



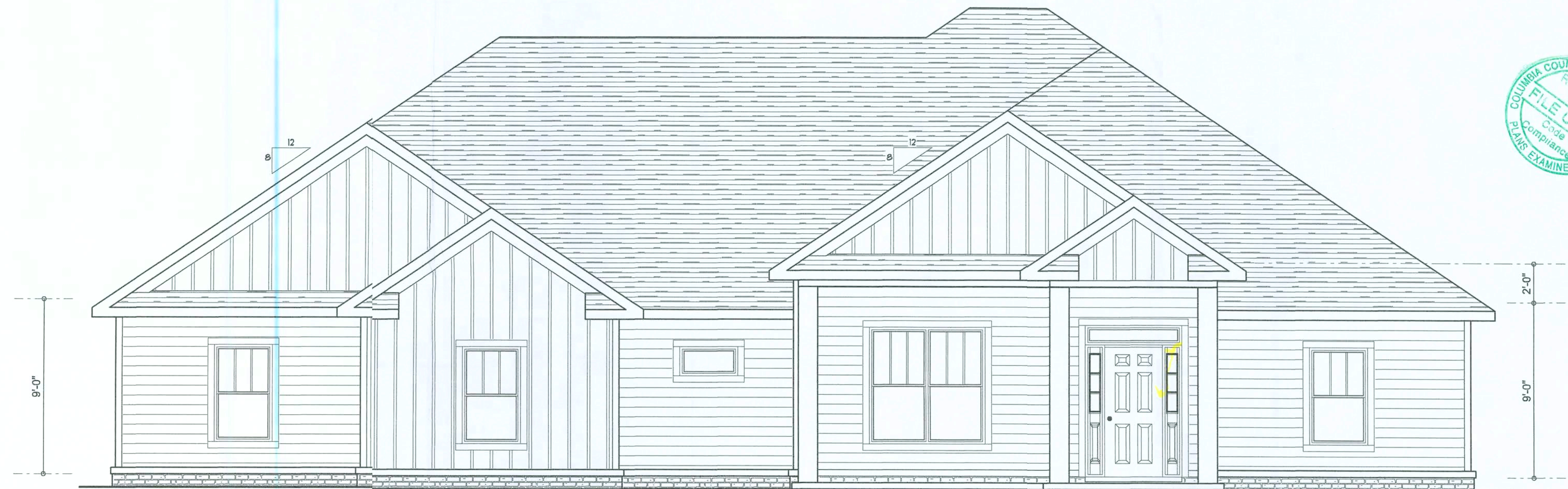
**RIGHT ELEVATION**  
SCALE: 1/4" = 1'-0"



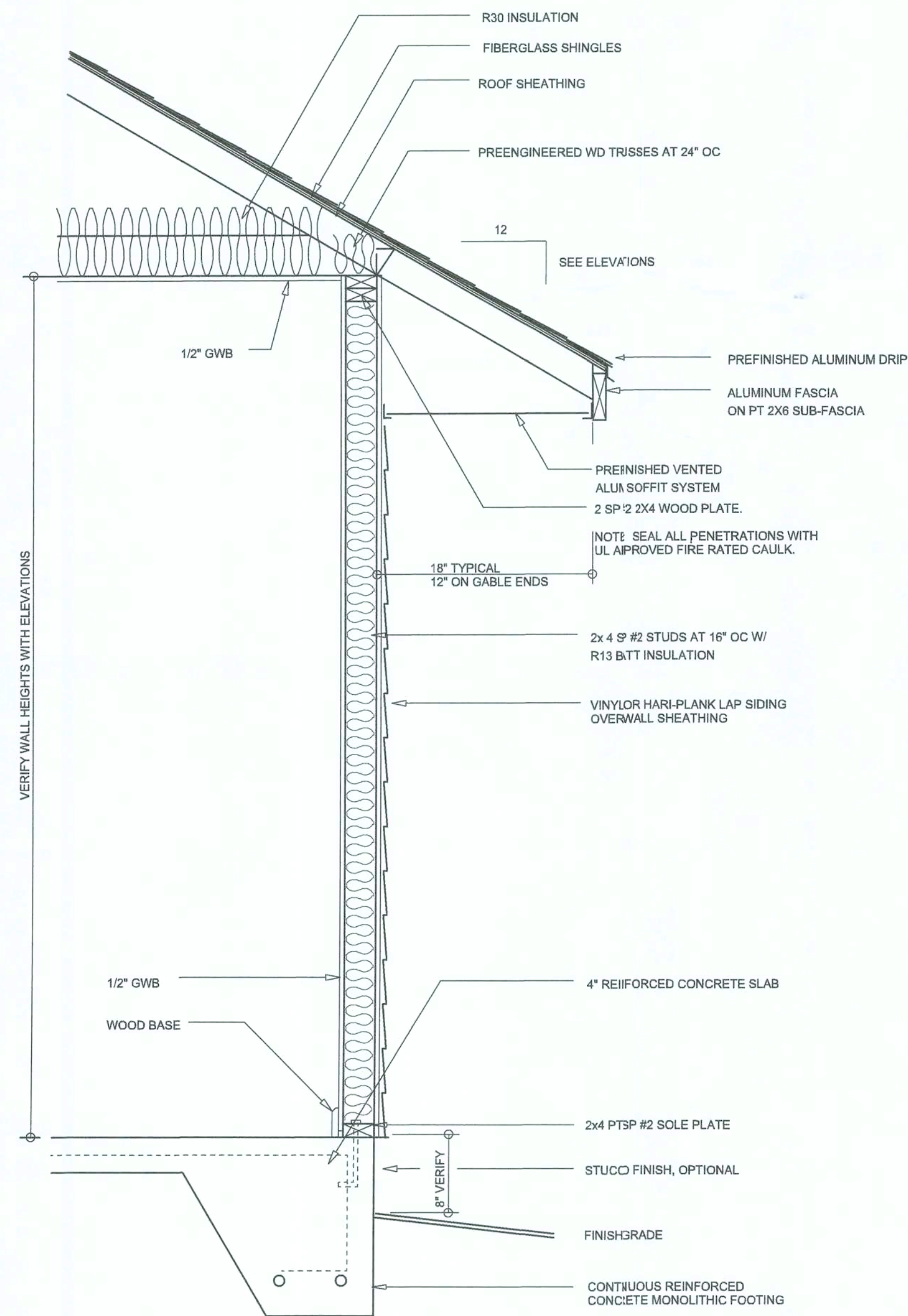
**LEFT ELEVATION**  
SCALE: 1/4" = 1'-0"



**REAR ELEVATION**  
SCALE: 1/4" = 1'-0"



**FRONT ELEVATION**  
SCALE: 1/4" = 1'-0"



**TYPICAL WALL SECTION**  
SCALE: 1" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

REVISIONS  
October 30, 2019

**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE

EXTERIOR ELEVATIONS  
SCALE: 1/4" = 1'-0"

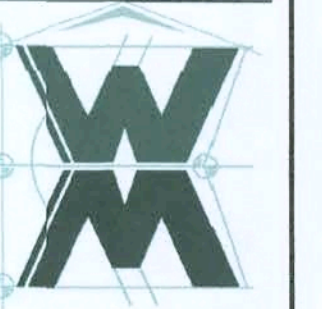
TYPICAL WALL SECTION  
SCALE: 1" = 1'-0"

THE WESTERN MODEL FOR:

**MIKE & JULIE BRANCH**  
Property Address: Lot 27, Forest Country, Lake City, FL 32025

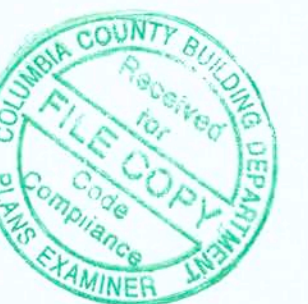
**GIBALTAR CONTRACTING, LLC.**  
LIC# 1259633 HIGH SPRINGS, FLORIDA

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426 SW COMMERCE DR, STE 130  
LAKE CITY, FL 32025  
(386) 758-6406  
vm@vmdesign.net



