

HERITAGE

BUILDING SYSTEMS.

May 10, 2023

DOUG MOSLEY
362 SW MCCLINTON DR
FORT WHITE, FL 32038

19-B-33112 (REVISED)
DOUG MOSLEY
FORT WHITE, FL 32038
MULTIPLE BUILDINGS

To Whom It May Concern:

This is to certify that materials for the subject structure have been designed in accordance with the order documents, specifically as shown per the attached Engineering Design Criteria Sheet.

Aspects of code compliance as related to use or occupancy, such as sprinkler requirements, are not addressed by these documents.

These materials, when properly erected on an adequate foundation in accordance with the erection drawings as supplied and using the components as furnished, will meet the attached loading requirements.

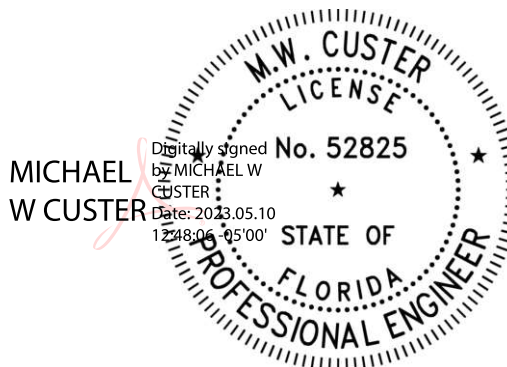
This certification does not cover field modifications, or the design of materials not furnished by Heritage Building Systems.

The attached design criteria and calculations are to remain with and form part of this Letter of Certification. This letter voids and supersedes the previous Letter of Certification, date April 17, 2023.

The calculations and the metal building they represent are the product of Heritage Building Systems or a division of its affiliate Cornerstone Building Brands. The engineer whose seal appears hereon is contracted by either Heritage Building Systems or a division of its affiliate Cornerstone Building Brands and is not the engineer of record for this project.

Cordially,

M.W. Custer, P.E.
642 Oakbend Drive
Coppell, TX 75019



This document has been
digitally signed.



DESIGN PACKAGE

BUILDER: DOUG MOSLEY

CUSTOMER: DOUG MOSLEY

JOB NUMBER: 19-B-33112

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Note to Detailing: Eds2Xds Drawings (Y)

Original Design Completed thru Change Order # 2

Date: 04/11/2023

Revision History

Rev#	Update Reactions?	Reason for Revision	Pages Revised	Date Revised	Eng.
1	Yes	CO #5	17, 19-20, 25	04/25/2023	MJA
2	Yes	Engineering Revision	18, 28	04/28/2023	APAC

Project Engineer: Mario Jimenez-Arias (Fairview)

Checking Engineer: Kal Yeau Chiok, P.E.

Signing Engineer: Mike W. Custer, P.E.

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, or ASTM A1011 with 55 ksi min. yield, except flanges wider than 12" and thicker than 3/8", all flanges thicker than 1", and all webs thicker than 3/8" are 50 ksi min. yield. Rod X-bracing conforms to ASTM A529 or ASTM A572 with 50 ksi min. yield. Cable X-bracing conforms to ASTM A475 7 Strand Extra High-Strength grade. Hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with 50 ksi min. yield. Hot rolled angles, other than flange braces, conform to ASTM A36 minimum. Round and rectangular HSS conforms to ASTM A500 Grade B. Cold-formed steel secondary framing Members conform to ASTM A1011 or ASTM A653 Grade 55 with 55 ksi min. yield. For Canada, material properties conform to CAN/CSA G40.20/G40.21 or equivalent.

Unless otherwise noted, special inspection of fabricated items is not required. Per IBC section 1704.2.5.1, fabricator is approved to perform such work without special inspection through maintenance of IAS AC 472 certification MB-136.

All bolted joints with A325 Type 1 bolts are specified as snug-tightened joints in accordance with the most recent edition of the RCSC Specification for Structural Joints Using ASTM A325 or A490 Bolts. Pre-tensioning methods, including turn-of-nut, calibrated wrench, twist-off-type tension-control bolts or direct-tension-indicator are NOT required. Installation inspection requirements for Snug Tight Bolts (Specification for Structural Joints Section 9.1) is suggested.

Design criteria as noted is as given within order documents and is applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the metal building manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for local provisions that may apply or for site specific parameters. The design criteria is supplied by the builder, project owner, or an Architect and/or Engineer of Record for the overall construction project.

Building A is designed as an Enclosed Building. Exterior and/or operable components including, but not limited to, doors, windows, vents, etc. ("Components") must be designed to withstand the required component and cladding wind pressures specified by the building code. In order to maintain the metal building system's Enclosed Building condition, all Components shall be closed when wind velocities reach half the designed wind load for the metal building system as shown on the drawings and design criteria documentation. Failure to maintain the metal building system's Enclosed Building condition will violate and void all warranties and certifications applicable to the material supplied by the metal building manufacturer.

The framing at building A, gridline 1 and building B, gridline 1 is NOT designed to receive a future bay addition. Corresponding frame reactions are calculated based upon actual tributary area.

Framed openings, walk doors, and open areas shall be located in the bay and elevation as shown in the erection drawings. The cutting or removal of girts shown on the erection drawings due to the addition of framed openings, walk doors, or open areas not shown may void the design certifications supplied by the metal building manufacturer.

Roof and wall panels have been designed in accordance with section 2222.4 of the Florida Building Code, 7th Edition. Product approval numbers for the State of Florida, Department of Community Affairs per Product Rule 9B-72:

1. Panel Walls

FL11917 PBR 26 gauge walls

2. Roofing Products

FL11819 PBR 26 gauge roofs

3. Walk doors

FL17900.1 Telstar 3070, Wind-rated to +/- 50 psf, Impact-rate

Job Number: 19-B-33112
 Builder: DOUG MOSLEY
 Jobsite Location: DOUG MOSLEY, FORT WHITE, FL

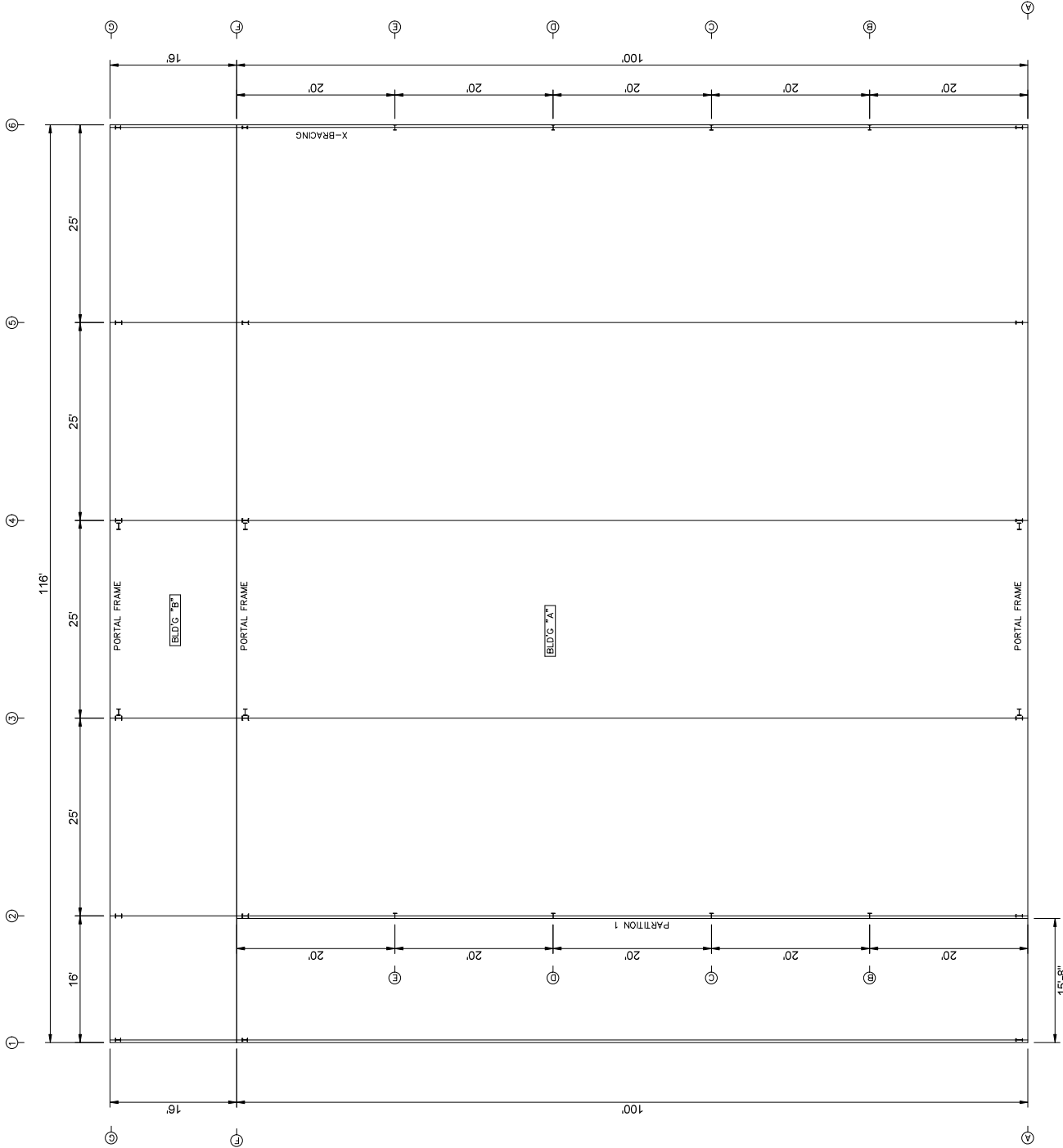
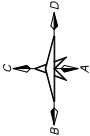
The material supplied by the manufacturer has been designed with the following minimum deflection criteria. The actual deflection may be less depending on actual load and actual member length.

