

DATE 05/06/2009

Columbia County Building Permit

PERMIT

This Permit Must Be Prominently Posted on Premises During Construction

000027792

APPLICANT J. VINCE RICHARDSON PHONE 386.755.5779
ADDRESS 692 SW ARLINGTON BLVD LAKE CITY FL 32025
OWNER MICHELLE BOATWRIGHT PHONE 386-965-2290
ADDRESS 146 NW HARRIS LAKE DRIVE LAKE CITY FL 32055
CONTRACTOR J. VINCE RICHARDSON PHONE 352-472-9101

LOCATION OF PROPERTY 90 W, R COMMERCE DR, R FAIRWAY DR, R NW GRET LN, R NW HARRIS
LAKE DRIVE, 6TH ON THE RIGHT

TYPE DEVELOPMENT SCREEN ENCLOSURE ESTIMATED COST OF CONSTRUCTION 8100.00

HEATED FLOOR AREA TOTAL AREA HEIGHT 8.00 STORIES

FOUNDATION WALLS ROOF PITCH FLOOR

LAND USE & ZONING RSF-1 MAX. HEIGHT

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 1 FLOOD ZONE DEVELOPMENT PERMIT NO.

PARCEL ID 35-3S-16-02309-047 SUBDIVISION FAIRWAY VIEW

LOT 47 BLOCK PHASE UNIT 2 TOTAL ACRES

110
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor
EXISTING X-09-119 CFS HD N
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: NOC ON FILE.

Check # or Cash 2154

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power Foundation Monolithic
date/app. by date/app. by date/app. by

Under slab rough-in plumbing Slab Sheathing/Nailing
date/app. by date/app. by date/app. by

Framing Insulation
date/app. by date/app. by

Rough-in plumbing above slab and below wood floor Electrical rough-in
date/app. by date/app. by

Heat & Air Duct Peri. beam (Lintel) Pool
date/app. by date/app. by date/app. by

Permanent power C.O. Final Culvert
date/app. by date/app. by date/app. by

Pump pole Utility Pole M/H tie downs, blocking, electricity and plumbing
date/app. by date/app. by date/app. by

Reconnection RV Re-roof
date/app. by date/app. by date/app. by

BUILDING PERMIT FEE \$ 45.00 CERTIFICATION FEE \$ 0.00 SURCHARGE FEE \$ 0.00

MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$

FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ CULVERT FEE \$ TOTAL FEE 95.00

INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

51 X-09-119 in Box
Columbia County Building Permit Application

For Office Use Only Application # 0905-03 Date Received 5/1 By JW Permit # 27792
Application Approved by - Zoning Official afz Date 5/16/09 Plans Examiner HO Date 5-5-09
Flood Zone N/A Development Permit RSF-2 Land Use Plan Map Category RLD
Comments Let's go. ENGINEER
☒ NOC ☒ EH ☐ Deed or PA ☒ Site Plan ☐ State Road Info ☐ Parent Parcel # ☐ Development Per
Name Authorized Person Signing Permit Vince Richardson 623-3173 Fax 386-755-7889
Address 692 S.W. Arlington Blvd. Lake City, FL 32025 Phone 386-755-5779
Owners Name Michelle Louise Boatwright Phone 386-965-2290
911 Address 146 NW Harris Lake Dr. Lake City, FL 32055
Contractors Name Richardson Aluminum LLC Phone 386-755-5779
Address 692 S.W. Arlington Blvd. Lake City, FL 32025
Fee Simple Owner Name & Address _____
Bonding Co. Name & Address _____
Architect/Engineer Name & Address Lawrence E. Bennett
Mortgage Lenders Name & Address _____
Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive En
Property ID Number 35-3S-16-02309-047 HX Estimated Cost of Construction 8,100.00
Subdivision Name _____
Driving Directions 90 west to Commerce Blvd go North. Turn right
on Egret then right on Harris Lake Drive. 6th House on right
146 NW Harris Lake Drive
Type of Construction Screen Enclosure Number of Existing Dwellings on Property 1
Total Acreage _____ Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing C
Actual Distance of Structure from Property Lines - Front _____ Side 51 Side 60 Rear 18
Total Building Height 8' Number of Stories 1 Heated Floor Area 0 Roof Pitch Custom

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

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Michelle Boatwright
Owner Builder or Authorized Person by Notarized Letter

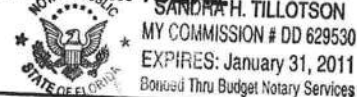
STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

this 30 day of April 2009

Personally known ☒ or Produced Identification ☐

Vince Richardson
Contractor Signature
Contractors License Number 000110
Competency Card Number _____
NOTARY STAMP/SEAL



Notary Signature

Columbia County Property

Appraiser, Columbia Fla., Co

DB Last Updated: 4/27/2009

2009 Preliminary Values

Tax Record

Property Card

Interactive GIS Map

Print

Parcel: 35-3S-16-02309-047 HX

Owner & Property Info

Search Result: 1 of 1

Owner's Name	BOATWRIGHT MICHELLE LOUISE		
Site Address	HARRIS LAKE		
Mailing Address	146 NW HARRIS LAKE DR LAKE CITY, FL 32055		
Use Desc. (code)	SINGLE FAM (000100)		
Neighborhood	026316.03	Tax District	2
UD Codes	MKTA06	Market Area	06
Total Land Area	0.000 ACRES		
Description	LOT 47 FAIRWAY VIEW S/D UNIT I ORB 426-222, 952-1724. DC GRACE GOODALL 1056-1898. WD 1071-794.		

GIS Aerial



Property & Assessment Values

Mkt Land Value	cnt: (1)	\$37,170.00
Ag Land Value	cnt: (0)	\$0.00
Building Value	cnt: (1)	\$151,959.00
XFOB Value	cnt: (1)	\$3,648.00
Total Appraised Value		\$192,777.00

Just Value	\$192,777.00
Class Value	\$0.00
Assessed Value	\$192,777.00
Exemptions	(code: HX) \$50,000.00
Total Taxable Value	County: \$142,777.00 City: \$142,777.00 Other: \$142,777.00 School: \$167,777.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
12/30/2005	1071/794	WD	I	Q		\$252,000.00
5/1/2002	952/1724	WD	V	Q		\$35,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	SINGLE FAM (000100)	2002	Common BRK (19)	1987	2638	\$151,959.00
Note: All S.F. calculations are based on exterior building dimensions.						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0166	CONC,PAVMT	2002	\$3,648.00	0001824.000	0 x 0 x 0	(000.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000100	SFR (MKT)	0000001.000 LT - (0000000.000AC)	1.00/1.00/1.40/1.00	\$37,170.00	\$37,170.00

Columbia County Property Appraiser

DB Last Updated: 4/27/2009

ZONING SITE PLAN

NO. _____

NOTE: PLEASE ACCURATELY SHOW AND LOCATE THE FOLLOWING:

- (1) DIMENSIONS OF PROPERTY
- (2) ALL STREETS BOUNDING YOUR PROPERTY
- (3) ALL PROPOSED AND/OR EXISTING IMPROVEMENTS
(BUILDING AND/OR STRUCTURES)
- (4) DISTANCE FROM PROPERTY LINE FOR THE PROPOSED IMPROVEMENT
- (5) ALL WELL AND SEPTIC TANKS - PROVIDE SETBACKS
- (6) MUST SHOW DRIVEWAY LOCATION AND DIMENSIONS
- (7) SURVEY CORNER MARKERS MUST BE VISIBLE FOR FIRST INSPECTION
- (8) IF CORNER LOT, PLEASE INDICATE THE FRONT

*** THE ZONING DEPARTMENT CAN NOT APPROVE YOUR ROUTING SLIP WITHOUT THE ABOVE INFORMATION ACCURATELY SUPPLIED.***

INCOMPLETE SITE PLAN WILL RESULT IN DELAYS.

<p style="text-align: center;">30'x55' Screen enclosure</p> <p style="text-align: center;">Front of home</p> <p style="text-align: center;">drive way</p> <p style="text-align: center;">Harris Lake Dr.</p>	<p style="text-align: right;">North</p> <p style="text-align: center;"><u>LOCATION MAP</u></p> <p style="text-align: center;">Z →</p> <p>APPROVED DATE: _____</p> <p>DENIED DATE: _____</p> <p>ANY CHANGES TO SITE PLAN MUST BE APPROVED BY THE ZONING DEPARTMENT.</p>
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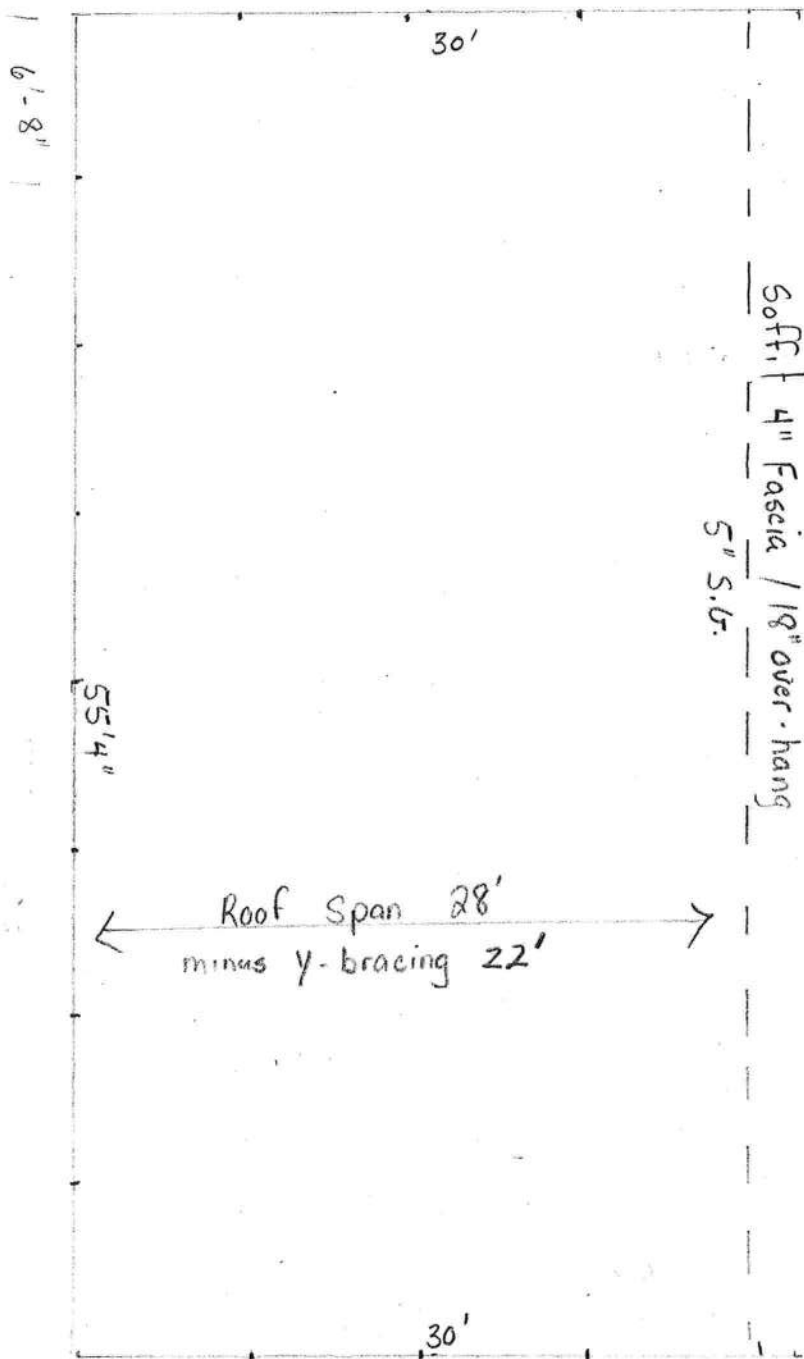
OFFICE USE ONLY

