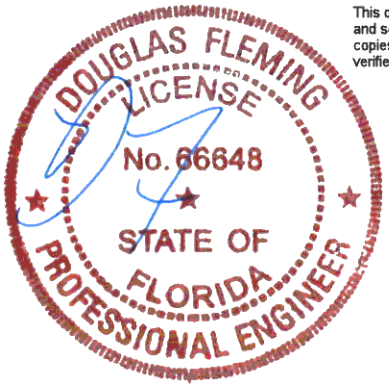

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ALPINE
AN ITW COMPANY

Alpine, an ITW Company
155 Harlem Ave
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Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

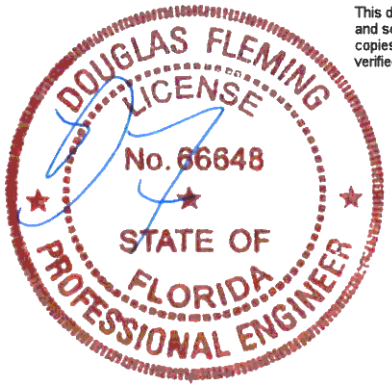
COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

Site Information:	**Page 1:**
Customer: W. B. Howland Company, Inc.	*Job Number:* 23-0295
Job Description: Stewart Residence	
Address:	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	*IntelliVIEW Version:* 21.02.00B through 22.01.00 *JRef #:* 1XWb2150004
Wind Standard: ASCE 7-16 *Wind Speed (mph):* 130 *Building Type:* Closed	*Design Loading (psf):* 40.00

This package contains general notes pages, 56 truss drawing(s) and 8 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	009.24.1215.39460	A01	2	009.24.1215.52600	A02
3	009.24.1216.14767	A03	4	009.24.1216.17347	A04
5	009.24.1216.45627	A05	6	009.24.1216.51317	A06
7	009.24.1216.56480	A07	8	009.24.1216.58973	A08
9	009.24.1217.00847	A09	10	009.24.1217.02810	A10
11	009.24.1217.04337	A11	12	009.24.1217.06117	A12
13	009.24.1311.34260	B01	14	009.24.1312.11293	B02
15	009.24.1312.16050	B03	16	009.24.1312.18340	B04
17	009.24.1312.21537	B05	18	009.24.1312.23530	B06
19	009.24.1312.31563	B07	20	009.24.1312.49617	B08
21	009.24.1312.55520	B09	22	009.24.1313.07027	B10
23	009.24.1313.10240	B11	24	009.24.1313.26027	B12
25	009.24.1313.29040	B13	26	009.24.1313.31247	B14
27	009.24.1313.38023	B15	28	009.24.1313.41860	B16
29	009.24.1313.44090	B17	30	009.24.1313.57520	B18
31	009.24.1313.59700	B19	32	009.24.1314.03797	B20
33	009.24.1314.08753	B21	34	009.24.1314.16520	B22
35	009.24.1314.39727	B23	36	009.24.1314.45250	C01
37	009.24.1314.47210	C02	38	009.24.1314.51837	C03
39	009.24.1314.54610	C04	40	009.24.1315.25863	C05
41	009.24.1315.30790	C06	42	009.24.1315.50860	C07
43	009.24.1315.59550	PB01	44	009.24.1316.03133	PB02
45	009.24.1316.05690	PB03	46	009.24.1316.09703	PB04
47	009.24.1316.11393	PB05	48	009.24.1316.14883	PB06
49	009.24.1316.28150	PB07	50	009.24.1316.30087	PB08



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Site Information:	Page 2:
Customer: W. B. Howland Company, Inc.	*Job Number:* 23-0295
Job Description: Stewart Residence	
Address:	

Item	Drawing Number	Truss
51	009.24.1316.32750	PB09
53	009.24.1316.39890	PB11
55	009.24.1316.51203	V02
57	A14030ENC160118	
59	GBLLETIN0118	
61	A14015ENC160118	
63	VAL180160118	

Item	Drawing Number	Truss
52	009.24.1316.37850	PB10
54	009.24.1316.42080	V01
56	009.24.1317.03290	V03
58	BRCLBSUB0119	
60	CNNAILSP1014	
62	PB160160118	
64	VALTN160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

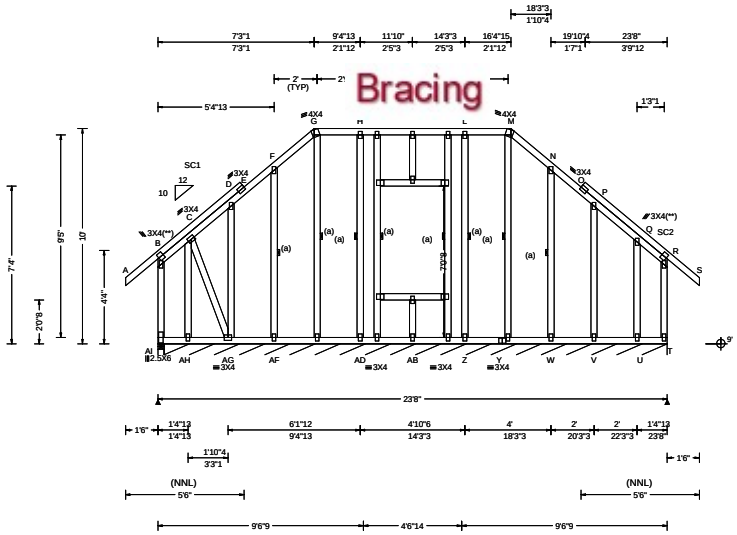
BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
FRT-PR = ProWood Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com.

SEQN: 16040 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** A01		Cust: R 215 JRef:1XWb2150004 T24 DrwNo: 009.24.1215.39460 GA / DF 01/09/2024	



Loading Criteria (psf)	
TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.54 ft
TCDL: 5.0 psf
BCDL: 2.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.005	J	999	240
VERT(CL): 0.010	J	999	180
HORZ(LL): 0.008	R	-	-
HORZ(TL): 0.015	R	-	-

Creep Factor: 2.0
Max TC CSI: 0.253
Max BC CSI: 0.063
Max Web CSI: 0.155

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Al	266	/-	/-	/203	/86	/114
T*	85	/-	/-	/49	/-	/-

Wind reactions based on MWFRS
Al Brg Wid = 3.5 Min Req = 1.5 (Truss)
T Brg Wid = 280 Min Req = -
Bearings Al & Al are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

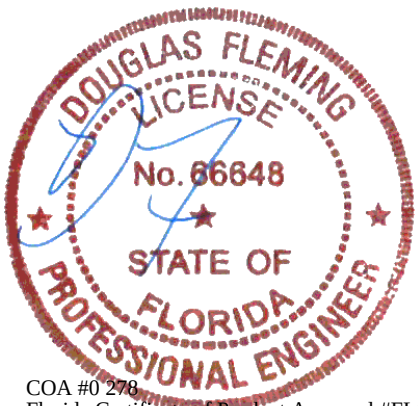
Wind loading based on both gable and hip roof types.

Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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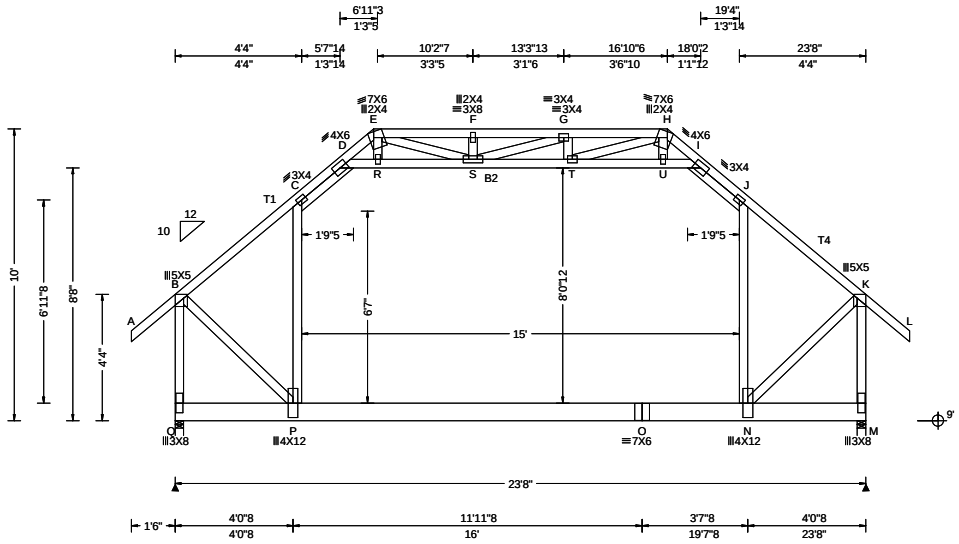
Glenview, IL 60025

SEQN: 24581 COMN FROM:	Ply: 1 Qty: 3	**Job Number:** 23-0295 Stewart Residence **Truss Label:** A02	Cust: R 215 JRef:1XWb2150004 T29 DrwNo: 009.24.1215.52600 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.54 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.087	N	999	240
VERT(CL): 0.192	N	999	180
HORZ(LL): 0.123	C	-	-
HORZ(TL): 0.271	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.507
Max BC CSI: 0.655
Max Web CSI: 0.706

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Q	2226	/-	/-	/618	/-	/163
M	2226	/-	/-	/618	/-	/-

Wind reactions based on MWFRS
Q Brg Wid = 3.5 Min Req = 1.8 (Truss)
M Brg Wid = 3.5 Min Req = 1.8 (Truss)
Bearings Q & M are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	192	- 1843	G - H	790	- 1093
C - D	314	- 1464	H - I	427	- 596
D - E	429	- 604	I - J	314	- 1464
E - F	808	- 1105	J - K	192	- 1844
F - G	808	- 1105			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
P - O	1304	- 59	O - N	1304	- 59

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - Q	217	- 2655	T - U	0	- 1244
B - P	1795	- 79	T - H	877	- 441
P - C	544	- 53	U - I	0	- 1284
D - R	0	- 1277	J - N	545	- 52
E - S	878	- 458	N - K	1795	- 79
R - S	0	- 1235	K - M	217	- 2656
S - T	26	- 680			

Lumber

Top chord: 2x4 SP #2; T1,T4 2x4 SP M-31;
Bot chord: 2x8 SP 2400f-2.0E; B2 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Attic room loading from 4-4-0 to 19-4-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

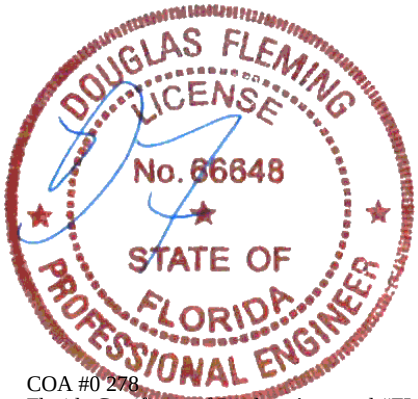
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.



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155 Harlem Ave
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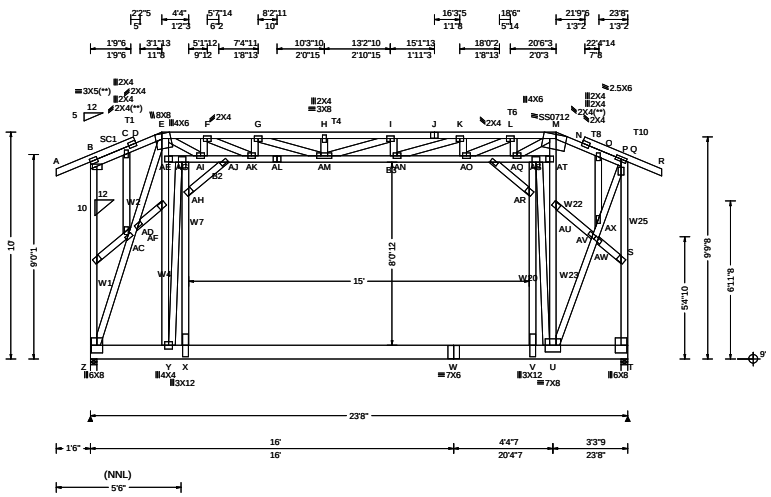
Glenview, IL 60025

SEQN: 17568 COMN FROM: Page 1 of 2	Ply: 3 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** A03	Cust: R 215 JRef:1XWb2150004 T15 DrwNo: 009.24.1216.14767 GA / DF 01/09/2024	

3 Complete Trusses Required



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.089 I 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.180 I 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.024 Q - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.049 Q - -
NCBCLL: 0.00	Mean Height: 16.29 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.117
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.613
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.737
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 4.50 ft	Plate Type(s):	
	GCpi: 0.18	WAVE, 18SS	VIEW Ver: 22.01.00.0314.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Z	6150	/-	/-	/548	/441	/39
T	6150	/-	/-	/548	/441	/-

Wind reactions based on MWFRS
Z Brg Wid = 3.5 Min Req = 1.7 (Truss)
T Brg Wid = 3.5 Min Req = 1.7 (Truss)
Bearings Z & T are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
E - F	86	- 1409	J - K	355	- 1870
F - G	246	- 1661	K - L	243	- 1596
G - H	361	- 1884	L - M	77	- 1283
H - I	361	- 1884	M - N	45	- 778
I - J	355	- 1870	N - Q	57	- 758

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
Z - Y	782	- 49	W - V	982	- 61
Y - X	940	- 58	V - U	942	- 58
X - W	982	- 61			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
Z -AC	154	- 2461	AN-AO	657	- 208
AC-AD	145	- 2393	AQ- M	617	- 53
AD- E	138	- 2331	AR- V	2138	- 83
E -AE	1723	- 91	AR-AS	2152	- 86
E -AI	910	- 68	AS- U	109	- 1866
AE-AF	1487	- 94	U -AV	2242	- 140
AF- Y	1526	- 98	AT-AU	24	- 393
Y -AG	94	- 1604	AU- U	22	- 394
AG-AH	2153	- 87	AV-AW	2224	- 139
AH- X	2127	- 83	AW-AX	2223	- 139
AI-AJ	499	- 65	AX- P	2015	- 122
AJ-AK	465	- 54	S - T	156	- 2327
AK-AL	717	- 211	P - S	153	- 2282
AL-AM	717	- 211	Q - P	57	- 449
AM-AN	906	- 310			

Lumber

Top chord: 2x4 SP #2; T1,T4,T6,T8,
T10 2x4 SP M-31;
Bot chord: 2x8 SP 2400f-2.0E; B2,B3 2x4 SP M-31;
Webs: 2x4 SP #3; W1,W2,W4,W7,W20,W22,W23,
W25 2x4 SP M-31;
Stack Chord: SC1 2x4 SP M-31;

Nailnote

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 6.25" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	-1.50 to	62 plf at	25.17
PLT: From	20 plf at	4.33 to	20 plf at	19.33
PLT: From	100 plf at	4.33 to	100 plf at	19.33
BC: From	4 plf at	-1.50 to	4 plf at	0.00
BC: From	360 plf at	0.00 to	360 plf at	23.67
BC: From	4 plf at	23.67 to	4 plf at	25.17

BC: 161 lb Conc. Load at 4.33,19.33

Plating Notes

All plates are 3X4 except as noted.

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

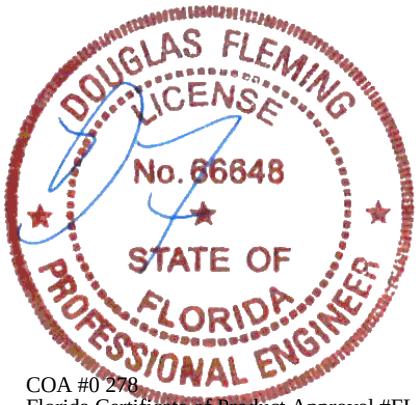
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.



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ALPINE
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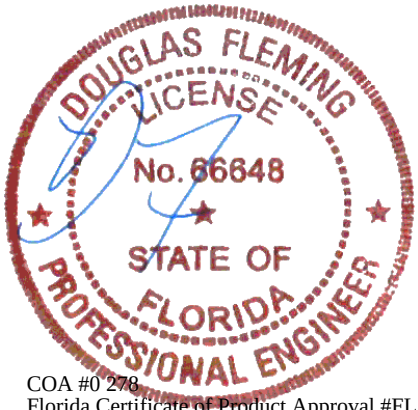
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Additional Notes

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)



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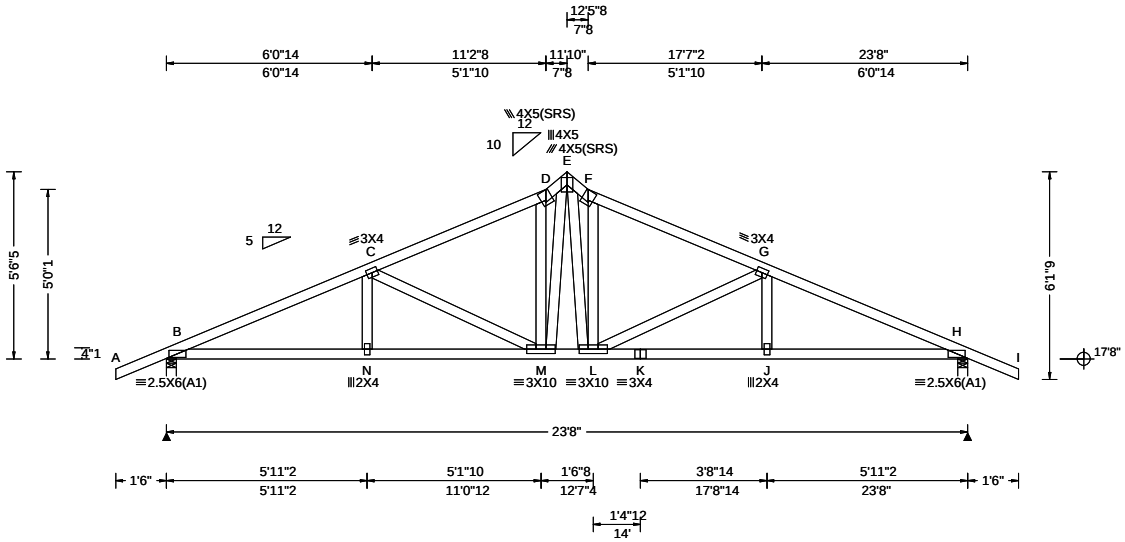
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24585 COMN FROM:	Ply: 1 Qty: 5	**Job Number:** 23-0295 Stewart Residence **Truss Label:** A04		Cust: R 215 JRef:1XWb2150004 T12 DrwNo: 009.24.1216.17347 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 20.29 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.090	F	999	240
VERT(CL): 0.181	F	999	180
HORZ(LL): 0.031	H	-	-
HORZ(TL): 0.062	H	-	-

Creep Factor: 2.0
Max TC CSI: 0.321
Max BC CSI: 0.454
Max Web CSI: 0.343

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1068	/-	/-	/578	/118	/101
H	1068	/-	/-	/578	/118	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
H Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	246	- 1898	E - F	308	- 1498
C - D	217	- 1367	F - G	217	- 1368
D - E	308	- 1498	G - H	246	- 1898

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - N	1695	- 155	L - K	1692	- 160
N - M	1692	- 157	K - J	1692	- 160
M - L	1092	- 31	J - H	1695	- 158

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - M	117	- 551	E - L	847	- 201
M - D	179	- 522	F - L	179	- 522
M - E	846	- 202	L - G	117	- 551

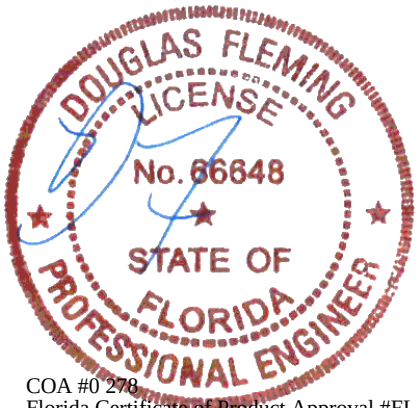
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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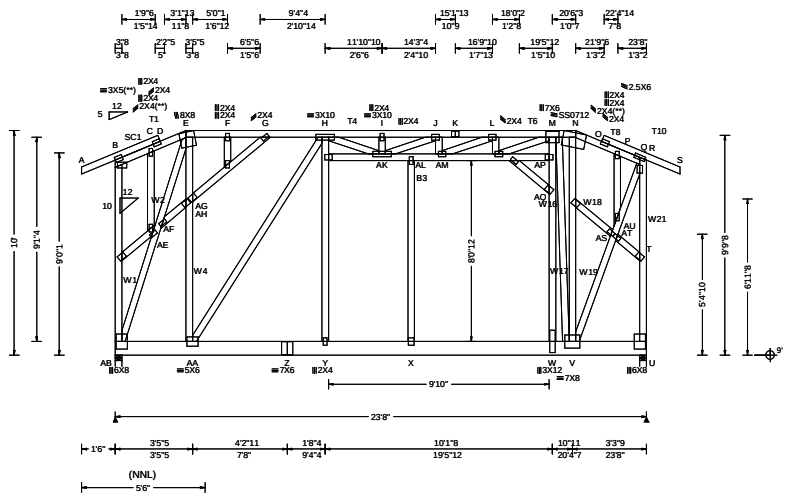
North Building, 4th Floor
Glenview, IL 60025

SEQN: 17554 COMN FROM: Page 1 of 2	Ply: 3 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** A05	Cust: R 215 JRef:1XWb2150004 T68 DrwNo: 009.24.1216.45627 GA / DF 01/09/2024	

3 Complete Trusses Required



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.250 AJ 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.517 AJ 549 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.295 AQ - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.610 AQ - -
NCBCLL: 0.00	Mean Height: 16.29 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.302
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.756
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.728
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 4.50 ft	Plate Type(s):	
	GCpi: 0.18	WAVE, 18SS	VIEW Ver: 22.01.00.0314.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AB	5676	/-	/-	/548	/441	/39
U	6004	/-	/-	/548	/441	/-

Wind reactions based on MWFRS
AB Brg Wid = 3.5 Min Req = 1.6 (Truss)
U Brg Wid = 3.5 Min Req = 1.7 (Truss)
Bearings AB & U are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
E - F	31	- 516	K - L	164	- 2106
F - G	31	- 516	L - M	128	- 1135
G - H	62	- 675	M - N	57	- 765
H - I	149	- 2184	N - O	55	- 754
I - J	149	- 2184	O - R	54	- 715
J - K	164	- 2106			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AB-AA	623	- 48	Y - X	1032	- 71
AA- Z	1047	- 72	X - W	1032	- 71
Z - Y	1047	- 72	W - V	989	- 68

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
AB-AE	145	- 1925	M -AP	1833	- 112
AE-AF	128	- 1798	M - V	168	- 2393
AF- E	113	- 1683	AP-AQ	1863	- 115
E -AG	1610	- 102	AQ- W	1655	- 100
AG-AH	1484	- 84	V -AS	2191	- 149
AH-AA	1557	- 93	AS-AT	2175	- 148
AA- H	40	- 705	AT-AU	2172	- 147
H -AK	1099	- 73	AU- Q	1956	- 134
AK-AL	1099	- 98	T - U	166	- 2253
AL- X	506	- 17	Q - T	163	- 2213
AL-AM	1099	- 98	R - Q	55	- 435
AM- L	986	- 59			

Lumber

Top chord: 2x4 SP #2; T1,T4,T6,T8,
T10 2x4 SP M-31;
Bot chord: 2x8 SP 2400f-2.0E; B3 2x4 SP #2;
Webs: 2x4 SP #3; W1,W2,W4,W16,W17,W18,W19,
W21 2x4 SP M-31;
Stack Chord: SC1 2x4 SP M-31;

Nailnote

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 6.25" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	-1.50 to	62 plf at	25.17
PLT: From	20 plf at	9.50 to	20 plf at	19.33
PLT: From	100 plf at	9.50 to	100 plf at	19.33
BC: From	4 plf at	-1.50 to	4 plf at	0.00
BC: From	360 plf at	0.00 to	360 plf at	23.67
BC: From	4 plf at	23.67 to	4 plf at	25.17

BC: 161 lb Conc. Load at 9.50,19.33

Plating Notes

All plates are 3X4 except as noted.

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

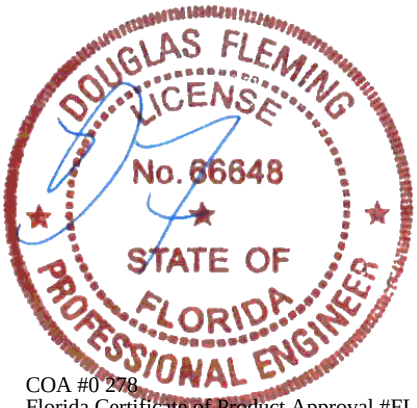
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.



