING RESPONSIBILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

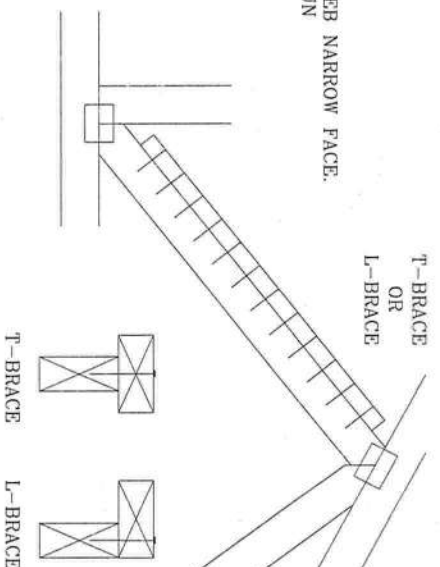
WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

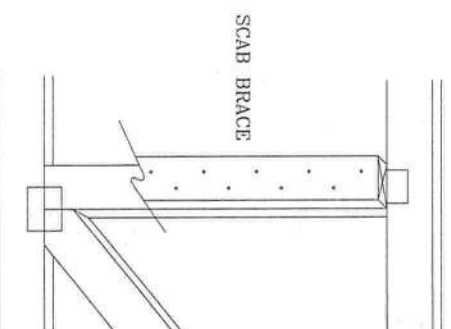
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE. ATTACH WITH 10d BOX OR GUN (0.128" x 3", MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d BOX OR GUN (0.128" x 3", MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



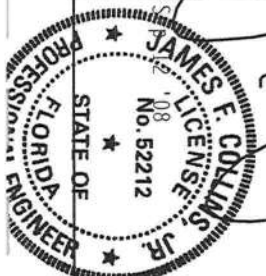
THIS DRAWING REPLACES DRAWING 579.640

ALPINE

ITV BUILDING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING, AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA 22314 AND VICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, HANSDEN, VI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY A/R/P/D) AND TPI. ITV, BCG CONNECTOR PLATES ARE MADE OF 2018/1664 C4H/55X0 ASTM A653 GRADE 40/60 C4H/4550 GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS PER DESIGN, POSITION PER DRAWING 16062. ANY DEVIATION FROM THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE ENGINEER. ENGINEERING RESPONSIBILITY SHALL BE THE TRUSS COMPONENT DESIGN SHOWN THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	BRCLBSUB0207
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

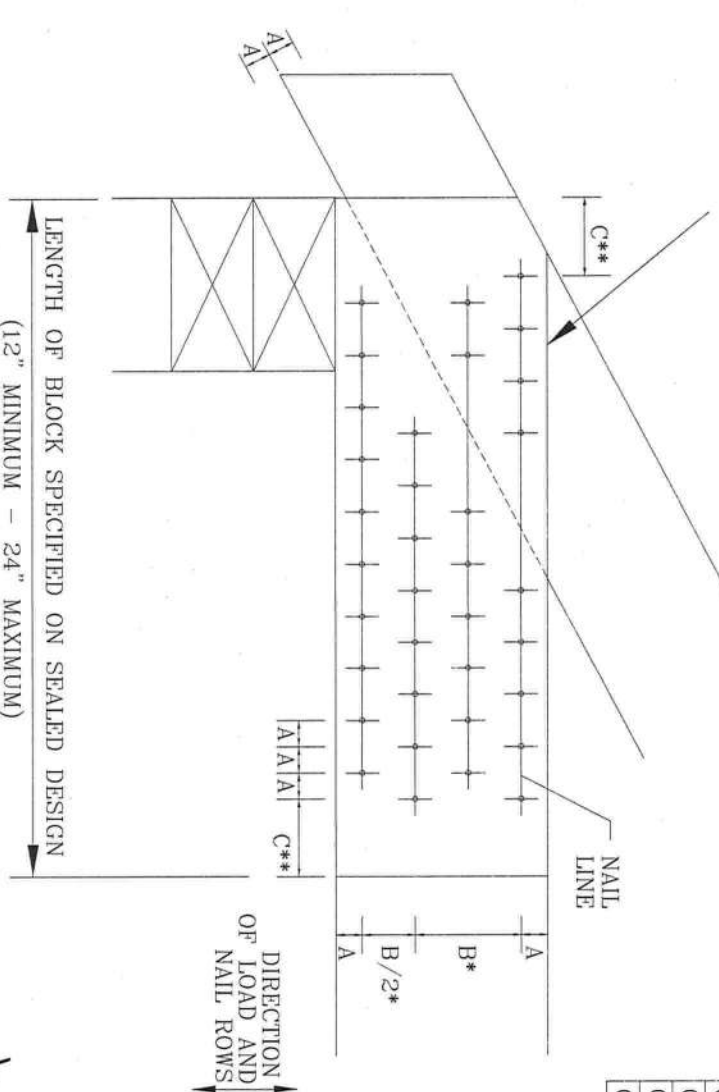
BEARING BLOCK NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BEARING BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

