

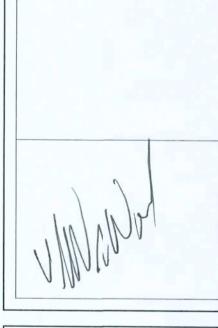
REVISIONS

SOFTPIAN ARCHITECTRAL DESIGN SOFTWARE

ELECTRICAL	COUNT	SYMBOL
ceiling fan	4	
ceiling lamp globe	1	
ceiling light vent square	2	
ceiling pull switch	1	(\$)
ceiling globe light	٦	0
double spotlight	5	₫₽
wall mount 1	2	©
switch	12	\$

Electrical Plan Notes:

- E-1 Wire all appliances, HVAC units and other equiptment per manufactures specifications.
- E-2 Consult the owner for the number or seperate telephone lines to be installed. Owner is responsible for all overages not noted on plan.
- E-3 All installations shall be per national code.
- E-4 All smoke detectors shall be 120v with battery back-up of the photoelectric type, and shall be interlocked together. Install inside and near all bedrooms.
- E-5 Telephone, television and other low voltage devices or outlets shall be as per the owners directions and in accordance with applicable sections of the National Electric Codes latest edition. Owner is responsible for all overages not noted on plan.
- E-6 Electrical contractor shall be responssible for the design and sizing of electrical service and circuits.
- E-7 Entry of service (underground or overhead) to to be determined by contractor agreement.
- E-8 All bedroom receptacles shall be AFCI (arc fault circuit interrupter).
- E-9 All outlets to be located above base flood elevation.
- E-10 All exterior GFI outlets shall be weatherproof.
- E-II Overcurrent Protection device shall be installed on the exterior ofstructures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equiptment ground.



Contractor William le War L

Mike Roberts
Owner

Chick Wood P.O Box 3535 Lake City, Florida 32025 Phone: (386) 755 - 8699 Fax: (386) 755 - 8699

PRIITED DATE:

DRAWN BY: CHECKED BY:
Chuck Wood
DE:IGNED BY:

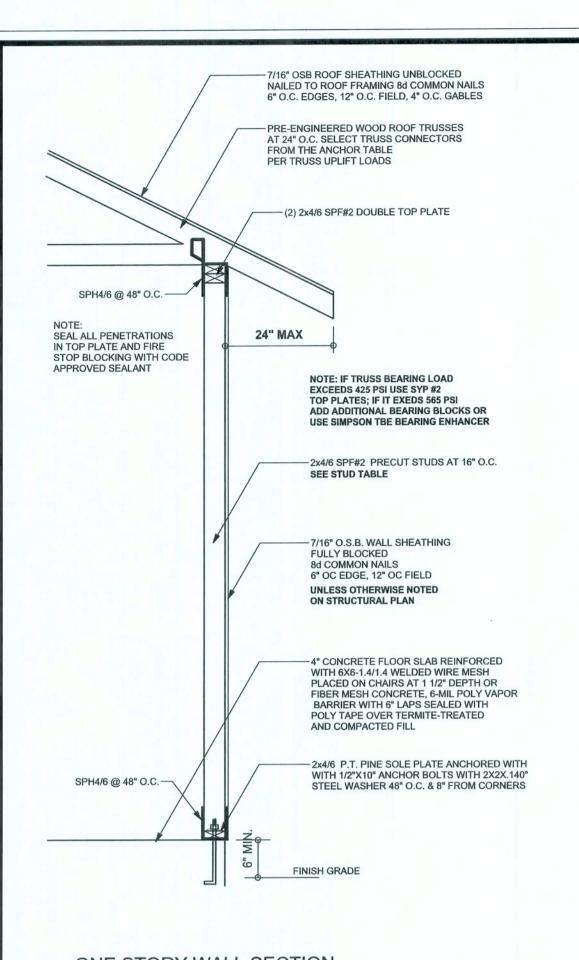
Chuck Wood

FINALES DATE 06 / OCT /04

JOB NUMBER:

DRAVING NUMBER

3



ONE STORY WALL SECTION SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, XTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

(2) 2X10 SYP #2 U.N.O.