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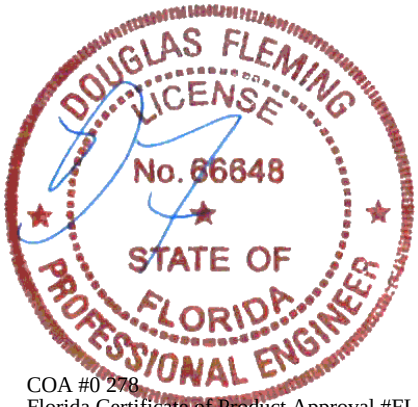
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Additional Notes

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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01/09/2024

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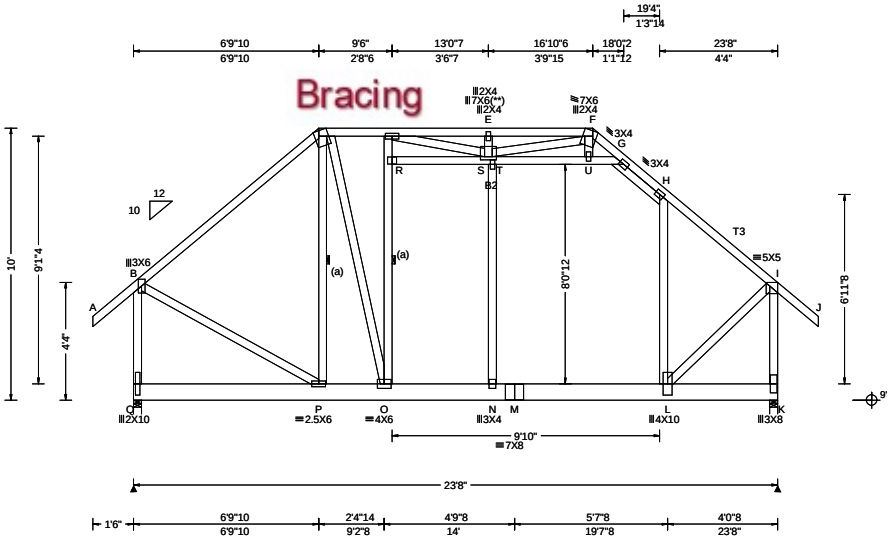
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24587 COMN FROM:	Ply: 1 Qty: 2	**Job Number:** 23-0295 Stewart Residence **Truss Label:** A06	Cust: R 215 JRef:1XWb2150004 T46 DrwNo: 009.24.1216.51317 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria

Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.54 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.099	D	999	240
VERT(CL): 0.225	R	999	180
HORZ(LL): 0.079	S	-	-
HORZ(TL): 0.191	S	-	-

Creep Factor: 2.0
Max TC CSI: 0.720
Max BC CSI: 0.332
Max Web CSI: 0.602

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Q	1721	/-	/-	/618	/-	/163
K	2071	/-	/-	/618	/-	/-

Wind reactions based on MWFRS
Q Brg Wid = 3.5 Min Req = 1.5 (Truss)
K Brg Wid = 3.5 Min Req = 1.7 (Truss)
Bearings Q & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	258	-1399	F - G	238	-1102
C - D	264	-1134	G - H	286	-1469
D - E	398	-2241	H - I	239	-1580
E - F	398	-2242			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
P - O	969	-74	N - M	1142	-86
O - N	1142	-86	M - L	1142	-86

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - Q	259	-1668	S - F	1516	-231
B - P	1099	-80	T - N	528	0
C - O	853	-181	T - U	89	-431
O - R	139	-452	U - G	93	-471
R - D	134	-408	L - I	1572	-117
D - S	1105	-132	I - K	282	-2288
S - T	89	-431			

Lumber

Top chord: 2x4 SP #2; T3 2x4 SP M-31;
Bot chord: 2x8 SP 2400f-2.0E; B2 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Attic room loading from 9-6-0 to 19-4-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

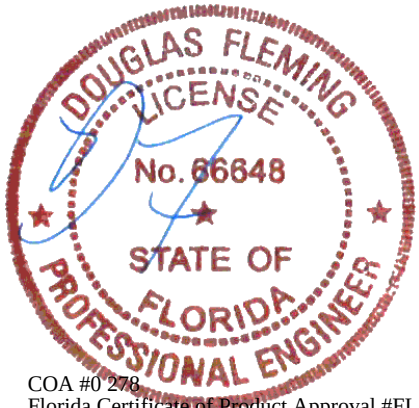
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

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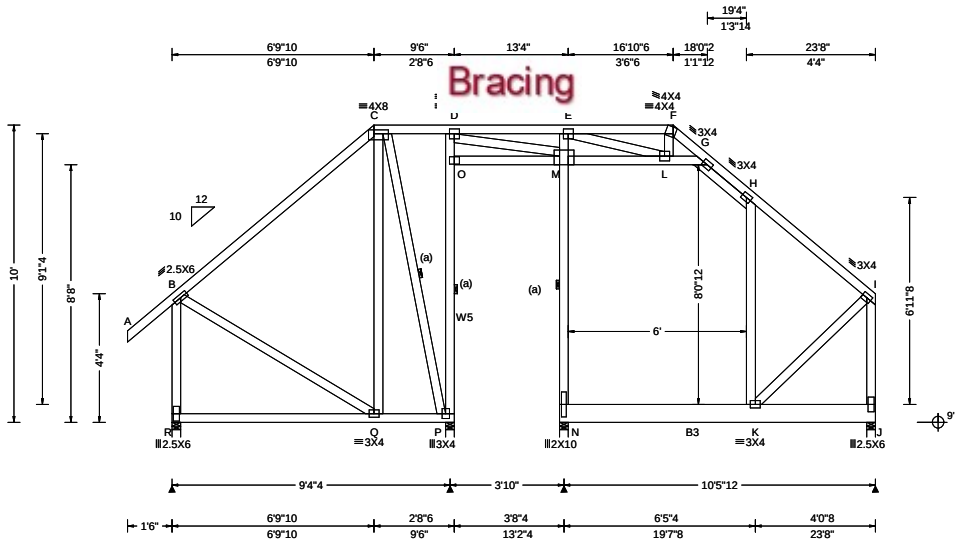
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24589 COMN FROM:	Ply: 1 Qty: 2	**Job Number:** 23-0295 Stewart Residence **Truss Label:** A07	Cust: R 215 JRef:1XWb2150004 T43 DrwNo: 009.24.1216.56480 GA / DF 01/09/2024



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria				
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in	loc	L/defl	L/#	
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL):	0.148	H	848	240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL):	0.318	H	394	180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL):	0.216	J	-	-
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL):	0.464	J	-	-
NCBCLL:	10.00	Mean Height: 15.54 ft	Building Code:	Creep Factor: 2.0				
Soffit:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI:	0.830			
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI:	0.444			
Spacing:	24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI:	0.864			
		C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)					
		Loc. from endwall: not in 9.00 ft	Plate Type(s):	VIEW Ver: 22.01.00.0314.20				
		GCpi: 0.18	WAVE					
		Wind Duration: 1.60						

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
R	513	/-	/-	/307	/6	/146
P	306	/-140	/-	/342	/83	/-
N	1949	/-	/-	/462	/-	/-
J	624	/-	/-	/269	/6	/-

Wind reactions based on MWFRS
R Brg Wid = 3.5 Min Req = 1.5 (Truss)
P Brg Wid = 3.5 Min Req = 1.5 (Truss)
N Brg Wid = 3.5 Min Req = 1.6 (Truss)
J Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings R, P, N, & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
D - E	1756	0	F - G	579	0
E - F	617	0			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
O - M	2	-600	L - G	60	-426
M - L	0	-1788			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - R	232	-453	M - E	82	-750
D - M	1	-1769	E - L	1399	0
M - N	51	-1188			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B3 2x8 SP 2400f-2.0E;
Webs: 2x4 SP #3; W5 2x4 SP M-31;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Attic room loading from 13-4-0 to 19-4-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

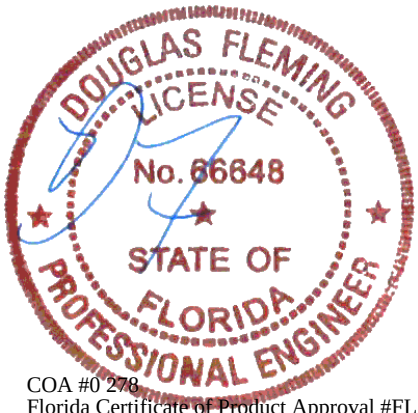
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.



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01/09/2024

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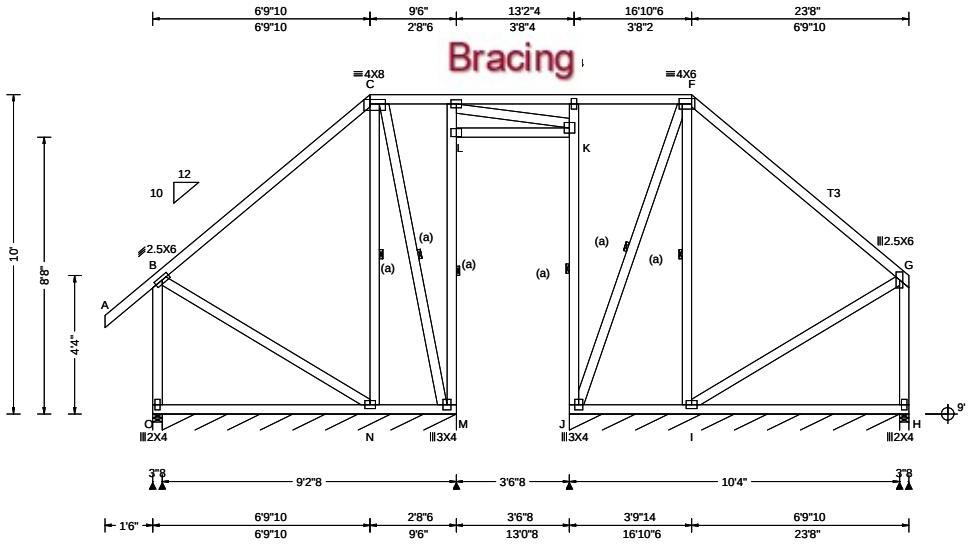
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 17578 COMN FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** A08	Cust: R 215 JRef:1XWb2150004 T54 DrwNo: 009.24.1216.58973 GA / DF 01/09/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.002 E 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.005 E 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 I - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.006 I - -
NCBCLL: 10.00	Mean Height: 15.54 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.817
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.366
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.321
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	VIEW Ver: 22.01.00.0314.20
	GCpi: 0.18	WAVE	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
O	425	/-	/-	/274	/32	/146
O*	70	/-	/-	/54	/4	/-
J*	71	/-	/-	/38	/-	/-
H	332	/-	/-	/230	/20	/-

Wind reactions based on MWFRS
O Brg Wid = 3.5 Min Req = 1.5 (Truss)
O Brg Wid = 110 Min Req = -
J Brg Wid = 123 Min Req = -
H Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings O, O, J, & H are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2; T3 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

Purlins

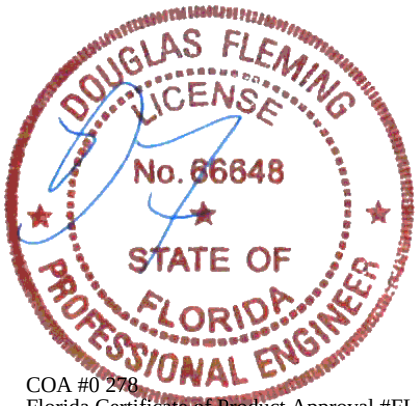
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278
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01/09/2024

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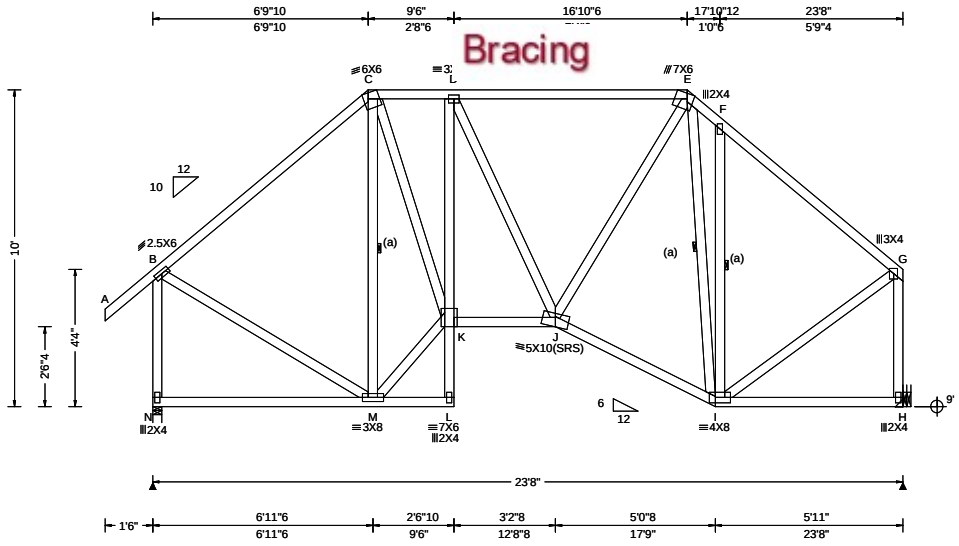
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 17564 COMN FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** A09	Cust: R 215 JRef:1XWb2150004 T38 DrwNo: 009.24.1217.00847 GA / DF 01/09/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.034 D 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.073 D 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.029 H - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.063 H - -
NCBCLL: 10.00	Mean Height: 15.54 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.678
Load Duration: 1.25	BCDL: 2.0 psf	TPI Std: 2014	Max BC CSI: 0.375
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.385
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 22.01.00.0314.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
N	1132	/-	/-	/651	/-	/150
H	1021	/-	/-	/576	/-	/-

Wind reactions based on MWFRS
N Brg Wid = 3.5 Min Req = 1.5 (Truss)
H Brg Wid = - Min Req = -
Bearing N is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	0	-840	E - F	118	-714
C - D	0	-865	F - G	0	-808
D - E	0	-811			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
K - J	876	0	J - I	649	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - N	0	-1073	M - K	795	0
B - M	603	0	J - E	496	0
C - M	0	-759	I - G	640	0
C - K	953	0	G - H	0	-970

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

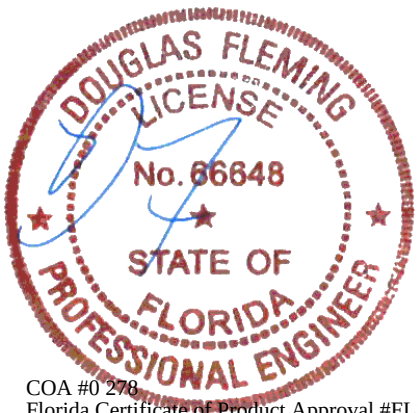
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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01/09/2024

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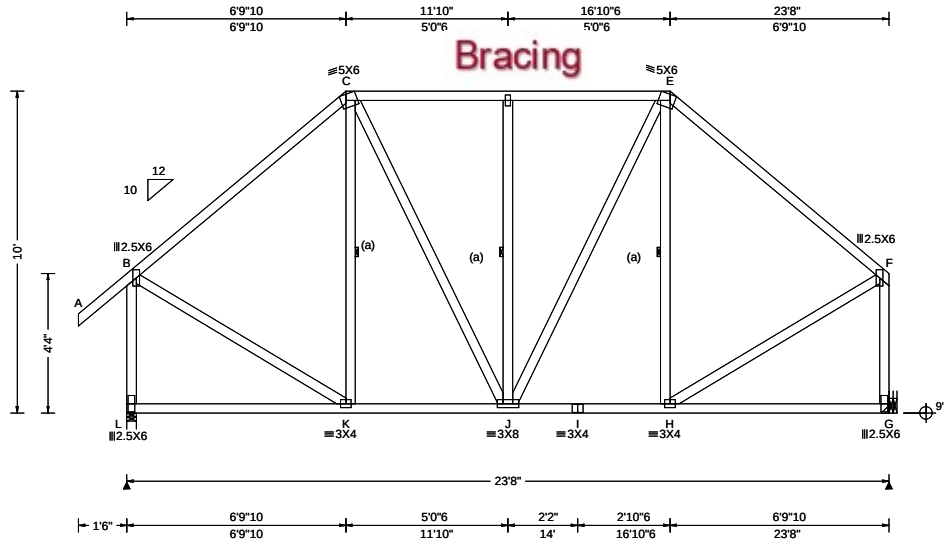
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24593 COMN FROM:	Ply: 1 Qty: 9	**Job Number:** 23-0295 Stewart Residence **Truss Label:** A10	Cust: R 215 JRef:1XWb2150004 T37 DrwNo: 009.24.1217.02810 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.54 ft
TCDL: 5.0 psf
BCDL: 2.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.025	D	999	240
VERT(CL): 0.047	D	999	180
HORZ(LL): 0.006	C	-	-
HORZ(TL): 0.010	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.720
Max BC CSI: 0.406
Max Web CSI: 0.433

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

		Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh		/ Rw	/ U	/ RL
L	1218	/-	/-		/516	/-	/150
G	1104	/-	/-		/438	/-	/-

Wind reactions based on MWFRS
L Brg Wid = 3.5 Min Req = 1.5 (Truss)
G Brg Wid = - Min Req = -
Bearing L is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	50	-918	D - E	65	-710
C - D	65	-710	E - F	37	-917

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
K - J	601	-8	I - H	606	0
J - I	606	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - L	34	-1163	H - F	681	0
B - K	680	0	F - G	3	-1050

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

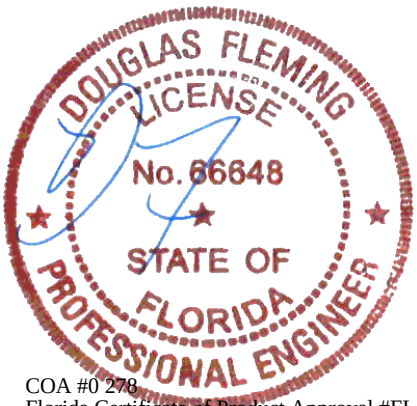
Bracing
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
(J) Hanger Support Required, by others

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



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01/09/2024

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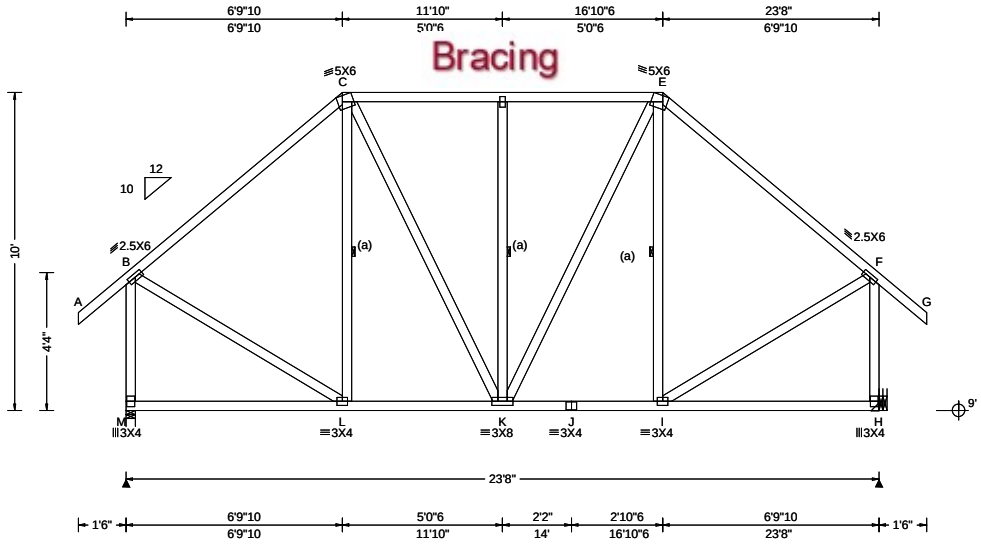
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24595 COMN FROM:	Ply: 1 Qty: 2	**Job Number:** 23-0295 Stewart Residence **Truss Label:** A11	Cust: R 215 JRef:1XWb2150004 T34 DrwNo: 009.24.1217.04337 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.54 ft
TCDL: 5.0 psf
BCDL: 2.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.025	D	999	240
VERT(CL): 0.048	D	999	180
HORZ(LL): 0.006	C	-	-
HORZ(TL): 0.011	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.606
Max BC CSI: 0.408
Max Web CSI: 0.405

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
M	1233	/-	/-	/644	/-	/166
H	1233	/-	/-	/644	/-	/-

Wind reactions based on MWFRS
M Brg Wid = 3.5 Min Req = 1.5 (Truss)
H Brg Wid = - Min Req = -
Bearing M is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	36	- 932	D - E	52	- 725
C - D	52	- 725	E - F	36	- 931

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
L - K	612	0	J - I	611	0
K - J	611	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - M	36	- 1179	I - F	692	0
B - L	693	0	F - H	36	- 1178

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

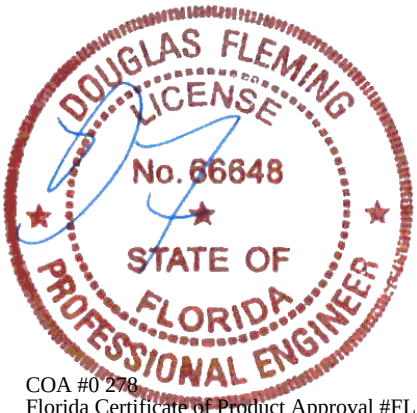
Bracing
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
(J) Hanger Support Required, by others

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0 278
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01/09/2024

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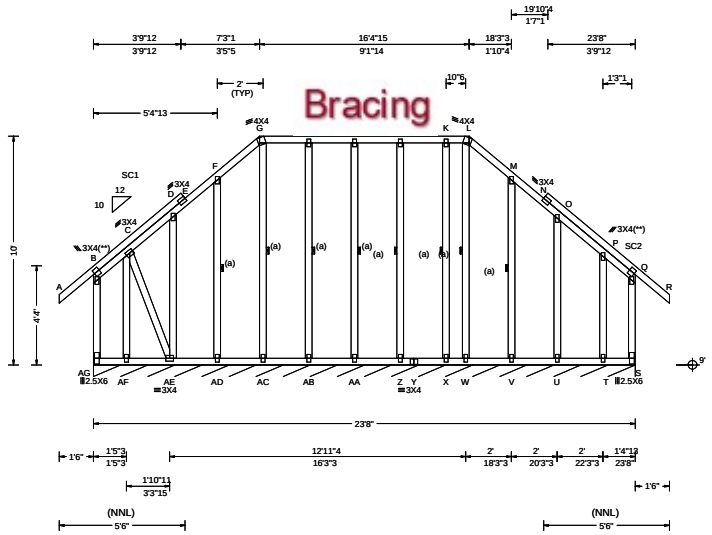
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24597 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** A12	Cust: R 215 JRef:1XWb2150004 T22 DrwNo: 009.24.1217.06117 GA / DF 01/09/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.003 N 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.007 N 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.014 N - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.024 N - -
NCBCLL: 10.00	Mean Height: 15.54 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.253
Load Duration: 1.25	BCDL: 2.0 psf	TPI Std: 2014	Max BC CSI: 0.038
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.230
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 22.01.00.0314.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
S*	95	/-	/-	/51	/-	/7

Wind reactions based on MWFRS
S Brg Wid = 284 Min Req = -
Bearing AG is a rigid surface.
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
AF- C	499	-495	C -AE	405	-405

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

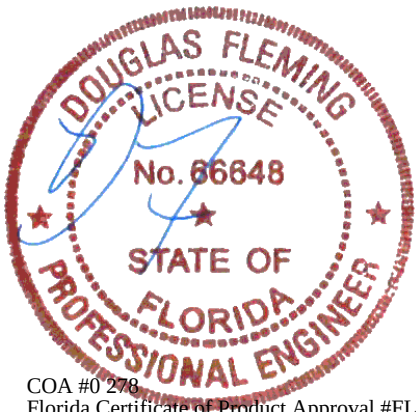
End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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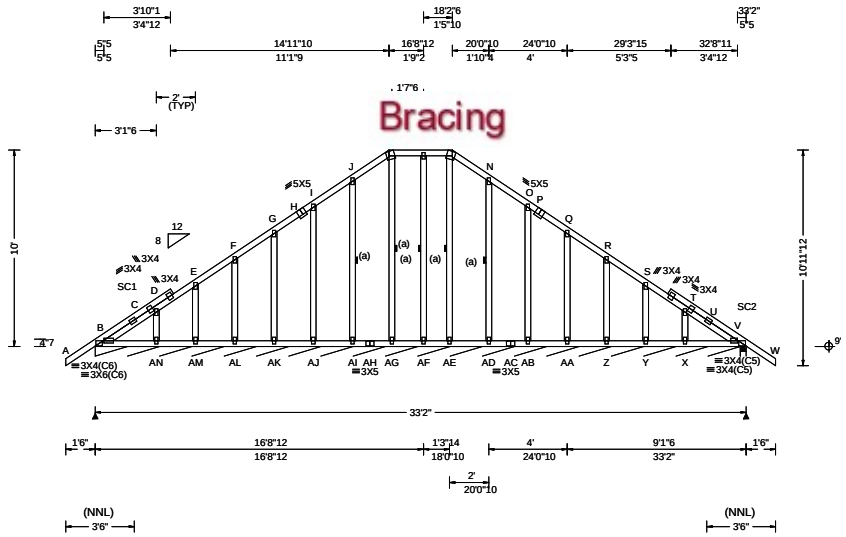
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24599 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B01	Cust: R 215 JRef:1XWb2150004 T21 DrwNo: 009.24.1311.34260 GA / DF 01/09/2024



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in	loc L/defl L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL):	0.001 C 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL):	0.003 C 999 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL):	0.003 R - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL):	0.004 R - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	
Soffit:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI:	0.189
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI:	0.077
Spacing:	24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI:	0.126
		C&C Dist a: 3.32 ft	FT/RT:20(0)/10(0)		
		Loc. from endwall: Any	Plate Type(s):		
		GCpi: 0.18	WAVE	VIEW Ver: 22.01.00.0314.20	
		Wind Duration: 1.60			

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	83	/-	/-	/44	/-	/6
V	280	/-	/-	/163	/0	/-

Wind reactions based on MWFRS
B Brg Wid = 394 Min Req = -
V Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & V are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

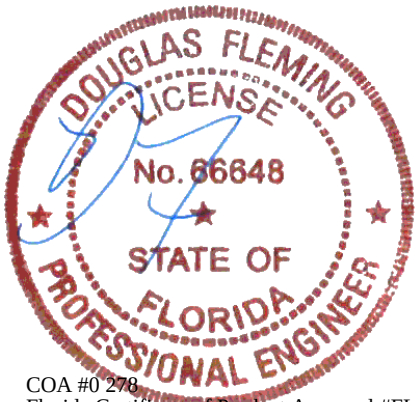
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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01/09/2024

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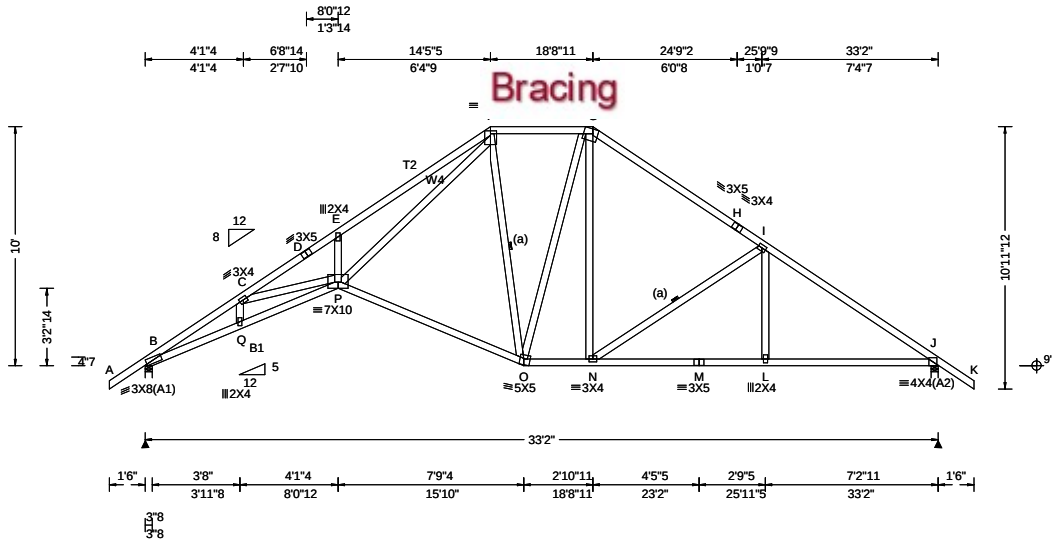
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24601 COMN FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B02		Cust: R 215 JRef:1XWb2150004 T20 DrwNo: 009.24.1312.11293 GA / DF 01/09/2024	



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.355 P 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.740 P 532 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.239 J - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.499 J - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.746
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.655
Spacing:	24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.847
		C&C Dist a: 3.32 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: Any	Plate Type(s):	
		GCpi: 0.18	WAVE	VIEW Ver: 22.01.00.0314.20
		Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1517	/-	/-	/879	/39	/221
J	1503	/-	/-	/872	/43	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
J Brg Wid = 3.5 Min Req = 1.8 (Truss)
Bearings B & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	401	- 4600	F - G	295	- 1185
C - D	397	- 4725	G - H	303	- 1509
D - E	405	- 4629	H - I	263	- 1549
E - F	544	- 4787	I - J	271	- 2088

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Q	3985	- 263	N - M	1637	- 107
Q - P	4083	- 270	M - L	1637	- 107
P - O	1378	0	L - J	1639	- 106
O - N	1174	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - P	183	- 386	G - N	415	- 38
P - F	3754	- 281	N - I	150	- 561
F - O	109	- 419			

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3; W4 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

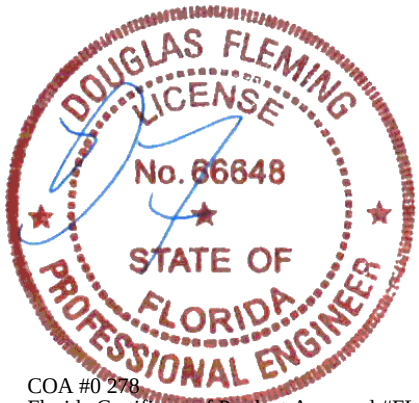
Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA #0 278
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01/09/2024