BUILDING DEFLECTION LIMITS: BLDG-A / BLDG-B

Roof Limits	Rafters	Purlins	Panels
-----	-----	-----	-----
Live: L/	180	180	60
Serviceability Wind: L/	180	180	60
Total Gravity: L/	120	120	60
Total Uplift: L/	N/A	N/A	60
Frame Limits	Sidesway	Portal Frame	Sidesway
-----	-----	-----	-----
Live: H/	60		
Serviceability Wind: H/	60		
Portal Serviceability Wind: H/	N/A	60	
Total Gravity: H/	60		
Wall Limits	Limit		
-----	-----		
Total Wind Panels: L/	60		
Total Wind Girts: L/	90		
Total Wind EW Columns: L/	120		

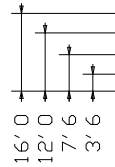
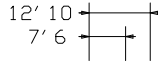
PROJECT LAYOUT

19-B-33112

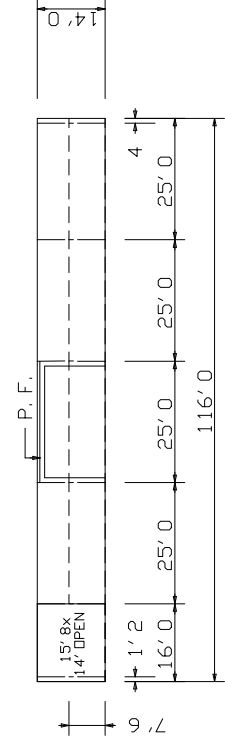
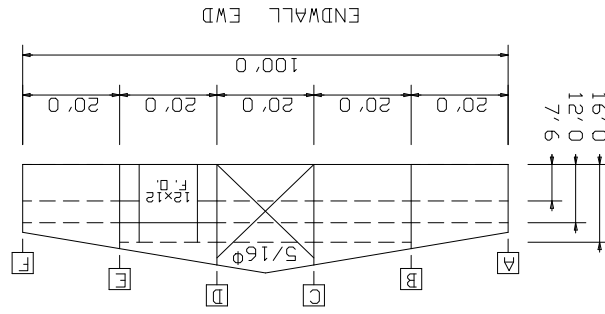
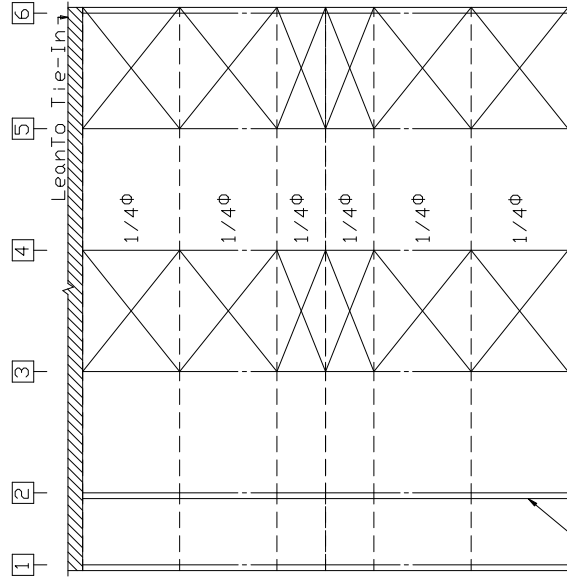
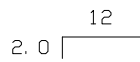


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Builder : DOUG MOSLEY
Job No: 33112A run01
Version: ver01-mjmenez
Tue Apr 11 15:17:18 2023
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SIDEWALL SWC



ENDWALL EWB



Owner :

Heritage

Design Summary Program

Design Summary Report

User: mjimenez

Job Number: 33112A

Version: 8.20.1

run01

Date: 04/11/23

Start Time: 15:17:00

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B U I L D I N G - A - D E S I G N S U M M A R Y R E P O R T

All connections use ASTM F3125 Gr. A325N bolts, unless noted otherwise.

All anchor rods are checked according to ASTM F1554 Gr. 36 strengths.

Roof Plane ----- RPA

R:\jobs\Active\Eng\19-B-33112\ver01-mjimenez\BLDG-A\run01\AroofRPA_01.edf

Panel PBR
Panel width 36 in.
Panel gage 26 ga.
Purlins 55.0 ksi yield strength
Eave struts 55.0 ksi yield strength

Horz. Purlin Spacing: 8@5'0 2@4.3989 1.2022

Bay #	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	16.000	8X2.5Z16	None	S 0.000	1.479 C
2	25.000	8X2.5Z16	None	C 1.479	2.479 C
3	25.000	8X2.5Z16	None	C 2.479	1.479 C
4	25.000	8X2.5Z16	None	C 1.479	3.146 C
5	25.000	8X2.5Z16	None	C 3.146	0.000 S

Area with Sub-Framing					
Bay #	Width (ft-in)	Start (ft)	Length (ft)	Number and size of edge purlin between main purlins	
1	16'0	0.00	48.80	1	See note <2>.
5	9'10	0.00	48.80	1	8X2.5Z16

Note(s):

- 1) Area with sub-framing distances are measured from sidewall steel line and are additive
- 2) Edge purlins to be the same gage as main purlins.
- 3) Sub-framing support is L2.5x2.5x3/16 angle on right.

Purlin Clip Use 2 A325 Bolts @ Level 2,3,4,5,6,7,8,9,10,11 @ Supports: 1,2,3,4,5,6

Purlin Stiffened Clips @ Level 2,6,11 @ Supports: 1,2,3,4,5,6

Purlin Backup Plate @ Level 2,6,11 @ Supports: 1,2,3,4,5,6

Heritage

Design Summary Program

Design Summary Report

User: mjimenez

Job Number: 33112A

Version: 8.20.1

run01

Date: 04/11/23

Start Time: 15:17:01

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Roof Plane ----- RPC

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Panel PBR
Panel width 36 in.
Panel gage 26 ga.
Purlins 55.0 ksi yield strength
Eave struts 55.0 ksi yield strength

Horz. Purlin Spacing: 8@5'0 2@4.3989 1.2022

Bay #	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	25.000	8X2.5Z16	None	S 0.000	3.146 C
2	25.000	8X2.5Z16	None	C 3.146	1.479 C
3	25.000	8X2.5Z16	None	C 1.479	2.479 C
4	25.000	8X2.5Z16	None	C 2.479	1.479 C
5	16.000	8X2.5Z16	None	C 1.479	0.000 S

Bay #	Area with Sub-Framing Width (ft-in)	Start (ft)	Length (ft)	Number and size of edge purlin between main purlins
1	9'10	0.00	48.80	1 8X2.5Z16
5	16'0	0.00	48.80	1 See note <2>.

Note(s):

- 1) Area with sub-framing distances are measured from sidewall steel line and are additive
- 2) Edge purlins to be the same gage as main purlins.
- 3) Sub-framing support is L2.5x2.5x3/16 angle on left.

Purlin Clip Use 2 A325 Bolts @ Level 2,3,4,5,6,7,8,9,10,11 @ Supports: 6,5,4,3,2,1

Purlin Stiffened Clips @ Level 2,6,11 @ Supports: 6,5,4,3,2,1

Purlin Backup Plate @ Level 2,6,11 @ Supports: 6,5,4,3,2,1

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Design Summary Program
Design Summary Report

User: mjimenez Job Number: 33112A
Version: 8.20.1 run01 Date: 04/11/23
Start Time: 15:17:01

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RPA  Purlin Strut @ 60.000 (ft) :8X2.5Z16 Bays 1
RPA  Purlin Strut @ 80.000 (ft) :8X2.5Z16 Bays 1
RPA  Purlin Strut @ 60.000 (ft) :8X2.5Z16 Bays 2
RPA  Purlin Strut @ 80.000 (ft) :8X2.5Z16 Bays 2
RPA  Purlin Strut @ 60.000 (ft) :8X2.5Z16 Bays 3
RPA  Purlin Strut @ 80.000 (ft) :8X2.5Z16 Bays 3
RPA  Purlin Strut @ 60.000 (ft) :8X2.5Z16 Bays 4
RPA  Purlin Strut @ 80.000 (ft) :8X2.5Z16 Bays 4
RPA  Purlin Strut @ 60.000 (ft) :8X2.5Z13 Bays 5
RPA  Purlin Strut @ 80.000 (ft) :8X2.5Z12 Bays 5
RPC  Purlin Strut @ 20.000 (ft) :8X2.5Z12 Bays 1
RPC  Purlin Strut @ 40.000 (ft) :8X2.5Z13 Bays 1
RPC  Purlin Strut @ 50.000 (ft) :8X2.5Z16 Bays 1
RPC  Purlin Strut @ 20.000 (ft) :8X2.5Z16 Bays 2
RPC  Purlin Strut @ 40.000 (ft) :8X2.5Z16 Bays 2
RPC  Purlin Strut @ 50.000 (ft) :8X2.5Z16 Bays 2
RPC  Purlin Strut @ 20.000 (ft) :8X2.5Z16 Bays 3
RPC  Purlin Strut @ 40.000 (ft) :8X2.5Z16 Bays 3
RPC  Purlin Strut @ 50.000 (ft) :8X2.5Z16 Bays 3
RPC  Purlin Strut @ 20.000 (ft) :8X2.5Z16 Bays 4
RPC  Purlin Strut @ 40.000 (ft) :8X2.5Z16 Bays 4
RPC  Purlin Strut @ 50.000 (ft) :8X2.5Z16 Bays 4
RPC  Purlin Strut @ 20.000 (ft) :8X2.5Z16 Bays 5
RPC  Purlin Strut @ 40.000 (ft) :8X2.5Z16 Bays 5
RPC  Purlin Strut @ 50.000 (ft) :8X2.5Z16 Bays 5
SWA  Eave Strut @ 14.000 (ft) :8X3.5E14 Bays 1
SWA  Eave Strut @ 14.000 (ft) :8X3.5E14 Bays 2
SWA  Eave Strut @ 14.000 (ft) :8X3.5E14 Bays 3
SWA  Eave Strut @ 14.000 (ft) :8X3.5E14 Bays 4
SWA  Eave Strut @ 14.000 (ft) :8X3.5E14 Bays 5
SWC  Eave Strut @ 14.000 (ft) :8X3.5E13 Bays 1
SWC  Eave Strut @ 14.000 (ft) :8X3.5E14 Bays 2
SWC  Eave Strut @ 14.000 (ft) :8X3.5E14 Bays 3
SWC  Eave Strut @ 14.000 (ft) :8X3.5E14 Bays 4
SWC  Eave Strut @ 14.000 (ft) :8X3.5E14 Bays 5

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Note: 1) All purlin strut locations for all roof planes are measured from back
sidewall

2) All purlin strut rows use the same lap lengths as the main purlin design.
Eave strut interior connection at SWA uses (2)-1/2" A325 bolts.
Eave strut interior connection at SWC uses (2)-1/2" A325 bolts.
Eave strut connection at end-frame uses (4)-1/2" A325 bolts.

Bracing ---- Roof: 2 bays Cable
Plane SWA :Portal Frame
Plane SWC :Portal Frame
Plane EWB :End Frame
Plane EWD : 1 bays Cable

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Design Summary Report

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run01

Date: 04/11/23

Start Time: 15:17:01

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Sidewall Plane SWA -- (8.250" Inset columns)

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Panel PBR
Panel width 36 in.
Panel gage 26 ga.
Girts 55.0 ksi yield strength

Girts Spacing: 7'6

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
2	7'6	25.000	8X2.5Z12	None	S 0.000	2.479 C
3	7'6	25.000	8X2.5Z16	None	C 2.479	1.479 C
4	7'6	25.000	8X2.5Z16	None	C 1.479	2.479 C
5	7'6	25.000	8X2.5Z13	None	C 2.479	0.000 S

Open Areas: OPEN TO REMAIN

Size	Wall	Distance
15'8 x 14'0	SWA	0'0

Sidewall Plane SWC -- (8.250" Inset columns)

R:\jobs\Active\Eng\19-B-33112\ver01-mjimenez\BLDG-A\run01\AwallSWC_01.edf

Panel PBR
Panel width 36 in.
Panel gage 26 ga.
Girts 55.0 ksi yield strength

Girts Spacing: 7'6 5'4

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	7'6	25.000	8X2.5Z16	F.O.	S 0.000	0.000 S
2	7'6	25.000	8X2.5Z12	None	S 0.000	2.479 C
3	7'6	25.000	8X2.5Z16	None	C 2.479	2.479 C
4	7'6	25.000	8X2.5Z12	None	C 2.479	0.000 S
1	12'10	25.000	8X3.5Z12	None	S 0.000	0.000 S

Note : Maximum distance to extend girt from adjacent bay is 36.00 inches.

