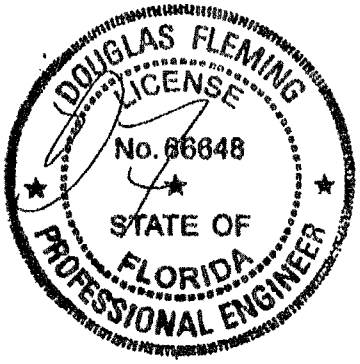


ALPINE
AN ITW COMPANY

COA #0 278
Florida Certificate of Product Approval #FL1999
02/20/2025

Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone (800)755-6001
www.alpineitw com

This item has been digitally signed by Douglas Fleming on the date adjacent to the seal

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

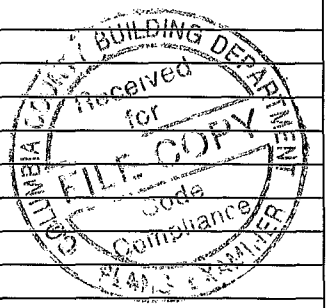
Site Information:	Page 1:
Customer: W B Howland Company, Inc	*Job Number:* 25-2238
Job Description 138 MERCURY CT	
Address 138 NW Mercury Ct, Lake City, FL 32025	

Job Engineering Criteria:	
Design Code FBC 8th Ed 2023 Res	*IntelliVIEW Version* 23 02.04 *JRef #.* 1Y7N2150006
Wind Standard ASCE 7-22 *Wind Speed (mph)* 130 *Building Type* Closed	*Design Loading (psf)* 40.00

This package contains general notes pages, 40 truss drawing(s) and 3 detail(s)

Item	Drawing Number	Truss
1	050 25 1406.24270	A01
3	050.25 1406 27253	A03
5	050.25 1406.52960	B02
7	050.25 1406 56847	C01
9	050.25 1407 07760	D01
11	050.25 1407 11550	D03
13	050 25.1407 24643	G02
15	050.25 1407.28163	G04
17	050 25.1407.31353	G06
19	050 25 1407.34543	G08
21	050 25.1407 37650	G10
23	050.25 1407.40603	G12
25	050.25 1408.05537	G14
27	050.25 1408 16657	HJ01
29	050 25.1408.33387	J02
31	050.25 1408.43500	J04
33	050.25.1408.46800	J06
35	050.25.1408.54273	J08
37	050 25.1408 57687	J10
39	050.25.1409 02097	PB02
41	BRCLBSUB0119	
43	PB160220723	

Item	Drawing Number	Truss
2	050.25 1406.25733	A02
4	050.25 1406.29543	B01
6	050.25 1406 55080	B03
8	050 25.1406 58273	C02
10	050.25.1407 09703	D02
12	050 25 1407.22880	G01
14	050.25 1407.26463	G03
16	050 25.1407 29850	G05
18	050.25 1407 32877	G07
20	050.25 1407 35990	G09
22	050 25 1407 39177	G11
24	050 25.1407.41990	G13
26	050.25.1408 15150	G15
28	050.25 1408.18023	HJ02
30	050.25 1408.41030	J03
32	050.25 1408.45203	J05
34	050 25.1408.52567	J07
36	050.25 1408.55993	J09
38	050.25 1409.00597	PB01
40	050.25 1409 05447	PB03
42	CNNAILSP1014	



General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:
The bearing area factor, Cb, is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:
Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
C = Coated lumber.
C-AT = AtTEK coated lumber.
C-FX = FX Lumber Guard coated lumber.
C -TE = TechWood 4400 coated lumber.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-BF = Boraflame Fire Retardant Treated lumber
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-ON = OnWood Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
FRT-PR = ProWood Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

General Notes (continued)

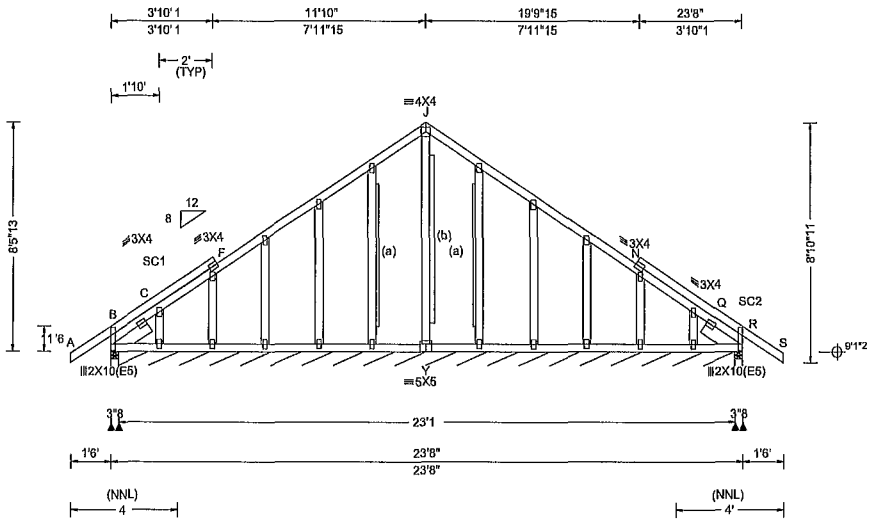
Key to Terms (continued):
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com

SEQN 800155 GABL FROM. CDM	Ply· 1 Qty 1	**Job Number·** 25-2238 138 MERCURY CT **Truss Label:** A01	Cust: R 215 JRef·1Y7N2150006 T2 DrwNo. 050 25 1406.24270 KD / DF 02/19/2025	



Loading Criteria (psf)	
TCLL	20 00
TCDL	10 00
BCLL.	0.00
BCDL	10.00
Des Ld	40 00
NCBCLL	10 00
Soffit:	2.00
Load Duration	1 25
Spacing	24 0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category· II
EXP B Kzt NA
Mean Height 15 00 ft
TCDL 5 0 psf
BCDL 5 0 psf
MWFRS Parallel Dist. 0 to h/2
C&C Dist a 3 00 ft
Loc from endwall Any
GCpi 0 18
Wind Duration 1 60

Snow Criteria (Pg Pf In PSF)
Pg NA Ct NA CAT NA
Pf· NA Ce NA
Lu NA Cs NA
Snow Duration NA
Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac. Yes
FT/RT.20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria				
PP Deflection in	loc	L/defl	L/#	
VERT(LL)	0.001	F	999	240
VERT(CL)	0.003	F	999	180
HORZ(LL)	0.003	R	-	-
HORZ(TL)	0.004	R	-	-
Creep Factor· 2.0				
Max TC CSI	0.245			
Max BC CSI	0.029			
Max Web CSI	0.906			
VIEW Ver· 23 02 04 0123 14				

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	284	/-	/-	/138	/7	/180
B*	70	/-	/-	/40	/3	/-
R	284	/-	/-	/141	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 1 5 (Truss)
B Brg Wid = 277 Min Req = -
R Brg Wid = 3 5 Min Req = 1.5 (Truss)
Bearings B, B, & R are a rigid surface
Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Stack Chord SC1 2x4 SP #2,
Stack Chord SC2 2x4 SP #2,
Lt Slider· 2x6 SP #2, block length = 1 500'
Rt Slider· 2x6 SP #2, block length = 1 500

Plating Notes

All plates are 2X4 except as noted

Loading

Gable end supports 8" max rake overhang Top chord must not be cut or notched

Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Gable meets L/120 deflection criteria for wind load applied to face Calculated deflection ratio is L/200

Gable Reinforcement

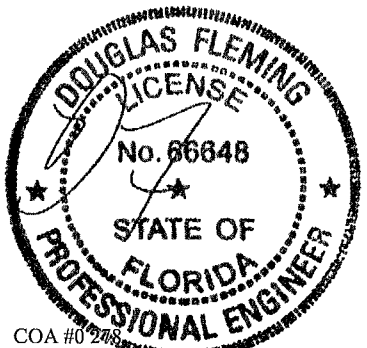
(a) 1x4 "L" reinforcement Any species and grade 80% length of web member Attach with 10d (0 131"x3",min) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder
(b) 2x4 "L" reinforcement. Any species and grade 80% length of web member Attach with 10d (0 131"x3",min) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" oc intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

The overall height of this truss excluding overhang is 8-5-13



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02/20/2025

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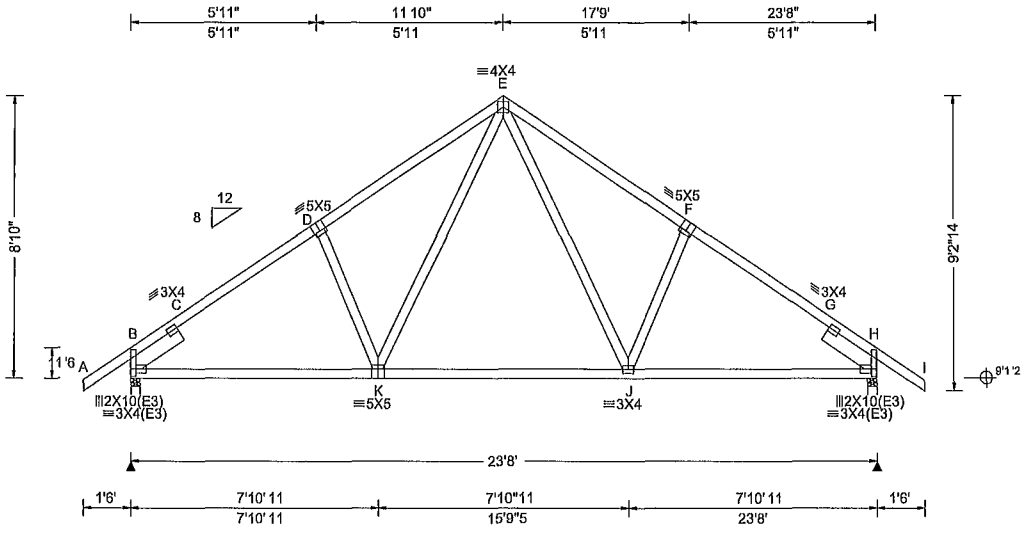
Trusses require extreme care in fabricating, handling, shipping, installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable, Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise Refer to drawings 160A-Z for standard plate positions Refer to job's General Notes page for additional information
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2
For more information see these web sites Alpine alpineitw com, TPI tpinst.org, SBCA sbcacomponents com, ICC iccsafe org, AWC awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800158 COMN FROM CDM	Ply 1 Qty 8	**Job Number** 25-2238 138 MERCURY CT **Truss Label** A02	Cust R 215 JRef 1Y7N2150006 T1 DrwNo 050.25 1406.25733 KD / DF 02/19/2025	



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg Pf In PSF)	**Defl/CSI Criteria**
TCLL	20.00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#
TCDL.	10.00	Speed 130 mph	Pf NA Ce NA	VERT(LL) 0 050 J 999 240
BCLL	0 00	Enclosure Closed	Lu NA Cs NA	VERT(CL) 0.096 J 999 180
BCDL.	10 00	Risk Category II	Snow Duration NA	HORZ(LL) 0 027 G - -
Des Ld	40 00	EXP B Kzt NA		HORZ(TL) 0 051 G - -
NCBCLL	10.00	Mean Height 15 00 ft	Building Code	Creep Factor 2 0
Soffit:	2 00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0.377
Load Duration	1 25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0 685
Spacing	24.0 "	MWFRS Parallel Dist 0 to h/2	Rep Fac. Yes	Max Web CSI 0 198
		C&C Dist a 3 00 ft	FT/RT 20(0)/10(0)	
		Loc from endwall Any	Plate Type(s)	
		GCpi 0 18	WAVE	VIEW Ver 23 02 04 0123 14
		Wind Duration 1 60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1182	/-	/-	/633	/31	/183
H	1182	/-	/-	/633	/31	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 1.5 (Truss)
H Brg Wid = 3 5 Min Req = 1.5 (Truss)
Bearings B & H are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - C	168	-1538	E - F	239	-1306
C - D	168	-1420	F - G	167	-1421
D - E	240	-1305	G - H	167	-1539

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - K	1101	-34	J - H	1102	-36
K - J	786	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens	Comp	Webs	Tens	Comp
K - E	517	-79	E - J	520	-78

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Lt Slider 2x6 SP #2, block length = 1 947'
Rt Slider 2x6 SP #2, block length = 1 947'

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

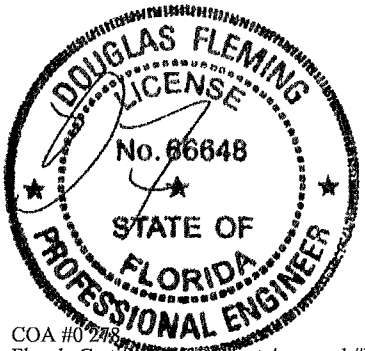
Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 8-10-0



COA #0 2[illegible]
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For more information see these web sites Alpine alpineitw com, TPI tpinst.org, SBCA. sbcacomponents com, ICC iccsafe.org, AWC awc.org

ALPINE
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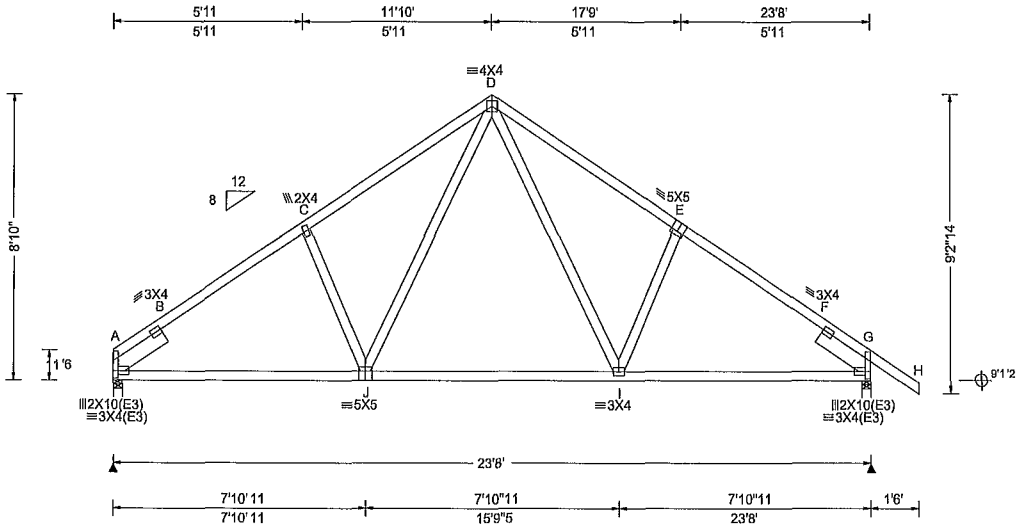
155 Harlem Ave
North Building, 4th Floor
Glenview IL 60025

SEQN 800161 COMN FROM CDM	Ply 1 Qty 3	**Job Number:** 25-2238 138 MERCURY CT **Truss Label** A03	Cust: R 215 JRef 1Y7N2150006 T3 DrwNo 050.25.1406.27253 KD / DF 02/19/2025	



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg Pf in PSF)	**Defl/CSI Criteria**
TCLL.	20.00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#
TCDL	10 00	Speed 130 mph	Pf NA Ce NA	VERT(LL) 0 053 I 999 240
BCLL	0.00	Enclosure Closed	Lu NA Cs NA	VERT(CL) 0 102 I 999 180
BCDL	10 00	Risk Category II	Snow Duration NA	HORZ(LL) 0 030 F - -
Des Ld	40.00	EXP B Kzt: NA		HORZ(TL) 0 057 F - -
NCBCLL	10.00	Mean Height: 15 00 ft	Building Code	Creep Factor 2 0
Soffit	2.00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0 507
Load Duration	1 25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0 821
Spacing	26 0 "	MWFRS Parallel Dist h to 2h	Rep Fac No	Max Web CSI 0 221
		C&C Dist a 3 00 ft	FT/RT 20(0)/10(0)	
		Loc from endwall not in 9.67 ft	Plate Type(s)	
		GCpi 0 18	WAVE	VIEW Ver 23.02 04.0123 14
		Wind Duration 1 60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	1165	/-	/-	/607	/-	/184
G	1284	/-	/-	/686	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 3 5 Min Req = 1 5 (Truss)
G Brg Wid = 3.5 Min Req = 1 5 (Truss)
Bearings A & G are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
A - B	104	-1700	D - E	159	-1420
B - C	98	-1552	E - F	90	-1545
C - D	167	-1431	F - G	98	-1672

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
A - J	1212	0	I - G	1199	0
J - I	856	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens	Comp	Webs	Tens	Comp
J - D	579	-46	D - I	563	-42

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Lt Slider 2x6 SP #2, block length = 1 947'
Rt Slider 2x6 SP #2, block length = 1 947'

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

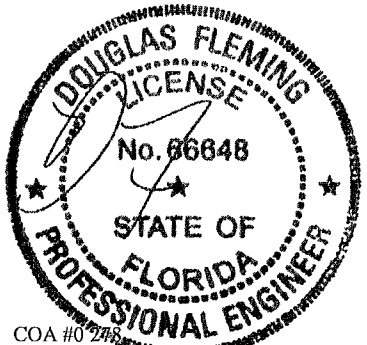
Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 8-10-0



COA #0 248
Florida Certificate of Product Approval #FL1999
02/20/2025

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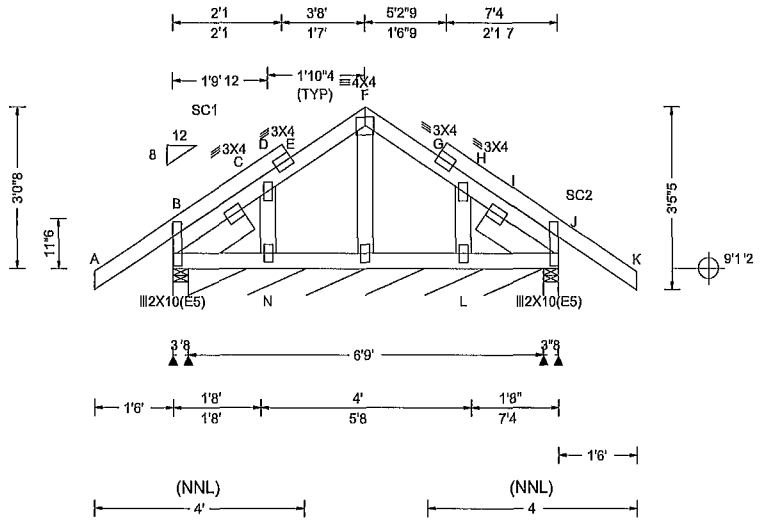
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800170 GABL FROM CDM	Ply 1 Qty 1	**Job Number:** 25-2238 138 MERCURY CT **Truss Label.** B01		Cust: R 215 JRef: 1Y7N2150006 T5 DrwNo. 050.25 1406.29543 KD / DF 02/19/2025



Loading Criteria (psf)	
TCLL	20 00
TCDL.	10 00
BCLL	0 00
BCDL.	10.00
Des Ld	40.00
NCBCLL	10 00
Soffit	2 00
Load Duration	1 25
Spacing	24 0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category II
EXP B Kzt NA
Mean Height 15 00 ft
TCDL. 5 0 psf
BCDL 5 0 psf
MWFRS Parallel Dist 0 to h/2
C&C Dist a 3 00 ft
Loc. from endwall Any
GCpl 0 18
Wind Duration 1 60

Snow Criteria (Pg Pf In PSF)
Pg NA Ct NA CAT NA
Pf NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac: Yes
FT/RT 20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL) 0 002	G	999	240
VERT(CL) 0 003	G	999	180
HORZ(LL) 0 001	E	-	-
HORZ(TL) 0 002	E	-	-

Creep Factor 2.0
Max TC CSI 0 187
Max BC CSI 0.026
Max Web CSI 0 103

VIEW Ver 23.02.04 0123 14

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	230	/-	/-	/126	/16	/78
B*	54	/-	/-	/32	/-	/-
J	231	/-	/-	/142	/16	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1 5 (Truss)
B Brg Wid = 81 0 Min Req = -
J Brg Wid = 3.5 Min Req = 1 5 (Truss)
Bearings B, B, & J are a rigid surface
Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Stack Chord SC1 2x4 SP #2,
Stack Chord SC2 2x4 SP #2,
Lt Slider 2x6 SP #2, block length = 1.495'
Rt Slider 2x6 SP #2, block length = 1.495'

Plating Notes

All plates are 2X4 except as noted

Loading

Gable end supports 8" max rake overhang Top chord must not be cut or notched

Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

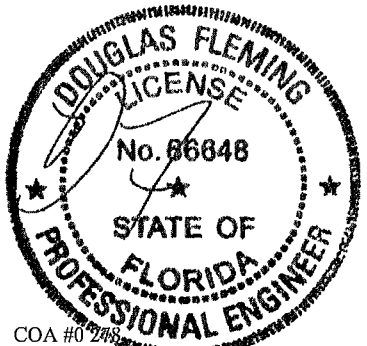
Gable meets L/120 deflection criteria for wind load applied to face Calculated deflection ratio is L/999

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms Connections and designs for diaphragms is the responsibility of the Building Designer In accordance with ANSI/TPI 1

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" oc intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

The overall height of this truss excluding overhang is 3-0-8



COA #0 2778
Florida Certificate of Product Approval #FL1999
02/20/2025

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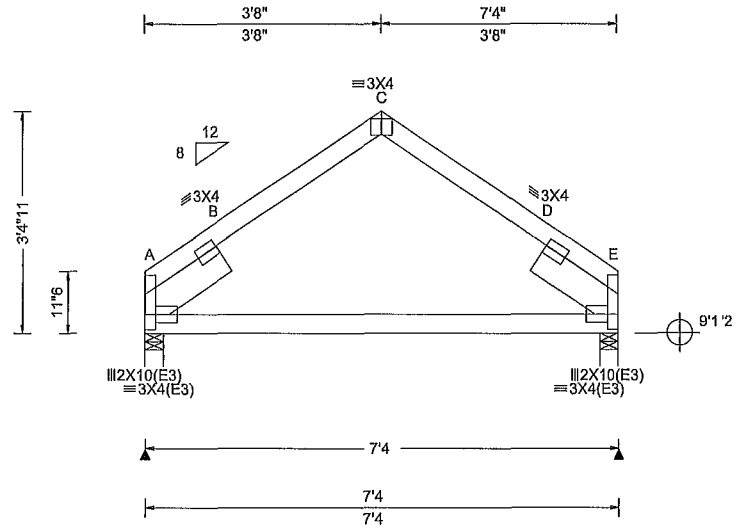
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800174 COMN FROM CDM	Ply 1 Qty 1	**Job Number:** 25-2238 138 MERCURY CT **Truss Label.** B02		Cust: R 215 JRef 1Y7N2150006 T4 DrwNo 050 25 1406.52960 KD / DF 02/19/2025



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg Pf In PSF)	**Defl/CSI Criteria**	**▲Maximum Reactions (lbs)**
TCLL.	20 00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#	Gravity / Non-Gravity
TCDL	10.00	Speed 130 mph	Pf NA Ce NA	VERT(LL) 0 009 B 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL	0.00	Enclosure Closed	Lu NA Cs NA	VERT(CL) 0.019 B 999 180	A 308 /- /- /171 /5 /45
BCDL	10.00	Risk Category II	Snow Duration NA	HORZ(LL) 0 008 B - -	E 308 /- /- /171 /5 /-
Des Ld	40.00	EXP B Kzt: NA		HORZ(TL) 0 017 B - -	Wind reactions based on MWFRS
NCBCLL	10 00	Mean Height: 15.00 ft	Building Code	Creep Factor 2.0	A Brg Wid = 3 5 Min Req = 1.5 (Truss)
Soffit:	2 00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0 199	E Brg Wid = 3 5 Min Req = 1.5 (Truss)
Load Duration	1 25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0.447	Bearings A & E are a rigid surface
Spacing	24 0 "	MWFRS Parallel Dist 0 to h/2	Rep Fac. Yes	Max Web CSI 0 062	Members not listed have forces less than 375#
		C&C Dist a 3 00 ft	FT/RT 20(0)/10(0)		
		Loc from endwall Any	Plate Type(s)		
		GCpi 0 18	WAVE		
		Wind Duration 1 60		VIEW Ver 23 02.04 0123 14	

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Lt Slider 2x6 SP #2, block length = 1 500'
Rt Slider 2x6 SP #2, block length = 1 500'

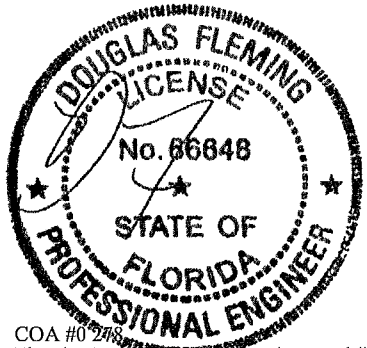
Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 3-4-11