-6X6 SYP #2 POST

w/ (12) - 16d & 5/8" x 10"

—SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

ANCHOR BOLT

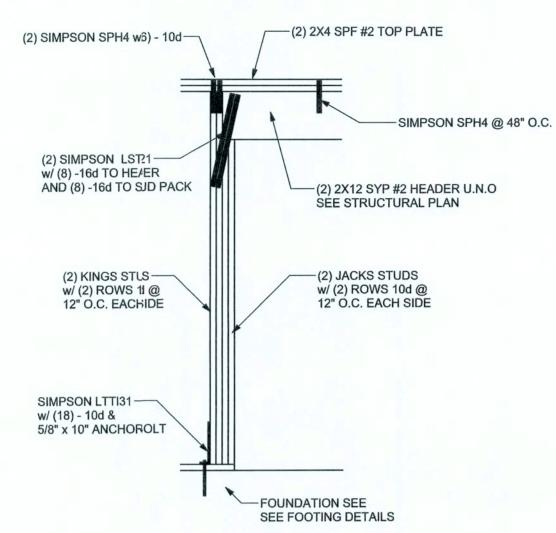
SEE STRUCTURAL PLAN

SIMPSON H2.5A U.N.O. -SEE STRUCTURAL PLAN

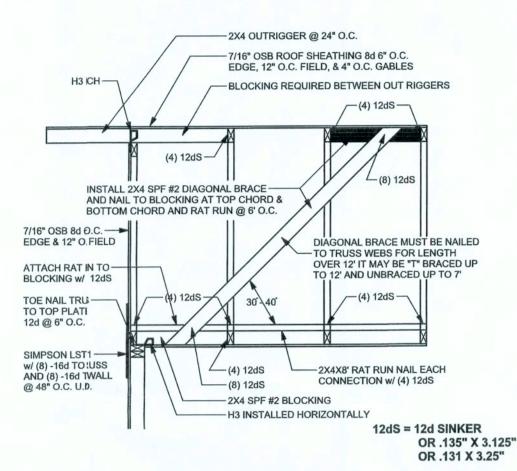
(2) SIMPSON LSTA21-

w/ (8) -16d TO HEADER

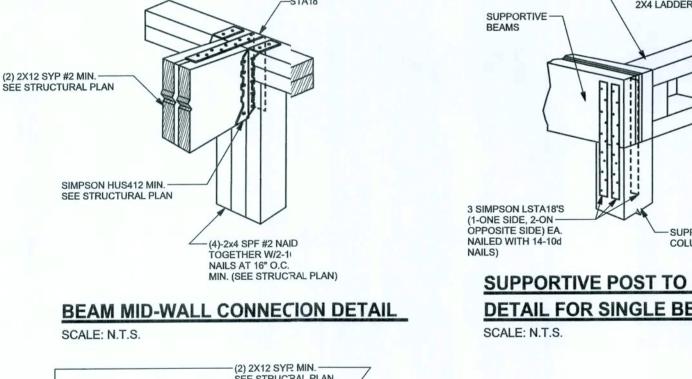
AND (8) -16d TO POST

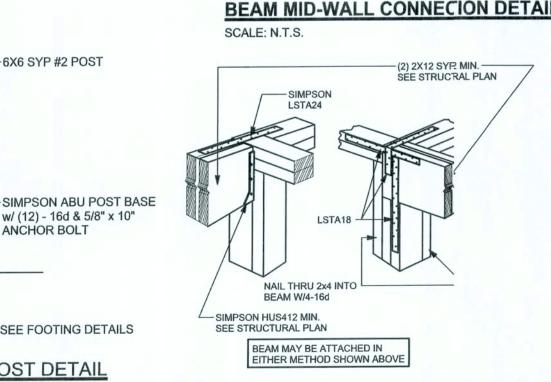


TYPICAL GRAGE DOOR HEADER STRAPING DETAIL SCALE: 1/2" = 1'-0"

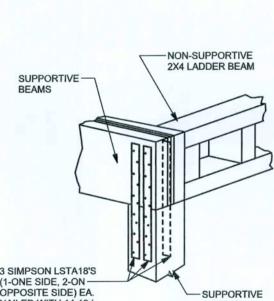


SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED GABLE BRACING DETAIL SCALE: 1/2" = 1'-0"

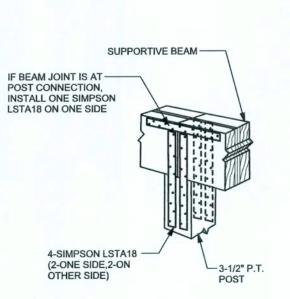




BEAM CORNER CONNECTION. ETAIL SCALE: N.T.S.