- A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
- B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
- C - END DISTANCE (15 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
 • SPACING MAY BE REDUCED BY 50%
 • SPACING MAY BE REDUCED BY 33%

BEARING BLOCK TO BE SAME SIZE AND SPECIES AS BOTTOM CHORD. BLOCKS MAY BE ANY GRADE WITHIN THE SPECIES, PROVIDED THE COMPRESSION PERPENDICULAR TO GRAIN VALUE (F_c -perp) IS AT LEAST THAT OF THE CHORD.



NAIL TYPE	CHORD SIZE				
	2X4	2X6	2X8	2X10	2X12
8d BOX (0.113"X 2.5", MIN)	3	6	9	12	15
10d BOX (0.128"X 3", MIN)	3	5	7	10	12
12d BOX (0.128"X 3.25", MIN)	3	5	7	10	12
16d BOX (0.135"X 3.5", MIN)	3	5	7	10	12
20d BOX (0.148"X 4", MIN)	2	4	5	6	8
8d COMMON (0.131"X 2.5", MIN)	3	5	7	10	12
10d COMMON (0.148"X 3", MIN)	2	4	6	8	10
12d COMMON (0.148"X 3.25", MIN)	2	4	6	8	10
16d COMMON (0.162"X 3.5", MIN)	2	4	6	8	10
GUN (0.120"X 2.5", MIN)	3	6	8	11	14
GUN (0.131"X 2.5", MIN)	3	5	7	10	12
GUN (0.120"X 3", MIN)	3	6	8	11	14
GUN (0.131"X 3", MIN)	3	5	7	10	12

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	
8d BOX (0.113"X 2.5", MIN)	3/4"	1 3/8"	1 3/4"	
10d BOX (0.128"X 3", MIN)	7/8"	1 5/8"	2"	
12d BOX (0.128"X 3.25", MIN)	7/8"	1 5/8"	2"	
16d BOX (0.135"X 3.5", MIN)	7/8"	1 5/8"	2 1/8"	
20d BOX (0.148"X 4", MIN)	1"	1 7/8"	2 1/4"	
8d COMMON (0.131"X 2.5", MIN)	7/8"	1 5/8"	2"	
10d COMMON (0.148"X 3", MIN)	1"	1 7/8"	2 1/4"	
12d COMMON (0.148"X 3.25", MIN)	1"	1 7/8"	2 1/4"	
16d COMMON (0.162"X 3.5", MIN)	1"	2"	2 1/2"	
GUN (0.120"X 2.5", MIN)	3/4"	1 1/2"	1 7/8"	
GUN (0.131"X 2.5", MIN)	7/8"	1 5/8"	2"	
GUN (0.120"X 3", MIN)	3/4"	1 1/2"	1 7/8"	
GUN (0.131"X 3", MIN)	7/8"	1 5/8"	2"	