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Design Summary Program

Design Summary Report

User: mjimenez

Job Number: 33112A

Version: 8.20.1

run01

Date: 04/11/23

Start Time: 15:17:01

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Sidewall C continued.

Framed Openings:

Width	Height	Sill Ht	Jamb	Header/Sill	Bay Distance
12'0	12'0	N/A	8X2.5C16	8X2.5C16	1 8'0
3'0	4'0	2'8	8X2.5C16	8X2.5C16	1 2'6
3'0	4'0	2'8	8X2.5C16	8X2.5C16	2 11'0

Open Areas: OPEN TO REMAIN

Size	Wall	Distance
15'8 x 14'0	SWC	100'4

Endwall Plane EWB DesignNON-EXPANDABLE FRAME

R:\jobs\Active\Eng\19-B-33112\ver01-mjimenez\BLDG-A\run01\AwallEWB_01.edf

Panel None

Open Areas: OPEN TO REMAIN

Size	Wall	Distance
100'0 x 22'4	EWB	0'0

Endwall Plane EWD DesignBEARING FRAME

R:\jobs\Active\Eng\19-B-33112\ver01-mjimenez\BLDG-A\run01\AwallEWD_01.edf

Panel PBR
Panel width 36 in.
Panel gage 26 ga.

Rafters -----

Mem #	Description	Length (ft)	Start (ft)	End (ft)
1	W8X10 50.0 ksi	42.389	0.000	42.389
	Connections... Left : Type-IV SEP 6.000" X 3/8" (2)-1/2" A325N Bolts			
	Right: Type-II MEP 6.000" X 3/8" (4)-1/2" A325N Bolts			
2	W8X10 50.0 ksi	7.097	42.389	49.486
	Connections... Left : Type-II MEP 6.000" X 3/8" (4)-1/2" A325N Bolts			
	Right: Type-II MEP 6.000" X 3/8" (4)-1/2" A325N Bolts			
3	W8X10 50.0 ksi	7.097	49.486	56.582
	Connections... Left : Type-II MEP 6.000" X 3/8" (4)-1/2" A325N Bolts			
	Right: Type-II MEP 6.000" X 3/8" (4)-1/2" A325N Bolts			
4	W8X10 50.0 ksi	42.389	56.582	98.972
	Connections... Left : Type-II MEP 6.000" X 3/8" (4)-1/2" A325N Bolts			
	Right: Type-IV SEP 6.000" X 3/8" (2)-1/2" A325N Bolts			

Flange Braces at following purlins (horizontal distance from eave) :

PLANE SWA:	44.399	Flange Brace Type "C"
PLANE SWC:	44.399	Flange Brace Type "C"

Heritage

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Job Number: 33112A

Version: 8.20.1

run01

Date: 04/11/23

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Endwall D continued.

Girts 55.0 ksi yield strength

Girts Spacing: 7'6 4'6 4'0

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	7'6	19.312	8X2.5Z13	None	S 0.000	0.000 S
2	7'6	20.000	8X2.5Z13	None	S 0.000	0.000 S
3	7'6	20.000	8X2.5Z13	None	S 0.000	0.000 S
4	7'6	20.000	8X2.5Z16	F.O.	S 0.000	0.000 S
5	7'6	19.312	8X2.5Z13	None	S 0.000	0.000 S
1	12'0	19.312	8X2.5Z14	None	S 0.000	0.000 S
2	12'0	20.000	8X2.5Z14	None	S 0.000	0.000 S
3	12'0	20.000	8X2.5Z14	None	S 0.000	0.000 S
4	12'0	20.000	8X2.5Z16	F.O.	S 0.000	0.000 S
5	12'0	19.312	8X2.5Z14	None	S 0.000	0.000 S
2	16'0	20.000	8X2.5Z14	None	S 0.000	0.000 S
3	16'0	20.000	8X2.5Z14	None	S 0.000	0.000 S
4	16'0	20.000	8X2.5Z12	None	S 0.000	0.000 S

Framed Openings:

Width	Height	Sill Ht	Jamb	Header/Sill	Bay Distance
12'0	12'0	N/A	8X2.5C16	8X2.5C16	4 4'0

Columns ----- (0.000" Inset columns)

Col #	Dist. from left	Description Member Size	Ident.	Base Elev (ft)	Base plate design information Thickness & rods
6-A	0.000'	W8X10	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
6-B	20.000'	W8X10	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
6-C	40.000'	W8X10	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
6-D	60.000'	W8X10	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
6-E	80.000'	W8X10	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
6-F	100.000'	W8X18	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36

Heritage

Design Summary Program

Design Summary Report

User: mjimenez

Job Number: 33112A

Version: 8.20.1

run01

Date: 04/11/23

Start Time: 15:17:01

R:\..\19-B-33112\ver01-mjimenez\BLDG-A\run01\33112A_bldg_A_01.cds

Endwall D continued.

Endwall Column to Bridge Channel Connections:

Col. No.	Strut-to-Column Clip Endwall Plane 6

Plane SWA:	
6-B	Aligns with Purlin, TYPE 3 CONN., (4)-1/2" A325N PC75 (0.375") w/ (4)-1/2" A325N bolts W8x10 Column Extension w/ 12.000" lap length No Bridge Channel
6-C	Aligns with Purlin, TYPE 3 CONN., (4)-1/2" A325N PC75 (0.375") w/ (4)-1/2" A325N bolts W8x10 Column Extension w/ 12.000" lap length No Bridge Channel
6-D	Aligns with Purlin, TYPE 3 CONN., (4)-1/2" A325N PC75 (0.375") w/ (4)-1/2" A325N bolts W8x10 Column Extension w/ 12.000" lap length No Bridge Channel
6-E	Aligns with Purlin, TYPE 3 CONN., (4)-1/2" A325N PC75 (0.375") w/ (4)-1/2" A325N bolts W8x10 Column Extension w/ 12.000" lap length No Bridge Channel

Plane SWC:

Frames	-----	Type	Span	Live	Wind	Eave	Trib	Grid Labels
		CS	100.000	20.00/120.00		14.00/	25.00	3-5
		CS	100.000	20.00/120.00		14.00/	19.92	2
		CS	100.000	20.00/120.00		14.00/	8.58	1

Note: Use square anchor rod layout.

Heritage

Design Summary Program

Design Summary Report

User: mjimenez

Job Number: 33112A

Version: 8.20.1

run01

Date: 04/11/23

Start Time: 15:17:01

R:\..\19-B-33112\ver01-mjimenez\BLDG-A\run01\33112A_bldg_B_01.cds

LEA N T O BLDG - B O N S I D E S W C D E S I G N S U M M A R Y R E P O R T

All connections use ASTM F3125 Gr. A325N bolts, unless noted otherwise.

All anchor rods are checked according to ASTM F1554 Gr. 36 strengths.

Roof Plane ----- RPC

R:\jobs\Active\Eng\19-B-33112\ver01-mjimenez\BLDG-A\run01\BroofRPC_01.edf

Panel PBR
Panel width 36 in.
Panel gage 26 ga.
Purlins 55.0 ksi yield strength
Eave struts 55.0 ksi yield strength

Horz. Purlin Spacing: 2@3'2 2@3'10 2'0

Bay #	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	25.000	8X2.5Z16	None	S 0.000	3.146 C
2	25.000	8X2.5Z16	None	C 3.146	1.479 C
3	25.000	8X2.5Z16	None	C 1.479	1.479 C
4	25.000	8X2.5Z16	None	C 1.479	1.479 C
5	16.000	8X2.5Z16	None	C 1.479	0.000 S

Purlin Clip Use 2 A325 Bolts @ Level 2,3,4,5 @ Supports: 6,5,4,3,2,1

Purlin Stiffened Clips @ Level 2,5 @ Supports: 6,5,4,3,2,1

Purlin Backup Plate @ Level 2,5 @ Supports: 6,5,4,3,2,1

SWC	Eave Strut	@	11.333 (ft)	:8X3.5E14	Bays	1
SWC	Eave Strut	@	11.333 (ft)	:8X3.5E14	Bays	2
SWC	Eave Strut	@	11.333 (ft)	:8X3.5E14	Bays	3
SWC	Eave Strut	@	11.333 (ft)	:8X3.5E14	Bays	4
SWC	Eave Strut	@	11.333 (ft)	:8X3.5E14	Bays	5

Note: 1) All purlin strut locations for all roof planes are measured from back
sidewall

2) All purlin strut rows use the same lap lengths as the main purlin design.
Eave strut interior connection at SWC uses (2)-1/2" A325 bolts.
Eave strut connection at end-frame uses (4)-1/2" A325 bolts.

Bracing ---- Roof: 2 bays Cable

Plane SWC :Portal Frame

Plane EWB :End Frame

Plane EWD :Transfer to main building

Heritage

Design Summary Program

User: mjimenez

Job Number: 33112A

Design Summary Report

Version: 8.20.1

run01

Date: 04/11/23

Start Time: 15:17:02

R:\..\19-B-33112\ver01-mjimenez\BLDG-A\run01\33112A_bldg_B_01.cds

Sidewall Plane SWC -- (8.250" Inset columns)

R:\jobs\Active\Eng\19-B-33112\ver01-mjimenez\BLDG-A\run01\BwallSWC_01.edf

Panel None

Open Areas: OPEN TO REMAIN

Size	Wall	Distance
116'0 x 11'4	SWC	0'0

Endwall Plane EWB DesignNON-EXPANDABLE FRAME

R:\jobs\Active\Eng\19-B-33112\ver01-mjimenez\BLDG-A\run01\BwallEWB_01.edf

Panel None

Open Areas: OPEN TO REMAIN

Size	Wall	Distance
16'0 x 14'0	EWB	0'0

Endwall Plane EWD DesignBEARING FRAME

R:\jobs\Active\Eng\19-B-33112\ver01-mjimenez\BLDG-A\run01\BwallEWD_01.edf

Panel PBR
Panel width 36 in.
Panel gage 26 ga.

Rafters ----

Mem #	Description	Length (ft)	Start (ft)	End (ft)
1	W8X10 50.0 ksi	15.017	0.000	15.017
Connections... Left : Type-IV SEP 6.000" X 3/8" (2)-1/2" A325N Bolts				
Right: Type-IV SEP 6.000" X 3/8" (2)-1/2" A325N Bolts				

Flange Braces at following purlins (horizontal distance from eave) :
PLANE SWC: 6.333 Flange Brace Type "C"

Girts Spacing:

Open Areas: OPEN TO REMAIN

Size	Wall	Distance
16'0 x 14'0	EWD	0'0

Heritage

Design Summary Program

Design Summary Report

User: mjimenez

Job Number: 33112A

Version: 8.20.1

run01

Date: 04/11/23

Start Time: 15:17:02

R:\..\19-B-33112\ver01-mjimenez\BLDG-A\run01\33112A_bldg_B_01.cds

Endwall D continued.