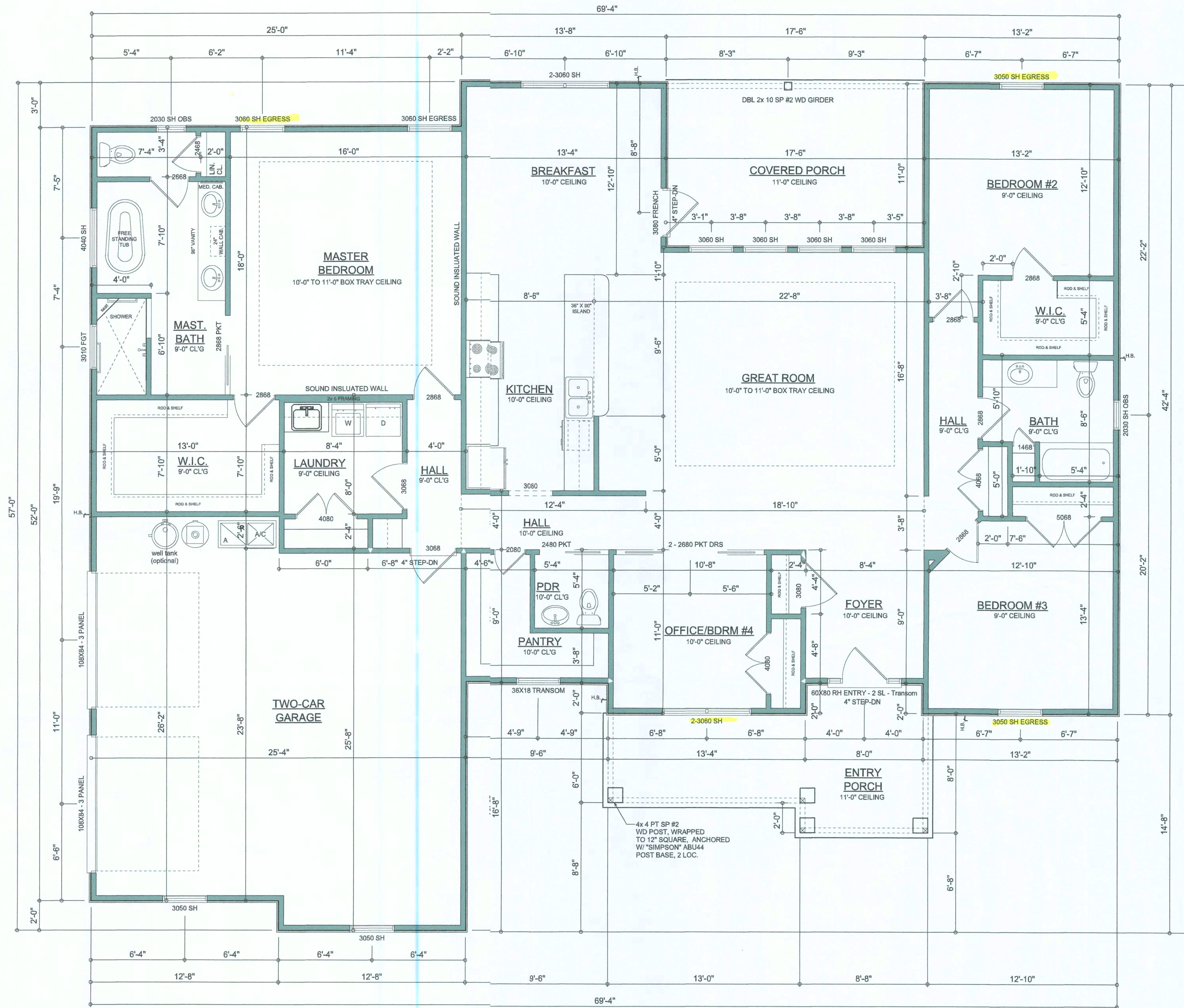
JOB NUMBER  
20191008

SHEET NUMBER  
**A.1**  
OF 3 SHEETS



*W. C. M.*





# FLOOR PLAN

SCALE: 1/4" = 1'-0"

NOTE: ALL WALLS SHALL BE 9'-0" UNLESS OTHERWISE NOTED.

Garage fire separations shall comply with the following:

- The private garage shall be separated from the dwelling unit and its attic area by means of a minimum 1/2-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.
- Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
- A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.
- When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

## AREA SUMMARY

|                    |              |             |
|--------------------|--------------|-------------|
| LIVING AREA        | 2,336        | S.F.        |
| GARAGE AREA        | 645          | S.F.        |
| COVERED PORCH AREA | 193          | S.F.        |
| ENTRY PORCH AREA   | 163          | S.F.        |
| <b>TOTAL AREA</b>  | <b>3,337</b> | <b>S.F.</b> |

THE "WESTERN" MODEL FOR:

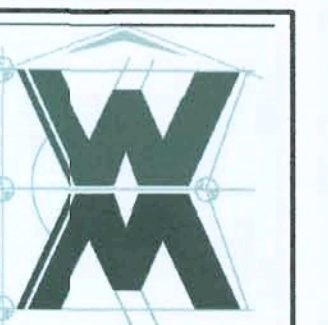
**MIKE & JULIE BRANCH**

Property Address: Lot 27, Forest Country, Lake City, FL 32025

**GIBALTAR CONTRACTING, LLC.**

LIC# 1259633 HIGH SPRINGS, FLORIDA

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426 SW COMMERCE DR., STE 130  
LAKE CITY, FL 32025  
(386) 758-8406  
wj@wjmyers.net



JOB NUMBER  
20191008

SHEET NUMBER

**A.2**

OF 3 SHEETS

REVISIONS

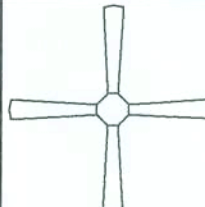













November 11, 2019



**DIMENSIONED FLOOR PLAN**

SCALE: 1/4" = 1'-0"



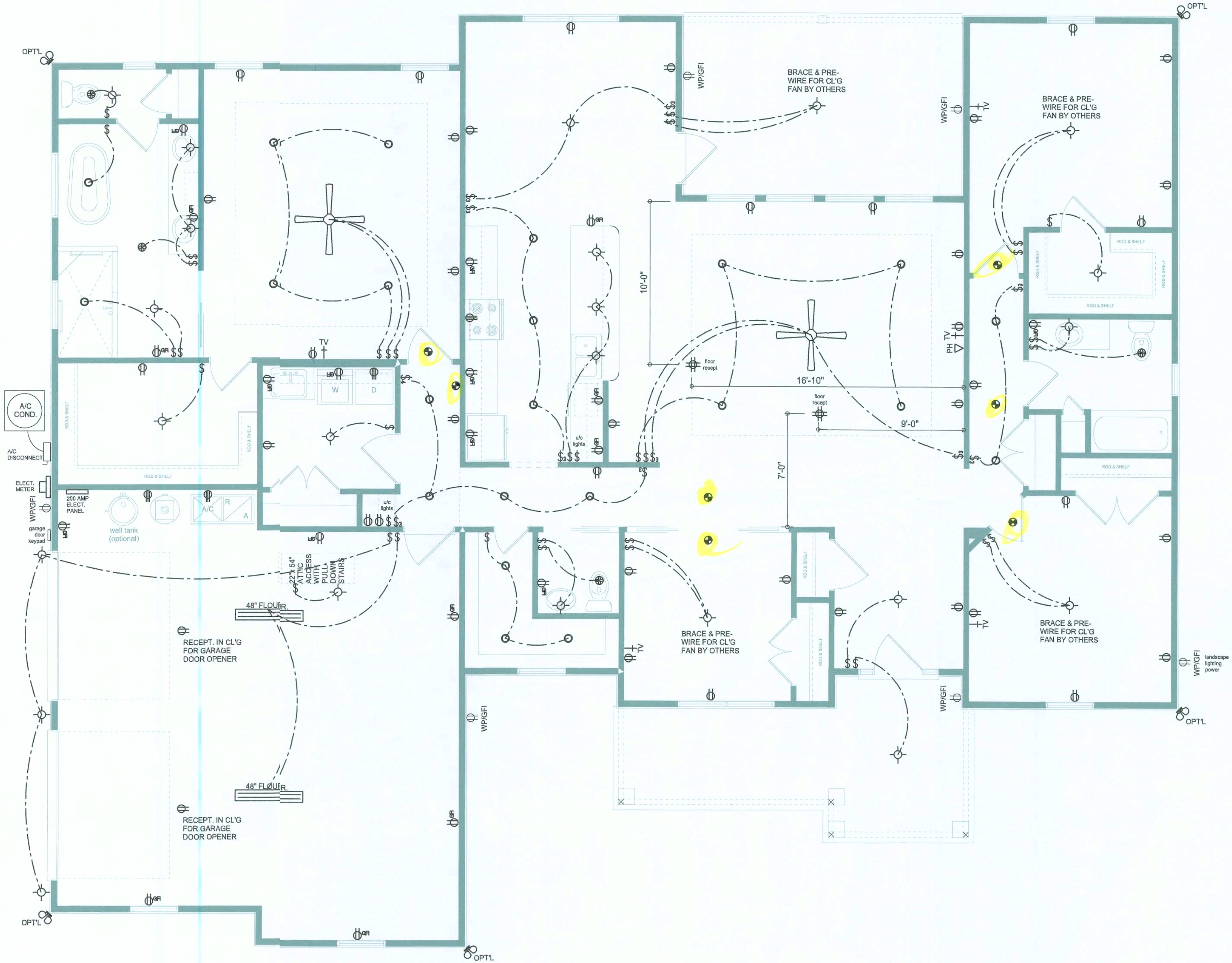
| ELECTRICAL LEGEND   |   |
|---|---|
|    | CEILING FAN<br>(PRE-WIRE FOR LIGHT KIT)           |
|    | DOUBLE SECURITY LIGHT                             |
|    | RECESSED CAN LIGHT                                |
|    | BATH EXHAUST FAN                                  |
|    | LIGHT FIXTURE                                     |
|    | DUPLEX OUTLET (AFCI & TAMPER RESISTANT)           |
|    | 220v OUTLET                                       |
|   | GFI DUPLEX OUTLET (PER NEC 06.8)                  |
|  | TELEVISION JACK                                   |
|  | TELEPHONE JACK                                    |
|  | SMOKE / CARBON MONOXIDE DITECTOR (see note below) |
|  | WALL SWITCH                                       |
|  | 3 WAY WALL SWITCH                                 |
|  | WATER PROOF GFI OUTLET                            |
| <div>48" FLOOR.</div>   | 2 OR 4 TUB FLUORESCENT FIXTURE                    |

NOTE:  
ALL INTERIOR RECEPTACLES SHALL BE AFCI  
(ARC FAULT CIRCUIT INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER  
NEC 406.11

ALL SMOKE DETECTORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR  
AND SHALL HAVE BATTERY BACKUP POWER  
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY  
ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE  
INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS.  
CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB  
PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR  
SHALL BE USED AS AN EQUIPMENT GROUND.

IT IS THE LICENSED ELECTRICAL CONTRACTORS RESPONSIBILITY TO INSURE THAT ALL  
WORK PERFORMED AND EQUIPMENT INSTALLED MEETS OR EXCEEDS THE NFPA70 2014 NATIONAL  
ELECTRIC CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.



