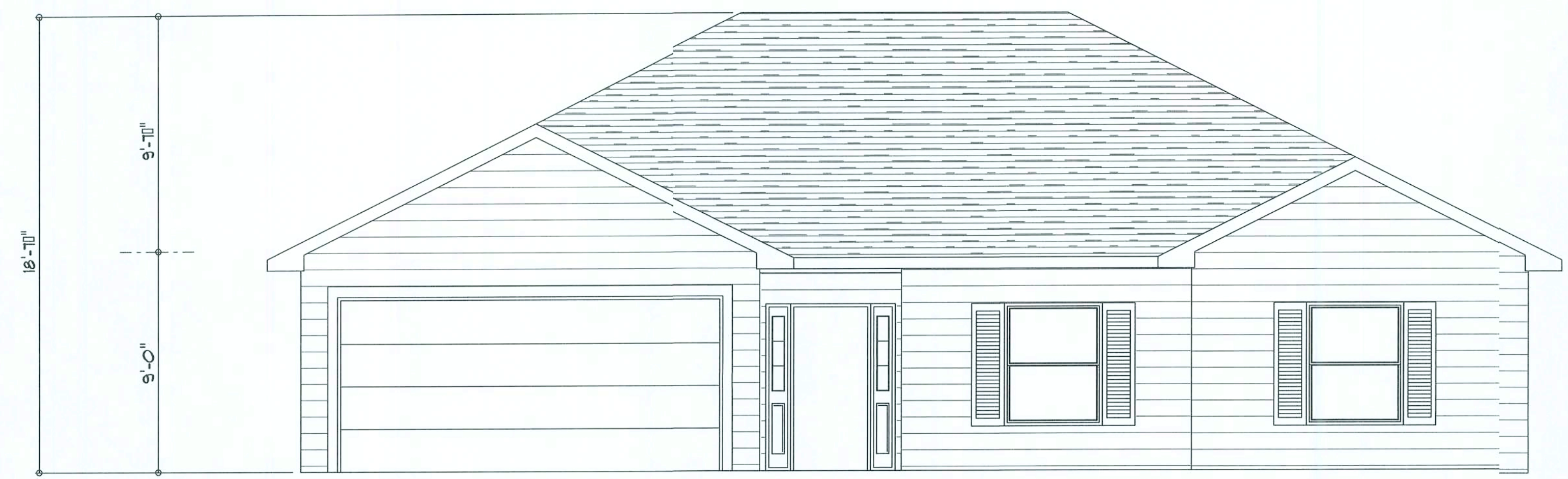
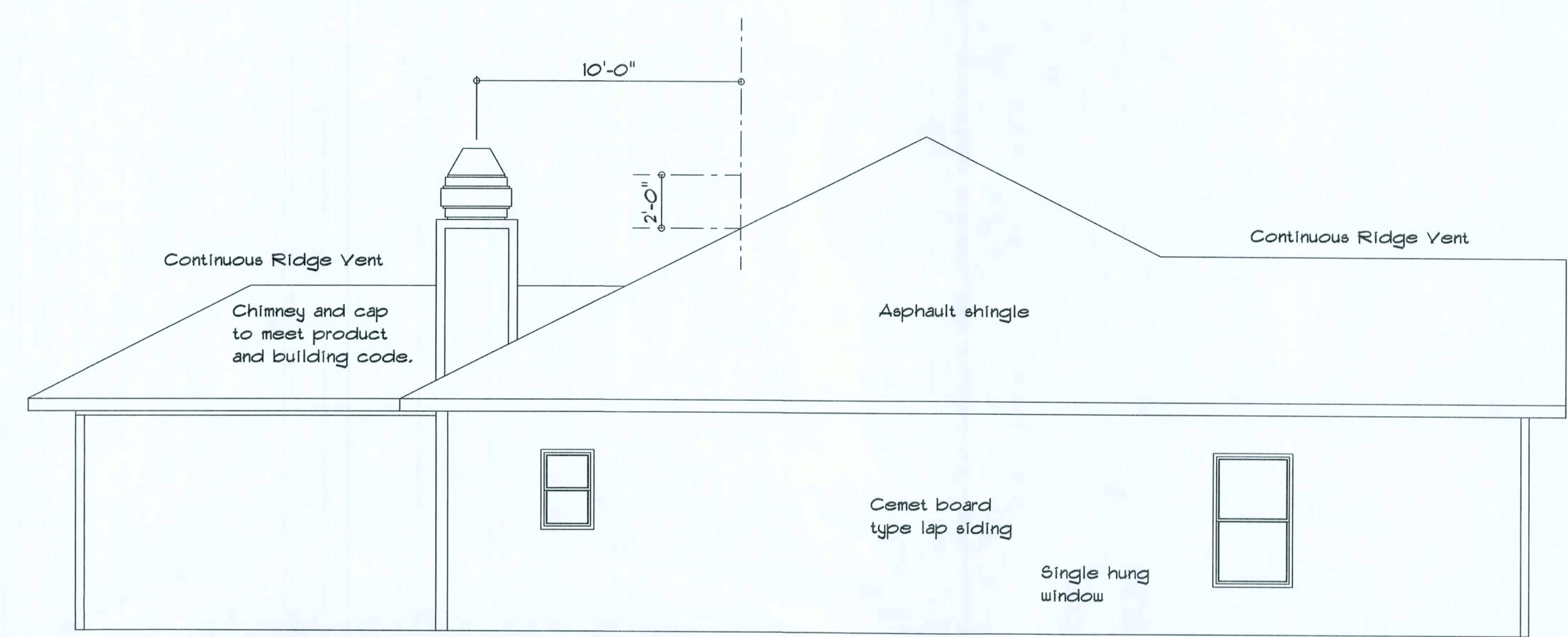