Columns ----- (0.000" Inset columns)

Col #	Dist. from left	Description Member Size	Ident.	Base Elev (ft)	Base plate design information Thickness & rods
6-G	16.000'	W8X10	50.0 ksi	Δ -0.2083'	0.375" BP thk w/(4)-0.625" A36

Frames	Type	Span	Live	Wind	Eave	Trib	Grid Labels
	LT	16.000	20.00/120.00		14.00/	25.00	3-5
	LT	16.000	20.00/120.00		14.00/	19.92	2
	LT	16.000	20.00/120.00		14.00/	8.58	1

Note: Use square anchor rod layout.

Endwall Plane Endwall Partition 1 Design

R:\jobs\Active\Eng\19-B-33112\ver02-mjimenez\BLDG-A\run01\AwallEP1_01.edf

Panel PBR
 Panel width 36 in.
 Panel gage 26 ga.
 Girts 55.0 ksi yield strength

Girts Spacing: 3'6 4'0 4'6 4'0

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	3'6	19.312	8X2.5Z16	None	S 0.000	0.000 S
2	3'6	20.000	8X2.5C16*	None	S 0.000	0.000 S
3	3'6	20.000	8X2.5Z16	None	S 0.000	0.000 S
4	3'6	20.000	8X2.5C16*	None	S 0.000	0.000 S
5	3'6	19.312	8X2.5Z16	None	S 0.000	0.000 S

*Channels to be toed down

1	7'6	19.312	8X2.5Z14	None	S 0.000	0.000 S
2	7'6	20.000	8X2.5Z14	None	S 0.000	0.000 S
3	7'6	20.000	8X2.5Z13	None	S 0.000	0.000 S
4	7'6	20.000	8X2.5Z14	None	S 0.000	0.000 S
5	7'6	19.312	8X2.5Z14	None	S 0.000	0.000 S
1	12'0	19.312	8X2.5Z14	None	S 0.000	0.000 S
2	12'0	20.000	8X2.5Z14	None	S 0.000	0.000 S
3	12'0	20.000	8X2.5Z14	None	S 0.000	0.000 S
4	12'0	20.000	8X2.5Z14	None	S 0.000	0.000 S
5	12'0	19.312	8X2.5Z14	None	S 0.000	0.000 S
2	16'0	20.000	8X2.5Z14	None	S 0.000	0.000 S
3	16'0	20.000	8X2.5Z14	None	S 0.000	0.000 S
4	16'0	20.000	8X2.5Z14	None	S 0.000	0.000 S

Heritage

Design Summary Program

Design Summary Report

User: mjimenez

Job Number: 33112A

Version: 8.20.1

run01

Date: 04/11/23

Start Time: 15:06:04

R:\...\19-B-33112\ver02-mjimenez\BLDG-A\run01\33112A_bldg_A_01.cds

Endwall Partition 1 continued.

Framed Openings:

Width	Height	Sill Ht	Jamb	Header/Sill	Bay Distance
6'0	7'0	N/A	8X2.5C16	Coverplate	3 7'0
5'0	3'0	3'8	8X2.5C16	8X2.5C16	2 7'6
5'0	3'0	3'8	8X2.5C16	8X2.5C16	4 7'6

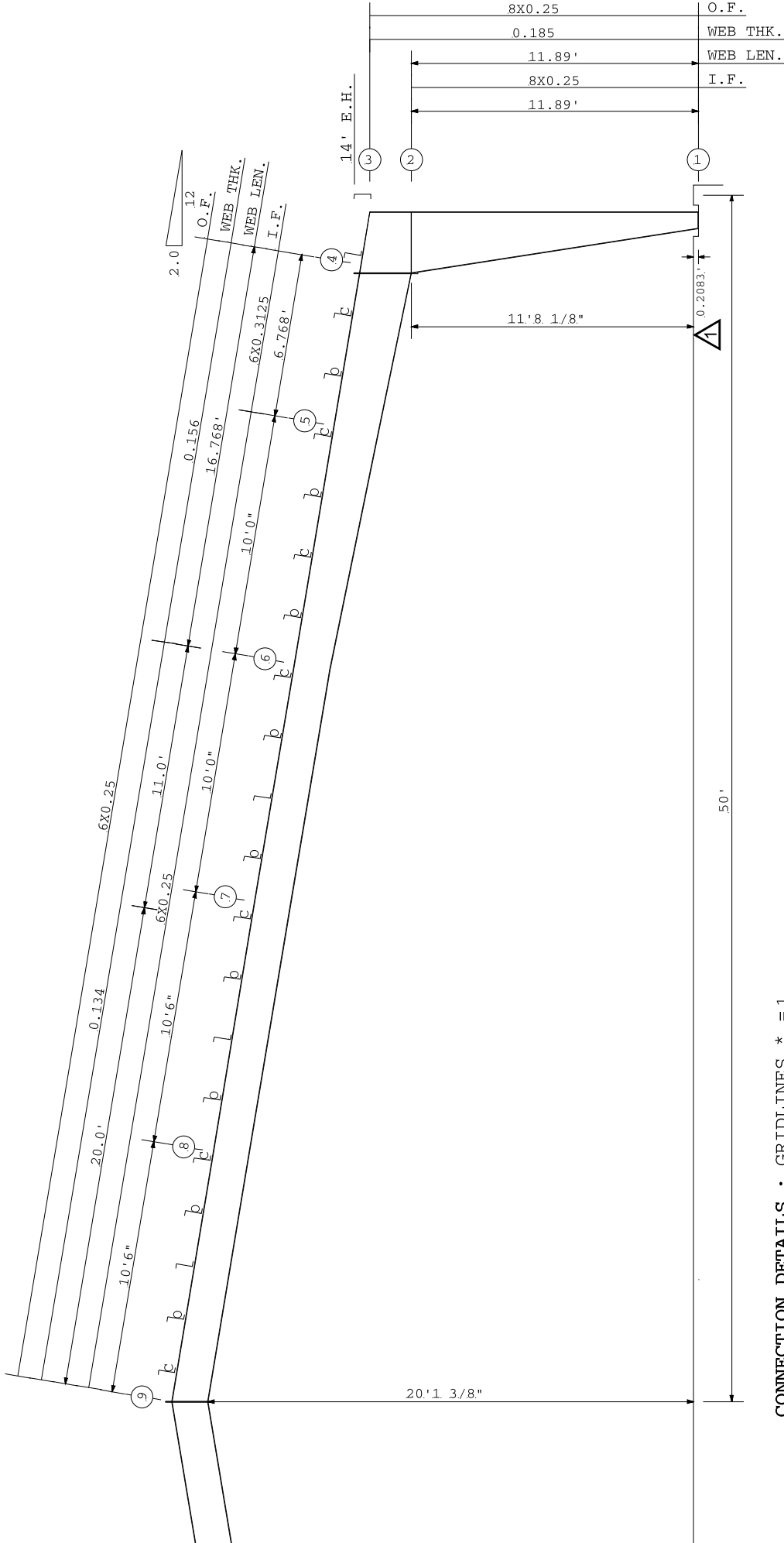
Columns ----- (0.000" Inset columns)

Col #	Dist. from left	Description Member Size Ident.	Base Elev (ft)	Base plate design information Thickness & rods
2-E	20.000'	W8X10 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
2-D	40.000'	W8X10 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
2-C	60.000'	W8X10 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
2-B	80.000'	W8X10 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36

Endwall Column to Bridge Channel Connections:

Col. No.	Strut-to-Column Clip
-----	-----
Plane SWC:	
2-E	Aligns with Purlin, TYPE 3 CONN., (4)-1/2" A325N PC75 (0.375") w/ (4)-1/2" A325N bolts W8x10 Column Extension w/ 16.000" lap length No Bridge Channel
2-D	Aligns with Purlin, TYPE 3 CONN., (4)-1/2" A325N PC75 (0.375") w/ (4)-1/2" A325N bolts W8x10 Column Extension w/ 16.000" lap length No Bridge Channel
2-C	Aligns with Purlin, TYPE 3 CONN., (4)-1/2" A325N PC75 (0.375") w/ (4)-1/2" A325N bolts W8x10 Column Extension w/ 16.000" lap length No Bridge Channel
2-B	Aligns with Purlin, TYPE 3 CONN., (4)-1/2" A325N PC75 (0.375") w/ (4)-1/2" A325N bolts W8x10 Column Extension w/ 16.000" lap length No Bridge Channel
Plane SWA:	

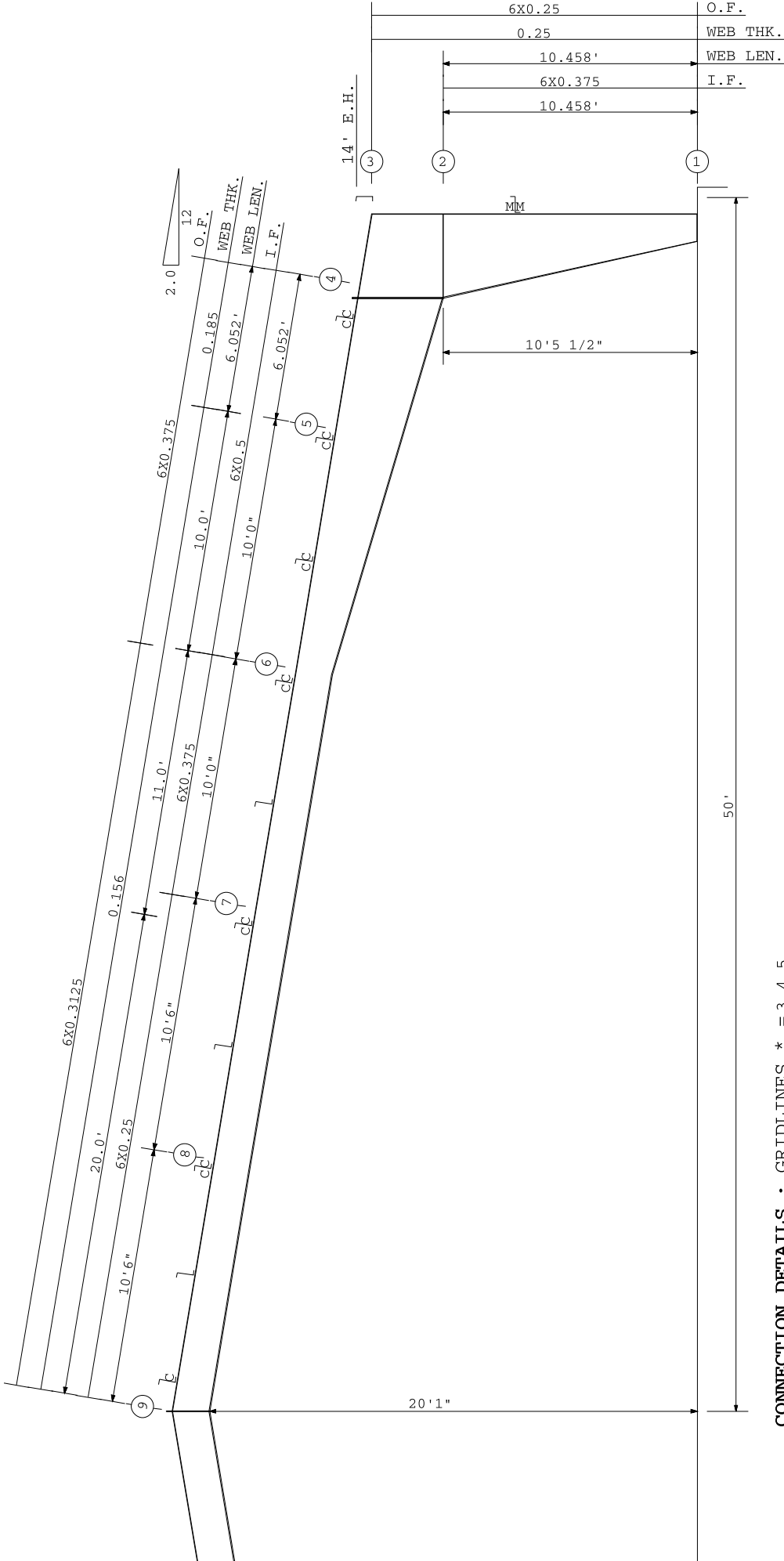
Member sizes and connection details same as left side



