

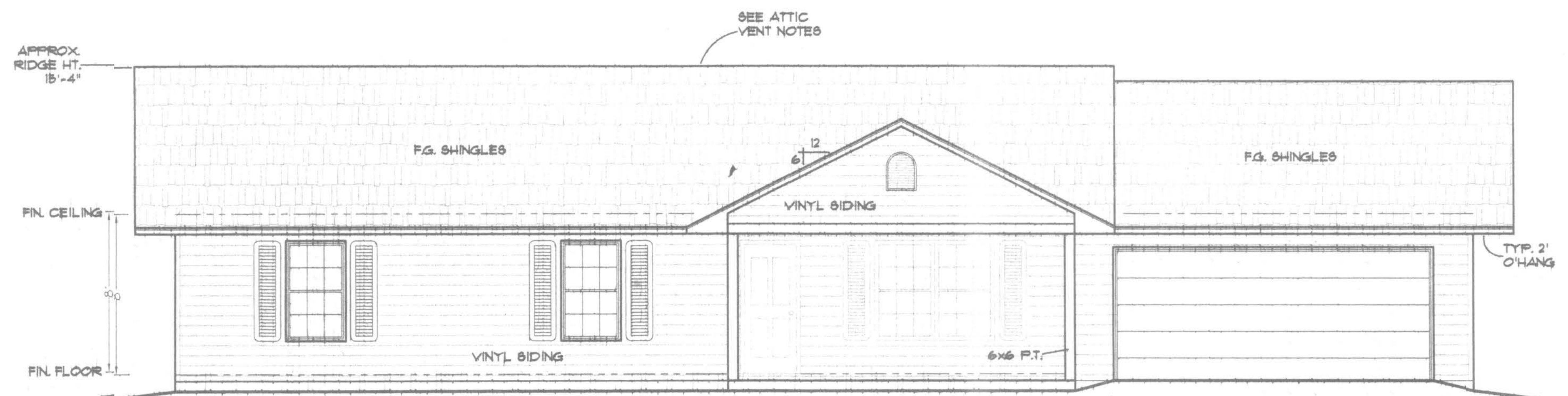
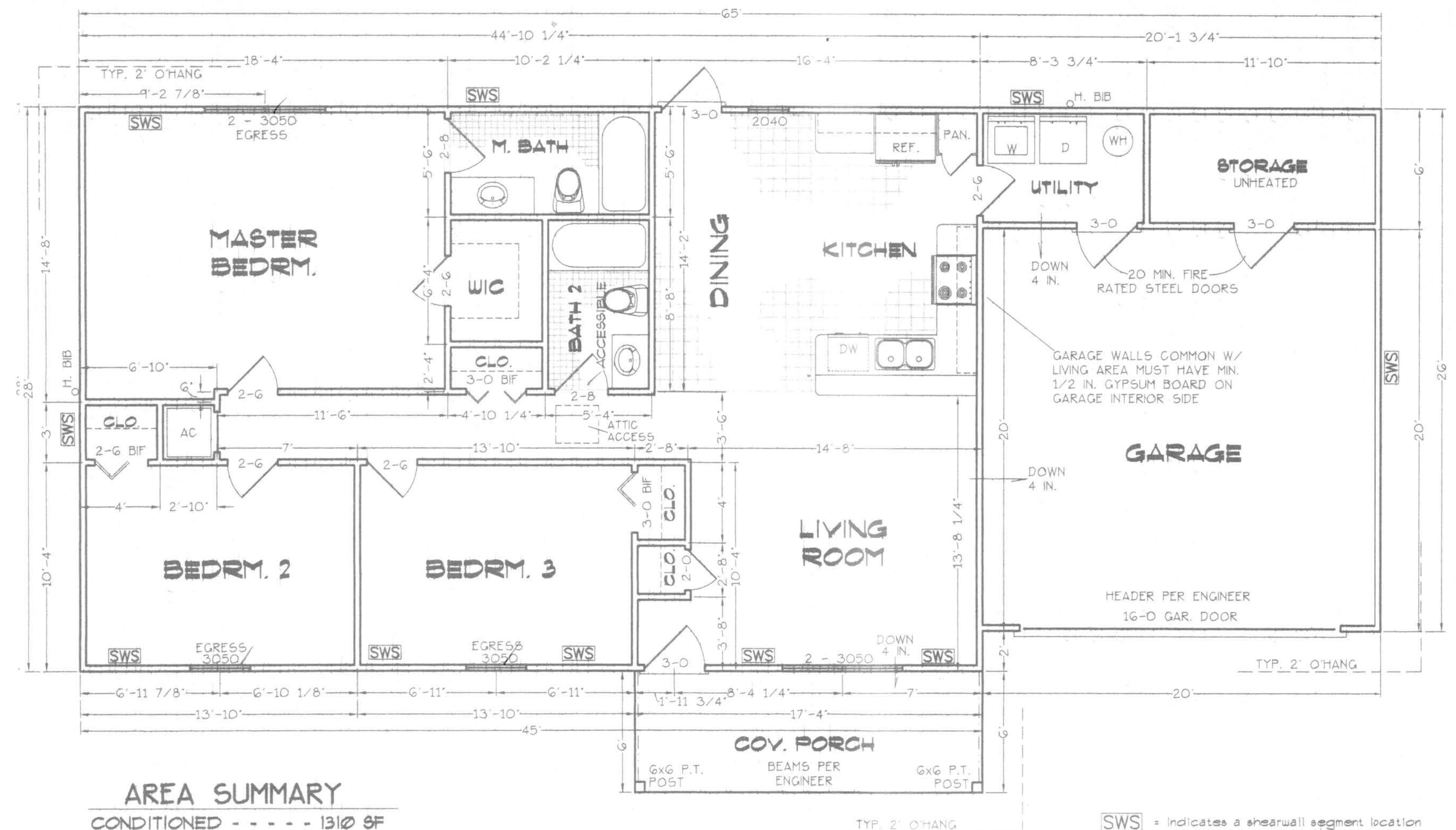
Index to Sheets