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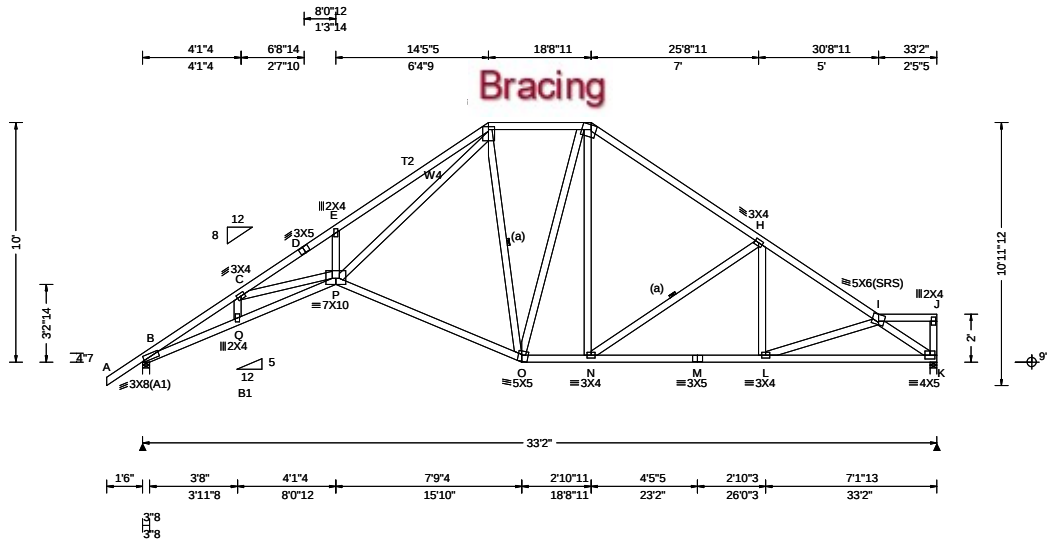
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24603 COMN FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B03	Cust: R 215 JRef:1XWb2150004 T59 DrwNo: 009.24.1312.16050 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.32 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.356	P	999	240
VERT(CL): 0.749	P	529	180
HORZ(LL): 0.241	K	-	-
HORZ(TL): 0.506	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.755
Max BC CSI: 0.717
Max Web CSI: 0.853

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1526	/-	/-	/884	/39	/194
K	1391	/-	/-	/762	/37	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
K Brg Wid = 3.5 Min Req = 1.6 (Truss)
Bearings B & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	312	- 4633	F - G	221	- 1200
C - D	321	- 4764	G - H	227	- 1565
D - E	330	- 4668	H - I	180	- 2078
E - F	445	- 4825			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Q	4014	- 273	N - M	1673	- 108
Q - P	4113	- 277	M - L	1673	- 108
P - O	1393	- 49	L - K	1816	- 174
O - N	1191	- 29			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - P	139	- 386	G - N	424	- 13
P - F	3779	- 251	N - H	97	- 588
F - O	98	- 414	I - K	215	- 2194

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3; W4 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

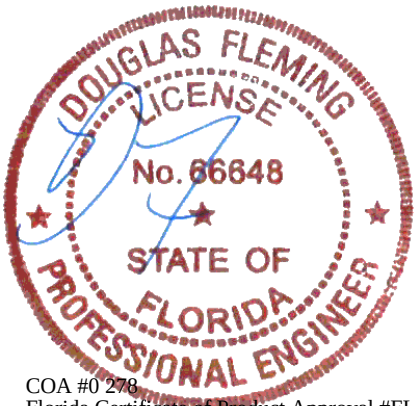
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

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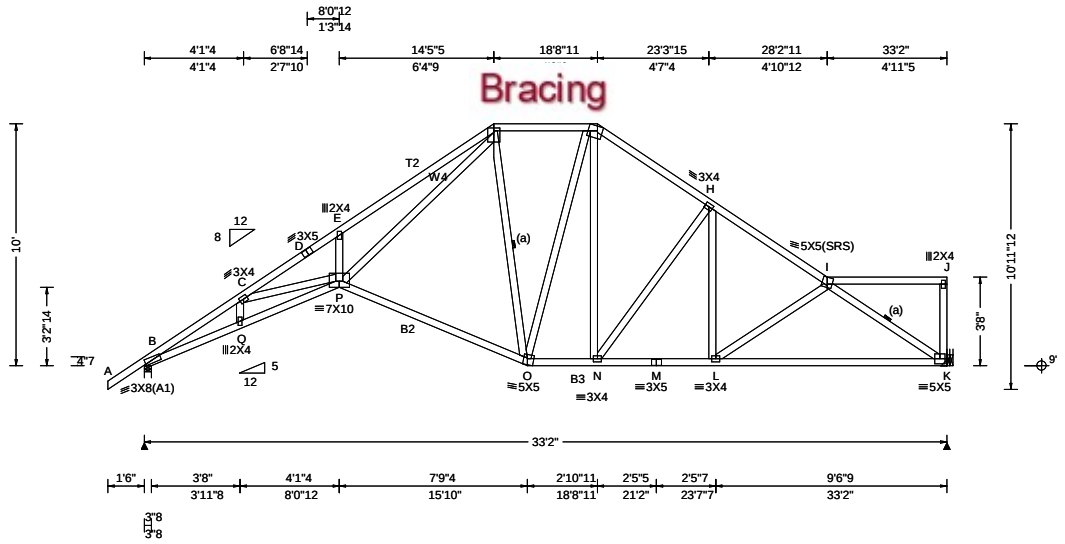
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24605 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B04		Cust: R 215 JRef:1XWb2150004 T62 DrwNo: 009.24.1312.18340 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.32 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.354	P	999	240
VERT(CL): 0.745	P	531	180
HORZ(LL): 0.235	K	-	-
HORZ(TL): 0.495	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.754
Max BC CSI: 0.650
Max Web CSI: 0.851

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1526	/-	/-	/886	/36	/192
K	1391	/-	/-	/732	/45	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
K Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	380	- 4634	F - G	224	- 1198
C - D	404	- 4761	G - H	248	- 1506
D - E	412	- 4664	H - I	200	- 1884
E - F	526	- 4819			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Q	4015	- 377	N - M	1503	- 121
Q - P	4114	- 383	M - L	1503	- 121
P - O	1395	- 102	L - K	1700	- 206
O - N	1174	- 74			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - P	137	- 381	N - H	81	- 566
P - F	3771	- 338	H - L	436	0
F - O	93	- 444	I - K	249	- 2033
G - N	459	- 60			

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x4 SP M-31; B2,B3 2x4 SP #2;
Webs: 2x4 SP #3; W4 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

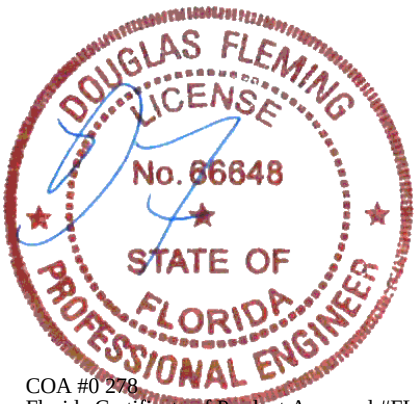
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

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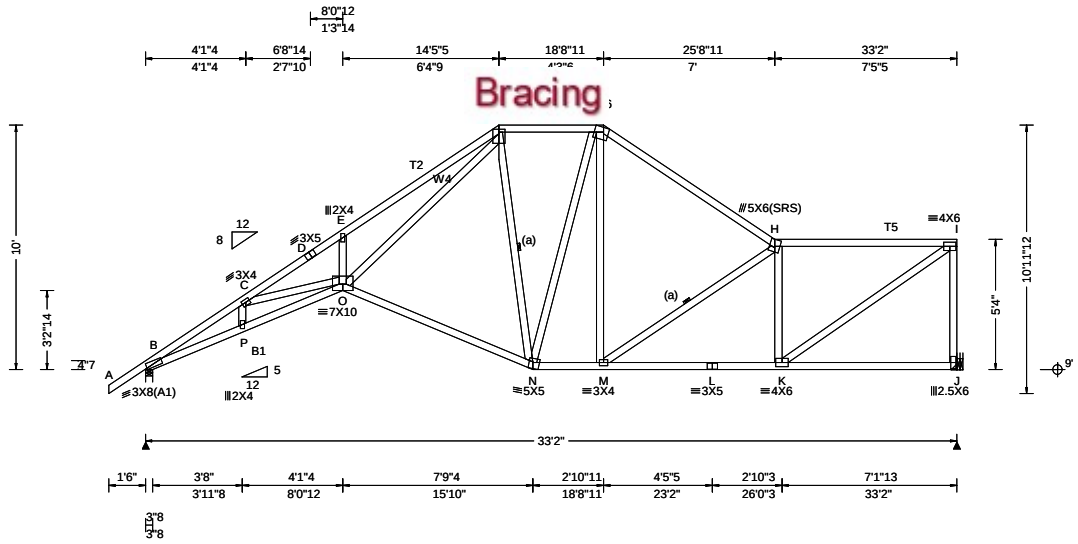
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24607 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B05		Cust: R 215 JRef:1XWb2150004 T55 DrwNo: 009.24.1312.21537 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.32 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.357	O	999	240
VERT(CL): 0.751	O	527	180
HORZ(LL): 0.230	K	-	-
HORZ(TL): 0.485	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.755
Max BC CSI: 0.711
Max Web CSI: 0.853

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1526	/-	/-	/890	/32	/189
J	1391	/-	/-	/721	/54	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
J Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	456	-4633	F - G	232	-1199
C - D	495	-4764	G - H	246	-1565
D - E	503	-4667	H - I	220	-1617
E - F	618	-4824			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - P	4014	-487	N - M	1189	-123
P - O	4113	-495	M - L	1673	-233
O - N	1393	-156	L - K	1673	-233

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - O	139	-385	G - M	424	-36
O - F	3778	-433	H - K	241	-974
F - N	99	-420	K - I	1978	-268
M - H	136	-596	I - J	261	-1334

Lumber
Top chord: 2x4 SP #2; T2,T5 2x4 SP M-31;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3; W4 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

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01/09/2024

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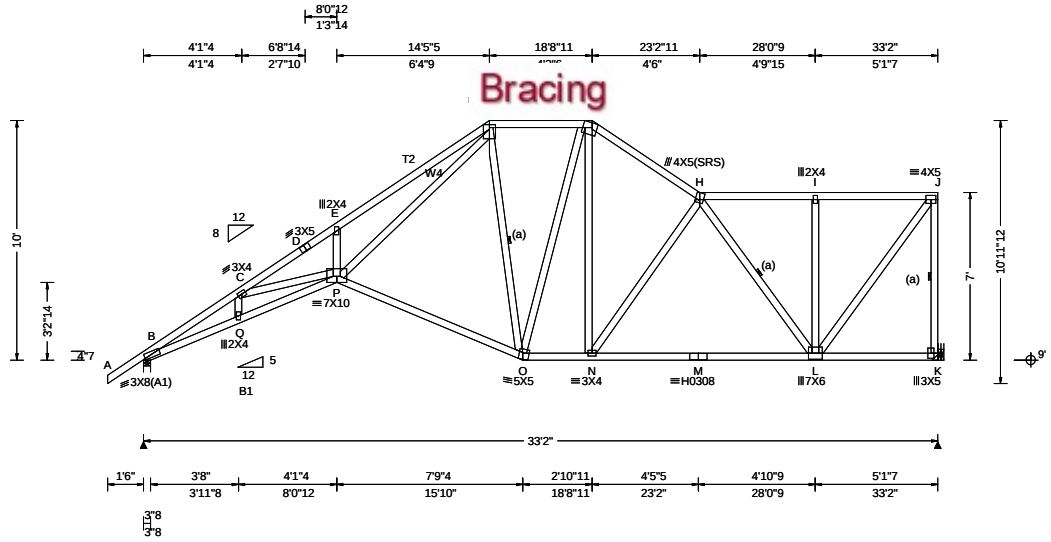
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24609 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B06		Cust: R 215 JRef:1XWb2150004 T80 DrwNo: 009.24.1312.23530 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.32 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.353	P	999	240
VERT(CL): 0.743	P	533	180
HORZ(LL): 0.231	L	-	-
HORZ(TL): 0.487	L	-	-

Creep Factor: 2.0
Max TC CSI: 0.754
Max BC CSI: 0.790
Max Web CSI: 0.852

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1526	/-	/-	/896	/26	/186
K	1391	/-	/-	/728	/65	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
K Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	537	-4634	F - G	241	-1197
C - D	591	-4763	G - H	262	-1507
D - E	599	-4667	H - I	170	-940
E - F	713	-4822	I - J	170	-940

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Q	4014	-602	O - N	1188	-169
Q - P	4114	-612	N - M	1442	-251
P - O	1395	-214	M - L	1442	-251

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - P	138	-381	N - H	148	-457
P - F	3775	-529	H - L	139	-859
F - O	114	-443	L - J	1549	-280
G - N	502	-52	J - K	291	-1363

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3; W4 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

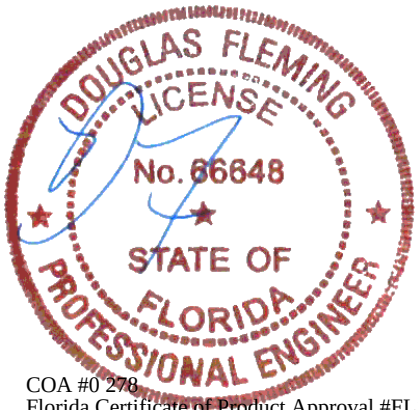
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

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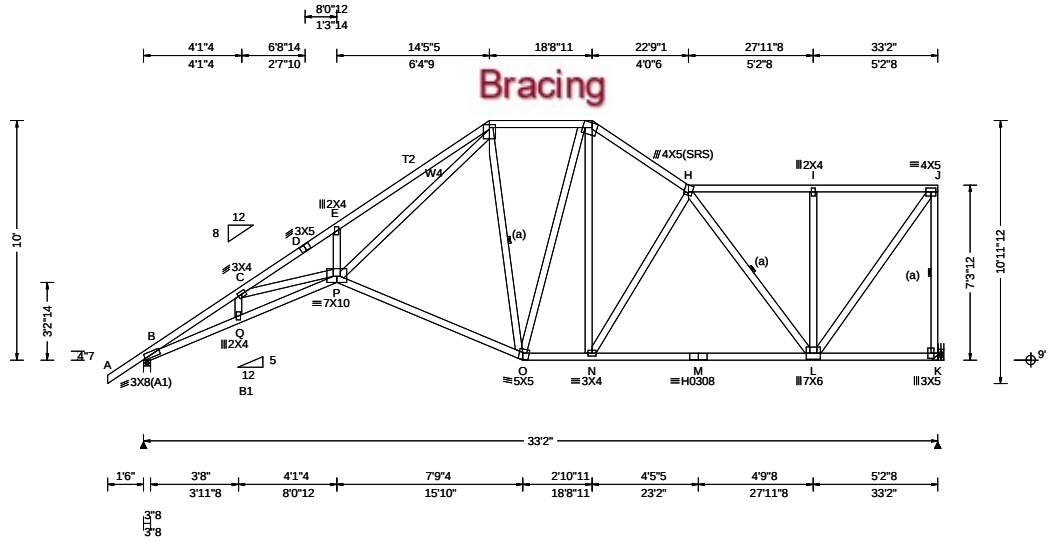
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24611 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B07		Cust: R 215 JRef:1XWb2150004 T57 DrwNo: 009.24.1312.31563 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.32 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.353	P	999	240
VERT(CL): 0.743	P	533	180
HORZ(LL): 0.231	L	-	-
HORZ(TL): 0.486	L	-	-

Creep Factor: 2.0
Max TC CSI: 0.754
Max BC CSI: 0.777
Max Web CSI: 0.852

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1526	/-	/-	/897	/24	/186
K	1391	/-	/-	/729	/67	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
K Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	552	- 4634	F - G	242	- 1197
C - D	609	- 4763	G - H	268	- 1499
D - E	617	- 4666	H - I	172	- 913
E - F	731	- 4821	I - J	172	- 912

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Q	4014	- 624	O - N	1186	- 179
Q - P	4114	- 634	N - M	1414	- 255
P - O	1395	- 225	M - L	1414	- 255

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - P	138	- 380	H - L	139	- 841
P - F	3774	- 547	I - L	206	- 387
F - O	119	- 447	L - J	1532	- 289
G - N	517	- 66	J - K	300	- 1361
N - H	156	- 458			

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3; W4 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

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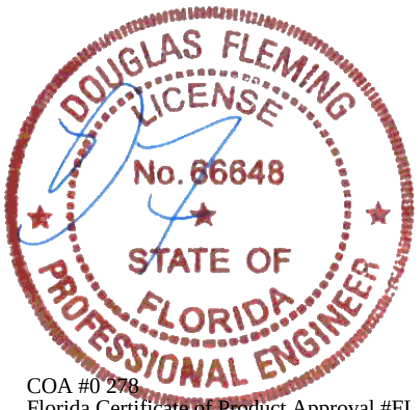

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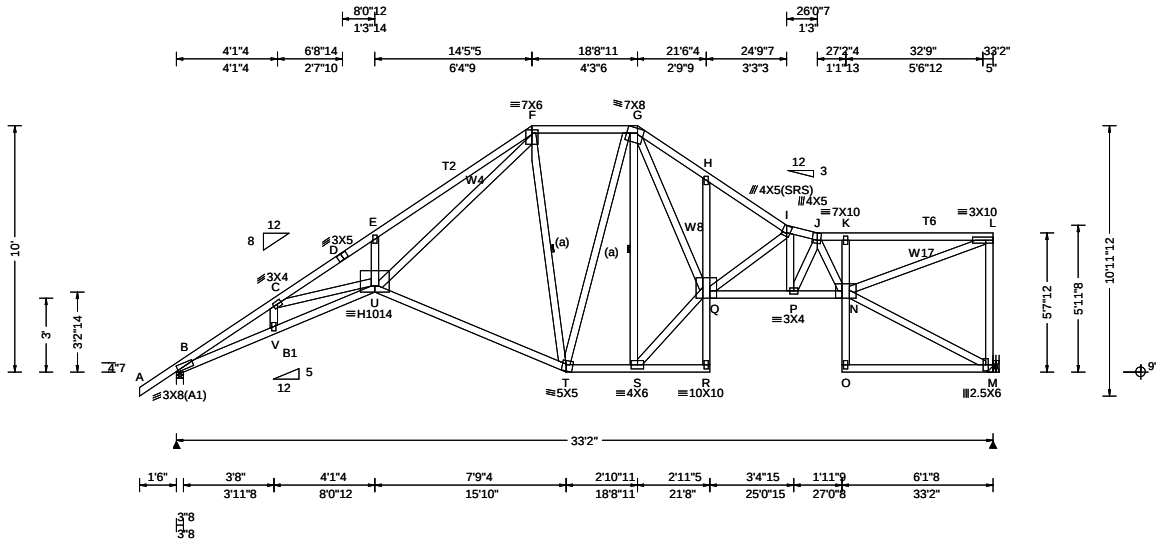
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24613 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B08		Cust: R 215 JRef:1XWb2150004 T47 DrwNo: 009.24.1312.49617 GA / DF 01/09/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.406 U 977 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.840 U 471 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.369 M - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.765 M - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.743
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.759
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.845
	C&C Dist a: 3.32 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	VIEW Ver: 22.01.00.0314.20
	GCpi: 0.18	WAVE, HS	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1515	/-	/-	/890	/-	/185
M	1359	/-	/-	/719	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
M Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	454	- 4593	G - H	434	- 2750
C - D	494	- 4716	H - I	378	- 2767
D - E	502	- 4620	I - J	448	- 3324
E - F	616	- 4775	J - K	494	- 3117
F - G	228	- 1179	K - L	491	- 3089

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - V	3979	- 496	T - S	1160	- 126
V - U	4077	- 504	Q - P	3258	- 436
U - T	1376	- 161	P - N	3346	- 469

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - U	137	- 380	Q - I	202	- 1239
U - F	3744	- 440	J - N	0	- 467
F - T	107	- 447	K - N	233	- 388
G - S	154	- 1248	N - L	3289	- 522
G - Q	2713	- 360	L - M	263	- 1287
S - Q	1726	- 188			

Lumber

Top chord: 2x4 SP #2; T2,T6 2x4 SP M-31;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3; W4,W8,W17 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

COA #0 278
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01/09/2024

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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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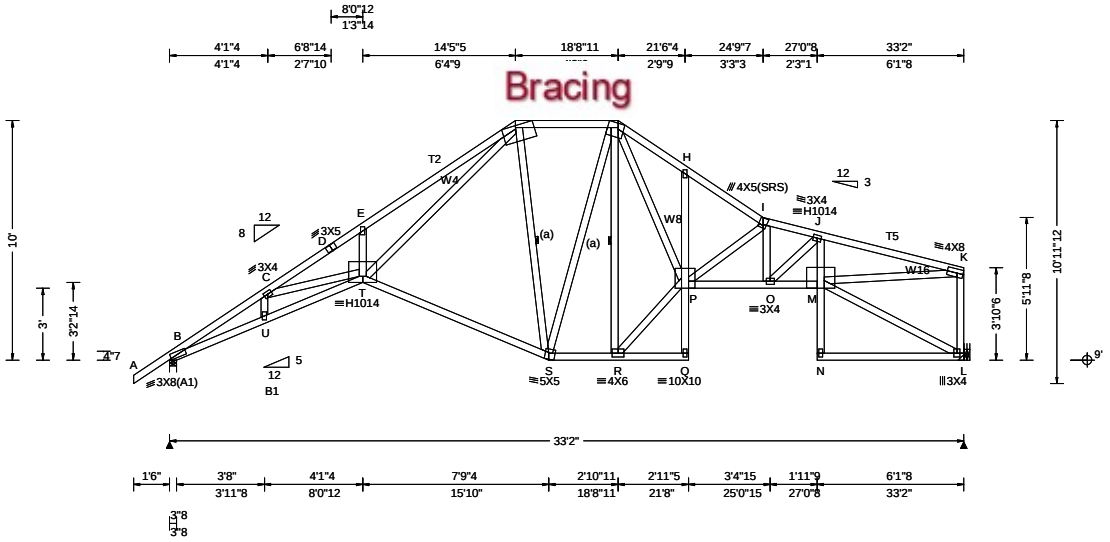
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24617 SPEC FROM:	Ply: 1 Qty: 2	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B09		Cust: R 215 JRef:1XWb2150004 T49 DrwNo: 009.24.1312.55520 GA / DF 01/09/2024	



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.410 T 967 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.849 T 466 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.381 L - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.789 L - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.743
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.780
Spacing:	24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.845
		C&C Dist a: 3.32 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 9.00 ft	Plate Type(s):	
		GCpi: 0.18	WAVE, HS	VIEW Ver: 22.01.00.0314.20
		Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1515	/-	/-	/885	/-	/172
L	1359	/-	/-	/710	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
L Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	353	-4593	G - H	351	-2751
C - D	383	-4716	H - I	294	-2766
D - E	391	-4620	I - J	338	-3364
E - F	505	-4775	J - K	387	-3717
F - G	213	-1179			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	3979	-370	S - R	1160	-68
U - T	4077	-375	P - O	3258	-266
T - S	1376	-99	O - M	3618	-354

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - T	137	-380	R - P	1726	-101
T - F	3744	-330	P - I	155	-1239
F - S	96	-447	O - J	131	-522
G - R	90	-1247	M - K	3558	-348
G - P	2714	-230	K - L	167	-1288

Lumber

Top chord: 2x4 SP #2; T2,T5 2x4 SP M-31;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3; W4,W8,W16 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

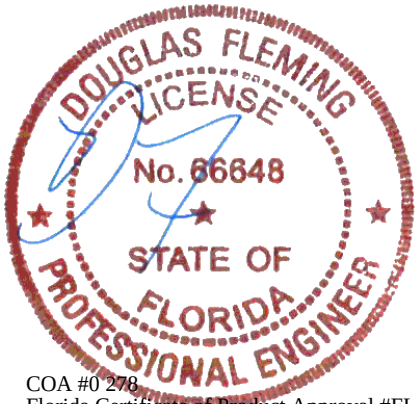
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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01/09/2024

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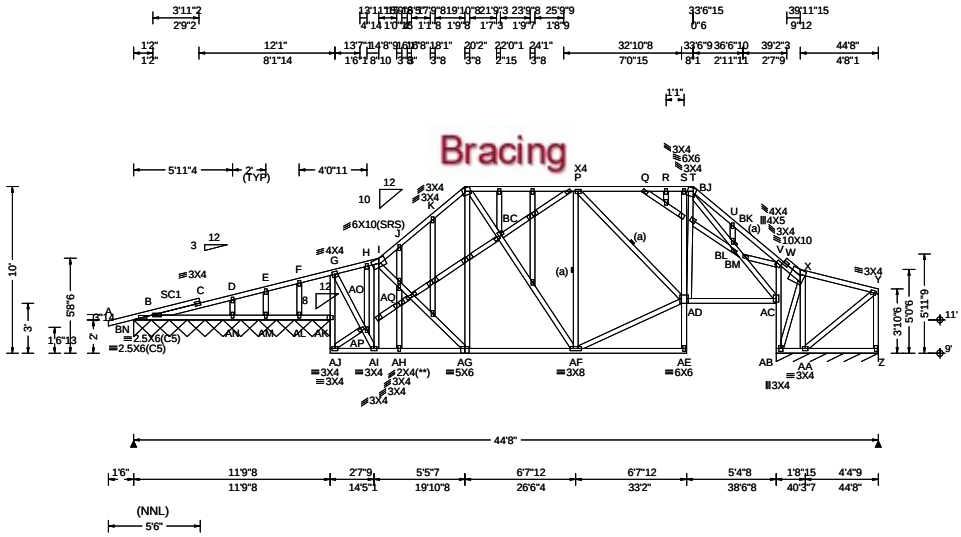
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24619 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B10		Cust: R 215 JRef:1XWb2150004 T9 DrwNo: 009.24.1313.07027 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.47 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.068	Q	999	240
VERT(CL): 0.139	Q	999	180
HORZ(LL): 0.021	BI	-	-
HORZ(TL): 0.042	BI	-	-

Creep Factor: 2.0
Max TC CSI: 0.487
Max BC CSI: 0.463
Max Web CSI: 0.470

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BN*184		/-	/-	/98	/-	/12
Z* 259		/-	/-	/142	/-	/-

Wind reactions based on MWFRS
BN Brg Wid = 141 Min Req = -
Z Brg Wid = 73.5 Min Req = -
Bearings BN & AB are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
G - H	128	-464	N - O	311	-904
H - I	138	-476	O - P	252	-764
I - J	171	-872	P - Q	228	-783
J - K	221	-900	Q - R	197	-704
K - L	260	-872	R - S	198	-706
L - M	295	-872	S - T	216	-759
M - N	298	-878			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AI-AH	505	-71	AG-AF	620	-41
AH-AG	511	-74	AD-AC	701	-49

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
AK- G	186	-1110	AF-AD	863	-97
G -AO	1001	-125	AD-BJ	717	-146
AO-AP	1018	-129	BJ- T	715	-145
AP-AI	884	-95	T -BK	3	-839
AI-AQ	86	-684	BK-BL	42	-944
AQ- I	79	-663	BL-BM	47	-951
L -BC	384	-130	BM-AC	82	-1083
AF- P	178	-432	AC-AB	141	-1148

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;