COA #0 2718
Florida Certificate of Product Approval #FL1999
02/20/2025

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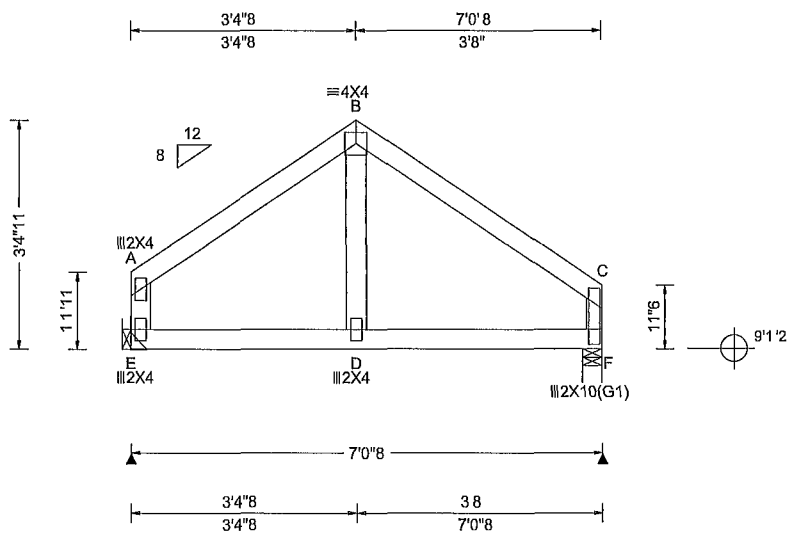
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800199 COMN FROM CDM	Ply 1 Qty 3	**Job Number:** 25-2238 138 MERCURY CT **Truss Label** B03		Cust: R 215 JRef 1Y7N2150006 T39 DrwNo 050 25 1406,55080 KD / DF 02/19/2025



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg Pf in PSF)	**Defl/CSI Criteria**				**▲ Maximum Reactions (lbs)**					
TCLL	20.00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in	loc	L/defl	L/#		Gravity			Non-Gravity	
TCDL	10 00	Speed 130 mph	Pf NA Ce NA	VERT(LL)	0 031 D	999	240	Loc	R+ / R-	/ Rh		/ Rw	/ U / RL
BCLL	0 00	Enclosure Closed	Lu NA Cs NA	VERT(CL)	0 065 D	999	180	E	296 /-	/-		/161	/5 /45
BCDL	10 00	Risk Category II	Snow Duration NA	HORZ(LL)	-0 046 A	-	-	F	296 /-	/-		/164	/4 /-
Des Ld	40 00	EXP B Kzt. NA		HORZ(TL)	0 096 A	-	-	Wind reactions based on MWFRS					
NCBCLL	10 00	Mean Height. 15.00 ft	Building Code	Creep Factor 2 0				E Brg Wid = - Min Req = -					
Soffit	2 00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0 172				F Brg Wid = 3 5 Min Req = 1.5 (Truss)					
Load Duration	1 25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0 185				Bearing F is a rigid surface					
Spacing	24 0 "	MWFRS Parallel Dist 0 to h/2	Rep Fac: Yes	Max Web CSI 0.244				Members not listed have forces less than 375#					
		C&C Dist a 3 00 ft	FT/RT 20(0)/10(0)										
		Loc from endwall not in 4.50 ft	Plate Type(s)										
		GCpi 0 18	WAVE										
		Wind Duration 1 60		VIEW Ver 23.02 04 0123 14									

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Rt Stub Wedge 2x6 SP #2,

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie Please refer to the most recent Simpson Strong-Tie catalog for additional information

Recommended hanger connections are based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

Bearing at location x=0' uses the following support conditions 0'
Bearing E (0', 9'1"2) LUS26
Supporting Member (1)2x6 SP 2400f-2.0E
(4) 0 148"x3" nails into supporting member,
(3) 0 148"x3" nails into supported member

Additional Notes

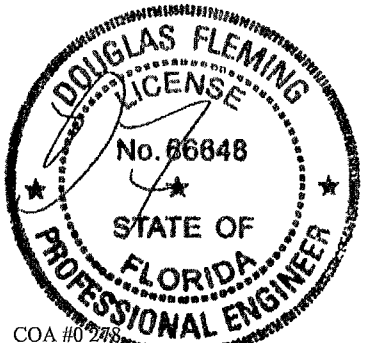
The overall height of this truss excluding overhang is 3-4-11

Wind

Wind loads based on MWFRS with additional C&C member design

Left end vertical not exposed to wind pressure

Wind loading based on both gable and hip roof types



COA #0 278
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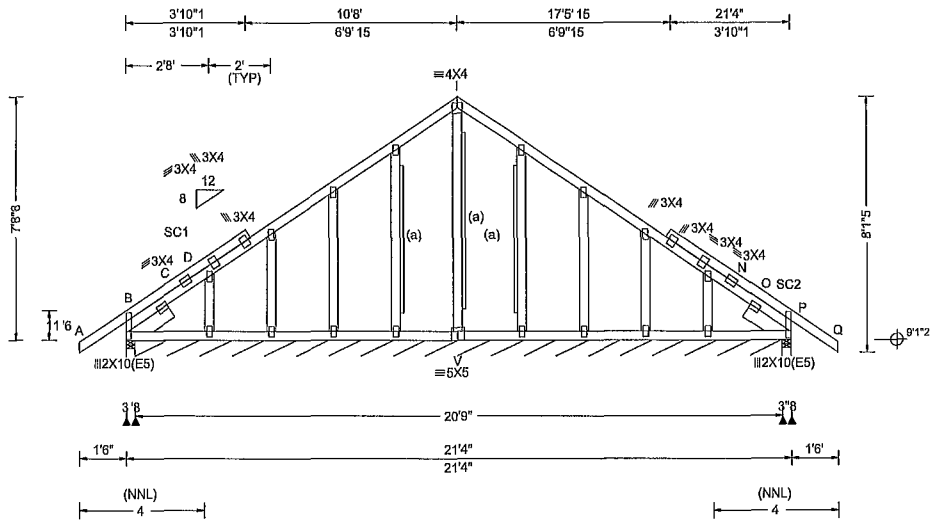
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800275 GABL FROM CDM	Ply 1 Qty 1	**Job Number:** 25-2238 138 MERCURY CT **Truss Label** C01		Cust. R 215 JRef 1Y7N2150006 T7 DrwNo 050.25 1406.56847 KD / DF 02/19/2025



Loading Criteria (psf)	
TCLL.	20 00
TCDL	10 00
BCLL	0 00
BCDL.	10.00
Des Ld	40.00
NCBCLL	10.00
Soffit	2 00
Load Duration	1 25
Spacing	24 0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category II
EXP B Kzt: NA
Mean Height 15.00 ft
TCDL 5 0 psf
BCDL. 5 0 psf
MWFRS Parallel Dist 0 to h/2
C&C Dist a 3 00 ft
Loc from endwall Any
GCpi 0 18
Wind Duration 1 60

Snow Criteria (Pg Pf in PSF)
Pg NA Ct NA CAT NA
Pf NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac: Yes
FT/RT 20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL) 0 001	H	999	240
VERT(CL) 0 002	H	999	180
HORZ(LL) 0.002	O	-	-
HORZ(TL) 0 003	O	-	-

Creep Factor 2.0
Max TC CSI 0 182
Max BC CSI 0 052
Max Web CSI 0 903

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▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	251	/-	/-	/119	/7	/140
B*	72	/-	/-	/40	/-	/-
P	251	/-	/-	/138	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 1 5 (Truss)
B Brg Wid = 248 Min Req = -
P Brg Wid = 3 5 Min Req = 1.5 (Truss)
Bearings B, B, & P are a rigid surface
Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Stack Chord SC1 2x4 SP #2,
Stack Chord SC2 2x4 SP #2,
Lt Slider 2x6 SP #2, block length = 1 500'
Rt Slider 2x6 SP #2, block length = 1 500'

Plating Notes

All plates are 2X4 except as noted

Loading

Gable end supports 8" max rake overhang Top chord must not be cut or notched

Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Gable meets L/120 deflection criteria for wind load applied to face Calculated deflection ratio is L/260

Gable Reinforcement

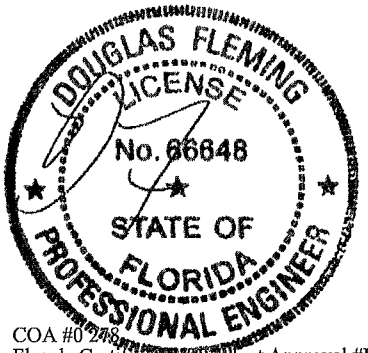
(a) 1x4 "L" reinforcement. Any species and grade 80% length of web member Attach with 10d (0 131"x3",min) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1

Stacked top chord must NOT be notched or cut in area (NNL) Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

The overall height of this truss excluding overhang is 7-8-8



COA #0 2148
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02/20/2025

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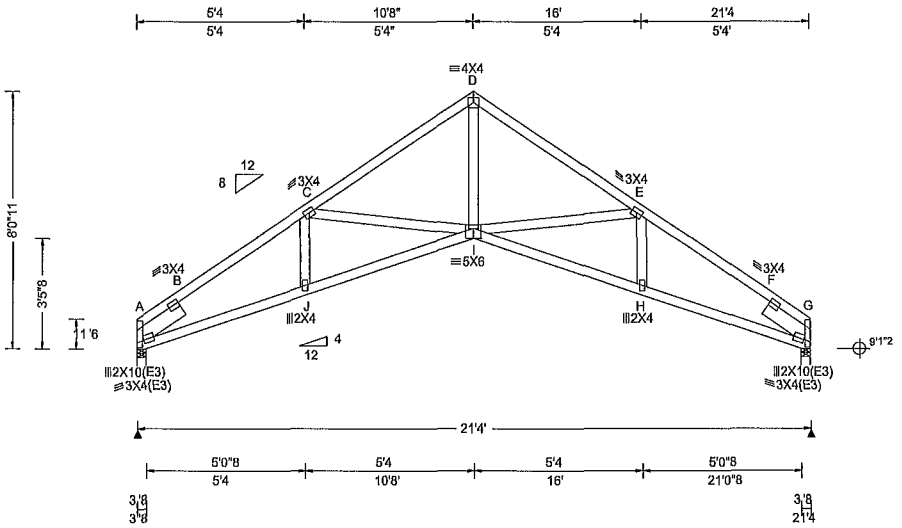
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800270 COMN FROM CDM	Ply: 1 Qty: 2	**Job Number:** 25-2238 138 MERCURY CT **Truss Label.** C02	Cust: R 215 JRef: 1Y7N2150006 T6 DrwNo 050.25.1406.58273 KD / DF 02/19/2025	



Loading Criteria (psf)

TCLL	20.00
TCDL	10 00
BCLL.	0.00
BCDL	10 00
Des Ld	40 00
NCBCLL	10 00
Soffit	2 00
Load Duration	1 25
Spacing	24 0 "

Wind Criteria

Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category: II
EXP B Kzt. NA
Mean Height: 15.00 ft
TCDL 5 0 psf
BCDL 5 0 psf
MWFRS Parallel Dist 0 to h/2
C&C Dist a 3 00 ft
Loc from endwall Any
GCpi 0 18
Wind Duration 1 60

Snow Criteria (Pg Pf in PSF)

Pg NA Ct NA CAT NA
Pf: NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac. Yes
FT/RT:20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL) 0 129	I	999	240
VERT(CL) 0 274	I	933	180
HORZ(LL) 0 129	F	-	-
HORZ(TL) 0 274	F	-	-

Creep Factor 2 0
Max TC CSI 0 645
Max BC CSI 0.595
Max Web CSI 0.452

VIEW Ver: 23 02 04 0123 14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	908	/-	/-	/510	/15	/133
G	908	/-	/-	/510	/15	/-

Wind reactions based on MWFRS
A Brg Wid = 3 5 Min Req = 1 5 (Truss)
G Brg Wid = 3 5 Min Req = 1 5 (Truss)
Bearings A & G are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
A - B	197	-1953	D - E	167	-1506
B - C	218	-1877	E - F	216	-1877
C - D	168	-1506	F - G	195	-1953

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp	Chords	Tens	Comp
A - J	1533	-125	I - H	1556	-126
J - I	1556	-129	H - G	1533	-122

Maximum Web Forces Per Ply (lbs)

Webs	Tens	Comp
D - I	1186	-68

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Lt Slider: 2x6 SP #2, block length = 1 773'
Rt Slider 2x6 SP #2, block length = 1 773'

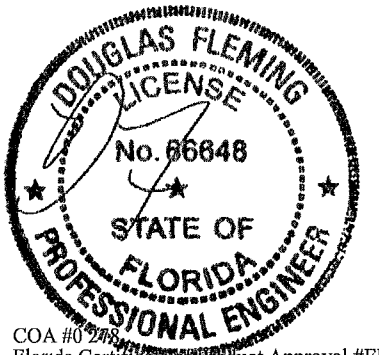
Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 8-0-11



COA #0 2708
Florida Certificate of Product Approval #FL1999
02/20/2025

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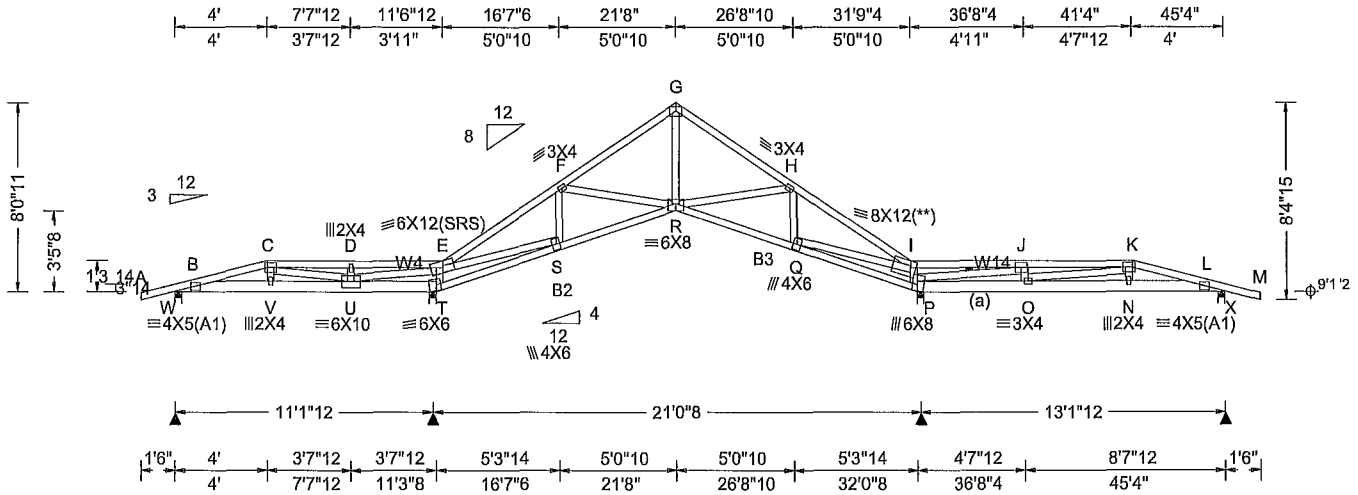
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800197 SPEC FROM CDM	Ply 1 Qty 1	**Job Number** 25-2238 138 MERCURY CT **Truss Label** D01	Cust R 215 JRef 1Y7N2150006 T31 DrwNo 050.25.1407.07760 KD / DF 02/19/2025	



Loading Criteria (psf)

TCLL	20.00
TCDL	10.00
BCLL	0 00
BCDL	10 00
Des Ld	40 00
NCBCLL	10 00
Soffit:	2 00
Load Duration	1 25
Spacing	24 0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category II
EXP B Kzt NA
Mean Height 15 00 ft
TCDL 5 0 psf
BCDL 5 0 psf
MWFRS Parallel Dist 0 to h/2
C&C Dist a 4 53 ft
Loc. from endwall not in 6.50 ft
GCpi 0 18
Wind Duration 1 60

Snow Criteria (Pg Pf In PSF)
Pg NA Ct NA CAT NA
Pf NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac Varies by Ld Case
FT/RT 20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL)	0 111	O 999	240
VERT(CL)	0.232	O 681	180
HORZ(LL)	0 059	L -	-
HORZ(TL)	0 120	L -	-

Creep Factor 2.0
Max TC CSI 0.549
Max BC CSI 0 772
Max Web CSI 0 744

VIEW Ver 23.02 04 0123 14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
W	794	/-	/-	/-	/51	/-
T	1842	/-	/-	/-	/55	/-
P	2002	/-	/-	/-	/66	/-
X	943	/-	/-	/-	/61	/-

Wind reactions based on MWFRS
W Brg Wid = 3 5 Min Req = 1.5 (Truss)
T Brg Wid = 3 5 Min Req = 2 2 (Truss)
P Brg Wid = 3 5 Min Req = 2.4 (Truss)
X Brg Wid = 3 5 Min Req = 1 5 (Truss)
Bearings W, T, P, & X are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - C	78	-2008	G - H	29	-1096
C - D	77	-1485	H - I	21	-1214
D - E	77	-1486	I - J	1210	-54
E - F	24	-1246	J - K	114	-2391
F - G	29	-1098	K - L	118	-2655

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - V	1935	-72	R - Q	1033	-19
V - U	2002	-69	Q - P	51	-1129
U - T	54	-1261	P - O	2265	-124
T - S	43	-993	O - N	2617	-105
S - R	1065	-23	N - L	2566	-112

Maximum Web Forces Per Ply (lbs)

Webs	Tens	Comp	Webs	Tens	Comp
C - V	435	0	G - R	814	0
C - U	0	-597	Q - I	1954	-45
U - E	2687	-134	I - P	53	-611
T - E	80	-1263	P - J	182	-3425
E - S	1881	-42	J - O	471	0

Lumber

Top chord 2x4 SP #2,
Bot chord 2x6 SP #2, B2,B3 2x4 SP #2
Webs 2x4 SP #3, W4,W14 2x4 SP #2,

Bracing

(a) Continuous lateral restraint equally spaced on member

Special Loads

-------(Lumber Dur Fac =1 25 / Plate Dur Fac =1.25)

TC	From	61 plf at	-1 50 to	61 plf at	11.56
TC	From	64 plf at	11 56 to	64 plf at	31 77
TC	From	61 plf at	31 77 to	61 plf at	46 83
BC	From	4 plf at	-1 50 to	4 plf at	0.00
BC	From	20 plf at	0 00 to	20 plf at	11 29
BC	From	21 plf at	11.29 to	21 plf at	32.04
BC	From	20 plf at	32.04 to	20 plf at	45.33
BC	From	4 plf at	45.33 to	4 plf at	46.83

TC 123 lb Conc Load at 41 33
BC 307 lb Conc Load at 4 06
BC 133 lb Conc Load at 6 06, 8 06,35.27,37.27 39 27
BC 155 lb Conc Load at 10 06,33 27
BC 184 lb Conc Load at 41 30

Plating Notes

All plates are 5X6 except as noted

(**) 1 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc

Wind

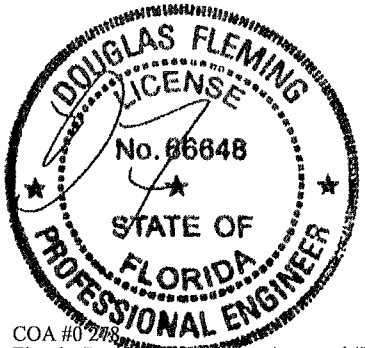
Wind loads and reactions based on MWFRS

Wind loading based on both gable and hip roof types

Additional Notes

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See "WARNING" note below

The overall height of this truss excluding overhang is 8-0-11



COA #0 2778
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02/20/2025

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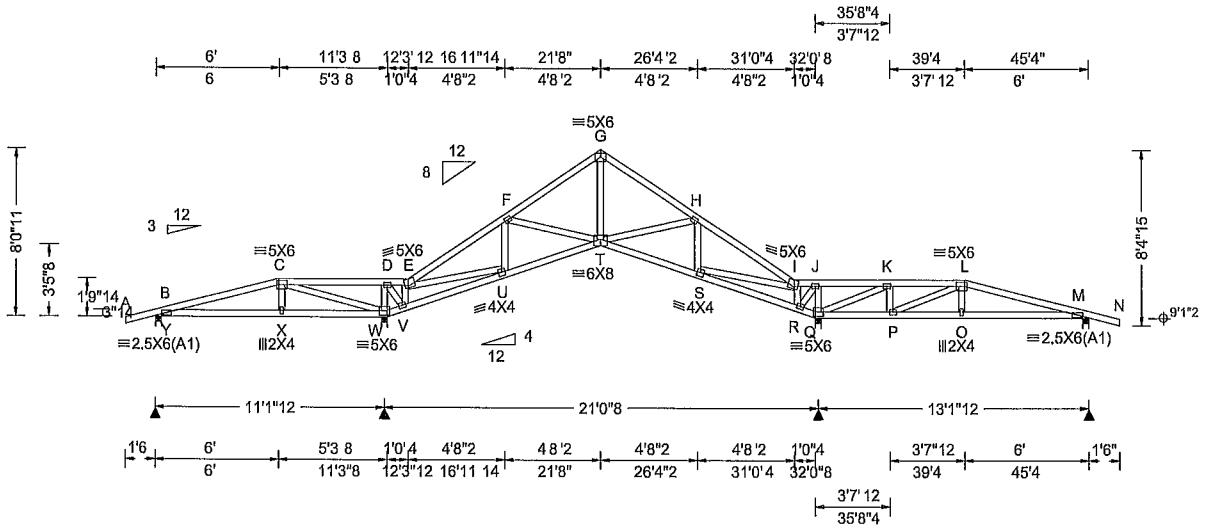
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800176 SPEC FROM CDM	Ply· 1 Qty· 1	**Job Number** 25-2238 138 MERCURY CT **Truss Label**· D02	Cust. R 215 JRef·1Y7N2150006 T10 DrwNo 050.25.1407.09703 KD / DF 02/19/2025	



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg Pf in PSF)	**Defl/CSI Criteria**
TCLL	20.00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#
TCDL.	10.00	Speed 130 mph	Pf· NA Ce NA	VERT(LL) 0 044 T 999 240
BCLL	0 00	Enclosure Closed	Lu NA Cs NA	VERT(CL) 0 093 T 999 180
BCDL	10 00	Risk Category· II	Snow Duration NA	HORZ(LL) 0 041 M - -
Des Ld	40 00	EXP B Kzt· NA		HORZ(TL) 0 084 M - -
NCBCLL	10 00	Mean Height. 15.00 ft	Building Code	Creep Factor· 2.0
Soffit	2.00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0.519
Load Duration	1.25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0 381
Spacing	24.0 "	MWFRS Parallel Dist h/2 to h	Rep Fac. Yes	Max Web CSI 0 597
		C&C Dist a 4 53 ft	FT/RT·20(0)/10(0)	
		Loc from endwall not in 6.50 ft	Plate Type(s)	
		GCpi 0 18	WAVE	VIEW Ver 23 02 04 0123 14
		Wind Duration 1 60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Y	494	/-	/-	/225	/43	/147
W	1434	/-	/-	/800	/34	/-
Q	1552	/-	/-	/847	/41	/-
M	558	/-	/-	/277	/36	/-

Wind reactions based on MWFRS
Y Brg Wid = 3.5 Min Req = 1.5 (Truss)
W Brg Wid = 3 5 Min Req = 1 7 (Truss)
Q Brg Wid = 3.5 Min Req = 1 8 (Truss)
M Brg Wid = 3 5 Min Req = 1 5 (Truss)
Bearings Y, W, Q, & M are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp	Chords	Tens	Comp
B - C	120	-609	H - I	22	-1166
C - D	683	0	I - J	651	-60
D - E	377	-27	J - K	987	-64
E - F	36	-1191	K - L	156	-401
F - G	31	-1043	L - M	195	-912
G - H	27	-1043			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - X	554	-86	S - R	117	-575
X - W	568	-81	R - Q	112	-1039
W - V	33	-722	P - O	865	-150
U - T	1003	-4	O - M	853	-154
T - S	983	-7			

Maximum Web Forces Per Ply (lbs)

Webs	Tens	Comp	Webs	Tens	Comp
C - W	103	-1131	S - I	1367	-13
D - W	142	-841	I - R	70	-731
D - V	701	0	R - J	728	-21
V - E	21	-716	J - Q	70	-695
E - U	1176	0	Q - K	229	-1210
G - T	778	0	P - L	46	-601

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,

Plating Notes

All plates are 3X4 except as noted

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

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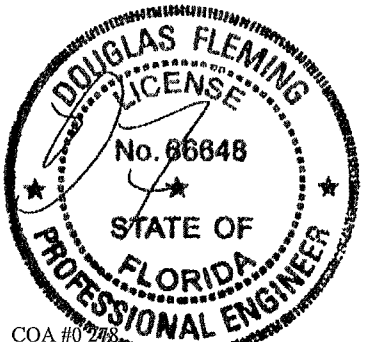
The overall height of this truss excluding overhang is 8-0-11



COA #0 [illegible]
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02/20/2025