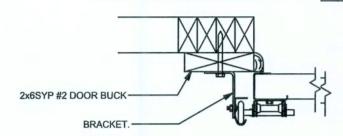
SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM



SUPPORTIVE CENTER POST TO BEAM DETAIL

2x6 SYP #2 GARAGE DOOR BUCK ATTA ACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK ATAT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD, CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/41/4" GN PER TABLE BELOW:

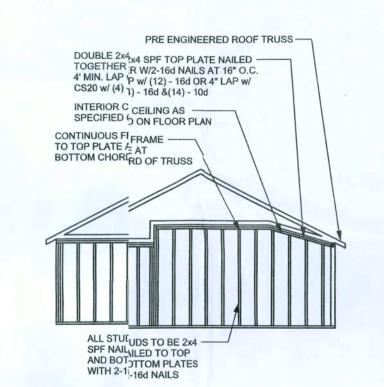
DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .13 (2) ROWS OF .13 (2) ROWS OF
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL

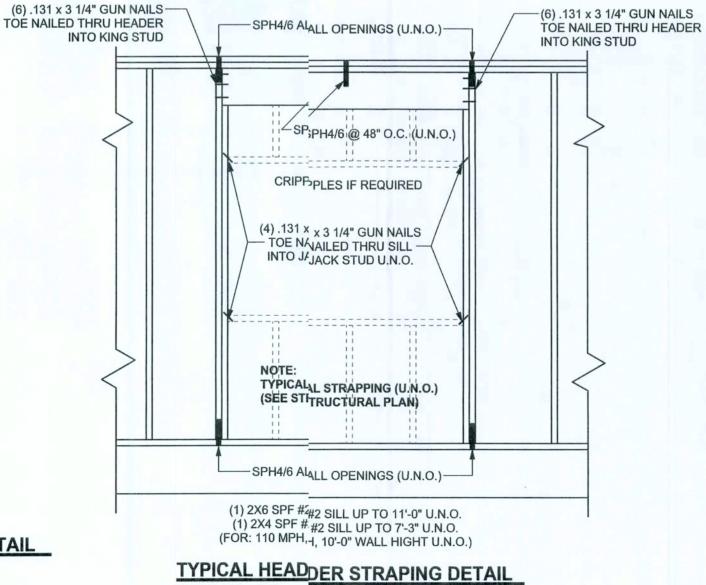
GRRADE & SPECIES TABLE

7.67		Fb (psi)	E (10 ⁶ psi)
2x8 _{k8}	SYP #2	1200	1.6
2x1(10	SYP #2	1050	1.6
2x1212	SYP #2	975	1.6
GLE_B	24F-V3 SP	2400	1.8
LSL _{3L}	TIMBERSTRAND	1700	1.7
LVL/L	MICROLAM	1600	1.9
PSISL	PARALAM	2900	2.0



CONTINUOUS FRAME TO **CEILING DIAPHRAGM DETAIL** SCALE: N.T. T.S.

IF TRUSS TO WALL STRAPS ARE IS NAILED TO THE HEADER THE SPH4/6 @ 4848" O.C. ARE NOT REQUIRED



GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004, TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE RUIL DER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO ELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS, BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" x 0" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACL 302, JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED. APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING. WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FRCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING, THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NO RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

MASONRY NOTES:

ACI530.1-02 Section

CMU standard

Clay brick standard

Reinforcing bars, #3 - #11

2.4F Coating for corrosion protection

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

2.4F Coating for corrosion protection Anchors, sheet metal ties completely

3.3.E.2 Pipes, conduits, and accessories Any not shown on the project drawings

8" block bearing walls F'm = 1500 psi

ASTM C 476, admixtures require approval

ASTM C 90-02, Normal weight, Hollow,

medium surface finish, 8"x8"x16" running

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap

splices min 48 bar dia. (30" for #5)

embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS

Joint reinforcement in walls exposed to

moisture or wire ties, anchors, sheet metal

ties not completely embedded in mortar or

Contractor assumes responsibility for type

and location of movement joints if not

grout, ASTM A153, Class B2, 1.50 oz/ft2

require engineering approval.

detailed on project drawings.

bond and 12"x12" or 16"x16" column

ASTM C 270, Type N, UNO

5.5"x2.75"x11.5"

or 304SS

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR* TO PLATES TO RAFTER/TRUSS

