

REVISIONS	December 03, 2007

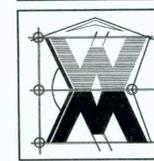


TYPICAL WALL SECTION  
SCALE: 1" = 1'-0"

FRONT & REAR ELEVATION  
SCALE: 1/4" = 1'-0"

A CUSTOM RESIDENCE FOR:  
**BUDDY & BRENDA KENNEDY**  
PROJECT ADDRESS: LOT 6 DEER RUN, COLUMBIA COUNTY, LAKE CITY, 32055

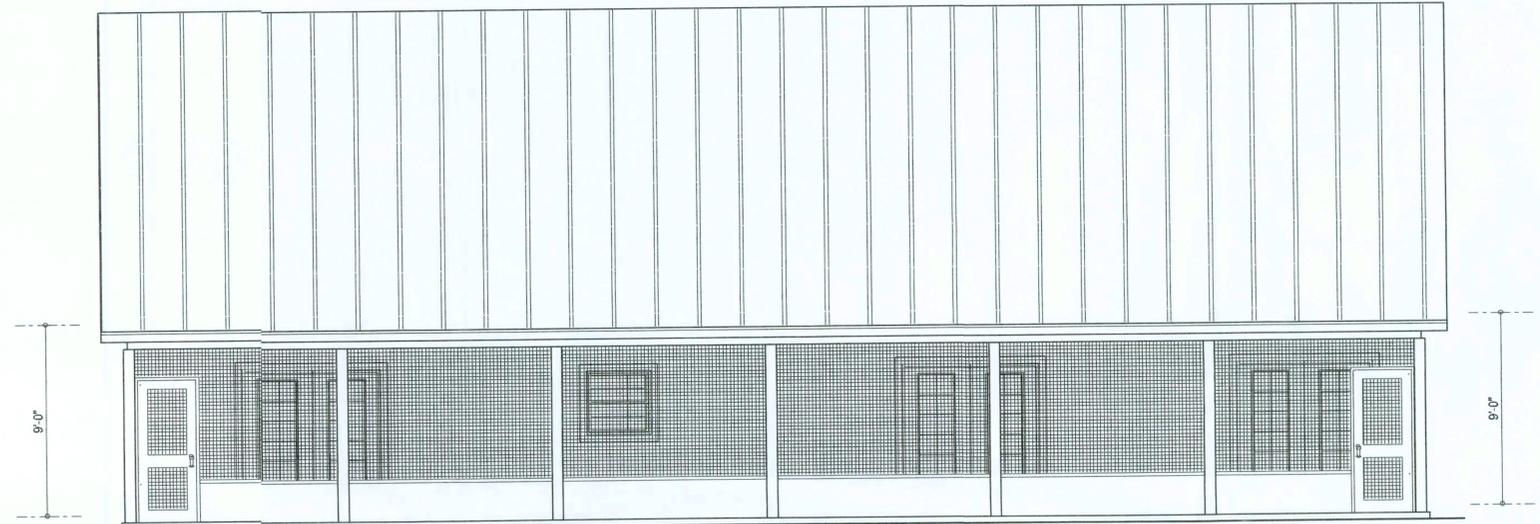
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P.O. BOX 1513  
LAKE CITY, FL 32055  
(386) 758-8406  
will@willmyers.net