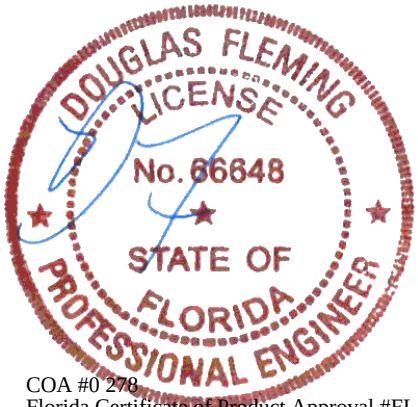
Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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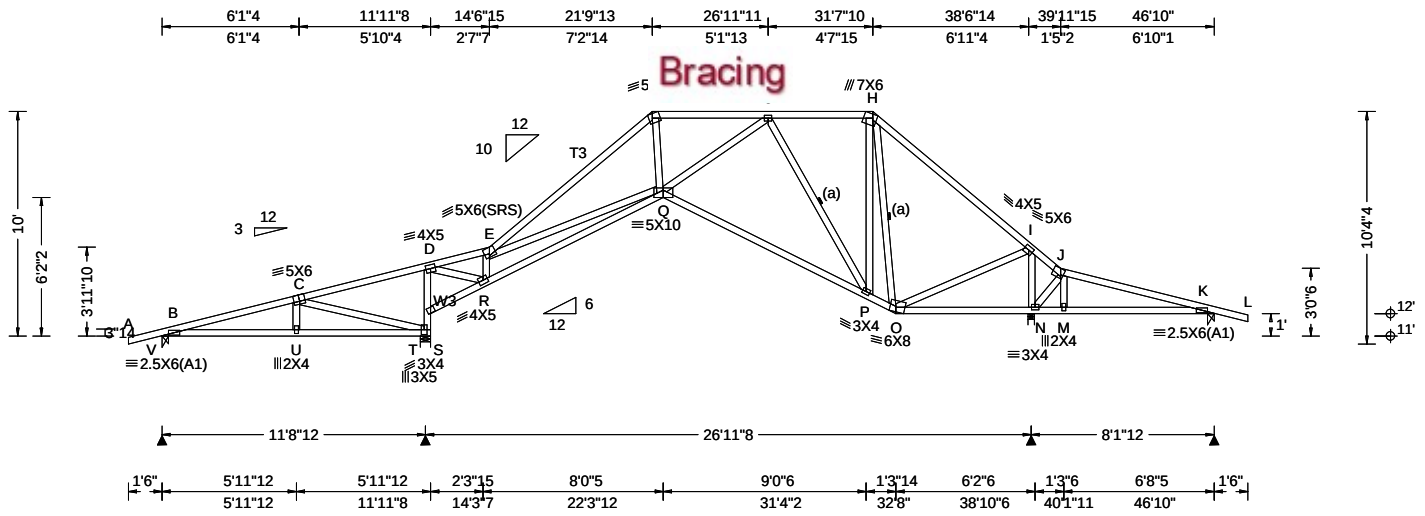
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 16005 COMN FROM:	Ply: 1 Qty: 11	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B11	Cust: R 215 JRef:1XWb2150004 T44 DrwNo: 009.24.1313.10240 GA / DF 01/09/2024	



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.100 Q 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.213 Q 999 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.071 O - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.153 O - -
NCBCLL:	10.00	Mean Height: 18.02 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.660
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.756
Spacing:	24.0 "	MWFRS Parallel Dist: > 2h	Rep Fac: Yes	Max Web CSI: 0.695
		C&C Dist a: 4.68 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 13.00 ft	Plate Type(s):	
		GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1216.15
		Wind Duration: 1.60		

Lumber

Top chord: 2x4 SP #2; T3 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W3 2x4 SP M-31;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

▲Maximum Reactions (lbs)

Loc	Gravity R+	Gravity / R-	Gravity / Rh	Non-Gravity / Rw	Non-Gravity / U	Non-Gravity / RL
V	484	/-	/-	/188	/79	/184
T	1822	/-	/-	/1062	/-	/-
N	1617	/-	/-	/924	/-	/-
K	349	/-	/-	/161	/57	/-

Wind reactions based on MWFRS
V Brg Wid = 3.5 Min Req = 1.5 (Truss)
T Brg Wid = 5.5 Min Req = 2.1 (Truss)
N Brg Wid = 3.5 Min Req = 1.5 (Truss)
K Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings V, T, N, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

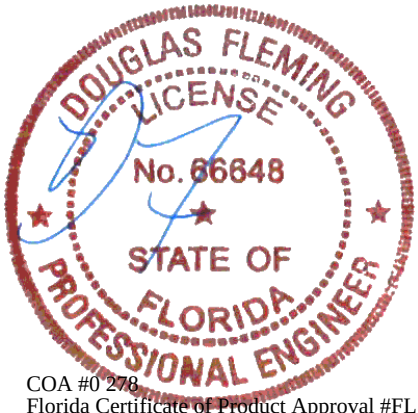
Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	185	-667	F - G	0	-1855
C - D	641	-71	G - H	26	-684
D - E	0	-1100	H - I	2	-891
E - F	0	-2382	I - J	535	0

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	618	-156	Q - P	1294	0
U - T	608	-160	P - O	764	0
S - R	0	-763	O - N	0	-509
R - Q	1467	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - T	160	-996	Q - G	928	0
T - S	15	-1560	G - P	0	-927
S - D	18	-1172	P - H	768	0
D - R	1800	0	H - O	0	-717
R - E	0	-1386	O - I	892	0
E - Q	556	0	I - N	0	-1488
F - Q	987	0	N - J	100	-418



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01/09/2024

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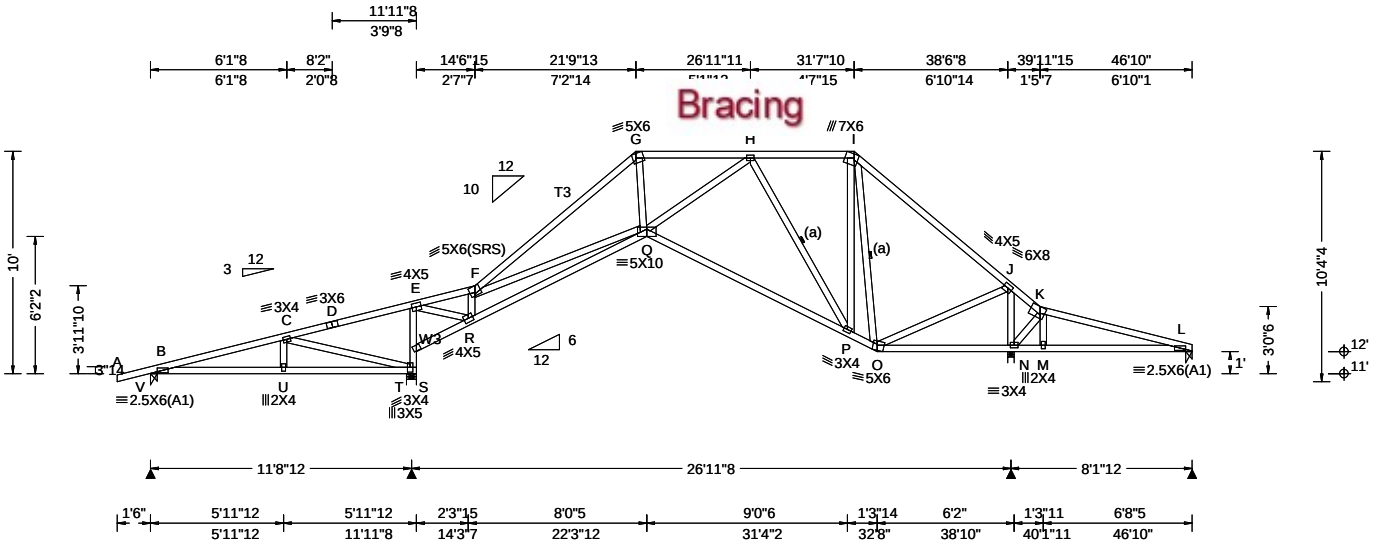
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24627 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B12	Cust: R 215 JRef:1XWb2150004 T6 DrwNo: 009.24.1313.26027 GA / DF 01/09/2024		



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.096 Q 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.203 Q 999 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.070 O - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.149 O - -
NCBCLL:	10.00	Mean Height: 15.97 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.658
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.746
Spacing:	24.0 "	MWFRS Parallel Dist: > 2h	Rep Fac: Yes	Max Web CSI: 0.701
		C&C Dist a: 4.68 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 13.00 ft	Plate Type(s):	VIEW Ver: 22.01.00.0314.20
		GCpi: 0.18	WAVE	
		Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
V	488	/-	/-	/191	/60	/174
T	1796	/-	/-	/1033	/-	/-
N	1624	/-	/-	/912	/-	/-
L	228	/-	/-	/91	/12	/-

Wind reactions based on MWFRS
V Brg Wid = 3.5 Min Req = 1.5 (Truss)
T Brg Wid = 5.5 Min Req = 2.1 (Truss)
N Brg Wid = 3.5 Min Req = 1.5 (Truss)
L Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings V, T, N, & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	133	-687	G - H	199	-1797
C - D	573	-83	H - I	201	-662
D - E	628	-78	I - J	187	-872
E - F	108	-1085	J - K	522	-25
F - G	183	-2313			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	638	-64	R - Q	1445	-116
U - T	627	-68	Q - P	1245	-57
S - R	99	-746	P - O	735	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - T	144	-1005	Q - H	910	0
T - S	214	-1531	H - P	112	-881
S - E	167	-1151	P - I	721	0
E - R	1767	-131	I - O	0	-671
R - F	194	-1358	O - J	879	-9
F - Q	521	0	J - N	204	-1457
G - Q	955	0			

Lumber

Top chord: 2x4 SP #2; T3 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W3 2x4 SP M-31;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

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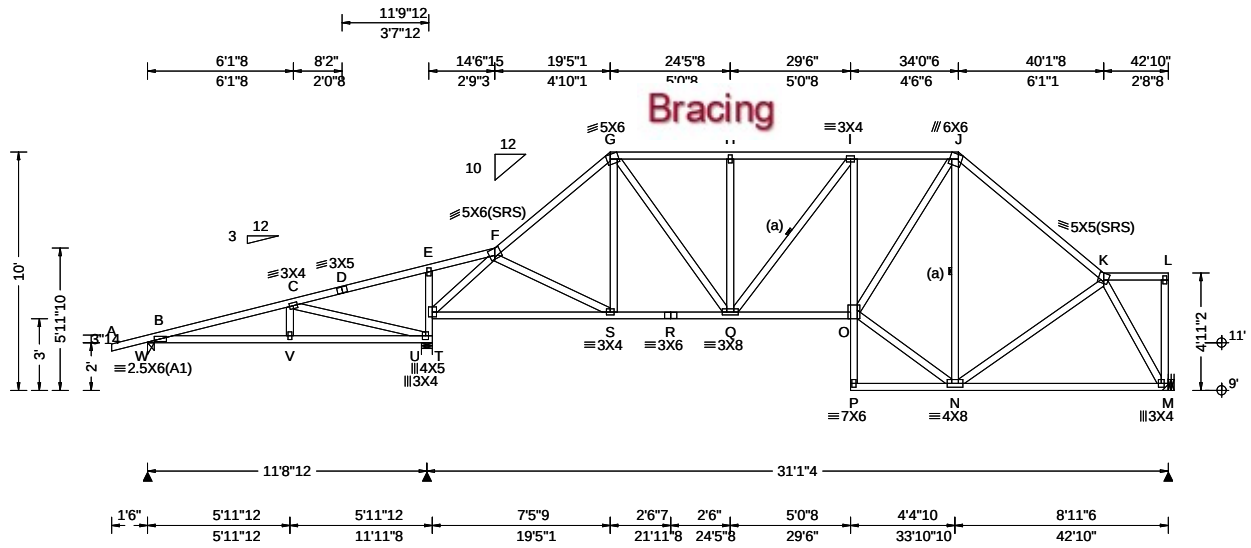
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24629 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B13		Cust: R 215 JRef:1XWb2150004 T5 DrwNo: 009.24.1313.29040 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 18.02 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 4.28 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.066	I	999	240
VERT(CL): 0.139	I	999	180
HORZ(LL): 0.032	M	-	-
HORZ(TL): 0.070	M	-	-

Creep Factor: 2.0
Max TC CSI: 0.451
Max BC CSI: 0.848
Max Web CSI: 0.773

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
W	534	/-	/-	/243	/48	/149
U	1869	/-	/-	/1040	/-	/-
M	1306	/-	/-	/739	/-	/-

Wind reactions based on MWFRS
W Brg Wid = 3.5 Min Req = 1.5 (Truss)
U Brg Wid = 5.5 Min Req = 2.2 (Truss)
M Brg Wid = - Min Req = -
Bearings W & U are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	98	-862	H - I	0	-1478
F - G	0	-1566	I - J	0	-1543
G - H	0	-1479	J - K	0	-1281

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - V	807	-151	R - Q	1132	0
V - U	797	-156	Q - O	1551	0
T - S	1160	0	N - M	708	0
S - R	1132	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - U	155	-971	O - N	1092	0
U - T	0	-1572	O - J	1175	0
T - F	0	-1831	N - J	0	-604
G - Q	582	0	K - M	0	-1396

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

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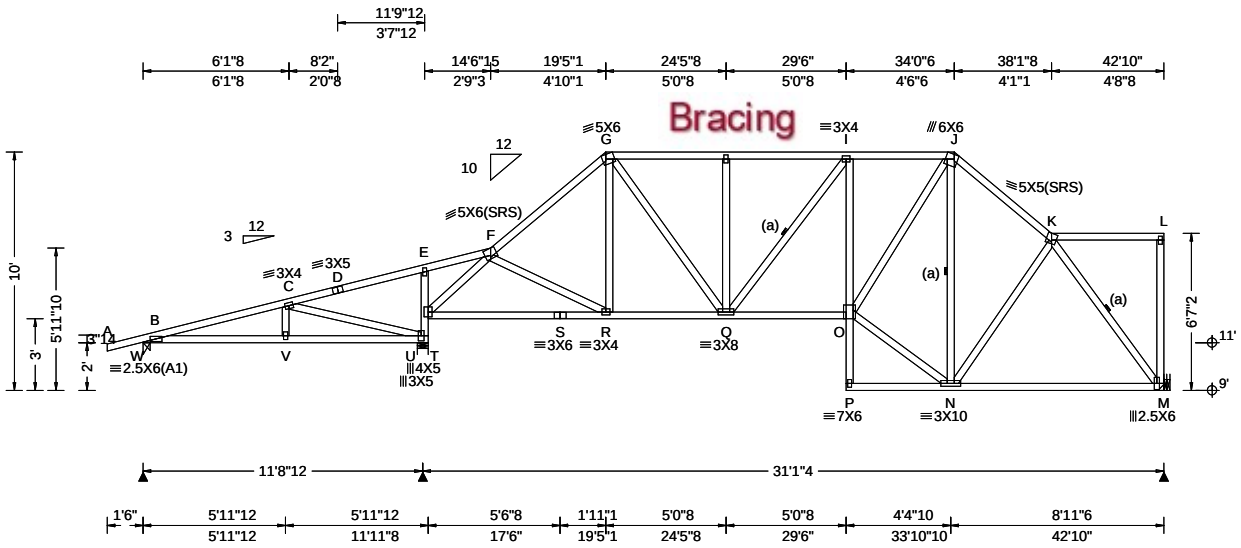
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24631 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B14		Cust: R 215 JRef:1XWb2150004 T7 DrwNo: 009.24.1313.31247 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 4.28 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.066	I	999	240
VERT(CL): 0.135	I	999	180
HORZ(LL): 0.031	M	-	-
HORZ(TL): 0.067	M	-	-

Creep Factor: 2.0
Max TC CSI: 0.406
Max BC CSI: 0.864
Max Web CSI: 0.759

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
W	536	/-	/-	/247	/22	/137
U	1826	/-	/-	/994	/-	/-
M	1260	/-	/-	/669	/-	/-

Wind reactions based on MWFRS
W Brg Wid = 3.5 Min Req = 1.5 (Truss)
U Brg Wid = 5.5 Min Req = 2.2 (Truss)
M Brg Wid = - Min Req = -
Bearings W & U are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	22	- 867	H - I	345	- 1408
F - G	288	- 1505	I - J	362	- 1473
G - H	346	- 1408	J - K	252	- 1184

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - V	811	- 116	R - Q	1084	- 166
V - U	802	- 120	Q - O	1481	- 278
T - S	1117	- 217	N - M	800	- 166
S - R	1115	- 218			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - U	155	- 971	O - N	1036	- 164
U - T	316	- 1529	O - J	1131	- 248
T - F	339	- 1768	N - J	146	- 498
G - Q	543	- 141	K - M	276	- 1332

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

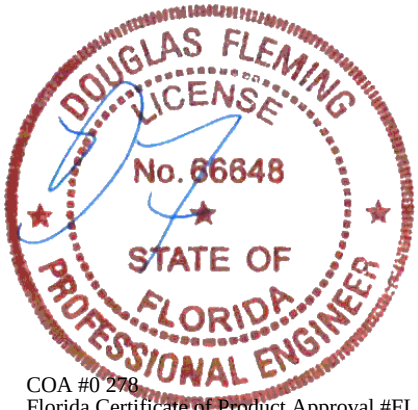
Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0 278
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01/09/2024

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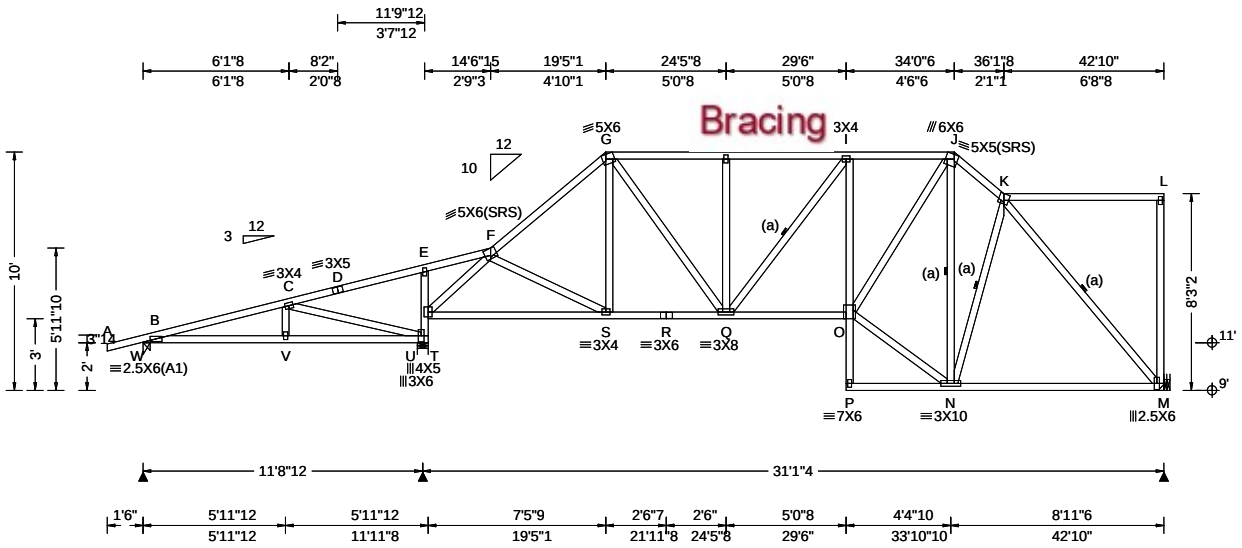
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24633 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B15		Cust: R 215 JRef:1XWb2150004 T18 DrwNo: 009.24.1313.38023 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 4.28 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.067	I	999	240
VERT(CL): 0.136	I	999	180
HORZ(LL): 0.031	M	-	-
HORZ(TL): 0.066	M	-	-

Creep Factor: 2.0
Max TC CSI: 0.634
Max BC CSI: 0.873
Max Web CSI: 0.759

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
W	536	/-	/-	/251	/16	/131
U	1824	/-	/-	/994	/-	/-
M	1251	/-	/-	/642	/16	/-

Wind reactions based on MWFRS
W Brg Wid = 3.5 Min Req = 1.5 (Truss)
U Brg Wid = 5.5 Min Req = 2.2 (Truss)
M Brg Wid = - Min Req = -
Bearings W & U are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	0	-867	H - I	333	-1403
F - G	271	-1502	I - J	361	-1469
G - H	333	-1404	J - K	276	-1173

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - V	811	-119	R - Q	1082	-202
V - U	802	-123	Q - O	1477	-321
T - S	1115	-236	N - M	848	-192
S - R	1082	-202			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - U	156	-971	O - N	1025	-198
U - T	329	-1527	O - J	1140	-276
T - F	358	-1765	K - M	298	-1314
G - Q	540	-145			

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

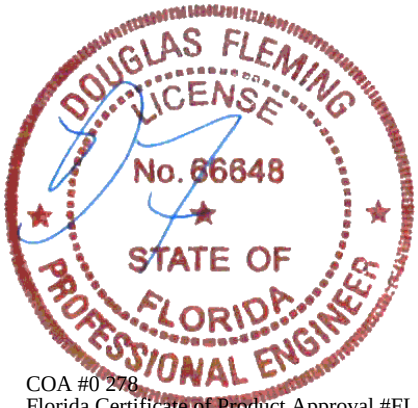
Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

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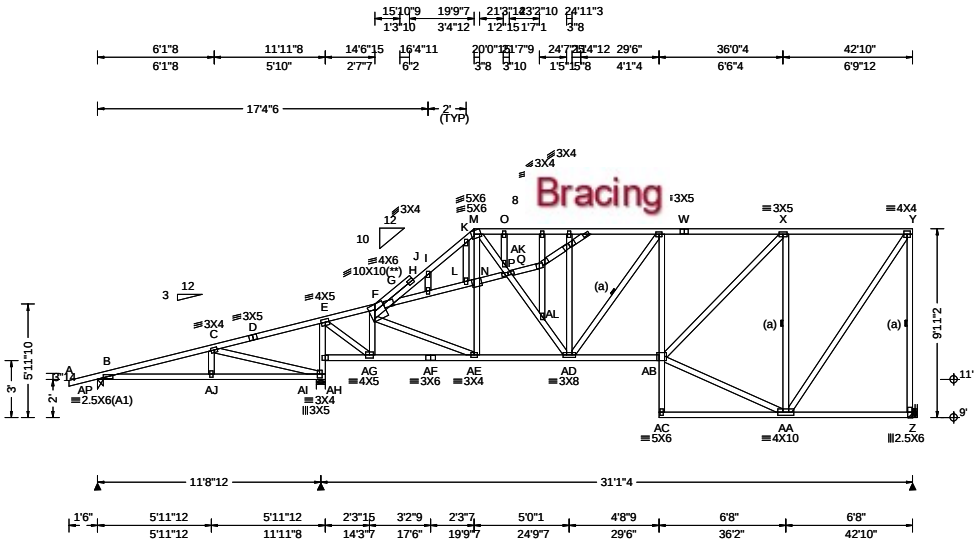
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24635 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B16		Cust: R 215 JRef:1XWb2150004 T35 DrwNo: 009.24.1313.41860 GA / DF 01/09/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.075 V 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.154 V 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.020 AA - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.044 AA - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.762
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.512
Spacing: 24.0 "	MWFRS Parallel Dist: > 2h	Rep Fac: Yes	Max Web CSI: 0.857
	C&C Dist a: 4.28 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 13.00 ft	Plate Type(s):	VIEW Ver: 22.01.00.0314.20
	GCpi: 0.18	WAVE	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AP	519	/-	/-	/238	/11	/127
AI	1864	/-	/-	/1014	/-	/-
Z	1232	/-	/-	/637	/54	/-

Wind reactions based on MWFRS
AP Brg Wid = 3.5 Min Req = 1.5 (Truss)
AI Brg Wid = 5.5 Min Req = 2.2 (Truss)
Z Brg Wid = - Min Req = -
Bearings AP & AI are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	0	- 806	M - O	199	- 1208
E - F	24	- 1026	O - R	206	- 1222
F - G	55	- 378	R - T	206	- 1223
F - G	156	- 1202	T - U	187	- 1187
G - H	56	- 1003	U - V	263	- 1403
H - I	105	- 1196	V - W	254	- 1430
I - K	136	- 1200	W - X	254	- 1430
K - M	169	- 1176	X - Y	124	- 704

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B -AJ	753	- 95	AF-AE	1136	- 148
AJ-AI	743	- 100	AE-AD	1197	- 240
AG-AF	1136	- 148	AD-AB	1437	- 257

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C -AI	141	- 981	Q -AL	398	- 55
AI-AH	274	- 1580	AB- X	1010	- 199
AH- E	277	- 1530	AB-AA	787	- 143
E -AG	1623	- 184	X -AA	322	- 1193
AG- F	169	- 983	AA- Y	1238	- 218
M -AK	527	- 115	Y - Z	262	- 1181
AK- P	433	- 69			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

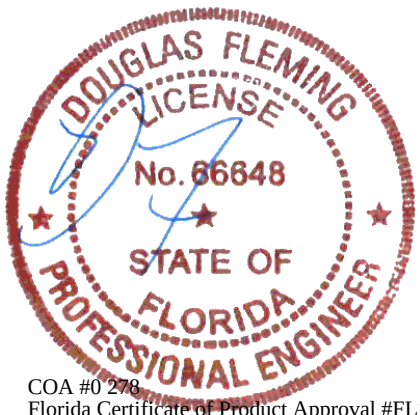
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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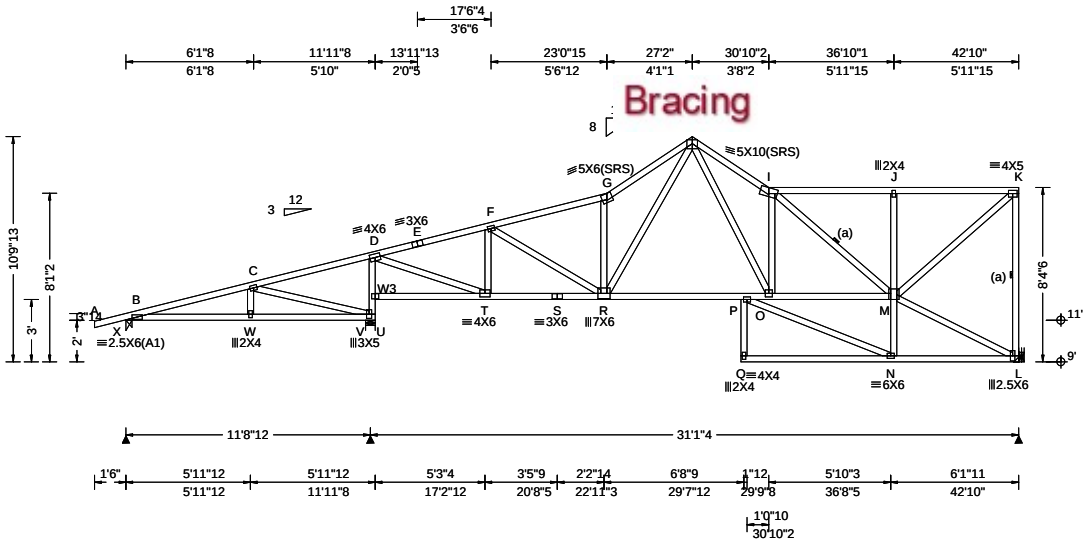
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24637 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B17	Cust: R 215 JRef:1XWb2150004 T17 DrwNo: 009.24.1313.44090 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.38 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 4.28 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.159	Q	999	240
VERT(CL): 0.323	Q	999	180
HORZ(LL): 0.039	L	-	-
HORZ(TL): 0.082	L	-	-

Creep Factor: 2.0
Max TC CSI: 0.505
Max BC CSI: 0.709
Max Web CSI: 0.737

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
X	514	/-	/-	/233	/11	/140
V	1869	/-	/-	/978	/-	/-
L	1236	/-	/-	/640	/-	/-