P.A. NO. _____ SEC _____ TWP _____ RGE _____ ZONING MAP _____

PLAT BK _____ PGE _____ ZONING CLASS. _____ TAZ _____

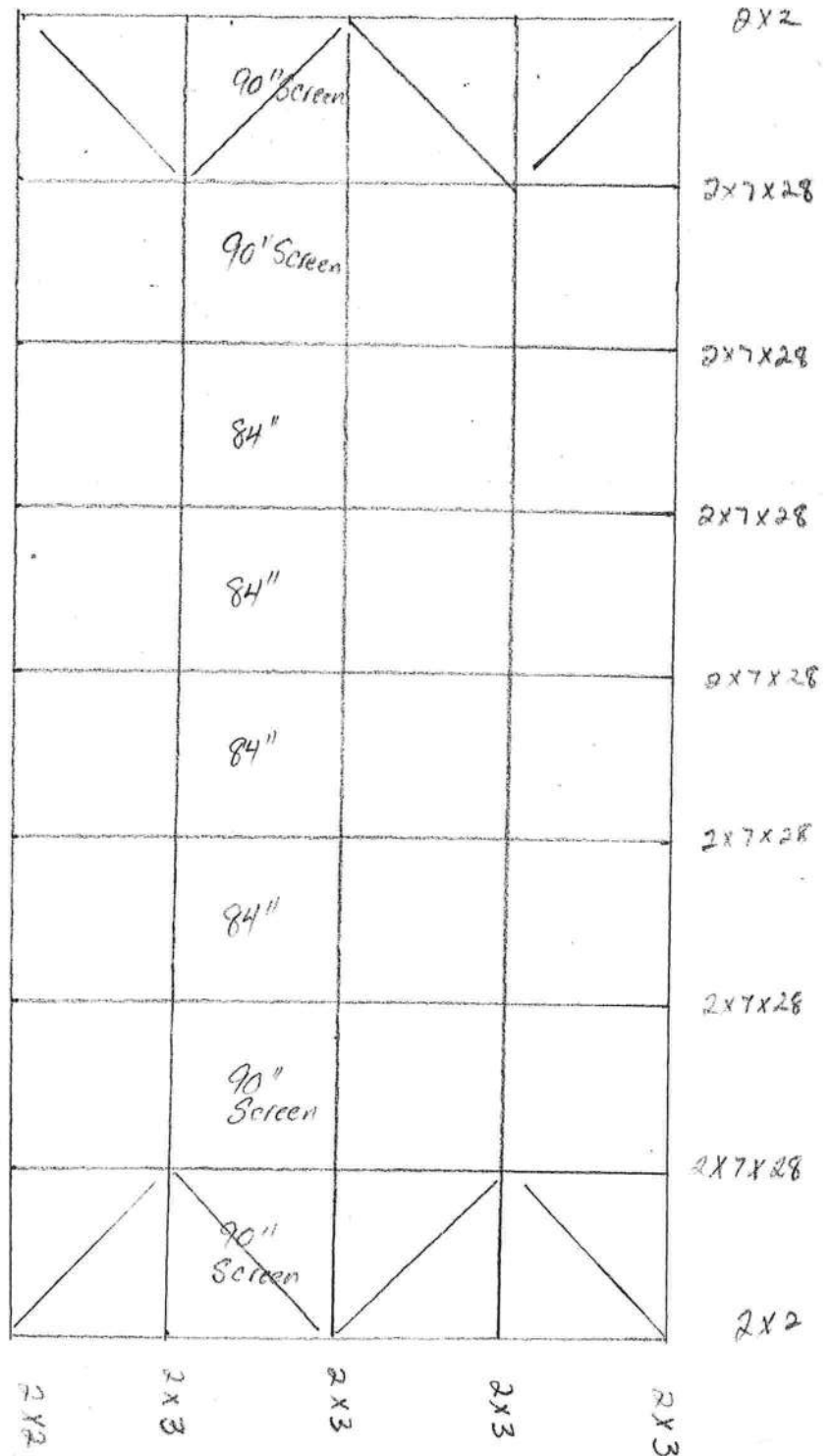
ESDZ _____ FLOOD ZONE _____ MAP NO. ELEV. _____ NOTES: _____

MINIMUM SETBACKS: FRONT _____ REAR _____ SIDE _____ SIDE _____



Floor Plan

(.88)



Roof 1568 S.Ft

✓ 2x7x28 (14)

✓ 2x2x30 (4)

✓ 2x3x30 (8)

✓ 90" Screen (116')

✓ 84" Screen (116')

ake - ✓ 35" Diagonal For Roof Beams (14) - 2x3x24 (2)

ake - ✓ H-channel 28 @ 4 1/4" → 10'

Roof Section

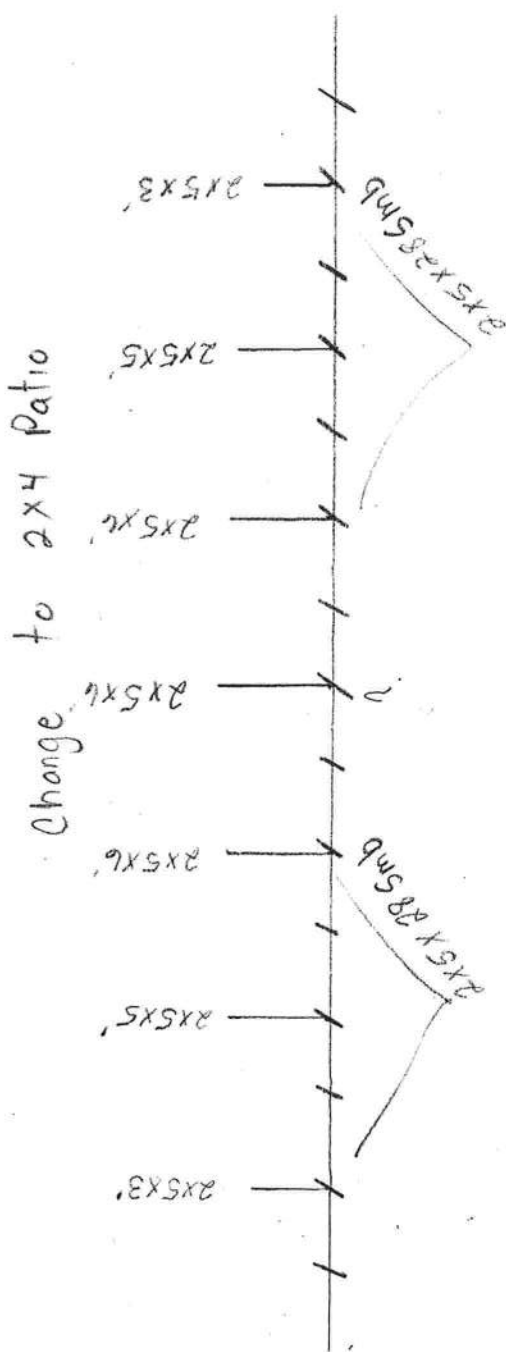
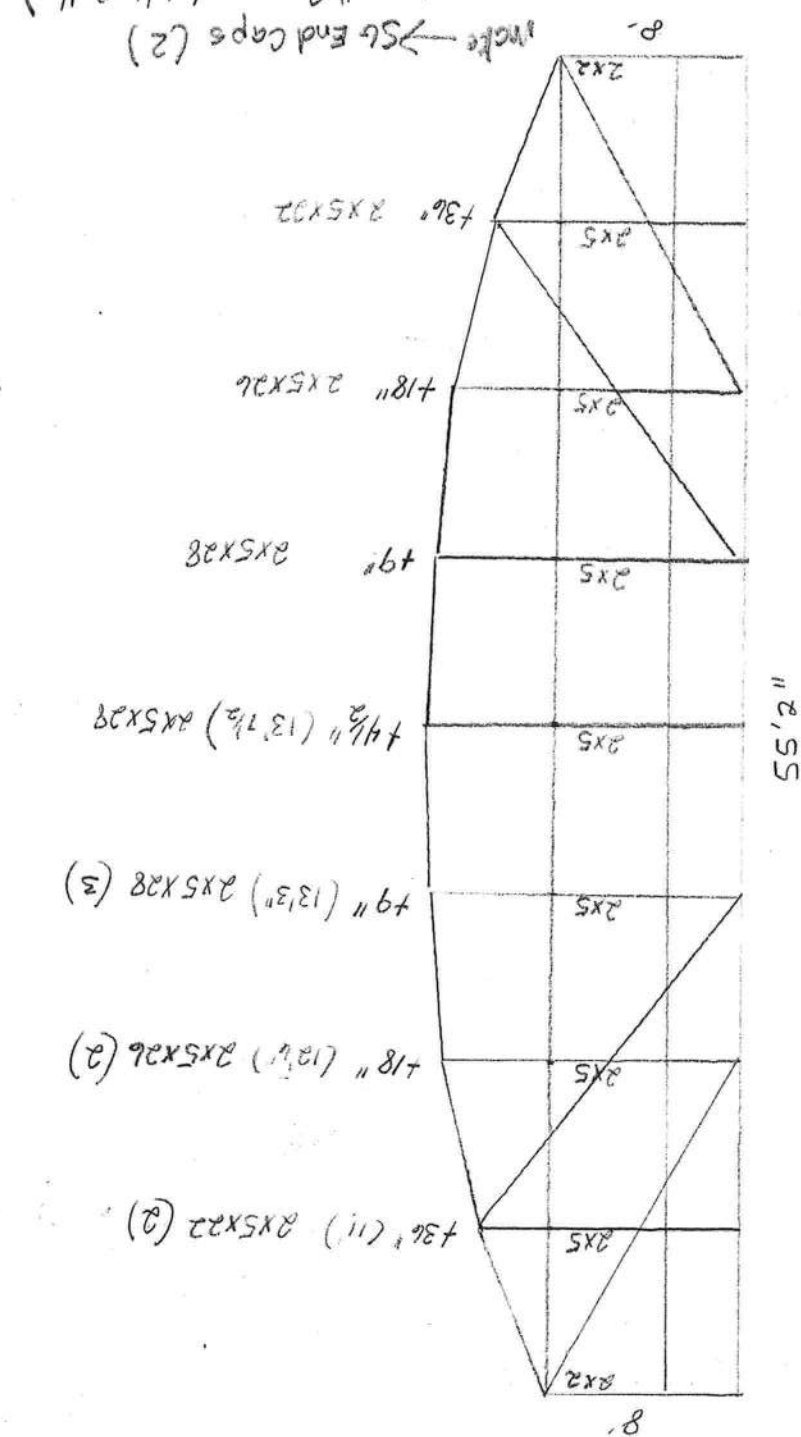
Front and Riser Wall Section

Front + Riser wall
300
440

90 1/2" Run

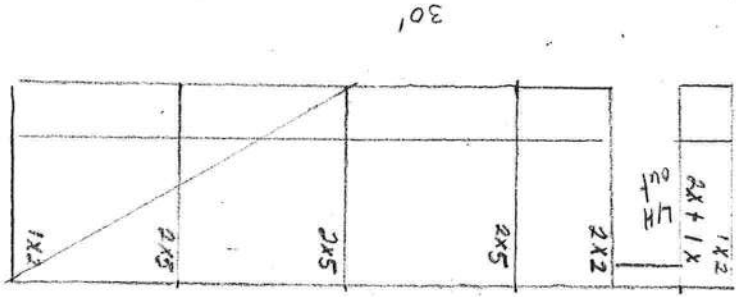
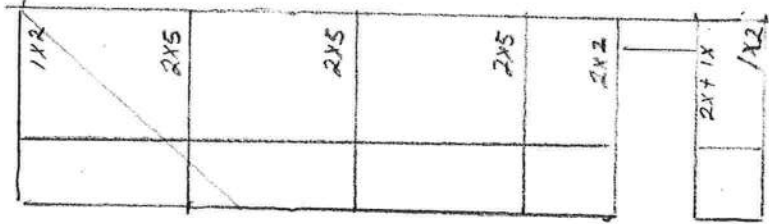
- Make → 5" gutter splice (1)
- Make → gutter straps (5)
- Make → 2x2x4" heavy angle (4)
- Make → 2x2x1 7/8 heavy angle (6)
- Make → 2x2x2 1/8 heavy angle (14)

- 1/8" Cable ties w/ bracket (2)
- 1/4" Cable tie w/ bracket (2)
- 2- 3x4 A-elbow
- 2- 3x4 oval outlet
- 2- 2x5x22
- 2- 2x5x26
- 3- 2x5x28
- 7- 1 1/2x4" Rec. channel
- 2- 2x4x24 patio



Side walls = 480 S.F.

- ✓ 12' Cable ties w/ brackets (2)
- ✓ 1x2x30 (5)
- ✓ 2x2x30 (4)
- ✓ 2x5x24 (4)
- ✓ 2x2x1 7/8 heavy angle (12)
- ✓ 2x2x2 7/8 heavy angle (12)
- ✓ 17' Z-bar (2)
- ✓ Door frames (2)
- ✓ Closer K.its (2)
- ✓ Bug sweeps (2)
- ✓ hinges (4)
- ✓ 84 Screen (75)



Side wall Section

SECTION 1

SCREENED ENCLOSURES

General Notes and Specifications:

1. The following structures are designed to be married to site built block or wood frame DCA approved modular structures of adequate structural capacity. The contractor / home owner shall verify that the host structure is in good condition and of sufficient strength to hold the proposed addition.
2. If the owner or contractor has a question about the host structure, the owner (at his own expense) shall hire an architect, engineer, or a certified home inspection company to verify host structure capacity.
3. The structures designed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16'. Structures larger than these limits shall have site specific engineering.
4. Spans are for enclosures with mean roof heights less than 30'. For greater heights, consult engineer.
5. Connections to fascia shall be limited to overhangs shown in table 1.11 or less unless site specific engineering is provided.
6. The proper structural name for a chair rail or top rail of an enclosure is a girt. Thus the terminology shall be interchangeable.
7. Screws that penetrate the water channel of the super gutter shall have ends clipped off for safety of cleaning gutter and the heads of screws through the gutter into the fascia shall be caulked.
8. Section 7 contains span tables and attachment details for pans and composite panels.
9. When using TEK screws in lieu of S.M.S., longer screws must be used to compensate for drill head.
10. An additional super gutter strap or ferrule is required to be located near the midpoint of the beam spacing. Straps shall be attached to each truss / rafter tail when a 2" sub-fascia does not exist. Straps at the beam are not required when straps are placed @ each truss / rafter tail and spacing of straps does not exceed 2'-0".
11. Super or extruded gutter details are applicable to all widths of super or extruded gutters, and gutters may be substituted. Gutter straps and/or ferrules shall be the width of the inside and outside of the super or extruded gutter respectively. The center of the knee braces shall not be more than 6" above the top of the super or extruded gutter.
12. If the sub-fascia is 3/4", and the sub-fascia is in good repair, a 3/4" P.T.P. strip the width of the fascia may be added to the existing sub-fascia by attaching the plywood with (2) 16d x 3" common nails or (2) #8 x 3" screws. This gives the equivalent of a 2" fascia.
13. Spans may be interpolated between values but not extrapolated outside values.
14. All 2" X 4" and larger purlins shall have an internal or external angle clip or screw boss to fasten the bottom of the purlin to the beam.
15. Load width and / or panel spacing used in determining spans / heights is measured from center to center of the members.
EXAMPLE:
Screen panel A is 6' center to center. Screen panel B is 7' center to center. The load width of the frame member between panel A and B is $(6'/2 + 7'/2) = 6.5'$ or 6'-6".
The distance, spacing or load width is not measured between frame members as that would add 2" to the load width if figured that way.
16. For Design Check List and Inspection Guides for Screened Enclosures, see Appendix (Section 10).
17. All aluminum extrusions shall meet the strength requirements of ASTM B221 after powder coating.
18. Other shapes than those shown in Section 8 with State Product Approvals may be used with the details of this section so long as the shapes are compatible with the details.
19. All aluminum shall be ordered as to the alloy and hardness after heat treatment and paint is applied.
Example: 6063-T6 after heat treatment and paint process.