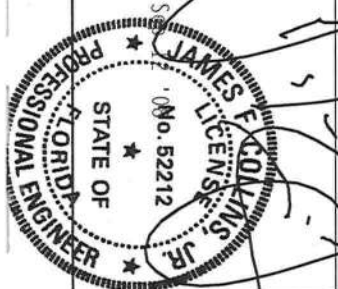
THIS DRAWING REPLACES DRAWING B139 AND CNBRGBLK0699



ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES IN DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPEC. BY ACPA AND TPI. ITW, BEG CONNECTOR PLATES ARE MADE OF 2018/1664 (V/H/SS) ASTM A653 GRADE 40/60 (V/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS PER DESIGN, POSITION PER DRAWING 160-12. ANY INSPECTION OF PLATES FOLLOWED BY CP SHALL BE PERFORMED BY A QUALIFIED INSPECTOR. THE INSPECTOR SHALL SIGN AND DATE THE INSPECTION REPORT. THE INSPECTOR'S SIGNATURE SHALL BE SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



REF	BEARING BLOCK
DATE	2/23/07
DRWG	CNBRGBLK0207
ENG	SJP/KAR

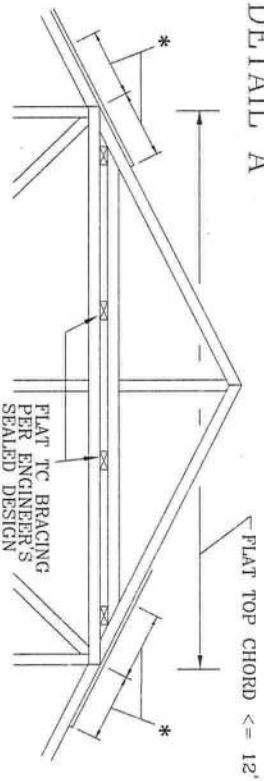
PIGGYBACK DETAIL

100 MPH WIND, 30.00 FT MEAN HGT. ASCE 7-02 OR ASCE 7-05, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

80 MPH WIND, 30.00 FT MEAN HGT, SBC, ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

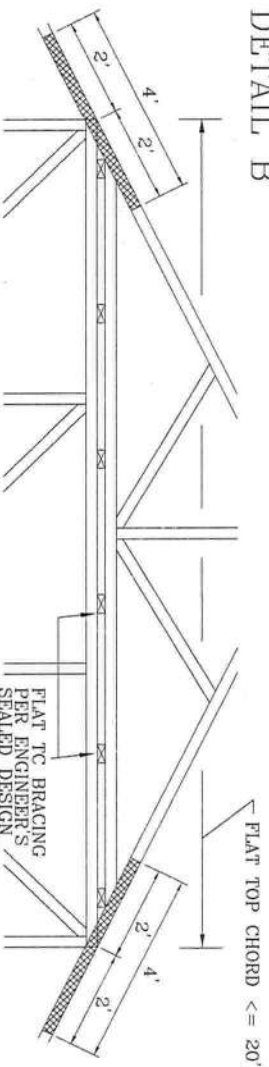
100 MPH WIND, 30.00 FT MEAN HGT. ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF. NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING OR PURLINS. PROVIDE DIAGONAL BRACING OR OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS.