Wind reactions based on MWFRS
X Brg Wid = 3.5 Min Req = 1.5 (Truss)
V Brg Wid = 5.5 Min Req = 2.2 (Truss)
L Brg Wid = - Min Req = -
Bearings X & V are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	0	- 788	G - H	372	- 2313
C - D	411	- 175	H - I	397	- 2210
D - E	148	- 1625	I - J	125	- 1078
E - F	157	- 1594	J - K	129	- 1083
F - G	245	- 1936			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - W	736	- 85	R - P	1256	- 186
W - V	726	- 89	P - O	930	- 22
T - S	1582	- 228	O - M	1463	- 121
S - R	1582	- 228			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - V	135	- 989	H - O	1211	- 223
V - U	265	- 1591	I - O	193	- 859
U - D	285	- 1507	I - M	208	- 929
D - T	1934	- 262	J - M	209	- 432
T - F	153	- 591	M - K	1398	- 166
R - G	231	- 1068	K - L	160	- 1024
R - H	1244	- 181			

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W3 2x4 SP #2;

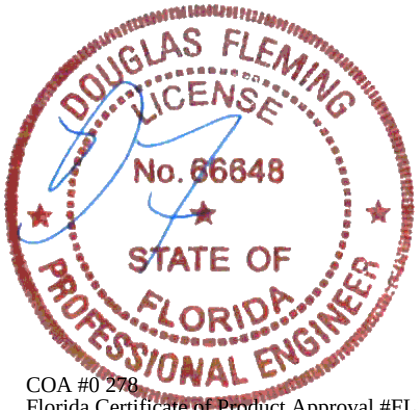
Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 3X4 except as noted.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)



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01/09/2024

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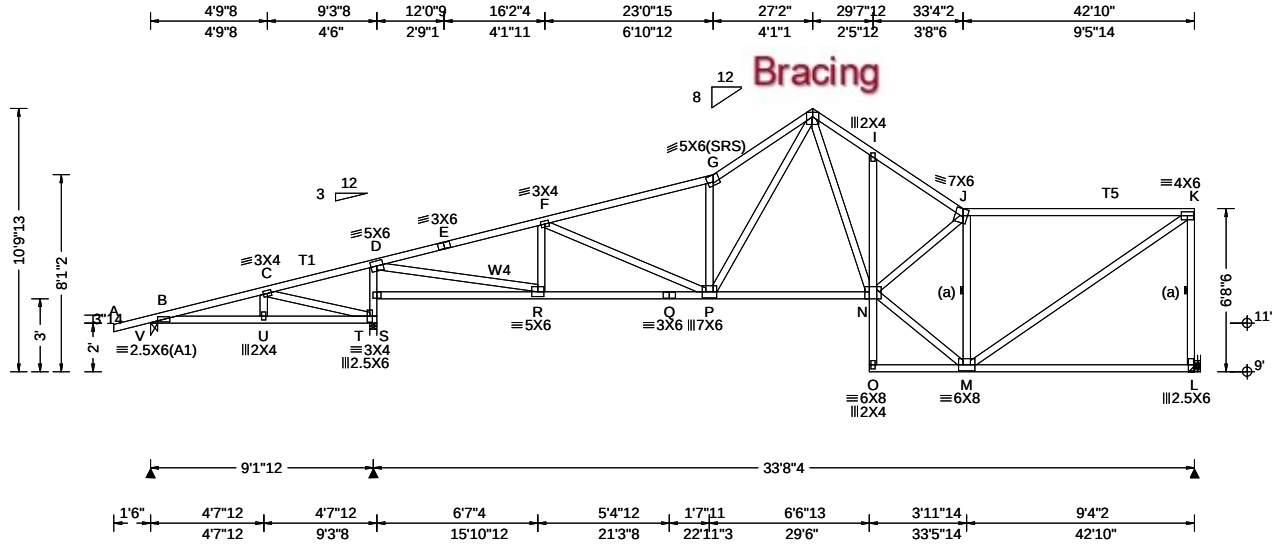
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24639 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B18		Cust: R 215 JRef:1XWb2150004 T30 DrwNo: 009.24.1313.57520 GA / DF 01/09/2024	



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.201 G 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.409 G 993 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.053 M - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.108 M - -
NCBCLL:	10.00	Mean Height: 15.38 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.797
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.792
Spacing:	24.0 "	MWFRS Parallel Dist: > 2h	Rep Fac: Yes	Max Web CSI: 0.814
		C&C Dist a: 4.28 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 13.00 ft	Plate Type(s):	VIEW Ver: 22.01.00.0314.20
		GCpi: 0.18	WAVE	
		Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
V	409	/-	/-	/170	/19	/144
T	1876	/-	/-	/975	/-	/-
L	1355	/-	/-	/698	/-	/-

Wind reactions based on MWFRS
V Brg Wid = 3.5 Min Req = 1.5 (Truss)
T Brg Wid = 3.5 Min Req = 2.2 (Truss)
L Brg Wid = - Min Req = -
Bearings V & T are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	75	- 488	G - H	433	- 2906
C - D	423	- 157	H - I	358	- 2310
D - E	244	- 2482	I - J	339	- 2420
E - F	254	- 2441	J - K	203	- 1507
F - G	292	- 2441			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	452	- 64	Q - P	2392	- 307
U - T	443	- 74	P - N	1532	- 167
R - Q	2392	- 307			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - T	77	- 685	H - N	1175	- 161
T - S	257	- 1700	N - J	465	- 8
S - D	281	- 1603	N - M	2020	- 271
D - R	2687	- 329	J - M	410	- 2140
R - F	142	- 393	M - K	1827	- 246
P - G	277	- 1322	K - L	260	- 1278
P - H	1682	- 252			

Lumber

Top chord: 2x4 SP #2; T1,T5 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W4 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

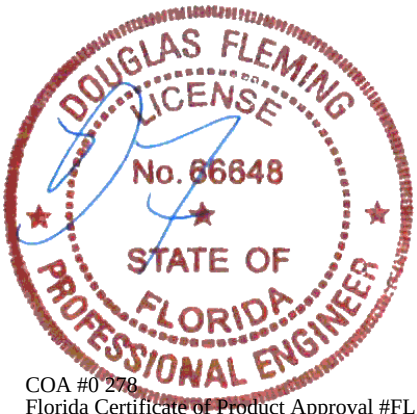
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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Florida Certificate of Product Approval #FL1999
01/09/2024

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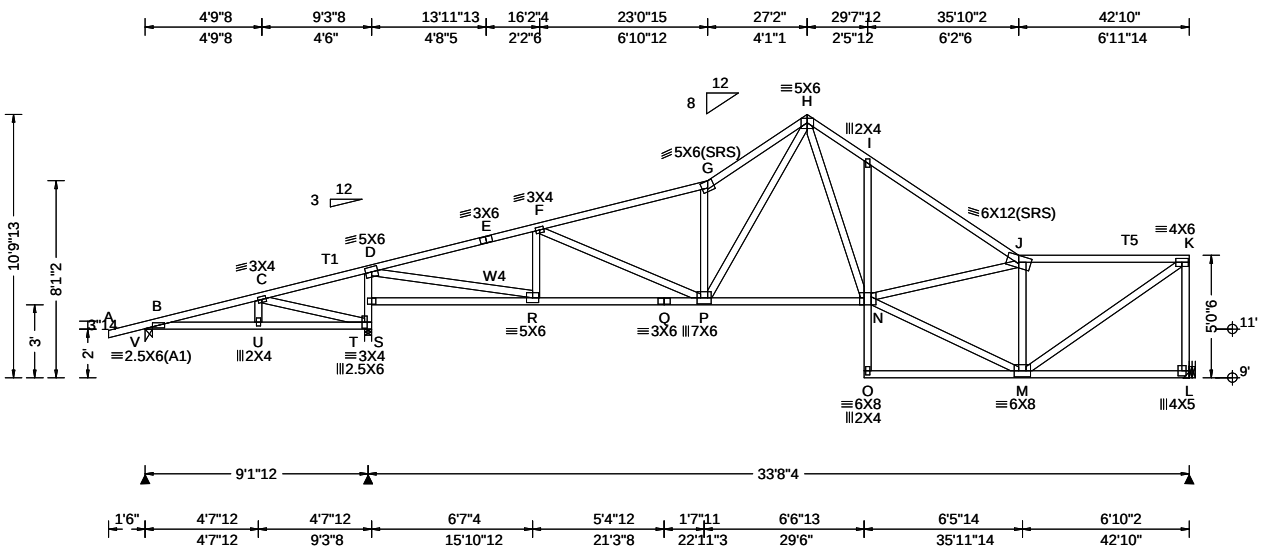
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24641 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B19	Cust: R 215 JRef:1XWb2150004 T42 DrwNo: 009.24.1313.59700 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.38 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 4.28 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.197	G	999	240
VERT(CL): 0.401	G	999	180
HORZ(LL): 0.052	M	-	-
HORZ(TL): 0.108	M	-	-

Creep Factor: 2.0
Max TC CSI: 0.618
Max BC CSI: 0.720
Max Web CSI: 0.821

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
V	410	/-	/-	/168	/26	/148
T	1873	/-	/-	/973	/-	/-
L	1362	/-	/-	/725	/-	/-

Wind reactions based on MWFRS
V Brg Wid = 3.5 Min Req = 1.5 (Truss)
T Brg Wid = 3.5 Min Req = 2.2 (Truss)
L Brg Wid = - Min Req = -
Bearings V & T are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	62	- 492	G - H	405	- 2921
C - D	415	- 118	H - I	340	- 2388
D - E	238	- 2489	I - J	267	- 2447
E - F	243	- 2420	J - K	168	- 1636
F - G	266	- 2450			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	456	- 51	Q - P	2399	- 259
U - T	446	- 61	P - N	1531	- 108
R - Q	2399	- 259			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - T	78	- 686	P - H	1702	- 251
T - S	234	- 1695	H - N	1272	- 155
S - D	258	- 1598	N - M	1880	- 196
D - R	2683	- 278	J - M	286	- 1818
R - F	131	- 392	M - K	1989	- 204
P - G	268	- 1335	K - L	207	- 1306

Lumber

Top chord: 2x4 SP #2; T1,T5 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W4 2x4 SP #2;

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

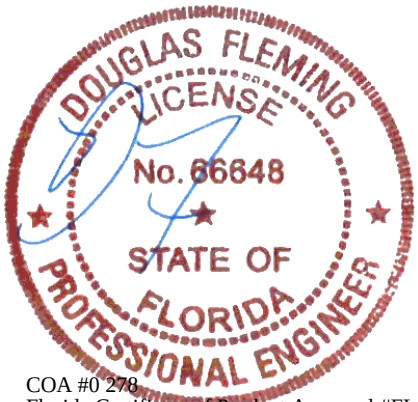
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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01/09/2024

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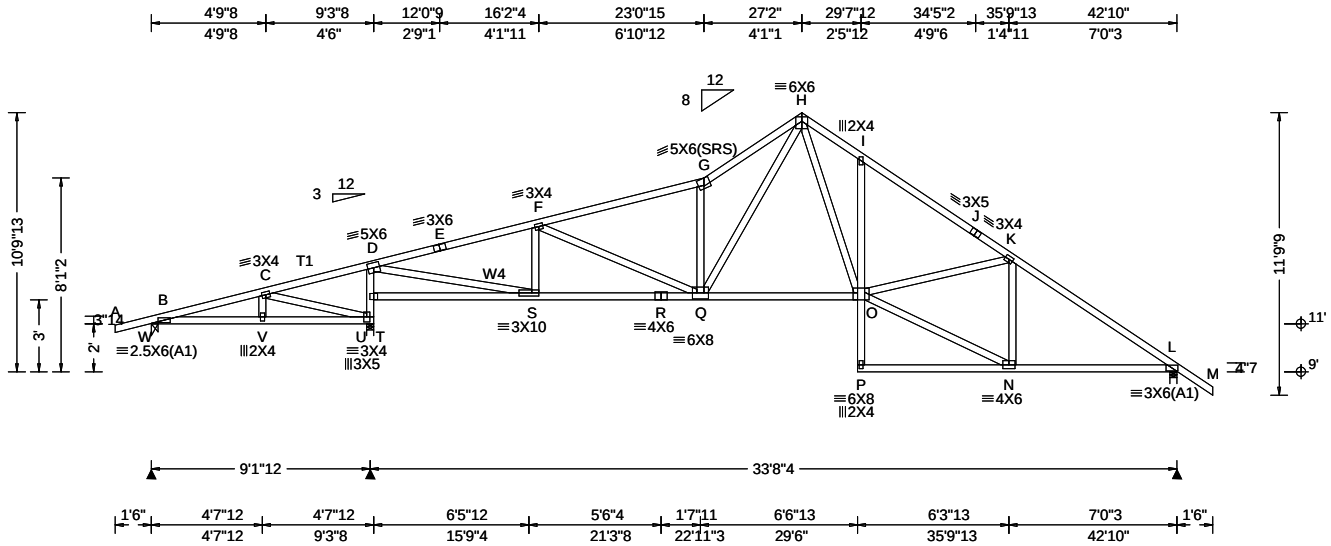
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 15993 SPEC FROM:	Ply: 1 Qty: 3	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B20	Cust: R 215 JRef:1XWb2150004 T31 DrwNo: 009.24.1314.03797 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 4.28 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.209	G	999	240
VERT(CL): 0.413	G	978	180
HORZ(LL): 0.065	L	-	-
HORZ(TL): 0.130	L	-	-

Creep Factor: 2.0
Max TC CSI: 0.641
Max BC CSI: 0.813
Max Web CSI: 0.817

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
W	405	/-	/-	/174	/48	/202
U	1930	/-	/-	/982	/-	/-
L	1557	/-	/-	/904	/-	/-

Wind reactions based on MWFRS
W Brg Wid = 3.5 Min Req = 1.5 (Truss)
U Brg Wid = 3.5 Min Req = 2.3 (Truss)
L Brg Wid = 3.5 Min Req = 1.8 (Truss)
Bearings W, U, & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	119	-476	G - H	351	-3109
C - D	427	-113	H - I	263	-2537
D - E	223	-2596	I - J	183	-2545
E - F	233	-2555	J - K	152	-2597
F - G	221	-2611	K - L	125	-2185

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - V	440	-69	R - Q	2505	-129
V - U	431	-79	Q - O	1629	0
S - R	2505	-129	N - L	1721	-19

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - U	69	-685	Q - G	238	-1398
U - T	177	-1755	Q - H	1825	-242
T - D	201	-1654	H - O	1356	-86
D - S	2796	-153	O - N	1916	-20
S - F	106	-428	N - K	82	-742

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W4 2x4 SP #2;

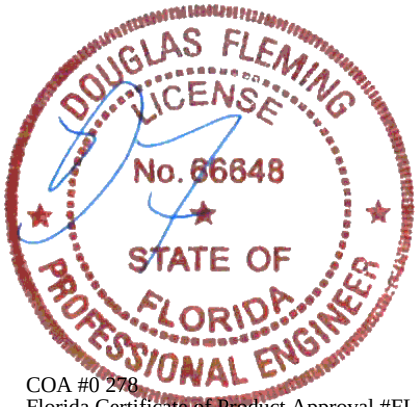
Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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01/09/2024

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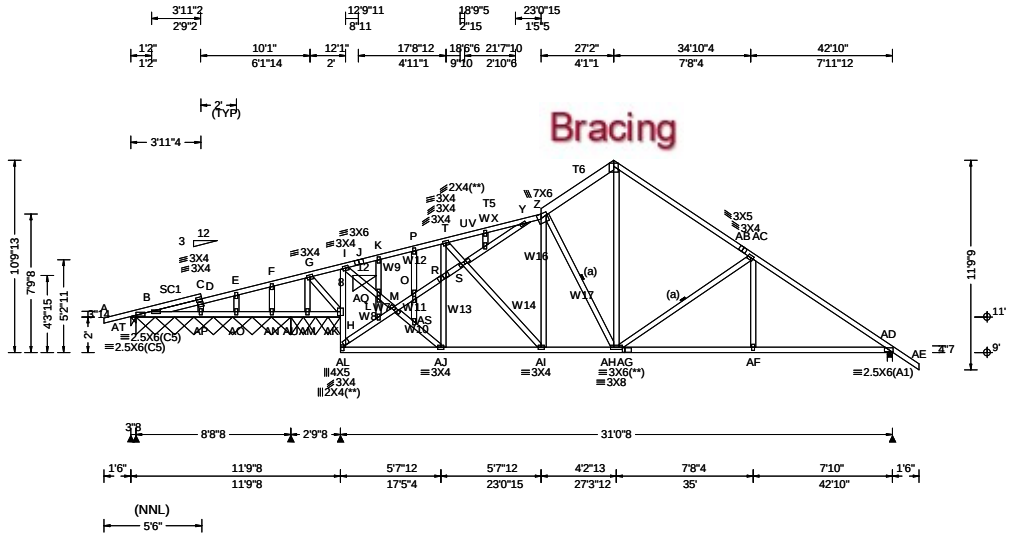
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 16031 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B21		Cust: R 215 JRef:1XWb2150004 T4 DrwNo: 009.24.1314.08753 GA / DF 01/09/2024	



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.046 W 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.093 W 999 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.014 H - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.029 H - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.265
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.266
Spacing:	24.0 "	MWFRS Parallel Dist: > 2h	Rep Fac: Yes	Max Web CSI: 0.532
		C&C Dist a: 4.28 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 13.00 ft	Plate Type(s):	
		GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1216.15
		Wind Duration: 1.60		

Lumber

Top chord: 2x4 SP M-31; T5 2x4 SP #2;
T6 2x6 SP 2400f-2.0E;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31; W7,W8,W9,W10,W11,W12,W13,W14, W16,W17 2x4 SP #3;
Stack Chord: SC1 2x4 SP M-31;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

▲Maximum Reactions (lbs), or *=PLF

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
AT	285	/-	/-	/126	/45	/202
AT*	61	/-	/-	/32	/-	/-
AU*	555	/-	/-	/289	/-	/-
AD	1385	/-	/-	/851	/-	/-

Wind reactions based on MWFRS
AT Brg Wid = 3.5 Min Req = 1.5 (Truss)
AT Brg Wid = 104 Min Req = -
AU Brg Wid = 33.5 Min Req = -
AD Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings AT, AT, AU, & AD are a rigid surface.

Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

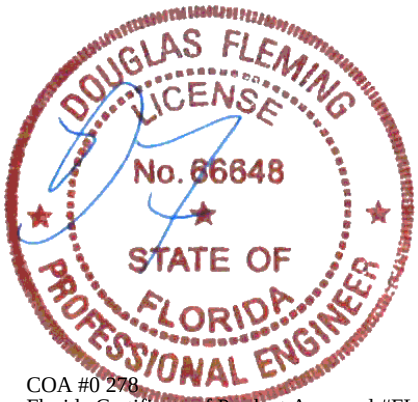
Chords	Tens.	Comp.	Chords	Tens.	Comp.
H - L	103	-611	T - W	121	-807
I - J	114	-650	V - X	74	-507
J - K	116	-638	W - Y	130	-782
K - P	125	-641	X - Y	58	-479
L - M	92	-580	Y - Z	186	-1205
M - O	76	-518	Z -AA	204	-1183
P - T	131	-631	AA-AB	190	-1257
O - S	69	-487	AB-AC	141	-1275
R - U	77	-522	AC-AD	120	-1859

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AJ-AI	1038	-50	AG-AF	1440	-5
AI-AH	1147	-15	AF-AD	1442	-4
AH-AG	1440	-5			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
AK- H	382	-36	AJ- S	53	-447
AK- I	81	-1041	R - S	86	-713
I -AQ	1006	-28	R - T	46	-418
AQ- M	991	-22	Z -AH	157	-437
M -AS	890	0	AA-AH	852	-104
AS-AJ	873	0	AH-AC	106	-603



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01/09/2024

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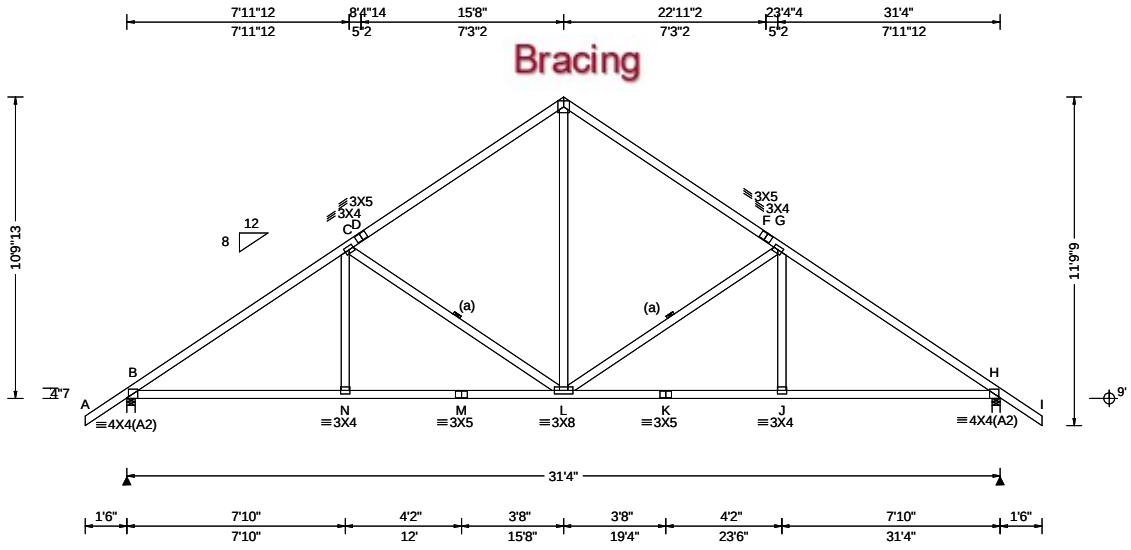
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 15988 COMN FROM:	Ply: 1 Qty: 3	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B22	Cust: R 215 JRef:1XWb2150004 T25 DrwNo: 009.24.1314.16520 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.13 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.070	L	999	240
VERT(CL): 0.145	L	999	180
HORZ(LL): 0.035	H	-	-
HORZ(TL): 0.073	H	-	-

Creep Factor: 2.0
Max TC CSI: 0.673
Max BC CSI: 0.657
Max Web CSI: 0.319

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
B	1420	/-	/-	/822	/-	/234
H	1420	/-	/-	/822	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.7 (Truss)
H Brg Wid = 3.5 Min Req = 1.7 (Truss)
Bearings B & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	95	-1919	E - F	148	-1316
C - D	99	-1334	F - G	99	-1334
D - E	148	-1316	G - H	95	-1919

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - N	1491	0	L - K	1489	0
N - M	1489	0	K - J	1489	0
M - L	1489	0	J - H	1491	0

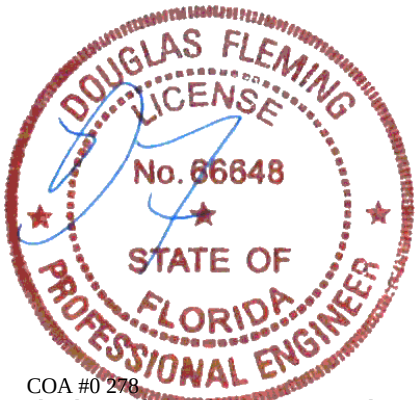
Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - L	108	-602	L - G	108	-602
E - L	836	-39			

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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01/09/2024

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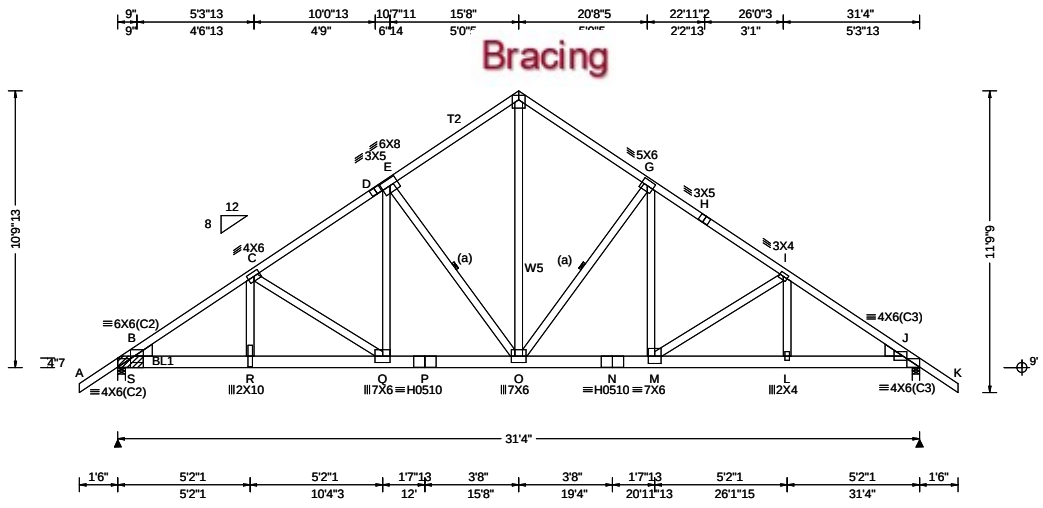
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 17566 SPEC FROM:	Ply: 2 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** B23	Cust: R 215 JRef:1XWb2150004 T48 DrwNo: 009.24.1314.39727 GA / DF 01/09/2024	

2 Complete Trusses Required



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	0.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.13 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.196	Q	999	240
VERT(CL): 0.389	Q	957	180
HORZ(LL): 0.065	C	-	-
HORZ(TL): 0.130	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.627
Max BC CSI: 0.558
Max Web CSI: 0.866

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
S	9315	/-	/-	/268	/-	/-
J	6724	/-	/-	/203	/-	/-

Wind reactions based on MWFRS
S Brg Wid = 3.5 Min Req = -
J Brg Wid = 3.5 Min Req = 2.8 (Truss)
Bearings S & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	0	-7232	F - G	0	-4284
C - D	0	-5778	G - H	0	-5438
D - E	0	-5692	H - I	0	-5494
E - F	0	-4283	I - J	0	-5646

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - R	6002	0	O - N	4480	0
R - Q	5975	0	N - M	4480	0
Q - P	4699	0	M - L	4665	0
P - O	4699	0	L - J	4665	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
R - C	1457	0	F - O	4525	0
C - Q	0	-1458	O - G	0	-1611
Q - E	2273	0	G - M	1809	0
E - O	0	-1984			

Lumber

Top chord: 2x4 SP M-31; T2 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W5 2x4 SP M-31;
Lt Wedge: 2x6 SP 2400f-2.0E;
Rt Wedge: 2x6 SP 2400f-2.0E;

Bearing Block(s)

Brg blocks:0.131"x3", min. nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
1	0.000'	1	12"	7	Rigid Surface

Brg block to be same size and species as chord.
Refer to drawing CNNAILSP1014 for more information.

Bracing

(a) Continuous lateral restraint equally spaced on member.

Nailnote

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 3.75" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

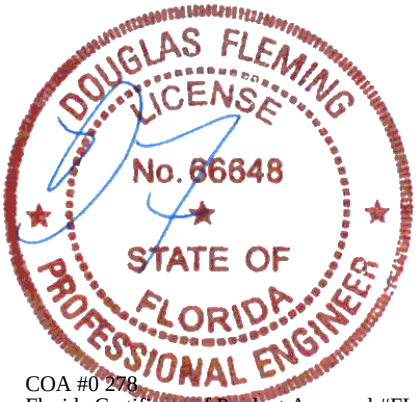
TC: From	64 plf at	-1.50 to	64 plf at	32.83
BC: From	5 plf at	-1.50 to	5 plf at	0.00
BC: From	10 plf at	0.00 to	10 plf at	22.77
BC: From	20 plf at	22.77 to	20 plf at	31.33
BC: From	5 plf at	31.33 to	5 plf at	32.83

BC: 1233 lb Conc. Load at 2.06, 4.06
BC: 1104 lb Conc. Load at 6.06, 8.06,10.06,12.06 14.06,16.06,18.06,20.06,20.77
BC: 1021 lb Conc. Load at 22.77

Wind

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.