REVISIONS

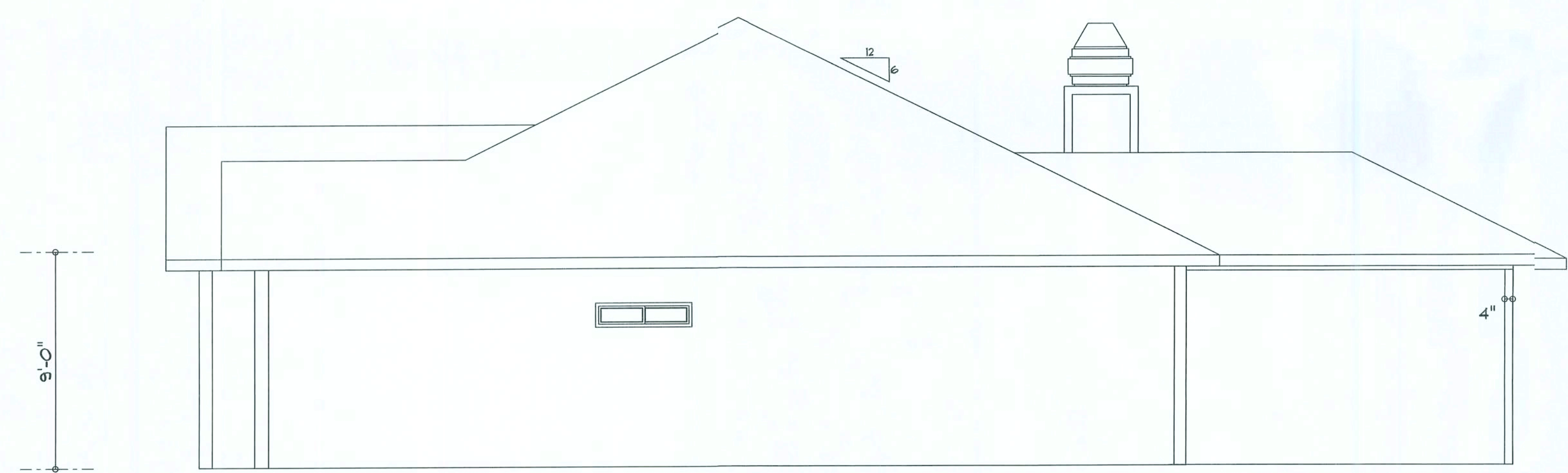

SCOTMAN  
ARCHITECTURAL DESIGN SOFTWARE



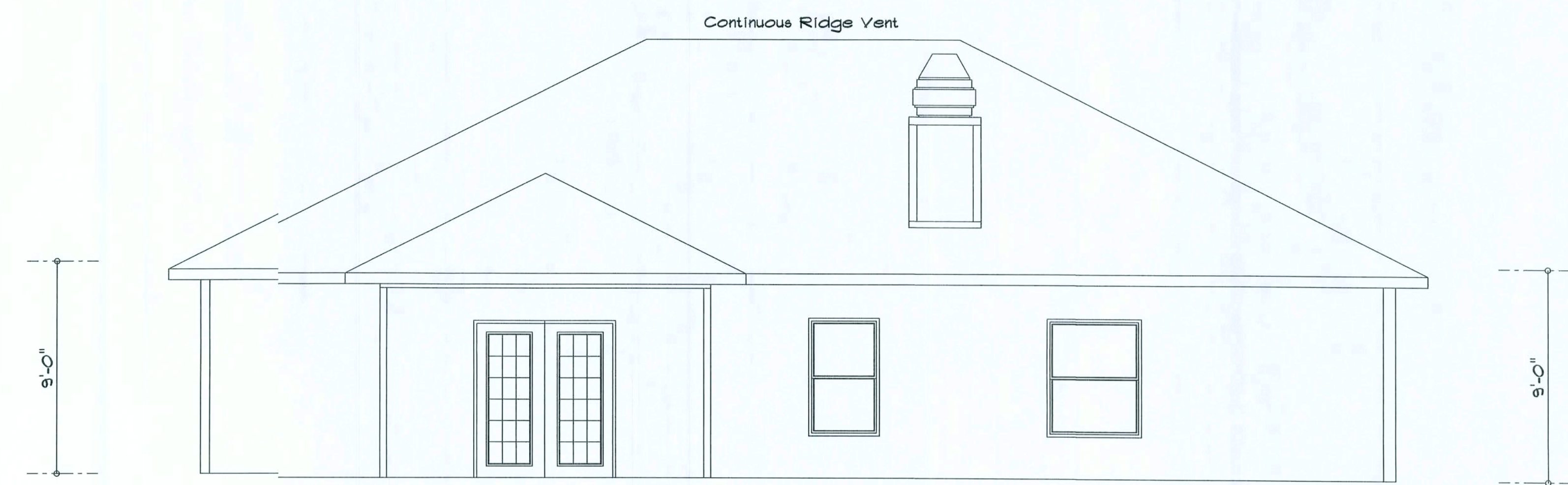
FRONT ELEVATION



RIGHT ELEVATION



LEFT ELEVATION



REAR ELEVATION

SCALE: 1/4" = 1'-0"

*William B. Wood*  
Mike Roberts  
Owner

Contractor  
*William B. Wood*  
Mike Roberts  
Owner

Chuck Wood  
P.O. Box 3535  
Lake City, Florida 32025  
Phone: (386) 755 - 8699  
Fax: (386) 755 - 8699

PRINTED DATE:  
December 26, 2007

DRAWN BY: Chuck Wood

CHECKED BY:

DESIGNED BY:  
Mike Roberts

FINALS DATE:  
06 / OCT / 04

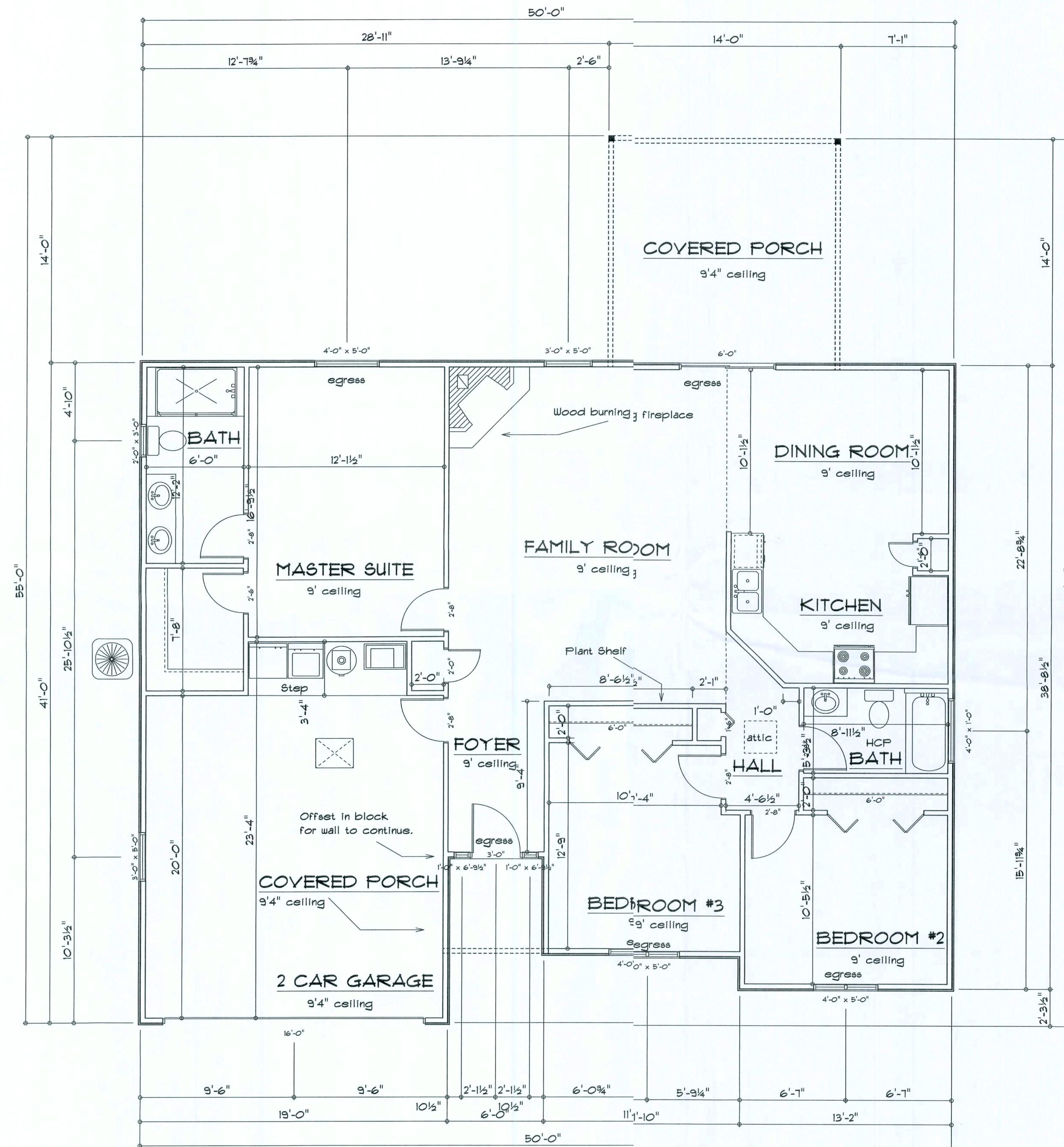
JOB NUMBER:  
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1



# REVISIONS

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



## AREA SUMMARY

Heated	1495
Garage	410
Porch	232
TOTAL	2137

## 1ST FLOOR LAYOUT

SCALE: 1/4" = 1'-0"

Contractor  
*Chuck Wood*  
Mike Roberts  
Owner

Chuck Wood  
P.O. Box 3535  
Lake City, Florida 32025  
Phone: (386) 755 - 8699  
Fax: (386) 755 - 8699

PRINTED DATE:

DRAWN BY: Chuck Wood

CHECKED BY:

DESIGNED BY:  
**Chuck Wood**

FINALES DATE:  
06 / OCT / 04

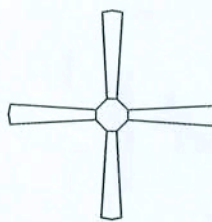







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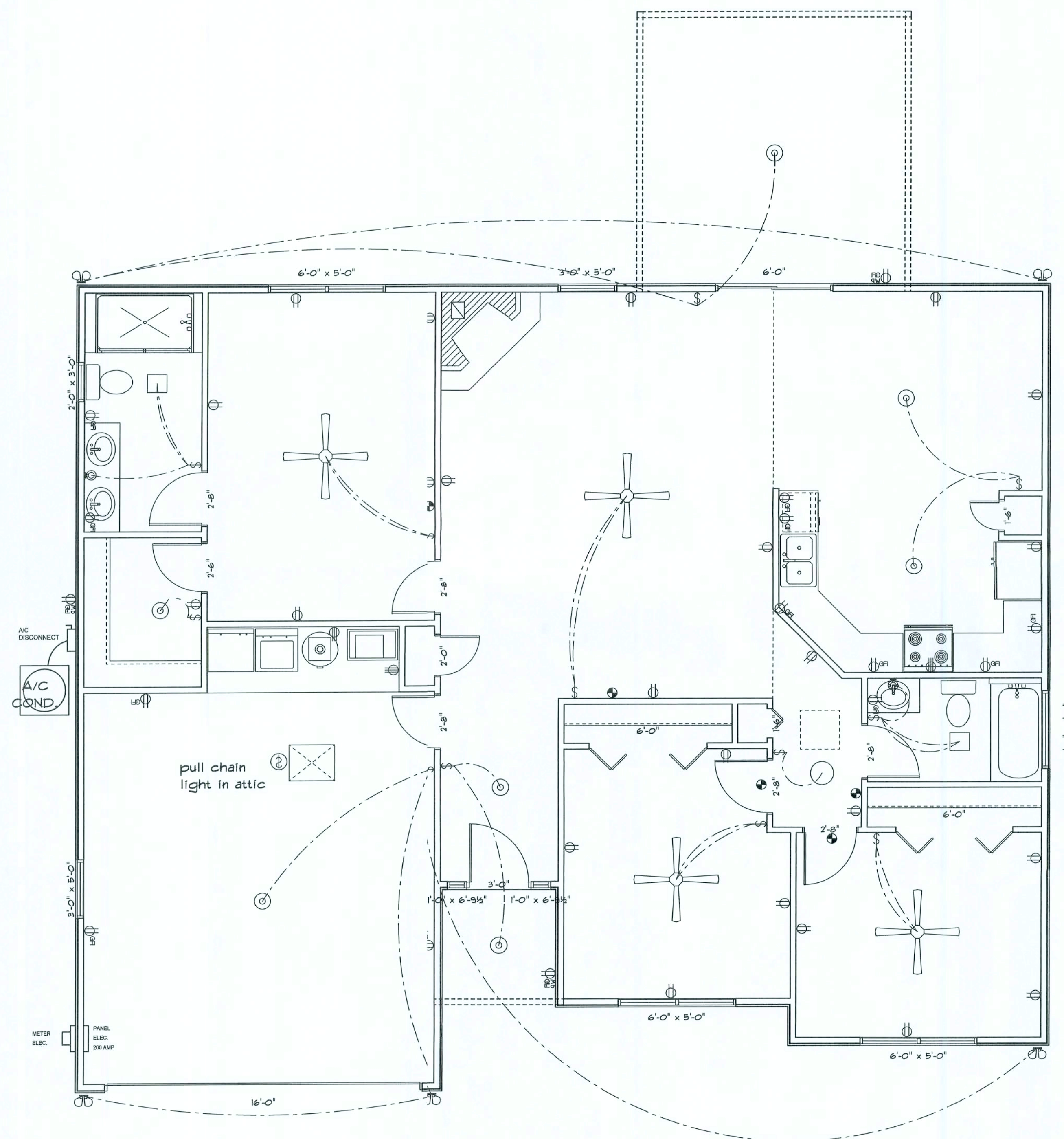
# REVISIONS

**SOFTPLAN**  
 ARCHITECTURAL DESIGN SOFTWARE

ELECTRICAL	COUNT	SYMBOL
ceiling fan	4	
ceiling lamp globe	1	
ceiling light vent square	2	
ceiling pull switch	1	
ceiling globe light	7	
double spotlight	5	
wall mount l	2	
switch	12	

## Electrical Plan Notes:

- E-1 Wire all appliances, HVAC units and other equipment per manufactures specifications.
- E-2 Consult the owner for the number or sepearate telephone lines to be installed. Owner is responsible for all overages not noted on plan.
- E-3 All installations shall be per national code.
- E-4 All smoke detectors shall be 120v with battery back-up of the photoelectric type, and shall be interlocked together. Install inside and near all bedrooms.
- E-5 Telephone, television and other low voltage devices or outlets shall be as per the owners directions and in accordance with applicable sections of the National Electric Codes latest edition. Owner is responsible for all overages not noted on plan.
- E-6 Electrical contractor shall be responsible for the design and sizing of electrical service and circuits.
- E-7 Entry of service (underground or overhead) to be determined by contractor agreement.
- E-8 All bedroom receptacles shall be AFCI (arc fault circuit interrupter).
- E-9 All outlets to be located above base flood elevation.
- E-10 All exterior GFI outlets shall be weatherproof.
- E-11 Overcurrent Protection device shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.



**ELECTRICAL PLAN**  
 SCALE: 1/4" = 1'-0"

## Contractor

*William G. Wood*

**Mike Roberts**  
 Owner

Chuck Wood  
 P.O. Box 3535  
 Lake City, Florida 32025  
 Phone: (886) 755 - 8699  
 Fax: (316) 755 - 8699

PRINTED DATE:

DRAWN BY: Chuck Wood  
 CHECKED BY:

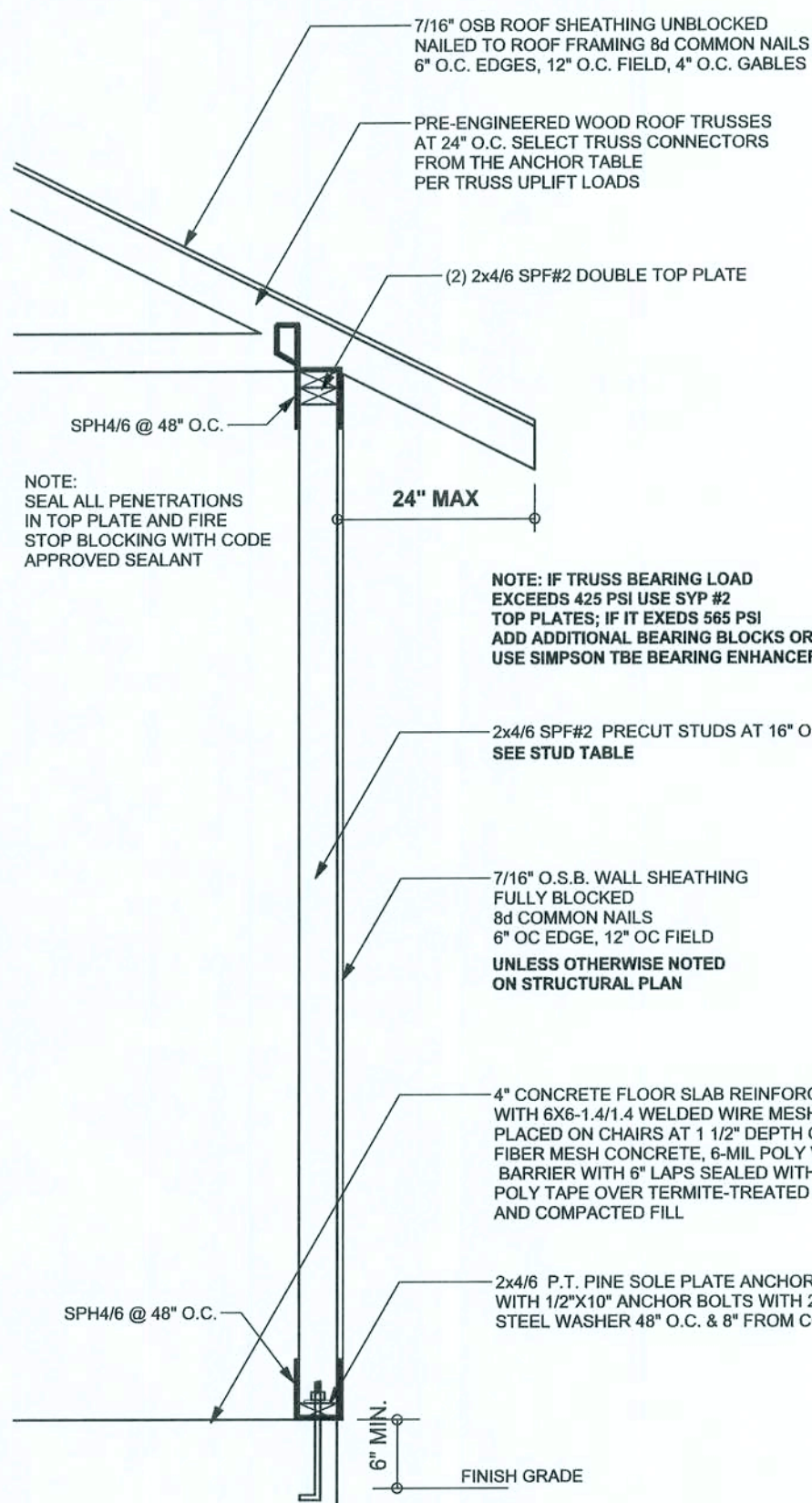
DESIGNED BY:  
**Chuck Wood**

FINALES DATE  
 06 / OCT ,04

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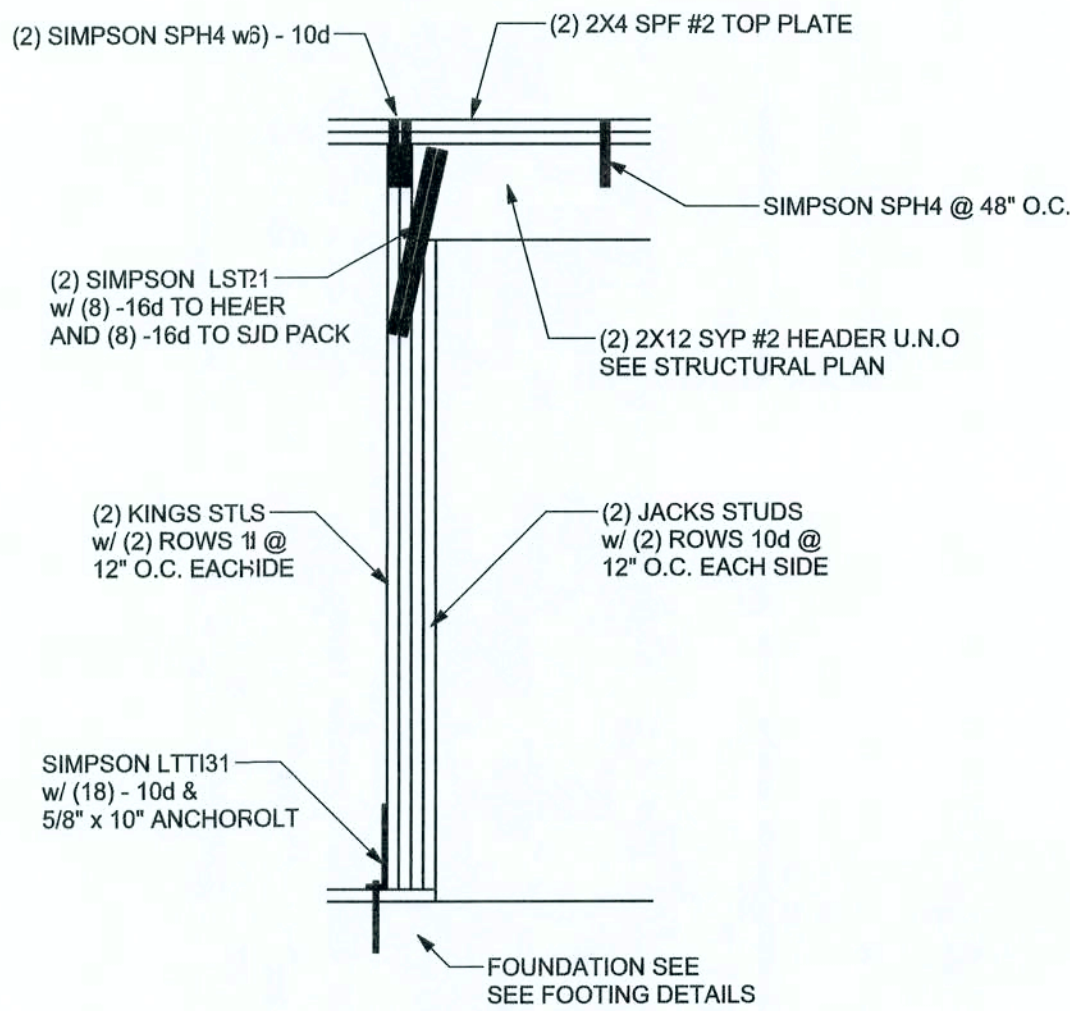
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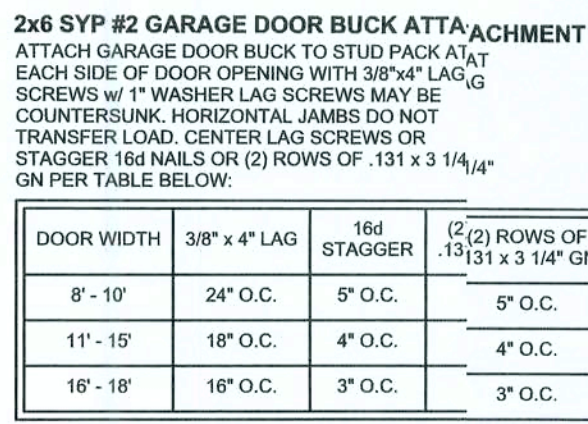
ONE STORY WALL SECTION

SCALE: 3/4\"/>



TYPICAL GRADE DOOR HEADER STRAPING DETAIL

SCALE: 1/2\"/>

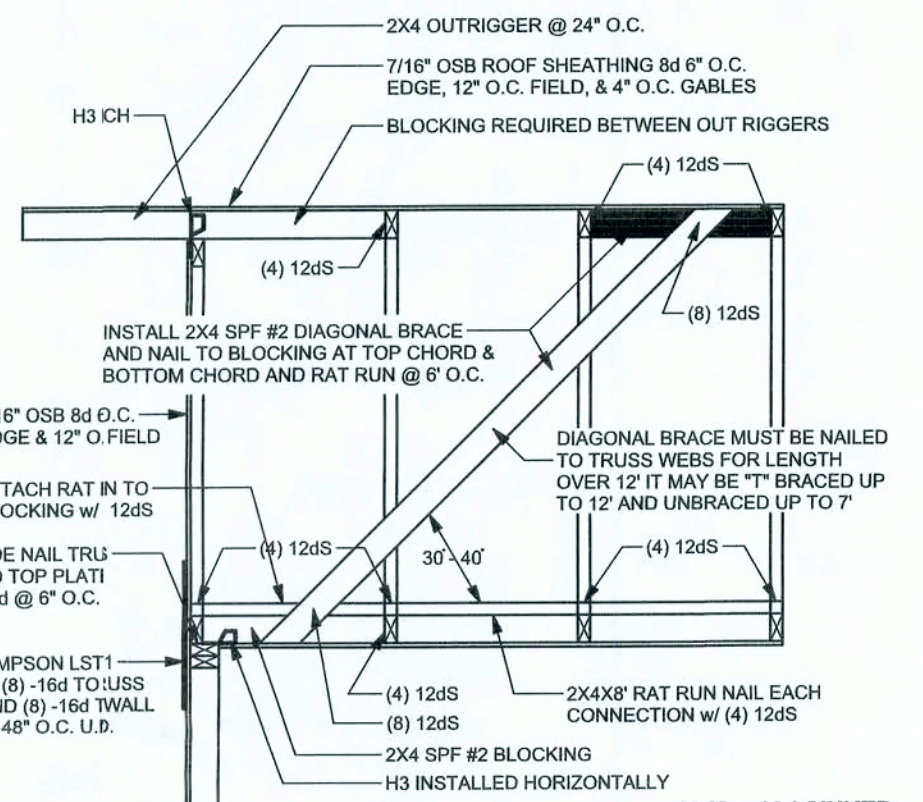


GARAGE DOOR BUCK INSTALLATION DETAIL

SCALE: N.T.S.

GRADE & SPECIES TABLE

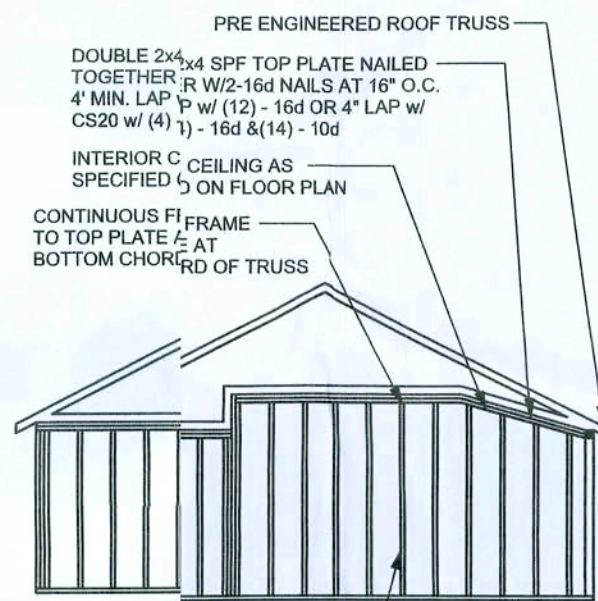
		Fb (psi)	E (10 <sup>6</sup> psi)
2x8 <sub>8</sub>	SYP #2	1200	1.6
2x11 <sub>10</sub>	SYP #2	1050	1.6
2x11 <sub>12</sub>	SYP #2	975	1.6
GLE <sub>8</sub>	24F-V3 SP	2400	1.8
LST <sub>1</sub>	TIMBERSTRAND	1700	1.7
LV <sub>1</sub> /L	MICROLAM	1600	1.9
PS <sub>1</sub> /L	PARALAM	2900	2.0