CONNECTION DETAILS : GRIDLINES * = 1

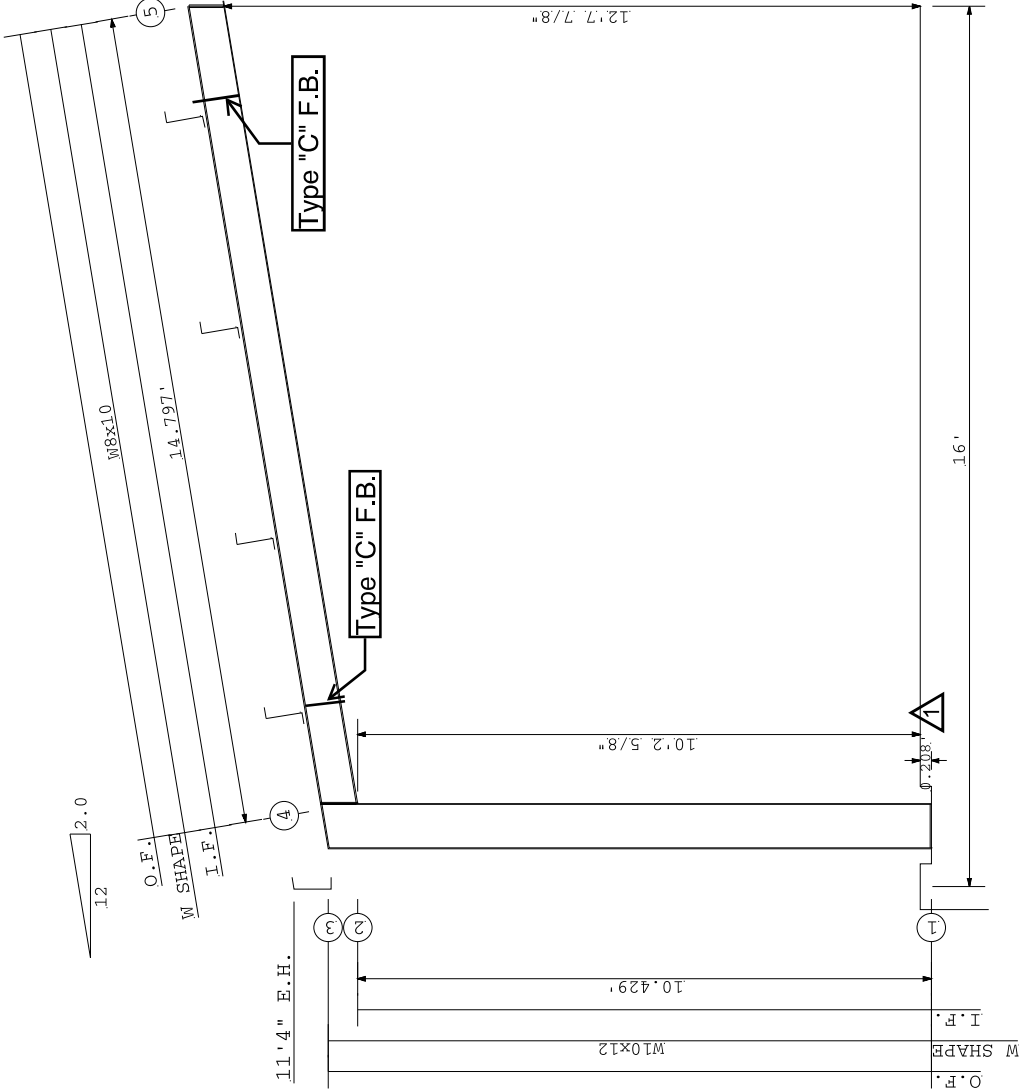
Location	①*-A	②	③	④	⑤	⑥	⑦	⑧	⑨
Web Dep.	8.0	30.0	N/A	25.0	21.9729	17.5	17.5	17.5	17.5
Type	BASE	HORZ STF	CAP (EXT)	3E/2E	SPLICE	SPLICE	SPLICE	SPLICE	2E/2E
Plate (DN)	8.0X0.375	3.75X0.3125	8.0X0.25	8.0X0.5	N/A	N/A	N/A	N/A	6.0X0.375
Plate (UP)	N/A	N/A	N/A	6.0X0.5	N/A	N/A	N/A	N/A	6.0X0.375
Bolts	(4) - 3/4	N/A	N/A	(10) - 3/4	N/A	N/A	N/A	N/A	(8) - 3/4

Member sizes and connection details same as left side



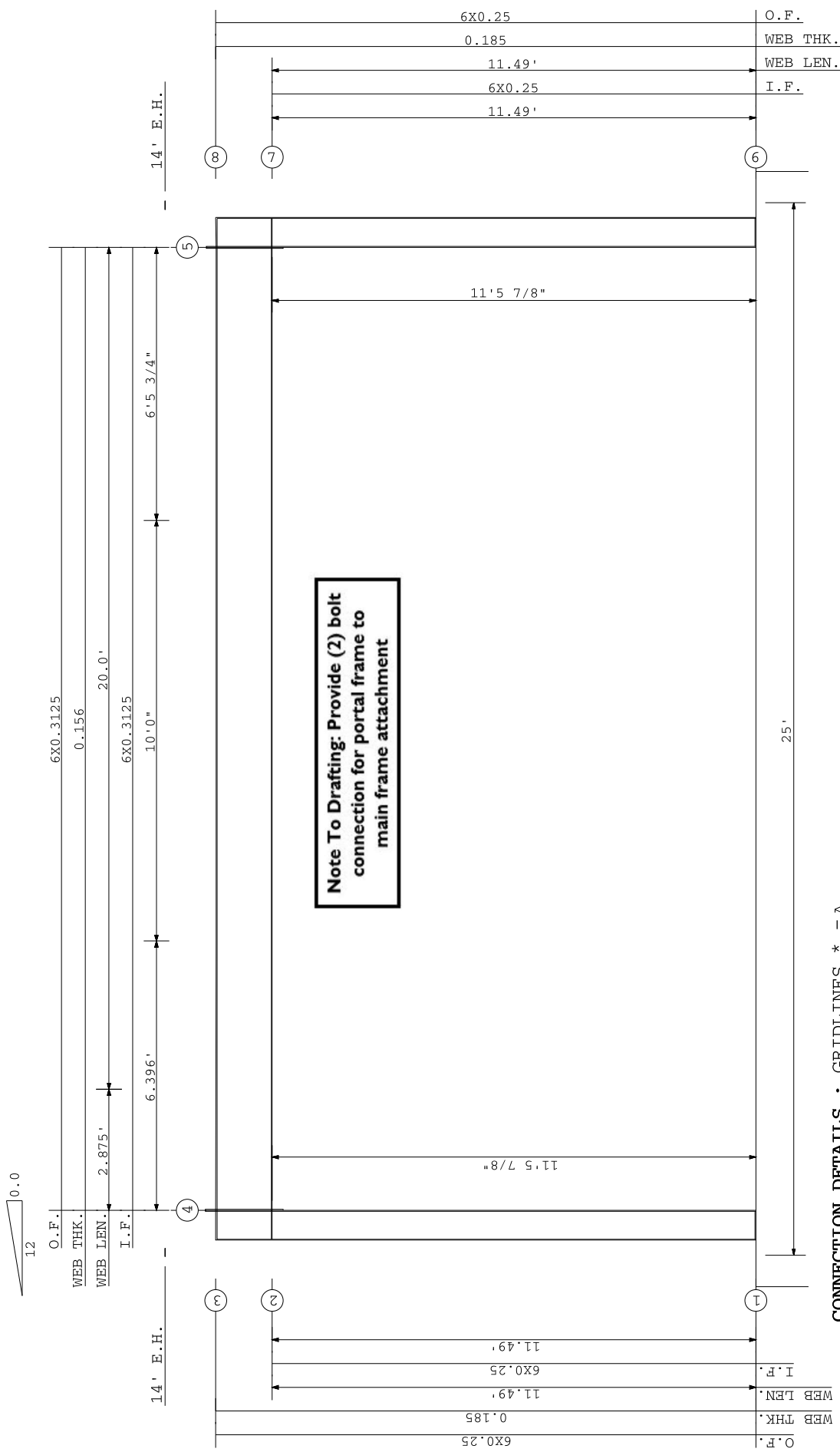
CONNECTION DETAILS : GRIDLINES * = 3 4 5

Location	①*-A	②	③	④	⑤	⑥	⑦	⑧	⑨
Web Dep.	13.0	41.0	N/A	41.0	32.1395	17.5	17.5	17.5	17.5
Type	BASE	HORZ STF	CAP (EXT)	4E/2F	SPLICE	SPLICE	SPLICE	SPLICE	2E/2E
Plate (DN)	8.0X0.625	2.75X0.5	6.0X0.375	6.0X0.625	N/A	N/A	N/A	N/A	6.0X0.375
Plate (UP)	N/A	N/A	N/A	6.0X0.625	N/A	N/A	N/A	N/A	6.0X0.375
Bolts	(4) -1	N/A	N/A	(12) -3/4	N/A	N/A	N/A	N/A	(8) -3/4