DETAIL A



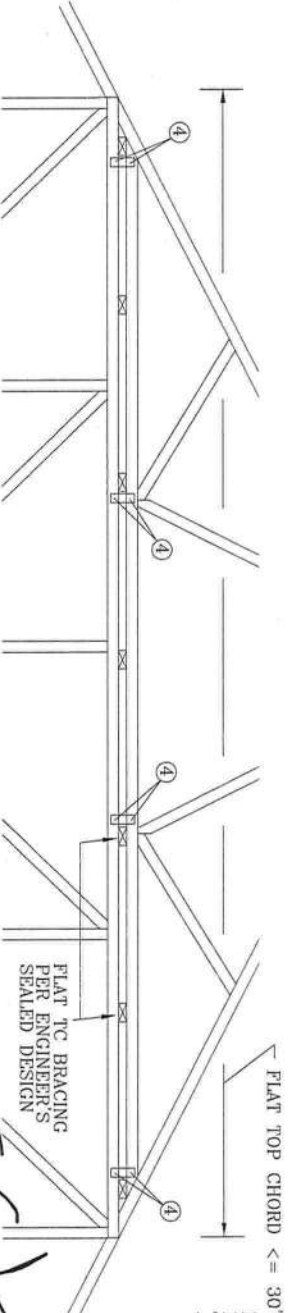
PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"x3") NAILS.
* 12" MIN RIGID SHEATHING OVERLAP WITH 8d COMMON (0.131"x2.5") OR CUN NAILS IN OVERLAP ZONE SPACED AT 4" O.C.

DETAIL B



PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"x3") NAILS AND SECURED WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY) ATTACHED WITH 10d COMMON NAILS AT 4" O.C.

DETAIL C



CAP TRUSS TOENAILED TO TOP CHORD BRACING AND SECURED WITH 3X8 TRULOX PLATES (EACH FACE) AT EACH END AND AT 1/3 POINTS. CIRCLED NUMBER INDICATES REQUIRED NUMBER OF 0.120" X 1.375" NAILS PER FACE. SEE DRAWING 1601L FOR TRULOX INFORMATION.

IN LIEU OF TRULOX CONNECTORS, ALPINE 62PB SPECIAL PIGGYBACK CONNECTORS MAY BE USED. SHOP APPLY TOOTHED PORTION FIELD ATTACH TO MATING TRUSS WITH (4) 0.120" X 0.375" NAILS MINIMUM EACH FACE.

(4) 8d COMMON NAILS (0.131"x2.5")
8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES. ATTACH WITH (8) 8d COMMON NAILS PER GUSSET. (4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

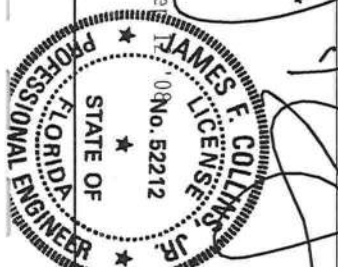
THIS DRAWING REPLACES DRAWINGS 561,670 & 961,660

ALPINE

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IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. TPI, BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING, DESIGNING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PAY) AND TPI. TPI, BCG CONNECTOR PLATES ARE MADE OF 2018/1604 C4H/SS/30 ASTH A653 GRADE 40/60 C4H/SS/30. GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS PERMANENTLY ATTACHED TO THE TRUSS. THE SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



TC LL	PSF	REF	PIGGYBACK
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	PIGGYBACK0207
BC LL	PSF	-ENG	DLJ/KAR
TOT. LD.	MAX 60 PSF		
DUR. FAC.	1.15		
SPACING	24.0"		

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

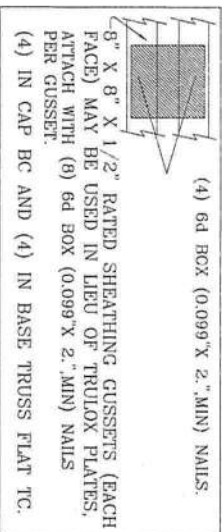
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