UPLIFT LDS. STP	UPLIFT LBS. SPF	TRUSS CONNECTOR	TOPLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS: MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.) BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

) BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B

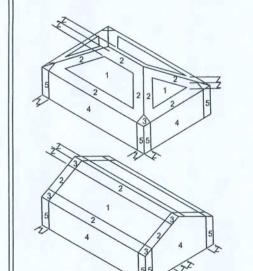
WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II

ROOF ANGLE = 10-45 DEGREES

MEAN ROOF HEIGHT = <30 FT INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

1 19.9 -21.8 18.1 -18.1 2 19.9 -25.5 18.1 -21.8 2 O'hg -40.6 -40.6 3 19.9 -25.5 18.1 -21.8 3 O'hg -68.3 -42.4 4 21.8 -23.6 18.5 -20.4 5 21.8 -29.1 18.5 -22.6 Doors & Windows Windows Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9 16x7 Garage Door 18.5 -21.0			1	10		100
2 O'hg		1	19.9	-21.8	_	_
3 19.9 -25.5 18.1 -21.8 3 O'hg -68.3 -42.4 4 21.8 -23.6 18.5 -20.4 5 21.8 -29.1 18.5 -22.6 Doors & Windows 21.8 -29.1 Worst Case (Zone 5, 10 ft2) 8×7 Garage Door 19.5 -22.9		2	19.9	-25.5	18.1	-21.8
3 O'hg		2 O'hg		-40.6		-40.6
4 21.8 -23.6 18.5 -20.4 5 21.8 -29.1 18.5 -22.6 Doors & Windows Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9		3	19.9	-25.5	18.1	-21.8
5 21.8 -29.1 18.5 -22.6 Doors & Windows 21.8 -29.1 Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9		3 O'hg		-68.3		-42.4
Doors & Windows 21.8 -29.1 Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9		4	21.8	-23.6	18.5	-20.4
Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9		5	21.8	-29.1	18.5	-22.6
		Wor	st Cas	e	21.8	-29.1
16x7 Garage Door 18.5 -21.0		8x7 Gar	age D	oor	19.5	-22.9
		16x7 Ga	arage l	Door	18.5	-21.0
	<u></u>					

Zone Effective Wind Area (ft2)

DESIGN	LOADS		
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)		
	30 PSF (SLEEPING ROOMS)		
	30 PSF (ATTICS WITH STORAGE)		
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)		
ROOF	20 PSF (FLAT OR <4:12)		
	16 PSF (4:12 TO <12:12)		
	12 PSF (12:12 AND GREATER)		
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)		
2000		 	

REVISIONS

SOFTPILAIN

32056, 386-754-5419 DIMENSIONS: dimensions. Refer all destions to Mark Disosway, P.E. fr resolution. Do not proceed withou clarification. COPYRIGHTS AND PROPERTY RIGHTS Mark Disosway, P.E. hreby expressly reserved s common law copyrints and property right in hese instruments of srvice. This document i not to be reproduced, Itered or copied in any orm or manner withou first the express written rmission and conseit of Mark Disosway. CERTIFICATION: I heaby certify that I have amined this plan, and that the applicable portions of the plan, reating to wind engineer comply with section RD1.2.1, florida building ode residential 2004, o the best of my

WINDLOAD ENGINER: Mark Disosway,

PE No.53915, POB 86, Lake City, FL

IMITATION: This design is valid for one uilding, at specified loation.

MARK ESOSWAY P.E.53915

Mike Roberts

SpecHouse Lot 8 Sadde Ridge S/D

> ADIRESS: Lot 8 Sadde Ridge S/D Columbia Cunty, Florida

Mark Disesway P.E. P.O. Fox 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

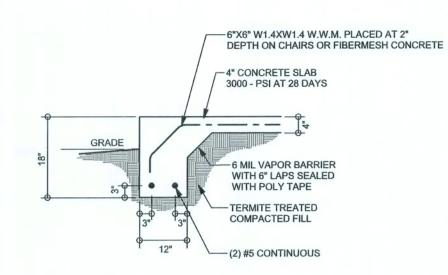
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FINALS DATE:

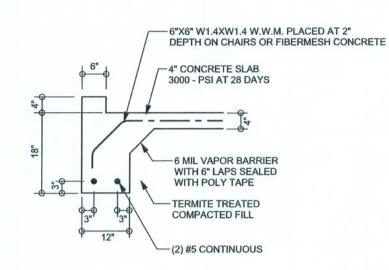
26 / Dec / 07 JOB NUMBER: 712263 DRAWIN3 NUMBER