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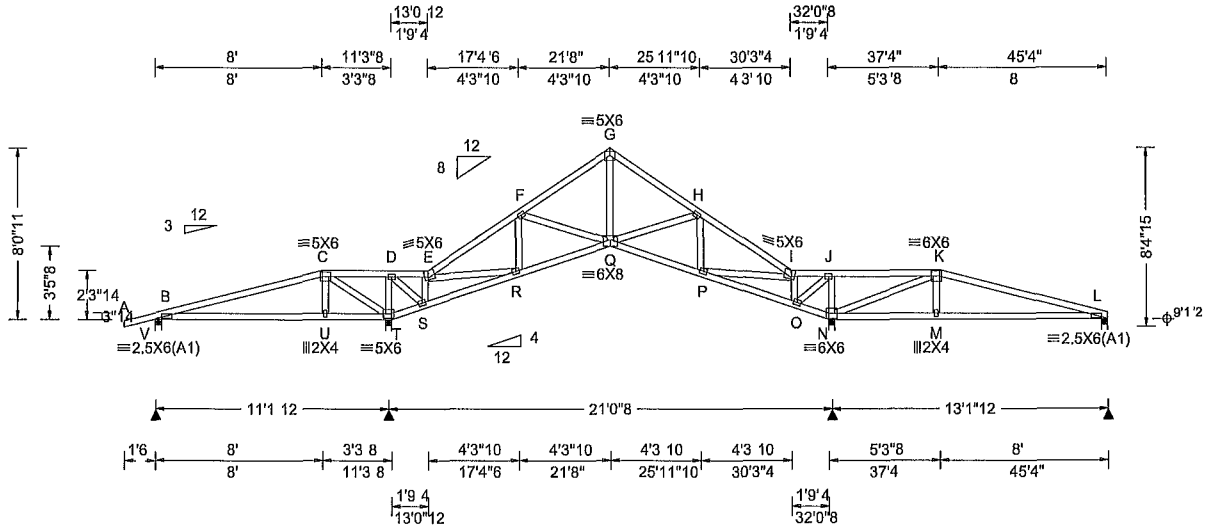
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800178 SPEC FROM CDM	Ply: 1 Qty: 1	**Job Number:** 25-2238 138 MERCURY CT **Truss Label:** D03	Cust: R 215 JRef: 1Y7N2150006 T16 DrwNo 050 25 1407 11550 KD / DF 02/19/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg Pf in PSF)	**Defl/CSI Criteria**
TCLL 20 00	Wind Std ASCE 7-22	Pg NA Ct. NA CAT NA	PP Deflection in loc L/defl L/#
TCDL 10 00	Speed 130 mph	Pf: NA Ce NA	VERT(LL) 0.074 L 999 240
BCLL 0.00	Enclosure Closed	Lu NA Cs NA	VERT(CL) 0 150 L 999 180
BCDL 10.00	Risk Category II	Snow Duration NA	HORZ(LL) 0 032 L - -
Des Ld 40.00	EXP B Kzt: NA		HORZ(TL) 0 065 L - -
NCBCLL 10 00	Mean Height: 15.00 ft	Building Code	Creep Factor: 2 0
Soffit: 2 00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0.943
Load Duration 1 25	BCDL. 5 0 psf	TPI Std 2014	Max BC CSI 0 729
Spacing 24 0 "	MWFRS Parallel Dist h/2 to h	Rep Fac: Yes	Max Web CSI 0 707
	C&C Dist a 4 53 ft	FT/RT 20(0)/10(0)	
	Loc from endwall not in 6.50 ft	Plate Type(s)	
	GCpi 0 18	WAVE	VIEW Ver: 23.02 04 0123 14
	Wind Duration 1 60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
V	480	/-	/-	/204	/45	/138
T	1469	/-	/-	/825	/34	/-
N	1555	/-	/-	/849	/41	/-
L	457	/-	/-	/214	/23	/-

Wind reactions based on MWFRS
V Brg Wid = 3 5 Min Req = 1 5 (Truss)
T Brg Wid = 3 5 Min Req = 1 7 (Truss)
N Brg Wid = 3.5 Min Req = 1 8 (Truss)
L Brg Wid = 3.5 Min Req = 1 5 (Truss)
Bearings V, T, N, & L are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp	Chords	Tens	Comp
C - D	782	-13	H - I	50	-1035
E - F	58	-1041	J - K	789	-6
F - G	52	-939	K - L	108	-647
G - H	49	-939			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
T - S	55	-821	O - N	52	-832
R - Q	877	-12	N - M	592	-64
Q - P	873	-13	M - L	577	-68

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp	Webs	Tens	Comp
C - T	65	-954	P - I	952	0
D - T	112	-749	I - O	50	-719
D - S	825	-42	O - J	786	-15
S - E	86	-744	J - N	160	-818
E - R	891	0	N - K	108	-1269
G - Q	695	0			

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,

Plating Notes

All plates are 3X4 except as noted

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc

Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

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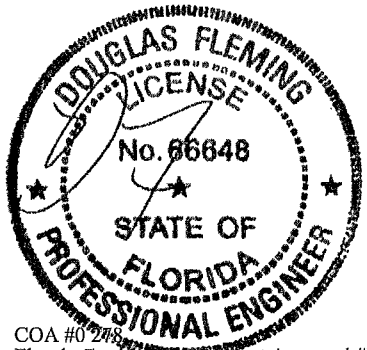

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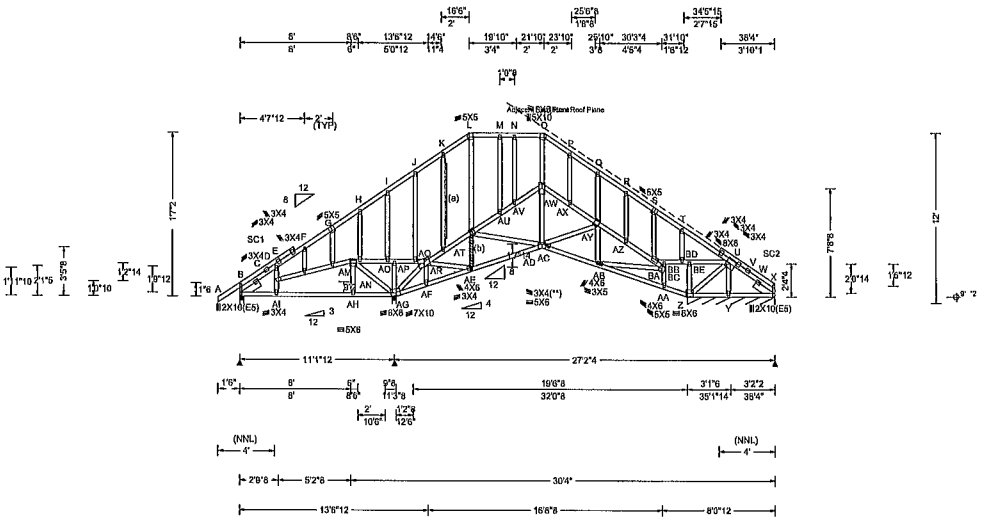
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800212 GABL FROM CDM Page 1 of 2	Ply 1 Qty 1	**Job Number** 25-2238 138 MERCURY CT **Truss Label.** G01	Cust: R 215 JRef 1Y7N2150006 T15 DrwNo 050.25.1407.22880 SSB / FV 02/19/2025	



Loading Criteria (psf)	
TCLL	20.00
TCDL	10.00
BCLL	0 00
BCDL	10 00
Des Ld	40 00
NCBCLL	10 00
Soffit:	2.00
Load Duration	1 25
Spacing	24 0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category II
EXP B Kzt: NA
Mean Height: 15 75 ft
TCDL. 5 0 psf
BCDL 5 0 psf
MWFRS Parallel Dist 0 to h/2
C&C Dist a 3 83 ft
Loc from endwall not in 9 00 ft
GCpi 0 18
Wind Duration 1.60

Snow Criteria (Pg Pf in PSF)
Pg NA Ct NA CAT NA
Pf NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac. Yes
FT/RT 20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL)	0 114	AK 999	240
VERT(CL)	0 245	AK 552	180
HORZ(LL)	0 078	F -	-
HORZ(TL)	0 169	F -	-

Creep Factor 2.0
Max TC CSI 0.420
Max BC CSI 0 792
Max Web CSI 0.860

VIEW Ver 23 02 04 0123 14

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	787	/-	/-	/409	/-	/211
AG	1019	/-	/-	/699	/-	/-
X*	245	/-	/-	/149	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 1 5 (Truss)
AG Brg Wid = 3 5 Min Req = 1 5 (Truss)
X Brg Wid = 75 5 Min Req = -
Bearings B, AG, & Z are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp	Chords	Tens	Comp
B - C	8	-658	L - M	86	-515
C - D	0	-657	M - N	86	-513
D - E	0	-683	N - O	87	-513
E - F	0	-664	O - P	88	-617
F - G	0	-637	P - Q	62	-665
G - H	0	-614	Q - R	6	-647
H - I	0	-694	R - S	0	-660
I - J	10	-651	S - T	0	-693
J - K	59	-661	T - U	0	-700
K - L	88	-630			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B -AI	545	0	AD-AC	950	0
AI-AH	545	-3	AC-AB	942	0
AH-AG	558	0	AB-AA	68	-607
AE-AD	745	0	AA- Z	91	-601

Maximum Web Forces Per Ply (lbs)

Webs	Tens	Comp	Webs	Tens	Comp
AM-AN	685	0	AX-AY	0	-501
AM-AG	24	-886	AY-AZ	0	-424
AN-AO	679	0	AB-BC	1427	0
AO-AP	678	0	AZ-BA	0	-484
AP-AQ	695	0	BA-BB	904	0
AG-AR	0	-423	BA-BC	0	-1498
AQ-AR	679	0	BB-BC	660	0
AR-AE	593	0	BB-BD	1013	0
AT-AU	0	-517	BD-BE	1021	0
AU-AV	0	-481	Z -BE	46	-518
AV-AW	0	-469	Z - U	43	-457
AW-AD	563	0	BE- U	990	0

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Stack Chord SC1 2x4 SP #2,
Stack Chord SC2 2x4 SP #2,
Lt Slider 2x6 SP #2, block length = 1 500'
Rt Slider 2x6 SP #2, block length = 1 500'

Plating Notes

All plates are 2X4 except as noted

(**) 1 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements

Loading

Gable end supports 8" max rake overhang Top chord must not be cut or notched

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc

Wind

Wind loads based on MWFRS with additional C&C member design

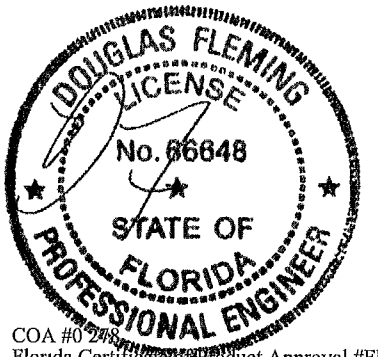
Wind loading based on both gable and hip roof types

Gable meets L/120 deflection criteria for wind load applied to face Calculated deflection ratio is L/221

Laterally brace top chord below filler and bottom chord above filler at 24" o c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)

Gable Reinforcement

(a) 1x4 "L" reinforcement Any species and grade 80% length of web member Attach with 10d (0 131"x3",min) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder
(b) 2x3 "T" reinforcement Any species and grade Full truss height along web member Attach to the wide face with 10d (0 131"x3",min) nails @ 4" oc in the web plus (2)10d (0 131"x3 ,min) nails in each chord



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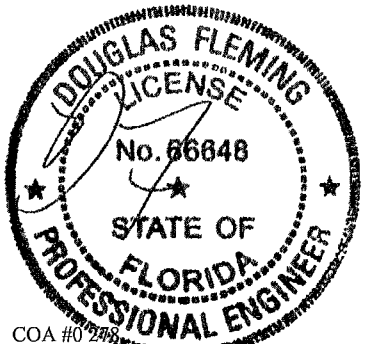
Additional Notes

AW-AX 0 -478

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1

Stacked top chord must NOT be notched or cut in area (NNL) Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

The overall height of this truss excluding overhang is 11-7-2



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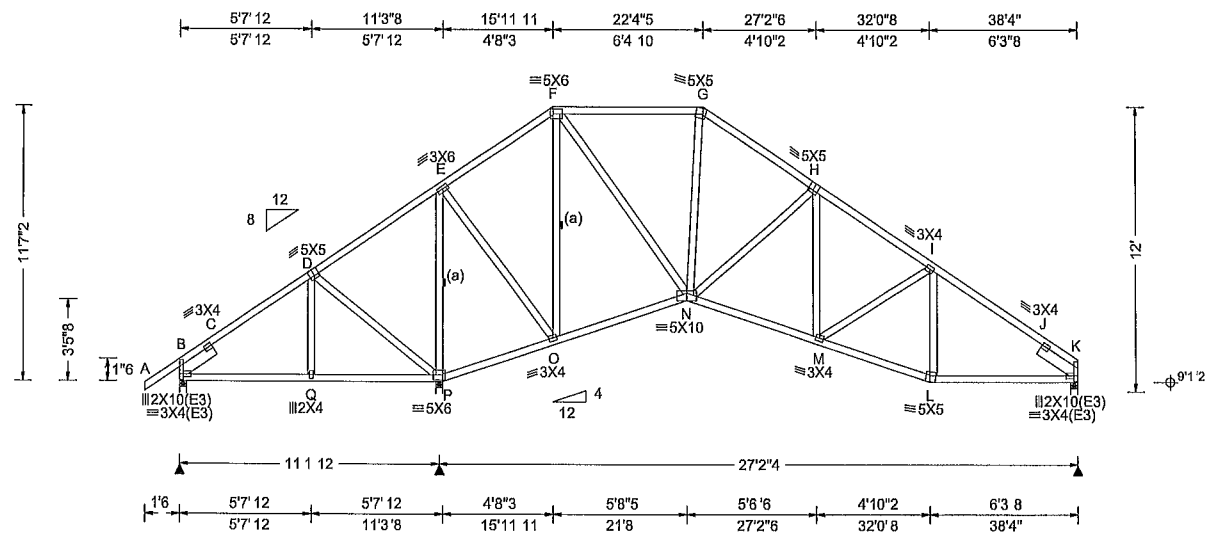
Glenview, IL 60025

SEQN 800216 COMN FROM CDM	Ply 1 Qty 3	**Job Number:** 25-2238 138 MERCURY CT **Truss Label** G02		Cust: R 215 JRef 1Y7N2150006 T8 DrwNo 050.25 1407.24643 KD / DF 02/19/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg Pf in PSF)	**Defl/CSI Criteria**
TCLL 20 00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#
TCDL 10 00	Speed 130 mph	Pf NA Ce NA	VERT(LL) 0 050 M 999 240
BCLL 0.00	Enclosure Closed	Lu NA Cs NA	VERT(CL) 0 102 M 999 180
BCDL 10 00	Risk Category II	Snow Duration NA	HORZ(LL) 0 035 K - -
Des Ld 40 00	EXP B Kzt NA		HORZ(TL) 0 071 K - -
NCBCLL 10 00	Mean Height 15 93 ft	Building Code	Creep Factor 2 0
Soffit: 2 00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0.479
Load Duration 1.25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0.445
Spacing 24 0 "	MWFRS Parallel Dist h/2 to h	Rep Fac Yes	Max Web CSI 0.535
	C&C Dist a 3 83 ft	FT/RT 20(0)/10(0)	
	Loc from endwall not in 9.00 ft	Plate Type(s)	
	GCpi 0 18	WAVE	VIEW Ver 23.02.04 0123 14
	Wind Duration 1.60		

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	442	/-	/-	/211	/76	/234
P	1927	/-	/-	/1132	/-	/-
K	1056	/-	/-	/643	/2	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 1 5 (Truss)
P Brg Wid = 3 5 Min Req = 2.3 (Truss)
K Brg Wid = 3 5 Min Req = 1.5 (Truss)
Bearings B, P, & K are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
D - E	429	0	H - I	71	-1343
E - F	134	-413	I - J	57	-1384
F - G	68	-760	J - K	65	-1484
G - H	51	-980			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
N - M	1112	0	L - K	1077	0
M - L	1145	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp	Webs	Tens	Comp
D - P	70	-446	F - O	0	-852
E - P	0	-1451	F - N	825	0
E - O	962	0	N - H	130	-378

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Lt Slider 2x6 SP #2, block length = 1 821'
Rt Slider 2x6 SP #2, block length = 1 972'

Bracing

(a) Continuous lateral restraint equally spaced on member

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc

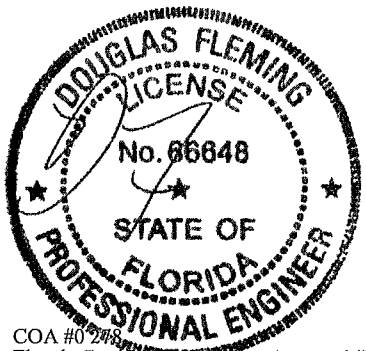
Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 11-7-2



COA #0 218
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02/20/2025

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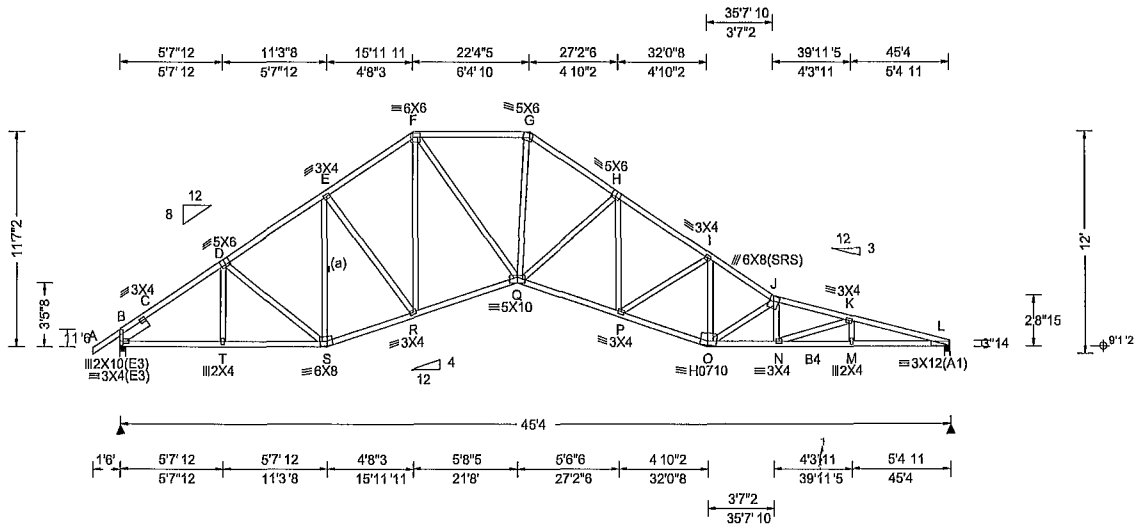
Glenview, IL 60025

SEQN 800224 COMN FROM CDM	Ply 1 Qty 1	**Job Number:** 25-2238 138 MERCURY CT **Truss Label** G03	Cust: R 215 JRef 1Y7N2150006 T12 DrwNo. 050.25 1407 26463 KD / DF 02/19/2025	



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg Pf in PSF)	**Defl/CSI Criteria**				
TCLL	20.00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in	loc	L/defl	L/#	
TCDL	10.00	Speed 130 mph	Pf NA Ce NA	VERT(LL)	0.474 O	999	240	
BCLL	0 00	Enclosure Closed	Lu NA Cs NA	VERT(CL)	0 987 O	549	180	
BCDL	10 00	Risk Category II	Snow Duration NA	HORZ(LL)	0 176 F	-	-	
Des Ld	40 00	EXP B Kzt: NA		HORZ(TL)	0.366 F	-	-	
NCBCLL	10 00	Mean Height 15 93 ft	Building Code	Creep Factor 2 0				
Soffit	2 00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI	0 872			
Load Duration	1 25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI	0 888			
Spacing	24 0 "	MWFRS Parallel Dist h to 2h	Rep Fac Yes	Max Web CSI	0 956			
		C&C Dist a 5 63 ft	FT/RT 20(0)/10(0)					
		Loc from endwall not in 13 00 ft	Plate Type(s)	VIEW Ver 23.02.04 0123 14				
		GCpi 0 18	WAVE, HS					
		Wind Duration 1 60						

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	2011	/-	/-	/1159	/-	/245
L	1889	/-	/-	/1054	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 2.4 (Truss)
L Brg Wid = 3 5 Min Req = 1 6 (Truss)
Bearings B & L are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - C	42	-2792	G - H	0	-3245
C - D	0	-2720	H - I	0	-3976
D - E	22	-2493	I - J	6	-4492
E - F	20	-2545	J - K	11	-6084
F - G	0	-2754	K - L	37	-6456

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - T	2141	0	P - O	3926	0
T - S	2138	0	O - N	5852	0
S - R	2124	0	N - M	6226	-19
R - Q	2172	0	M - L	6226	-15
Q - P	3412	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens	Comp	Webs	Tens	Comp
E - S	0	-447	H - P	592	-38
F - Q	1209	0	P - I	116	-567
Q - G	1397	0	O - J	65	-2597
Q - H	153	-771	N - K	61	-417

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2, B4 2x4 SP M-31,
Webs 2x4 SP #3,
Lt Slider 2x6 SP #2, block length = 1 821'

Bracing

(a) Continuous lateral restraint equally spaced on member

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

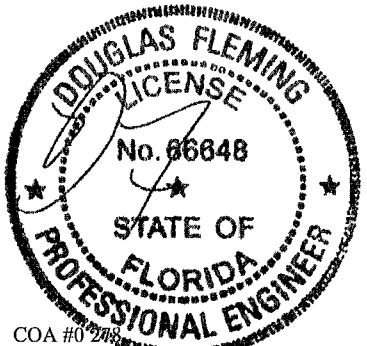
Blocking

Blocking reinforcement required to prevent buckling of members over the bearings
Bearing 1 located at 0 0' (blocking >= 3 50" if used)

Additional Notes

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See "WARNING" note below

The overall height of this truss excluding overhang is 11-7-2



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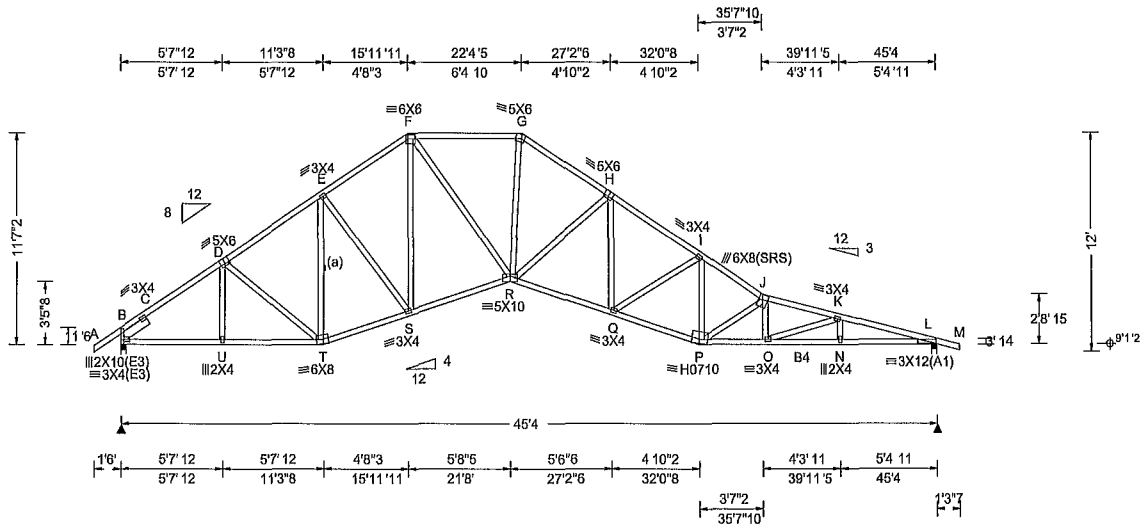
Glenview, IL 60025

SEQN 800221 COMN FROM CDM	Ply 1 Qty 5	**Job Number:** 25-2238 138 MERCURY CT **Truss Label** G04		Cust: R 215 JRef 1Y7N2150006 T40 DrwNo 050.25 1407.28163 KD / DF 02/19/2025



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg Pf in PSF)	Defl/CSI Criteria
TCLL	20 00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection In loc L/defl L/#
TCDL	10 00	Speed 130 mph	Pf NA Ce NA	VERT(LL) 0.491 P 999 240
BCLL	0.00	Enclosure Closed	Lu NA Cs NA	VERT(CL) 0 984 P 550 180
BCDL.	10 00	Risk Category II	Snow Duration NA	HORZ(LL) 0 182 F - -
Des Ld	40 00	EXP B Kzt NA		HORZ(TL) 0.365 F - -
NCBCLL	10.00	Mean Height 15 93 ft	Building Code	Creep Factor 2 0
Soffit.	2 00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0 871
Load Duration	1 25	BCDL. 5 0 psf	TPI Std 2014	Max BC CSI 0 886
Spacing	24 0 "	MWFRS Parallel Dist h to 2h	Rep Fac. Yes	Max Web CSI 0 952
		C&C Dist a 5 63 ft	FT/RT 20(0)/10(0)	
		Loc from endwall not in 13 00 ft	Plate Type(s)	
		GCpi 0 18	WAVE, HS	VIEW Ver 23 02.04.0123 14
		Wind Duration 1 60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	2009	/-	/-	/1159	/-	/249
L	1974	/-	/-	/1101	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 2.4 (Truss)
L Brg Wid = 3 5 Min Req = 1 6 (Truss)
Bearings B & L are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - C	42	-2790	G - H	0	-3240
C - D	0	-2718	H - I	0	-3968
D - E	22	-2490	I - J	4	-4480
E - F	18	-2543	J - K	8	-6062
F - G	0	-2750	K - L	29	-6403

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - U	2139	0	Q - P	3916	0
U - T	2136	0	P - O	5833	0
T - S	2122	0	O - N	6174	-5
S - R	2169	0	N - L	6173	-1
R - Q	3406	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens	Comp	Webs	Tens	Comp
E - T	0	-446	H - Q	589	-39
F - R	1206	0	Q - I	117	-562
R - G	1394	0	P - J	63	-2585
R - H	154	-768	O - K	57	-416