Section 1 Design Statement:

The structures designed for Section 1 are framing systems with screen roofs & walls and loads have been determined by wind tunnel test that include any negative internal pressure coefficient. Since these structures are open, the negative internal pressure coefficient is considered to be 0.00. The design loads used are from Chapter 20 of the 2004 Florida Building Code w/ 2006 Supplements. The loads assume a mean roof height of less than 30'; roof slope of 0° to 20°; $I = 0.87$ for 100 MPH and 0.77 for 110 or higher. All loads are based on 20 / 20 screen or larger. Multiply wall heights by 1.10 for members controlled by bending(b) and 1.07 for members controlled by deflection(d) when using 18 / 14 screen. All pressures shown in the below table are in PSF (#/SF). All framing components are considered to be 6063-T6 alloy. For components of 6005-T5 and 6061-T6 multiply spans by 1.13.

General Notes and Specifications for Section 1 Tables:

SECTION 1 Uniform Loads for Structures with Screen Roof & Walls

Wind Velocity MPH	Basic Wind Pressure	Exposure 'B'			Exposure 'C'		
		Roofs	Windward Walls	Leeward Walls	Roofs	Windward Walls	Leeward Walls
100	13	3	12	10	5	17	13
110	14	4	13	9	5	18	14
120	17	4	15	13	6	21	17
123	18	4.3	15.9	13.3	6.3	22.2	17.6
130	20	5	18	14	7	25	19
1401 & 2	23	6	21	15	8	29	23
150	26	7	24	18	9	33	27

Loads per table 2002.4

Multipliers only apply to members when spans / heights are controlled by wind pressure, not by point load.

Conversion Table 1A

Wind Zone Conversion Factors for Screen Roof or Wall Frame Members

From 120 MPH Wind Zone to Others; Exposure 'B'

Wind Zone MPH	Roofs		Walls	
	Applied Load #/ SF	Conversion Factor	Applied Load #/ SF	Conversion Factor
100	3	1.15	12	1.12
110	4	1.00	13	1.07
120	4	1.00	15	1.00
123	4.3	0.96	15.9	0.97
130	5	0.89	18	0.91
1401 & 2	6	0.82	21	0.85
150	7	0.76	24	0.79

Note:

Multipliers are for wall loads only.

Multipliers only apply to members when spans / heights are controlled by wind pressure, not by point load.

Conversion Table 1B

Load Conversion Factors Based on Mean Roof Height from Exposure "B" to "C" & "D"

Mean Roof Height*	Exposure "B" to "C"			Exposure "B" to "D"		
	Load Conversion Factor	Span Multiplier		Load Conversion Factor	Span Multiplier	
		Bending	Deflection		Bending	Deflection
0 - 15'	1.21	0.91	0.94	1.47	0.83	0.88
15' - 20'	1.29	0.88	0.92	1.54	0.81	0.87
20' - 25'	1.34	0.86	0.91	1.60	0.79	0.86
25' - 30'	1.40	0.85	0.89	1.66	0.78	0.85
30' - 40'	1.37	0.85	0.90	1.61	0.79	0.85

* Use larger mean roof height of host structure or enclosure

Values are from ASCE 7-02

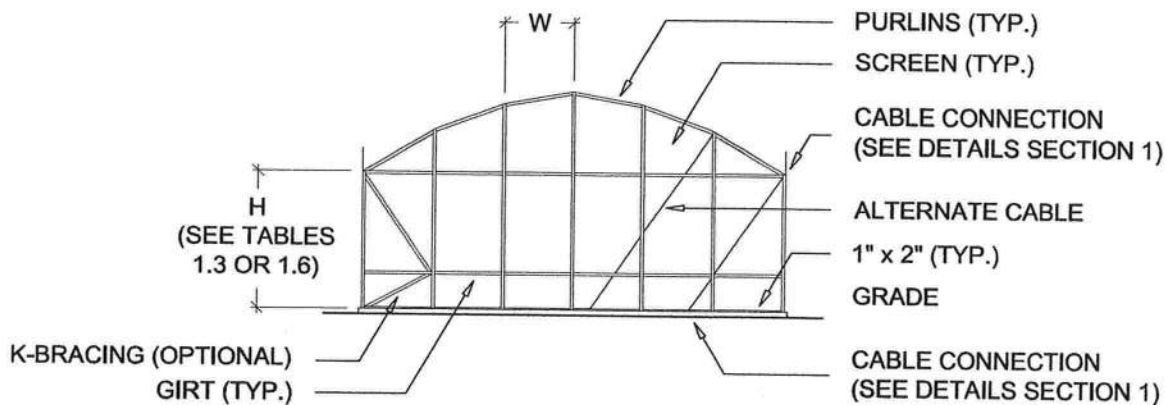
Multipliers only apply to members when spans / heights are controlled by wind pressure, not by point load.

Conversion Example (Convert span for Exposure "B" to "C"):

If max span found from span tables for Exposure "B" = 31'-11" = 31.92'

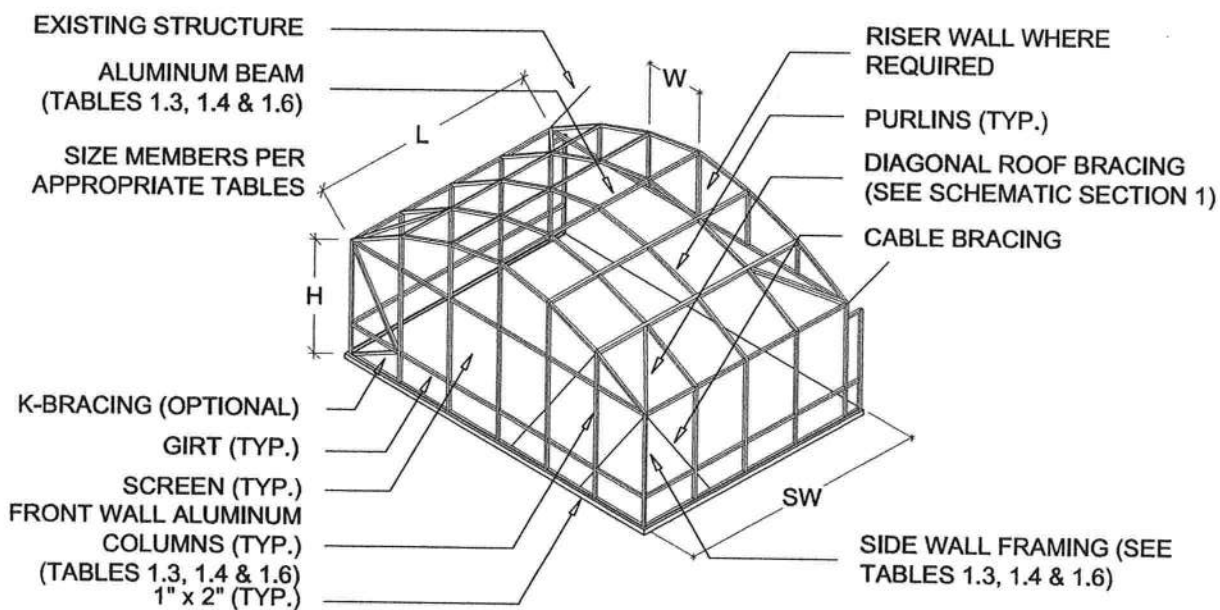
and the mean roof height of the structure is 0-15' then multiply span by 0.91

the span for Exposure "C" is 31.92' * 0.91 = 29.05' = 29'-1"



TYPICAL DOME ROOF - FRONT WALL ELEVATION

SCALE: N.T.S.



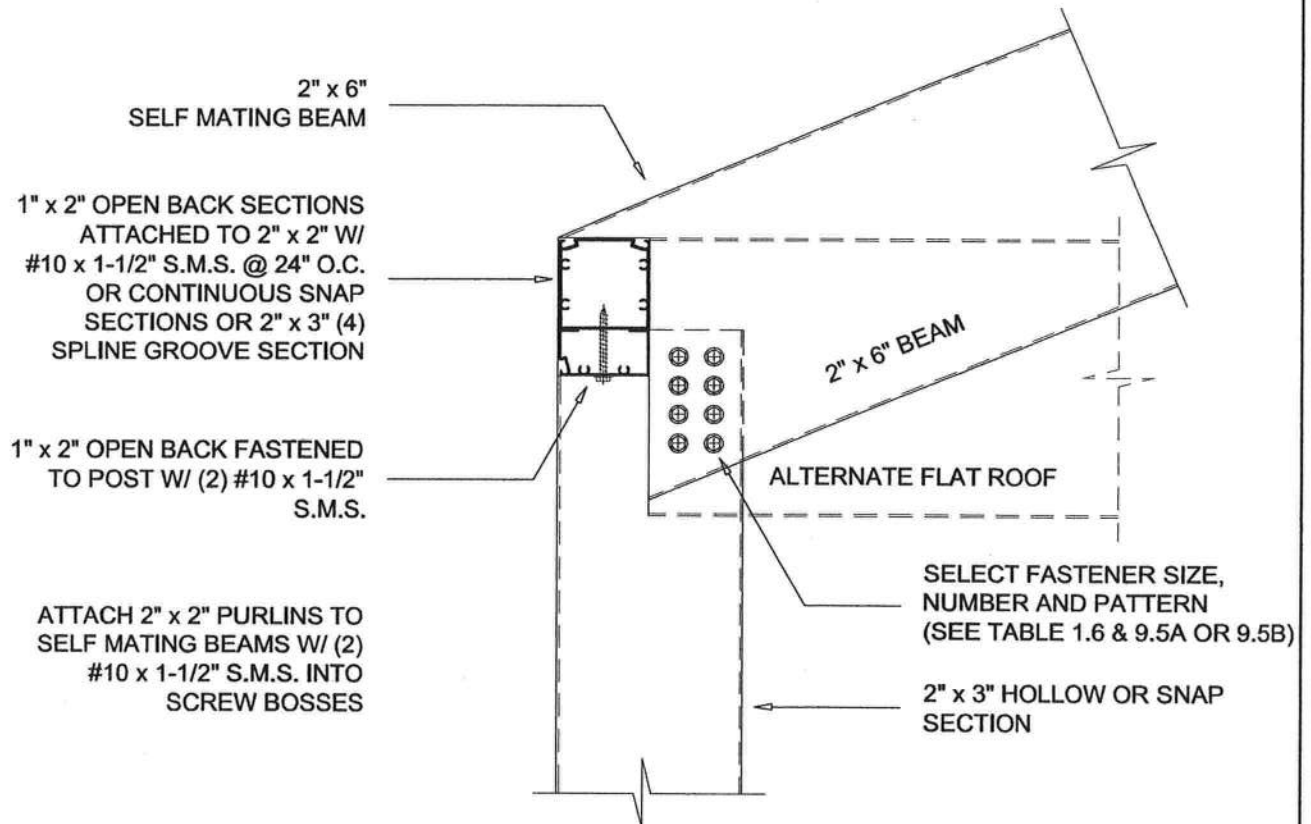
TYPICAL DOME ROOF - ISOMETRIC

SCALE: N.T.S.

CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES.