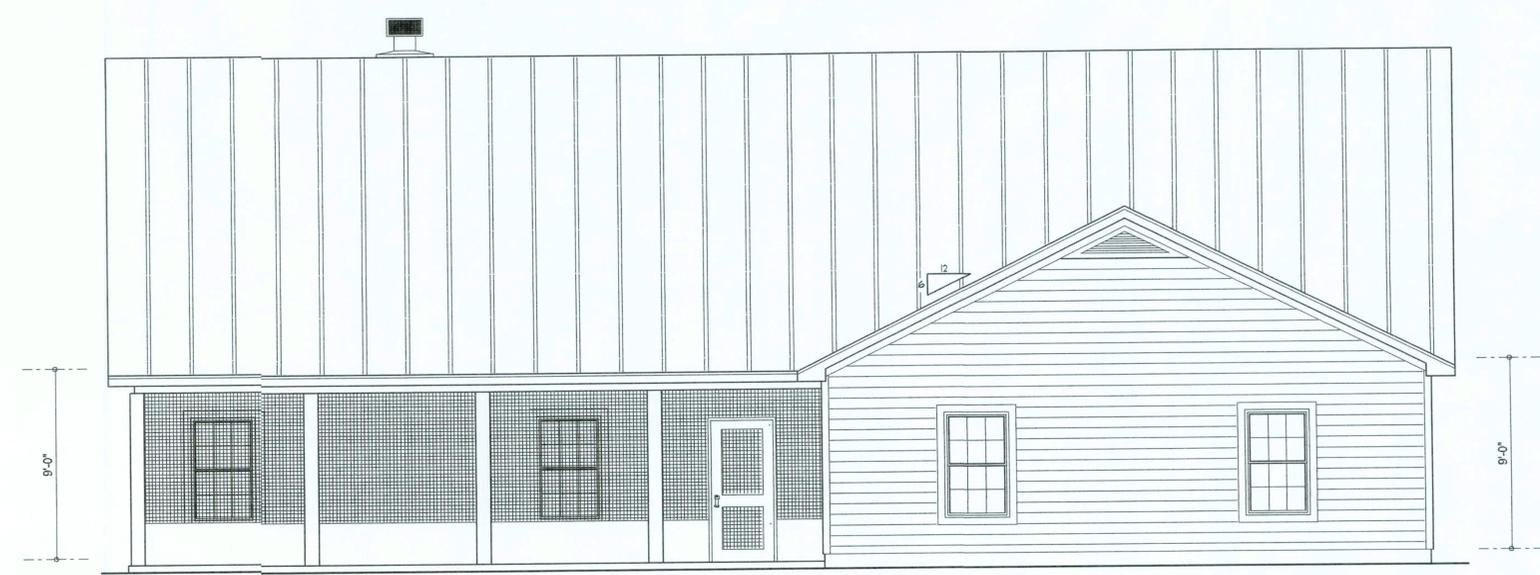
JOB NUMBER  
071110

SHEET NUMBER  
**A.1**  
OF 4 SHEETS

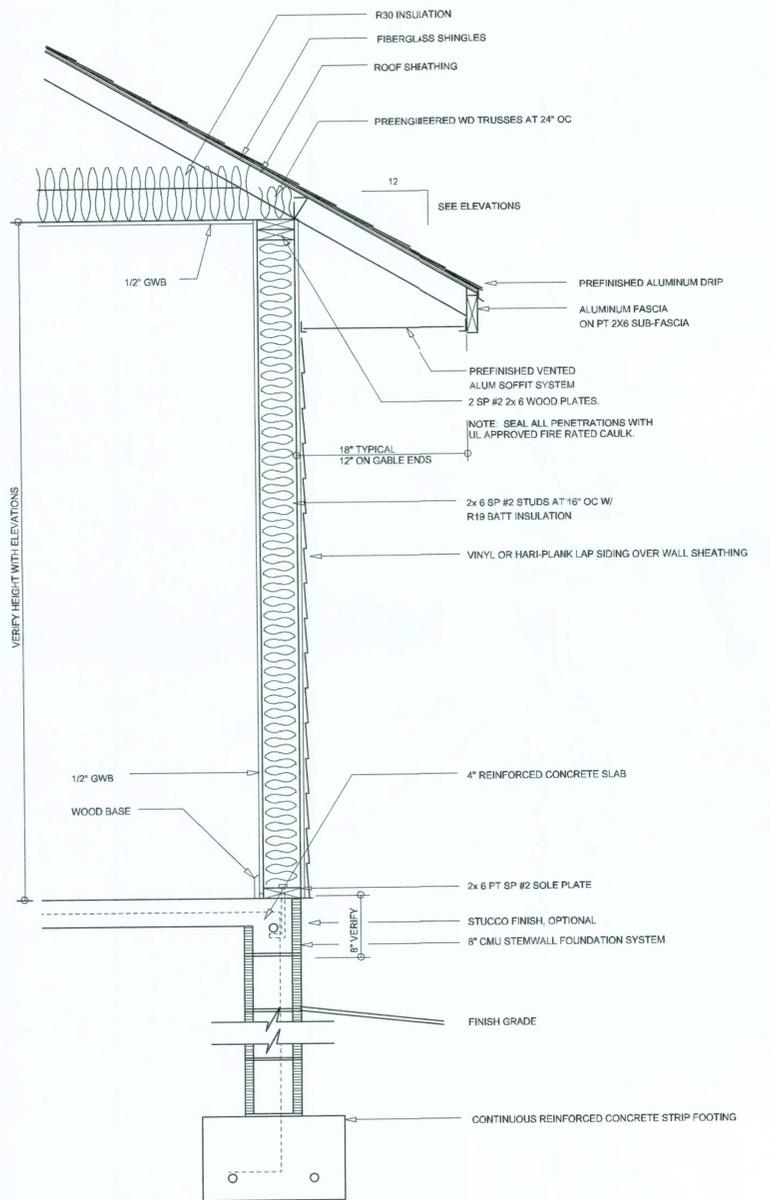
*William Myers*



**REAR ELEVATION**  
SCALE: 1/4" = 1'-0"



**FRONT ELEVATION**  
SCALE: 1/4" = 1'-0"



**TYPICAL WALL SECTION**  
SCALE: 1" = 1'-0"

VERIFY HEIGHT WITH ELEVATIONS



**LEFT ELEVATION**  
SCALE: 1/4" = 1'-0"



**RIGHT ELEVATION**  
SCALE: 1/4" = 1'-0"

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December 03, 2007



**LEFT ELEVATION**  
SCALE: 1/4" = 1'-0"

**RIGHT ELEVATION**  
SCALE: 1/4" = 1'-0"

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JOB NUMBER  
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**A.2**  
OF 4 SHEETS