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01/09/2024

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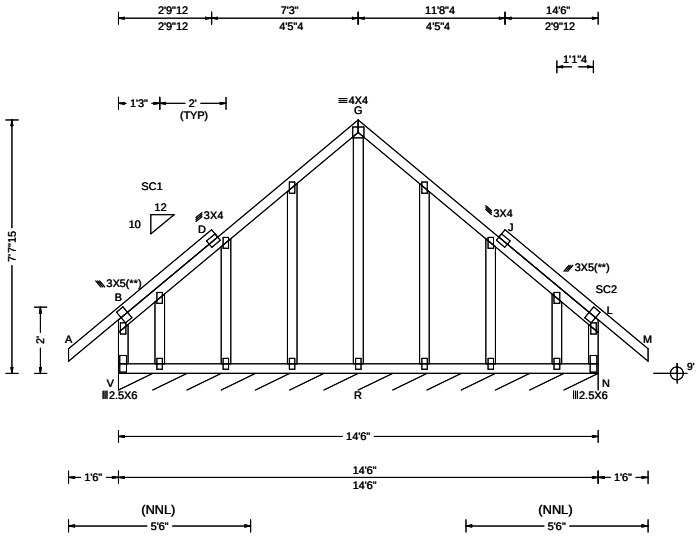
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24660 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** C01	Cust: R 215 JRef:1XWb2150004 T66 DrwNo: 009.24.1314.45250 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.002	K	999	240
VERT(CL): 0.006	K	999	180
HORZ(LL): 0.061	B	-	-
HORZ(TL): 0.104	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.204
Max BC CSI: 0.089
Max Web CSI: 0.323

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs), or *=PLF

Loc	Gravity R+	Gravity / R-	Gravity / Rh	Non-Gravity / Rw	Non-Gravity / U	Non-Gravity / RL
V*	101	/-	/-	/54	/1	/11

Wind reactions based on MWFRS
V Brg Wid = 174 Min Req = -
Bearing V is a rigid surface.
Members not listed have forces less than 375#

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.Comp.
G - R	1 - 383

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Bracing

Fasten rated sheathing to one face of this frame.

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.

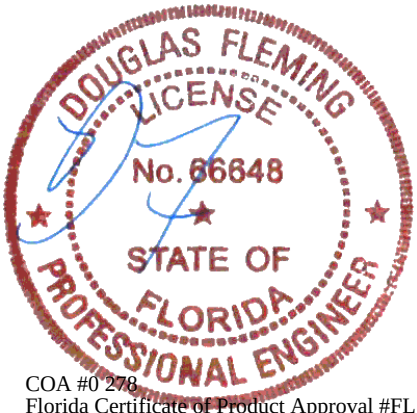
End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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01/09/2024

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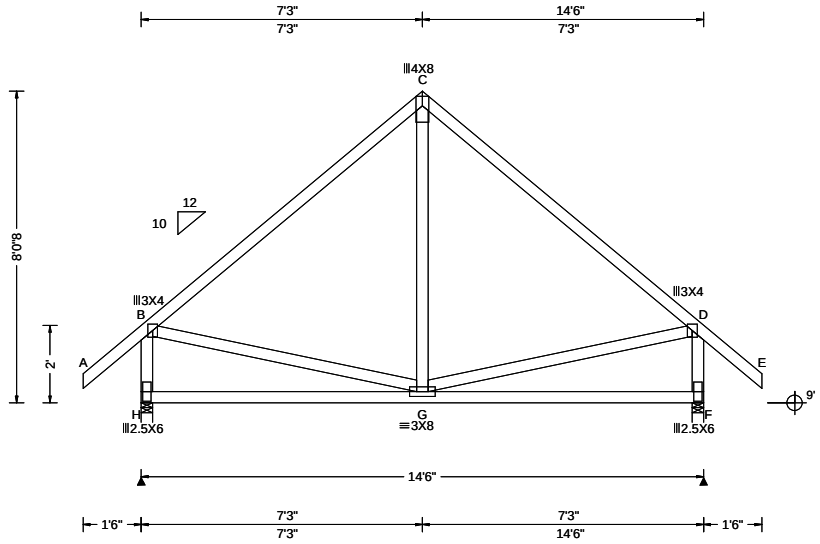
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24662 COMN FROM:	Ply: 1 Qty: 2	**Job Number:** 23-0295 Stewart Residence **Truss Label:** C02	Cust: R 215 JRef:1XWb2150004 T1 DrwNo: 009.24.1314.47210 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.007	G	999	240
VERT(CL): 0.013	G	999	180
HORZ(LL): 0.002	C	-	-
HORZ(TL): 0.003	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.675
Max BC CSI: 0.470
Max Web CSI: 0.196

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
H	731	/-	/-	/418	/14	/169
F	731	/-	/-	/418	/14	/-

Wind reactions based on MWFRS
H Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings H & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	181	-580	C - D	181	-580

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - H	228	-672	D - F	228	-672

Lumber

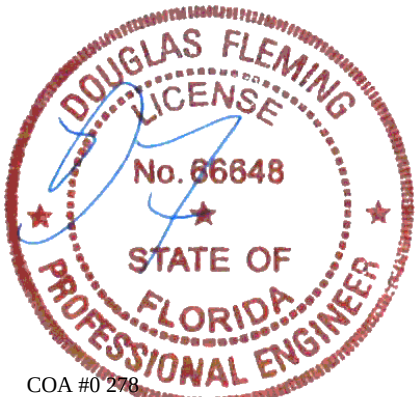
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.



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01/09/2024

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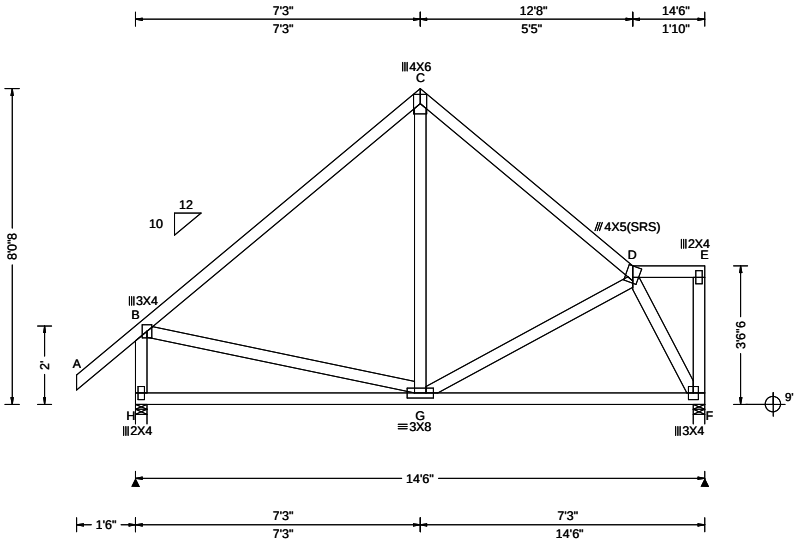
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24664 COMN FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** C03	Cust: R 215 JRef:1XWb2150004 T3 DrwNo: 009.24.1314.51837 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.007	G	999	240
VERT(CL): 0.013	G	999	180
HORZ(LL): 0.002	F	-	-
HORZ(TL): 0.004	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.630
Max BC CSI: 0.512
Max Web CSI: 0.227

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
H	736	/-	/-	/424	/6	/140
F	618	/-	/-	/352	/18	/-

Wind reactions based on MWFRS
H Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings H & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	97	- 582	C - D	108	- 530

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - H	109	- 678	D - F	134	- 618

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

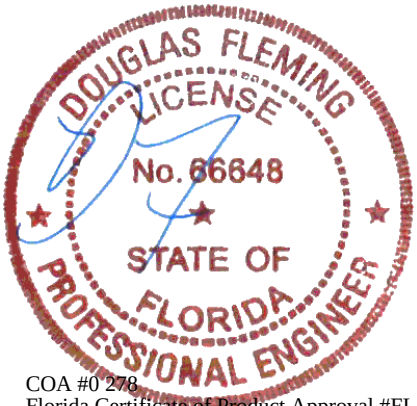
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.



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01/09/2024

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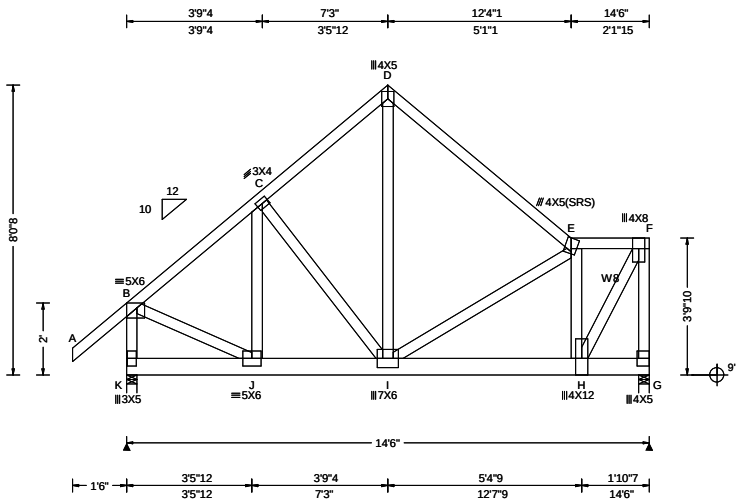
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24666 COMN FROM:	Ply: 2 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** C04		Cust: R 215 JRef:1XWb2150004 T65 DrwNo: 009.24.1314.54610 GA / DF 01/09/2024	

2 Complete Trusses Required



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	0.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.044	I	999	240
VERT(CL): 0.089	I	999	180
HORZ(LL): 0.021	C	-	-
HORZ(TL): 0.041	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.251
Max BC CSI: 0.462
Max Web CSI: 0.847

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
K	4894	/-	/-	/-	/153	/-
G	5690	/-	/-	/28	/-	/-

Wind reactions based on MWFRS
K Brg Wid = 3.5 Min Req = 2.0 (Truss)
G Brg Wid = 3.5 Min Req = 2.4 (Truss)
Bearings K & G are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	64	- 2177	D - E	45	- 1862
C - D	40	- 1834	E - F	0	- 1561

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
J - I	1619	- 43	I - H	1655	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - K	75	- 2228	E - H	65	- 1110
B - J	1789	- 48	H - F	3246	0
J - C	440	- 23	F - G	0	- 2872
D - I	2224	- 33			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W8 2x4 SP #2;

Nailnote

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @ 4.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	66 plf at	-1.50 to	66 plf at	2.06
TC: From	33 plf at	2.06 to	33 plf at	10.06
TC: From	66 plf at	10.06 to	66 plf at	14.50
BC: From	5 plf at	-1.50 to	5 plf at	0.00
BC: From	10 plf at	0.00 to	10 plf at	14.50

BC: 1391 lb Conc. Load at 2.06, 4.06, 6.06, 8.06
BC: 1359 lb Conc. Load at 10.06,12.06,13.23

Purlins

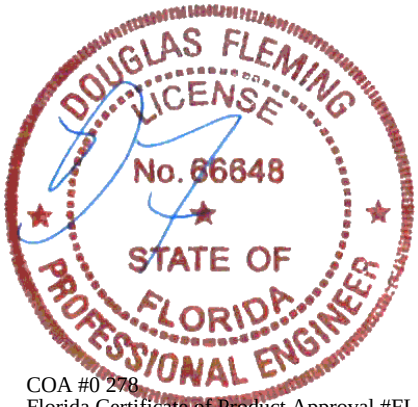
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.



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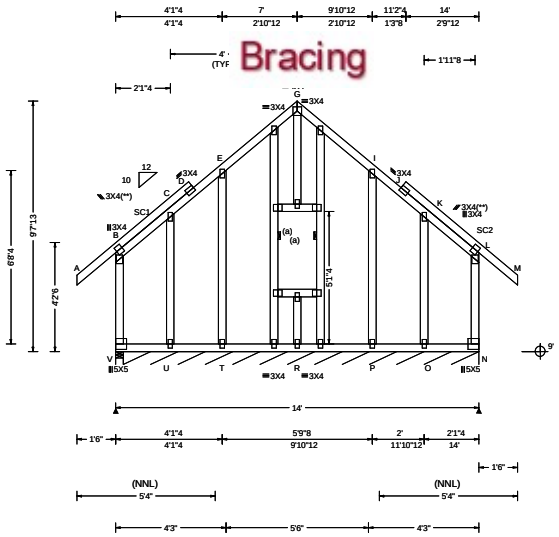
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24668 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** C05	Cust: R 215 JRef:1XWb2150004 T50 DrwNo: 009.24.1315.25863 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.02 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.006	E	999	240
VERT(CL): 0.016	E	999	180
HORZ(LL): 0.481	K	-	-
HORZ(TL): 0.819	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.227
Max BC CSI: 0.221
Max Web CSI: 0.478

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
V	239	/-	/-	/266	/91	/160
N*	86	/-	/-	/57	/-	/-

Wind reactions based on MWFRS
V Brg Wid = 3.5 Min Req = 1.5 (Truss)
N Brg Wid = 164 Min Req = -
Bearings V & V are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Fasten rated sheathing to one face of this frame.

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

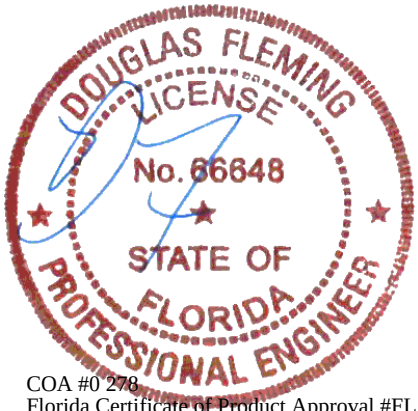
Laterally brace chord above/below filler at 24" OC (or as designed) including a lateral brace on chord directly above/ below both ends of filler (if no rigid diaphragm exists at that point)

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



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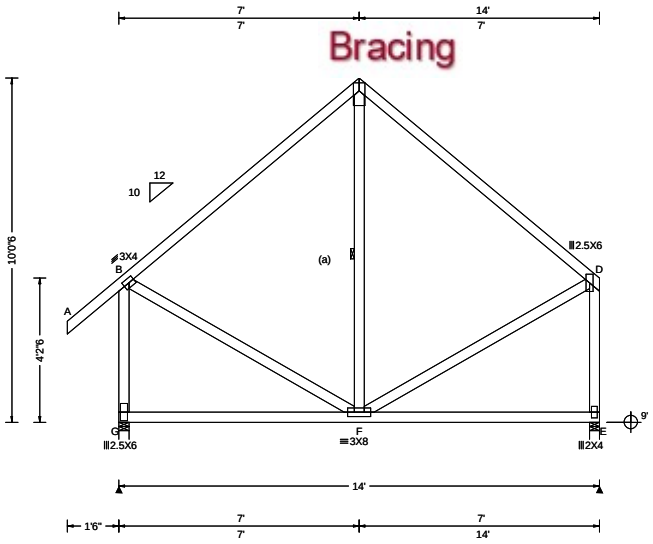
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24670 COMN FROM:	Ply: 1 Qty: 2	**Job Number:** 23-0295 Stewart Residence **Truss Label:** C06	Cust: R 215 JRef:1XWb2150004 T11 DrwNo: 009.24.1315.30790 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.49 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.006	C	999	240
VERT(CL): 0.012	C	999	180
HORZ(LL): 0.002	C	-	-
HORZ(TL): 0.003	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.669
Max BC CSI: 0.437
Max Web CSI: 0.290

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G	715	/-	/-	/385	/13	/149
E	597	/-	/-	/361	/13	/-

Wind reactions based on MWFRS
G Brg Wid = 3.5 Min Req = 1.5 (Truss)
E Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings G & E are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	190	-465	C - D	179	-459

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - G	220	-659	D - E	170	-540

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

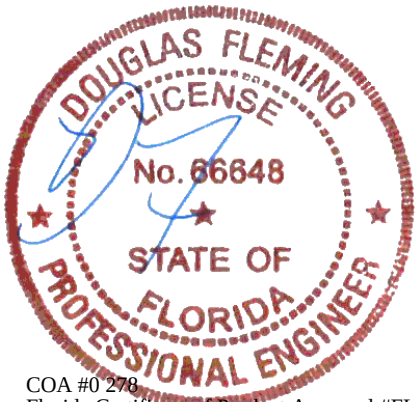
(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

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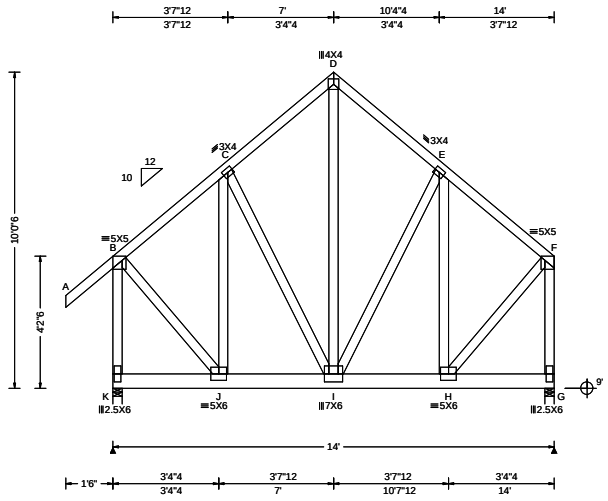
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24672 COMN FROM:	Ply: 2 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** C07	Cust: R 215 JRef:1XWb2150004 T61 DrwNo: 009.24.1315.50860 GA / DF 01/09/2024

2 Complete Trusses Required



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 0.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.49 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.026	I	999	240
VERT(CL): 0.052	I	999	180
HORZ(LL): 0.013	C	-	-
HORZ(TL): 0.026	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.136
Max BC CSI: 0.206
Max Web CSI: 0.560

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
K	4957	/-	/-	/68	/-	/-
G	4789	/-	/-	/203	/-	/-

Wind reactions based on MWFRS
K Brg Wid = 3.5 Min Req = 2.1 (Truss)
G Brg Wid = 3.5 Min Req = 2.0 (Truss)
Bearings K & G are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	0	-1311	D - E	3	-1186
C - D	3	-1184	E - F	0	-1280

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
J - I	974	0	I - H	964	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - K	0	-2026	H - F	1451	0
B - J	1469	0	F - G	0	-1923
D - I	1423	0			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Nailnote

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @ 5.00" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

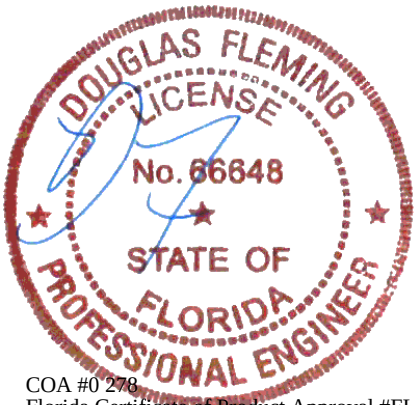
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	66 plf at	-1.50 to	66 plf at	1.06
TC: From	33 plf at	1.06 to	33 plf at	14.00
BC: From	5 plf at	-1.50 to	5 plf at	0.00
BC: From	10 plf at	0.00 to	10 plf at	14.00

BC: 1362 lb Conc. Load at 1.06
BC: 1355 lb Conc. Load at 3.06
BC: 1235 lb Conc. Load at 5.06
BC: 1232 lb Conc. Load at 7.06
BC: 1251 lb Conc. Load at 9.06
BC: 1260 lb Conc. Load at 11.06
BC: 1306 lb Conc. Load at 13.06

Wind

Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

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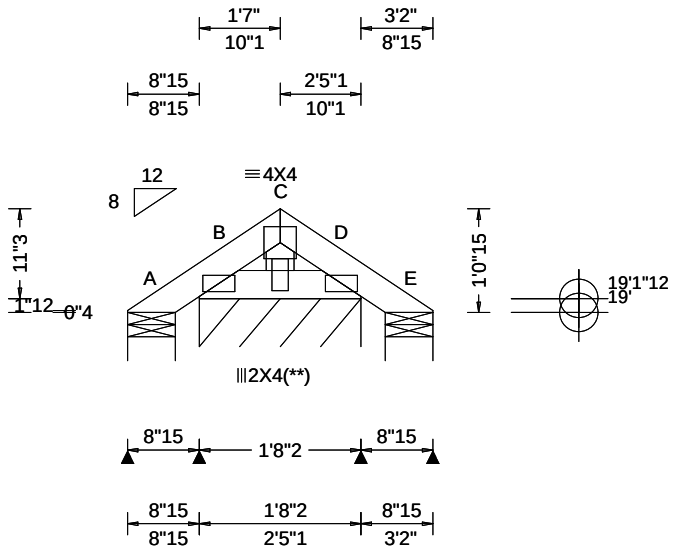
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24692 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** PB01	Cust: R 215 JRef:1XWb2150004 T71 DrwNo: 009.24.1315.59550 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.000	B	999	240
VERT(CL): 0.000	B	999	180
HORZ(LL): 0.000	B	-	-
HORZ(TL): 0.000	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.007
Max BC CSI: 0.003
Max Web CSI: 0.005

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	20	/-	/-	/20	/5	/17
B*	86	/-	/-	/63	/7	/-
E	20	/-	/-	/17	/4	/-

Wind reactions based on MWFRS
A Brg Wid = 5.9 Min Req = 1.5 (Truss)
B Brg Wid = 20.1 Min Req = -
E Brg Wid = 5.9 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

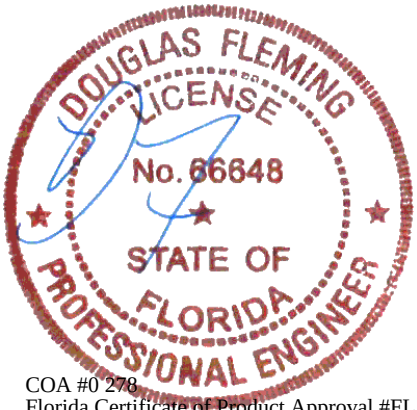
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

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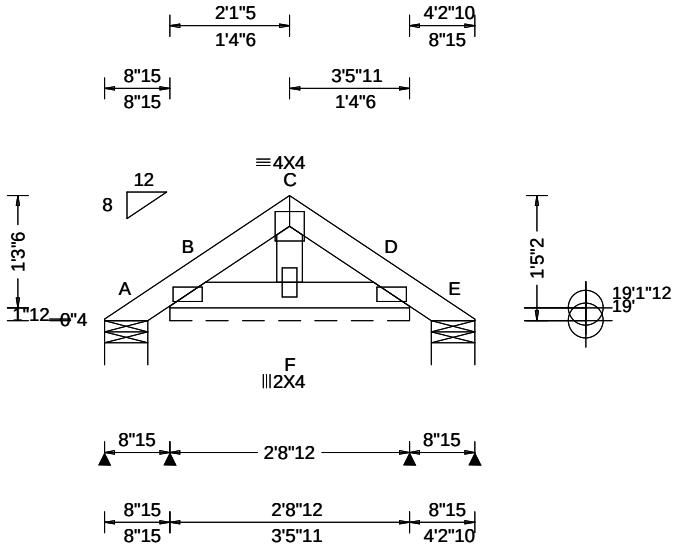
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24694 SPEC FROM:	Ply: 1 Qty: 9	**Job Number:** 23-0295 Stewart Residence **Truss Label:** PB02	Cust: R 215 JRef:1XWb2150004 T2 DrwNo: 009.24.1316.03133 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 0.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.000	B	999	240
VERT(CL): 0.000	B	999	180
HORZ(LL): 0.000	B	-	-
HORZ(TL): 0.000	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.015
Max BC CSI: 0.011
Max Web CSI: 0.008

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	16	/-	/-	/23	/10	/24
B*	82	/-	/-	/81	/-	/-
E	16	/-	/-	/21	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 5.9 Min Req = 1.5 (Truss)
B Brg Wid = 32.7 Min Req = -
E Brg Wid = 5.9 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

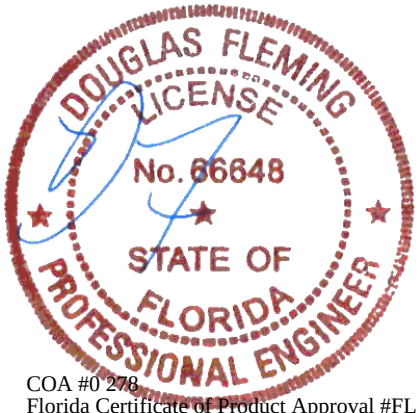
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details.