Lawrence E. Bennett, P.E. FL # 16644
CIVIL & STRUCTURAL ENGINEERING
 P.O. Box 214368, South Daytona, FL 32121
 Telephone #: (386) 767-4774 Fax #: (386) 767-6556
 Email: lebpe@bellsouth.net

MINIMUM POST SIZES
REQUIRED FOR EACH BEAM
SIZE (SEE TABLE 1.6)



SLOPING BEAM TO UPRIGHT CONNECTION DETAIL (PARTIAL LAP)

SCALE: 3" = 1'-0"

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CIVIL & STRUCTURAL ENGINEERING

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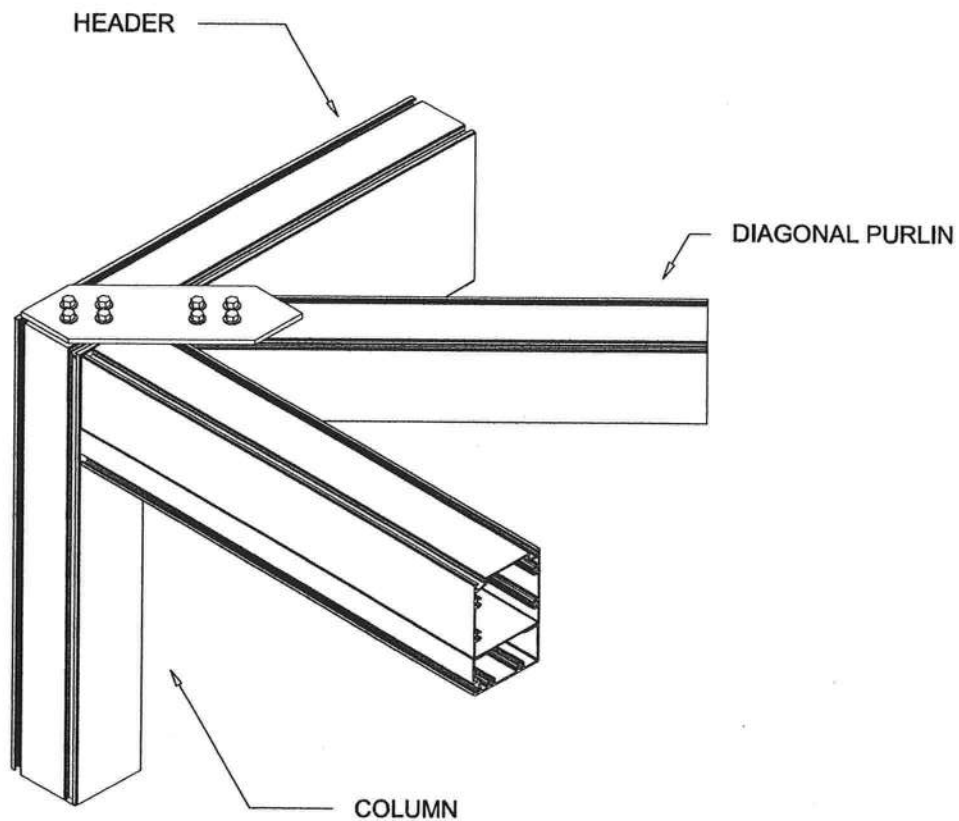
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Email: lebpe@bellsouth.net

SCREENED ENCLOSURES



NOT TO BE REPRODUCED IN WHOLE OR IN PART WITHOUT THE WRITTEN PERMISSION OF LAWRENCE E. BENNETT, P.E.



WIND BRACE CONNECTION DETAIL

SCALE: 3" = 1'-0"

NOTES:

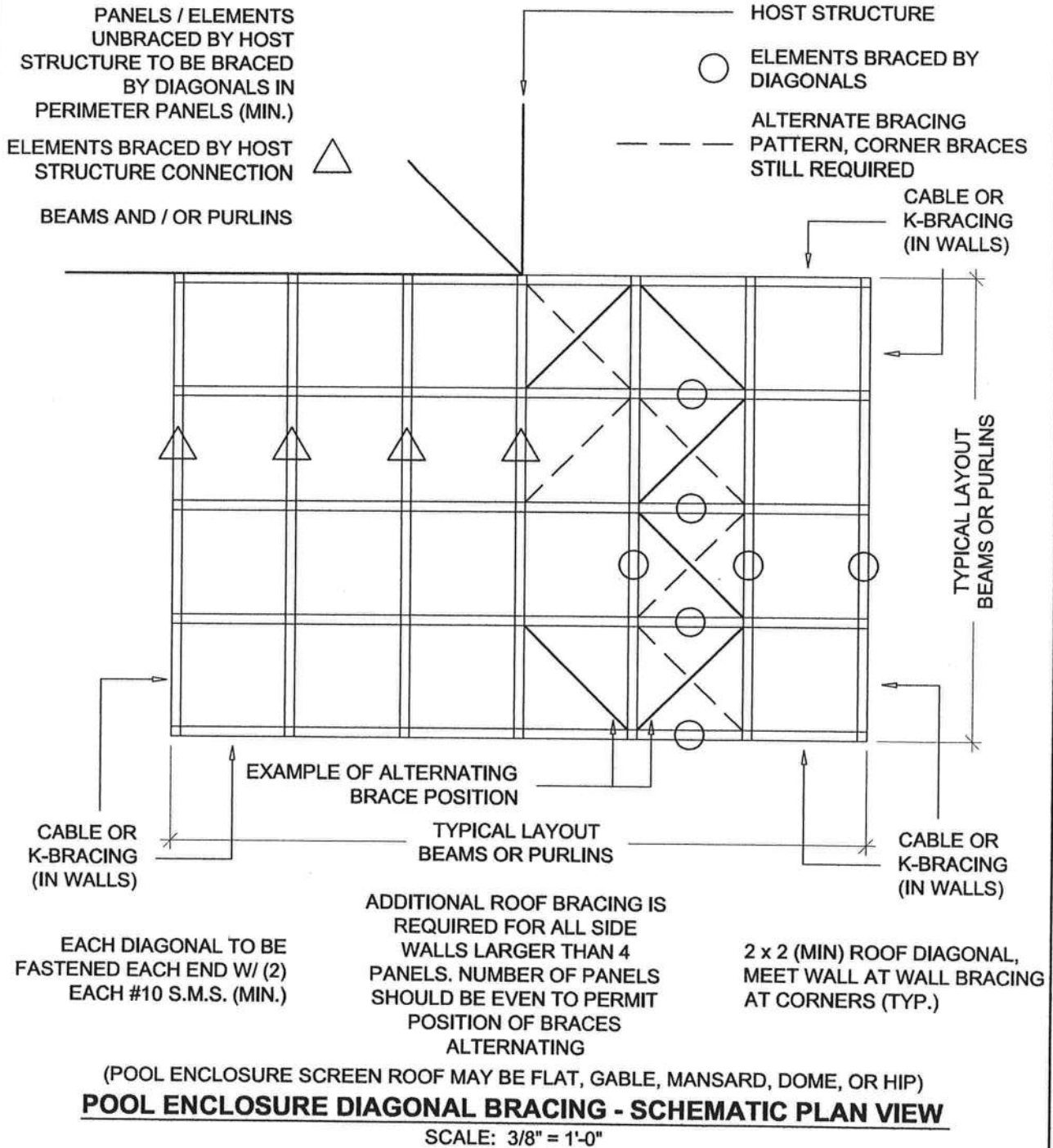
1. Wind bracing shall be provided at each side wall panel when enclosure projects more than three panels from host structure. Structures of four or more panels shall be spaced for even number of panels for opposing wind bracing.
2. Cut brace parts with min. 12" lap of larger and smaller brace.
3. Cut receiving channel with angle.

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 Email: lebpe@bellsouth.net

SECTION 1

SCREENED ENCLOSURES



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 CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121
 Telephone #: (386) 767-4774 Fax #: (386) 767-6556
 Email: lebpe@bellsouth.net

PAGE

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CABLE BRACING**General Notes and Specifications:**

- 1) The following shall apply to the installation of cables as additional bracing to DIAGONAL bracing for pool enclosures:

a) FRONT WALL CABLES - 7 x 19 STAINLESS STEEL

CABLE DIAMETER	TOTAL ALLOWABLE WALL AREA *
3/32"	233 Sq. Ft. / PAIR OF CABLES
1/8"	445 Sq. Ft. / PAIR OF CABLES

* TOTAL WALL AREA = 100% OF FRONT WALL + 50% OF ONE SIDE WALL

EXAMPLE: FRONT WALL AREA @ 100% (8' x 32') = 256 Sq. Ft.

SIDE WALL AREA @ 50% (8' x 20') = 80 Sq. Ft.

TOTAL WALL AREA = 336 Sq. Ft.

233 Sq. Ft. x 2 sets = 466 Sq. Ft. > 336 Sq. Ft.; thus two sets of 3/32" cables is required.

b) SIDE WALL CABLES - 7 x 19 STAINLESS STEEL

CABLE DIAMETER	SIDE WALL CABLE **
3/32"	ONE PER 233 Sq. Ft. OF WALL
1/8"	ONE PER 445 Sq. Ft. OF WALL

** SIDE WALL CABLES ARE NOT REQUIRED FOR SIDE WALLS LESS THAN 233 Sq. Ft.

- c) To calculate the required pair of cables for free standing pool enclosures use 100% of each wall area & 50% of the area of one adjacent wall.

NOTES:

1. Where wall height is such that a girt is required between the top or eave rail and the chair rail, (i.e. a mid-rise girt), then the front wall shall have two cable pairs and they shall be attached to the top rail and the mid-rise rail. If more than one additional girt is required between the top or eave rail and the chair rail, then there shall be an additional front wall cable pair at that girt also.
2. Side walls do not require cables until the side wall area is greater than 233 Sq. Ft.. The side wall cable may be attached at the mid-rise girt or the top rail.
3. Standard rounding off rules apply. ie: if the number of cables calculated is less than 2.5 pairs use two cables; if the number of cables calculated is 2.5 pairs or greater use 3 pairs of cables.
4. Additional roof bracing is required for all side walls larger than 4 panels. Number of panels shall be even and position shall be alternating.

Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

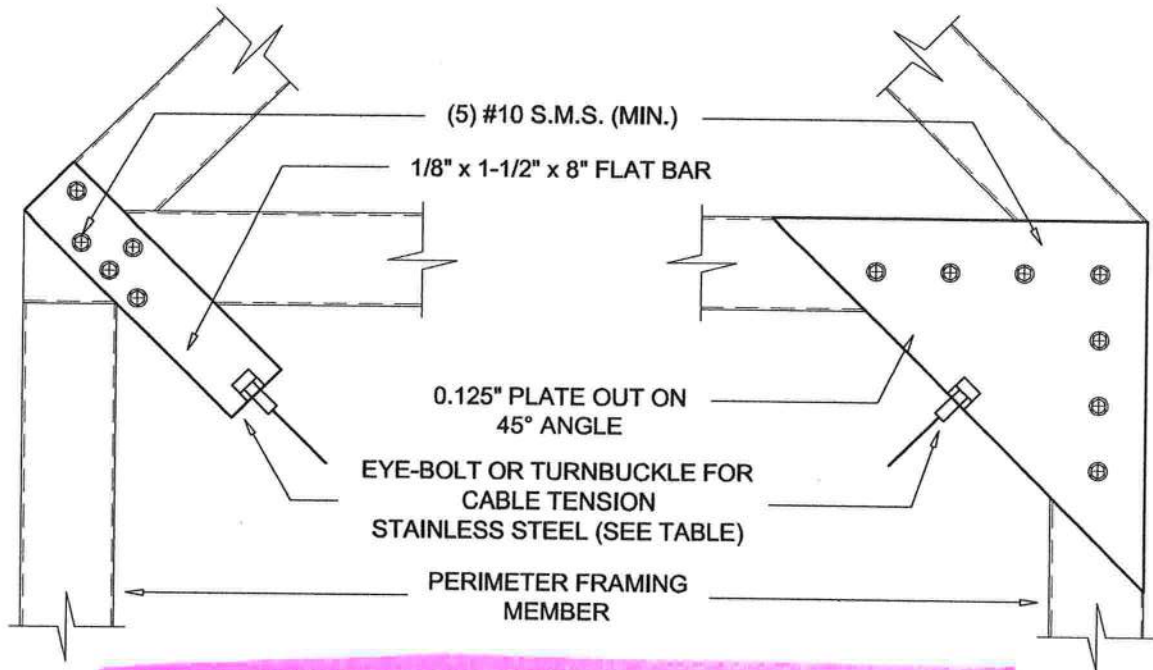
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Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