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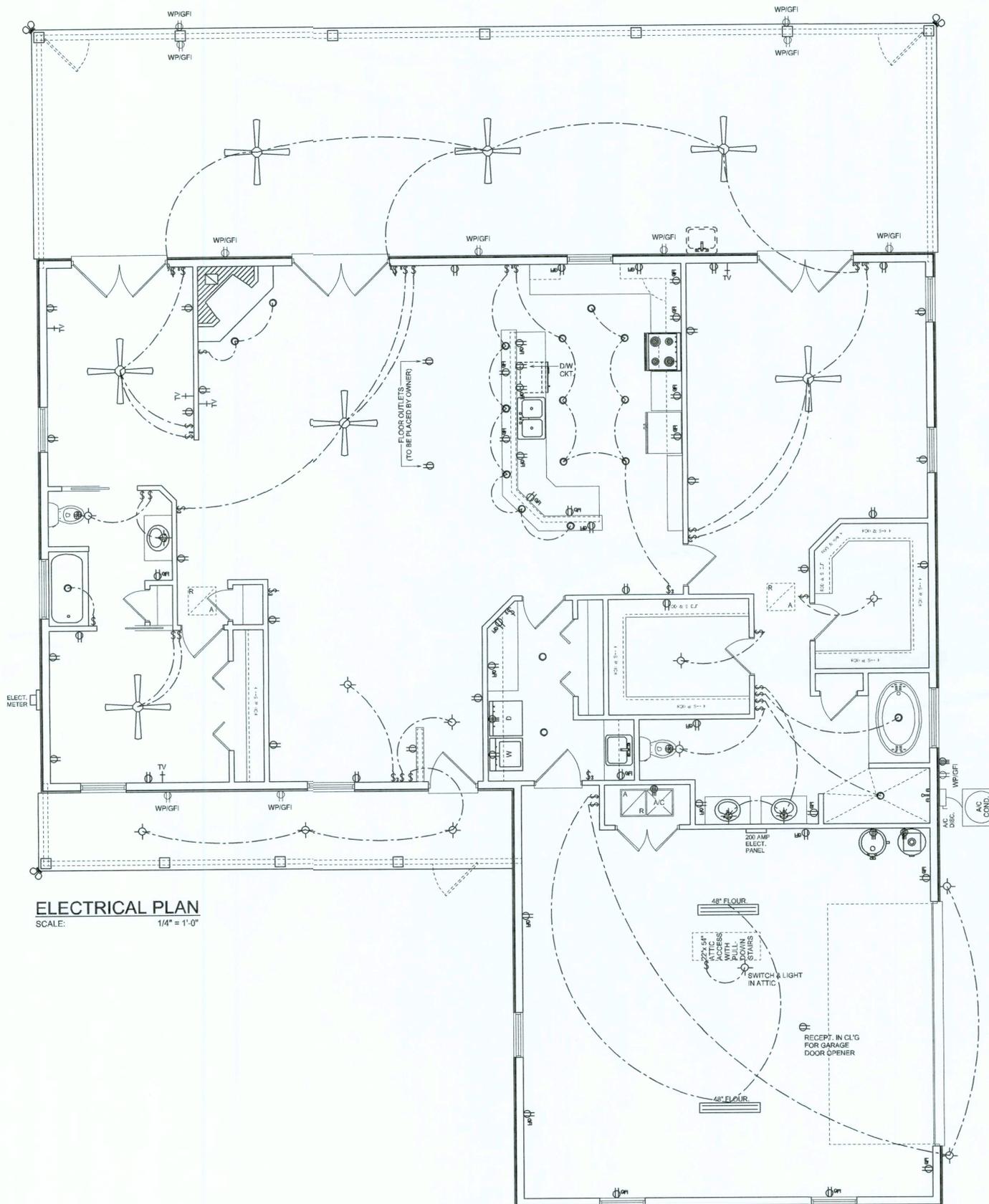


ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220V OUTLET
	GFI DUPLEX OUTLET
	TELEVISION JACK
	TELEPHONE JACK
	SMOKE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	2 OR 4 TUB FLUORESCENT FIXTURE

NOTE:  
ALL BEDROOM RECEPTACLES SHALL BE AFCI  
(ARC FAULT CIRCUIT INTERRUPT)

ALL SMOKE DETECTORS SHALL HAVE BATTERY BACKUP POWER  
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY  
ALL ACTIVATE

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE  
INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS.  
CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB  
PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR  
SHALL BE USED AS AN EQUIPMENT GROUND.