S-1

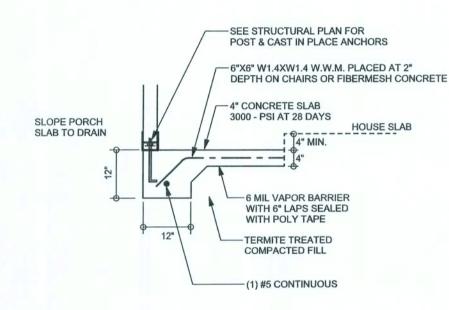
OF 3 SHEETS



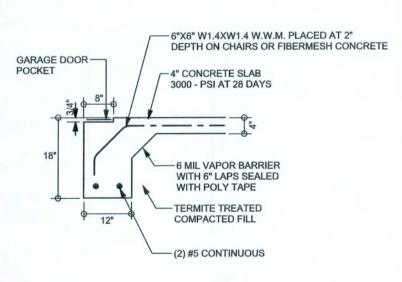




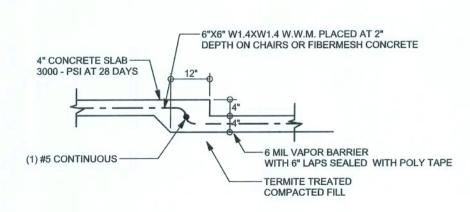
GARAGE CURB FOOTING SCALE: 1/2" = 1'-0"



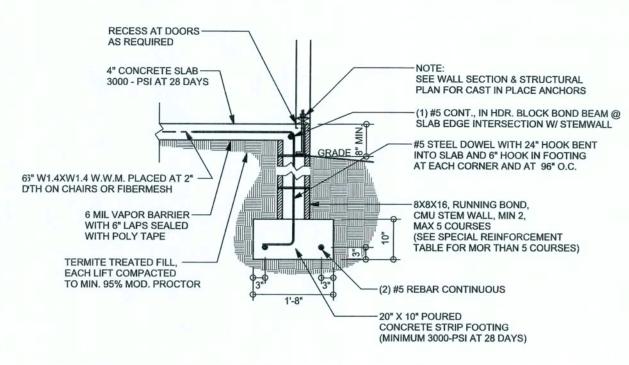
F5 PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



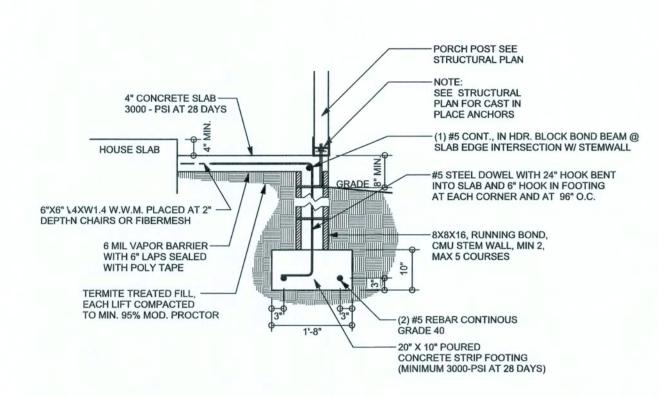
F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"



S-2 SCALE: 1/2" = 1'-0"



OPTIONAL STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"