SPACE RAT RUN & DIAGONAL BRACE 6'-0\"/>

GABLE BRACING DETAIL

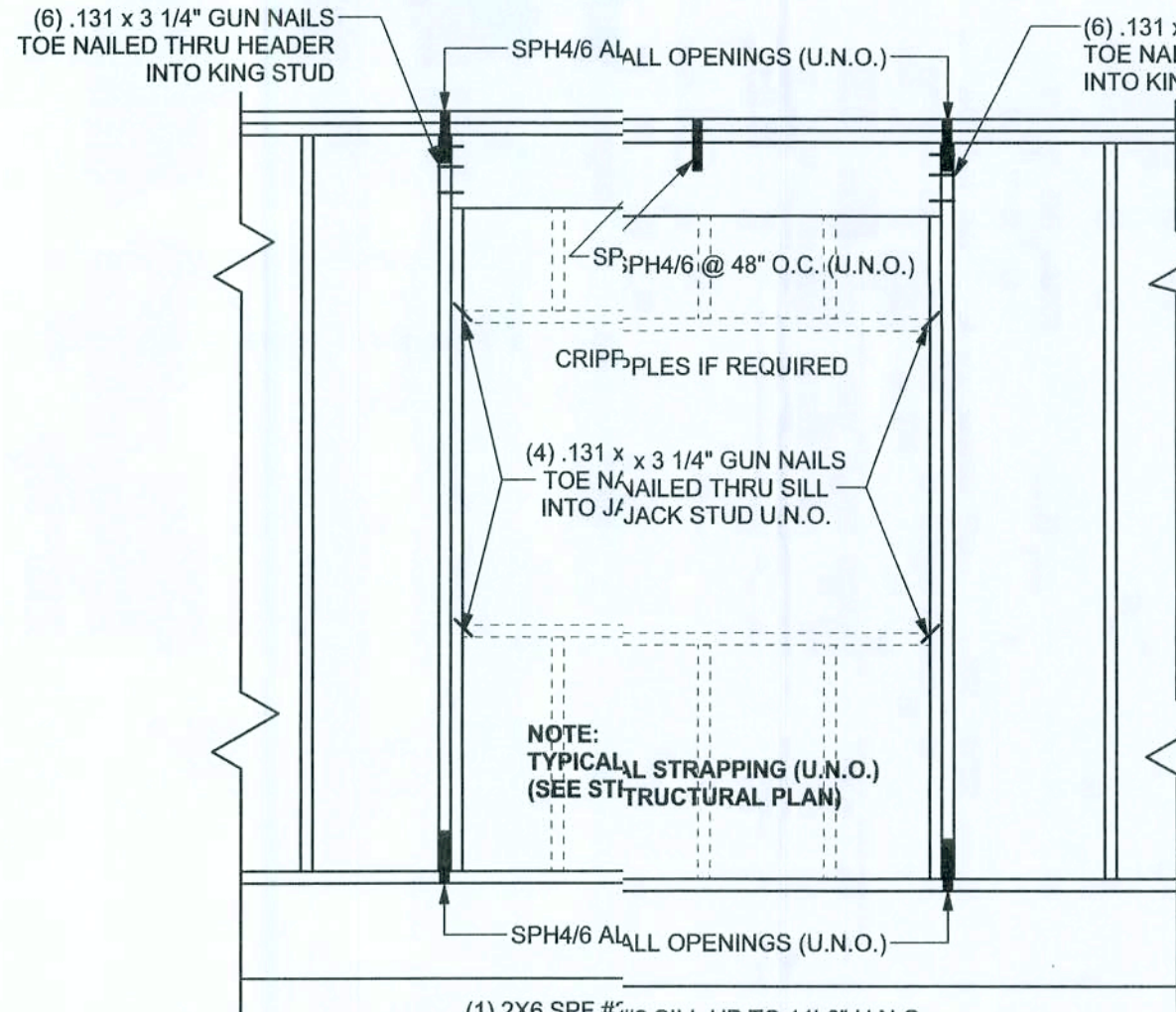
SCALE: 1/2\"/>



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.

NOTE: IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48\"/>



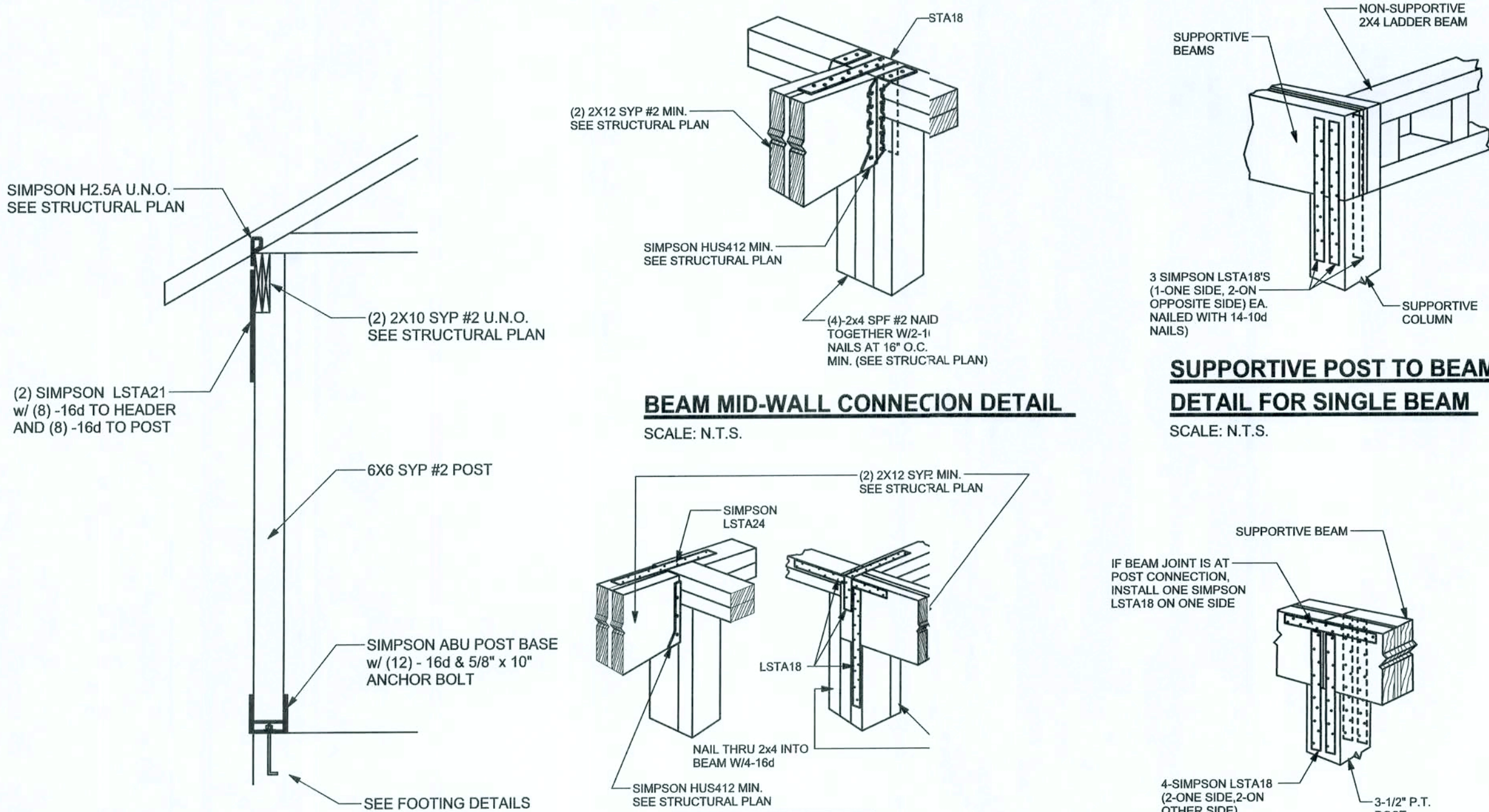
TYPICAL HEADER STRAPING DETAIL

SCALE: 1/2\"/>

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

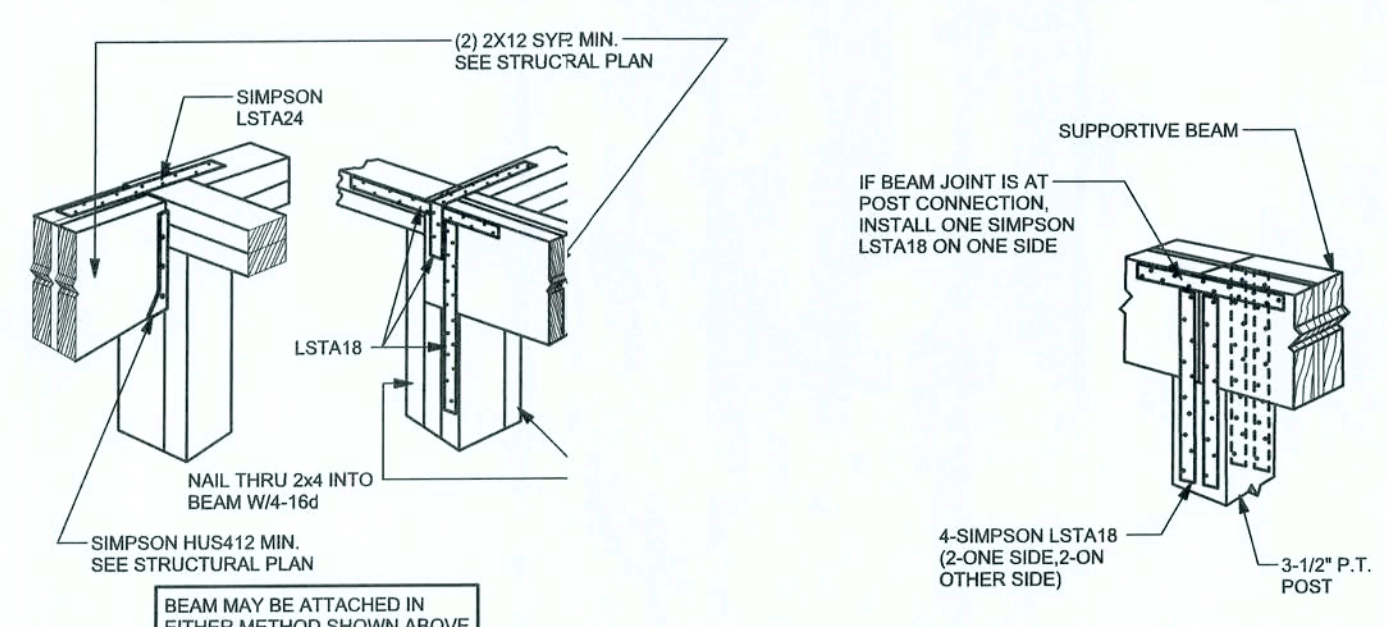
(1) 2x4 @ 16\"/>	TO 11'-9\"/>
(1) 2x4 @ 12\"/>	TO 13'-0\"/>
(1) 2x6 @ 16\"/>	TO 18'-10\"/>
(1) 2x6 @ 12\"/>	TO 20'-0\"/>