**ELECTRICAL PLAN**  
SCALE: 1/4" = 1'-0"

REVISIONS
December 03, 2007

**SOFTPLAN**  
FOR ARCHITECTURAL DESIGN SOFTWARE

**ELECTRICAL PLAN**  
SCALE: 1/4" = 1'-0"

A CUSTOM RESIDENCE FOR:  
**BUDDY & BRENDA KENNEDY**  
PROJECT ADDRESS: LOT 6 DEER RUN, COLUMBIA COUNTY, LAKE CITY, 32085

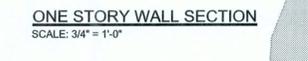
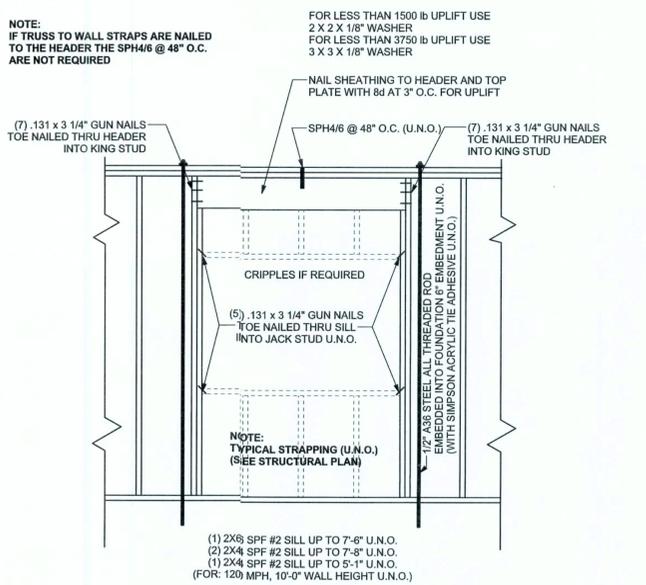
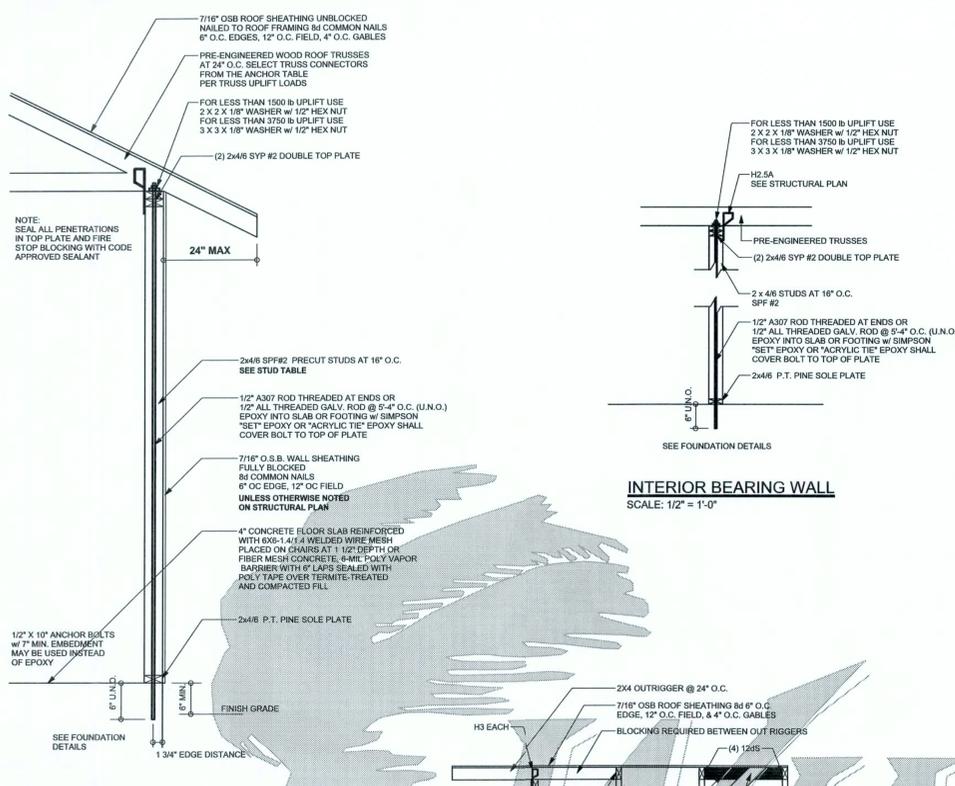
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**A.4**  
OF 4 SHEETS