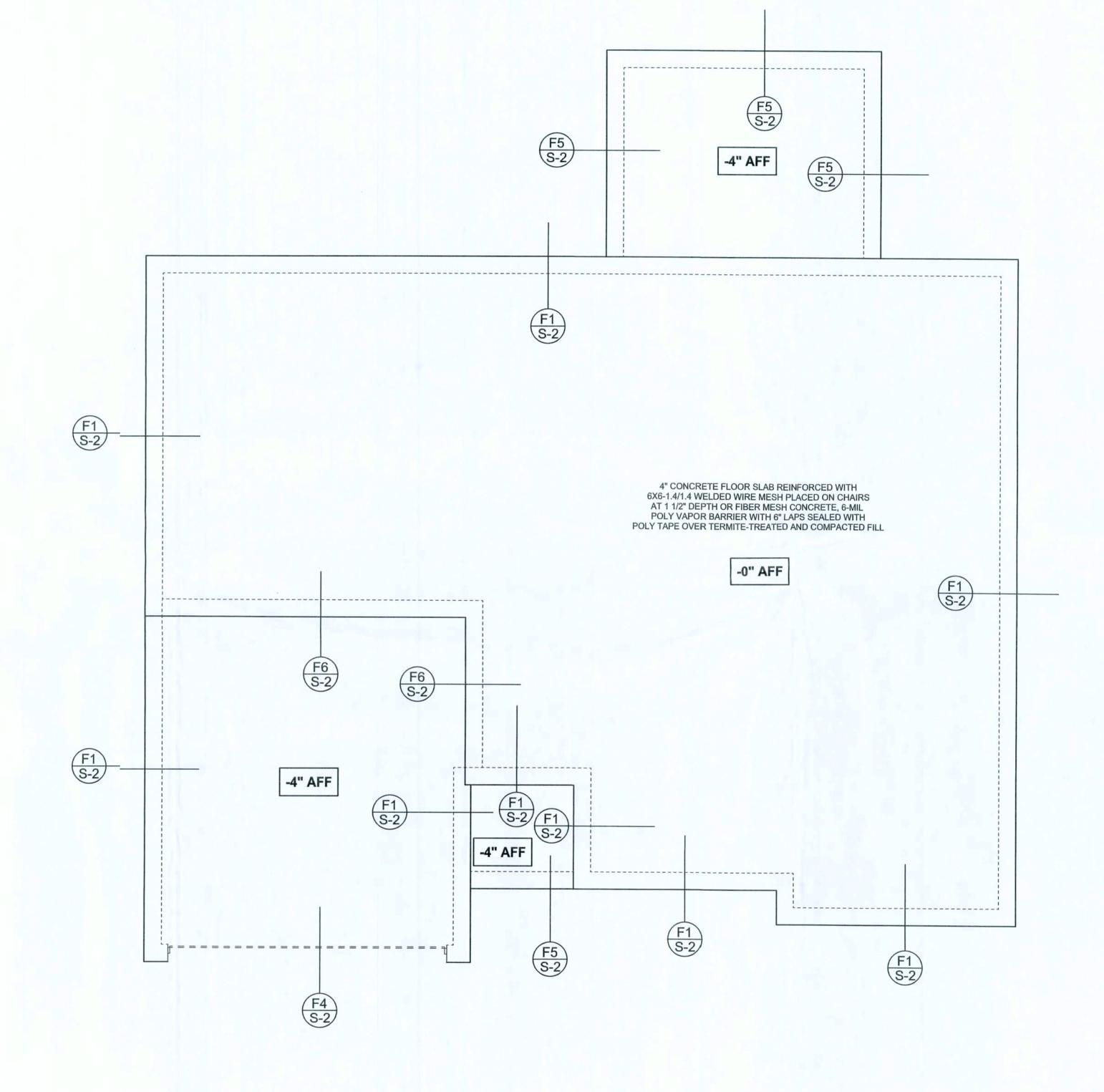


OPTIONAL STEM WALL PORCH FOOTING SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)		BACKFILL FOR 8" CMU STEMWALL FOR 12" CMU STEMWA				MWALL
		#5	#7	#8	#5	#7	#8	
3.3	3.0	96	96	96	96	96	96	
4.0	3.7	96	96	96	96	96	96	
4.7	4.3	88	96	96	96	96	96	
5.3	5.0	56	96	96	96	96	96	
6.0	5.7	40	80	96	80	96	96	
6.7	6.3	32	56	80	56	96	96	
7.3	7.0	24	40	56	40	80	96	
8.0	7.7	16	32	48	32	64	80	
8.7	8.3	8	24	32	24	48	64	
9.3	9.0	8	16	24	16	40	48	



FOUNDATION PLAN

DIMENSIONS ON STRUCTURAL SHEETS

FLOOR PLAN FOR ACTUAL DIMENSIONS

ARE NOT EXACT. REFER TO ARCHITECTURAL

SCALE: 1/4" = 1'-0"

WINDLOAD ENGINER: Mark Disosway, PE No.53915, POB 88, Lake City, FL 32056, 386-754-5419 Stated dimensions suercede scaled dimensions. Refer allquestions to Mark Disosway, P.E. or resolution. Do not proceed without clarification. COPYRIGHTS AND IROPERTY RIGHTS: Mark Disosway, P.E. lereby expressly reserve its common law copyghts and property right in these instruments of ervice. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and conset of Mark Disosway.