Lumber

Top chord 2x4 SP #2
Bot chord 2x4 SP #2, B4 2x4 SP M-31,
Webs 2x4 SP #3,
Lt Slider 2x6 SP #2, block length = 1 821'

Bracing

(a) Continuous lateral restraint equally spaced on member

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc

Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

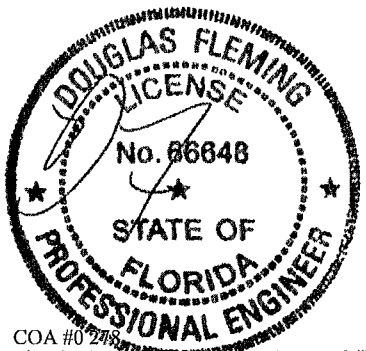
Blocking

Blocking reinforcement required to prevent buckling of members over the bearings
Bearing 1 located at 0 0' (blocking >= 3.50" if used)

Additional Notes

WARNING Furnish a copy of this DWG to the Installation contractor Special care must be taken during handling, shipping and installation of trusses See "WARNING" note below

The overall height of this truss excluding overhang is 11-7-2



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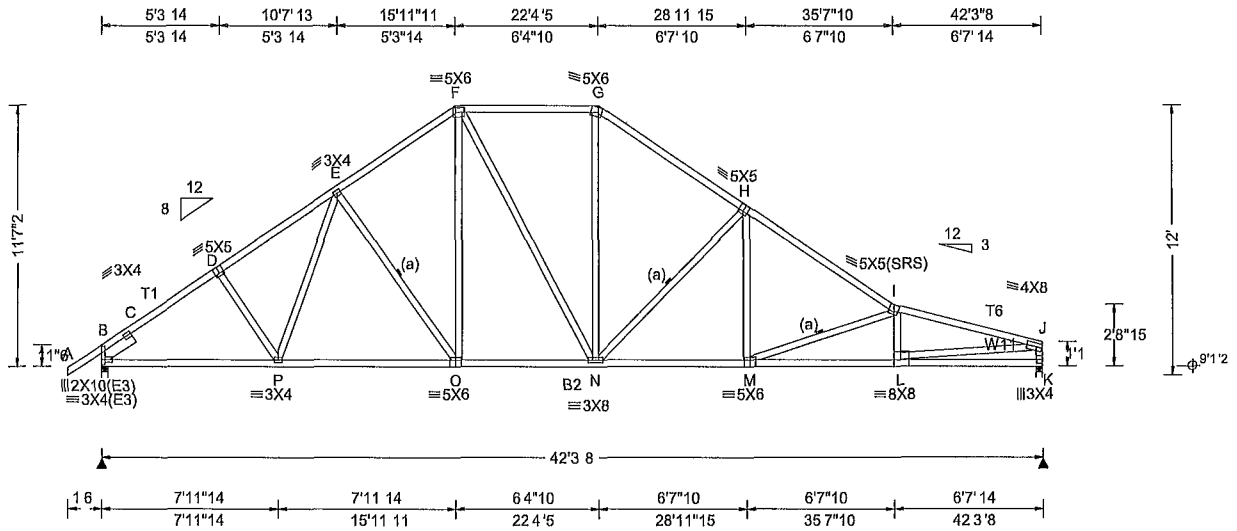
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800229 COMN FROM CDM	Ply 1 Qty 1	**Job Number** 25-2238 138 MERCURY CT **Truss Label.** G05	Cust R215 JRef 1Y7N2150006 T30 DrwNo 050.25 1407.29850 KD / DF 02/19/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg Pf in PSF)	**Defl/CSI Criteria**
TCLL 20 00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#
TCDL 10 00	Speed 130 mph	Pf NA Ce NA	VERT(LL) 0.219 M 999 240
BCLL 0.00	Enclosure Closed	Lu NA Cs NA	VERT(CL) 0.454 M 999 180
BCDL 10 00	Risk Category II	Snow Duration NA	HORZ(LL) 0 068 F - -
Des Ld 40.00	EXP B Kzt NA		HORZ(TL) 0 142 F - -
NCBCLL 10.00	Mean Height. 15 93 ft	Building Code	Creep Factor 2.0
Soffit: 2.00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0.677
Load Duration 1 25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0.681
Spacing 24 0 "	MWFRS Parallel Dist h to 2h	Rep Fac Yes	Max Web CSI 0.902
	C&C Dist a 4.23 ft	FT/RT 20(0)/10(0)	
	Loc from endwall not in 13 00 ft	Plate Type(s)	
	GCpi 0 18	WAVE	VIEW Ver 23 02 04.0123 14
	Wind Duration 1.60		

Lumber

Top chord 2x4 SP #2, T1,T6 2x4 SP M-31,
Bot chord 2x4 SP M-31, B2 2x4 SP #2
Webs 2x4 SP #3, W11 2x4 SP #2,
Lt Slider 2x6 SP #2, block length = 1 732'

Bracing

(a) Continuous lateral restraint equally spaced on member

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc

Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Wind loading based on both gable and hip roof types

Blocking

Blocking reinforcement required to prevent buckling of members over the bearings
Bearing 1 located at 0 0' (blocking >= 3 50" if used)

Additional Notes

The overall height of this truss excluding overhang is 11-7-2

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1879	/-	/-	/1086	/-	/233
K	1753	/-	/-	/975	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 1 6 (Truss)
K Brg Wid = 3 5 Min Req = 1 5 (Truss)
Bearings B & K are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

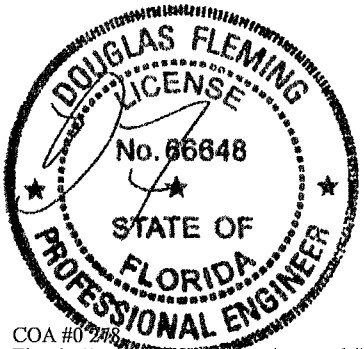
Chords	Tens.	Comp	Chords	Tens	Comp
B - C	44	-2600	F - G	78	-1673
C - D	0	-2524	G - H	61	-2122
D - E	33	-2379	H - I	17	-3010
E - F	62	-1997	I - J	14	-4249

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - P	1980	0	N - M	2387	0
P - O	1841	0	M - L	4126	0
O - N	1578	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp	Webs	Tens	Comp
E - O	128	-457	M - I	104	-1831
F - O	543	-36	I - L	71	-376
N - G	697	-30	L - J	3995	0
N - H	126	-1036	J - K	34	-1685
H - M	794	0			



COA #0 2[illegible]
Florida Certificate of Product Approval #FL1999
02/20/2025

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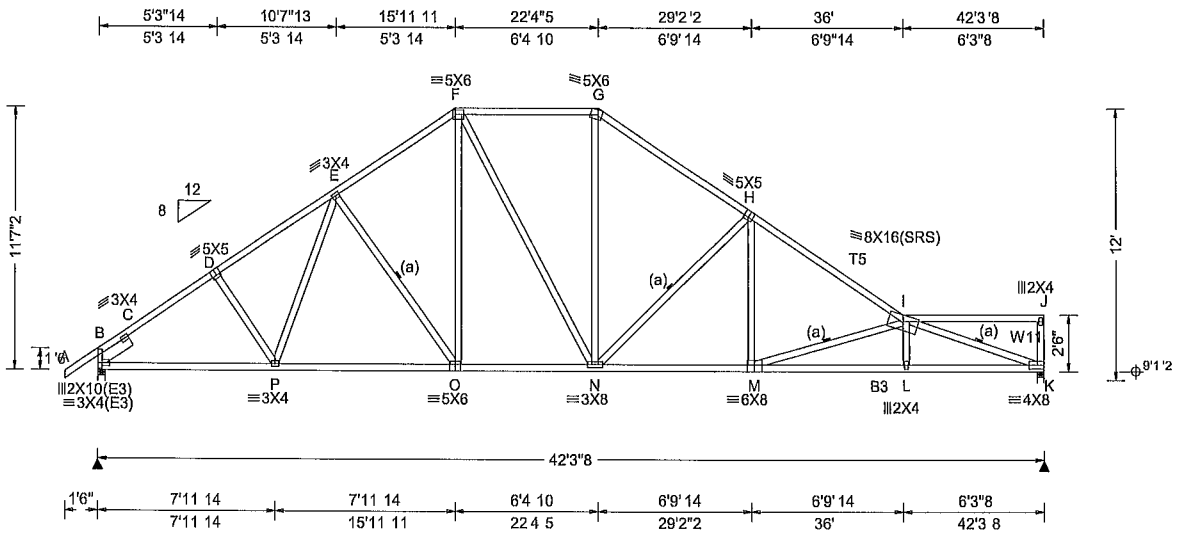
For more information see these web sites Alpine alpineitw com, TPI tpinst.org, SBCA. sbcacomponents com, ICC iccsafe org, AWC awc.org

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800232 SPEC FROM CDM	Ply 1 Qty 1	**Job Number** 25-2238 138 MERCURY CT **Truss Label** G06		Cust. R 215 JRef 1Y7N2150006 T25 DrwNo 050 25 1407.31353 KD / DF 02/19/2025



Loading Criteria (psf)	
TCLL	20.00
TCDL	10 00
BCLL	0.00
BCDL	10 00
Des Ld	40 00
NCBCLL	10.00
Soffit:	2.00
Load Duration	1 25
Spacing	24.0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category II
EXP B Kzt NA
Mean Height 15 93 ft
TCDL 5 0 psf
BCDL. 5 0 psf
MWFRS Parallel Dist h to 2h
C&C Dist a 4 23 ft
Loc from endwall not in 13 00 ft
GCpi 0 18
Wind Duration 1.60

Snow Criteria (Pg Pf In PSF)
Pg NA Ct. NA CAT NA
Pf NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac. Yes
FT/RT 20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL)	0 231 M	999	240
VERT(CL)	0.483 M	999	180
HORZ(LL)	0 075 K	-	-
HORZ(TL)	0 157 K	-	-

Creep Factor 2 0
Max TC CSI 0 803
Max BC CSI 0 755
Max Web CSI 0 928

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▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1881	/-	/-	/1089	/-	/225
K	1774	/-	/-	/966	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 2.2 (Truss)
K Brg Wid = 3 5 Min Req = 1.5 (Truss)
Bearings B & K are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - C	36	-2608	F - G	95	-1676
C - D	7	-2524	G - H	81	-2133
D - E	44	-2380	H - I	56	-3044
E - F	76	-2001			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - P	1980	0	N - M	2429	0
P - O	1844	0	M - L	4318	-115
O - N	1581	0	L - K	4325	-110

Maximum Web Forces Per Ply (lbs)

Webs	Tens	Comp	Webs	Tens	Comp
E - O	128	-457	H - M	775	0
F - O	544	-35	M - I	152	-1962
N - G	703	-42	I - K	105	-4538
N - H	159	-1068			

Lumber

Top chord 2x4 SP #2, T5 2x4 SP M-31,
Bot chord 2x4 SP #2, B3 2x4 SP M-31,
Webs 2x4 SP #3, W11 2x4 SP #2,
Lt Slider 2x6 SP #2, block length = 1 732'

Bracing

(a) Continuous lateral restraint equally spaced on member

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc

Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

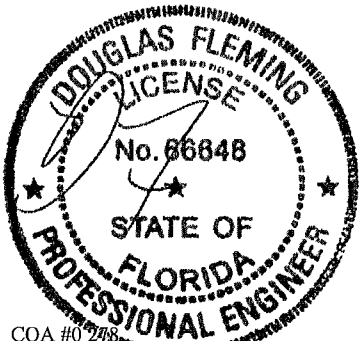
Wind loading based on both gable and hip roof types

Blocking

Blocking reinforcement required to prevent buckling of members over the bearings
Bearing 1 located at 0 0' (blocking >= 3 50" if used)

Additional Notes

The overall height of this truss excluding overhang is 11-7-2



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Florida Certificate of Product Approval #FL1999
02/20/2025

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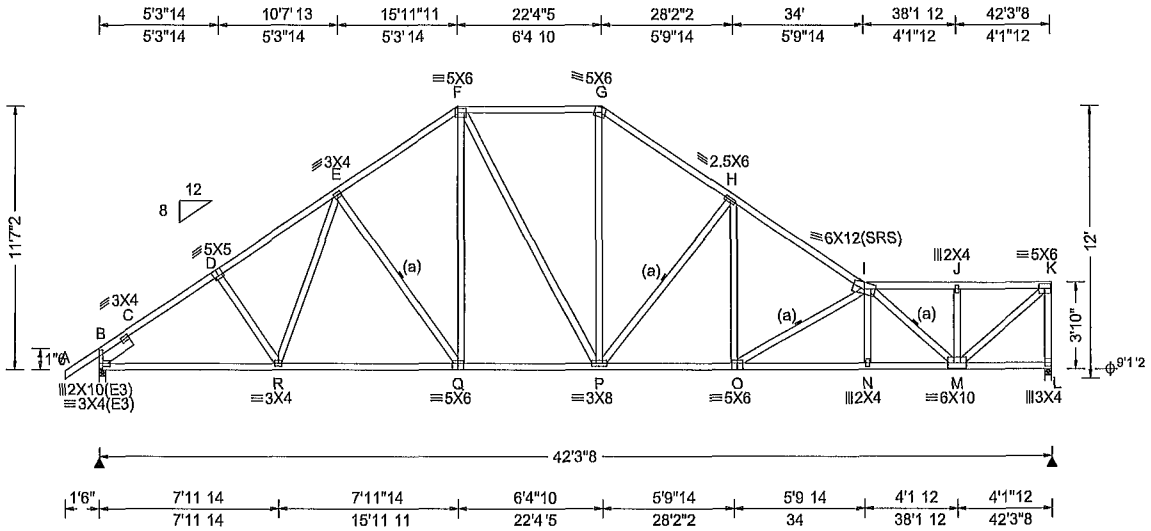
For more information see these web sites Alpine alpineitw com, TPI tpinst.org, SBCA sbcacomponents com, ICC iccsafe.org, AWC awc.org

ALPINE
ANITW COMPANY
155 Harlem Ave
North Building 4th Floor
Glenview, IL 60025

SEQN 800235 SPEC FROM CDM	Ply 1 Qty 1	**Job Number:** 25-2238 138 MERCURY CT **Truss Label:** G07	Cust: R 215 JRef 1Y7N2150006 T23 DrwNo 050.25 1407.32877 KD / DF 02/19/2025	



Loading Criteria (psf)	
TCLL	20 00
TCDL	10 00
BCLL	0.00
BCDL	10.00
Des Ld	40.00
NCBCLL	10.00
Soffit	2 00
Load Duration	1.25
Spacing	24 0 "

Wind Criteria	
Wind Std	ASCE 7-22
Speed	130 mph
Enclosure	Closed
Risk Category	II
EXP B	Kzt NA
Mean Height	15 93 ft
TCDL	5 0 psf
BCDL	5 0 psf
MWFRS Parallel Dist	h to 2h
C&C Dist a	4 23 ft
Loc from endwall	not in 13 00 ft
GCpi	0 18
Wind Duration	1 60

Snow Criteria (Pg Pf In PSF)			
Pg NA	Ct NA	CAT NA	
Pf NA		Ce NA	
Lu NA	Cs NA		
Snow Duration	NA		

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria				
PP Deflection in	loc	L/defl	L/#	
VERT(LL)	0 186	O	999	240
VERT(CL)	0.388	O	999	180
HORZ(LL)	0 066	L	-	-
HORZ(TL)	0 139	L	-	-
Creep Factor	2.0			
Max TC CSI	0.803			
Max BC CSI	0.828			
Max Web CSI	0 951			

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Lumber

Top chord 2x4 SP #2
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Lt Slider: 2x6 SP #2, block length = 1 732'

Bracing

(a) Continuous lateral restraint equally spaced on member

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc

Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Wind loading based on both gable and hip roof types

Blocking

Blocking reinforcement required to prevent buckling of members over the bearings
Bearing 1 located at 0 0' (blocking >= 3 50" if used)

Additional Notes

The overall height of this truss excluding overhang is 11-7-2

▲Maximum Reactions (lbs)

Loc	Gravity R+	Gravity / R-	Gravity / Rh	Non-Gravity / Rw	Non-Gravity / U	Non-Gravity / RL
B	1881	/-	/-	/1092	/-	/220
L	1774	/-	/-	/942	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 2 2 (Truss)
L Brg Wid = 3 5 Min Req = 2 1 (Truss)
Bearings B & L are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

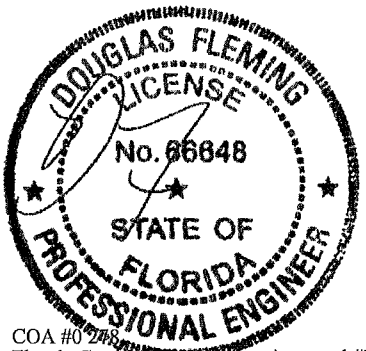
Chords	Tens	Comp	Chords	Tens	Comp
B - C	36	-2608	G - H	99	-2106
C - D	16	-2524	H - I	89	-2857
D - E	52	-2380	I - J	125	-1904
E - F	86	-2001	J - K	125	-1903
F - G	106	-1673			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - R	1980	-28	P - O	2278	-3
R - Q	1844	0	O - N	3377	-143
Q - P	1581	0	N - M	3381	-141

Maximum Web Forces Per Ply (lbs)

Webs	Tens	Comp	Webs	Tens	Comp
E - Q	127	-455	O - I	164	-1268
F - Q	549	-33	I - M	22	-1940
P - G	727	-61	M - K	2495	-162
P - H	165	-988	K - L	159	-1735
H - O	797	-25			



COA #0 2758
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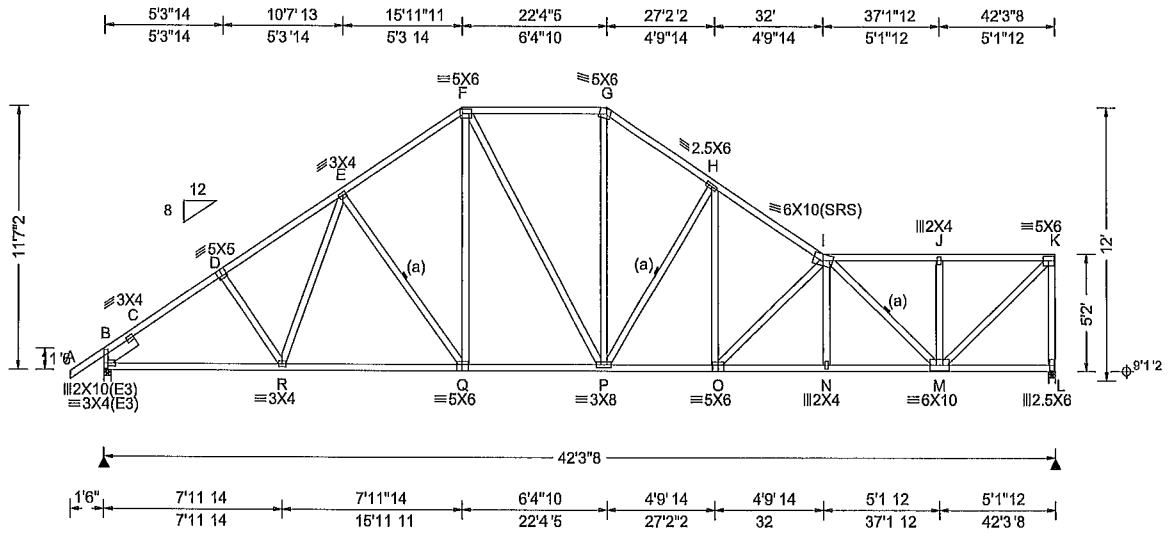
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800238 SPEC FROM CDM	Ply 1 Qty 1	**Job Number** 25-2238 138 MERCURY CT **Truss Label** G08	Cust R 215 JRef 1Y7N2150006 T21 DrwNo 050.25 1407.34543 KD / DF 02/19/2025	



Loading Criteria (psf)

TCLL	20.00
TCDL	10 00
BCLL.	0.00
BCDL	10 00
Des Ld	40 00
NCBCLL	10 00
Soffit:	2.00
Load Duration	1 25
Spacing	24 0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category II
EXP B Kzt NA
Mean Height 15 93 ft
TCDL 5 0 psf
BCDL 5 0 psf
MWFRS Parallel Dist h to 2h
C&C Dist a 4 23 ft
Loc from endwall not in 13 00 ft
GCpi 0 18
Wind Duration 1 60

Snow Criteria (Pg Pf in PSF)
Pg NA Ct NA CAT NA
Pf NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac. Yes
FT/RT 20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL)	0 169	O 999	240
VERT(CL)	0.352	O 999	180
HORZ(LL)	0 060	M -	-
HORZ(TL)	0 126	M -	-

Creep Factor 2.0
Max TC CSI 0.803
Max BC CSI 0 752
Max Web CSI 0.899

VIEW Ver 23 02 04 0123 14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1881	/-	/-	/1095	/-	/217
L	1774	/-	/-	/920	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 2 2 (Truss)
L Brg Wid = 3.5 Min Req = 2 1 (Truss)
Bearings B & L are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - C	37	-2608	G - H	116	-2079
C - D	22	-2524	H - I	110	-2683
D - E	59	-2379	I - J	131	-1690
E - F	93	-2001	J - K	131	-1689
F - G	114	-1672			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - R	1980	-65	P - O	2145	-31
R - Q	1844	-7	O - N	2860	-135
Q - P	1582	0	N - M	2862	-133

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp	Webs	Tens	Comp
E - Q	126	-454	O - I	148	-1001
F - Q	551	-31	I - M	3	-1614
P - G	749	-80	J - M	183	-376
P - H	172	-933	M - K	2328	-180
H - O	819	-58	K - L	186	-1732

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Lt Slider 2x6 SP #2, block length = 1 732'

Bracing

(a) Continuous lateral restraint equally spaced on member

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc

Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

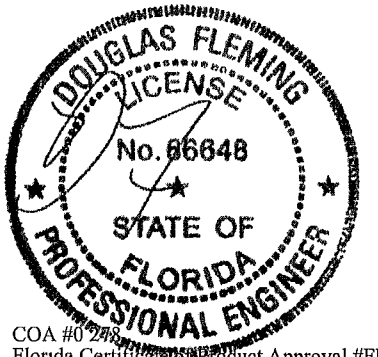
Wind loading based on both gable and hip roof types

Blocking

Blocking reinforcement required to prevent buckling of members over the bearings
Bearing 1 located at 0 0' (blocking >= 3 50" if used)

Additional Notes

The overall height of this truss excluding overhang is 11-7-2



COA #0 278
Florida Certificate of Product Approval #FL1999
02/20/2025

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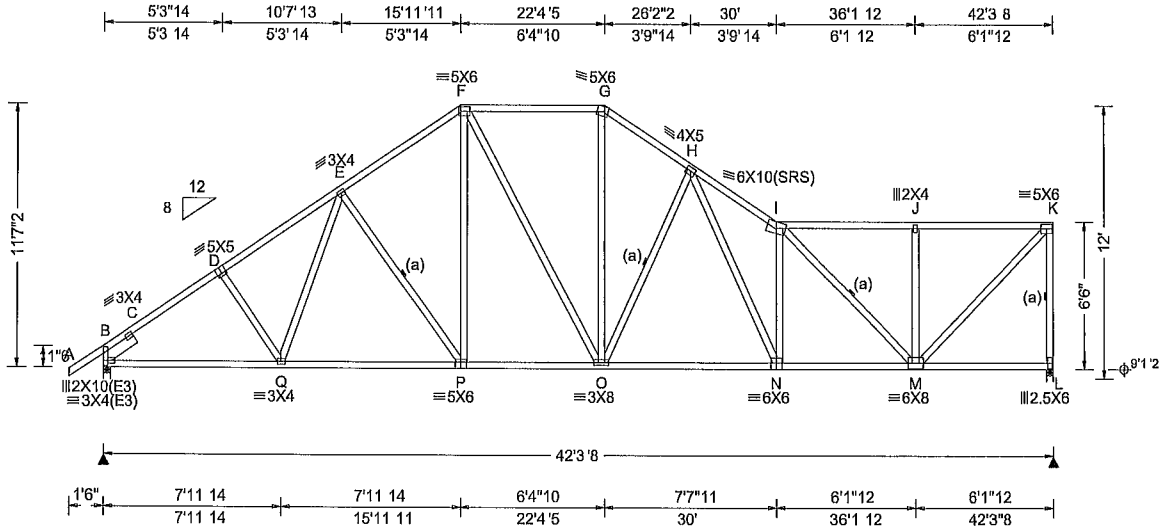
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800241 SPEC FROM CDM	Ply 1 Qty 1	**Job Number:** 25-2238 138 MERCURY CT **Truss Label** G09	Cust: R 215 JRef 1Y7N2150006 T24 DrwNo 050.25 1407.35990 KD / DF 02/19/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg Pf In PSF)	**Defl/CSI Criteria**
TCLL 20 00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#
TCDL 10.00	Speed 130 mph	Pf NA Ce NA	VERT(LL) 0 162 I 999 240
BCLL. 0 00	Enclosure Closed	Lu NA Cs NA	VERT(CL) 0.339 I 999 180
BCDL. 10.00	Risk Category II	Snow Duration NA	HORZ(LL) 0 054 M - -
Des Ld 40 00	EXP B Kzt NA		HORZ(TL) 0 113 M - -
NCBCLL 10 00	Mean Height. 15 93 ft	Building Code	Creep Factor 2 0
Soffit: 2.00	TCDL. 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0 804
Load Duration 1.25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0 758
Spacing 24.0 "	MWFRS Parallel Dist h to 2h	Rep Fac Yes	Max Web CSI 0 842
	C&C Dist a 4 23 ft	FT/RT 20(0)/10(0)	
	Loc from endwall not in 13 00 ft	Plate Type(s)	
	GCpi 0 18	WAVE	VIEW Ver 23 02 04 0123 14
	Wind Duration 1 60		