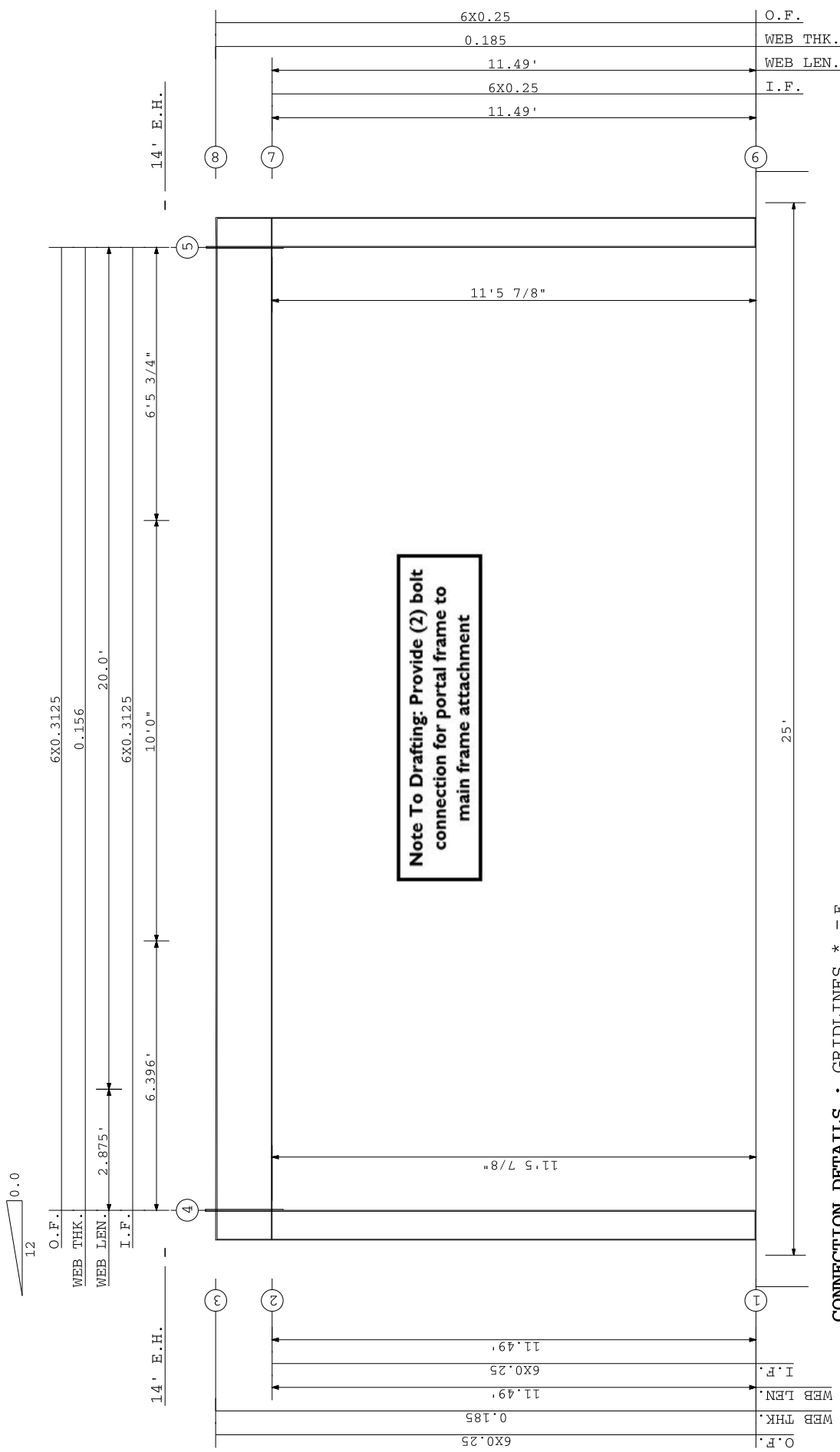
CONNECTION DETAILS : GRIDLINES * = 1 2 3 4 5

Location	① *-G	②	③	④	⑤
Web Dep.	9.87	9.87	N/A	7.89	7.89
Type	BASE	N/A	CAP (EXT)	1P/0	1P/0
Plate (DN)	6.0X0.375	N/A	5.0X0.25	3.96X0.21	5.0X0.25
Plate (UP)	N/A	N/A	N/A	5.0X0.25	5.0X0.25
Bolts	(4) - 3/4	N/A	N/A	(2) - 1/2	(2) - 1/2



CONNECTION DETAILS : GRIDLINES * = A

Location	① *-LEFT	②	③	④	⑤	⑥ *-RIGHT	⑦	⑧
Web Dep.	8.0	8.0	N/A	15.5	15.5	8.0	8.0	N/A
Type	BASE	HORZ STF	CAP (EXT)	2E/2E	2E/2E	BASE	HORZ STF	CAP (EXT)
Plate (DN)	6.0X0.375	2.75X0.3125	6.0X0.3125	6.0X0.375 C	6.0X0.375 C	6.0X0.375	2.75X0.3125	6.0X0.3125
Plate (UP)	N/A	N/A	N/A	6.0X0.375 R	6.0X0.375 R	N/A	N/A	N/A
Bolts	(4) -3/4	N/A	N/A	(8) -3/4	(8) -3/4	(4) -3/4	N/A	N/A

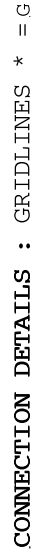


CONNECTION DETAILS : GRIDLINES * = F

Location	① *-LEFT	②	③	④	⑤	⑥ *-RIGHT	⑦	⑧
Web Dep.	8.0	8.0	N/A	15.5	15.5	8.0	8.0	N/A
Type	BASE	HORZ STF	CAP (EXT)	2E/2E	2E/2E	BASE	HORZ STF	CAP (EXT)
Plate (DN)	6.0X0.375	2.75X0.3125	6.0X0.3125	6.0X0.375 C	6.0X0.375 C	6.0X0.375	2.75X0.3125	6.0X0.3125
Plate (UP)	N/A	N/A	N/A	6.0X0.375 R	6.0X0.375 R	N/A	N/A	N/A
Bolts	(4) -3/4	N/A	N/A	(8) -3/4	(8) -3/4	(4) -3/4	N/A	N/A

(1) All sectional dimensions are in inches.
(2) All Flange lengths are measured along outer flange.

Rafter Offset	: (.14")	
Column Offsets	: (.4.25")-	LEFT COLUMN
	: (.4.25")-	RIGHT COLUMN



28
04/28/2023

REACTIONS

BUILDER: DOUG MOSLEY
CUSTOMER: DOUG MOSLEY
JOB NUMBER: 19-B-33112

Notes

1. The reactions provided are based on the Order Documents at the time of mailing. Any changes to building loads or dimensions may change the reactions. The reactions will be superseded and voided by any future mailing.
2. The reactions provided have been created with the following layout (unless noted otherwise).
 - a. A reaction table is provided with the reactions for each load group.
 - b. Rigid Frames
See Note 3.
 - c. Endwalls
1. See Note 3.
 - d. X-Bracing
 1. X- Bracing reactions are included in values shown in the reaction tables.
 2. For IBC and UBC based building codes, when x-bracing is present in the sidewall, individual longitudinal seismic loads (RBUPEQ, RBDWEQ, E+, E- and LEQ) do not include the amplification factor, W
 3. For IBC and UBC based building codes, when x-bracing is present in the endwall, individual transverse seismic loads (EL & ER) do not include the amplification factor, W
 - e. The metal building manufacturer is responsible only for the portion of the anchor rod design pertaining to the transfer of forces between the base plate bearing and the anchor rod's shear and tension. The metal building manufacturer is not responsible for the anchor rod embedment for transfer of forces to the foundation. The metal building manufacturer does not design and is not responsible for the design, material, and construction of the foundation embedments. The end use customer shall assure that adequate provisions are made to the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site. It is recommended that the anchorage and foundation of the building be designed by a registered professional engineer competent in the design of such structures.
 - i. (ref. Appendix A3 of the MBMA Metal Building Systems Manual)
 - f. Anchor rods are ASTM F1554 Gr. 36 material unless noted otherwise on the anchor rod layout drawing.
3. Reactions are provided as un-factored for each load group applied to the column. The factors applied to load groups for the steel column design may be different than the factors used in the foundation design. The foundation engineer shall apply the appropriate load factors and combine the reactions in accordance with the building code and design specifications for proper foundation design.
 - a. For projects using ultimate design wind speeds such as 2012 IBC, 2015 IBC, or Florida building code, the wind load reactions are at a strength value with a load factor of 1.0.
 - b. For IBC codes, the seismic reactions provided are at a strength level with a load factor of 1.0, and do not contain the rho factor.

The manufacturer does not provide "maximum" load combination reactions. However, the individual load reactions provided may be used by the foundation engineer to determine the applicable load combinations for his/her design procedures and allow for an economical foundation design.

SUPPORT REACTIONS FOR EACH LOAD GROUP

*LOCATION: Gridlines: 1

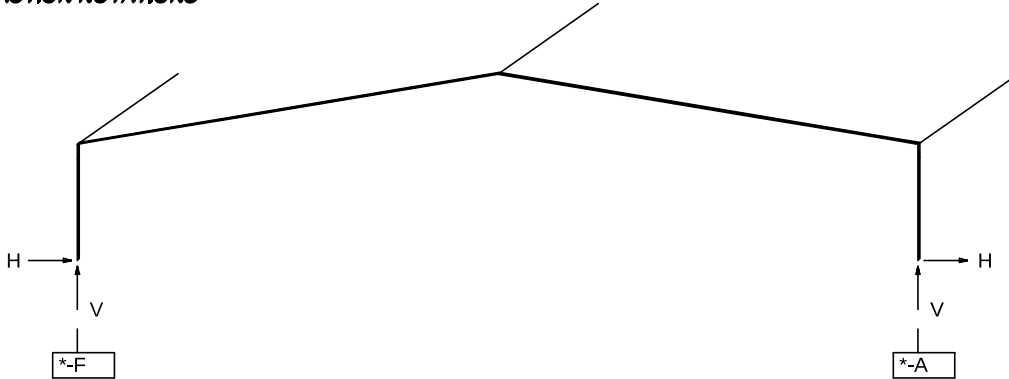
NOTES:(1) All reactions are in kips and kip-ft.

(2) Primary wind load cases are not concurrent.

(3) X-bracing reactions (RBPULW and RBUPEQ) are combined withLWL and LEQ groups only.