ELECTRICAL PLAN  
SCALE: 1/4\"/>

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

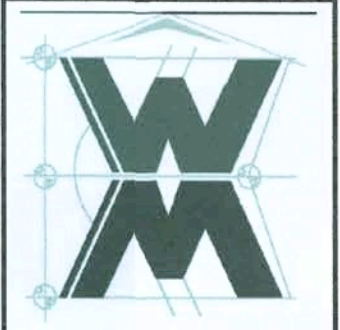
| REVISIONS         |
|-------------------|
| December 03, 2019 |
|                   |
|                   |
|                   |



ELECTRICAL PLAN  
SCALE: 1/4\"/>

THE "WESTERN" MODEL FOR:  
**MIKE & JULIE BRANCH**  
Property Address: Lot 27, Forest Country, Lake City, FL 32025  
**GIBALTAR CONTRACTING, LLC.**  
LIC# 1259633 HIGH SPRINGS, FLORIDA

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ASSOCIATES, INC.  
426 SW COMMERCE DR. STE 130  
LAKE CITY, FL 32025  
(386) 758-8406  
will@villmyers.net

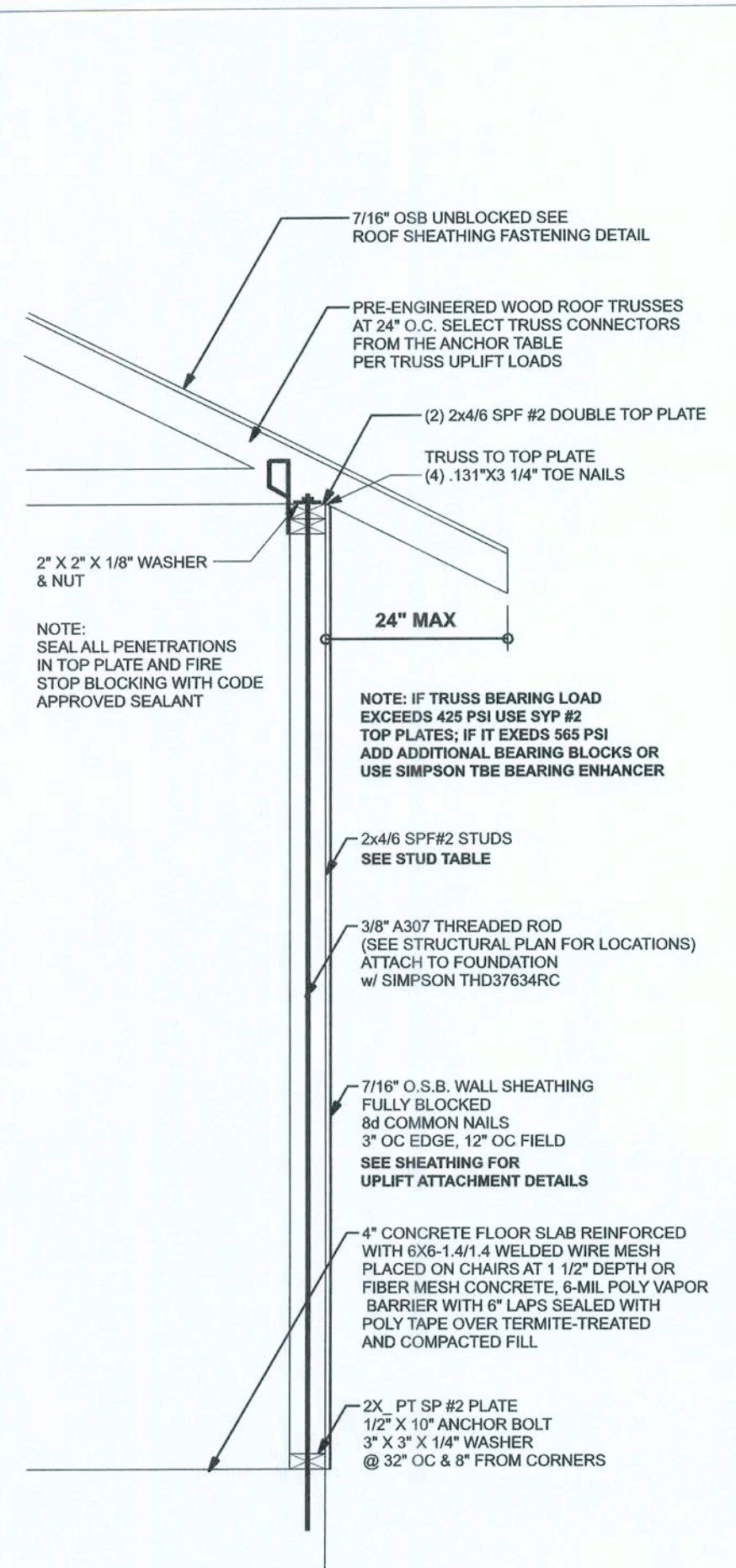


JOB NUMBER  
20191008

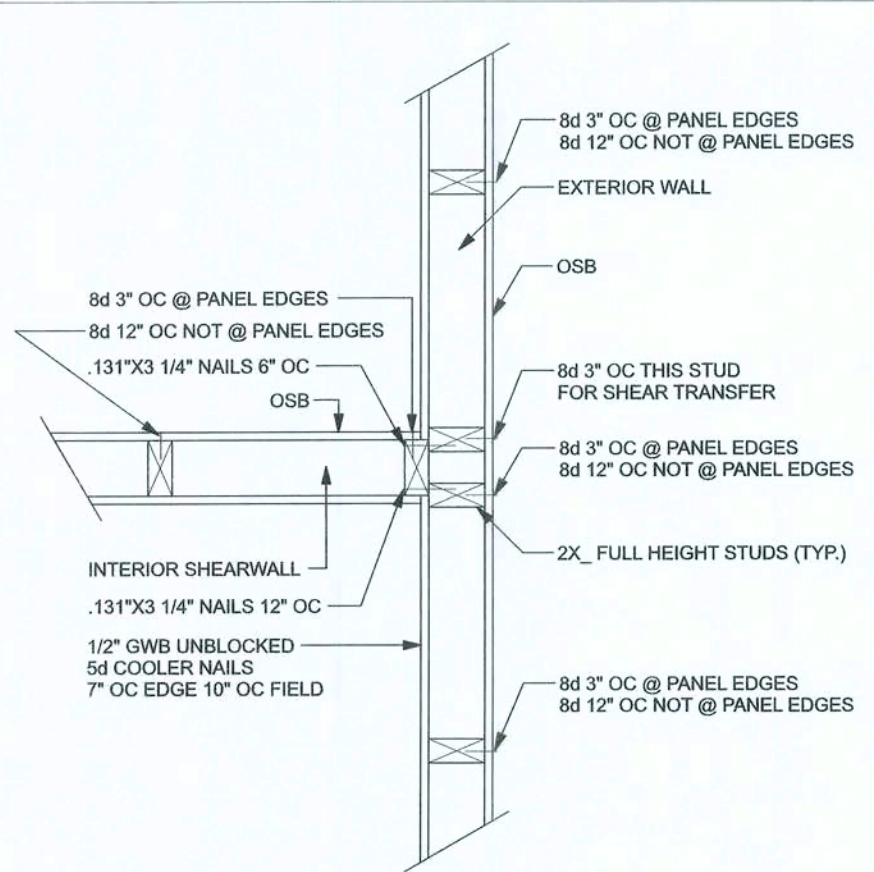
SHEET NUMBER  
**A.3**  
OF 3 SHEETS

*Will C. Myers*

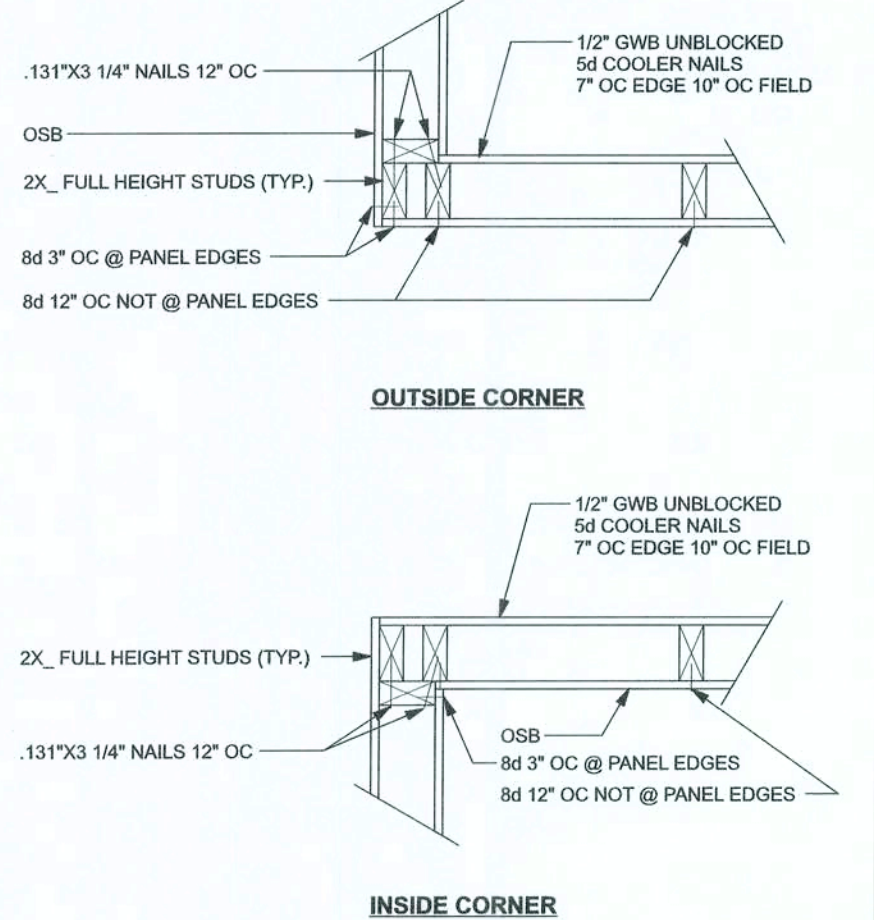




**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"



**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME

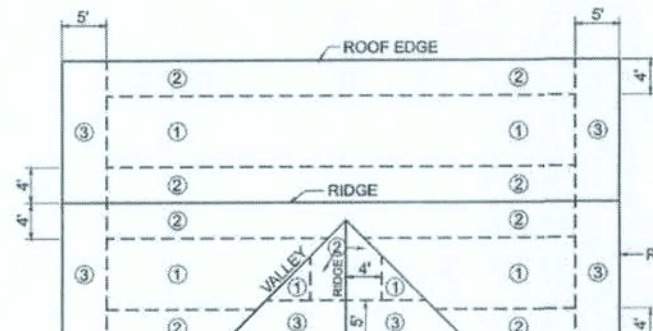


**(TYP.) CORNER FRAMING**  
WOOD FRAME

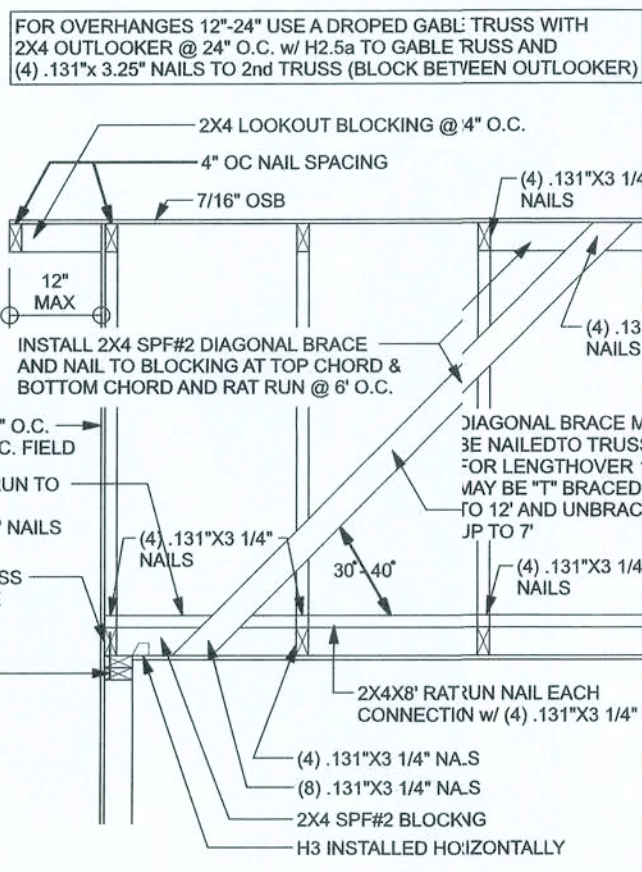
- RING-SHANK NAILS SHALL HAVE THE FOLLOWING MINIMUM DIMENSIONS:
- 0.113 INCH NOMINAL ROOT SHANK DIAMETER
  - RING DIAMETER OF 0.010 OVER SHANK DIAMETER
  - 16 TO 20 RINGS PER INCH
  - 0.280 INCH FULL ROUND HEAD DIAMETER
  - 2-3/8 INCH NAIL LENGTH