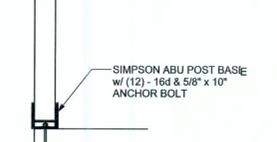
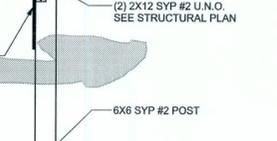
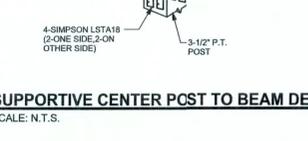
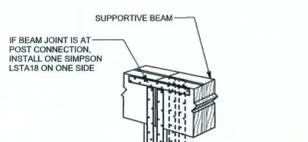
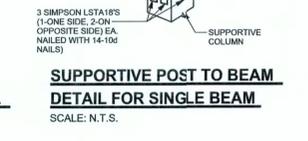
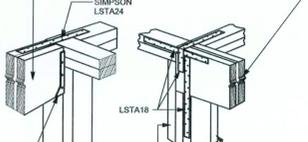
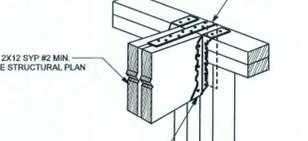
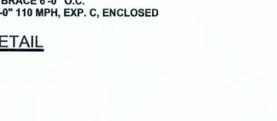
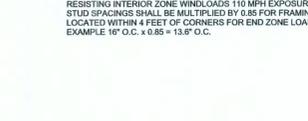
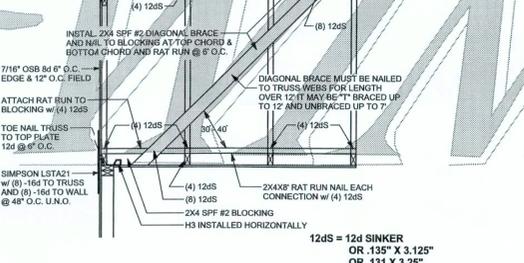
*William Myers*



**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS**

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.88 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.88 = 13.8" O.C.

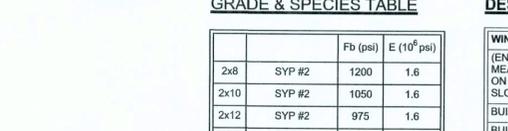


**ANCHOR TABLE**  
OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 850	< 820	H8	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 455	< 650	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2000	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14-16d	14-16d	
<b>HEAVY GIRDER TIEDOWNS*</b>					
< 3965	< 3330	MGT		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
<b>STUD STRAP CONNECTOR*</b>					
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 800	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4		6-10d, 1 1/2"	
< 1240	< 1065	SPH4		10-10d, 1 1/2"	
< 885	< 760	SP6		6-10d, 1 1/2"	
< 1240	< 1065	SPH6		10-10d, 1 1/2"	
< 1235	< 1165	LSTA18		14-10d	
< 1235	< 1235	LSTA21		16-10d	
< 1030	< 1030	CS20		18-8d	
< 1705	< 1705	CS16		28-8d	
<b>STUD ANCHORS*</b>					
< 1350	< 1305	LTT19		8-16d	1/2" AB
< 2310	< 2310	LTT31		18-10d, 1 1/2"	1/2" AB
< 2775	< 2570	HD2A		2-5/8" BOLTS	5/8" AB
< 4175	< 3885	HTT16		18-16d	5/8" AB
< 1400	< 1400	PAV4D2		16-16d	
< 3335	< 3335	HPAHD22		16-16d	
< 2200	< 2200	ABU44		12-16d	1/2" AB
< 2390	< 2300	ABU66		12-16d	1/2" AB
< 2320	< 2320	ABU88		18-16d	2-5/8" AB

**GRADE & SPECIES TABLE**

	Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200 1.6
2x10	SYP #2	1050 1.6
2x12	SYP #2	975 1.6
GLB	24F-V3 SP	2400 1.8
LVL	MICROLAM	2900 2.0
PSL	PARALAM	2900 2.0



**WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1**  
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT.; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT. IN EXP. B, 30 FT. IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1) BASIC WIND SPEED = 110 MPH

2) WIND EXPOSURE = B

3) WIND IMPORTANCE FACTOR = 1.0

4) BUILDING CATEGORY = II

5) ROOF ANGLE = 10-45 DEGREES

6) MEAN ROOF HEIGHT = <30 FT

7) INTERNAL PRESSURE COEFFICIENT = NA (ENCLOSED BUILDING)

8) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft <sup>2</sup> )	10	15	20
1	19.9	21.8	18.1	-18.1
2	19.9	25.5	18.1	-21.8
2 Oth	-	-40.8	-	-40.8
3	19.9	25.5	18.1	-21.8
3 Oth	-	-68.3	-	-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
Doors & Windows Worst Case (Zone 5, 10 ft <sup>2</sup> )		21.8	-29.1	
8x7 Garage Door		19.5	-22.9	
10x7 Garage Door		18.5	-21.0	