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Lt Slider 2x6 SP #2, block length = 1 732'

Bracing

(a) Continuous lateral restraint equally spaced on member

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc

Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Wind loading based on both gable and hip roof types

Blocking

Blocking reinforcement required to prevent buckling of members over the bearings
Bearing 1 located at 0 0' (blocking >= 3.50" if used)

Additional Notes

The overall height of this truss excluding overhang is 11-7-2

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
B	1881	/-	/-	/1099	/-	/213
L	1774	/-	/-	/916	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 2 2 (Truss)
L Brg Wid = 3 5 Min Req = 2 1 (Truss)
Bearings B & L are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

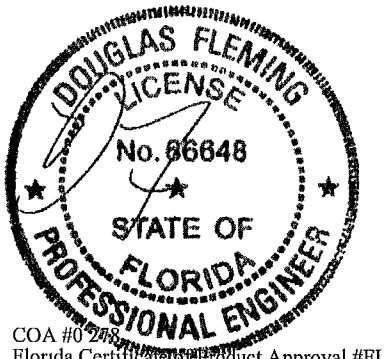
Chords	Tens	Comp	Chords	Tens	Comp
B - C	37	-2608	G - H	122	-2059
C - D	24	-2525	H - I	190	-3092
D - E	61	-2380	I - J	131	-1554
E - F	96	-2000	J - K	130	-1554
F - G	115	-1674			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - Q	1980	-101	O - N	2032	-55
Q - P	1843	-42	N - M	2519	-116
P - O	1581	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp	Webs	Tens	Comp
E - P	125	-455	I - N	219	-993
F - P	536	-34	I - M	0	-1370
O - G	765	-91	J - M	216	-464
O - H	189	-869	M - K	2209	-185
H - N	1230	-161	K - L	205	-1723



COA #0 2418
Florida Certificate of Product Approval #FL1999
02/20/2025

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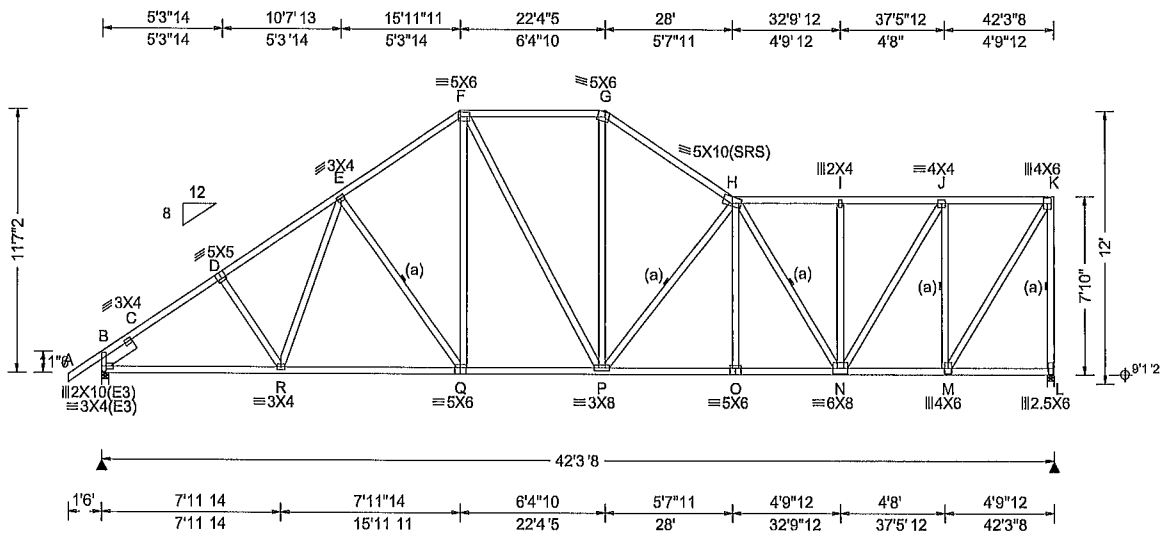
For more information see these web sites Alpine alpineitw com, TPI tpinst.org, SBCA sbcacomponents.com, ICC iccsafe org, AWC awc.org

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building 4th Floor
Glenview, IL 60025

SEQN 800244 SPEC FROM CDM	Ply 1 Qty 1	**Job Number.** 25-2238 138 MERCURY CT **Truss Label.** G10	Cust: R 215 JRef 1Y7N2150006 T22 DrwNo 050 25 1407.37650 KD / DF 02/19/2025	



Loading Criteria (psf)	
TCLL	20.00
TCDL	10.00
BCLL	0 00
BCDL	10 00
Des Ld	40 00
NCBCLL	10 00
Soffit	2.00
Load Duration	1 25
Spacing	24 0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category II
EXP B Kzt NA
Mean Height 15 93 ft
TCDL 5 0 psf
BCDL 5 0 psf
MWFRS Parallel Dist h/2 to h
C&C Dist a 4 23 ft
Loc from endwall not in 13.00 ft
GCpi 0 18
Wind Duration 1 60

Snow Criteria (Pg,Pf in PSF)
Pg NA Ct NA CAT NA
Pf NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac. Yes
FT/RT 20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL) 0 143	P	999	240
VERT(CL) 0.299	P	999	180
HORZ(LL) 0 052	M	-	-
HORZ(TL) 0 109	M	-	-

Creep Factor 2 0
Max TC CSI 0.803
Max BC CSI 0 753
Max Web CSI 0 720

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▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1881	/-	/-	/1104	/-	/210
L	1774	/-	/-	/922	/16	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 2.2 (Truss)
L Brg Wid = 3 5 Min Req = 2 1 (Truss)
Bearings B & L are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - C	37	-2608	G - H	108	-2094
C - D	28	-2524	H - I	143	-1746
D - E	65	-2379	I - J	143	-1746
E - F	99	-2001	J - K	95	-994
F - G	122	-1675			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - R	1980	-139	P - O	2246	-130
R - Q	1844	-79	O - N	2248	-129
Q - P	1581	-7	N - M	1052	-103

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp	Webs	Tens	Comp
E - Q	126	-456	N - J	1349	-81
F - Q	548	-32	J - M	207	-1501
P - G	707	-55	M - K	1890	-180
P - H	181	-952	K - L	213	-1736
H - N	0	-932			

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Lt Slider 2x6 SP #2, block length = 1 732'

Bracing

(a) Continuous lateral restraint equally spaced on member

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

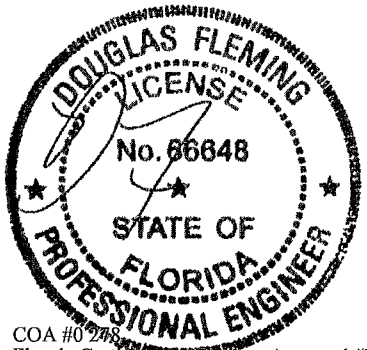
Wind loading based on both gable and hip roof types

Blocking

Blocking reinforcement required to prevent buckling of members over the bearings
Bearing 1 located at 0 0' (blocking >= 3.50" if used)

Additional Notes

The overall height of this truss excluding overhang is 11-7-2



COA #0 2758
Florida Certificate of Product Approval #FL1999
02/20/2025

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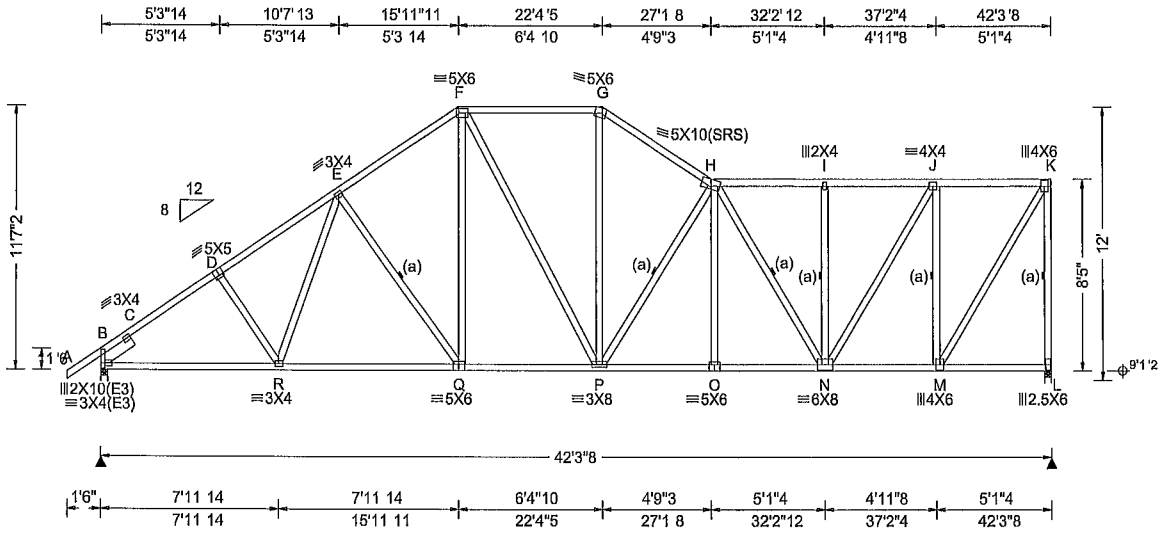
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800247 SPEC FROM CDM	Ply 1 Qty 1	**Job Number:** 25-2238 138 MERCURY CT **Truss Label** G11	Cust: R 215 JRef 1Y7N2150006 T20 DrwNo 050.25 1407.39177 KD / DF 02/19/2025	



Loading Criteria (psf)

TCLL.	20.00
TCDL	10 00
BCLL.	0.00
BCDL	10 00
Des Ld	40 00
NCBCLL	10 00
Soffit:	2 00
Load Duration	1 25
Spacing	24 0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category II
EXP B Kzt NA
Mean Height: 15 93 ft
TCDL 5 0 psf
BCDL 5 0 psf
MWFRS Parallel Dist h/2 to h
C&C Dist a 4.23 ft
Loc from endwall not in 6.50 ft
GCpi 0 18
Wind Duration 1 60

Snow Criteria (Pg Pf In PSF)
Pg NA Ct. NA CAT NA
Pf NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac: Yes
FT/RT 20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL)	0 141 P	999	240
VERT(CL)	0 294 P	999	180
HORZ(LL)	0 051 M	-	-
HORZ(TL)	0 106 M	-	-

Creep Factor 2.0
Max TC CSI 0.803
Max BC CSI 0 752
Max Web CSI 0 713

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▲Maximum Reactions (lbs)

Loc	Gravity R+	Gravity / R-	Gravity / Rh	Non-Gravity / Rw	Non-Gravity / U	Non-Gravity / RL
B	1881	/-	/-	/1107	/-	/209
L	1774	/-	/-	/924	/20	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 2 2 (Truss)
L Brg Wid = 3 5 Min Req = 2 1 (Truss)
Bearings B & L are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp	Chords	Tens	Comp
B - C	37	-2608	G - H	119	-2073
C - D	31	-2524	H - I	147	-1693
D - E	68	-2379	I - J	147	-1693
E - F	99	-2001	J - K	96	-974
F - G	125	-1672			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp	Chords	Tens	Comp
B - R	1980	-155	P - O	2138	-132
R - Q	1844	-95	O - N	2140	-131
Q - P	1582	-24	N - M	1028	-103

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp	Webs	Tens	Comp
E - Q	125	-455	N - J	1306	-88
F - Q	552	-30	J - M	214	-1485
P - G	735	-73	M - K	1871	-184
P - H	184	-919	K - L	221	-1734
H - N	0	-840			

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Lt Slider 2x6 SP #2, block length = 1 732'

Bracing

(a) Continuous lateral restraint equally spaced on member

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc

Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

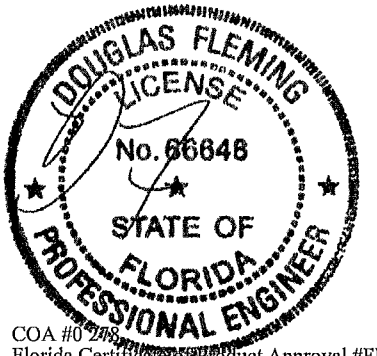
Wind loading based on both gable and hip roof types

Blocking

Blocking reinforcement required to prevent buckling of members over the bearings
Bearing 1 located at 0 0' (blocking >= 3.50" if used)

Additional Notes

The overall height of this truss excluding overhang is 11-7-2



COA #0 2[illegible]
Florida Certificate of Product Approval #FL1999
02/20/2025

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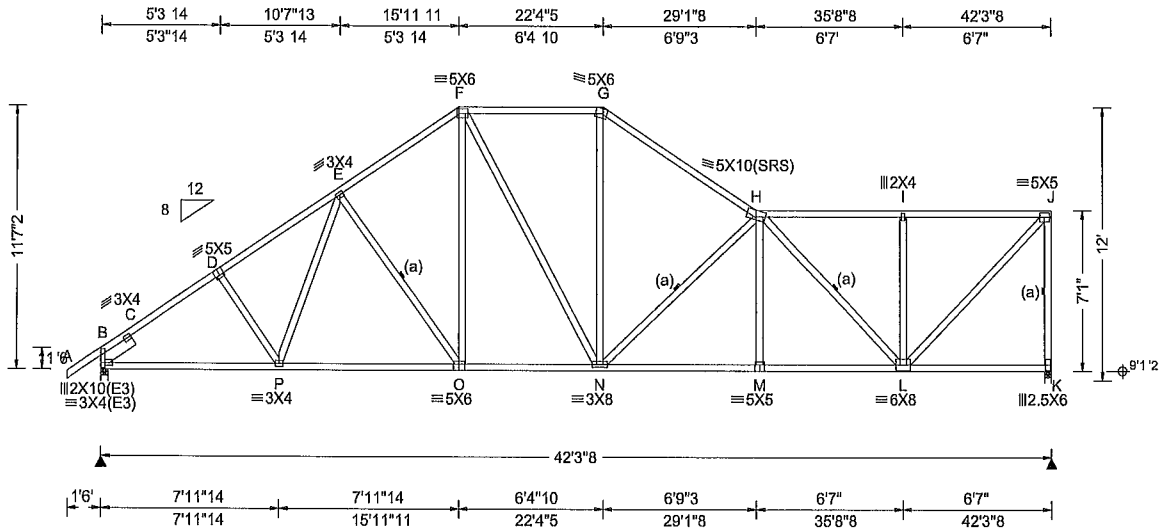
ALPINE
ANITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800250 SPEC FROM CDM	Ply 1 Qty 1	**Job Number** 25-2238 138 MERCURY CT **Truss Label:** G12		Cust R 215 JRef 1Y7N2150006 T18 DrwNo 050.25 1407.40603 KD / DF 02/19/2025



Loading Criteria (psf)	
TCLL	20 00
TCDL	10 00
BCLL	0.00
BCDL	10.00
Des Ld	40.00
NCBCLL	10.00
Soffit	2 00
Load Duration	1.25
Spacing	24 0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category II
EXP B Kzt NA
Mean Height. 15 93 ft
TCDL 5 0 psf
BCDL 5 0 psf
MWFRS Parallel Dist h/2 to h
C&C Dist a 4 23 ft
Loc from endwall not in 6.50 ft
GCpi 0 18
Wind Duration 1 60

Snow Criteria (Pg Pf in PSF)
Pg NA Ct. NA CAT NA
Pf NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac. Yes
FT/RT 20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection In	loc	L/defl	L/#
VERT(LL)	0 145 N	999	240
VERT(CL)	0.303 N	999	180
HORZ(LL)	0 055 L	-	-
HORZ(TL)	0 115 L	-	-

Creep Factor 2 0
Max TC CSI 0.803
Max BC CSI 0 755
Max Web CSI 0 824

VIEW Ver 23.02 04 0123 14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1881	/-	/-	/1102	/-	/212
K	1774	/-	/-	/919	/11	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 2.2 (Truss)
K Brg Wid = 3 5 Min Req = 2 1 (Truss)
Bearings B & K are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - C	37	-2608	F - G	122	-1679
C - D	25	-2524	G - H	102	-2124
D - E	62	-2380	H - I	132	-1506
E - F	98	-2001	I - J	132	-1506

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp	Chords	Tens	Comp
B - P	1980	-118	N - M	2406	-130
P - O	1844	-58	M - L	2409	-129
O - N	1580	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens	Comp	Webs	Tens	Comp
E - O	127	-458	H - L	0	-1296
F - O	543	-34	I - L	216	-478
N - G	683	-37	L - J	2163	-189
N - H	184	-1030	J - K	213	-1721

Lumber
Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Lt Slider 2x6 SP #2, block length = 1 732'

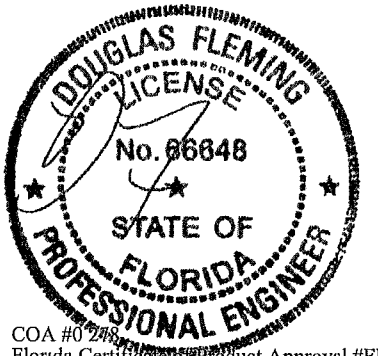
Bracing
(a) Continuous lateral restraint equally spaced on member

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc

Wind
Wind loads based on MWFRS with additional C&C member design
Right end vertical not exposed to wind pressure
Wind loading based on both gable and hip roof types

Blocking
Blocking reinforcement required to prevent buckling of members over the bearings
Bearing 1 located at 0 0' (blocking >= 3.50" if used)

Additional Notes
The overall height of this truss excluding overhang is 11-7-2



COA #0 248
Florida Certificate of Product Approval #FL1999
02/20/2025

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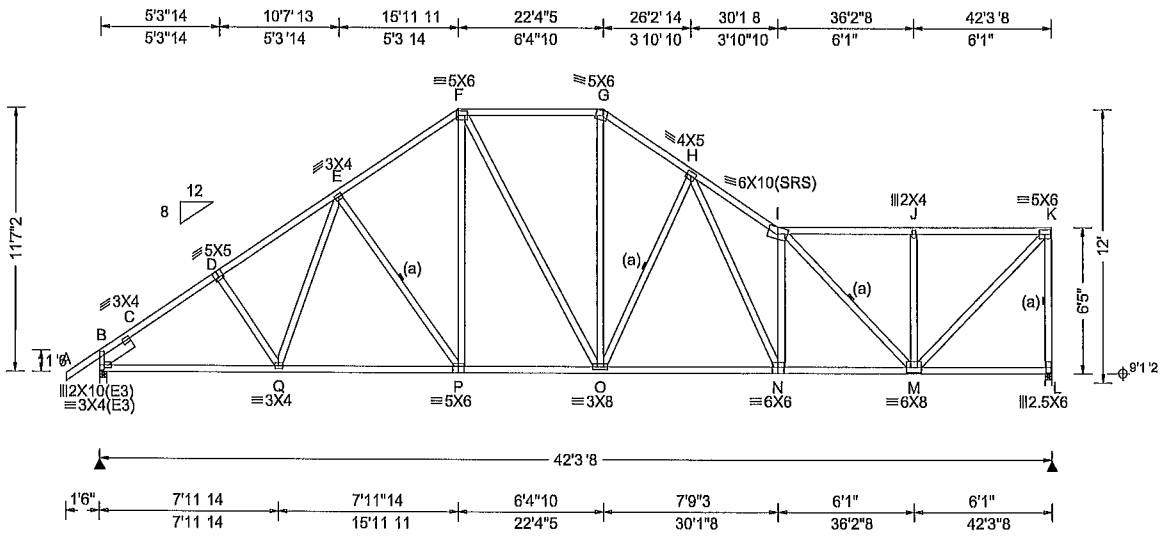
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800253 SPEC FROM CDM	Ply 1 Qty 1	**Job Number:** 25-2238 138 MERCURY CT **Truss Label** G13	Cust: R 215 JRef 1Y7N2150006 T13 DrwNo 050.25 1407.41990 KD / DF 02/19/2025	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg Pf in PSF)	Defl/CSI Criteria
TCLL. 20 00	Wind Std ASCE 7-22	Pg NA Ct. NA CAT NA	PP Deflection in loc L/defl L/#
TCDL 10 00	Speed 130 mph	Pf NA Ce NA	VERT(LL) 0 163 I 999 240
BCLL 0 00	Enclosure Closed	Lu NA Cs NA	VERT(CL) 0.340 I 999 180
BCDL. 10 00	Risk Category II	Snow Duration NA	HORZ(LL) 0 054 M - -
Des Ld 40 00	EXP B Kzt: NA		HORZ(TL) 0 113 M - -
NCBCLL 10.00	Mean Height: 15.93 ft	Building Code	Creep Factor 2.0
Soffit: 2.00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0.804
Load Duration 1 25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0 758
Spacing 24.0 "	MWFRS Parallel Dist h/2 to h	Rep Fac Yes	Max Web CSI 0.844
	C&C Dist a 4 23 ft	FT/RT 20(0)/10(0)	
	Loc from endwall not in 6 50 ft	Plate Type(s)	
	GCpi 0 18	WAVE	VIEW Ver 23.02.04 0123 14
	Wind Duration 1 60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1881	/-	/-	/1099	/-	/213
L	1774	/-	/-	/916	/7	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 2 2 (Truss)
L Brg Wid = 3 5 Min Req = 2 1 (Truss)
Bearings B & L are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - C	37	-2608	G - H	121	-2061
C - D	24	-2525	H - I	192	-3115
D - E	61	-2380	I - J	130	-1561
E - F	96	-2000	J - K	130	-1561
F - G	115	-1674			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - Q	1980	-99	O - N	2039	-54
Q - P	1843	-40	N - M	2538	-115
P - O	1581	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp	Webs	Tens	Comp
E - P	125	-455	I - N	218	-996
F - P	535	-34	I - M	0	-1385
O - G	764	-90	J - M	214	-459
O - H	189	-871	M - K	2215	-185
H - N	1239	-160	K - L	204	-1723

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Lt Slider 2x6 SP #2, block length = 1 732'

Bracing

(a) Continuous lateral restraint equally spaced on member

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

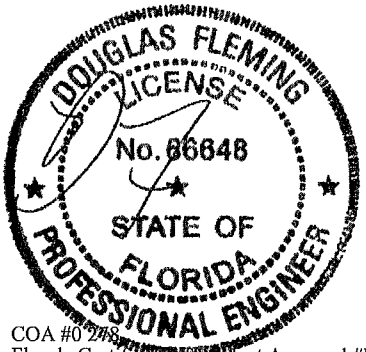
Wind loading based on both gable and hip roof types

Blocking

Blocking reinforcement required to prevent buckling of members over the bearings
Bearing 1 located at 0 0' (blocking >= 3.50" if used)

Additional Notes

The overall height of this truss excluding overhang is 11-7-2



COA #0 2485
Florida Certificate of Product Approval #FL1999
02/20/2025

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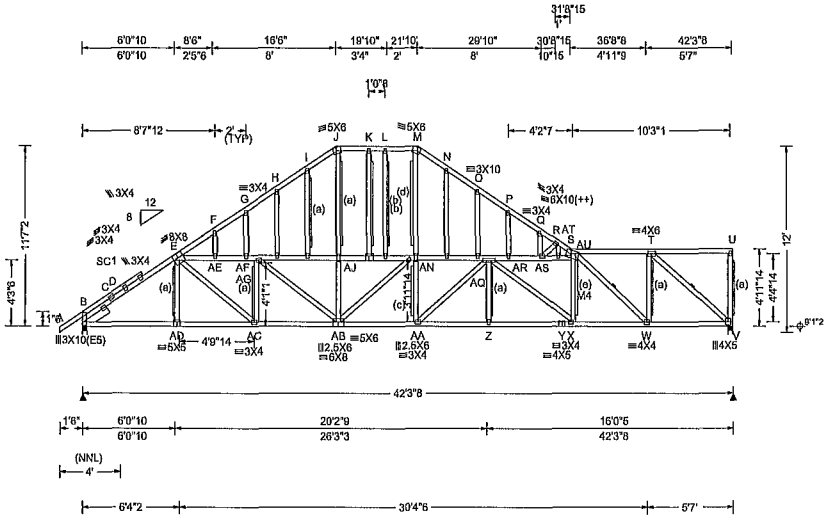
For more information see these web sites Alpine alpineitw com, TPI tpinst.org, SBCA sbcacomponents.com, ICC. iccsafe.org, AWC awc.org

ALPINE
ANITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800268 GABL	Ply 1	**Job Number** 25-2238	Cust R 215 JRef 1Y7N2150006 T11
FROM CDM	Qty 1	138 MERCURY CT	DrwNo 050.25 1408.05537
Page 1 of 2		**Truss Label.** G14	SSB / FV 02/19/2025



Loading Criteria (psf)	
TCLL	20 00
TCDL	10 00
BCLL	0 00
BCDL	10 00
Des Ld	40 00
NCBCLL	10 00
Soffit	2 00
Load Duration	1.25
Spacing	24 0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category II
EXP B Kzt NA
Mean Height 15 75 ft
TCDL 5 0 psf
BCDL 5 0 psf
MWFRS Parallel Dist 0 to h/2
C&C Dist a 4 23 ft
Loc from endwall not in 6 50 ft
GCpi 0 18
Wind Duration 1.60