NAILING PATTERN SHALL BE:

- 4" OC GABLE END (SEE GABLE BRACING DETAIL)
- 6" OC @ EDGES ALL ZONES
- 6" OC @ INTERMEDIATE FRAMING IN ZONE 3 &
- 12" OC @ INTERMEDIATE FRAMING IN ZONE 1 &

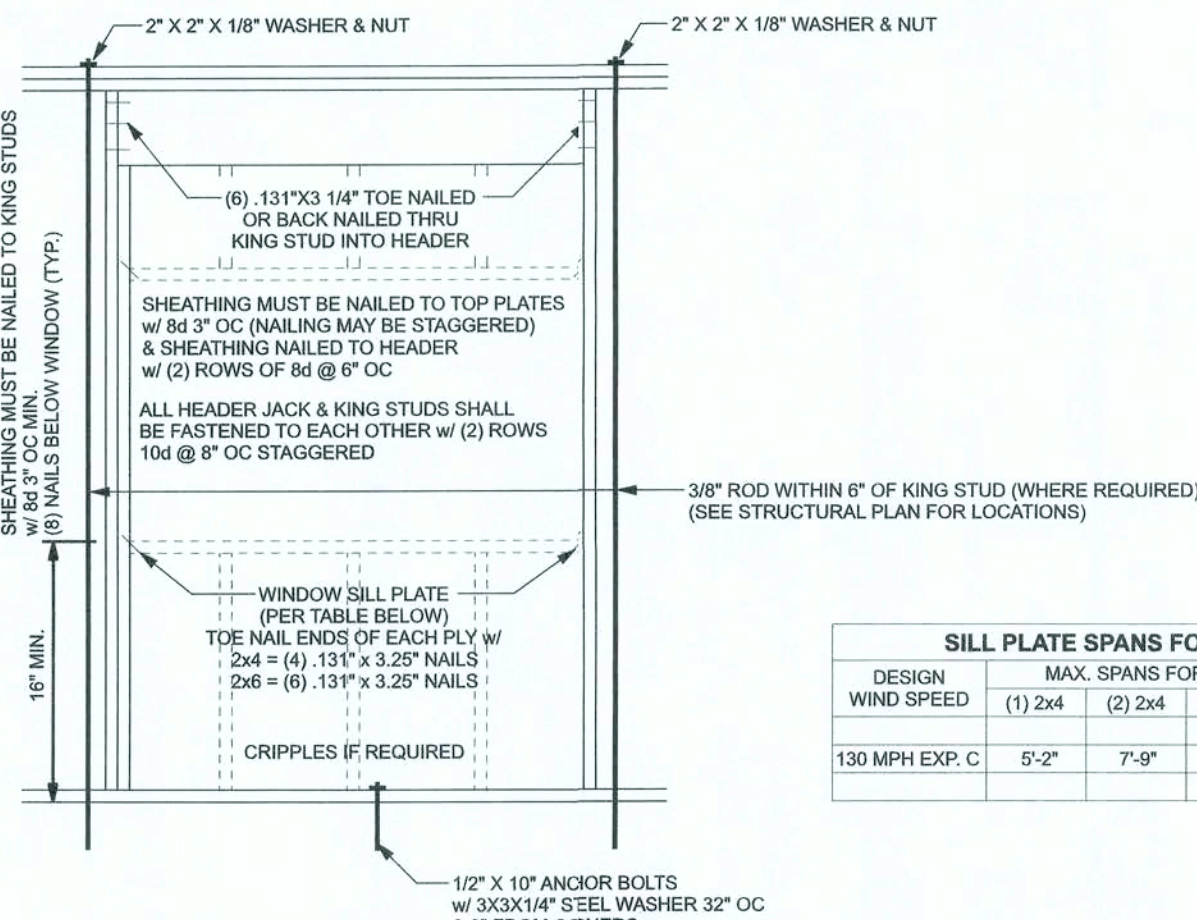


**ROOF SHEATHING FASTENING**

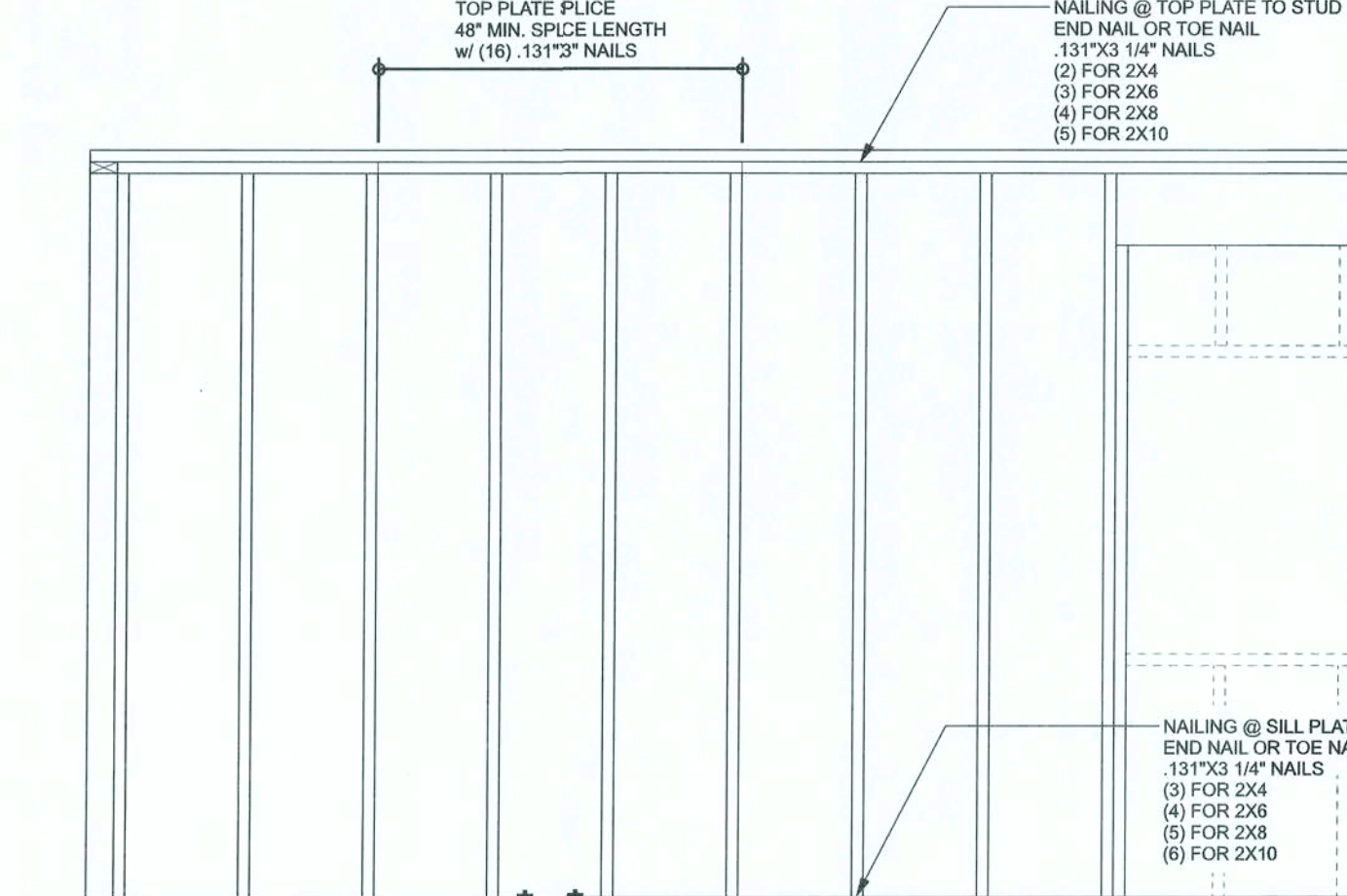


**SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.**  
FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. ENCLOSED