**DESIGN LOADS**

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3-12)
ROOF	20 PSF (FLAT OR <4-12)
	16 PSF (4-12 TO <12-12)
	12 PSF (12-12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TRUSS CONNECTIONS AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS ENGINEERING FULLY SATISFIES ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. TRUSS RAFTERS 700 LB EACH END, CONNECTION 415 LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOIL TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, F<sub>y</sub> = 80ksi, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A188, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT W/ OR CROW FOOT JOINTS. LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F<sub>y</sub> = 60 KSI. ALL LAP SPICES 40" DB (25" FOR #5 BARS), UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-98, UNO.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F<sub>b</sub> = 2.4ksi, E = 1800ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC. ALL ROOFERS PER HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (13), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICE OF THE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A 307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 3" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 9/16", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

**BUILDER'S RESPONSIBILITY**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

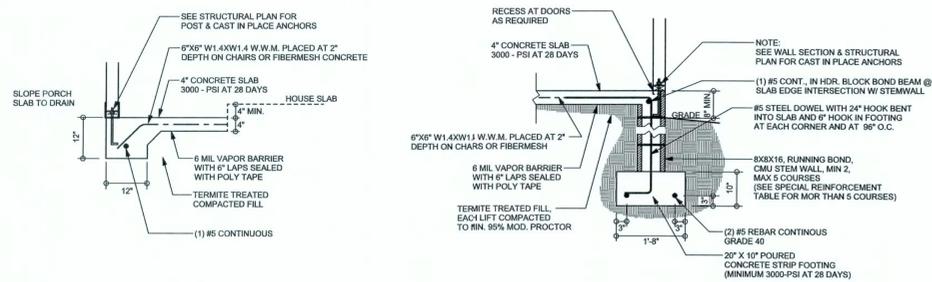
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM

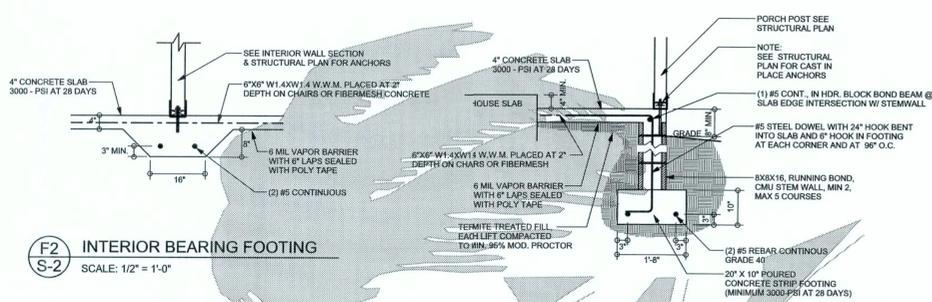
REVISIONS	

**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE



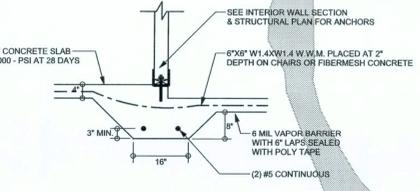
**F5 PORCH FOOTING**  
SCALE: 1/2" = 1'-0"