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY THE FOLLOWING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16\"/>



BEAM MID-WALL CONNECTION DETAIL

SCALE: N.T.S.



BEAM CORNER CONNECTION DETAIL

SCALE: N.T.S.

SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 8\"/>

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH/ WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WYM OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F<sub>y</sub> = 60 KSI. ALL LAP SPICES 40\"/>

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F<sub>b</sub> = 2.4ksi, E = 1800ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16\"/>

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE THE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7\"/>

WASHERS: WASHERS USED WITH 1/2\"/>

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REQUIREMENTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OWNS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

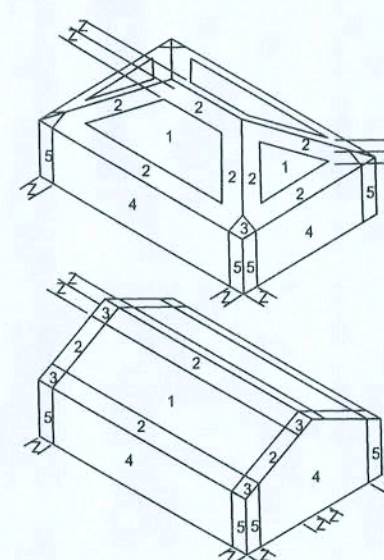
UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2500	< 2490	2 - HTS24			
< 2050	< 1785	LG2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MGT		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*				TO STUDS	TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2675	H20A	2-5/8" BOLTS		5/8" AB
< 4176	< 3576	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAH42	16-16d		
< 3335	< 3335	HPAH422	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU68	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE  
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1.) BASIC WIND SPEED = 110 MPH
- 2.) WIND EXPOSURE = B
- 3.) WIND IMPORTANCE FACTOR = 1.0
- 4.) BUILDING CATEGORY = II
- 5.) ROOF ANGLE = 10-45 DEGREES
- 6.) MEAN ROOF HEIGHT = <30 FT
- 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft <sup>2</sup> )	
	10	100
1	19.9	21.8
2	19.9	25.5
2.0H		40.6
3	19.9	25.5
3.0H		68.3
4	21.8	23.6
5	21.8	29.1
Doors & Windows		21.8
Worst Case (Zone 5, 10 R2)		21.8
8x7 Garage Door		19.5
16x7 Garage Door		18.5

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3.12)
ROOF	20 PSF (FLAT OR <4.12)
	16 PSF (4.12 TO <12.12)
	12 PSF (12.12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS

SOFTPLAN  
ARCHITECTURAL SOFTWARE

WINDLOAD ENGINEER: Mark Discovery, P.E. No. 53915, POB 88, Lake City, FL 32656, 385-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discovery, P.E. If resolution, Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, or the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK COSWAY  
P.E. 53915

Mike Roberts  
26 DEC 07  
SAL

Mike Roberts

SpecHouse  
Lot 8 Saddle Ridge S/D

ADDRESS:  
Lot 8 Saddle Ridge S/D  
Columbia County, Florida  
Mark Discovery P.E.  
P.O. box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE:  
December 26, 2007

STRUCTURAL BY:

FINALS DATE:  
26 / Dec / 07

JOB NUMBER:  
712263

DRAWING NUMBER

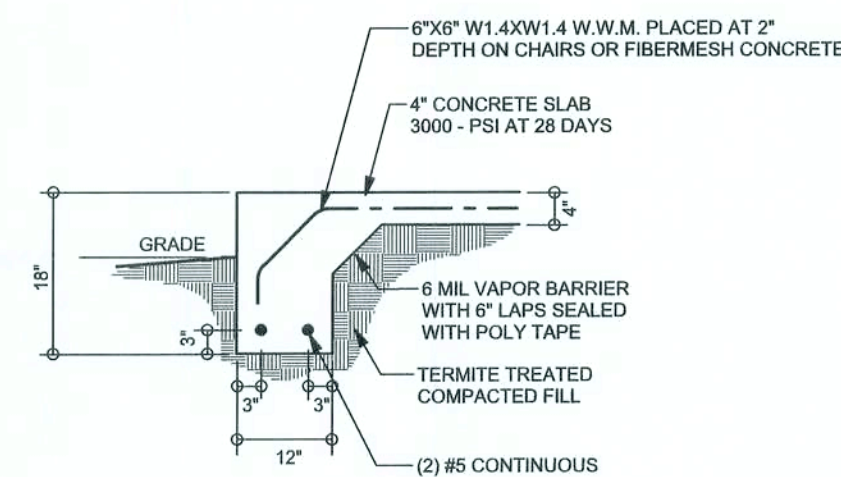
S-1

OF 3 SHEETS

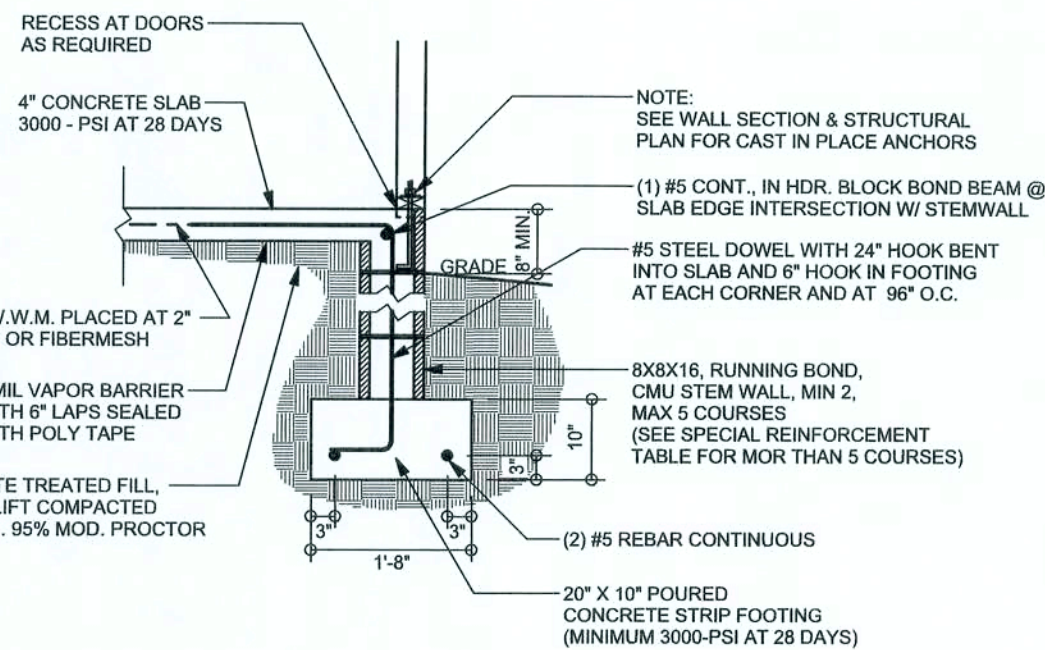


REVISIONS

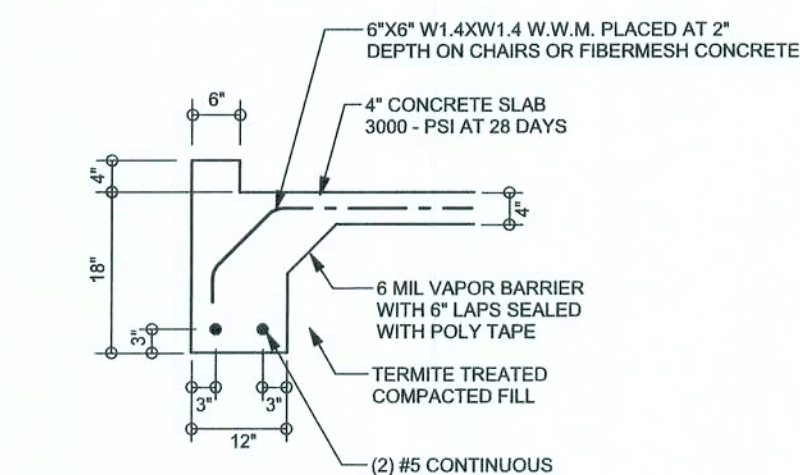
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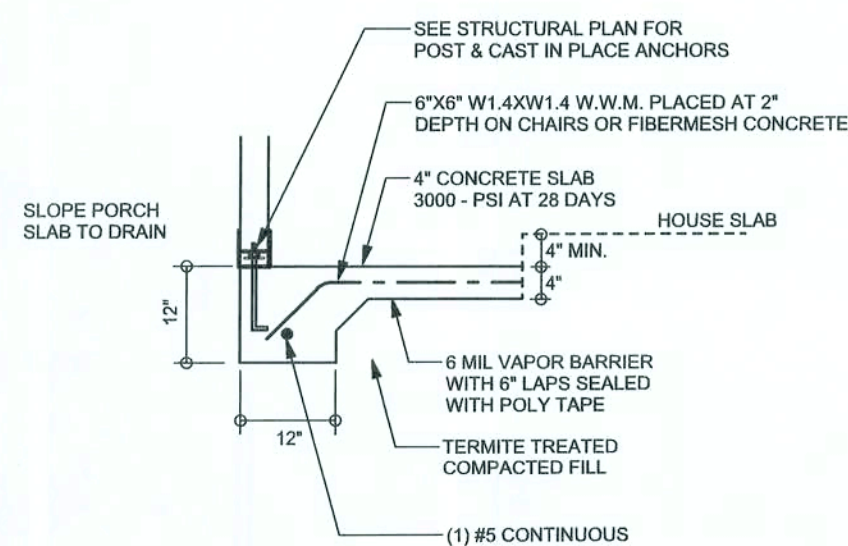
**F1 MONOLITHIC FOOTING**  
SCALE: 1/2" = 1'-0"



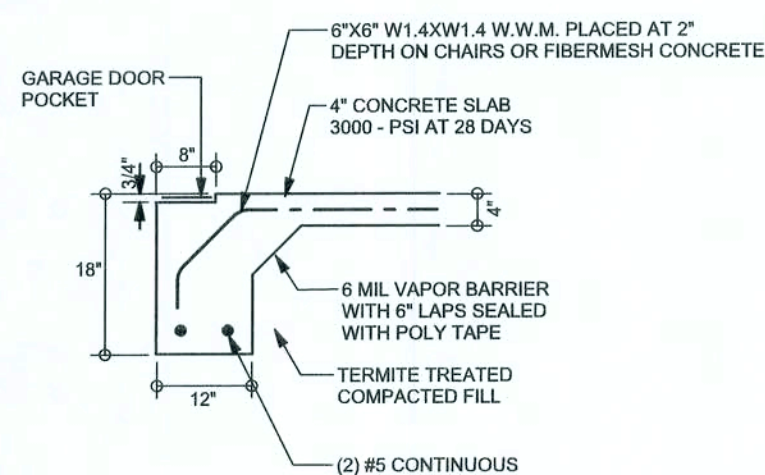
**F9 OPTIONAL STEM WALL FOOTING**  
SCALE: 1/2" = 1'-0"



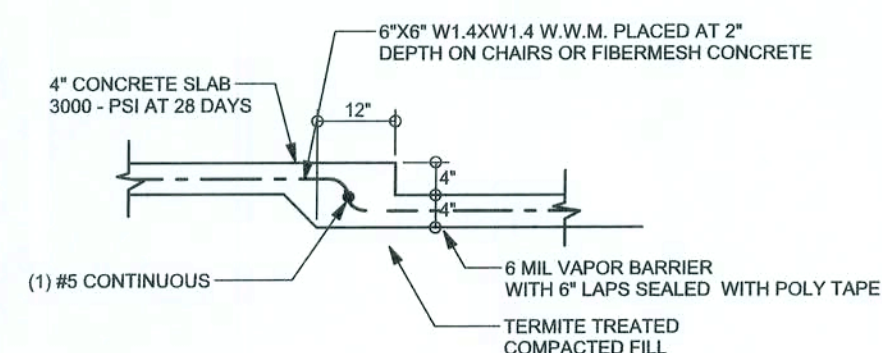
**F8 GARAGE CURB FOOTING**  
SCALE: 1/2" = 1'-0"



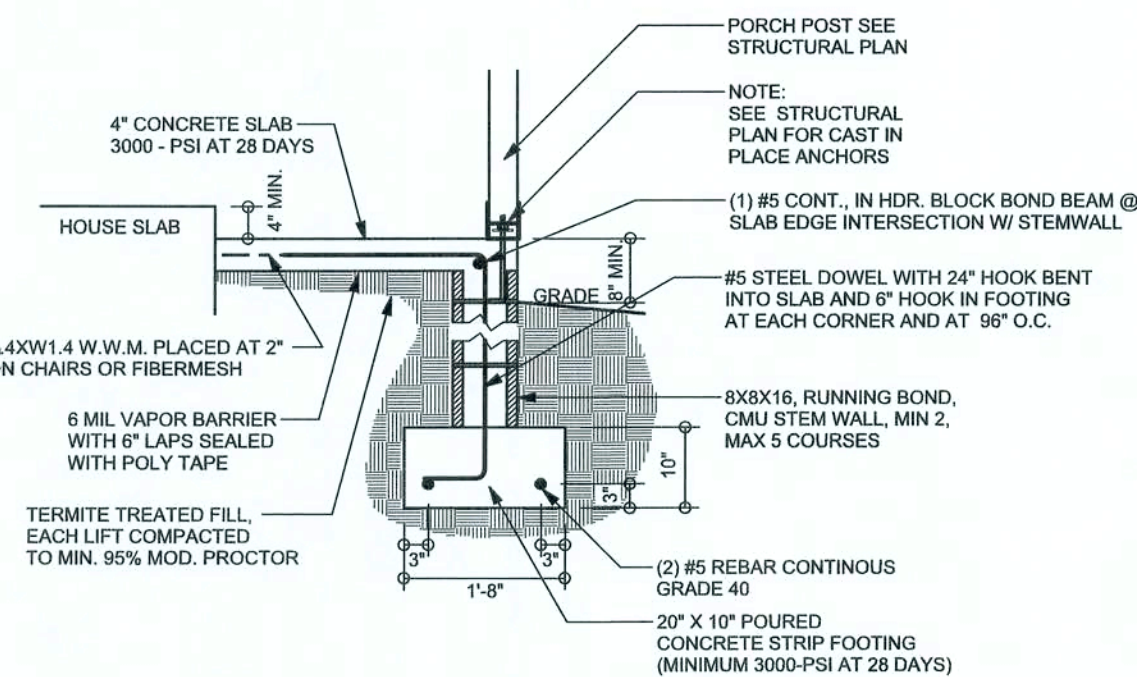
**F5 PORCH FOOTING**  
SCALE: 1/2" = 1'-0"