TIME:13:44:30

REACTION NOTATIONS



LOAD GROUP REACTION TABLE GRIDLINES * = 1

COLUMN	*-F			*-A		
LOAD GROUP	H	V	L	H	V	L
DL	2.1	2.9	0.0	-2.2	2.4	0.0
LL	8.6	12.2	0.0	-8.7	8.6	0.0
COLL	0.4	0.6	0.0	-0.4	0.4	0.0
WL1	-11.9	-20.2	0.0	13.8	-13.3	0.0
WL2	-9.6	-11.3	0.0	9.0	-8.9	0.0
LWL1	-6.2	-15.4	0.0	9.3	-8.6	0.0
LWL2	-8.3	-12.8	0.0	9.7	-11.7	0.0
LWL3	-3.9	-6.5	0.0	4.5	-4.2	0.0
LWL4	-6.0	-4.0	0.0	4.9	-7.3	0.0
WL3	-12.8	-17.7	0.0	15.1	-16.3	0.0
WL4	-10.5	-8.8	0.0	10.4	-11.9	0.0

LOAD GROUP DESCRIPTION

DL	:	Roof Dead Load
LL	:	Roof Live Load
COLL	:	Roof Collateral Load
WL1	:	Wind from Left to Right with +GCpi
WL2	:	Wind from Left to Right with -GCpi
LWL1	:	Windward Corner Left with +GCpi
LWL2	:	Windward Corner Right with +GCpi
LWL3	:	Windward Corner Left with -GCpi
LWL4	:	Windward Corner Right with -GCpi
WL3	:	Wind from Right to Left with +GCpi
WL4	:	Wind from Right to Left with -GCpi

SUPPORT REACTIONS FOR EACH LOAD GROUP

*LOCATION: Gridlines: 2

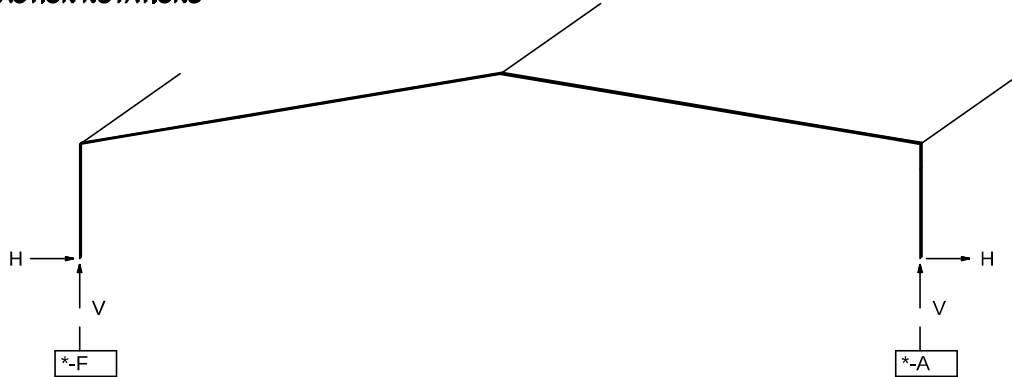
NOTES:(1) All reactions are in kips and kip-ft.

(2) Primary wind load cases are not concurrent.

(3) X-bracing reactions (RBPULW and RBUPEQ) are combined withLWL and LEQ groups only.

TIME:17:29:40

REACTION NOTATIONS



LOAD GROUP REACTION TABLE GRIDLINES * = 2

COLUMN	*-F			*-A		
LOAD GROUP	H	V	L	H	V	L
DL	3.8	4.4	0.0	-3.8	3.9	0.0
LL	21.4	23.6	0.0	-21.5	19.9	0.0
COLL	1.1	1.2	0.0	-1.1	1.0	0.0
WL1	-20.2	-27.2	0.0	19.4	-19.3	0.0
WL2	-12.9	-12.8	0.0	9.6	-9.5	0.0
LWL1	-12.0	-22.6	0.0	15.2	-15.2	0.0
LWL2	-14.7	-19.6	0.0	15.9	-19.4	0.0
LWL3	-4.7	-8.3	0.0	5.4	-5.3	0.0
LWL4	-7.4	-5.3	0.0	6.1	-9.6	0.0
WL3	-18.9	-23.8	0.0	23.8	-23.7	0.0
WL4	-11.5	-9.5	0.0	14.0	-13.9	0.0

LOAD GROUP DESCRIPTION

DL	:	Roof Dead Load
LL	:	Roof Live Load
COLL	:	Roof Collateral Load
WL1	:	Wind from Left to Right with +GCpi
WL2	:	Wind from Left to Right with -GCpi
LWL1	:	Windward Corner Left with +GCpi
LWL2	:	Windward Corner Right with +GCpi
LWL3	:	Windward Corner Left with -GCpi
LWL4	:	Windward Corner Right with -GCpi
WL3	:	Wind from Right to Left with +GCpi
WL4	:	Wind from Right to Left with -GCpi

SUPPORT REACTIONS FOR EACH LOAD GROUP

*LOCATION: Gridlines: 3 4 5

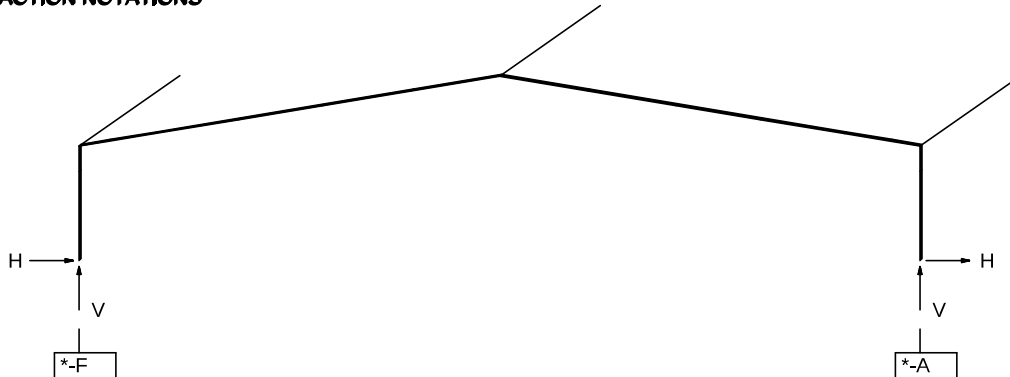
NOTES:(1) All reactions are in kips and kip-ft.

(2) Primary wind load cases are not concurrent.

(3) X-bracing reactions (RBPULW and RBUPEQ) are combined withLWL and LEQ groups only.

TIME:16:59:08

REACTION NOTATIONS



LOAD GROUP REACTION TABLE GRIDLINES * = 3 4 5

COLUMN	*-F			*-A		
LOAD GROUP	H	V	L	H	V	L
DL	4.4	4.9	0.0	-4.4	4.4	0.0
LL	27.0	28.6	0.0	-27.1	25.0	0.0
COLL	1.3	1.4	0.0	-1.4	1.2	0.0
WL1	-22.0	-28.3	0.0	17.8	-19.4	0.0
WL2	-13.0	-11.5	0.0	6.3	-7.1	0.0
LWL1	-14.3	-27.0	0.0	17.6	-19.0	0.0
LWL2	-17.6	-23.6	0.0	18.5	-24.3	0.0
LWL3	-5.3	-10.2	0.0	6.2	-6.7	0.0
LWL4	-8.6	-6.8	0.0	7.1	-12.0	0.0
WL3	-17.6	-24.1	0.0	25.9	-25.3	0.0
WL4	-8.7	-7.3	0.0	14.4	-13.0	0.0

LOAD GROUP DESCRIPTION

DL	:	Roof Dead Load
LL	:	Roof Live Load
COLL	:	Roof Collateral Load
WL1	:	Wind from Left to Right with +GCpi
WL2	:	Wind from Left to Right with -GCpi
LWL1	:	Windward Corner Left with +GCpi
LWL2	:	Windward Corner Right with +GCpi
LWL3	:	Windward Corner Left with -GCpi
LWL4	:	Windward Corner Right with -GCpi
WL3	:	Wind from Right to Left with +GCpi
WL4	:	Wind from Right to Left with -GCpi

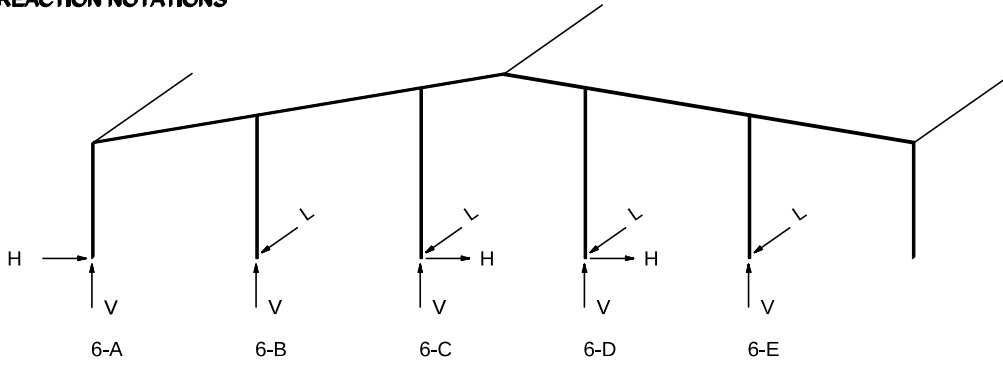
PATH: R:\jobs\Active\Eng\19-B-33112\ver02-mjimenez\BLDG-A\run01\

SUPPORT REACTIONS FOR EACH LOAD GROUP

NOTE: All reactions are in kips and kip-ft.

TIME:17:13:17

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	6-A			6-B			6-C			6-D			6-E		
LOAD GROUP	H	V	L	H	V	L	H	V	L	H	V	L	H	V	L
D	0.0	0.5	0.	0.	1.1	0.	0.	1.0	0.	0.	1.1	0.	0.	1.2	0.
C	0.0	0.1	0.	0.	0.3	0.	0.	0.2	0.	0.	0.2	0.	0.	0.3	0.
L	0.1	2.2	0.	0.	5.5	0.0	0.	5.0	0.0	0.	5.0	0.0	0.	5.5	0.0
W+	-0.1	-2.7	0.	0.	-6.3	3.7	0.	-8.2	4.4	0.	-8.2	4.4	0.	-6.3	3.7
W-	-0.1	-2.7	0.	0.	-6.3	-4.1	0.	-8.2	-4.9	0.	-8.2	-4.9	0.	-6.3	-4.1
WR	-0.1	-2.7	0.	0.	-6.3	0.0	0.	-4.9	0.0	3.2	-11.6	0.0	0.	-6.3	0.0
WL	-0.1	-2.7	0.	0.	-6.3	0.0	-4.4	-12.8	0.0	0.	-3.7	0.0	0.	-6.3	0.0

LOAD GROUP DESCRIPTION

- D : Dead load
- C : Collateral load
- L : Live load
- W+ : Wind load as an inward acting pressure
- W- : Wind load as an outward acting suction
- WR : Wind force from the right
- WL : Wind force from the left

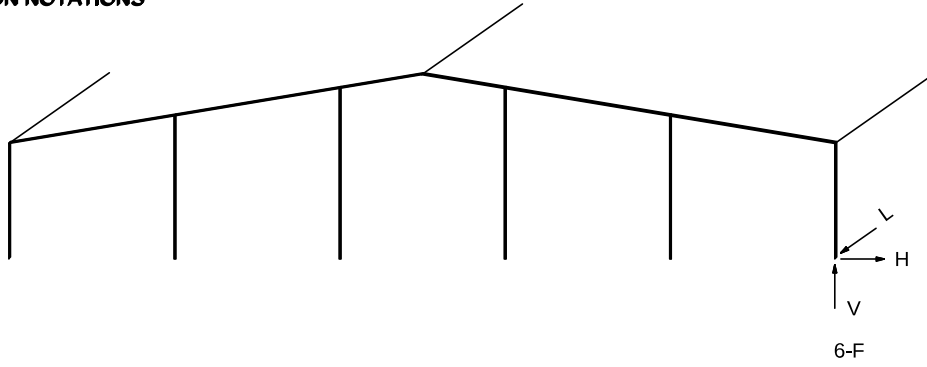
PATH: R:\jobs\Active\Eng\19-B-33112\ver02-mjimenez\BLDG-A\run01\

SUPPORT REACTIONS FOR EACH LOAD GROUP

NOTE: All reactions are in kips and kip-ft.