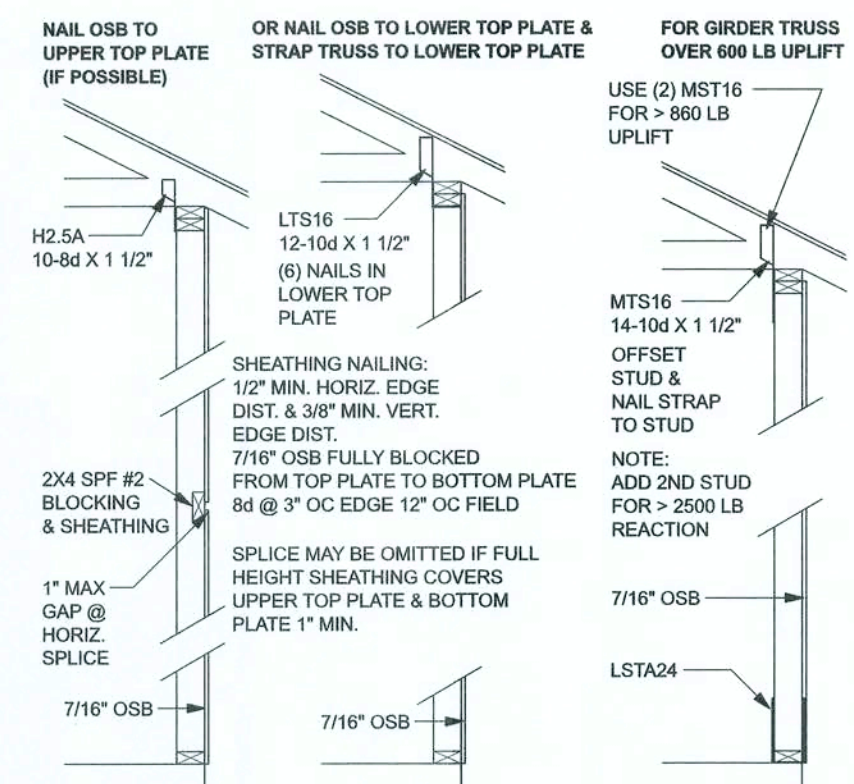
**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



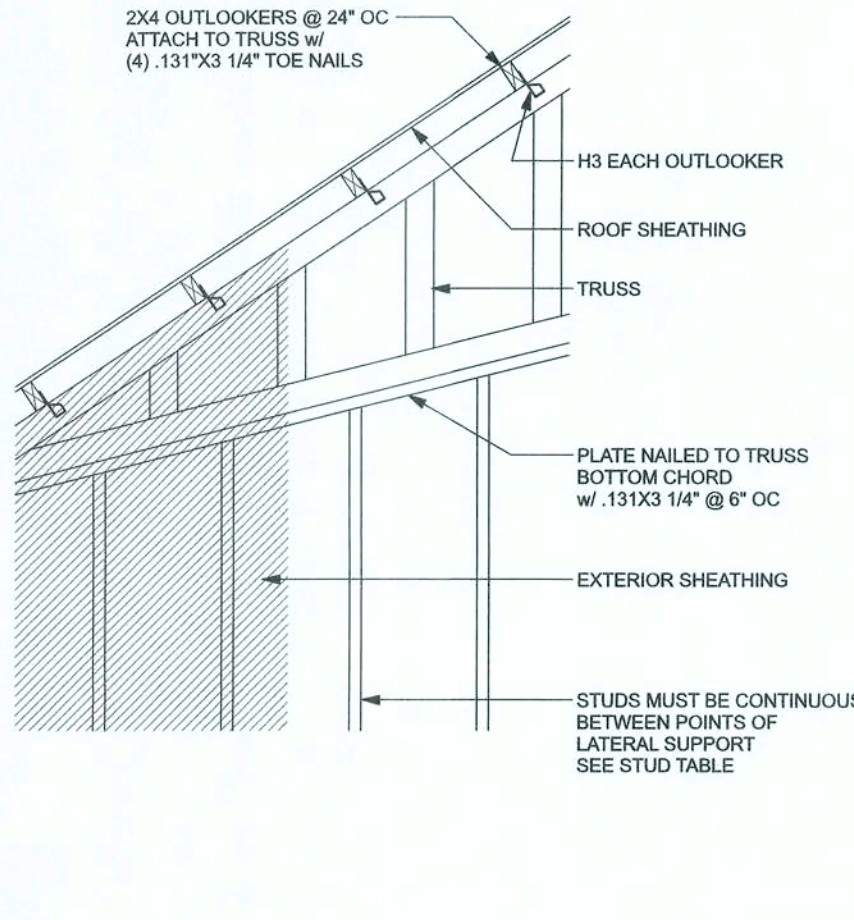
**TYPICAL HEADER STRAPING DETAIL**  
ONE STORY WOOD FRAME



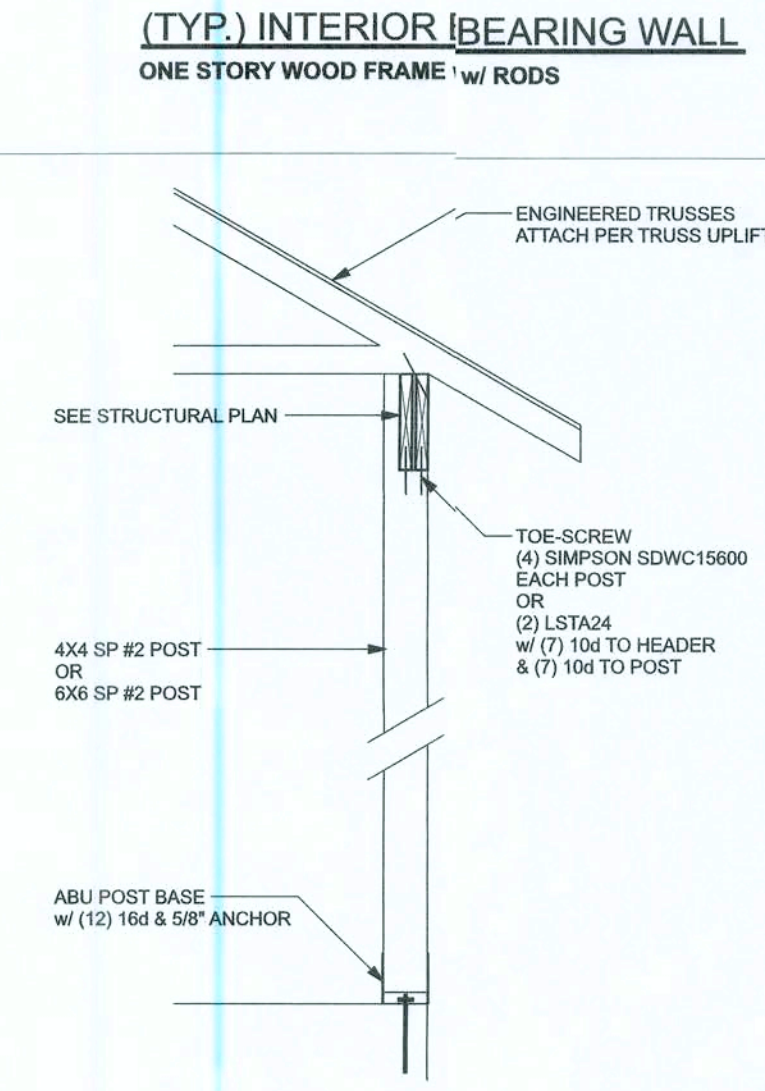
**(TYP.) WALL CONNECTIONS**  
ONE STORY WOOD FRAME



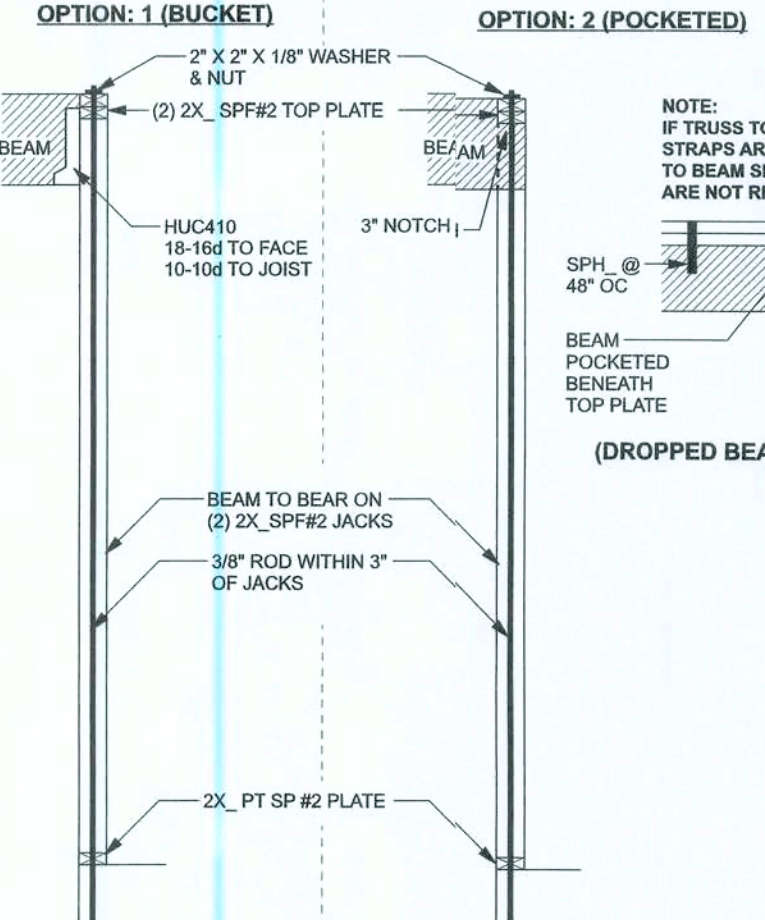
**SHEATHING FOR UPLIFT ATTACHMENT DETAILS**  
ONE STORY WOOD FRAME



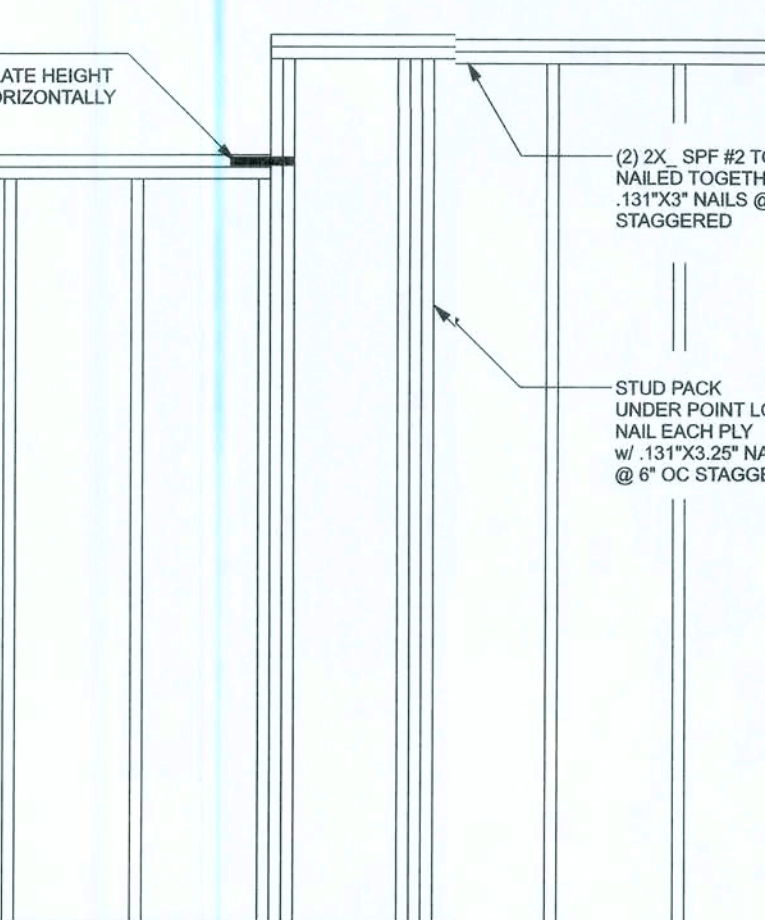
**(TYP.) GABLE WALL w/ VAULTED CEILING**  
WOOD FRAME



**(TYP.) PORCH POST**  
ONE STORY WOOD



**(TYP.) BEAM TO WALL**  
WOOD FRAME w/ RODS



**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $f_c = 2500$  PSI.

WELDED WIRE REINFORCED SLAB: 8" x 8" W14 x W14, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/WW OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS,  $F_y = 40$  KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, U.N.C.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GRADED CMU.