**F4 GARAGE DOOR FOOTING**  
SCALE: 1/2" = 1'-0"



**F6 TYPICAL NON-BEARING STEP FOOTING**  
SCALE: 1/2" = 1'-0"

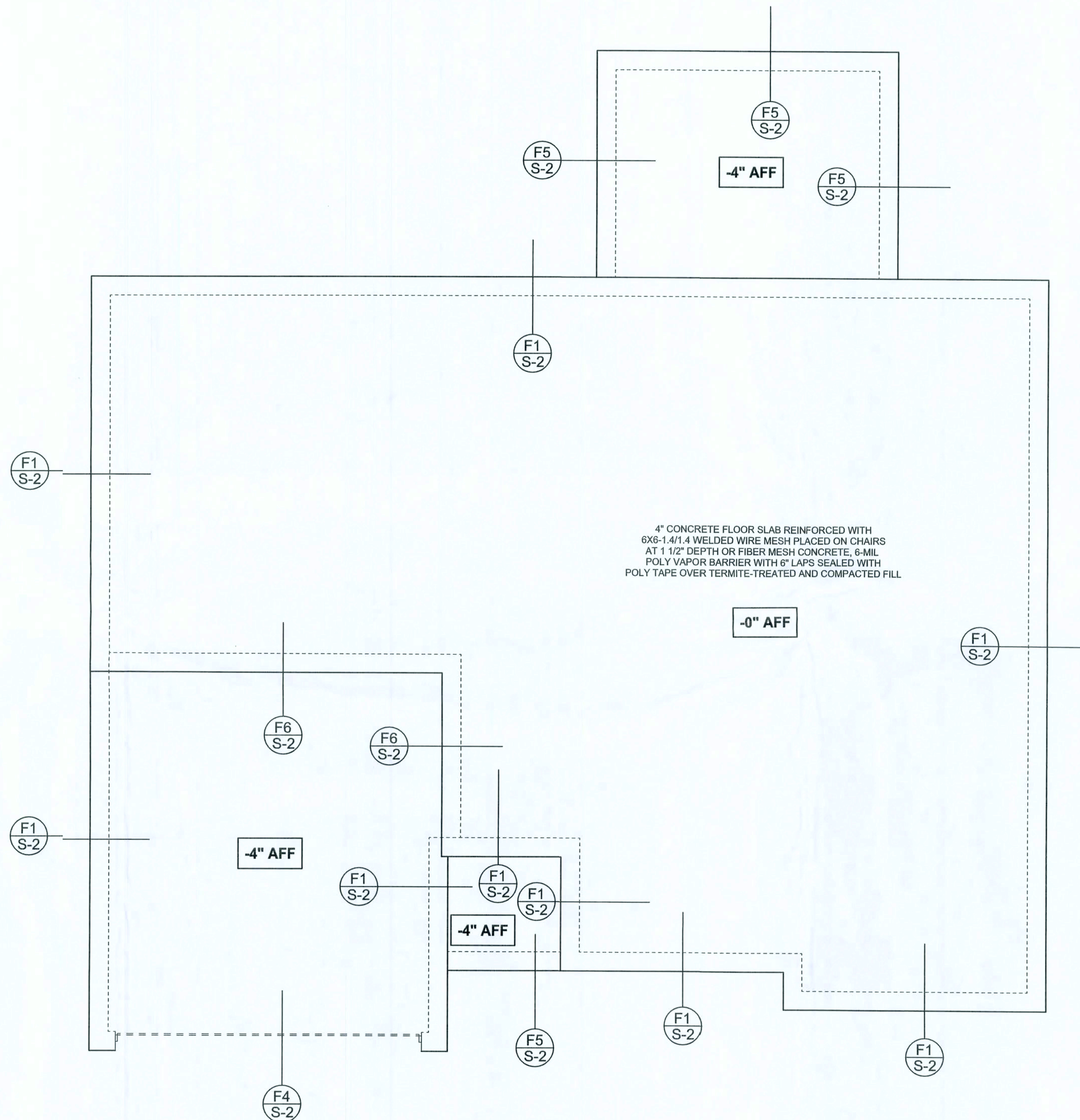


**F12 OPTIONAL STEM WALL PORCH FOOTING**  
SCALE: 1/2" = 1'-0"

**TALL STEM WALL TABLE**

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 16S continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



**FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Dissway  
PE No. 53915, POB 88, Lake City, FL  
32056, 386-754-5418

**DIMENSIONS:**  
Stated dimensions are scaled dimensions. Refer all questions to Mark Dissway, P.E., or resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with section 901.2.1, Florida building code residential 2004 to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISSWAY  
P.E. 53915

*Mark Dissway*  
20DEC07  
SEAL

Mike Roberts

SpecHouse  
Lot 8 Saddle Ridge S/D

ADDRESS:  
Lot 8 Saddle Ridge S/D  
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PRINTED DATE:  
December 26, 2007

STRUCTURAL BY:

FINALS DATE:  
26 / Dec / 07

JOB NUMBER:  
712263

DRAWING NUMBER

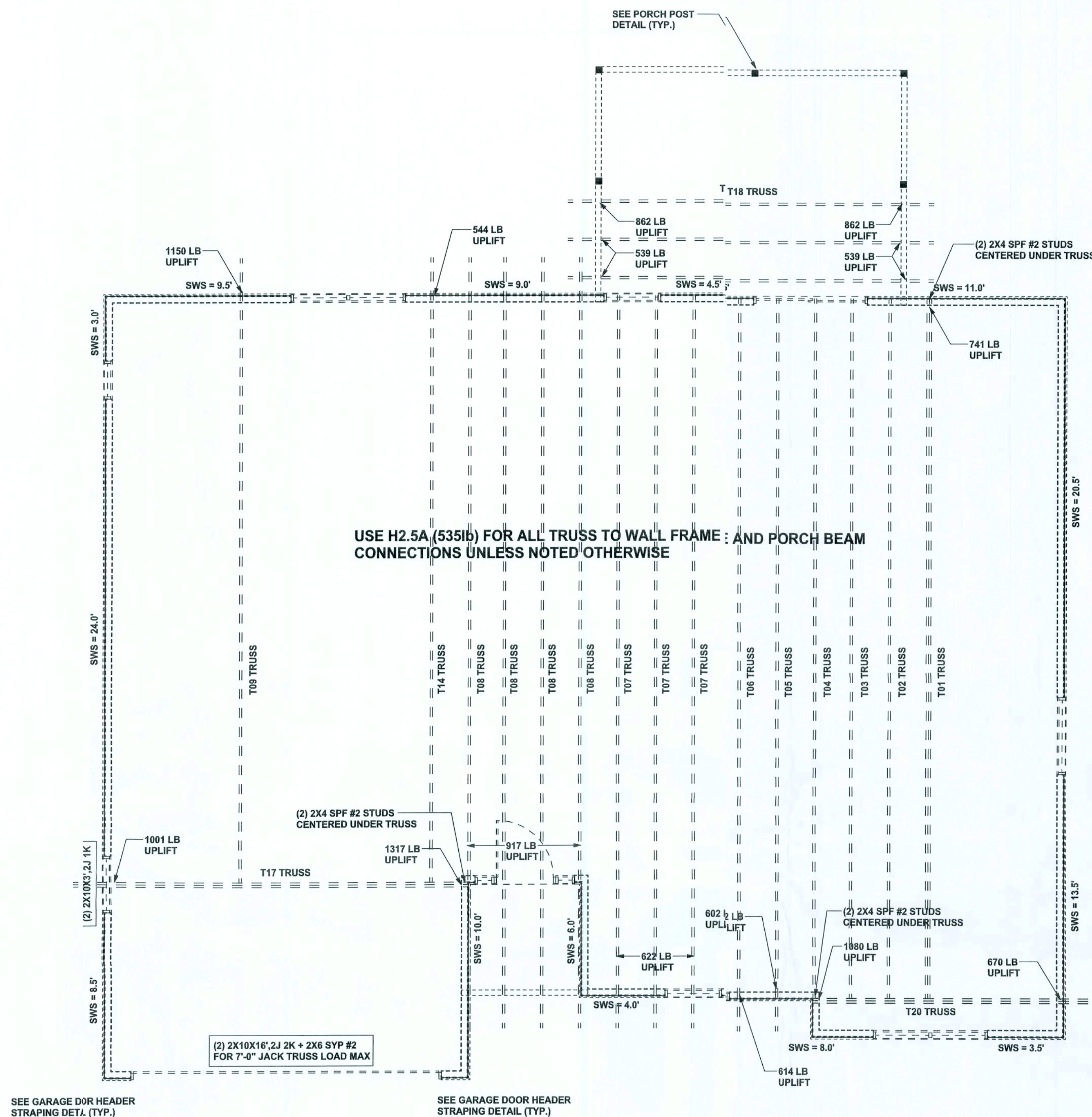
**S-2**

OF 3 SHEETS



# REVISIONS

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



## STRUCTURAL PLAN SCALE: 1/4" = 1'-0"

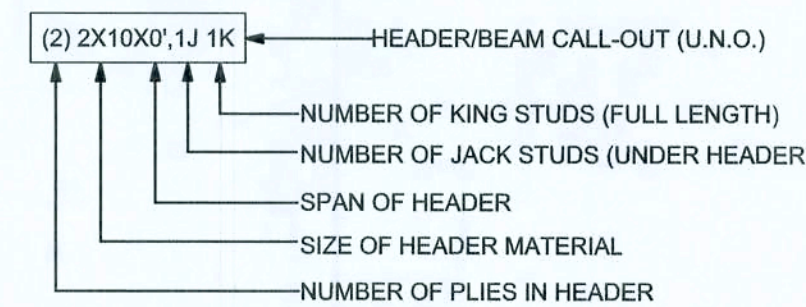
### STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS11-03, BC-B1, BCS1-B2, & BCS1-B3. BCS1-B1, BCS1-B2, & BCS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

### WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL
IBW	1ST FLOOR INTERIOR BEARING WALL
IBW	2ND FLOOR INTERIOR BEARING WALL

### HEADER LEGEND



### TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	33.6'	85.5'
LONGITUDINAL	28.6'	49.5'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L284527

WINDLOAD ENGINEER: Mark Disoway,  
PE No. 53915, POB 88, Lake City, FL  
32056, 386-754-5411

DIMENSIONS:  
Stated dimensions are to be scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with section 1301.2.1, Florida building code residential 200+ to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53915

Mike Roberts  
26 DEC 07  
SEAL

### Mike Roberts

Spec House  
Lot 8 Saddle Ridge S/D

ADDRESS:  
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S-3

OF 3 SHEETS