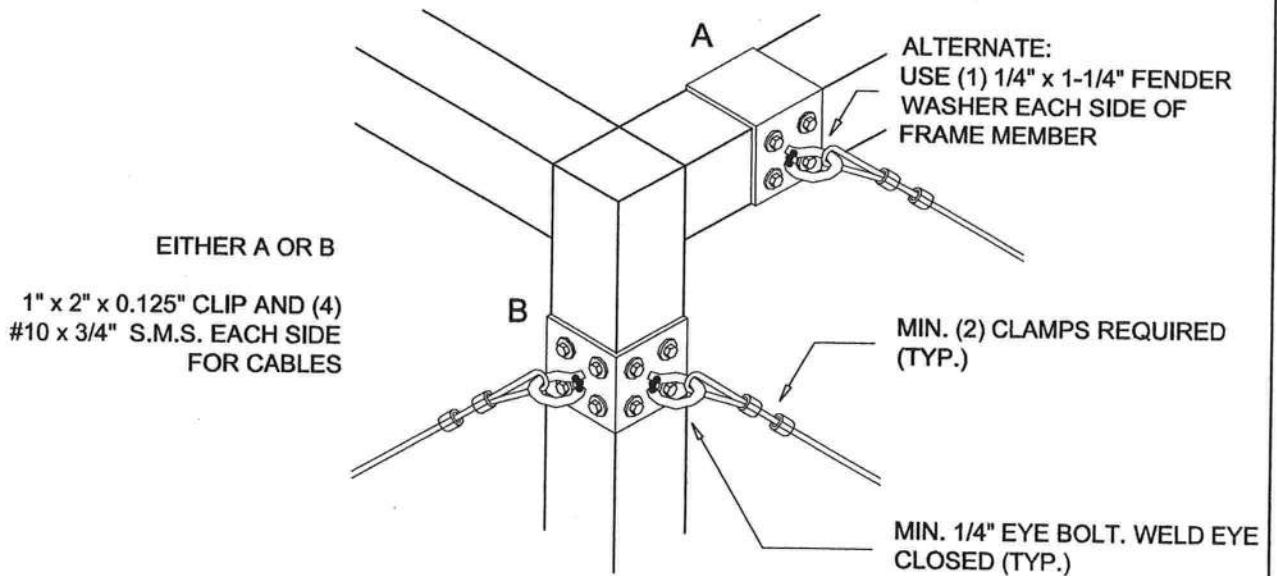
SCREENED ENCLOSURES

SECTION 1



TYPICAL CABLE CONNECTIONS AT CORNER - DETAIL 1

SCALE: 3" = 1'-0"



ALTERNATE TOP CORNER OF CABLE CONNECTION - DETAIL 1A

SCALE: 3" = 1'-0"

Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

SCREENED ENCLOSURES**SECTION 1**

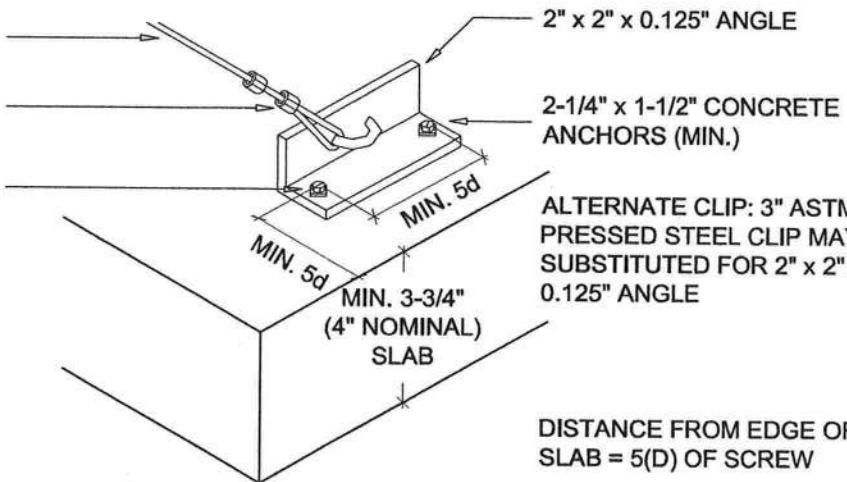
S.S. CABLE @ 40° TO 60° MAX.
ANGLE TO SLAB

CABLE CLAMP

(SEE TABLE)

NOTE:
SEE PAGE 1-50 FOR NUMBER
OF CABLES REQUIRED

3-1/2" ASTM A-36 PRESSED
STEEL CLIP MAY BE
SUBSTITUTED FOR
2" x 2" x 0.125" ANGLE

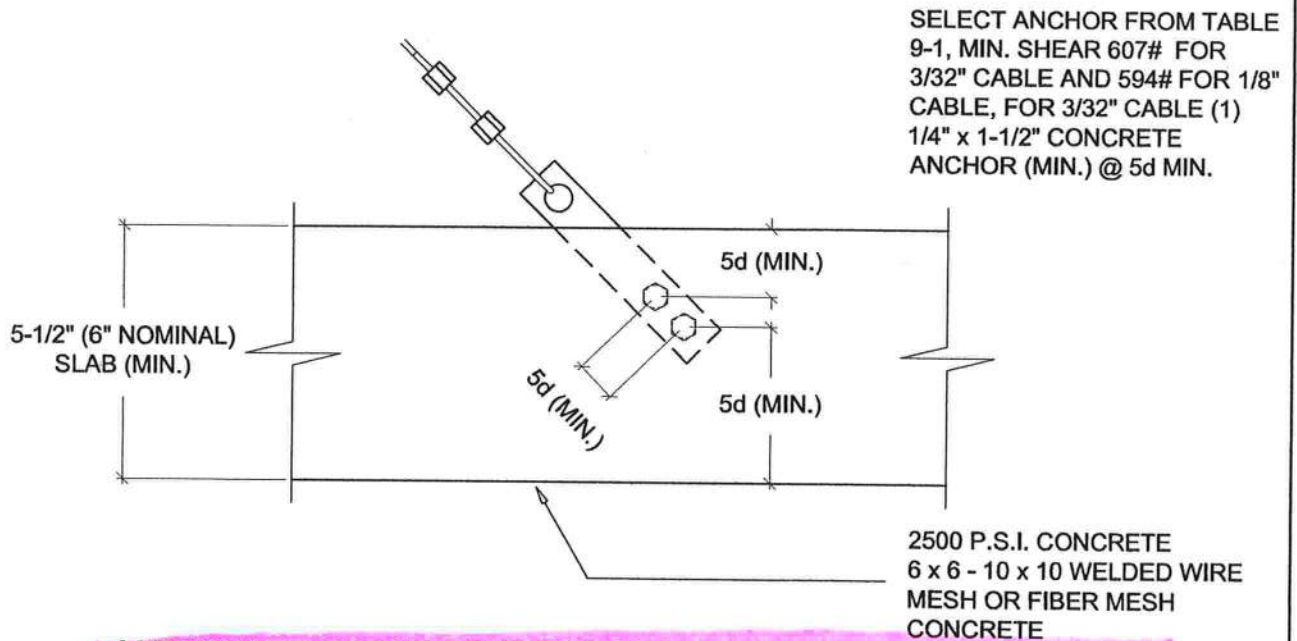


ALTERNATE CLIP: 3" ASTM A-36
PRESSED STEEL CLIP MAY BE
SUBSTITUTED FOR 2" x 2" x
0.125" ANGLE

DISTANCE FROM EDGE OF
SLAB = 5(D) OF SCREW

ALTERNATE CABLE CONNECTION AT SLAB DETAIL - DETAIL 2B

SCALE: 3" = 1'-0"



SELECT ANCHOR FROM TABLE
9-1, MIN. SHEAR 607# FOR
3/32" CABLE AND 594# FOR 1/8"
CABLE, FOR 3/32" CABLE (1)
1/4" x 1-1/2" CONCRETE
ANCHOR (MIN.) @ 5d MIN.

2500 P.S.I. CONCRETE
6 x 6 - 10 x 10 WELDED WIRE
MESH OR FIBER MESH
CONCRETE

ALTERNATE CABLE CONNECTIONS AT FOUNDATION - DETAIL 2C

SCALE: 3" = 1'-0"

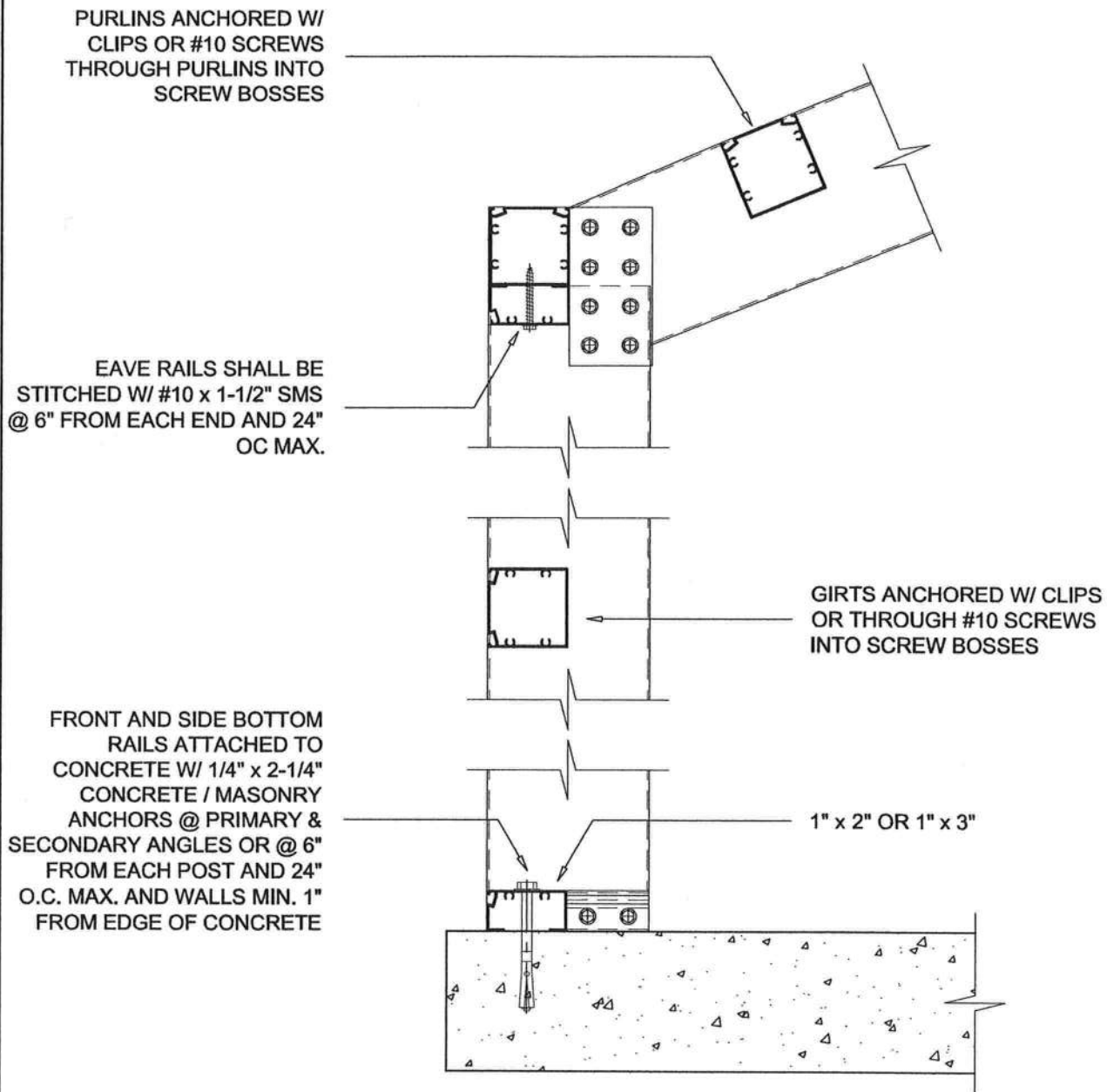
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CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

SECTION 1**SCREENED ENCLOSURES****PURLIN & CHAIR RAIL DETAIL**

SCALE: 3" = 1'-0"

Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

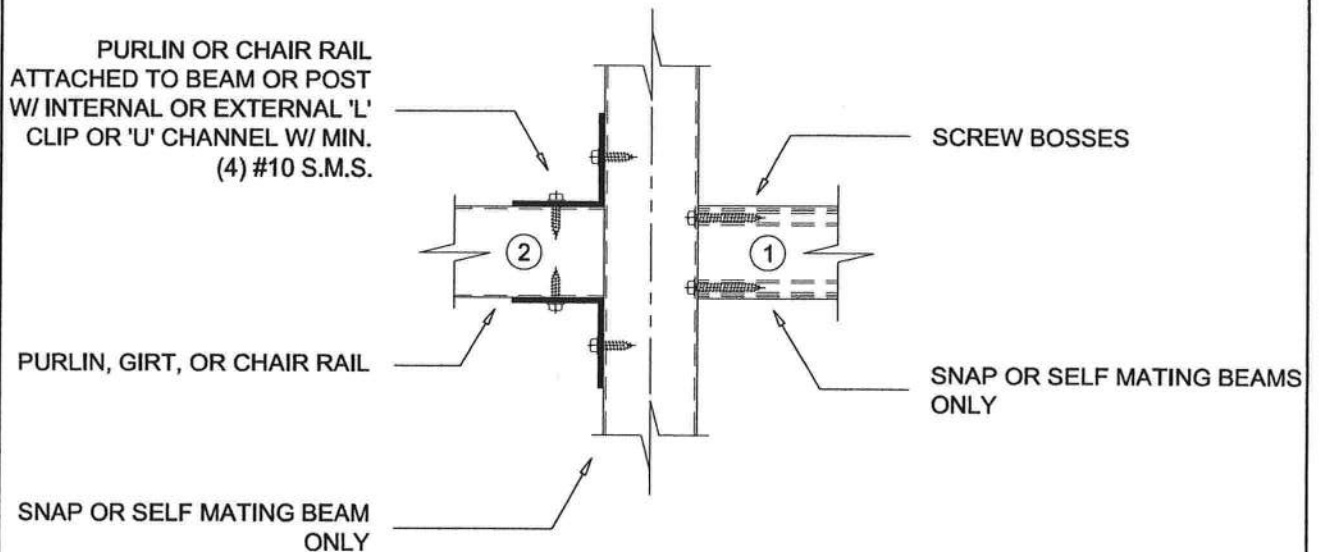
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PURLIN TO BEAM OR GIRT TO POST DETAIL

SCALE: 3" = 1'-0"

- ① FOR WALLS LESS THAN 6'-8" FROM TOP OF PLATE TO CENTER OF BEAM CONNECTION OR BOTTOM OF TOP RAIL THE GIRT IS DECORATIVE AND SCREW HEADS MAY BE REMOVED AND INSTALLED IN PILOT HOLES
- ② FOR ALL OTHER PURLINS AND GIRTS IF THE SCREW HEADS ARE REMOVED THEN THE OUTSIDE OF THE CONNECTION MUST BE STRAPPED FROM GIRT TO POST WITH 0.050" x 1-3/4" x 4" STRAP AND (4) #10 x 3/4" S.M.S. SCREWS TO POST AND GIRT

IF GIRT IS ON BOTH SIDES OF THE POST THEN STRAP SHALL BE 6" LONG AND CENTERED ON THE POST AND HAVE A TOTAL (12) #10 x 3/4" S.M.S.