REVISIONS

SOFIPILAN

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section F301.2.1, florida building code residential 2004 to the best of my knowledge. LIMITATION: This deign is valid for one building, at specified leation.

MARK JISOSWAY P.E 53915

MikeRoberts

SpecHouse Lot 8 Sadde Ridge S/D

> ADDRESS: Lot 8 Sadde Ridge S/D Columbia County, Florida

Mark Dissway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386 269 - 4871

> PRINTED DATE: December 26, 2007 STRUCTURAL BY:

FINALS DATE:

26 / Dec / 07 JOB NJMBER: 712263

> **S-2** OF 33HEETS

DRAWING NUMBER

TYPICAL NON - BEARING STEP FOOTING

SEE PORCH POST -DETAIL (TYP.) T T18 TRUSS UPLIFT UPLIFT (2) 2X4 SPF #2 STUDS CENTERED UNDER TRUSS 1150 LB — UPLIFT -539 LB UPLIFT SWS = 9.5 CONNECTIONS UNLESS NOTED OTHERWISE UPLIFT UPLIFT II T17 TRUSS (2) 2X4 SPF #2 STUDS CENTERED UNDER TRUSS UPLILIFT -1080 LB 622 LB 670 LB-|| UPLIFT UPLIFT || SWS | 4.0' || (2) 2X10X16',2J 2K + 2X6 SYP #2 SWS = 8.0' SWS = 3.5' FOR 7'-0" JACK TRUSS LOAD MAX UPLIFT SEE GARAGE DOR HEADER SEE GARAGE DOOR HEADER STRAPING DETAIL (TYP.) STRAPING DETA (TYP.) STRUCTRAL PLAN SCALE: 1/4" = '-0" STRUCTRAL PLAN NOTES WALL LEGEND HEADER LEGEND ALLOAD BEARING FRAME WALL & PORCH HEADERS SWS = 0.0SHLL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.) 1ST FLOOR EXTERIOR WAIALL (2) 2X10X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.) ALLOAD BEARING FRAME WALL HEADERS SHLL HAVE (1) JACK STUD & (1) KING STUD NUMBER OF KING STUDS (FULL LENGTH) SWS = 0.0' 2ND FLOOR EXTERIOR WAYALL EAH SIDE (U.N.O.) -NUMBER OF JACK STUDS (UNDER HEADER) -SPAN OF HEADER DIENSIONS ON STRUCTURAL SHEETS ARNOT EXACT. REFER TO ARCHITECTURAL FLOR PLAN FOR ACTUAL DIMENSIONS SIZE OF HEADER MATERIAL 1ST FLOOR INTERIOR BEAIARING WALL ----NUMBER OF PLIES IN HEADER PEMANENT TRUSS BRACING IS TO BE INSTALLED AT 2ND FLOOR INTERIOR BEA-ARING WALL LOATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LA:RAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BC-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARFURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TOTAL SHEAR WALL SEGMENTS TRSS PACKAGE SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

REVISIONS

SOFIPIAN

WINDLOAD ENGINER: Mark Disosway, PE No.53915, POB 68, Lake City, FL

Stated dimensions spercede scaled dimensions. Refer alquestions to Mark Disosway, P.E.for resolution. Do not proceed withat clarification.

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form or manner without first the express written
permission and consent of Mark Disosway.

CERTIFICATION: I breby certify that I have examined this plan, and that the applicable portions of the plan, elating to wind engineering comply with section (301.2.1, florida building code residential 2004 to the best of my knowledge.

LIMITATION: This deign is valid for one building, at specified ocation.

P.I. 53915

MikeRoberts

Spec House Lot 8 Sadde Ridge S/D

> ADDRESS: Lot 8 Sadlle Ridge S/D Columbia County, Florida

Mark Disosway P.E. P.O.3ox 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

> PRIN'ED DATE: Decemier 26, 2007

STRUCTURAL BY

FINALS DATE: 26 / Dec / 07

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING

FURNISHED BY BUILDER. BUILDERS FIRST SOURCE

JOB #L264527

JOB NUMBER: 712263 DRAWING NUMBER

\$-3 OF 3SHEETS

REQUIRED ACTUAL
TRANSVERSE 33.6' 85.5' LONGITUDINAL 28.6'