COA #0 278
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01/09/2024

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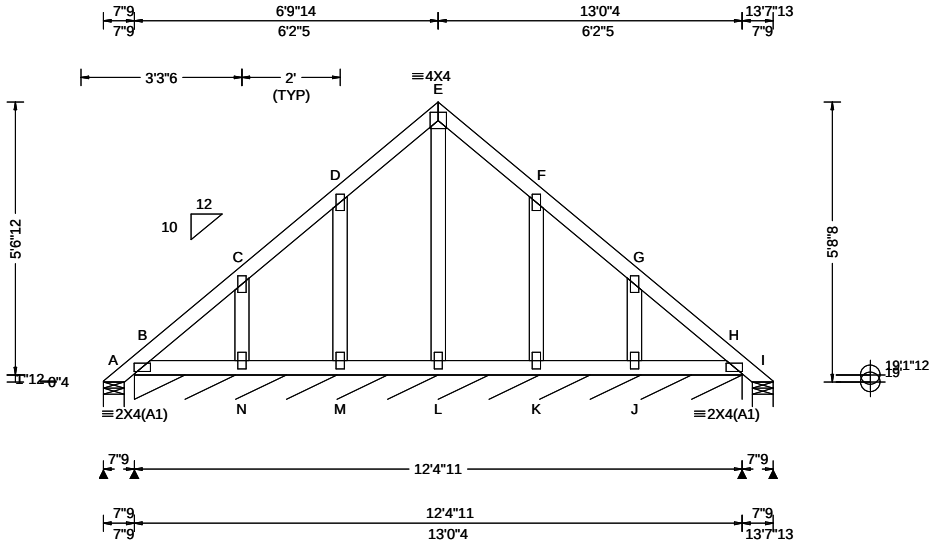
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 169810 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** PB03		Cust: R 215 JRef:1XWb2150004 T10 DrwNo: 009.24.1316.05690 GA / DF 01/09/2024	



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 D 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.001 D 999 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.001 F - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.002 F - -
NCBCLL:	10.00	Mean Height: 21.87 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.053
Load Duration:	1.25	BCDL: 2.0 psf	TPI Std: 2014	Max BC CSI: 0.019
Spacing:	24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.044
		C&C Dist a: 3.70 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 13.00 ft	Plate Type(s):	
		GCpi: 0.18	WAVE	VIEW Ver: 21.02.00B.1108.20
		Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	3	/-	/-	/93	/-	/120
B*	73	/-	/-	/49	/2	/-
I	4	/-	/-	/10	/6	/-

Wind reactions based on MWFRS
A Brg Wid = 5.1 Min Req = 1.5 (Truss)
B Brg Wid = 148 Min Req = -
I Brg Wid = 5.1 Min Req = 1.5 (Truss)
Bearings A, B, & I are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

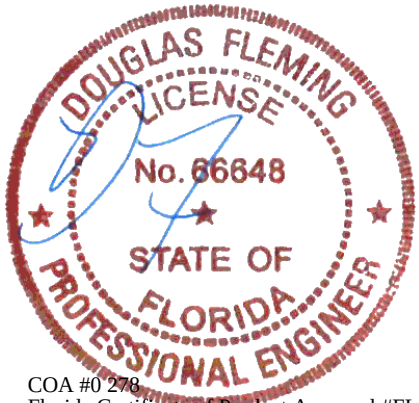
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

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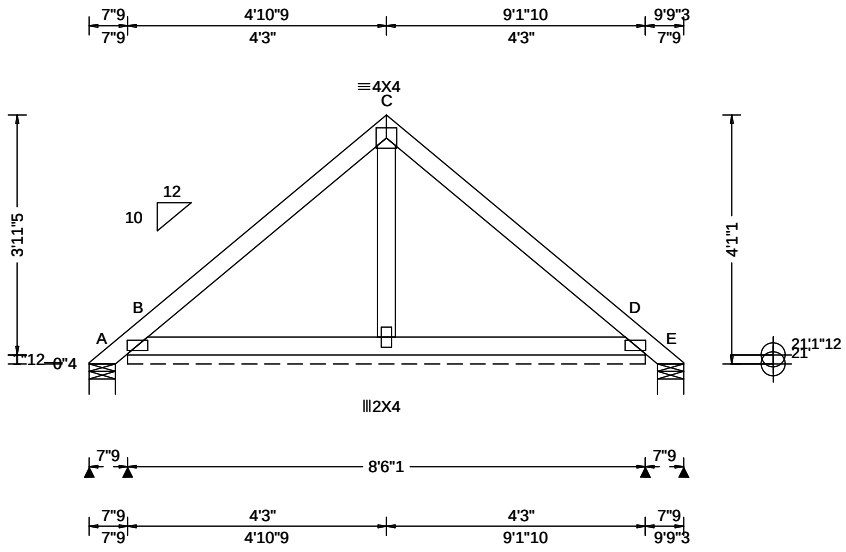
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 16011 COMN FROM:	Ply: 1 Qty: 11	**Job Number:** 23-0295 Stewart Residence **Truss Label:** PB04	Cust: R 215 JRef:1XWb2150004 T13 DrwNo: 009.24.1316.09703 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 18.02 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 3.10 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	B	999	240
VERT(CL): 0.002	D	999	180
HORZ(LL): -0.001	D	-	-
HORZ(TL): 0.002	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.211
Max BC CSI: 0.089
Max Web CSI: 0.028

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs), or *=PLF

		Gravity		Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-129	/-	/102	/169	/86
B*	108	/-	/-	/78	/1	/-
E	-	/-129	/-	/46	/113	/-

Wind reactions based on MWFRS
A Brg Wid = 5.2 Min Req = 1.5 (Truss)
B Brg Wid = 102 Min Req = -
E Brg Wid = 5.2 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

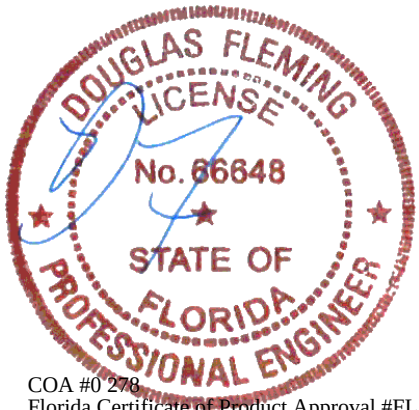
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes
All plates are 2X4(A1) except as noted.

Loading
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Refer to DWG PB160160118 for piggyback details.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

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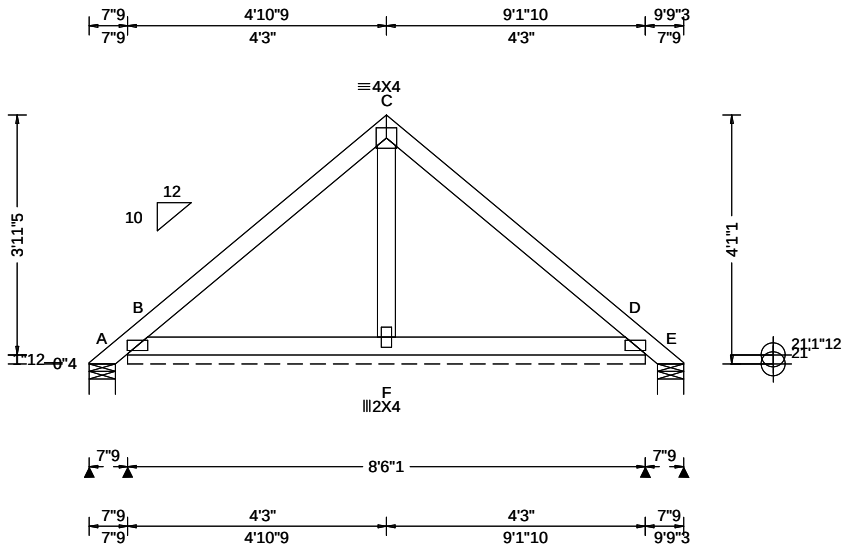
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24706 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** PB05	Cust: R 215 JRef:1XWb2150004 T19 DrwNo: 009.24.1316.11393 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 0.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	B	999	240
VERT(CL): 0.003	D	999	180
HORZ(LL): -0.001	D	-	-
HORZ(TL): 0.002	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.211
Max BC CSI: 0.145
Max Web CSI: 0.028

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs), or *=PLF

		Gravity		Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-129	/-	/99	/183	/86
B*	108	/-	/-	/102	/-	/-
E	-	/-129	/-	/46	/120	/-

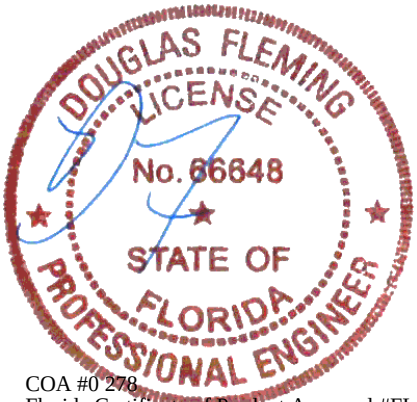
Wind reactions based on MWFRS
A Brg Wid = 5.2 Min Req = 1.5 (Truss)
B Brg Wid = 102 Min Req = -
E Brg Wid = 5.2 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes
All plates are 2X4(A1) except as noted.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
Refer to DWG PB160160118 for piggyback details.



COA #0 278
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01/09/2024

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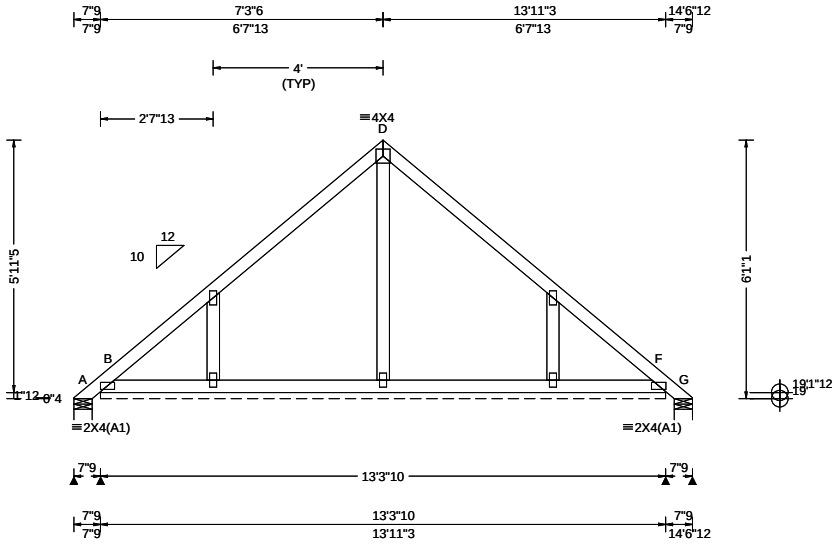
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24708 SPEC FROM:	Ply: 1 Qty: 3	**Job Number:** 23-0295 Stewart Residence **Truss Label:** PB06	Cust: R 215 JRef:1XWb2150004 T39 DrwNo: 009.24.1316.14883 GA / DF 01/09/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 D 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.002 D 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 E - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.002 E - -
NCBCLL: 10.00	Mean Height: 18.02 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.221
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.058
Spacing: 24.0 "	MWFRS Parallel Dist: > 2h	Rep Fac: Yes	Max Web CSI: 0.102
	C&C Dist a: 3.34 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 13.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 22.01.00.0314.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	23	/-	/-	/100	/84	/129
B*	71	/-	/-	/56	/-	/-
G	23	/-	/-	/14	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 5.2 Min Req = 1.5 (Truss)
B Brg Wid = 159 Min Req = -
G Brg Wid = 5.2 Min Req = 1.5 (Truss)
Bearings A, B, & G are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

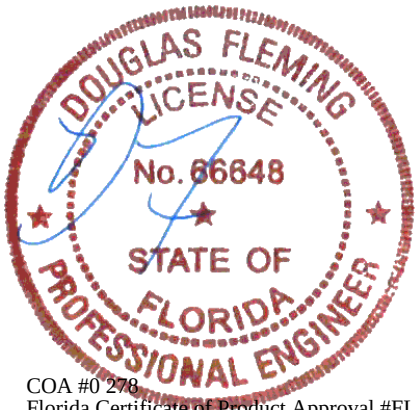
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

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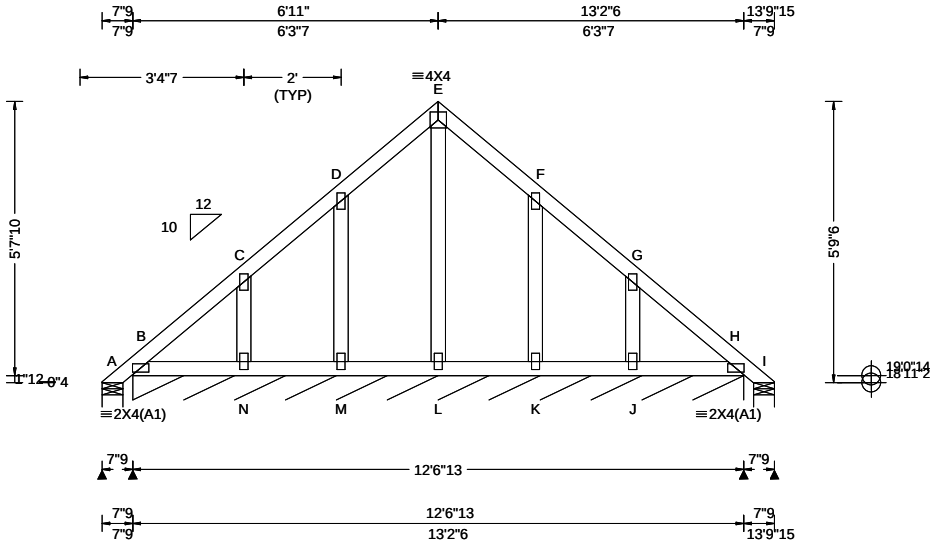
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24710 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** PB07	Cust: R 215 JRef:1XWb2150004 T36 DrwNo: 009.24.1316.28150 GA / DF 01/09/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 D 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.001 F 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.000 D - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.001 D - -
NCBCLL: 10.00	Mean Height: 21.83 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.052
Load Duration: 1.25	BCDL: 2.0 psf	TPI Std: 2014	Max BC CSI: 0.014
Spacing: 24.0 "	MWFRS Parallel Dist: > 2h	Rep Fac: Yes	Max Web CSI: 0.045
	C&C Dist a: 3.63 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 13.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 22.01.00.0314.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	0	/-1	/-	/0	/-	/-
B*	74	/-	/-	/0	/-	/-
I	1	/-	/-	/0	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 5.1 Min Req = 1.5 (Truss)
B Brg Wid = 150 Min Req = -
I Brg Wid = 5.1 Min Req = 1.5 (Truss)
Bearings A, B, & I are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

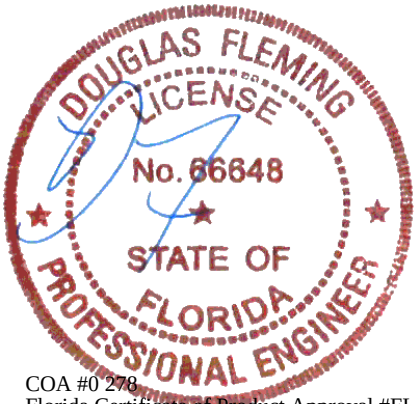
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details.



COA #0 278
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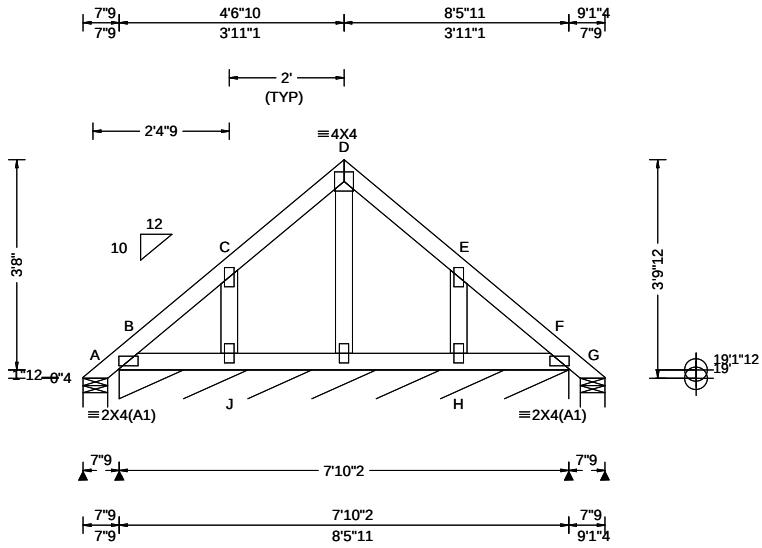
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24718 GABL FROM:	Ply: 1 Qty: 2	**Job Number:** 23-0295 Stewart Residence **Truss Label:** PB08	Cust: R 215 JRef:1XWb2150004 T28 DrwNo: 009.24.1316.30087 GA / DF 01/09/2024	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.92 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 D 999 240 VERT(CL): 0.000 D 999 180 HORZ(LL): 0.001 E - - HORZ(TL): 0.001 E - - Creep Factor: 2.0 Max TC CSI: 0.051 Max BC CSI: 0.018 Max Web CSI: 0.031 VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	12	/-	/-	/61	/49	/76
B*	75	/-	/-	/57	/11	/-
G	12	/-	/-	/10	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 5.2 Min Req = 1.5 (Truss)
B Brg Wid = 94.1 Min Req = -
G Brg Wid = 5.2 Min Req = 1.5 (Truss)
Bearings A, B, & G are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

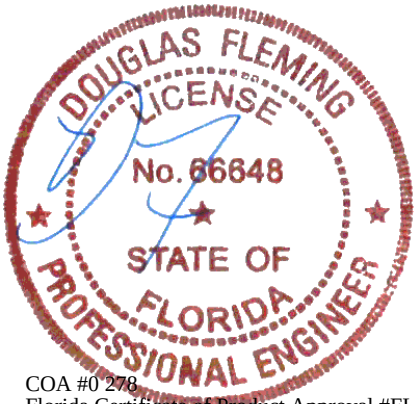
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



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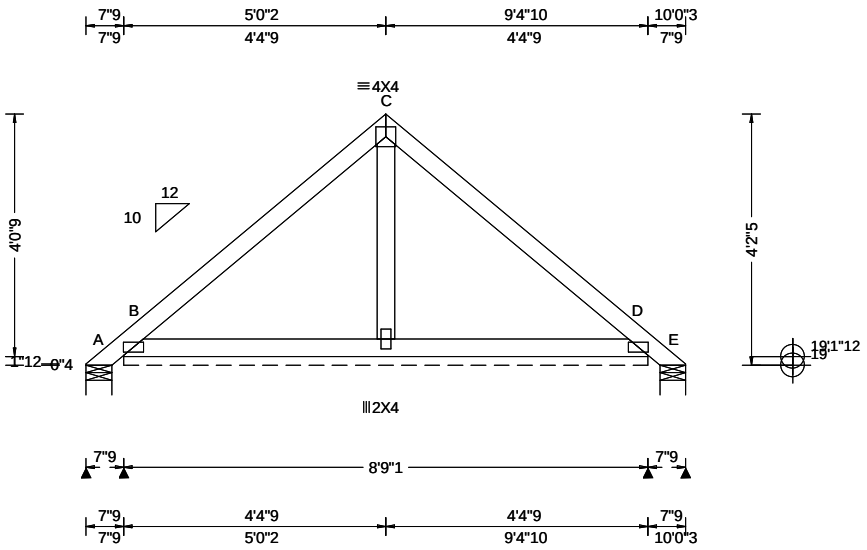
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24712 COMN FROM:	Ply: 1 Qty: 11	**Job Number:** 23-0295 Stewart Residence **Truss Label:** PB09	Cust: R 215 JRef:1XWb2150004 T23 DrwNo: 009.24.1316.32750 GA / DF 01/09/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 D 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.001 D 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 D - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.002 D - -
NCBCLL: 10.00	Mean Height: 17.64 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.226
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.086
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.029
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 22.01.00.0314.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-139	/-	/104	/172	/84
B*	109	/-	/-	/75	/28	/-
E	-	/-139	/-	/75	/117	/-

Wind reactions based on MWFRS
A Brg Wid = 5.2 Min Req = 1.5 (Truss)
B Brg Wid = 105 Min Req = -
E Brg Wid = 5.2 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

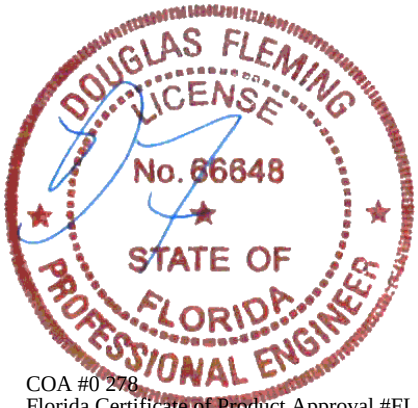
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



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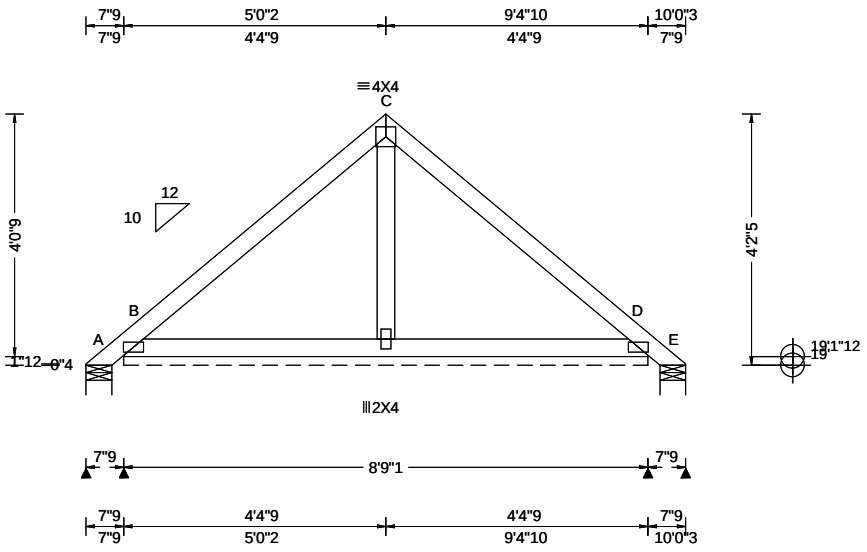
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 17557 COMN FROM:	Ply: 1 Qty: 9	**Job Number:** 23-0295 Stewart Residence **Truss Label:** PB10	Cust: R 215 JRef:1XWb2150004 T16 DrwNo: 009.24.1316.37850 GA / DF 01/09/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 17.64 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	D	999	240
VERT(CL): 0.002	D	999	180
HORZ(LL): -0.001	D	-	-
HORZ(TL): 0.002	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.226
Max BC CSI: 0.090
Max Web CSI: 0.029

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-139	/-	/104	/172	/84
B*	109	/-	/-	/76	/8	/-
E	-	/-140	/-	/48	/119	/-

Wind reactions based on MWFRS
A Brg Wid = 5.2 Min Req = 1.5 (Truss)
B Brg Wid = 105 Min Req = -
E Brg Wid = 5.2 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

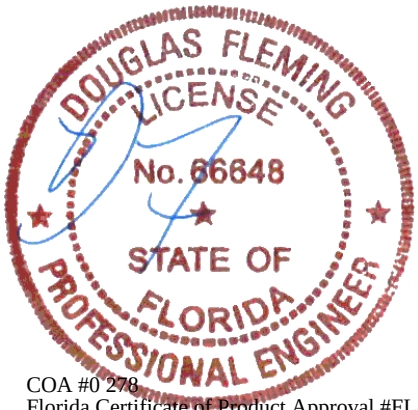
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes
All plates are 2X4(A1) except as noted.

Loading
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Refer to DWG PB160160118 for piggyback details.



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01/09/2024

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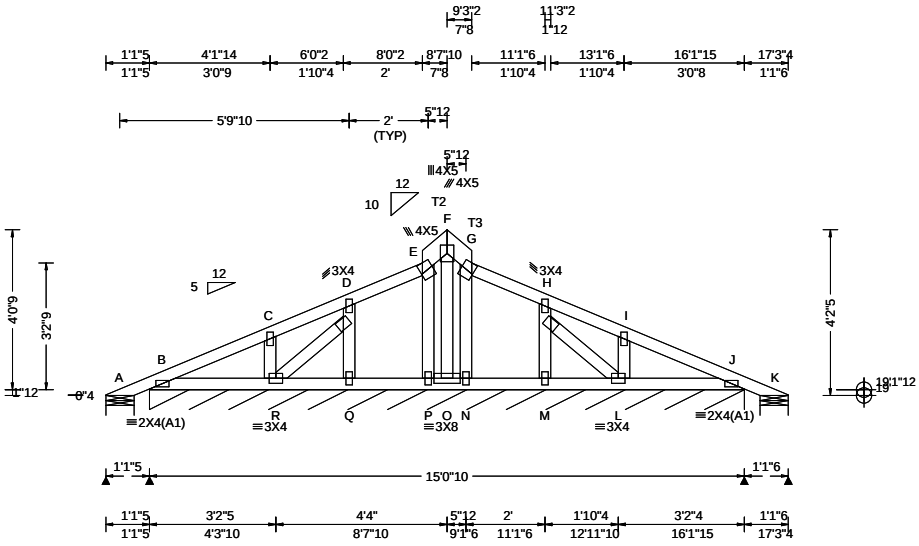
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 17570 COMN FROM:	Ply: 1 Qty: 2	**Job Number:** 23-0295 Stewart Residence **Truss Label:** PB11	Cust: R 215 JRef:1XWb2150004 T27 DrwNo: 009.24.1316.39890 GA / DF 01/09/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 B 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.002 B 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 J - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.001 J - -
NCBCLL: 10.00	Mean Height: 21.11 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.062
Load Duration: 1.25	BCDL: 2.0 psf	TPI Std: 2014	Max BC CSI: 0.035
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.030
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 4.50 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 22.01.00.0314.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	3	/0	/-	/21	/27	/56
B*	72	/-	/-	/46	/17	/-
K	3	/-	/-	/-	/5	/-

Wind reactions based on MWFRS
A Brg Wid = 8.6 Min Req = 1.5 (Truss)
B Brg Wid = 180 Min Req = -
K Brg Wid = 8.6 Min Req = 1.5 (Truss)
Bearings A, B, & K are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2; T2,T3 2x6 SP 2400f-2.0E;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

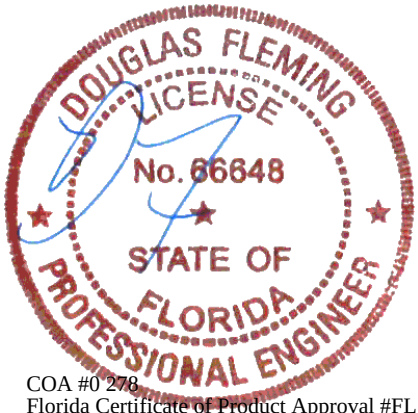
Wind

Wind loads based on MWFRS.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details.