**BUILDER'S RESPONSIBILITY:**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK:

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND GROUNDWATER HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN:**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS. IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER, IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

| CONNECTOR TABLE |            |                    |                   |                          |
|-----------------|------------|--------------------|-------------------|--------------------------|
| Uplift SP       | Uplift SPF | Truss Connector    | To Plate          | To Truss/Rafter          |
| 615             | 485        | SDWC15600          | -                 | -                        |
| 415             | 290        | H3                 | 4-8x1 1/2"        | 4-8x1 1/2"               |
| 575             | 495        | H2.5A              | 5-8x1 1/2"        | 5-8x1 1/2"               |
| 1340            | 1015       | H10A               | 9-10x1 1/2"       | 9-10x1 1/2"              |
| 720             | 620        | LTS12-20           | 6-10x1 1/2"       | 6-10x1 1/2"              |
| 1000            | 860        | MTS12-30           | 7-10x1 1/2"       | 7-10x1 1/2"              |
| 1450            | 1245       | HTS20-30           | 12-10x1 1/2"      | 12-10x1 1/2"             |
| Uplift SP       | Uplift SPF | Truss Ties         | To One Member     | To Other Member          |
| 1225            | 1225       | LST421             | 8-10d             | 8-10d                    |
| 1540            | 1455       | MTS24              | 9-10d             | 9-10d                    |
| 1030            | 1030       | CS20               | 7-10d             | 7-10d                    |
| Uplift SP       | Uplift SPF | Stud Plate Ties    | To Stud           | To Plate                 |
| 585             | 535        | SP1                | 8-10d             | 4-10d                    |
| 1065            | 805        | SP2                | 8-10d             | 8-10d                    |
| 771             | 771        | LST424             | 10-10d            | wrap under or over plate |
| 1235            | 1235       | LST424             | 14-10d            | wrap under or over plate |
| Uplift SP       | Uplift SPF | Holdowns @ Stemm   | To Stud / Post    | Anchor                   |
| 1625            | 1600       | DTT22              | 8-SDS 1/4"x1 1/2" | 12"x12" Titen HD         |
| 4235            | 3640       | HTT4               | 18-16x62 1/2"     | 12"x12" Titen HD         |
| Uplift SP       | Uplift SPF | Holdowns @ Mono    | To Stud / Post    | Anchor                   |
| 1625            | 1600       | DTT22              | 8-SDS 1/4"x1 1/2" | 12"x12" Titen HD         |
| 4235            | 3640       | HTT4               | 18-16x62 1/2"     | 12"x12" Titen HD         |
| Uplift SP       | Uplift SPF | Post Bases @ Stemm | To Post           | Anchor                   |
| 2200            |            | ABU44              | 12-16d            | 5/8"x12" Drill & Epoxy   |
| 2300            |            | ABU66              | 12-16d            | 5/8"x12" Drill & Epoxy   |
| Uplift SP       | Uplift SPF | Post Bases @ Mono  | To Post           | Anchor                   |
| 2200            |            | ABU44              | 12-16d            | 5/8"x12" Drill & Epoxy   |
| 2300            |            | ABU66              | 12-16d            | 5/8"x12" Drill & Epoxy   |

**EXTERIOR WALL STUD TABLE FOR SPSF #2 STUDS:**

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 12" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

|                  |                       |
|------------------|-----------------------|
| (1) 2x4 @ 16" OC | TO 10'-1" STUD HEIGHT |
| (1) 2x4 @ 12" OC | TO 11'-2" STUD HEIGHT |
| (1) 2x6 @ 16" OC | TO 15'-7" STUD HEIGHT |
| (1) 2x6 @ 12" OC | TO 17'-3" STUD HEIGHT |

**GRADE & SPECIES TABLE**

|      |              | Fb   | E   |
|------|--------------|------|-----|
| 2x8  | SP #2        | 925  | 1.4 |
| 2x10 | SP #2        | 800  | 1.4 |
| 2x12 | SP #2        | 750  | 1.4 |
| GLB  | 24F-V3 SP    | 2600 | 1.9 |
| LSL  | TIMBERSTRAND | 1700 | 1.7 |
| LVL  | MICROLAM     | 2950 | 2.0 |
| PSL  | PARALAM      | 2900 | 2.0 |

Gibraltar Contracting, LLC

Western Model  
Lot 27 Forest Country  
Mike & Julie Branch Res.

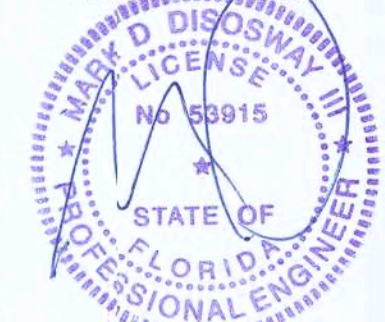
PROJECT ADDRESS:  
Lot 27 Forest Country  
Lake City, FL 32025

DIMENSIONS  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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LIMITATION: This design is valid for one building, at specified location.

MARI DISOWAY PE, 53915



Saturday, December 7, 2019

Mart Disoway P.E.  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disowaydesign@gmail.com

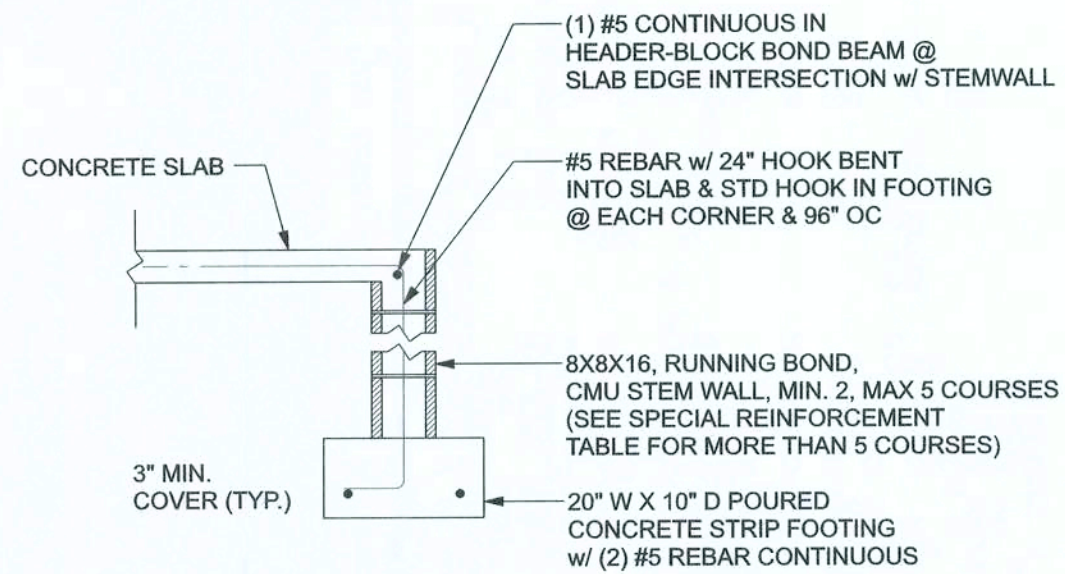
JOB NUMBER:  
191326

**S-1**  
OF 3 SHEETS

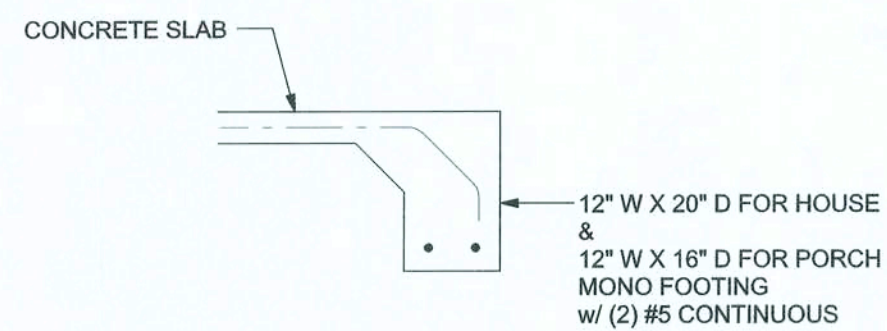
| DESIGN CRITERIA & LOADS:                     |   |
|--|---|
| BUILDING CODE                                | 6TH EDITION<br>FLORIDA BUILDING CODE RESIDENTIAL (2017) |
| CODE FOR DESIGN LOADS                        | ASCE 7-10   |
| <b>WINDLOADS</b>                             |   |
| BASIC WIND SPEED (ASCE 7-10, 3S GUST)        | 130 MPH   |
| WIND EXPOSURE (BUILDER MUST FIELD VERIFY)    | C   |
| TOPOGRAHY FACTOR (BUILDER MUST FIELD VERIFY) | 1   |
| RISK CATEGORY                                | II  |
| ENCLOSURE CLASSIFICATION                     | ENCLOSED  |
| INTERNAL PRESSURE COEFFICIENT                | 0.18  |
| ROOF ANGLE                                   | 7-45 DEGREES  |
| MEAN ROOF HEIGHT                             | 30 FT   |
| <b>C&amp;C DESIGN PRESSURES</b>              | SEE TABLE   |
| <b>FLOOR LOADING</b>                         |   |
| ROOMS OTHER THAN SLEEPING ROOM               | 40 PSF LIVE LOAD  |
| SLEEPING ROOMS                               | 30 PSF LIVE LOAD  |
| <b>ROOF LOADING</b>                          |   |
| FLAT OR < 4:12                               | 20 PSF LIVE LOAD  |
| 4:12 TO < 12:12                              | 16 PSF LIVE LOAD  |
| 12:12 & GREATER                              | 12 PSF LIVE LOAD  |
| <b>SOIL BEARING CAPACITY</b>                 | 1500 PSF  |
| <b>FLOOD ZONE</b>                            | THIS BUILDING IS NOT IN THE FLOOD ZONE                  |

| COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C) (Vult) |                 |                                       |
|---|-----------------|---------------------------------------|
| EFFECTIVE WIND AREA (Ft2)                                   | ZONE 4 INTERIOR | ZONE 5 END 4' FROM ALL OUTSIDE CORNER |
| 0 - 20  | +22.6 -46.2     | +42.6 -57                             |
| <b>GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C) (ASD)</b>   |                 |                                       |
| 167 GARAGE DOOR   | +22.6 -35.5     |                                       |
| 167 GARAGE DOOR   | +21.7 -34.1     |                                       |