TIME:17:13:17

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	6-F		
LOAD GROUP	H	V	L
D	0.0	0.9	0.
C	0.0	0.2	0.
L	0.1	4.0	0.
W+	-0.3	-7.0	3.4
W-	-0.3	-7.0	-3.9
WR	-0.3	-7.0	0.
WL	-0.3	-7.0	0.

LOAD GROUP DESCRIPTION

D : Dead load
C : Collateral load
L : Live load
W+ : Wind load as an inward acting pressure
W- : Wind load as an outward acting suction
WR : Wind force from the right
WL : Wind force from the left

SUPPORT REACTIONS FOR EACH LOAD GROUP

LOCATION:bays 3-(Gridline A)

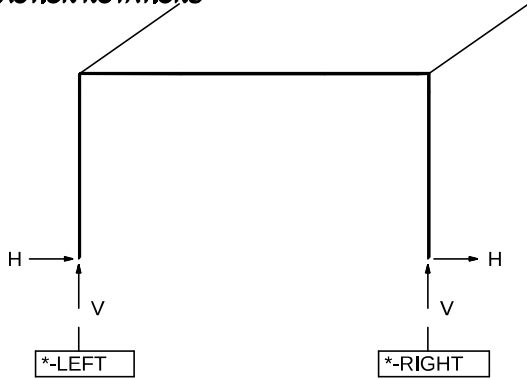
NOTES:(1) All reactions are in kips and kip-ft.

(2) Primary wind load cases are not concurrent.

(3) X-bracing reactions (RBPULW and RBUPEQ) are combined withLWL and LEQ groups only.

TIME:17:33:13

REACTION NOTATIONS



LOAD GROUP REACTION TABLE GRIDLINES * = A

COLUMN	*-LEFT			*-RIGHT		
	H	V	L	H	V	L
DL	0.0	0.4	0.0	-0.0	0.4	0.0
LWL1	-4.1	-4.6	0.0	-4.3	4.6	0.0
LWL2	4.3	4.6	0.0	4.1	-4.6	0.0

LOAD GROUP DESCRIPTION

DL : Roof Dead Load
LWL1 : Wind from Left to Right with +GCpi
LWL2 : Wind from Right to Left with -GCpi

SUPPORT REACTIONS FOR EACH LOAD GROUP

LOCATION:bays 3-(Gridline F)

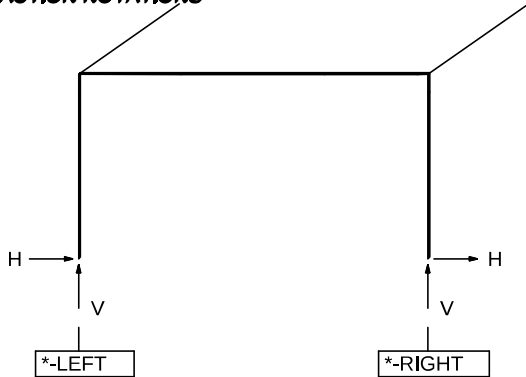
NOTES:(1) All reactions are in kips and kip-ft.

(2) Primary wind load cases are not concurrent.

(3) X-bracing reactions (RBPULW and RBUPEQ) are combined withLWL and LEQ groups only.

TIME:17:04:07

REACTION NOTATIONS



LOAD GROUP REACTION TABLE GRIDLINES * = F

COLUMN	*-LEFT			*-RIGHT		
	H	V	L	H	V	L
DL	0.0	0.4	0.0	-0.0	0.4	0.0
LWL1	-4.7	-5.2	0.0	-4.9	5.2	0.0
LWL2	4.9	5.2	0.0	4.7	-5.2	0.0

LOAD GROUP DESCRIPTION

DL : Roof Dead Load
LWL1 : Wind from Left to Right with +GCpi
LWL2 : Wind from Right to Left with -GCpi

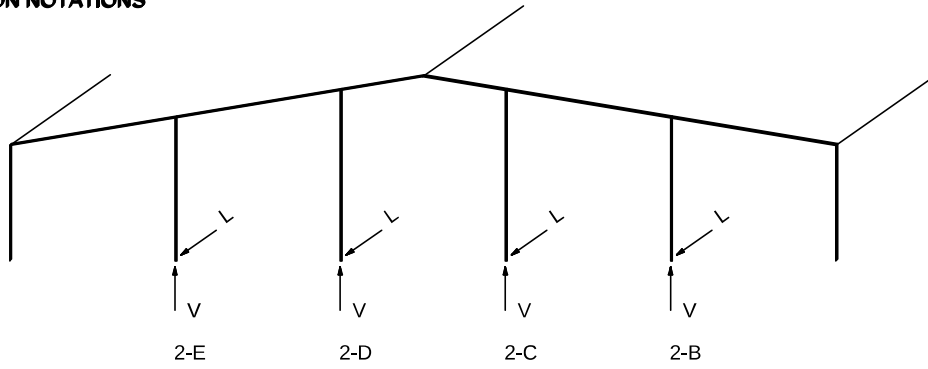
PATH: R:\jobs\Active\Eng\19-B-33112\ver02-mjimenez\BLDG-A\run01\

SUPPORT REACTIONS FOR EACH LOAD GROUP

NOTE: All reactions are in kips and kip-ft.

TIME:15:06:02

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	2-E			2-D			2-C			2-B		
LOAD GROUP	H	V	L	H	V	L	H	V	L	H	V	L
D	0.	0.2	0.	0.	0.2	0.	0.	0.2	0.	0.	0.2	0.
W+	0.	0.	3.7	0.	0.	4.4	0.	0.	4.4	0.	0.	3.7
W-	0.	0.	-4.1	0.	0.	-4.9	0.	0.	-4.9	0.	0.	-4.1

LOAD GROUP DESCRIPTION

D : Dead load
W+ : Wind load as an inward acting pressure
W- : Wind load as an outward acting suction

SUPPORT REACTIONS FOR EACH LOAD GROUP

*LOCATION: Gridlines: 1 2 3 4 5

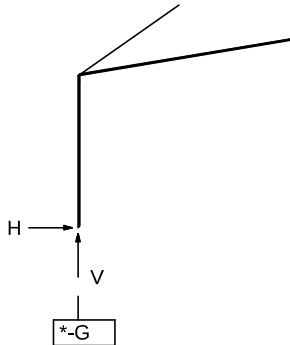
NOTES:(1) All reactions are in kips and kip-ft.

(2) Primary wind load cases are not concurrent.

(3) X-bracing reactions (RBPULW and RBUPEQ) are combined withLWL and LEQ groups only.

TIME:13:57:25

REACTION NOTATIONS



LOAD GROUP REACTION TABLE GRIDLINES * =

1 2 3 4 5

COLUMN	*-G		
LOAD GROUP	H	V	L
DL	0.0	0.7	0.0
LL	0.1	4.4	0.0
COLL	0.0	0.2	0.0
WL1	-0.2	-7.5	0.0
WL2	-0.3	-0.8	0.0
LWL1	0.1	-7.5	0.0
LWL2	0.1	-5.6	0.0
LWL3	-0.1	-0.8	0.0
LWL4	0.0	1.1	0.0
WL3	0.1	-5.8	0.0
WL4	-0.0	0.9	0.0

LOAD GROUP DESCRIPTION

DL : Roof Dead Load
 LL : Roof Live Load
 COLL : Roof Collateral Load
 WL1 : Wind from Left to Right with +GCpi
 WL2 : Wind from Left to Right with -GCpi
 LWL1 : Windward Corner Left with +GCpi
 LWL2 : Windward Corner Right with +GCpi
 LWL3 : Windward Corner Left with -GCpi
 LWL4 : Windward Corner Right with -GCpi
 WL3 : Wind from Right to Left with +GCpi
 WL4 : Wind from Right to Left with -GCpi

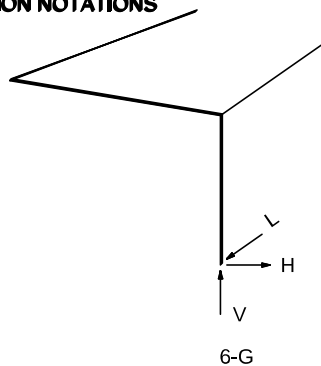
PATH: R:\jobs\Active\Eng\19-B-33112\ver02-mjimenez\BLDG-A\run01\

SUPPORT REACTIONS FOR EACH LOAD GROUP

NOTE: All reactions are in kips and kip-ft.

TIME:17:13:17

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	6-G		
LOAD GROUP	H	V	L
D	0.0	0.5	0.
C	0.0	0.1	0.
L	-0.1	2.2	0.
W+	0.1	-4.1	1.5
W-	0.1	-4.1	-1.7
WR	2.4	-4.1	0.
WL	-2.5	-4.1	0.

LOAD GROUP DESCRIPTION

D : Dead load
C : Collateral load
L : Live load
W+ : Wind load as an inward acting pressure
W- : Wind load as an outward acting suction
WR : Wind force from the right
WL : Wind force from the left

SUPPORT REACTIONS FOR EACH LOAD GROUP

LOCATION:bays 3-(Gridline G)

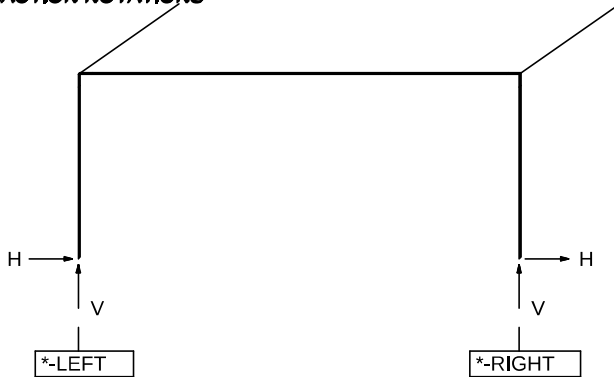
NOTES:(1) All reactions are in kips and kip-ft.

(2) Primary wind load cases are not concurrent.

(3) X-bracing reactions (RBPULW and RBUPEQ) are combined withLWL and LEQ groups only.

TIME:17:06:27

REACTION NOTATIONS



LOAD GROUP REACTION TABLE GRIDLINES * = G

COLUMN	*-LEFT			*-RIGHT		
	H	V	L	H	V	L
DL	0.0	0.3	0.0	-0.0	0.3	0.0
LWL1	-0.4	-0.4	0.0	-0.4	0.4	0.0
LWL2	0.4	0.4	0.0	0.4	-0.4	0.0

LOAD GROUP DESCRIPTION

DL : Roof Dead Load
LWL1 : Wind from Left to Right with +GCpi
LWL2 : Wind from Right to Left with -GCpi