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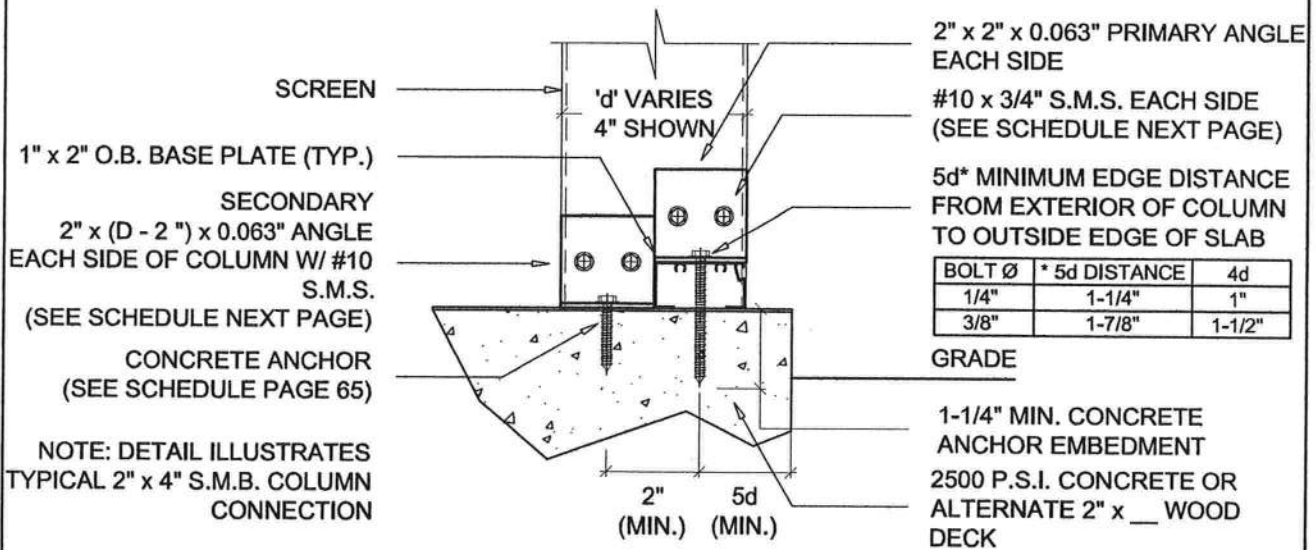
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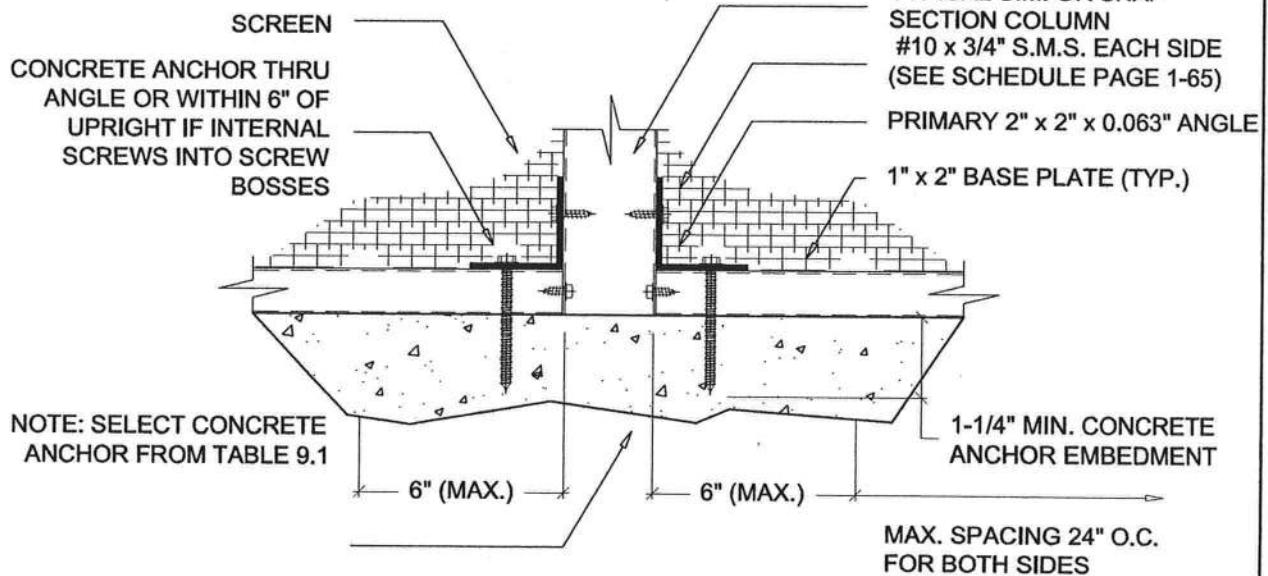
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SECTION 1

SCREENED ENCLOSURES



SIDE VIEW



FRONT VIEW

2" x 4" OR LARGER SELF MATING OR SNAP SECTION POST TO DECK DETAILS

SCALE: 3" = 1'-0"

NOTE:

1. FOR SIDE WALLS OF 2" x 4" OR SMALLER ONLY ONE ANGLE IS REQUIRED.
2. PREDRILL PAVERS W/ MIN. 1/4" MASONRY BIT.

Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

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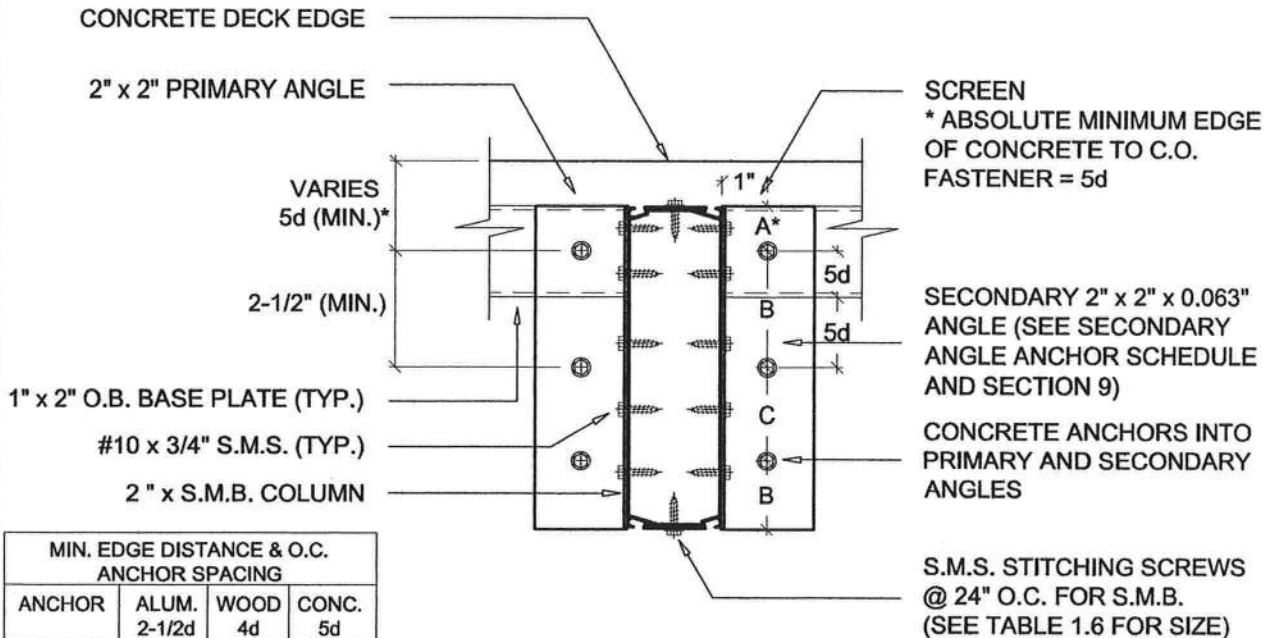
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SCREENED ENCLOSURES

SECTION 1

DETAIL ILLUSTRATES TYPICAL
2" x 4" S.M.B. THRU 2" x 9" SUB
CONNECTIONS



MIN. EDGE DISTANCE & O.C. ANCHOR SPACING			
ANCHOR	ALUM. 2-1/2d	WOOD 4d	CONC. 5d
1/4"	5/8"	1"	1-1/4"
5/16"	25/32"	1-1/4"	1-9/16"
3/8"	15/16"	1-1/2"	1-7/8"

TOP VIEW POST TO DECK DETAIL

SCALE: 3" = 1'-0"

Primary and Secondary Anchor Schedule

Column Size	Secondary Angle				Maximum Number and Spacing Anchors											
	Angle Length "L"	Number of Anchors														
		1/4"	5/16"	3/8"	#	"A"	"B"	"C"	#	"A"	"B"	"C"	#	"A"	"B"	"C"
2 x 4	2"	4	4	4	4	1"	1"	1"	4	1"	1"	1"	4	1"	1"	1"
2 x 5	3"	4	4	4	4	1"	1-1/2"	-	4	1"	1-1/2"	-	4	1"	1-1/2"	-
2 x 6	4"	4	4	4	4	1"	2"	-	4	1"	2"	-	4	1"	2"	-
2 x 7	5"	6	4	4	6	1"	5/8"	1-7/8"	4	1"	2-1/2"	-	4	1"	2-1/2"	-
2 x 8	6"	6	4	4	6	1"	5/8"	2-3/8"	4	1"	3"	-	4	1"	3"	-
2 x 9	7"	6	6	4	6	1"	5/8"	2-7/8"	6	1"	13/16"	2-7/8"	4	1"	3-1/2"	-
2 x 10	8"	8	6	6	8	1"	5/8"	2"	6	1"	13/16"	3-3/16"	6	1"	3-1/4"	3-1/4"

Example:

Calculate the number of anchors required: $1.5 \times \text{beam span} / 2 \times \text{beam spacing} \times \text{roof wind pressure (PSF)} = \text{total \#}$;

if $1.5 \times 30' / 2 \times 6' \times 10 \text{ PSF} = 1350\#$ and $1/4" \times 1/4"$ Tapcon in tension @ 5d = 427# / ea. (see table 9.1)

then $1350\# / 427\# / \text{ea.} = 3.16 \text{ ea.}$ use (3) ea., secondary angle not required

Actual Edge Distance Example:

From edge of concrete to fastener = $2" / \text{dia. of } 0.25" = 8d$

Note:

For attachment to wood deck substitute wood fasteners for concrete fasteners & calculate the required number of fasteners using tables from section 9.