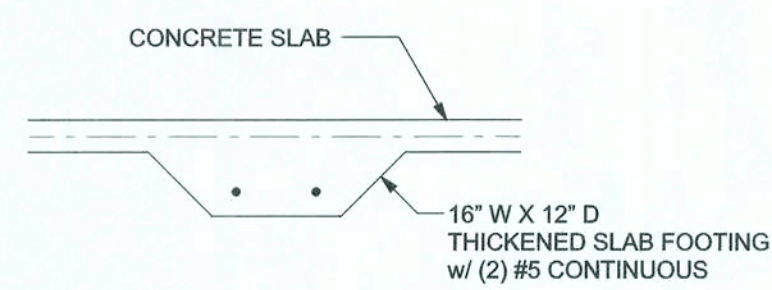




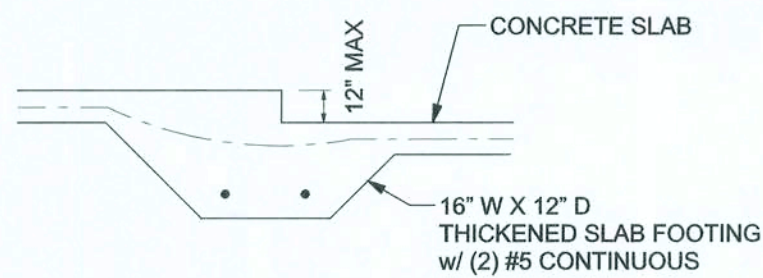
**F1 S-2** OPTIONAL STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"



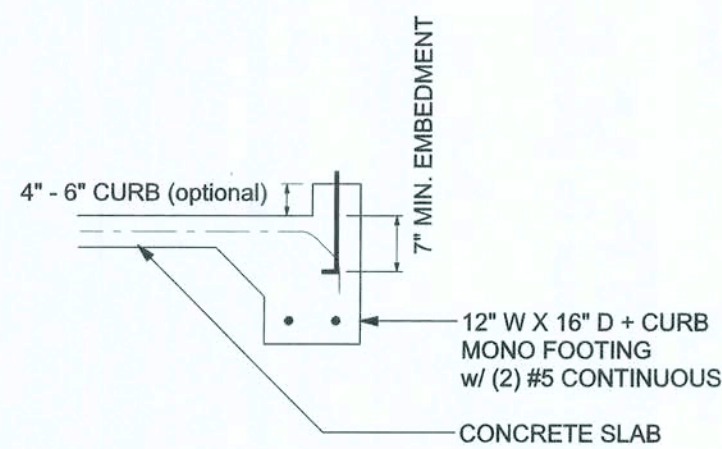
**F1 S-2** MONOLITHIC FOOTING  
SCALE: 1/2" = 1'-0"



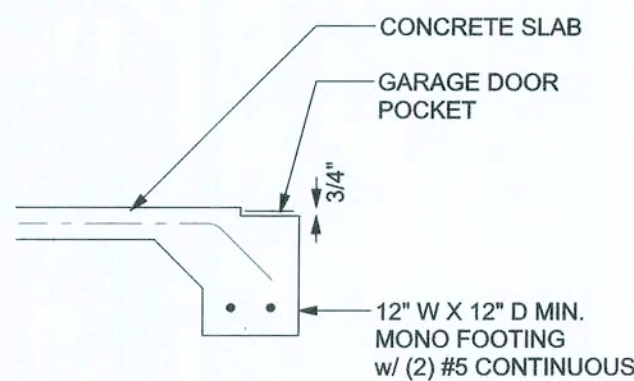
**F2 S-2** INTERIOR BEARING FOOTING  
SCALE: 1/2" = 1'-0"



**F3 S-2** INTERIOR BEARING STEP FOOTING  
SCALE: 1/2" = 1'-0"



**F4 S-2** MONOLITHIC CURB FOOTING  
SCALE: 1/2" = 1'-0"



**F5 S-2** GARAGE DOOR POCKET FOOTING  
SCALE: 1/2" = 1'-0"

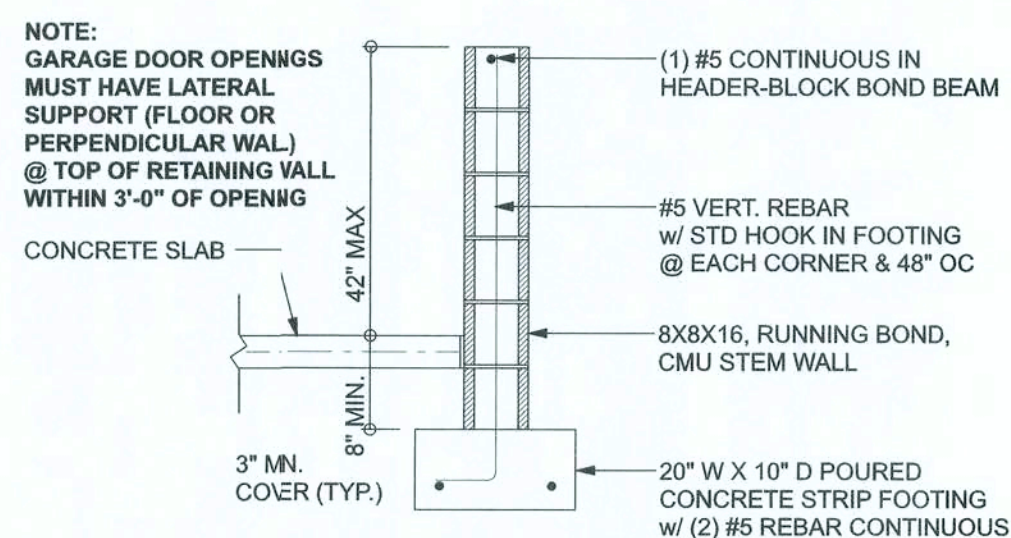
**TALL STEM WALL TABLE:**  
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with #5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

| STEM WALL HEIGHT (FEET) | UNBALANCED BACKFILL HEIGHT | VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.) |    |    | VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.) |    |    |
|-------------------------|----------------------------|---|----|----|--|----|----|
|                         |                            | #5  | #7 | #8 | #5   | #7 | #8 |
| 3.3                     | 3.0                        | 96  | 96 | 96 | 96   | 96 | 96 |
| 4.0                     | 3.7                        | 96  | 96 | 96 | 96   | 96 | 96 |
| 4.7                     | 4.3                        | 88  | 96 | 96 | 96   | 96 | 96 |
| 5.3                     | 5.0                        | 56  | 96 | 96 | 96   | 96 | 96 |
| 6.0                     | 5.7                        | 40  | 80 | 96 | 80   | 96 | 96 |
| 6.7                     | 6.3                        | 32  | 56 | 80 | 56   | 96 | 96 |
| 7.3                     | 7.0                        | 24  | 40 | 56 | 40   | 80 | 96 |
| 8.0                     | 7.7                        | 16  | 32 | 48 | 32   | 64 | 80 |
| 8.7                     | 8.3                        | 8   | 24 | 32 | 24   | 48 | 64 |
| 9.3                     | 9.0                        | 8   | 16 | 24 | 16   | 40 | 48 |