**F9 STEM WALL FOOTING**  
SCALE: 1/2" = 1'-0"

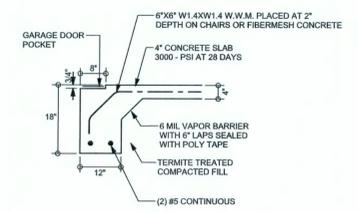


**F2 INTERIOR BEARING FOOTING**  
SCALE: 1/2" = 1'-0"

**F12 ALT. STEM WALL PORCH FOOTING**  
SCALE: 1/2" = 1'-0"



**F3 INTERIOR BEARING STEP FOOTING**  
SCALE: 1/2" = 1'-0"

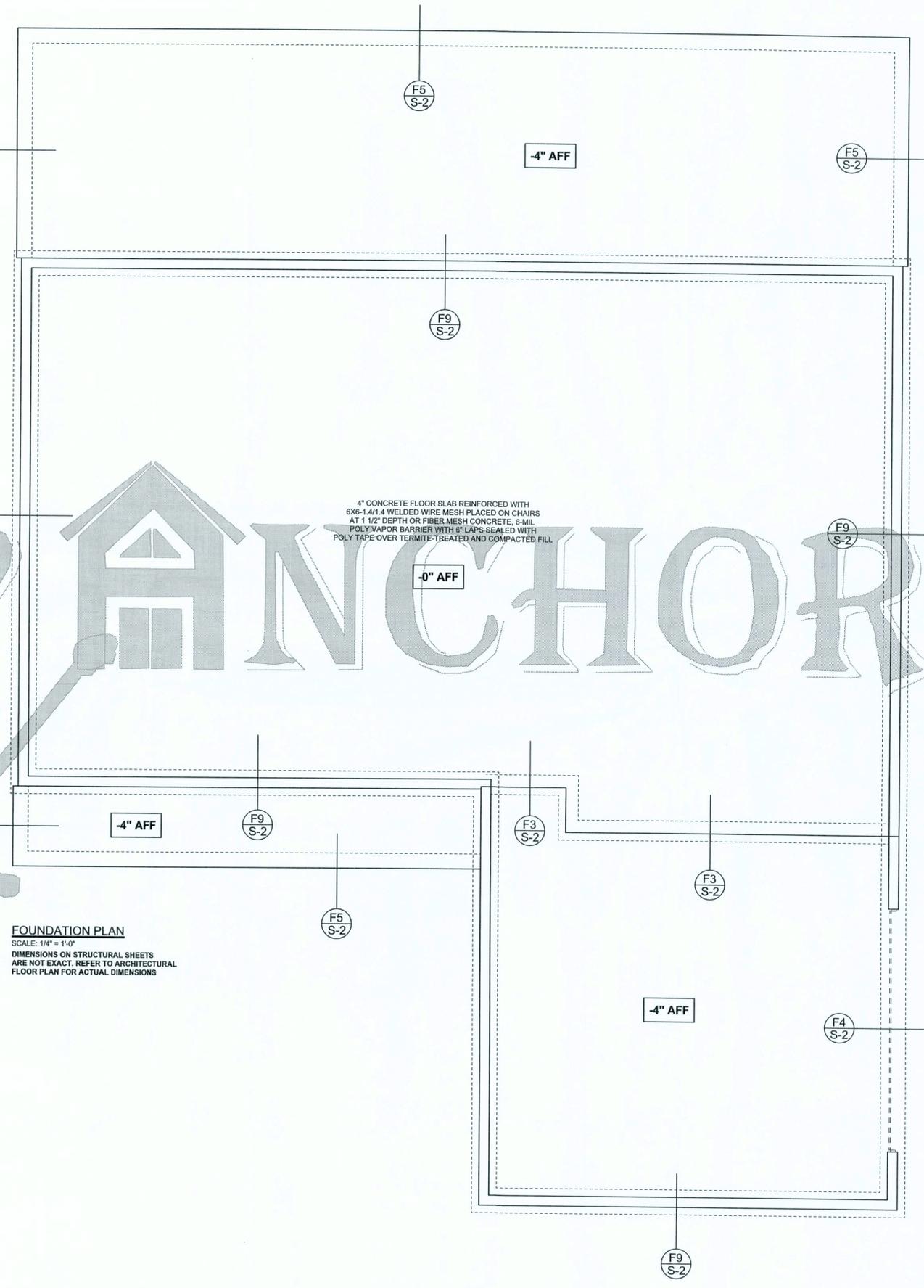


**F4 GARAGE DOOR FOOTING**  
SCALE: 1/2" = 1'-0"

**TALL STEM WALL TABLE**

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 18"OC vertically or a horizontal bond beam with 160 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



**FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

**WINDLOAD ENGINEER:** Mark Disoway, PE No. 53815, P.O. Box 868, Lake City, FL 32056, 386-754-5419

**DIMENSIONS:** Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section F301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53815  
*[Signature]*  
SEAL

**Seth Heitzman Construction**

**Buddy & Brenda Kennedy Residence**

ADDRESS:  
Lot 6 Deer Run S/D  
Columbia County, Florida

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P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE:  
December 14, 2007

DRAWN BY: David Disoway      CHECKED BY:

FINALS DATE:  
14 / Dec / 07

JOB NUMBER:  
712136

DRAWING NUMBER  
**S-2**  
OF 3 SHEETS

REVISIONS	



**STRUCTURAL PLAN NOTES**

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03. BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER WITH THE SEALED TRUSS PACKAGE