Lawrence E. Bennett, P.E. FL # 16644

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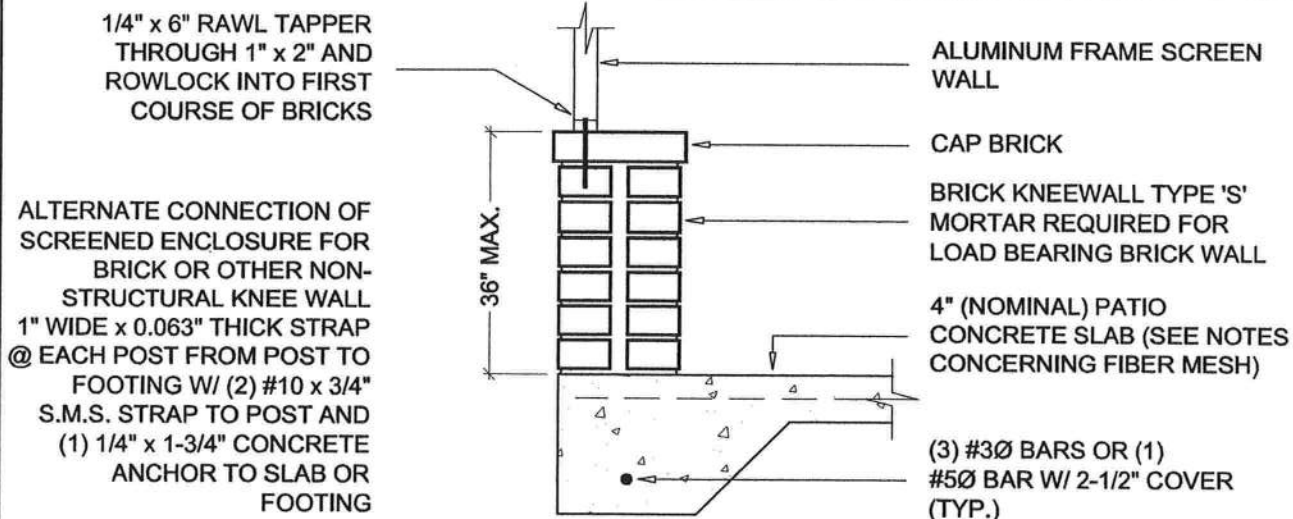
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Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

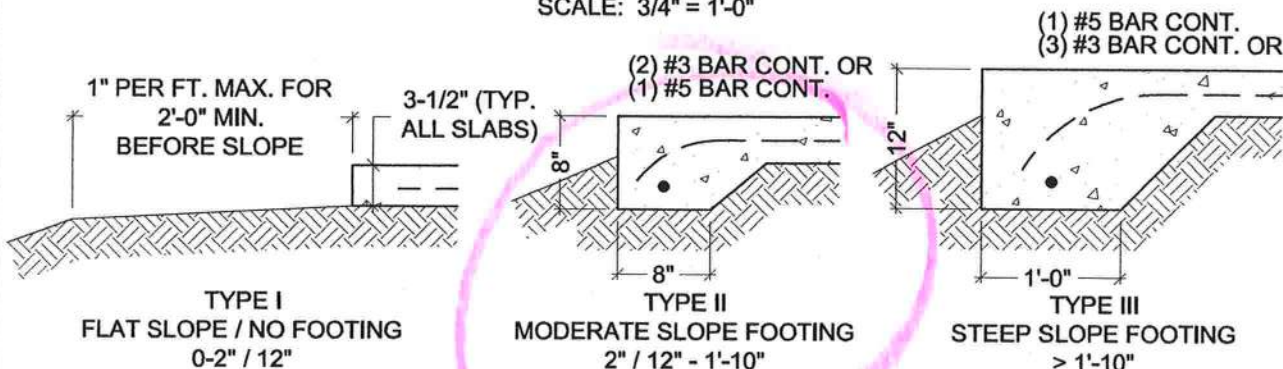
SCREENED ENCLOSURES

SECTION 1



BRICK KNEEWALL AND FOUNDATION FOR SCREEN WALLS

SCALE: 3/4" = 1'-0"



Notes for all foundation types:

1. The foundations shown are based on a minimum soil bearing pressure of 1,500 PSF. Bearing capacity of soil shall be verified prior to placing slab by field soil test (soil penetrometer) or a soil testing lab.
2. The slab / foundation shall be cleared of debris, roots and compacted prior to placement of concrete.
3. No footing is required except when addressing erosion until the slab width in the direction of the primary beams exceeds the span per table on page 1-69, then a type II slab is required under the load bearing wall only unless the side wall exceeds 16' in height or the enclosure is in a "C" exposure category in which case a type II footing is required.
4. Monolithic slabs and footings shall be minimum 2,500 psi concrete with 6 x 6 - 10 x 10 welded wire mesh or crack control fiber mesh; Fibermesh® Mesh, InForce™ e3™ (Formerly Fibermesh MD) per manufacturer's specification may be used in lieu of wire mesh. All slabs / footings shall be allowed to cure for 7 days before installing anchors.
5. If local codes require a minimum footing use Type II footing or footing section required by local code. Local codes govern.

SLAB-FOOTING DETAILS

SCALE: 3/4" = 1'-0"

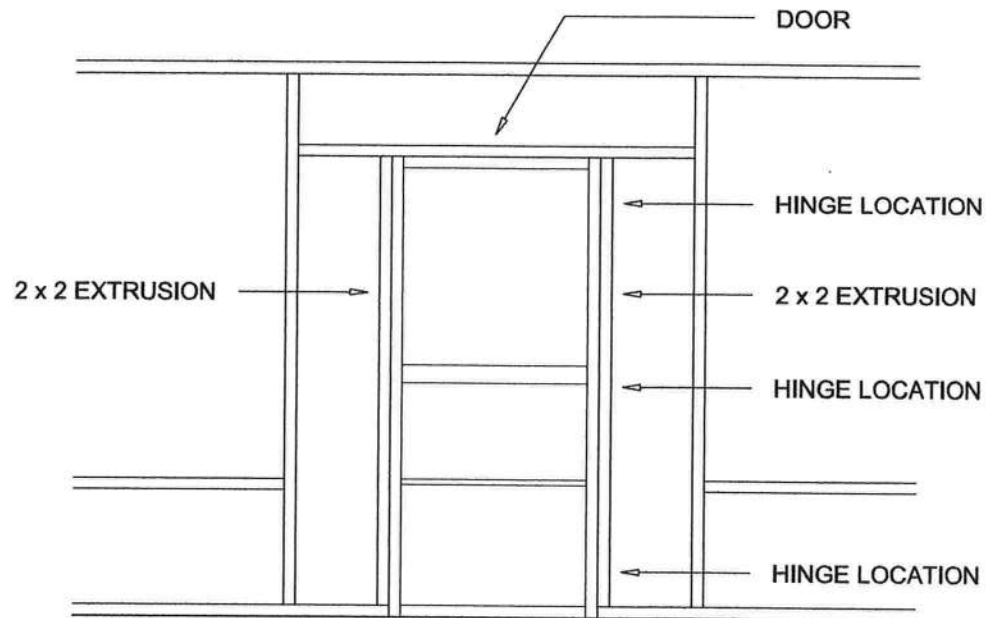
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CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net



NOTES:

1. Door to be attached to structure with minimum two (2) hinges.
2. Each hinge to be attached to structure with minimum four (4) #12 x 3/4" S.M.S..
3. Each hinge to be attached to door with minimum three (3) #12 x 3/4" S.M.S..
4. Bottom hinge to be mounted between 10 inches and 20 inches from ground.
5. Top hinge to be mounted between 10 inches and 20 inches from top of door.
6. If door location is adjacent to upright a 1" x 2" x 0.044" may be fastened to upright with #12 x 1" S.M.S. at 12" on center and within 3" from end of upright.

TYPICAL SCREEN DOOR CONNECTION DETAIL

SCALE: N.T.S.

Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

SECTION 1

SCREENED ENCLOSURES

Table 1.1 120 Allowable Spans for Primary Screen Roof Frame Members
Aluminum Alloy 6063 T-6

For Wind Zones up to 120 M.P.H., Exposure "B" and Latitudes Below 30°-30'-00" North (Jacksonville, FL)
 Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

Hollow Sections	Tributary Load Width "W" = Beam Spacing							
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)							
2" x 2" x 0.044"	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb
2" x 2" x 0.050"	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb
2" x 2" x 0.090"	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb
2" x 3" x 0.045"	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb
2" x 4" x 0.050"	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb
2" x 5" x 0.062"	20'-5"	Pb	20'-5"	Pb	20'-4"	Ud	19'-4"	Ud

Self Mating Sections	Tributary Load Width "W" = Beam Spacing							
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)							
2" x 4" x 0.044 x 0.100"	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd
2" x 5" x 0.050 x 0.100"	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	15'-9"	Ud
2" x 6" x 0.050 x 0.120"	20'-4"	Pd	20'-4"	Pd	20'-3"	Ud	19'-3"	Ud
2" x 7" x 0.055 x 0.120"	24'-9"	Pd	24'-9"	Pd	24'-6"	Ud	21'-11"	Ud
2" x 8" x 0.072 x 0.224"	34'-2"	Pd	32'-9"	Ud	30'-5"	Ud	28'-7"	Ud
2" x 9" x 0.072 x 0.224"	39'-3"	Pd	35'-11"	Ud	33'-4"	Ud	31'-5"	Ud
2" x 9" x 0.082 x 0.310"	42'-5"	Ud	38'-7"	Ud	35'-10"	Ud	33'-8"	Ud
2" x 10" x 0.092 x 0.369"	49'-3"	Ud	44'-9"	Ud	41'-7"	Ud	39'-1"	Ud

Snap Sections	Tributary Load Width "W" = Beam Spacing							
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)							
2" x 2" x 0.044"	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd
2" x 3" x 0.045"	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd
2" x 4" x 0.045"	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd
2" x 6" x 0.062"	22'-2"	Pd	22'-2"	Pd	22'-2"	Pd	21'-5"	Ud
2" x 7" x 0.062"	26'-8"	Pd	26'-8"	Pd	25'-9"	Ud	24'-3"	Ud

Note:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. The structures designed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16'. Structures larger than these limits shall have site specific engineering.
3. Span is measured from center of beam and upright connection to fascia or wall connection.
4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
5. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable. Other conditions may offer better spans w/ enclosure site specific engineering.
6. Spans may be interpolated.
7. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

Example: Max. 'L' for 2" x 4" x 0.050" hollow section with "W" = 5'-0" = 9'-1"

Lawrence E. Bennett, P.E. FL # 16644
 CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121
 Telephone #: (386) 767-4774 Fax #: (386) 767-6556
 Email: lebpe@bellsouth.net

SECTION 1

SCREENED ENCLOSURES

Table 1.3 110 Allowable Post / Upright Heights for Primary Screen Wall Frame Members
Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 #/sq. ft.

Hollow Sections	Tributary Load Width "W" = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 2" x 0.044"	7'-5"	d	6'-5"	b	5'-8"	b	5'-1"	b	4'-8"	b	4'-3"	b	3'-11"	b
2" x 2" x 0.050"	7'-10"	d	7'-1"	b	6'-3"	b	5'-8"	b	5'-2"	b	4'-9"	b	4'-5"	b
2" x 2" x 0.090"	8'-11"	d	8'-2"	d	7'-10"	d	7'-1"	b	6'-7"	b	6'-1"	b	5'-9"	b
2" x 3" x 0.045"	8'-4"	d	7'-7"	d	7'-9"	d	6'-11"	d	6'-5"	d	5'-11"	b	5'-6"	b
2" x 4" x 0.050"	11'-2"	b	9'-7"	b	8'-6"	b	7'-9"	b	7'-1"	b	6'-7"	b	6'-1"	b
2" x 5" x 0.062"	17'-3"	b	14'-10"	b	13'-2"	b	11'-11"	b	11'-0"	b	10'-3"	b	9'-7"	b

Self Mating Sections	Tributary Load Width "W" = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 4" x 0.044 x 0.100"	11'-11"	d	10'-10"	d	10'-0"	d	9'-5"	b	8'-8"	b	8'-0"	b	7'-6"	b
2" x 5" x 0.050" x 0.100"	14'-9"	d	13'-5"	d	12'-5"	d	11'-7"	b	10'-8"	b	9'-11"	b	9'-4"	b
2" x 6" x 0.050" x 0.120"	17'-3"	d	15'-8"	d	14'-4"	b	13'-1"	b	12'-0"	b	11'-3"	b	10'-6"	b
2" x 7" x 0.055" x 0.120"	19'-8"	d	17'-6"	b	15'-7"	b	14'-2"	b	13'-1"	b	12'-2"	b	11'-5"	b
2" x 8" x 0.072" x 0.224"	24'-4"	d	22'-1"	d	20'-6"	d	19'-4"	d	18'-4"	d	17'-6"	d	16'-10"	d
2" x 9" x 0.072" x 0.224"	26'-8"	d	24'-3"	d	22'-6"	d	21'-2"	d	20'-1"	d	19'-3"	d	18'-2"	b
2" x 9" x 0.082" x 0.310"	28'-8"	d	26'-0"	d	24'-2"	d	22'-9"	d	21'-7"	d	20'-8"	d	19'-10"	d
2" x 10" x 0.092" x 0.369"	33'-3"	d	30'-3"	d	28'-1"	d	26'-5"	d	25'-1"	d	23'-11"	d	23'-1"	d

Snap Sections	Tributary Load Width "W"= Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 2" x 0.044"	6'-7"	d	5'-11"	d	5'-7"	d	5'-3"	d	4'-10"	b	4'-5"	b	4'-1"	b
2" x 3" x 0.045"	8'-10"	d	8'-1"	d	7'-6"	d	6'-11"	b	6'-3"	b	5'-9"	b	5'-3"	b
2" x 4" x 0.045"	11'-2"	d	10'-2"	d	9'-2"	b	8'-2"	b	7'-5"	b	6'-9"	b	6'-2"	b
2" x 6" x 0.062"	18'-3"	d	16'-7"	d	15'-5"	d	14'-6"	d	13'-9"	d	13'-2"	d	12'-8"	d
2" x 7" x 0.062"	20'-7"	d	18'-9"	d	17'-5"	d	16'-4"	d	15'-7"	d	14'-10"	d	14'-2"	b

Notes:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Using screen panel width 'W' select upright length 'H'.
3. Above heights do not include length of knee brace. Add vertical distance from upright to center of brace to beam connection to the above spans for total beam spans.
4. Site specific engineering required for pool enclosures over 30' in mean roof height.
5. Height is to be measured from center of beam and upright connection to fascia or wall connection.
6. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.
7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
8. Spans may be interpolated.
9. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-II.