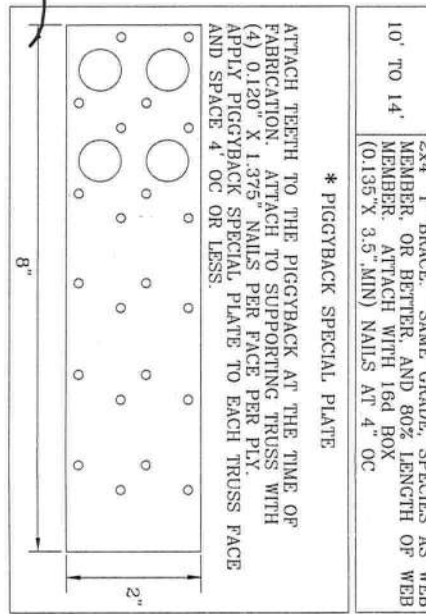
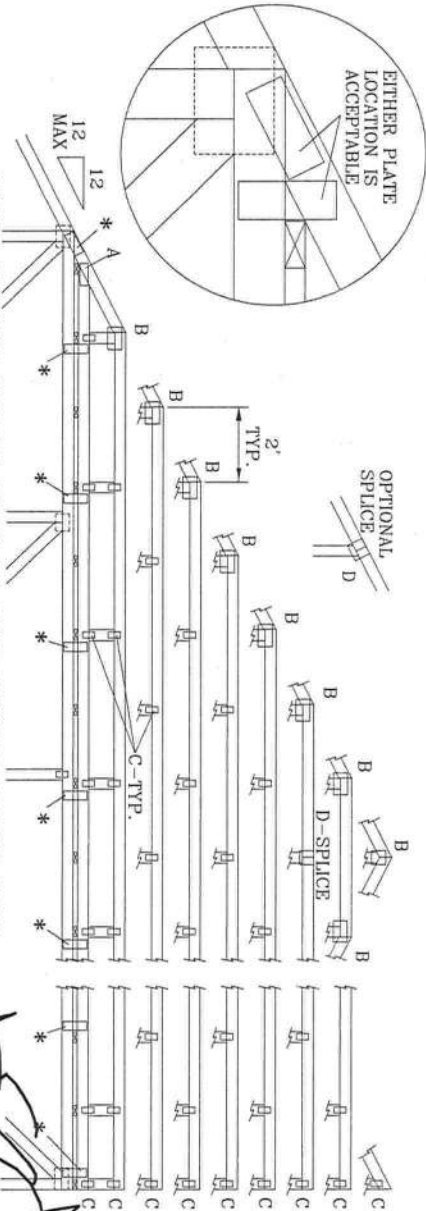
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

130 MPH WIND, 30' MEAN HGT, ASCE 7-98, ASCE 7-02 OR ASCE 7-06, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



*ATTACH PIGGYBACK WITH 3X8 TRUJOX OR ALPINE PIGGYBACK SPECIAL PLATE.

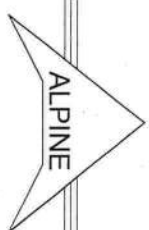
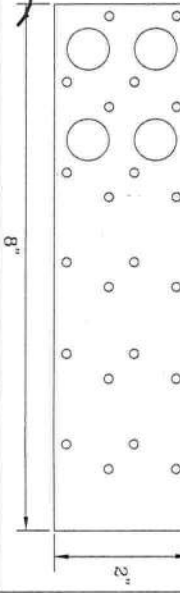


JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRUJOX AT 4' OC, ROTATED VERTICALLY			

WEB LENGTH	WEB BRACING CHART
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 6d BOX (0.113" X 2.5" MIN) NAILS AT 4" OC.
10' TO 14'	2X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135" X 3.5" MIN) NAILS AT 4" OC

* PIGGYBACK SPECIAL PLATE

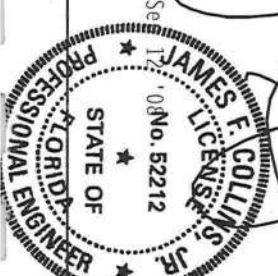
ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



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MAX LOADING	REF	PIGGYBACK
55 PSF AT 1.33 DUR. FAC.	DATE	2/23/07
50 PSF AT 1.25 DUR. FAC.	DRWG	PIGBACKB0207
47 PSF AT 1.15 DUR. FAC.	ENG	DLJ/KAR
SPACING	24.0"	