**WALL LEGEND**

	1ST FLOOR EXTERIOR WALL
	2ND FLOOR EXTERIOR
	1ST FLOOR INTERIOR BEARING WALL
	2ND FLOOR INTERIOR BEARING WALL

**THREADED ROD LEGEND**

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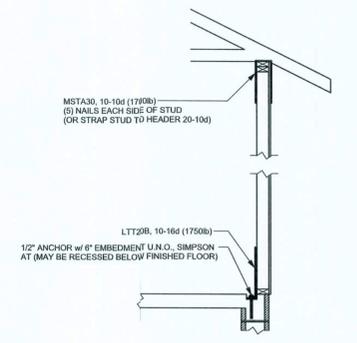
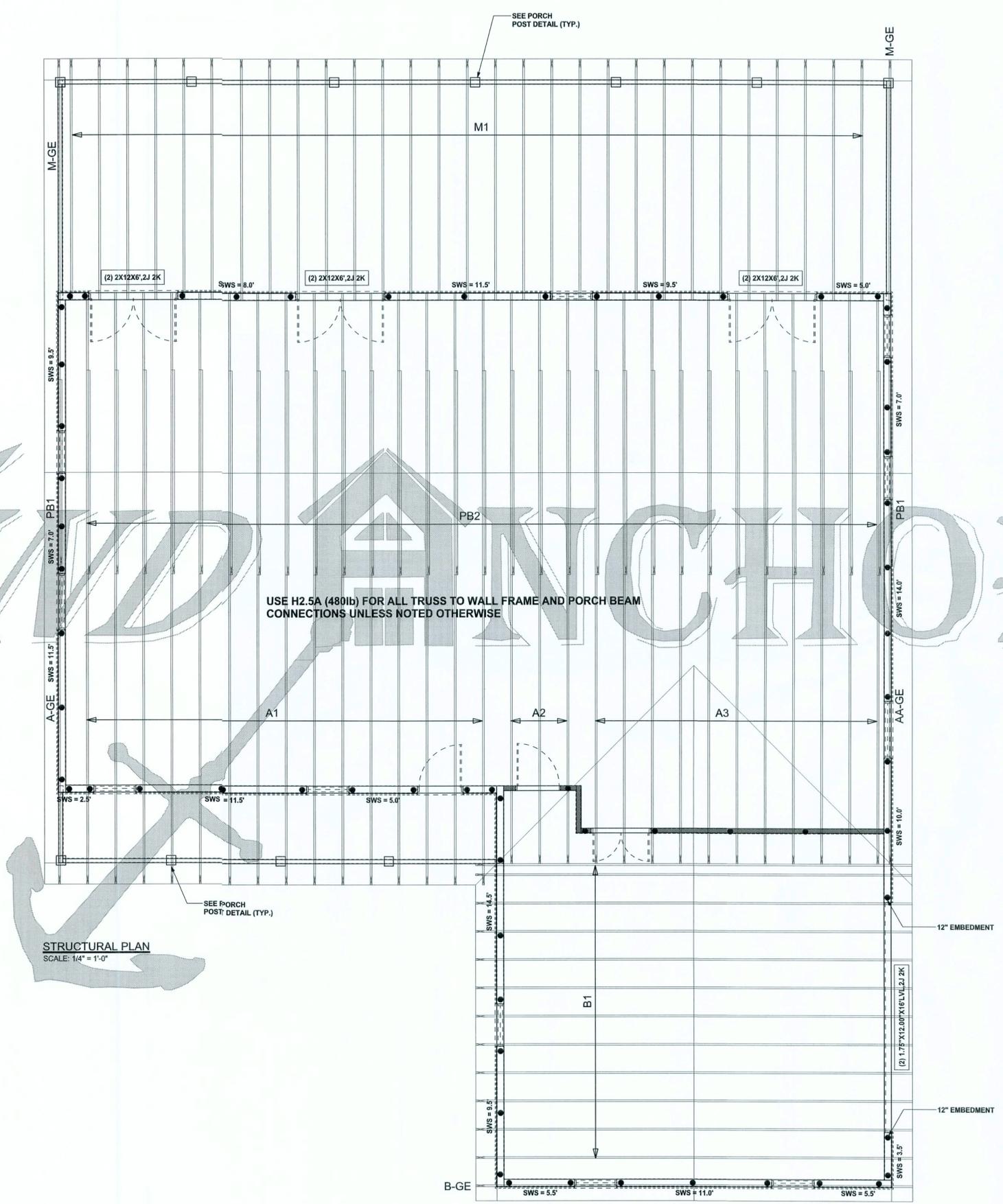
**HEADER LEGEND**

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**TOTAL SHEAR WALL SEGMENTS**

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	33.6'	86.5'
LONGITUDINAL	38.2'	75.0'



**ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED IN WALL.**  
SCALE: 1/2" = 1'-0"

**USE H2.5A (480lb) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE**

**STRUCTURAL PLAN**  
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 33915, PCB 888, Lake City, FL 32056, 386-754-5419

**DIMENSIONS:**  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 33915  
*Mark Disoway*  
14 Dec 07  
SEAL

**Seth Heitzman Construction**

Buddy & Brenda Kennedy Residence

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PRINTED DATE:  
December 14, 2007

DRAWN BY:  
David Disoway

CHECKED BY:

FINALS DATE:  
14 / Dec / 07

**JOB NUMBER:**  
712136

DRAWING NUMBER  
**S-3**

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, ANDERSON TRUSS JOB #7-389