COA #0 278
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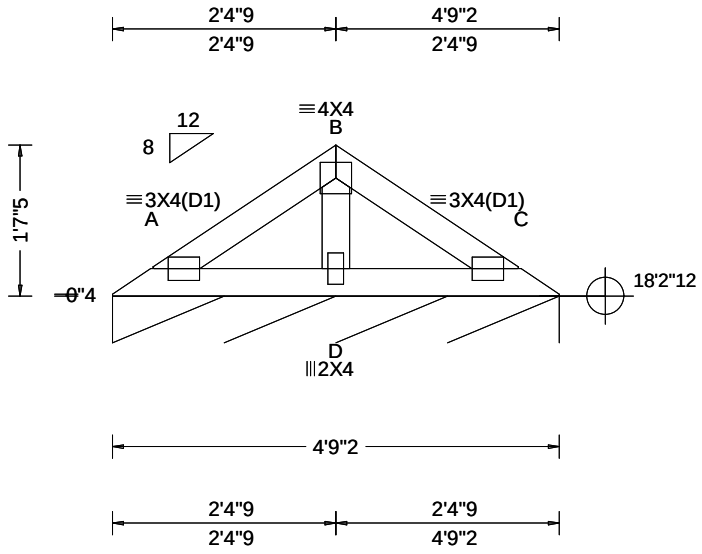
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24720 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** V01	Cust: R 215 JRef:1XWb2150004 T40 DrwNo: 009.24.1316.42080 GA / DF 01/09/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 A 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.003 A 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.001 C - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.001 C - -
NCBCLL: 10.00	Mean Height: 19.19 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.058
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.044
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.031
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 22.01.00.0314.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	83	/-	/-	/40	/-	/5

Wind reactions based on MWFRS
C Brg Wid = 57.1 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

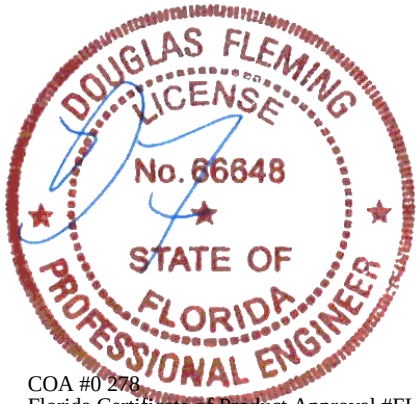
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



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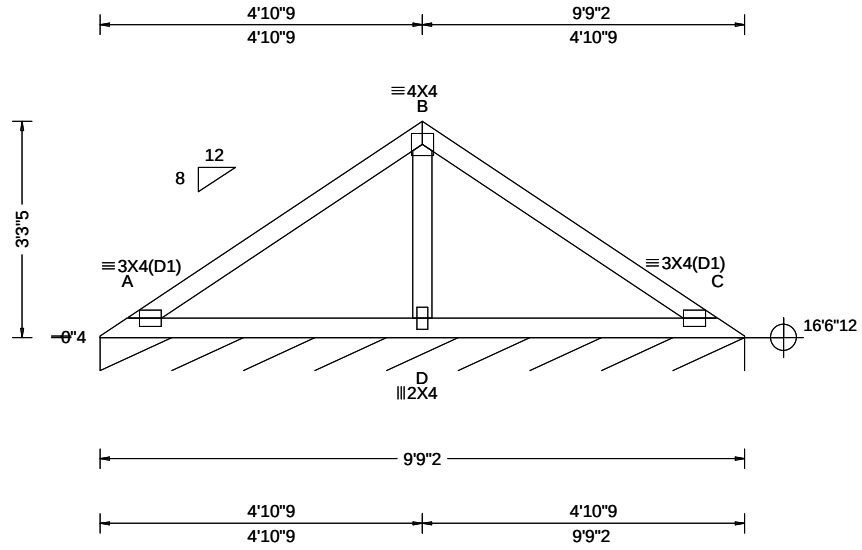
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24722 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** V02	Cust: R 215 JRef:1XWb2150004 T45 DrwNo: 009.24.1316.51203 GA / DF 01/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 18.35 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.011 A	999	240
VERT(CL):	0.024 A	999	180
HORZ(LL):	-0.006 C	-	-
HORZ(TL):	0.012 C	-	-

Creep Factor: 2.0
Max TC CSI: 0.326
Max BC CSI: 0.276
Max Web CSI: 0.123

VIEW Ver: 22.01.00.0314.20

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	84	/-	/-	/43	/-	/6

Wind reactions based on MWFRS
C Brg Wid = 117 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
B - D	189	-526

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

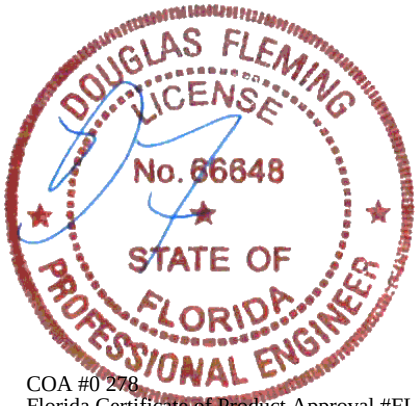
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



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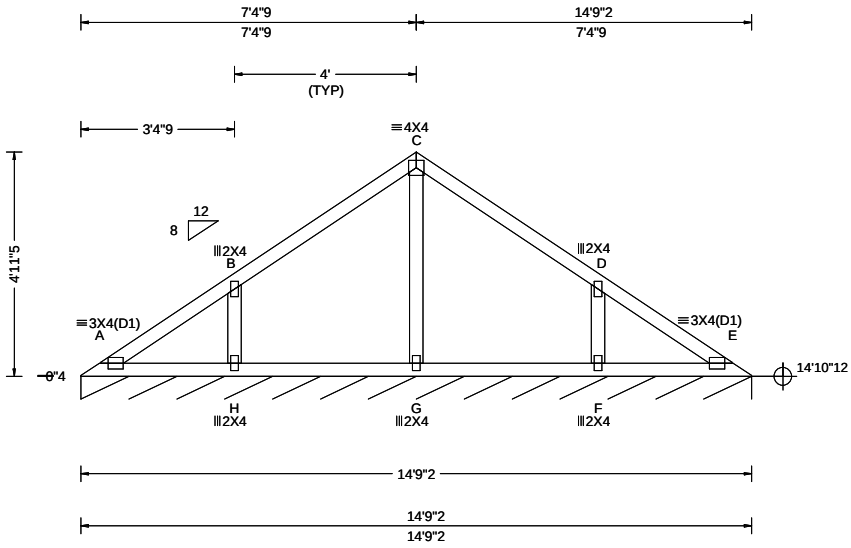
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24724 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 23-0295 Stewart Residence **Truss Label:** V03		Cust: R 215 JRef:1XWb2150004 T14 DrwNo: 009.24.1317.03290 GA / DF 01/09/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**	**▲Maximum Reactions (lbs), or *=PLF**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 17.52 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 E 999 240 VERT(CL): 0.005 E 999 180 HORZ(LL): -0.001 E - - HORZ(TL): 0.002 E - - Creep Factor: 2.0 Max TC CSI: 0.259 Max BC CSI: 0.117 Max Web CSI: 0.093 VIEW Ver: 22.01.00.0314.20	Gravity / Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 84 /- /- /44 /- /6 Wind reactions based on MWFRS E Brg Wid = 177 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

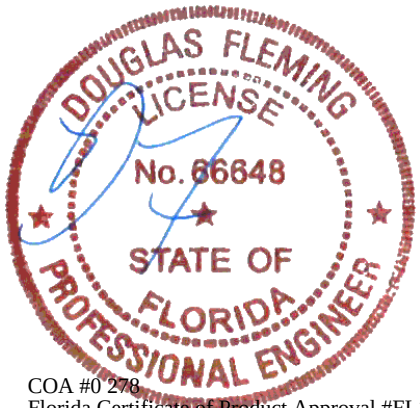
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL1999
01/09/2024

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
		#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
		Stud	3' 10"	6' 2"	6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
		Standard	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"
	SP DFL	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
		#2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
		#3	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
		Stud	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
		Standard	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"
16" o.c.	SPF HF	#1 / #2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 5"	6' 5"	6' 10"	8' 7"	9' 2"	11' 0"	11' 6"	13' 6"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

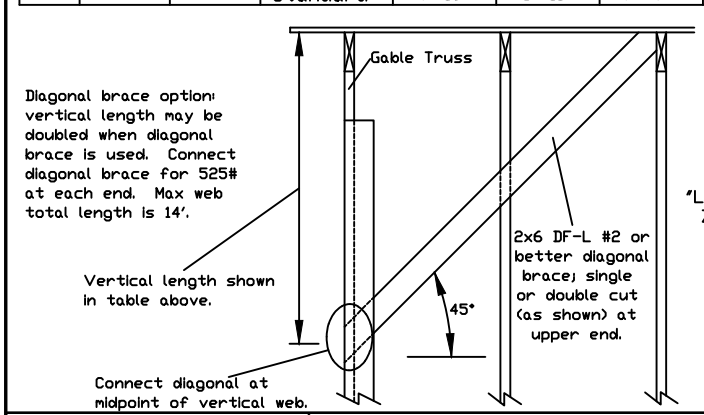
"L" bracing must be a minimum of 80% of web member length.

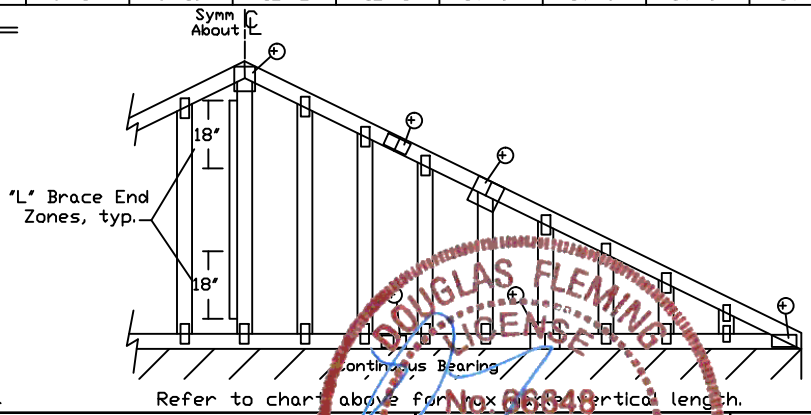
Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

COA #0 278

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-16-GAB14030
DATE	01/26/2018
DRWG	A14030ENC160118

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

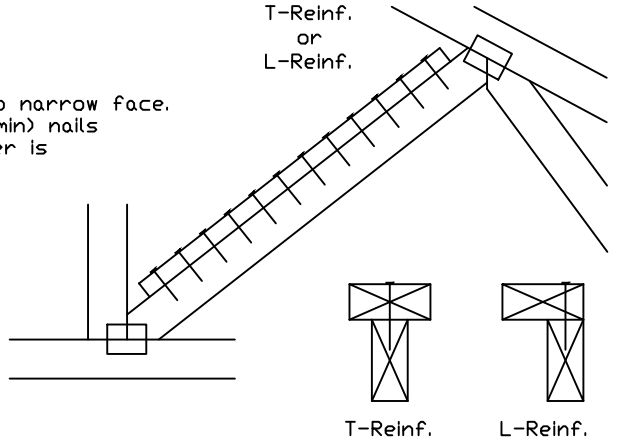
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

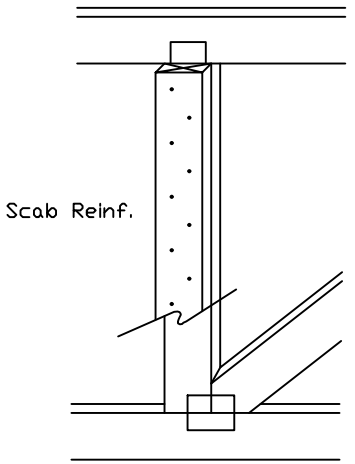
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/09/2024

COA #0 278

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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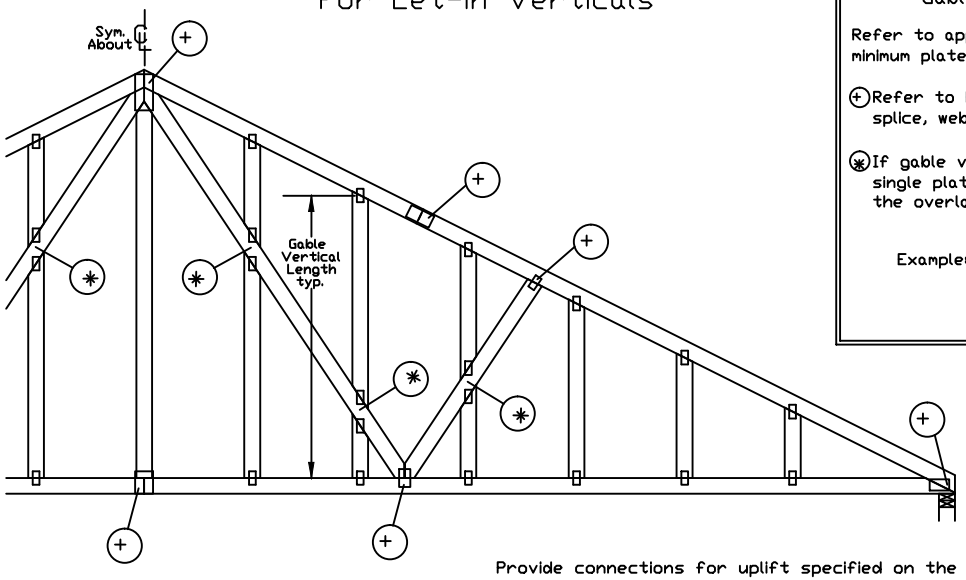
For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

TC LL		PSF
TC DL		PSF
BC DL		PSF
BC LL		PSF
TOT. LD.		PSF
DUR. FAC.		
SPACING		

REF	CLR Subst.
DATE	01/02/19
DRWG	BRCLBSUB0119

Gable Detail
For Let-in Verticals



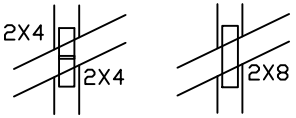
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

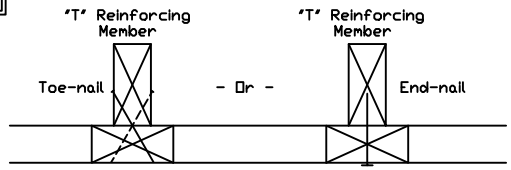
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊛ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



"T" Reinforcement Attachment Detail



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
10d Common (0.148"x 3.",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

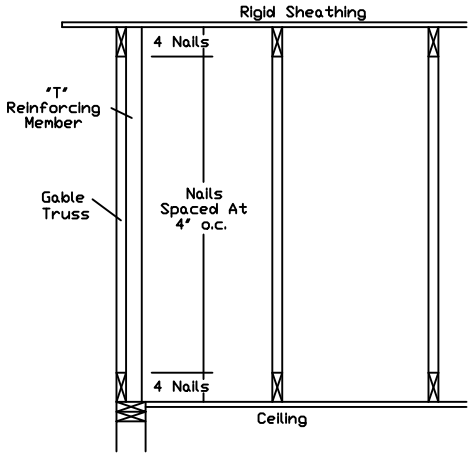
Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118, A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/09/2024

COA #0 278

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH,GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

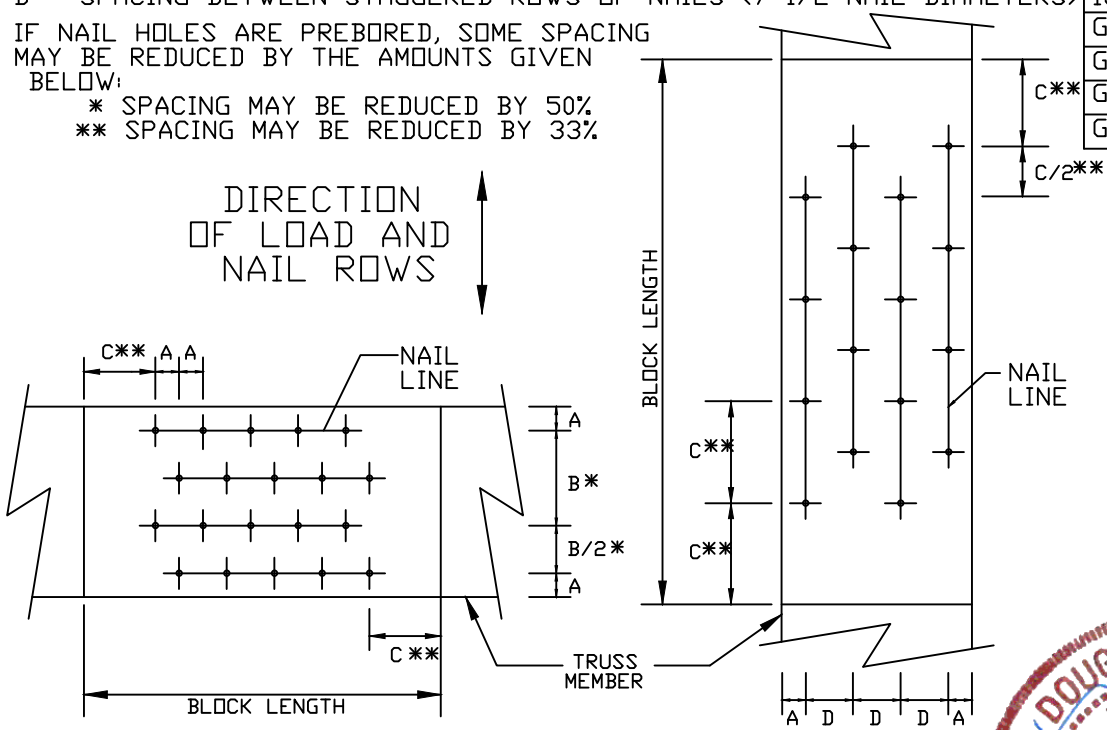
LOAD PERPENDICULAR TO GRAIN
A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN
A - EDGE DISTANCE (6 NAIL DIAMETERS)
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
* SPACING MAY BE REDUCED BY 50%
** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3.",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1'	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN LOAD APPLIED PARALLEL TO GRAIN

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REF	NAIL SPACE
DATE	10/01/14
DRWG	CNNAILSP1014

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Stud	4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"
	SP DFL	#1	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
		Stud	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
		Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

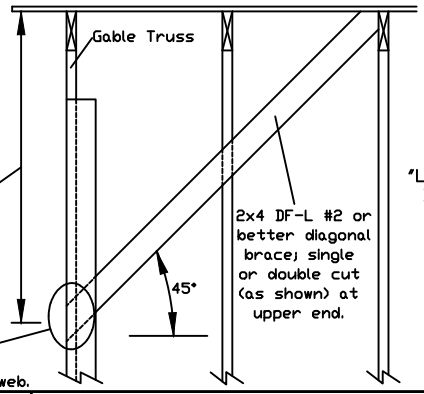
Gable Vertical Plate Sizes

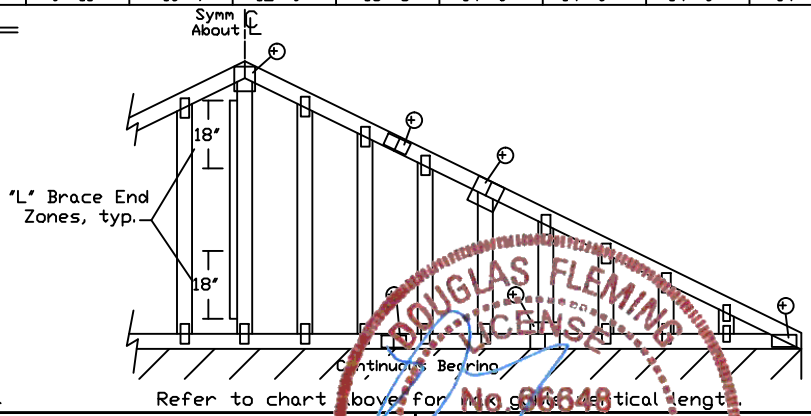
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450# at each end. Max web total length is 14'.





Refer to chart above for Max Gable Vertical Length.

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MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-16-GAB14015
DATE 01/26/2018
DRWG A14015ENC160118

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

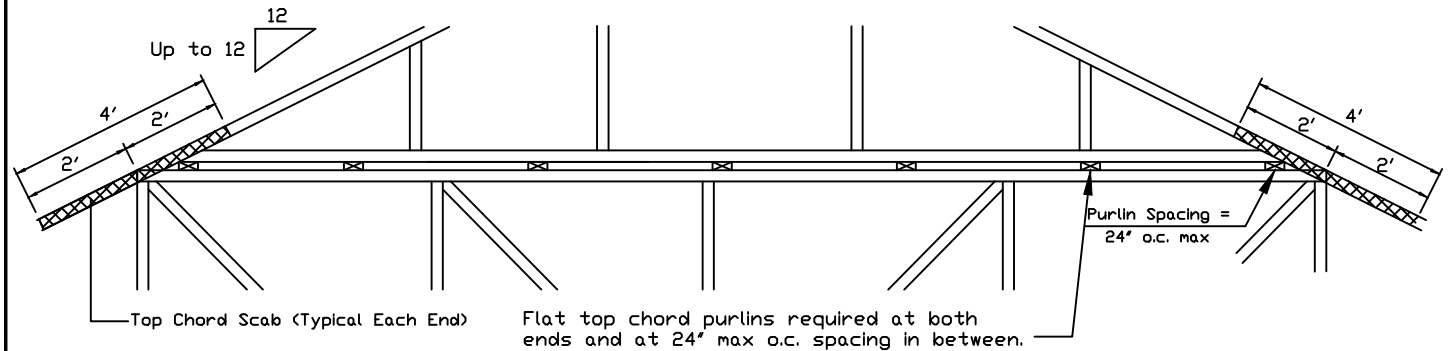
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

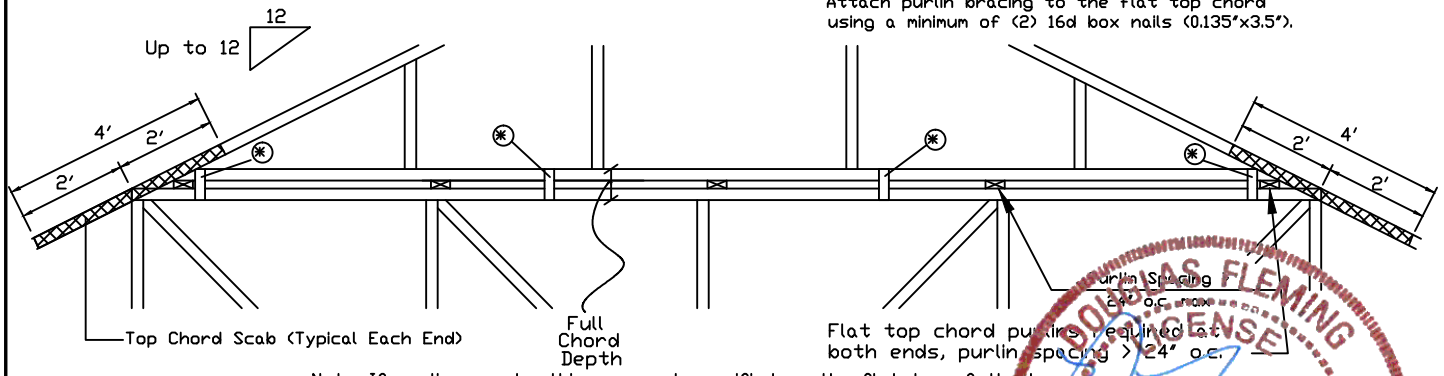
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24" o.c. max. and use Detail A

* In addition, provide connection with one of the following methods:

Trulox
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

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LICENSE
No. 66848
STATE OF FLORIDA
PROFESSIONAL ENGINEER

01/09/2024

COA #0 278

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118
SPACING	24.0"

Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
535# connection or with (1) Simpson H2.5A or equivalent connector for
ASCE 7-16 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
Or
ASCE 7-16 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

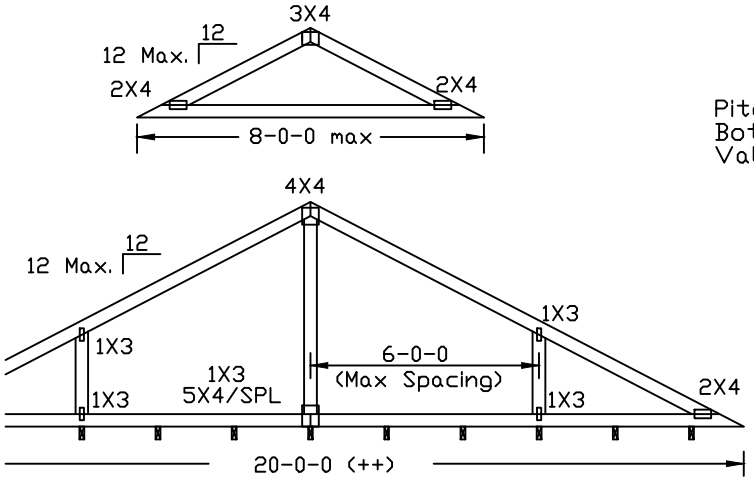
All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

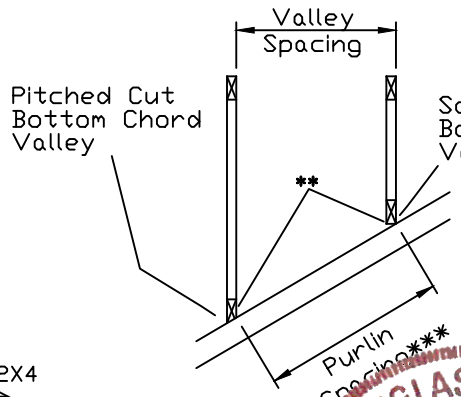
Top chord of truss beneath valley set must be braced with:
properly attached, rated sheathing applied prior to valley truss installation.
Or
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

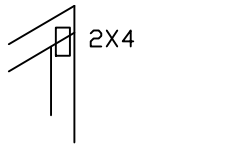
*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

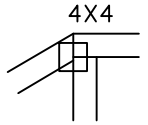
++ Larger spans may be built as long as the vertical height does not exceed 14'-0".

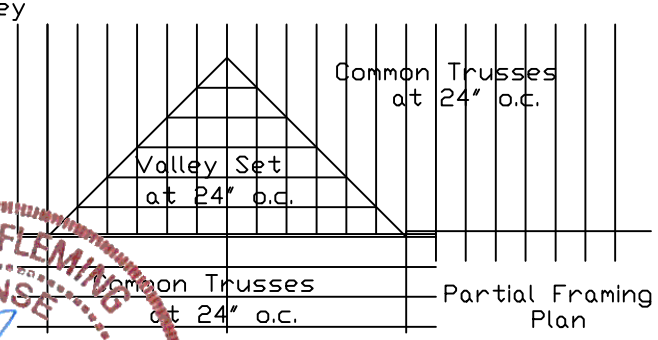


Supporting trusses at 24" o.c. maximum spacing.









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COA #0 278

TC LL	30	30	40	PSF
TC DL	20	15	7	PSF
BC DL	10	10	10	PSF
BC LL	0	0	0	PSF
TOT. LD.	60	55	57	PSF
DUR.FAC.	1.25/1.33	1.15	1.15	
SPACING		24.0"		

REF	VALLEY DETAIL
DATE	01/26/2018
DRWG	VAL180160118

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
(2) 16d box (0.135" x 3.5") nails toe-nailed for
ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C,
Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
supporting truss material at connection location:
170 mph for SP (G = 0.55, min.),
155 mph for DF-L (G = 0.50, min.), or
120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

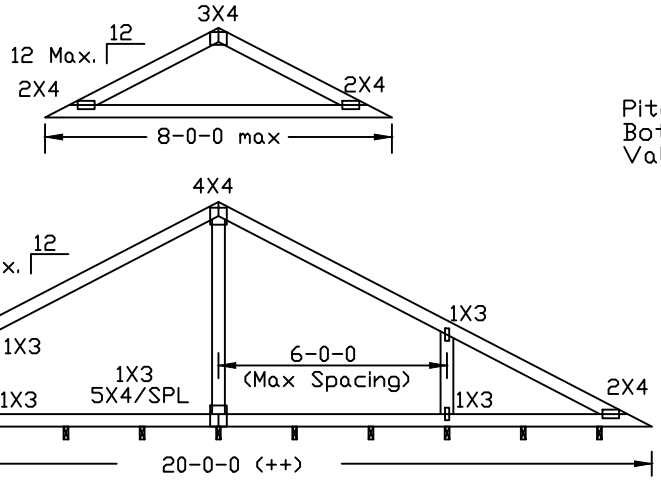
All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
properly attached, rated sheathing applied prior to valley truss installation.
Or
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



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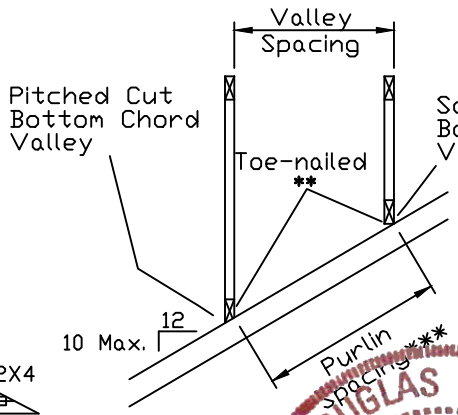

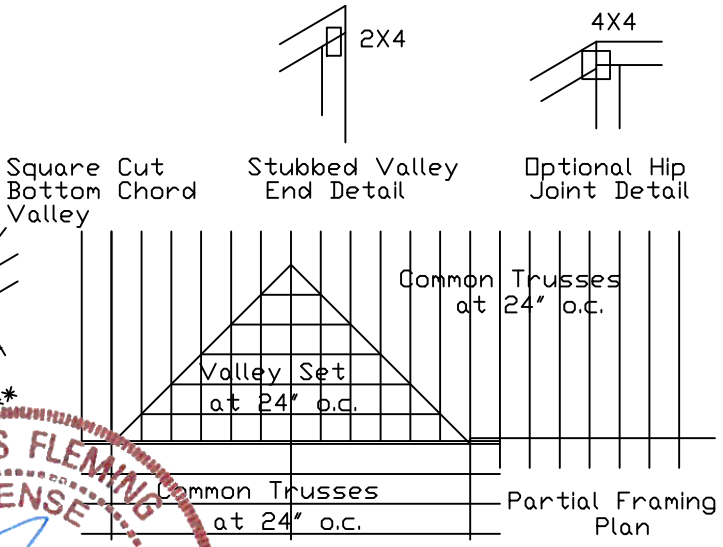

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IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

TC LL	30	30	40 PSF
TC DL	20	15	7 PSF
BC DL	10	10	10 PSF
BC LL	0	0	0 PSF
TOT. LD.	60	55	57 PSF
DUR.FAC.	1.25/1.33	1.15	1.15
SPACING		24.0"	

REF	VALLEY DETAIL
DATE	01/26/2018
DRWG	VALTN160118