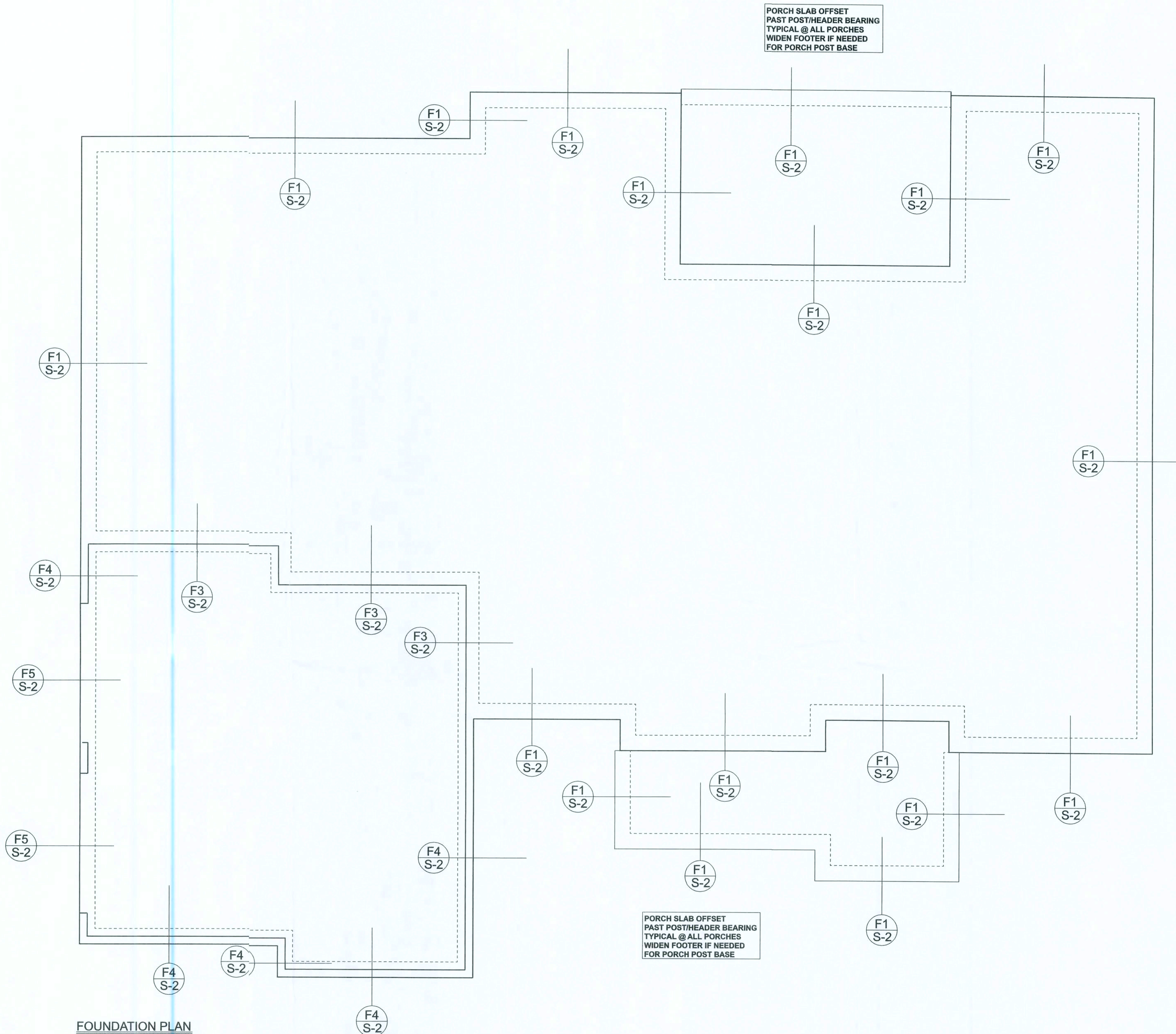
**MASONRY NOTE:**  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFIRM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

| ACI 530.1-02 Section                     | Specific Requirements   |
|--|---|
| 1.4A Compressive strength                | 8" block bearing walls $F_m = 1500$ psi   |
| 2.1 Mortar                               | ASTM C 270, Type N, UNO   |
| 2.2 Grout                                | ASTM C 476, admixtures require approval   |
| 2.3 CMU standard                         | ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block  |
| 2.3 Clay brick standard                  | ASTM C 216-02, Grade SW, Type FBS, S-2/S-2/S-11/S-2   |
| 2.4 Reinforcing bars, #3 - #11           | ASTM 615, Grade 40, $F_y = 40$ ksi, Lap splices min 40 bar dia. (25" for #5)  |
| 2.4F Coating for corrosion protection    | Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class 60, 0.80 oz/ft <sup>2</sup> or 304SS   |
| 2.4F Coating for corrosion protection    | Joint reinforcement in walls exposed to moisture or water, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft <sup>2</sup> or 304SS |
| 3.3.E.2 Pipes, conduits, and accessories | Any not shown on the project drawings require engineering approval.   |
| 3.3.E.7 Movement joints                  | Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.   |

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL PER FBC 2017 RES. SECTION R403.1.4



**F4 S-2** OPTIONAL STEM WALL CURB FOOTING  
SCALE: 1/2" = 1'-0"



#### FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

**FOUNDATION NOTES**

1. DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
2. CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
3. THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ 6x6-1/4" WELDED WIRE MESH PLACED ON CHAIRS #1 1/2" DEPTH OR FIBER MESH CONCRETE 65-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITES-TREATED & COMPACTED FILL.

Gibraltar Contracting, LLC

Western Model  
Lot 27 Forest Country  
Mike & Julie Branch Res.

PROJECT ADDRESS:  
Lot 27 Forest Country  
Lake City, FL 32025

**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53845



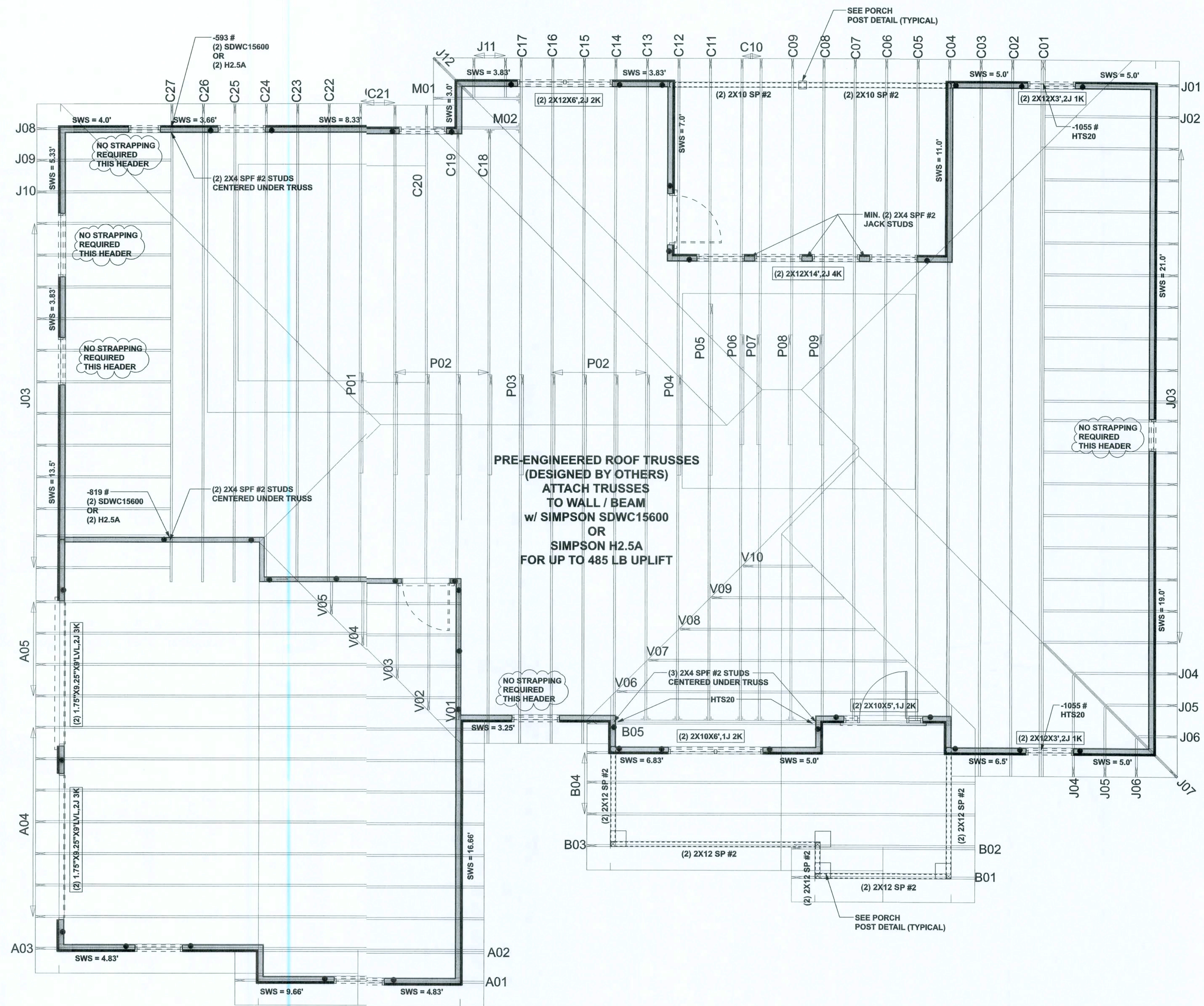
Saturday, December 7, 2019

Mark Disoway P.E.  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disowaydesign@gmail.com

JOB NUMBER:  
19-326

**S-2**  
OF 3 SHEETS





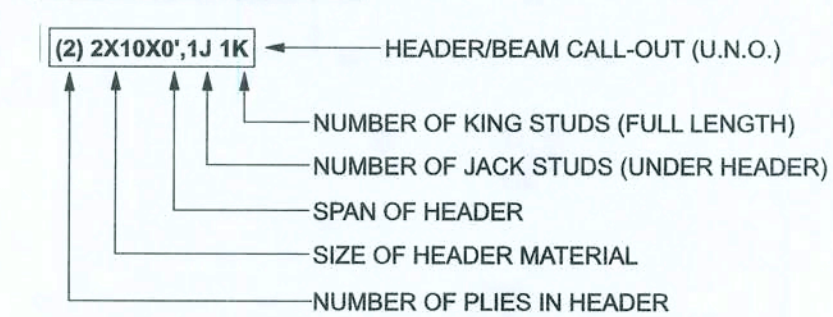
### STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

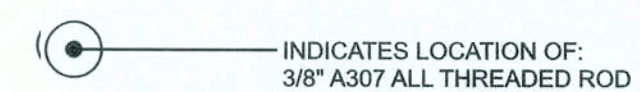
### STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-4 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-5 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

### HEADER LEGEND



### THREADED ROD LEGEND



### ACTUAL vs REQUIRED SHEARWALL

|          | TRANSVERSE | LONGITUDINAL |
|----------|------------|--------------|
| ACTUAL   | 21674 LBF  | 19550 LBF    |
| REQUIRED | 15031 LBF  | 12593 LBF    |

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND TRUSS CO. JOB #19-3718

Gibraltar Contracting, LLC

Western Model  
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PROJECT ADDRESS:  
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Lake City, FL 32025

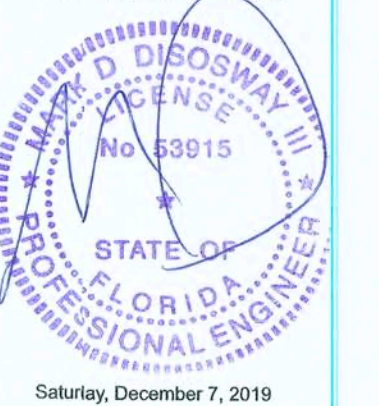
DIMENSIONS:  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARKDISOSWAY P.E.-63915



Saturday, December 7, 2019

Mark Disosway P.E.  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disoswaydesign@gmail.com

JOB NUMBER:  
191326

S-3  
OF 3 SHEETS