Snow Criteria (Pg Pf in PSF)
Pg NA Ct NA CAT NA
Pf NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac. Yes
FT/RT 20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection In	loc	L/defl	L/#
VERT(LL)	0 210 Q	999	240
VERT(CL)	0.439 Q	999	180
HORZ(LL)	0 074 V	-	-
HORZ(TL)	0 155 V	-	-

Creep Factor 2.0
Max TC CSI 0.469
Max BC CSI 0 715
Max Web CSI 0.991

VIEW Ver 23 02 04 0123 14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1882	/-	/-	/1070	/-	/196
V	1775	/-	/-	/920	/6	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 2.2 (Truss)
V Brg Wid = 3 5 Min Req = 2 1 (Truss)
Bearings B & V are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - C	0	-2577	K - L	124	-1590
C - D	0	-2575	L - M	124	-1591
D - E	11	-2651	M - N	130	-1901
E - F	0	-1981	N - O	109	-1957
F - G	7	-1911	O - P	53	-1910
G - H	56	-1935	P - Q	5	-1894
H - I	100	-1943	Q - R	8	-2118
I - J	123	-1898	R - S	78	-2699
J - K	124	-1591	S - T	144	-1872

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B -AD	2161	-41	Z - Y	2708	-111
AD-AC	2163	-37	Y - X	2708	-111
AC-AB	2153	-41	X - W	3012	-148
AB-AA	1794	0	W - V	1798	-147
AA- Z	2708	-111			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp	Webs	Tens	Comp
E -AE	86	-577	AR-AS	207	-1495
AE-AF	87	-578	AS- R	130	-986
AF-AG	89	-583	AS-AT	115	-789
AG-AB	143	-690	R -AT	590	-87
AJ-AB	718	-65	AT-AU	109	-751
AN-AA	919	-101	S - W	6	-1567
AA-AQ	218	-1208	W - T	1196	0
AQ-AR	209	-1500	T - V	195	-2394
AQ- X	473	-59			

Maximum Gable Forces Per Ply (lbs)

Gables	Tens	Comp	Gables	Tens	Comp
J -AJ	810	-63	S -AU	119	-443

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3, M4 2x4 SP #2,
Stack Chord SC1 2x4 SP #2,
Lt Slider 2x6 SP #2, block length = 1 797'

Plating Notes

All plates are 2X4 except as noted

(++) - This plate works for both joints covered

Loading

Gable end supports 8" max rake overhang Top chord must not be cut or notched

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc

Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Wind loading based on both gable and hip roof types

Gable meets L/120 deflection criteria for wind load applied to face Calculated deflection ratio is L/173

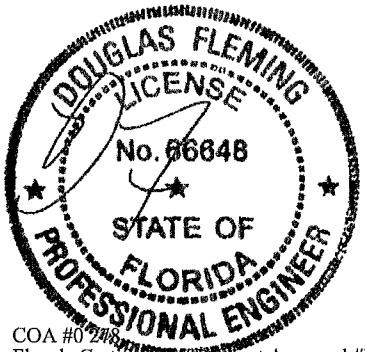
Laterally brace top chord below filler and bottom chord above filler at 24" o c ,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1

Stacked top chord must NOT be notched or cut in area (NNL) Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

The overall height of this truss excluding overhang is 11-7-2



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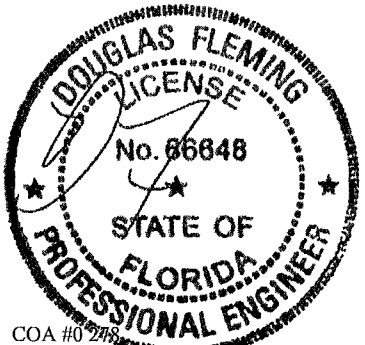
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Gable Reinforcement AN-M 827 -72

(a) 1x4 "L" reinforcement. Any species and grade 80% length of web member Attach with 10d (0 131"x3",min) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder
(b) 1x4 "L" reinforcement. Same species and grade as web 80% length of web member Attach with 10d (0 131"x3",min) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder
(c) 2x4 "L" reinforcement. Same species and grade as web 80% length of web member Attach with 10d (0 131"x3",min) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder
(d) 2x4 "L" reinforcement Any species and grade 80% length of web member Attach with 10d (0 131"x3",min) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder
(e) 2x6 "L" reinforcement. Any species and grade 80% length of web member Attach with 10d (0 131"x3",min) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder



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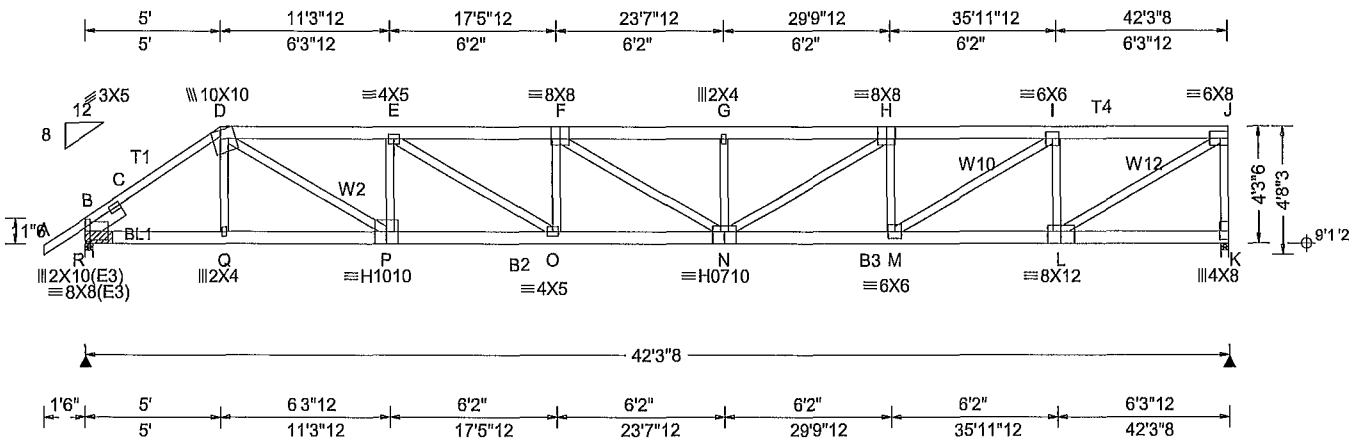
155 Harlem Ave
North Building 4th Floor
Glenview, IL 60025

SEQN 800147 HIPM FROM CDM	Ply 1 Qty 1	**Job Number.** 25-2238 138 MERCURY CT **Truss Label** G15		Cust. R 215 JRef 1Y7N2150006 T19 DrwNo 050.25 1408 15150 KD / DF 02/19/2025



Loading Criteria (psf)

TCLL	20 00
TCDL	10 00
BCLL.	0.00
BCDL	10.00
Des Ld	40.00
NCBCLL	10.00
Soffit	2 00
Load Duration	1.25
Spacing	24.0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category II
EXP B Kzt: NA
Mean Height: 15.00 ft
TCDL 5 0 psf
BCDL. 5 0 psf
MWFRS Parallel Dist 0 to h/2
C&C Dist a 4 23 ft
Loc from endwall not in 6 50 ft
GCpi 0 18
Wind Duration 1 60

Snow Criteria (Pg Pf In PSF)
Pg NA Ct NA CAT NA
Pf NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac: Varies by Ld Case
FT/RT 20(0)/10(0)
Plate Type(s)
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL)	0.425 G	999	240
VERT(CL)	0.860 G	589	180
HORZ(LL)	0 086 D	-	-
HORZ(TL)	0 174 D	-	-

Creep Factor 2 0
Max TC CSI 0 852
Max BC CSI 0 788
Max Web CSI 0 789

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▲Maximum Reactions (lbs)

	Gravity		Non-Gravity		
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
R	3142 /-	/-	/-	/375	/-
K	2952 /-	/-	/-	/414	/-

Wind reactions based on MWFRS
R Brg Wid = 3 5 Min Req = -
K Brg Wid = 3 5 Min Req = 3.5 (Truss)
Bearings R & K are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - C	578	-4618	F - G	1144	-8237
C - D	568	-4562	G - H	1144	-8237
D - E	893	-6638	H - I	959	-6868
E - F	1113	-8110	I - J	590	-4209

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp	Chords	Tens	Comp
B - Q	3672	-456	O - N	8152	-1122
Q - P	3672	-452	N - M	6966	-975
P - O	6750	-911	M - L	4378	-616

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp	Webs	Tens	Comp
D - P	3494	-519	H - M	204	-1292
P - E	249	-1492	M - I	2970	-409
E - O	1622	-241	I - L	343	-2223
O - F	113	-547	L - J	4939	-693
G - N	100	-504	J - K	407	-2814
N - H	1506	-201			

Lumber

Top chord 2x6 SP 2400f-2 0E, T1 2x4 SP #2,
T4 2x6 SP #2,
Bot chord 2x6 SP #2, B2,B3 2x6 SP 2400f-2 0E,
Webs 2x4 SP #3, W2,W10 2x4 SP #2,
W12 2x4 SP M-31,
Lt Slider 2x6 SP #2, block length = 1.579'

Special Loads

------(Lumber Dur Fac =1 25 / Plate Dur Fac.=1 25)

TC	From	64 plf at	-1 50 to	64 plf at	5.00
TC	From	32 plf at	5 00 to	32 plf at	42 29
BC	From	5 plf at	-1.50 to	5 plf at	0.00
BC	From	20 plf at	0 00 to	20 plf at	5.03
BC	From	10 plf at	5 03 to	10 plf at	42 29

TC 288 lb Conc Load at 5 03
TC 103 lb Conc Load at 7 06, 9 06,11.06,13 06 15 06,17 06,19.06,21 06,23 06,25 06,27.06,29.06 31 06,33 06,35.06,37 06,39 06,41 06
BC 212 lb Conc Load at 5 03
BC 92 lb Conc Load at 7.06, 9 06,11.06,13 06 15.06,17 06,19.06,21.06,23 06,25 06,27 06,29 06 31 06,33 06,35.06,37.06,39 06,41 06

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc

Wind

Wind loads and reactions based on MWFRS
Right end vertical not exposed to wind pressure
Wind loading based on both gable and hip roof types

Bearing Block(s)

Brg blocks 0 131"x3", min nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
1	0 000'	1	12"	4	Rigid Surface

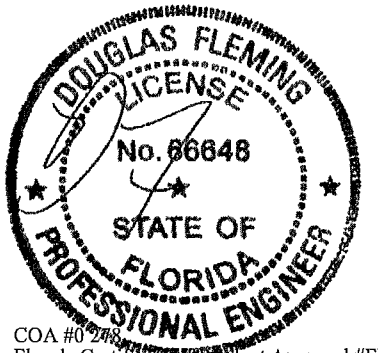
Brg block to be same size and species as chord
Refer to drawing CNNAILSP1014 for more information

Blocking

Blocking reinforcement required to
prevent buckling of members over the bearings
Bearing 1 located at 0 0' (blocking >= 3.50" if used)

Additional Notes

The overall height of this truss excluding overhang is 4-3-6



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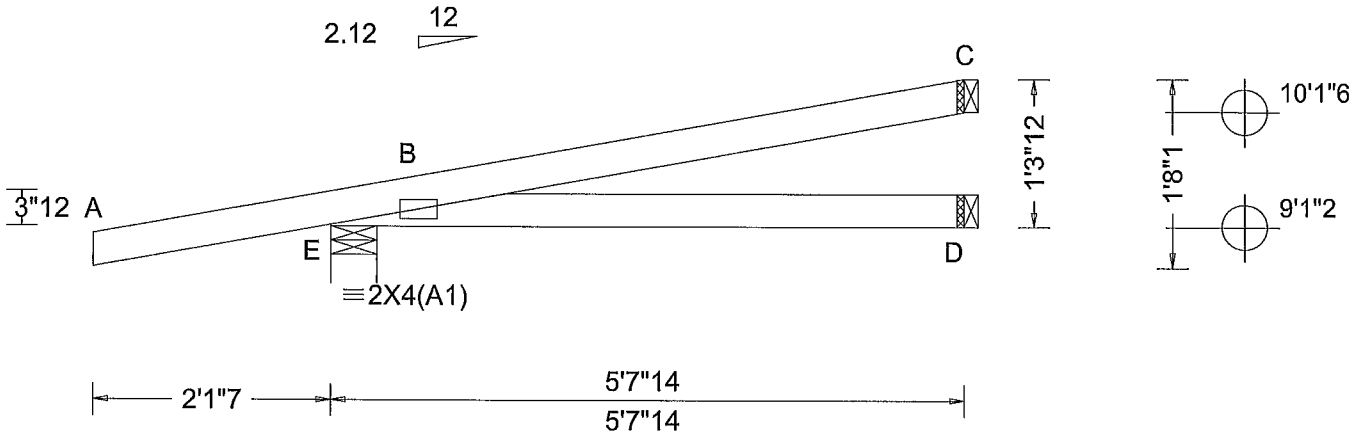
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800189 HIP_ FROM CDM	Ply 1 Qty 2	**Job Number:** 25-2238 138 MERCURY CT **Truss Label** HJ01	Cust: R 215 JRef 1Y7N2150006 T29 DrwNo 050 25 1408 16657 KD / DF 02/19/2025	



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg Pf in PSF)	**Defl/CSI Criteria**	**▲Maximum Reactions (lbs)**
TCLL	20.00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#	Gravity / Non-Gravity
TCDL	10.00	Speed 130 mph	Pf NA Ce NA	VERT(LL) NA	Loc R+ / R- / Rh / Rw / U / RL
BCLL	0 00	Enclosure Closed	Lu NA Cs NA	VERT(CL) NA	E 213 /- /- /- /21 /-
BCDL	10 00	Risk Category II	Snow Duration NA	HORZ(LL) 0 003 B - -	D 51 /- /- /18 /- /-
Des Ld	40 00	EXP B Kzt: NA		HORZ(TL) 0 006 B - -	C 123 /- /- /- /24 /-
NCBCLL	0 00	Mean Height: 15 00 ft	Building Code	Creep Factor 2 0	Wind reactions based on MWFRS
Soffit	2.00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0.284	E Brg Wid = 4 9 Min Req = 1.5 (Truss)
Load Duration	1 25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0 164	D Brg Wid = 1 5 Min Req = -
Spacing	24 0 "	MWFRS Parallel Dist 0 to h/2	Rep Fac: No	Max Web CSI 0 000	C Brg Wid = 1 5 Min Req = -
		C&C Dist a 3 00 ft	FT/RT 20(0)/10(0)		Bearing E is a rigid surface
		Loc. from endwall NA	Plate Type(s)		Members not listed have forces less than 375#
		GCpi 0 18	WAVE	VIEW Ver 23.02 04 0123 14	
		Wind Duration 1.60			

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,

Loading

Hipjack supports 4-0-0 setback jacks with no webs

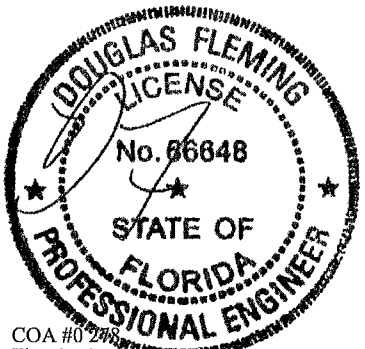
Wind

Wind loads and reactions based on MWFRS

Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 1-3-12



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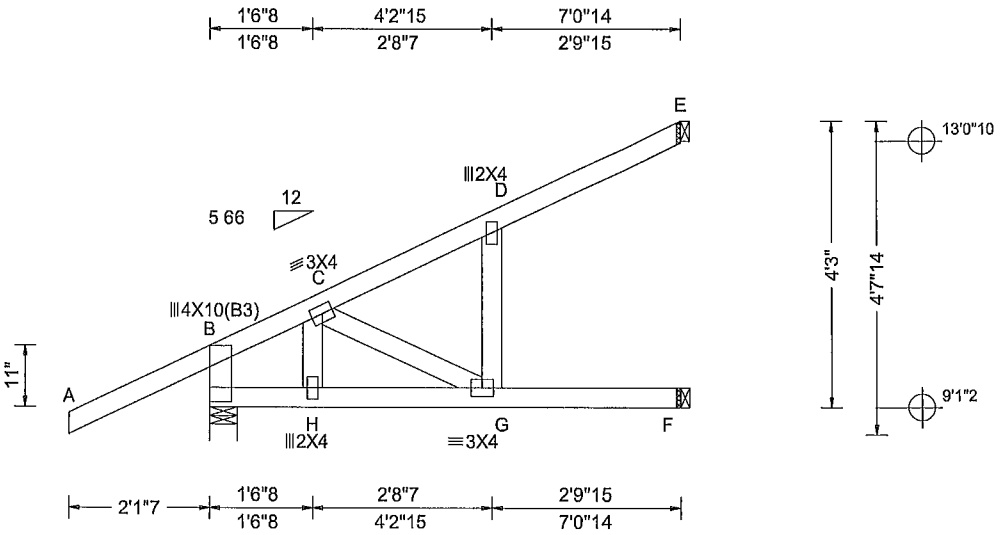
Glenview, IL 60025

SEQN 800143 HIP_ FROM CDM	Ply 1 Qty 1	**Job Number** 25-2238 138 MERCURY CT **Truss Label.** HJ02		Cust: R 215 JRef 1Y7N2150006 T38 DrwNo 050 25 1408 18023 KD / DF 02/19/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg Pf in PSF)	**Defl/CSI Criteria**
TCLL 20.00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#
TCDL 10.00	Speed 130 mph	Pf NA Ce NA	VERT(LL) 0.085 D 994 240
BCLL. 0 00	Enclosure Closed	Lu NA Cs NA	VERT(CL) 0 172 D 493 180
BCDL 10 00	Risk Category II	Snow Duration NA	HORZ(LL) 0 047 D - -
Des Ld 40 00	EXP B Kzt: NA		HORZ(TL) 0 095 D - -
NCBCLL 0 00	Mean Height: 15.00 ft	Building Code	Creep Factor 2.0
Soffit: 2.00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0.514
Load Duration 1 25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0.480
Spacing 24.0 "	MWFRS Parallel Dist 0 to h/2	Rep Fac: No	Max Web CSI 0.087
	C&C Dist a 3 00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall NA	Plate Type(s)	
	GCpi 0 18	WAVE	VIEW Ver 23.02.04 0123 14
	Wind Duration 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	308	/-	/-	/-	/18	/-
F	121	/-	/-	/15	/-	/-
E	185	/-	/-	/-	/20	/-

Wind reactions based on MWFRS
B Brg Wid = 4 9 Min Req = 1 5 (Truss)
F Brg Wid = 1 5 Min Req = -
E Brg Wid = 1 5 Min Req = -
Bearing B is a rigid surface
Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Lt Stub Wedge 2x6 SP #2,

Loading

Hipjack supports 5-0-0 setback jacks with no webs

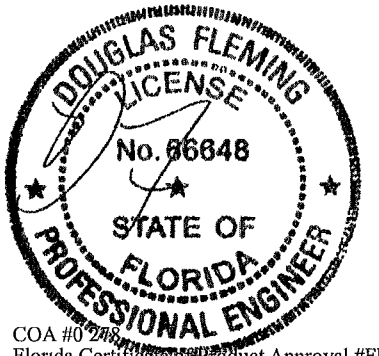
Wind

Wind loads and reactions based on MWFRS

Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 4-3-0



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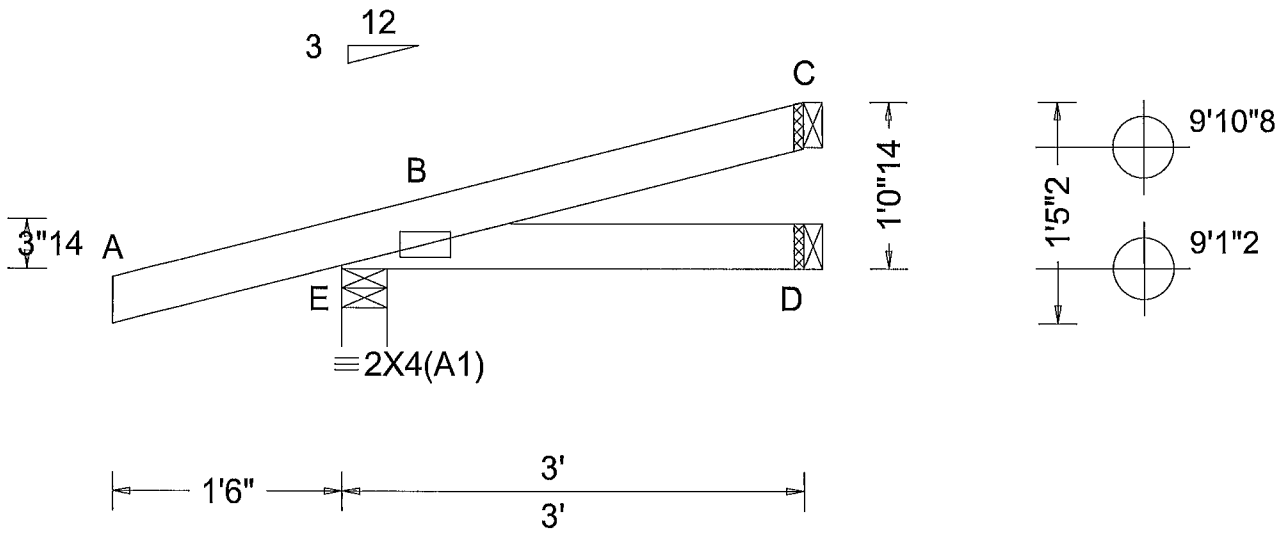
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800183 JACK FROM CDM	Ply 1 Qty 4	**Job Number** 25-2238 138 MERCURY CT **Truss Label** J02		Cust: R 215 JRef 1Y7N2150006 T27 DrwNo. 050.25 1408.33387 KD / DF 02/19/2025



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL	20.00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#
TCDL	10.00	Speed 130 mph	Pf NA Ce NA	VERT(LL) NA
BCLL.	0.00	Enclosure Closed	Lu NA Cs NA	VERT(CL) NA
BCDL	10.00	Risk Category II	Snow Duration NA	HORZ(LL) 0 000 B - -
Des Ld	40.00	EXP B Kzt: NA		HORZ(TL) 0 001 B - -
NCBCLL	10.00	Mean Height: 15 00 ft	Building Code	Creep Factor 2 0
Soffit.	2.00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0 148
Load Duration	1.25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0.058
Spacing	24.0 "	MWFRS Parallel Dist 0 to h/2	Rep Fac Yes	Max Web CSI 0.000
		C&C Dist a 3 00 ft	FT/RT 20(0)/10(0)	
		Loc from endwall Any	Plate Type(s)	
		GCpi 0 18	WAVE	
		Wind Duration 1 60		VIEW Ver 23 02 04 0123 14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	255	/-	/-	/132	/42	/28
D	46	/-	/-	/25	/-	/-
C	59	/-	/-	/26	/12	/-

Wind reactions based on MWFRS
E Brg Wid = 3 5 Min Req = 1.5 (Truss)
D Brg Wid = 1 5 Min Req = -
C Brg Wid = 1 5 Min Req = -
Bearing E is a rigid surface
Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,

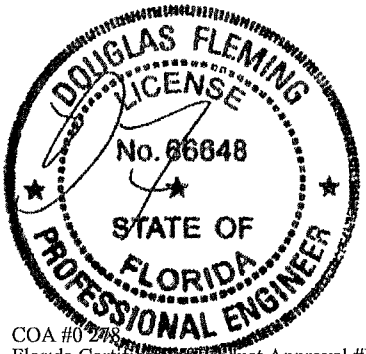
Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 1-0-14



COA #0 [illegible]
Florida Certificate of Product Approval #FL1999
02/20/2025

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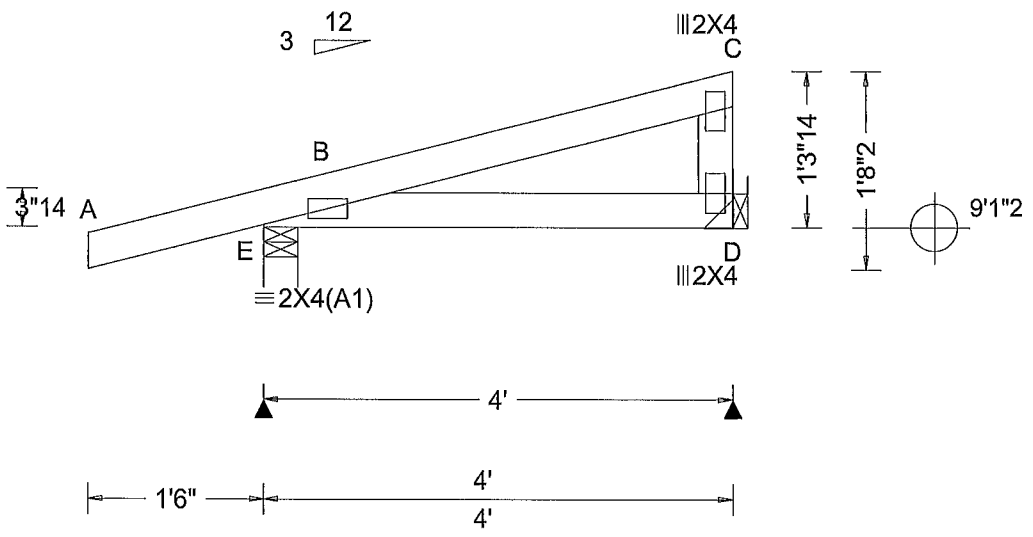
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800191 MONO FROM CDM Page 1 of 2	Ply 1 Qty 1	**Job Number:** 25-2238 138 MERCURY CT **Truss Label:** J03		Cust: R 215 JRef 1Y7N2150008 T26 DrwNo 050 25.1408.41030 KD / DF 02/19/2025