Lawrence E. Bennett, P.E. FL # 16644
 CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121
 Telephone #: (386) 767-4774 Fax #: (386) 767-6556
 Email: lebpe@bellsouth.net

SECTION 1

SCREENED ENCLOSURES

Table 1.4 110 Allowable Post / Girt / Chair Rail Spans, Header Spans & Upright Heights for Secondary Screen Wall Frame Members

Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 # / sq. ft.

A. Sections As Horizontals Fastened To Posts With Clips

Hollow Sections	Tributary Load Width "W" = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" or Span "L" / bending (b), deflection (d)													
2" x 2" x 0.044"	7'-5"	d	6'-5"	b	5'-8"	b	5'-1"	b	4'-8"	b	4'-3"	b	3'-11"	b
2" x 2" x 0.050"	7'-10"	d	7'-1"	b	6'-3"	b	5'-8"	b	5'-2"	b	4'-9"	b	4'-5"	b
2" x 2" x 0.090"	8'-11"	d	8'-2"	d	7'-10"	d	7'-1"	b	6'-7"	b	6'-1"	b	5'-9"	b
3" x 2" x 0.045"	8'-4"	d	7'-4"	b	6'-6"	b	5'-10"	b	5'-4"	b	4'-11"	b	4'-7"	b
3" x 2" x 0.070"	9'-5"	d	8'-6"	d	7'-9"	b	7'-0"	b	6'-5"	b	5'-11"	b	5'-7"	b
2" x 3" x 0.045"	8'-4"	d	7'-7"	d	7'-9"	d	6'-11"	d	6'-5"	d	5'-11"	b	5'-6"	b
2" x 4" x 0.050"	11'-2"	b	9'-7"	b	8'-6"	b	7'-9"	b	7'-1"	b	6'-7"	b	6'-1"	b
2" x 5" x 0.062"	17'-3"	b	14'-10"	b	13'-2"	b	11'-11"	b	11'-0"	b	10'-3"	b	9'-7"	b

Snap Sections	Tributary Load Width "W"= Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" or Span "L" / bending (b), deflection (d)													
2" x 2" x 0.044"	6'-7"	d	5'-11"	d	5'-7"	d	5'-3"	d	4'-10"	b	4'-5"	b	4'-1"	b

B. Sections As Horizontals Fastened To Posts Through Side Into Screw Bosses

Hollow Sections	Tributary Load Width "W" = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" or Span "L" / bending (b), deflection (d)													
3" x 2" x 0.045"	9'-7"	b	8'-3"	b	7'-3"	b	6'-6"	b	5'-11"	b	5'-6"	b	5'-1"	b
3" x 2" x 0.070"	11'-5"	b	9'-10"	b	8'-8"	b	7'-10"	b	7'-2"	b	6'-8"	b	6'-3"	b
2" x 3" x 0.045"	11'-2"	d	9'-9"	b	8'-8"	b	7'-10"	b	7'-2"	b	6'-8"	b	6'-2"	b
2" x 4" x 0.050"	12'-6"	b	10'-9"	b	9'-6"	b	8'-7"	b	7'-11"	b	7'-4"	b	6'-10"	b
2" x 5" x 0.062"	19'-3"	b	16'-7"	b	14'-9"	b	13'-5"	b	12'-4"	b	11'-6"	b	10'-9"	b

Snap Sections	Tributary Load Width "W"= Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" or Span "L" / bending (b), deflection (d)													
2" x 2" x 0.044"	8'-10"	d	7'-8"	b	6'-9"	b	6'-0"	b	5'-5"	b	4'-11"	b	4'-7"	b

Note:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Using screen panel width "W" select girt lengths.
3. Site specific engineering required for pool enclosures over 30' in mean roof height.
4. Span/height is to be measured from center of beam and upright connection to fascia or wall connection.
5. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential guardrails provided they are attached with min. (3) #10 x 1-1/2" s.m.s. into the screw bosses and do not exceed 8'-0" o.c.
6. Girt spacing shall not exceed 6'-8".
7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
8. 2" x 4" & 2" x 5" hollow girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
9. Spans/heights may be interpolated.
10. To convert spans/heights to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

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SECTION 1

SCREENED ENCLOSURES

Table 1.6 Minimum Upright Sizes and Number of Screws for Connection of Roof Beams To Wall Uprights or Beam Splicing

Beam/Upright or Post	Upright or Post/Beam	Minimum Purlin, Girt & Knee Brace Size	Notes	Minimum Number of Screws*			Beam Stitching Screw at 24" OC
				#8 x 1/2"	#10 x 1/2"	#12 x 1/2"	
2 x 4 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	8	6	4	#10
2 x 5 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	8	6	4	#8
2 x 6 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	10	8	6	#10
2 x 7 SMB	2 x 4 SMB or H	2" x 3" x 0.044"	Full Lap	14	12	10	#12
2 x 8 SMB	2 x 5 SMB or H	2" x 3" x 0.044"	Full Lap	16	14	12	#14
2 x 9 SMB	2 x 6 SMB	2" x 3" x 0.045"	Full Lap	18	16	14	#14**
2 x 9 SMB *	2 x 7 SMB	2" x 4" x 0.050"	Full Lap	20	18	16	#14**
2 x 10 SMB	2 x 8 SMB	2" x 5" x 0.050"	Full Lap	20	18	16	#14**

Screw Size	Minimum Distance and Spacing of Screws		Gusset Plate Thickness	
	Edge To Center	Center To Center	Beam Size	Thickness
#8	5/16"	5/8"	2" x 7" x 0.055" x 0.120"	0.063"
#10	3/8"	3/4"	2" x 8" x 0.072" x 0.224"	0.125"
#12	1/2"	1"	2" x 9" x 0.072" x 0.224"	0.125"
#14 or 1/4"	3/4"	1-1/2"	2" x 9" x 0.082" x 0.306"	0.190"
5/16"	7/8"	1-3/4"	2" x 10" x 0.092" x 0.369"	0.250"
3/8"	1"	2"		

* 0.082" wall thickness, 0.310" flange thickness

** (1) Stitching screw at 16" O.C. max.

Connection Example:

2" x 7" beam & 2" x 5" at beam & gusset plate, (14) #8 x 1/2" sms & upright & gusset plate
(14) #8 x 1/2" sms ea. side of beam & upright.

Note:

1. Connection of 2" x 6" to 2" x 4" shall use a full lap cut or 1/16" gusset plate.
2. For beam splice connections the number of screws shown is the total for each splice with 1/2 the screws on each side of the cut.
3. The number of screws is based on the maximum allowable moment of the beam.
4. The number of deck anchors is based on RAWL R Tapper allowable load data for 2,500 psi concrete and / or equal anchors may be used. The number shown is the total use 1/2 per side.
5. Hollow splice connections can be made provided the connection is approved by the engineer.
6. If a larger than minimum upright is used the number of screws is the same for each splice with 1/2 the screws on each side of the cut.
7. The side wall upright shall have a minimum beam size as shown above, ie., a 2" x 4" upright shall have a 2" x 3" beam.
8. For minimum girt size read upright size as a beam and purlin size is minimum girt size. (i.e. 2" x 9" x 0.072" x 0.224" s.m.b. w/ 2" x 6" x 0.050 x 0.120" s.m.b. upright requires a 2" x 3" x 0.045" girt / chair rail.)

Lawrence E. Bennett, P.E. FL # 16644
CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121
Telephone #: (386) 767-4774 Fax #: (386) 767-6556
Email: lebpe@bellsouth.net

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Table 1.7 Minimum Size Screen Enclosure Knee Braces and Anchoring Required
Aluminum 6063 T-6

Brace Length*	Extrusion	Anchoring System
0' - 2'-0"	2" x 2" x 0.044"	2" H-Channel With (3) #10 x 1/2" each leg of channel
To 3'-0"	2" x 3" x 0.045"	2" H-Channel With (3) #10 x 1/2" each leg of channel
To 6'-0"	2" x 4" x 0.044" x 0.125"	2" H-Channel With (4) #10 x 1/2" each leg of channel

* Knee brace length shall be the horizontal and vertical length @ a 45° angle from the center of the connection to the face of the beam or upright.

Note:

1. For required knee braces greater than 4'-6" contact engineer for specifications and details.
2. Cantilever beam detail shown on page 1-40 shall be used for transom wall to host structure attachment when knee brace length exceeds 6'-0".

Table 1.8 K-Bracing Fastening Schedule

Maximum Wall Width =	Number of #10 x 3/4" S.M.S. Required				
	Corner Post @ Top	Diagonals (K) per End	Intermediate Post @ Chair Rail	Corner Post @ Bottom	Plate to Sole Plate
20'-0"	2	2	4	2	2
30'-0"	2	2	4	2	2
40'-0"	3	4	6	2	2
50'-0"	4	5	8	3	3
60'-0"	6	7	12	3	3

Use front wall width when determining number of s.m.s. for the side wall K-bracing.

Use side wall width when determining number of s.m.s. for the front and / or back wall K-bracing.

Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING

P.O. Box 214368, South Daytona, FL 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556

Email: lebpe@bellsouth.net

Bennett Engineering Group, Inc.

Lawrence E. Bennett, P.E.

315 Herbert Street
Port Orange, FL 32129
386-767-4774 fax: 386-767-6556

January 1, 2009

TO ALL BUILDING DEPARTMENTS

Re: Master File Engineering
"ALUMINUM STRUCTURES DESIGN MANUAL"
2004 Florida Building Code with 2006 Supplements

Dear Building Official/Plans Examiner:

This is to certify that the following contractor/company is hereby authorized to use my "ALUMINUM STRUCTURES DESIGN MANUAL" during the year 2009.

Authorization is on a January to January basis regardless of the edition of the manual. This authorization also applies to contractor master file drawings, "ONE PERMIT ONLY" drawings or any "site specific" drawings that I may furnish the contractor.

The following contractor/company is hereby added to my 2009 MASTERFILE LIST:

Vince Richardson
Richardson Aluminum LLC
692 SW Arlington Blvd
Lake City, FL 32025

Should you have any questions, please contact me at your convenience.

Sincerely,



Lawrence E. Bennett, P.E. #16644

NOTICE OF COMMENCEMENT

PERMIT NUMBER: _____
STATE OF: FLORIDA COUNTY OF: Columbia CITY OF: _____

THE UNDERSIGNED HEREBY gives notice that improvement(s) will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

DESCRIPTION OF PROPERTY

LOT: _____ BLOCK: _____ SECTION: _____ TOWNSHIP: _____ RANGE: _____
TAX PARCEL NUMBER: 35-35-16-02309-047-HX
SUBDIVISION: _____ PLATBOOK: _____ MAP PAGE: _____
STREET ADDRESS: 146 NW Harris Lake Dr. Lake City Fla. 32055

GENERAL DESCRIPTION OF IMPROVEMENTS

TO CONSTRUCT: Screen enclosure

OWNER INFORMATION

OWNER NAME: Michelle Louise Boatwright
ADDRESS: 146 NW Harris Lake Dr. PHONE NUMBER: _____
CITY: Lake City STATE: Fla. ZIP CODE: 32055

INTEREST IN PROPERTY: _____

FEE SIMPLE TITLEHOLDER NAME: _____

Inst: 200912007553 Date: 5/6/2009 Time: 1:08 PM

22 DC, P. DeWitt Cason, Columbia County Page 1 of 1 B: 1172 P: 2037

FEE SIMPLE TITLEHOLDER ADDRESS: _____
(if other than owner)

CONTRACTOR NAME: Vince Richardson Richardson Aluminum LLC
ADDRESS: 692 SW Arlington Blvd PHONE NUMBER: 386-755-5779
CITY: Lake City STATE: Fla. ZIP CODE: 32025

BONDING COMPANY: _____
ADDRESS: _____ PHONE NUMBER: _____
CITY: _____ STATE: _____ ZIP CODE: _____

LENDER NAME: _____
ADDRESS: _____ PHONE NUMBER: _____
CITY: _____ STATE: _____ ZIP CODE: _____

Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a) 7., Florida Statutes:

NAME: _____ ADDRESS: _____

In addition to himself, Owner designates _____
of _____ to receive a copy of Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

Expiration date is one (1) year from date of recording unless a different date is specified.

SIGNATURE OF OWNER: Michelle Boatwright
SWORN to and subscribed before me this 30 day of April, A.D. 2009

Notary Public: _____

My commission Expires:



SANDRA H. TILLOTSON
MY COMMISSION # DD 629530
EXPIRES: January 31, 2011
Bonded Thru Budget Notary Services

COLUMBIA COUNTY FLORIDA DEPARTMENT OF BUILDING AND ZONING

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 35-3S-16-02309-047

Building permit No. 000027792

Use Classification SCREEN ENCLOSURE

Fire: 0.00

Permit Holder J. VINCE RICHARDSON

Waste:

Owner of Building MICHELLE BOATWRIGHT

Total: 0.00

Location: 146 NW HARRIS LAKE DRIVE, LAKE CITY, FL

Date: 05/12/2009

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