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg Pf in PSF)	**Defl/CSI Criteria**	**▲Maximum Reactions (lbs)**
TCLL	20.00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#	Gravity / Non-Gravity
TCDL	10.00	Speed 130 mph	Pf NA Ce NA	VERT(LL) NA	Loc R+ / R- / Rh / Rw / U / RL
BCLL	0 00	Enclosure Closed	Lu NA Cs NA	VERT(CL) NA	E 287 /- /- /- /33 /-
BCDL	10.00	Risk Category II	Snow Duration NA	HORZ(LL) 0 002 B - -	D 307 /- /- /- /8 /-
Des Ld	40 00	EXP B Kzt: NA		HORZ(TL) 0 003 B - -	Wind reactions based on MWFRS
NCBCLL	10 00	Mean Height 15 00 ft	Building Code	Creep Factor 2 0	E Brg Wid = 3 5 Min Req = 1.5 (Truss)
Soffit:	2.00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0 169	D Brg Wid = - Min Req = -
Load Duration	1.25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0 119	Bearing E is a rigid surface
Spacing	24.0 "	MWFRS Parallel Dist 0 to h/2	Rep Fac Varies by Ld Case	Max Web CSI 0.032	Members not listed have forces less than 375#
		C&C Dist a 3 00 ft	FT/RT 20(0)/10(0)		
		Loc from endwall not in 4.50 ft	Plate Type(s)		
		GCpi 0 18	WAVE		
		Wind Duration 1 60		VIEW Ver 23 02 04.0123 14	

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,

Special Loads

------(Lumber Dur Fac.=1 25 / Plate Dur Fac.=1.25)

TC	From	61 plf at	-1 50 to	61 plf at	4.00
BC	From	4 plf at	-1 50 to	4 plf at	0 00
BC	From	20 plf at	0 00 to	20 plf at	4 00
TC	123 lb Conc Load at 4 00				
BC	51 lb Conc Load at 4 00				

Wind

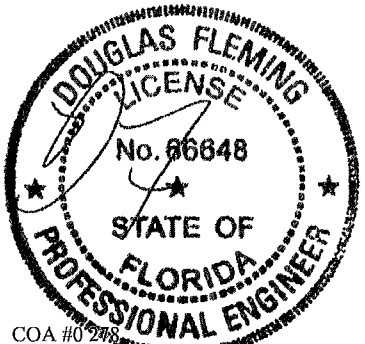
Wind loads and reactions based on MWFRS

Right end vertical not exposed to wind pressure

Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 1-3-14



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02/20/2025

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155 Harlem Ave
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Glenview, IL 60025

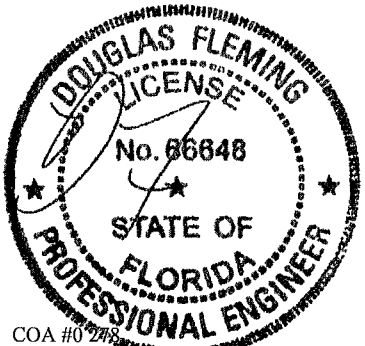
Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie Please refer to the most recent Simpson Strong-Tie catalog for additional information

Recommended hanger connections are based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

Bearing at location x=3'9" ,y=9'1"2 uses the following support conditions 3'9"
Bearing D (3'9", 9'1"2) LUS26
Supporting Member: (1)2x6 SP #2
(4) 0 148"x3" nails into supporting member,
(3) 0 148"x3" nails into supported member



COA #0 248
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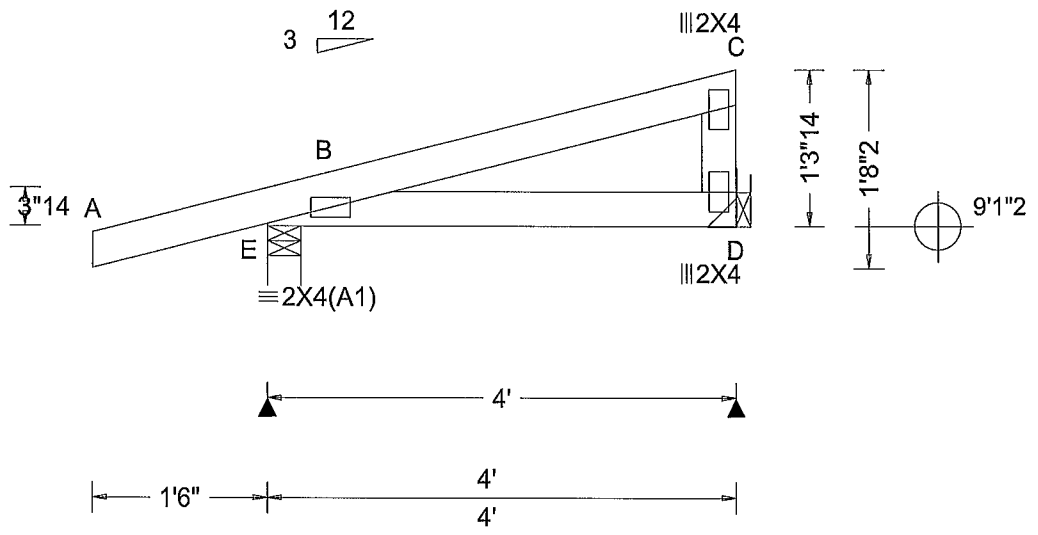
02/20/2025

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800185 MONO FROM CDM	Ply 1 Qty 5	**Job Number:** 25-2238 138 MERCURY CT **Truss Label** J04		Cust: R 215 JRef 1Y7N2150006 T17 DrwNo 050.25 1408.43500 KD / DF 02/19/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg Pf in PSF)	**Defl/CSI Criteria**
TCLL 20.00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#
TCDL 10.00	Speed 130 mph	Pf NA Ce NA	VERT(LL) NA
BCLL 0.00	Enclosure Closed	Lu NA Cs NA	VERT(CL) NA
BCDL 10.00	Risk Category II	Snow Duration NA	HORZ(LL) 0 002 B - -
Des Ld 40 00	EXP B Kzt: NA		HORZ(TL) 0 003 B - -
NCBCLL 10 00	Mean Height 15 00 ft	Building Code	Creep Factor 2 0
Soffit 2 00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0 148
Load Duration 1 25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0 119
Spacing 24 0 "	MWFRS Parallel Dist h/2 to h	Rep Fac. Yes	Max Web CSI 0.033
	C&C Dist a 3 00 ft	FT/RT.20(0)/10(0)	
	Loc from endwall not in 4 50 ft	Plate Type(s)	
	GCpi 0 18	WAVE	VIEW Ver 23.02 04 0123 14
	Wind Duration 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	287	/-	/-	/148	/39	/33
D	133	/-	/-	/74	/7	/-

Wind reactions based on MWFRS
E Brg Wid = 3 5 Min Req = 1.5 (Truss)
D Brg Wid = - Min Req = -
Bearing E is a rigid surface
Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,

Hangers / Ties

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Bearing at location x=3'9" ,y=9'1"2 uses the following support conditions 3'9"
Bearing D (3'9", 9'1"2) LUS26
Supporting Member (1)2x6 SP #2
(4) 0 148"x3" nails Into supporting member,
(3) 0 148"x3" nails Into supported member

Additional Notes

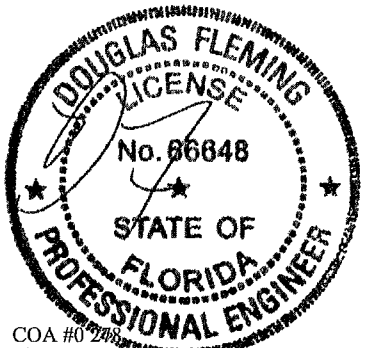
The overall height of this truss excluding overhang is 1-3-14.

Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Wind loading based on both gable and hip roof types



COA #0 2[illegible]
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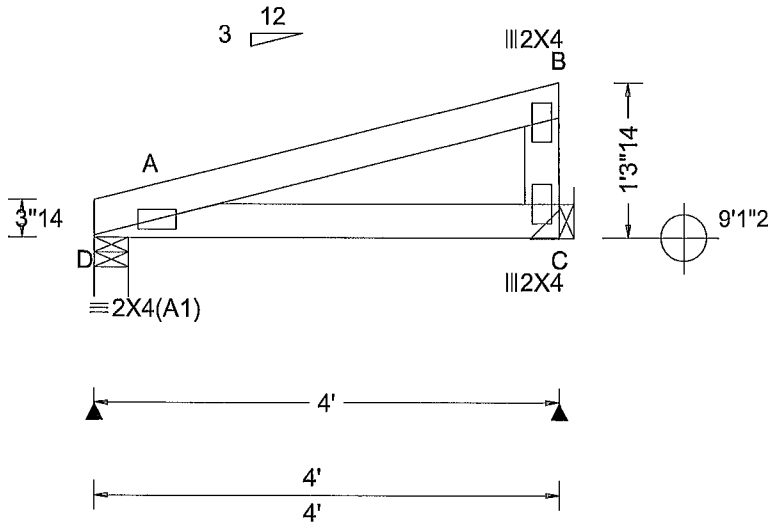
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800187 MONO FROM CDM	Ply: 1 Qty: 2	**Job Number:** 25-2238 138 MERCURY CT **Truss Label:** J05	Cust: R 215 JRef: 1Y7N2150006 T14 DrwNo. 050 25 1408.45203 KD / DF 02/19/2025



Loading Criteria (psf)	
TCLL	20 00
TCDL	10 00
BCLL	0.00
BCDL	10 00
Des Ld	40 00
NCBCLL	10 00
Soffit:	2 00
Load Duration	1 25
Spacing	24 0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category: II
EXP B Kzt: NA
Mean Height 15 00 ft
TCDL 5 0 psf
BCDL. 5 0 psf
MWFRS Parallel Dist h/2 to h
C&C Dist a 3 00 ft
Loc. from endwall not in 9.00 ft
GCpi 0 18
Wind Duration 1 60

Snow Criteria (Pg Pf in PSF)
Pg NA Ct. NA CAT NA
Pf: NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac. Yes
FT/RT 20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL)	NA		
VERT(CL)	NA		
HORZ(LL)	0 003	A	- -
HORZ(TL)	0 006	A	- -

Creep Factor: 2 0
Max TC CSI 0 184
Max BC CSI 0 170
Max Web CSI 0 041

VIEW Ver: 23.02.04 0123 14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D	167	/-	/-	/86	/3	/24
C	155	/-	/-	/80	/10	/-

Wind reactions based on MWFRS
D Brg Wid = 3 5 Min Req = 1 5 (Truss)
C Brg Wid = - Min Req = -
Bearing D is a rigid surface
Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,

Hangers / Ties

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Recommended hanger connections are based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

Bearing at location x=3'9" ,y=9'1"2 uses the following support conditions 3'9"
Bearing C (3'9", 9'1"2) LUS26
Supporting Member: (1)2x6 SP #2
(4) 0 148"x3" nails into supporting member,
(3) 0 148"x3" nails into supported member

Additional Notes

The overall height of this truss excluding overhang is 1-3-14

Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Wind loading based on both gable and hip roof types

DOUGLAS FLEMING LICENSE No. 86848 STATE OF FLORIDA PROFESSIONAL ENGINEER

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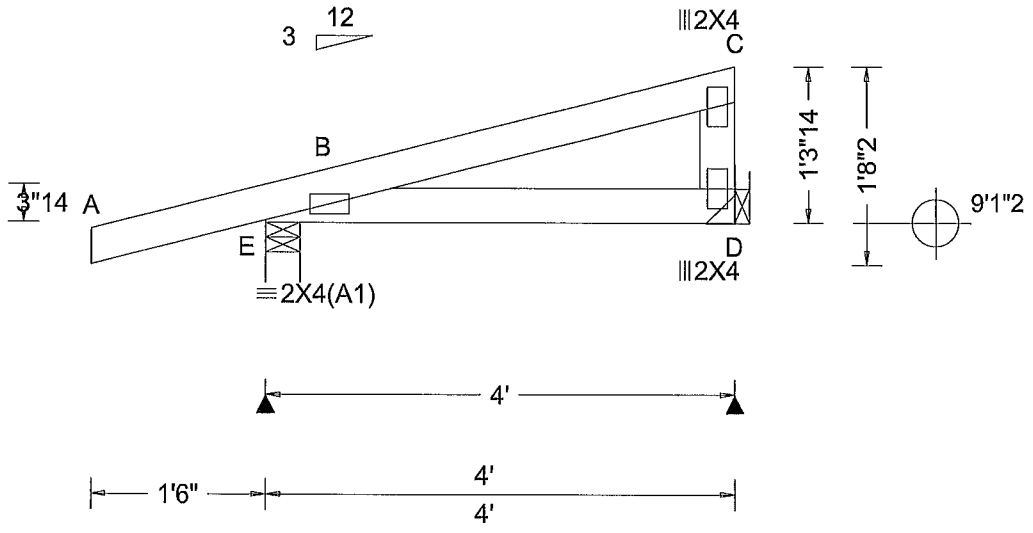
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800193 MONO FROM CDM	Ply· 1 Qty· 1	**Job Number:** 25-2238 138 MERCURY CT **Truss Label** J06		Cust: R 215 JRef· 1Y7N2150006 T32 DrwNo 050.25 1408.46800 KD / DF 02/19/2025



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg Pf In PSF)	**Defl/CSI Criteria**
TCLL	20 00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#
TCDL	10 00	Speed 130 mph	Pf· NA Ce NA	VERT(LL) NA
BCLL	0 00	Enclosure Closed	Lu NA Cs NA	VERT(CL) NA
BCDL	10 00	Risk Category II	Snow Duration NA	HORZ(LL) 0 002 B - -
Des Ld	40.00	EXP B Kzt NA		HORZ(TL) 0 003 B - -
NCBCLL	10.00	Mean Height 15 00 ft	Building Code	Creep Factor· 2 0
Soffit	2 00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0 148
Load Duration	1 25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0 119
Spacing	24.0 "	MWFRS Parallel Dist 0 to h/2	Rep Fac. Yes	Max Web CSI 0 033
		C&C Dist a 3 00 ft	FT/RT·20(0)/10(0)	
		Loc from endwall not in 4.50 ft	Plate Type(s)	
		GCpi 0 18	WAVE	
		Wind Duration 1 60		VIEW Ver· 23 02.04 0123 14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	287	/-	/-	/148	/39	/33
D	133	/-	/-	/74	/7	/-

Wind reactions based on MWFRS
E Brg Wid = 3 5 Min Req = 1 5 (Truss)
D Brg Wid = - Min Req = -
Bearing E is a rigid surface
Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,

Wind

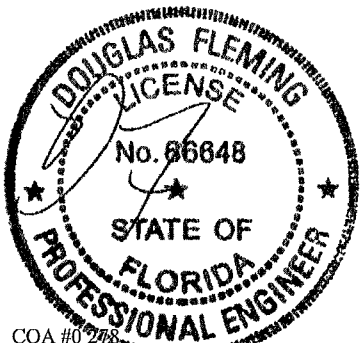
Wind loads based on MWFRS

Right end vertical not exposed to wind pressure

Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 1-3-14



COA #0 2782
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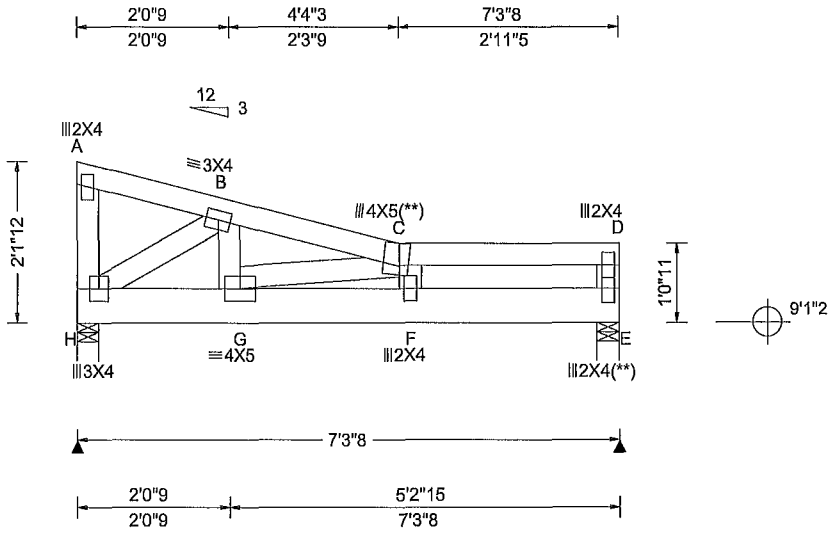
North Building, 4th Floor
Glenview, IL 60025

SEQN 800203 SPEC FROM CDM	Ply: 1 Qty: 1	**Job Number** 25-2238 138 MERCURY CT **Truss Label:** J07	Cust: R 215 JRef: 1Y7N2150006 T37 DrwNo 050 25 1408.52567 KD / DF 02/19/2025	



Loading Criteria (psf)

TCLL	20.00
TCDL	10.00
BCLL	0.00
BCDL	10.00
Des Ld	40.00
NCBCLL	10.00
Soffit:	2.00
Load Duration	1.25
Spacing	24.0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category II
EXP B Kzt NA
Mean Height 15.00 ft
TCDL 5.0 psf
BCDL 5.0 psf
MWFRS Parallel Dist 0 to h/2
C&C Dist a 3.00 ft
Loc from endwall not in 9.00 ft
GCpi 0.18
Wind Duration 1.60

Snow Criteria (Pg,Pf in PSF)
Pg NA Ct. NA CAT NA
Pf: NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac Varies by Ld Case
FT/RT 20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL)	0.094 F	931	240
VERT(CL)	0.179 F	488	180
HORZ(LL)	0.040 A	-	-
HORZ(TL)	0.076 A	-	-

Creep Factor 2.0
Max TC CSI 0.413
Max BC CSI 0.457
Max Web CSI 0.315

VIEW Ver: 23.02.04.0123.14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
H	737	/-	/-	/-	/40	/-
E	666	/-	/-	/-	/38	/-

Wind reactions based on MWFRS
H Brg Wid = 3.5 Min Req = 1.5 (Truss)
E Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings H & E are a rigid surface
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens	Comp
B - C	57	-1091

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp
H - G	927	-50

Maximum Web Forces Per Ply (lbs)

Webs	Tens	Comp	Webs	Tens	Comp
H - B	60	-1108	G - C	826	-33
B - G	667	-10	C - F	56	-555

Lumber
Top chord 2x4 SP #2,
Bot chord 2x6 SP 2400f-2.0E,
Webs 2x4 SP #3,

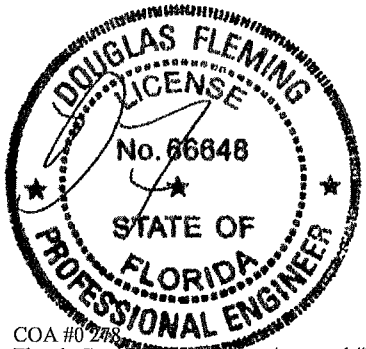
Special Loads
------(Lumber Dur Fac =1.25 / Plate Dur Fac =1.25)
TC From 61 plf at 0.00 to 61 plf at 7.29
BC From 10 plf at 0.00 to 10 plf at 7.29
BC 296 lb Conc Load at 1.35, 3.35, 5.35

Plating Notes
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads and reactions based on MWFRS
End verticals not exposed to wind pressure
Wind loading based on both gable and hip roof types

Additional Notes
The overall height of this truss excluding overhang is 2-1-12



COA #0 2[illegible]
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02/20/2025

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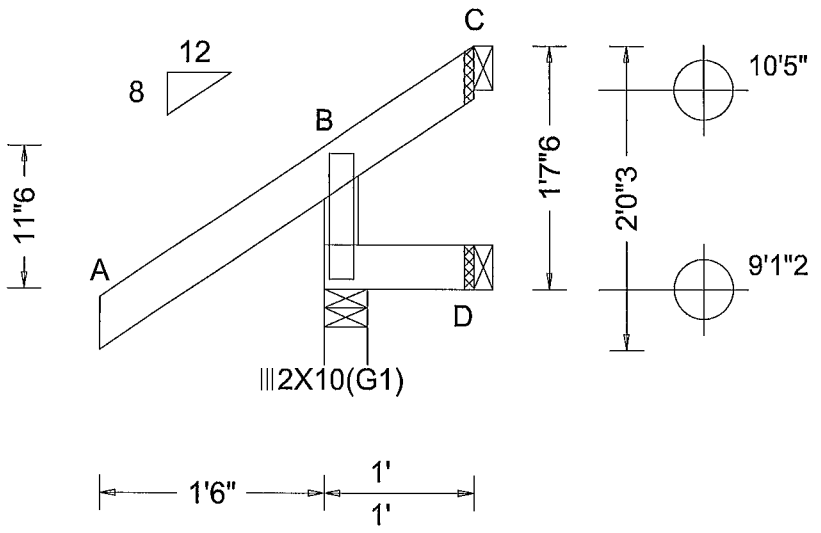
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800137 JACK FROM CDM	Ply 1 Qty 2	**Job Number:** 25-2238 138 MERCURY CT **Truss Label.** J08	Cust: R 215 JRef 1Y7N2150006 T35 DrwNo 050 25.1408.54273 KD / DF 02/19/2025	



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**	**▲Maximum Reactions (lbs)**
TCLL	20 00	Wind Std ASCE 7-22	Pg NA Ct. NA CAT NA	PP Deflection In loc L/defl L/#	Gravity / Non-Gravity
TCDL	10 00	Speed 130 mph	Pf NA Ce NA	VERT(LL) NA	Loc R+ / R- / Rh / Rw / U / RL
BCLL	0.00	Enclosure Closed	Lu NA Cs NA	VERT(CL) NA	B 223 /- /- /139 /7 /34
BCDL	10.00	Risk Category II	Snow Duration NA	HORZ(LL) -0.001 C - -	D 14 /-3 /- /9 /4 /-
Des Ld	40 00	EXP B Kzt NA		HORZ(TL) 0 003 C - -	C - /-33 /- /14 /30 /-
NCBCLL	10.00	Mean Height 15.00 ft	Building Code	Creep Factor 2.0	Wind reactions based on MWFRS
Soffit:	2 00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0 181	B Brg Wid = 3 5 Min Req = 1.5 (Truss)
Load Duration	1 25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0 034	D Brg Wid = 1 5 Min Req = -
Spacing	24 0 "	MWFRS Parallel Dist 0 to h/2	Rep Fac Yes	Max Web CSI 0.000	C Brg Wid = 1 5 Min Req = -
		C&C Dist a 3 00 ft	FT/RT 20(0)/10(0)		Bearing B is a rigid surface
		Loc from endwall Any	Plate Type(s)		Members not listed have forces less than 375#
		GCpi 0 18	WAVE		
		Wind Duration 1.60		VIEW Ver 23.02.04 0123 14	

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Lt Stub Wedge 2x6 SP #2,

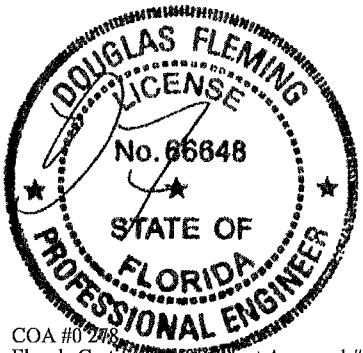
Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 1-7-6



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02/20/2025

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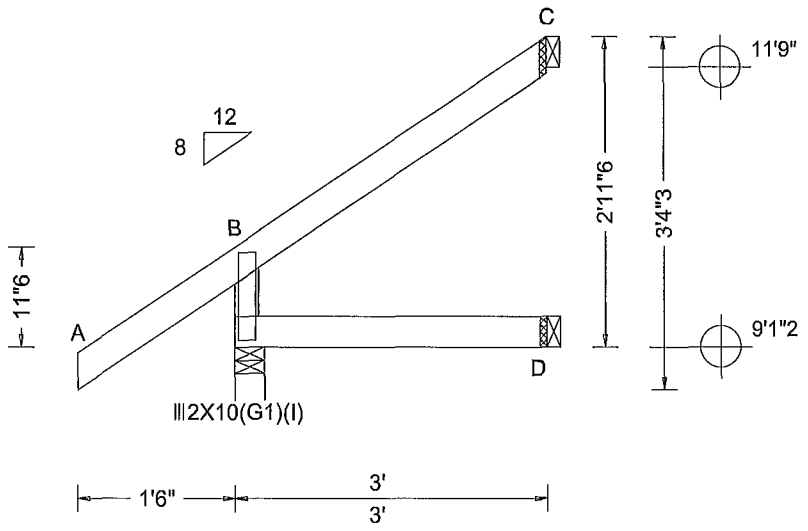
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800139 JACK FROM CDM	Ply 1 Qty 2	**Job Number** 25-2238 138 MERCURY CT **Truss Label** J09		Cust R 215 JRef 1Y7N2150006 T34 DrwNo 050 25 1408.55993 KD / DF 02/19/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg Pf in PSF)	**Defl/CSI Criteria**
TCLL 20.00 TCDL. 10.00 BCLL 0 00 BCDL 10 00 Des Ld 40 00 NCBCLL 10.00 Soffit: 2 00 Load Duration 1.25 Spacing 24 0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure Closed Risk Category II EXP B Kzt: NA Mean Height: 15.00 ft TCDL 5 0 psf BCDL. 5 0 psf MWFRS Parallel Dist 0 to h/2 C&C Dist a 3 00 ft Loc from endwall not in 4.50 ft GCpi 0 18 Wind Duration 1 60	Pg NA Ct NA CAT NA Pf NA Ce NA Lu NA Cs NA Snow Duration NA Building Code FBC 8th Ed 2023 Res TPI Std 2014 Rep Fac. Yes FT/RT 20(0)/10(0) Plate Type(s) WAVE	PP Deflection in loc L/defl L/# VERT(LL) NA VERT(CL) NA HORZ(LL) -0.003 C - - HORZ(TL) 0 004 C - - Creep Factor 2 0 Max TC CSI 0 167 Max BC CSI 0 085 Max Web CSI 0 000 VIEW Ver 23 02.04 0123 14

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	255	/-	/-	/156	/-	/66
D	56	/-	/-	/31	/-	/-
C	73	/-	/-	/46	/34	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 1 5 (Truss)
D Brg Wid = 1 5 Min Req = -
C Brg Wid = 1 5 Min Req = -
Bearing B is a rigid surface
Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Lt Stub Wedge 2x6 SP #2,

Plating Notes

(I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance and/or zero Positioning Tolerance

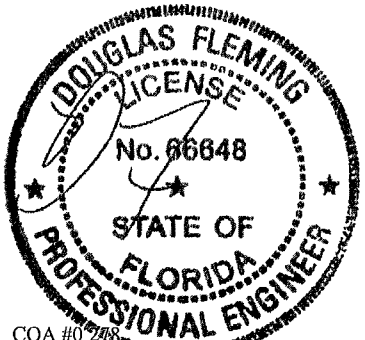
Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 2-11-6



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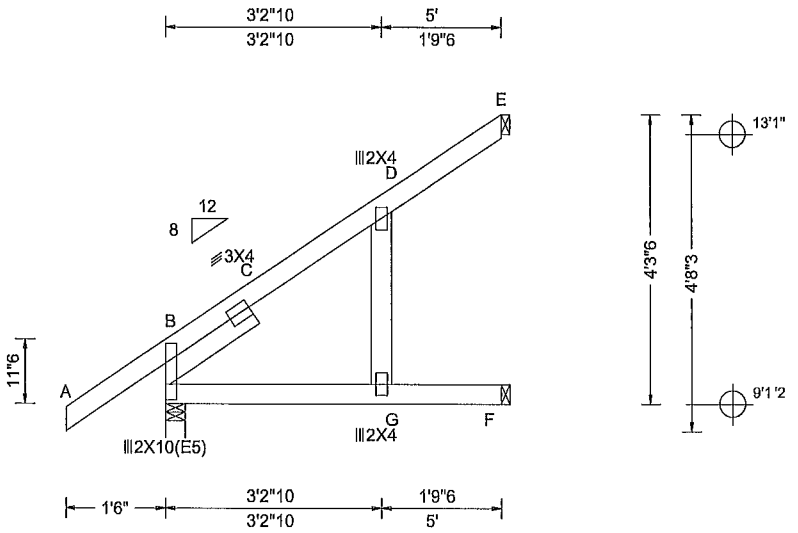
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800135 EJAC FROM CDM	Ply 1 Qty 19	**Job Number:** 25-2238 138 MERCURY CT **Truss Label:** J10		Cust: R 215 JRef 1Y7N2150006 T36 DrwNo 050 25.1408.57687 KD / DF 02/19/2025



Loading Criteria (psf)

TCLL.	20 00
TCDL.	10 00
BCLL	0.00
BCDL	10.00
Des Ld	40 00
NCBCLL	10 00
Soffit.	2 00
Load Duration	1 25
Spacing	24 0 "

Wind Criteria
Wind Std ASCE 7-22
Speed 130 mph
Enclosure Closed
Risk Category II
EXP B Kzt. NA
Mean Height: 15 00 ft
TCDL 5 0 psf
BCDL 5 0 psf
MWFRS Parallel Dist h/2 to h
C&C Dist a 3 00 ft
Loc. from endwall not in 9 00 ft
GCpi 0 18
Wind Duration 1 60

Snow Criteria (Pg Pf In PSF)
Pg NA Ct NA CAT NA
Pf NA Ce NA
Lu NA Cs NA
Snow Duration NA

Building Code
FBC 8th Ed 2023 Res
TPI Std 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s)
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL)	0 030 D	999	240
VERT(CL)	0 060 D	995	180
HORZ(LL)	0 028 D	-	-
HORZ(TL)	0 056 D	-	-

Creep Factor 2.0
Max TC CSI 0.263
Max BC CSI 0.238
Max Web CSI 0 098

VIEW Ver 23 02.04.0123 14

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	329	/-	/-	/201	/-	/98
F	92	/-	/-	/70	/17	/-
E	103	/-	/-	/71	/21	/-

Wind reactions based on MWFRS
B Brg Wid = 3 5 Min Req = 1.5 (Truss)
F Brg Wid = 1 5 Min Req = -
E Brg Wid = 1 5 Min Req = -
Bearing B is a rigid surface
Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,
Lt Slider 2x4 SP #3, block length = 1 671'

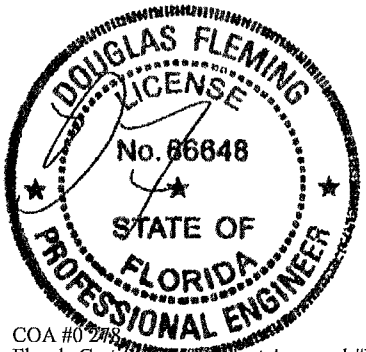
Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 4-3-6



COA #0 2778
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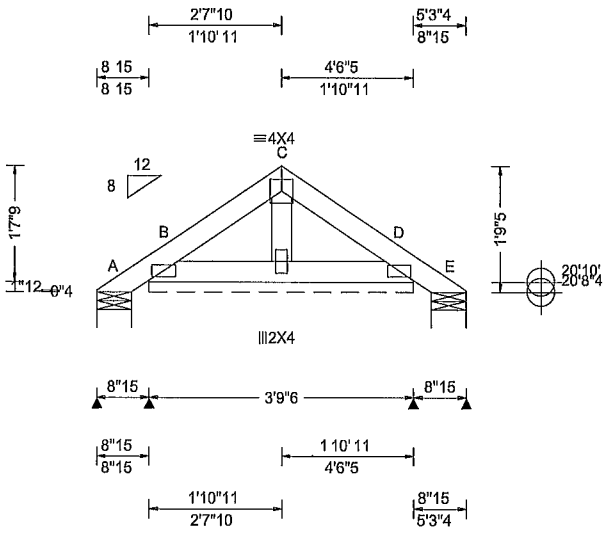
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800277 GABL FROM CDM	Ply· 1 Qty· 2	**Job Number:** 25-2238 138 MERCURY CT **Truss Label** PB01		Cust: R 215 JRef·1Y7N2150006 T9 DrwNo 050 25 1409.00597 KD / DF 02/19/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL 20.00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#
TCDL 10.00	Speed 130 mph	Pf· NA Ce NA	VERT(LL) 0 000 D 999 240
BCLL 0.00	Enclosure Closed	Lu NA Cs NA	VERT(CL) 0 000 B 999 180
BCDL 10 00	Risk Category· II	Snow Duration NA	HORZ(LL) -0.000 D - -
Des Ld 40 00	EXP B Kzt NA		HORZ(TL) 0 000 B - -
NCBCLL 10 00	Mean Height 15 75 ft	Building Code	Creep Factor· 2 0
Soffit: 2 00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0 033
Load Duration 1.25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0 024
Spacing 24 0 "	MWFRS Parallel Dist 0 to h/2	Rep Fac. Yes	Max Web CSI 0 010
	C&C Dist a 3 00 ft	FT/RT·20(0)/10(0)	
	Loc. from endwall not in 6.50 ft	Plate Type(s)	
	GCpi 0 18	WAVE	VIEW Ver· 23.02.04 0123 14
	Wind Duration 1 60		

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	11	/-	/-	/26	/15	/32
B*	101	/-	/-	/59	/4	/-
E	11	/-	/-	/7	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 5 9 Min Req = 1.5 (Truss)
B Brg Wid = 45.4 Min Req = -
E Brg Wid = 5 9 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface
Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,

Plating Notes

All plates are 2X4(A1) except as noted

Loading

Gable end supports 8" max rake overhang Top chord must not be cut or notched

Wind

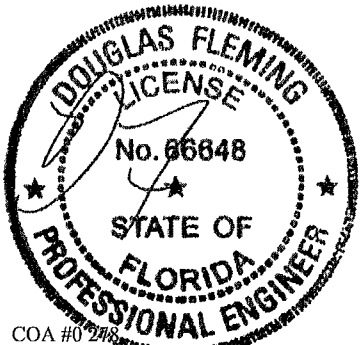
Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

Refer to DWG PB160220723 for piggyback details

The overall height of this truss excluding overhang is 1-9-5



COA #0 218
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02/20/2025

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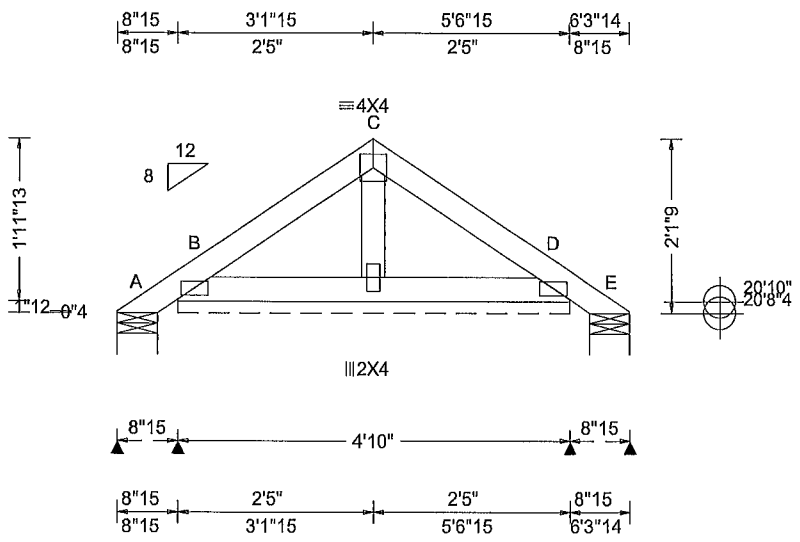
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 800279 COMN FROM CDM	Ply 1 Qty 10	**Job Number:** 25-2238 138 MERCURY CT **Truss Label.** PB02	Cust: R 215 JRef 1Y7N2150006 T33 DrwNo 050.25 1409.02097 KD / DF 02/19/2025	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg Pf in PSF)	Defl/CSI Criteria
TCLL 20 00 TCDL 10 00 BCLL 0.00 BCDL. 10 00 Des Ld 40.00 NCBCLL 10.00 Soffit: 2 00 Load Duration 1 25 Spacing 24 0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure Closed Risk Category II EXP B Kzt NA Mean Height 15 93 ft TCDL 5 0 psf BCDL 5 0 psf MWFRS Parallel Dist h to 2h C&C Dist a 3 00 ft Loc. from endwall not in 13 00 ft GCpi 0 18 Wind Duration 1 60	Pg NA Ct NA CAT NA Pf NA Ce NA Lu NA Cs NA Snow Duration NA Building Code FBC 8th Ed 2023 Res TPI Std 2014 Rep Fac. Yes FT/RT 20(0)/10(0) Plate Type(s) WAVE	PP Deflection In loc L/defl L/# VERT(LL) 0.000 B 999 240 VERT(CL) 0 001 B 999 180 HORZ(LL) 0 000 B - - HORZ(TL) 0 001 B - - Creep Factor 2.0 Max TC CSI 0 058 Max BC CSI 0 043 Max Web CSI 0 012 VIEW Ver 23 02 04 0123 14

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	1	/-3	/-	/29	/26	/40
B*	105	/-	/-	/61	/-	/-
E	1	/-3	/-	/6	/4	/-

Wind reactions based on MWFRS
A Brg Wid = 5 9 Min Req = 1.5 (Truss)
B Brg Wid = 58 0 Min Req = -
E Brg Wid = 5 9 Min Req = 1 5 (Truss)
Bearings A, B, & E are a rigid surface
Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2
Webs 2x4 SP #3,

Plating Notes

All plates are 2X4(A1) except as noted

Loading

Gable end supports 8" max rake overhang Top chord must not be cut or notched

Wind

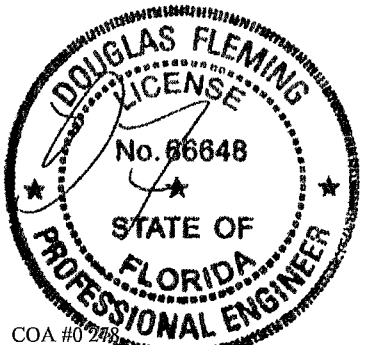
Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

Refer to DWG PB160220723 for piggyback details

The overall height of this truss excluding overhang is 2-1-9



COA #0 [illegible]
Florida Certificate of Product Approval #FL1999
02/20/2025

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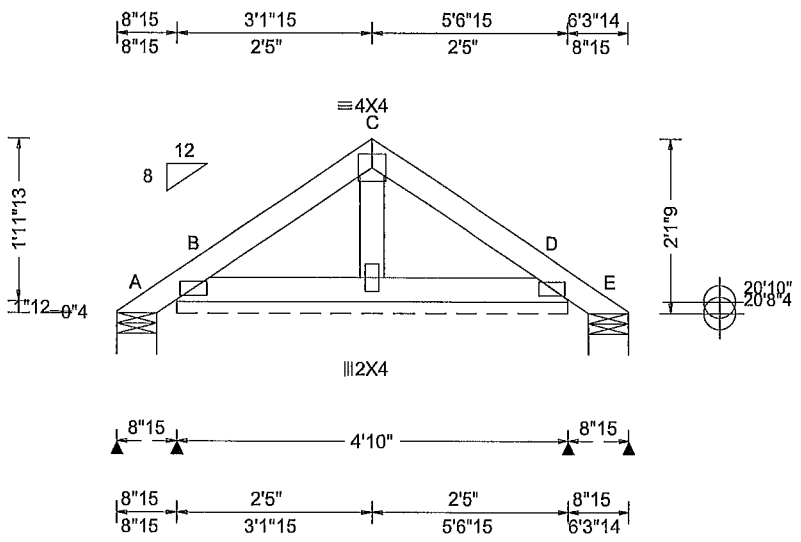
SEQN 800281 SPEC FROM CDM	Ply 1 Qty 8	**Job Number** 25-2238 138 MERCURY CT **Truss Label.** PB03	Cust R 215 JRef 1Y7N2150006 T41 DrwNo 050 25 1409.05447 KD / DF 02/19/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg Pf In PSF)	**Defl/CSI Criteria**
TCLL 20 00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#
TCDL 10 00	Speed 130 mph	Pf NA Ce NA	VERT(LL) 0 000 B 999 240
BCLL 0.00	Enclosure Closed	Lu NA Cs NA	VERT(CL) 0 001 B 999 180
BCDL 10 00	Risk Category II	Snow Duration NA	HORZ(LL) 0 000 B - -
Des Ld 40 00	EXP B Kzt NA		HORZ(TL) 0 001 B - -
NCBCLL 10 00	Mean Height 15 93 ft	Building Code	Creep Factor 2.0
Soffit 2 00	TCDL 5 0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0 058
Load Duration 1.25	BCDL 5 0 psf	TPI Std 2014	Max BC CSI 0 042
Spacing 24 0 "	MWFRS Parallel Dist h/2 to h	Rep Fac. Yes	Max Web CSI 0 012
	C&C Dist a 3 00 ft	FT/RT 20(0)/10(0)	
	Loc from endwall not in 6 50 ft	Plate Type(s)	
	GCpi 0 18	WAVE	VIEW Ver 23.02.04 0123 14
	Wind Duration 1 60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	1	/-4	/-	/28	/26	/39
B*	103	/-	/-	/60	/5	/-
E	1	/-4	/-	/6	/4	/-

Wind reactions based on MWFRS
A Brg Wid = 5 9 Min Req = 1.5 (Truss)
B Brg Wid = 58 0 Min Req = -
E Brg Wid = 5 9 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface
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Lumber

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Bot chord 2x4 SP #2,
Webs 2x4 SP #3,

Plating Notes

All plates are 2X4(A1) except as noted

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Wind

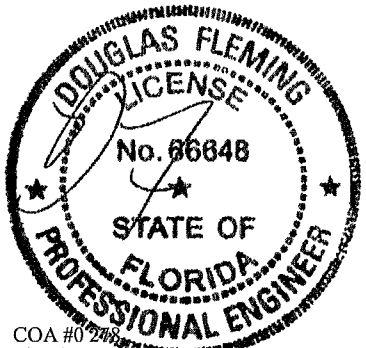
Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types

Additional Notes

Refer to DWG PB160220723 for piggyback details

The overall height of this truss excluding overhang is 2-1-9



COA #0 2478
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CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

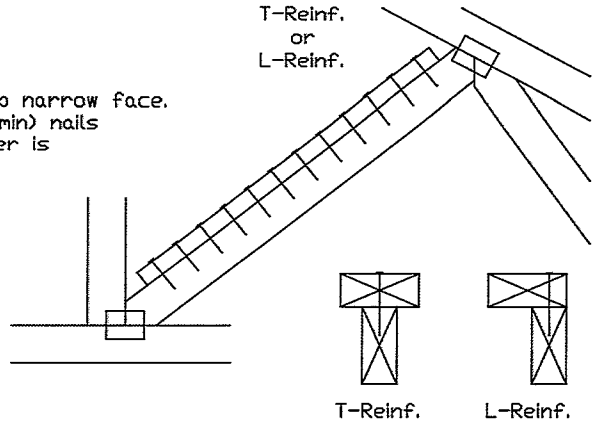
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4Ⓒ
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6Ⓒ

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

Ⓒ Center scab on wide face of web. Apply (1) scab to each face of web.

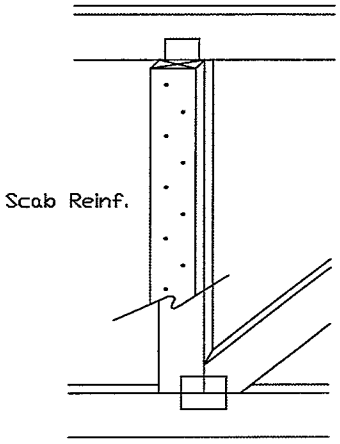
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

02/02/2025

COA #0 278

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TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH,GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

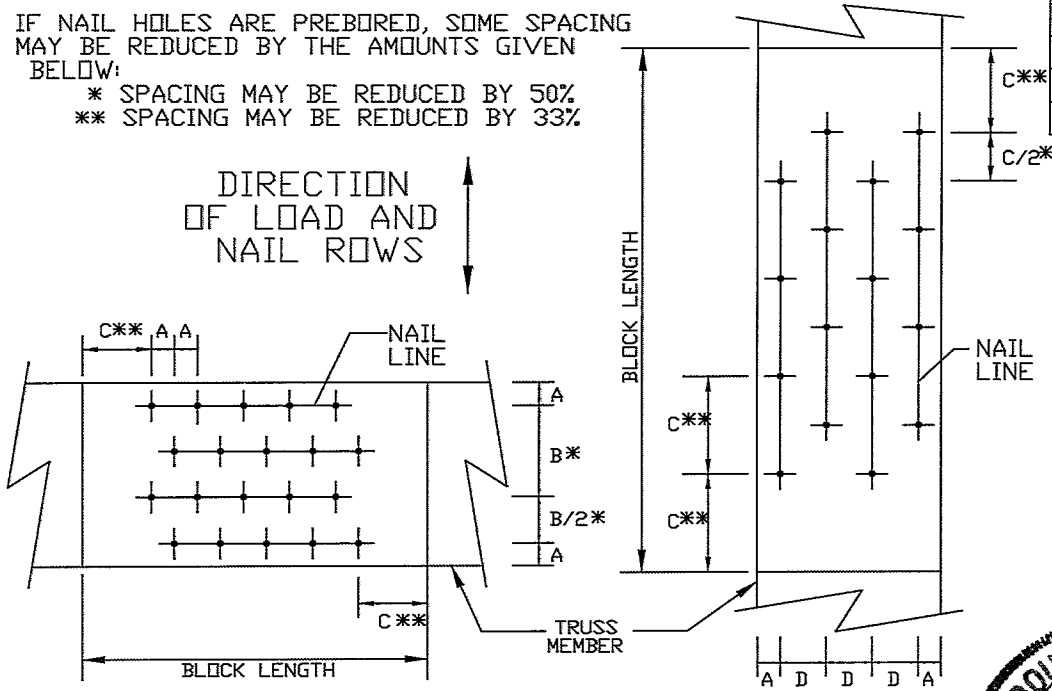
LOAD PERPENDICULAR TO GRAIN
A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN
A - EDGE DISTANCE (6 NAIL DIAMETERS)
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
* SPACING MAY BE REDUCED BY 50%
** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3.",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1'	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN LOAD APPLIED PARALLEL TO GRAIN

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

02/20/2025

COA #0 278

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REF	NAIL SPACE
DATE	10/01/14
DRWG	CNNAILSP1014

Piggyback Detail - ASCE 7-22: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

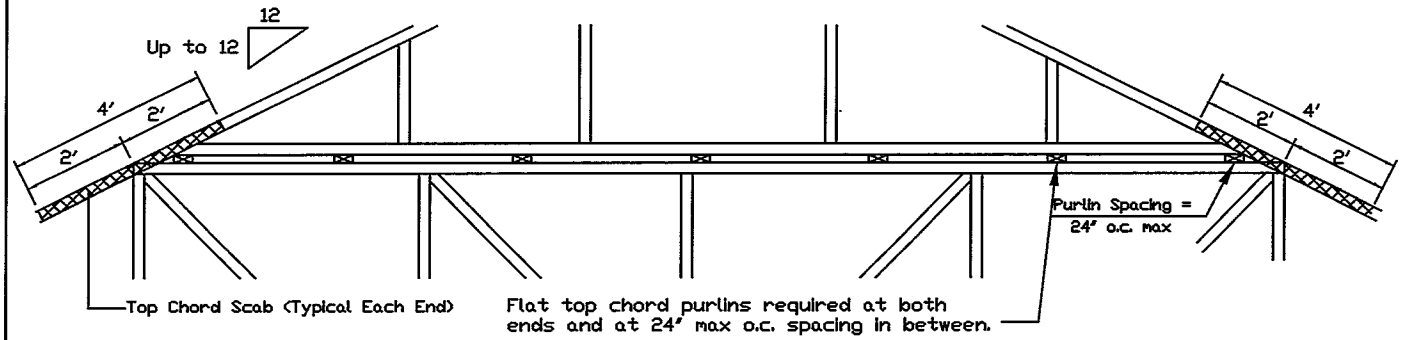
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-22, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-22, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building designer shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. Detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

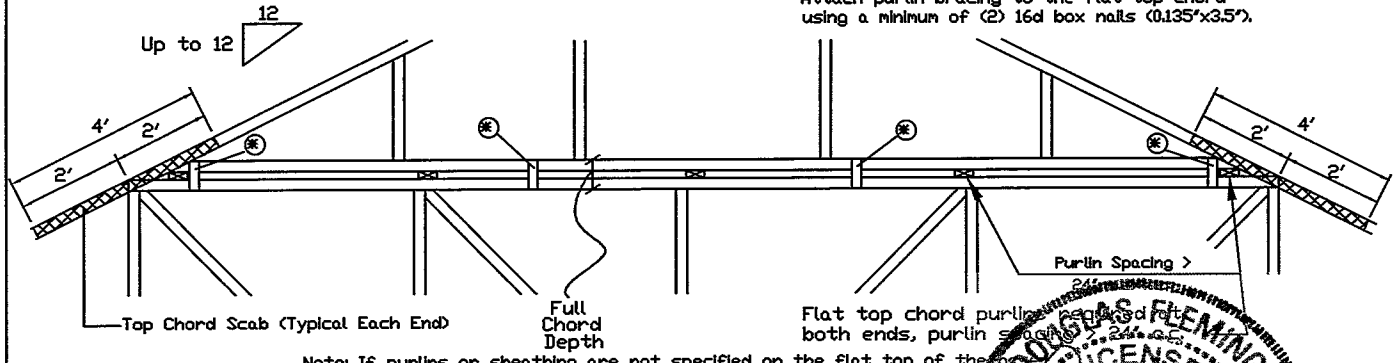
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the truss, purlins must be installed at 24" o.c. max. and use Detail A

* In addition, provide connection with one of the following methods:

Trulox
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

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07/20/2025

COA #0 278

SPACING 24.0"

REF	PIGGYBACK
DATE	07/03/2023
DRWG	PB160220723