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SHEET A-2	ELEVATIONS + GENERAL NOTES
SHEET A-3	FOUNDATION + SECTIONS
SHEET A-4	ELECTRICAL
SHEET S-1	WIND LOAD DETAILS

ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with a 1/8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.



WINDLOAD ENGINEER: Mark Diboway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

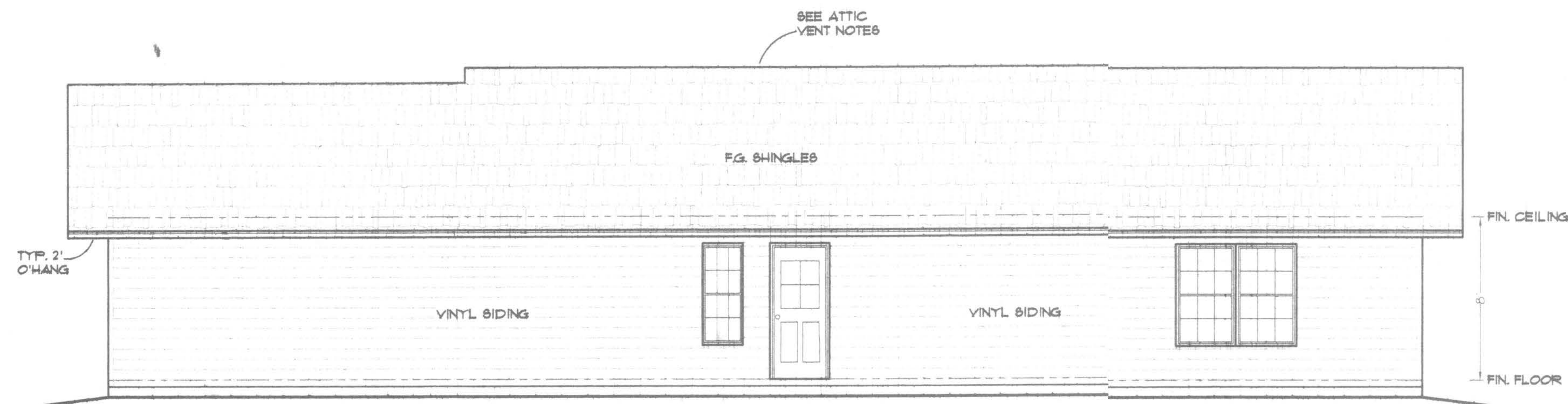
LOTS 1 or 2, 341 ESTATES

Location: Columbia County, FL

Job No.:

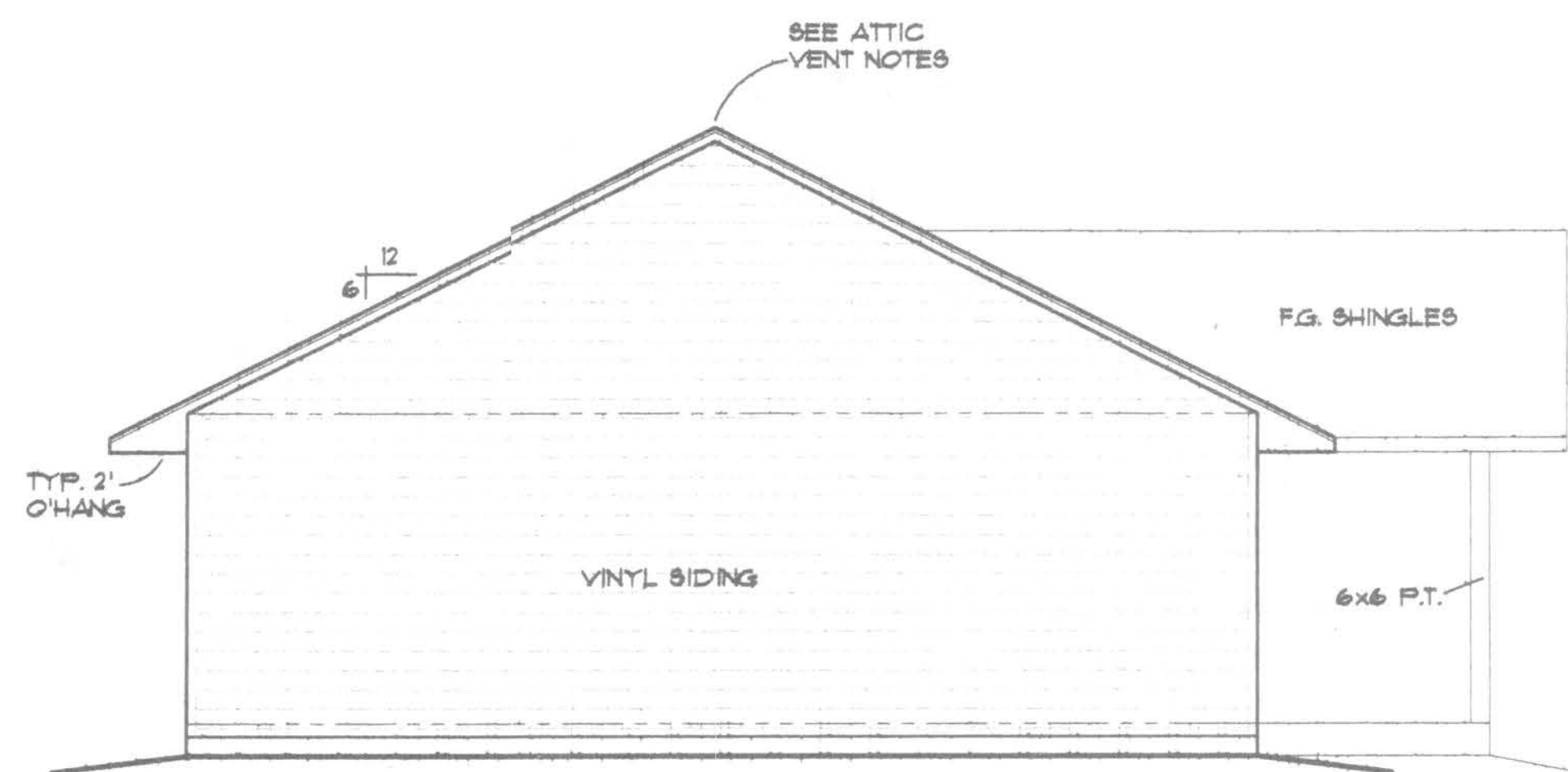
A-1

FILE: OG-021	RESIDENCE By Blake Const.	SHEET 1 OF 4
DATE: 4-4-06	PREPARED BY: TIM DELBENE Drafting & Technical Services	CAD FILE: OG021
DRAWN: T A D	142 SW Sagewood Gln. Lake City, FL 32024	REV:
CHECK: T A D	Phone (386) 755-5891	REV:



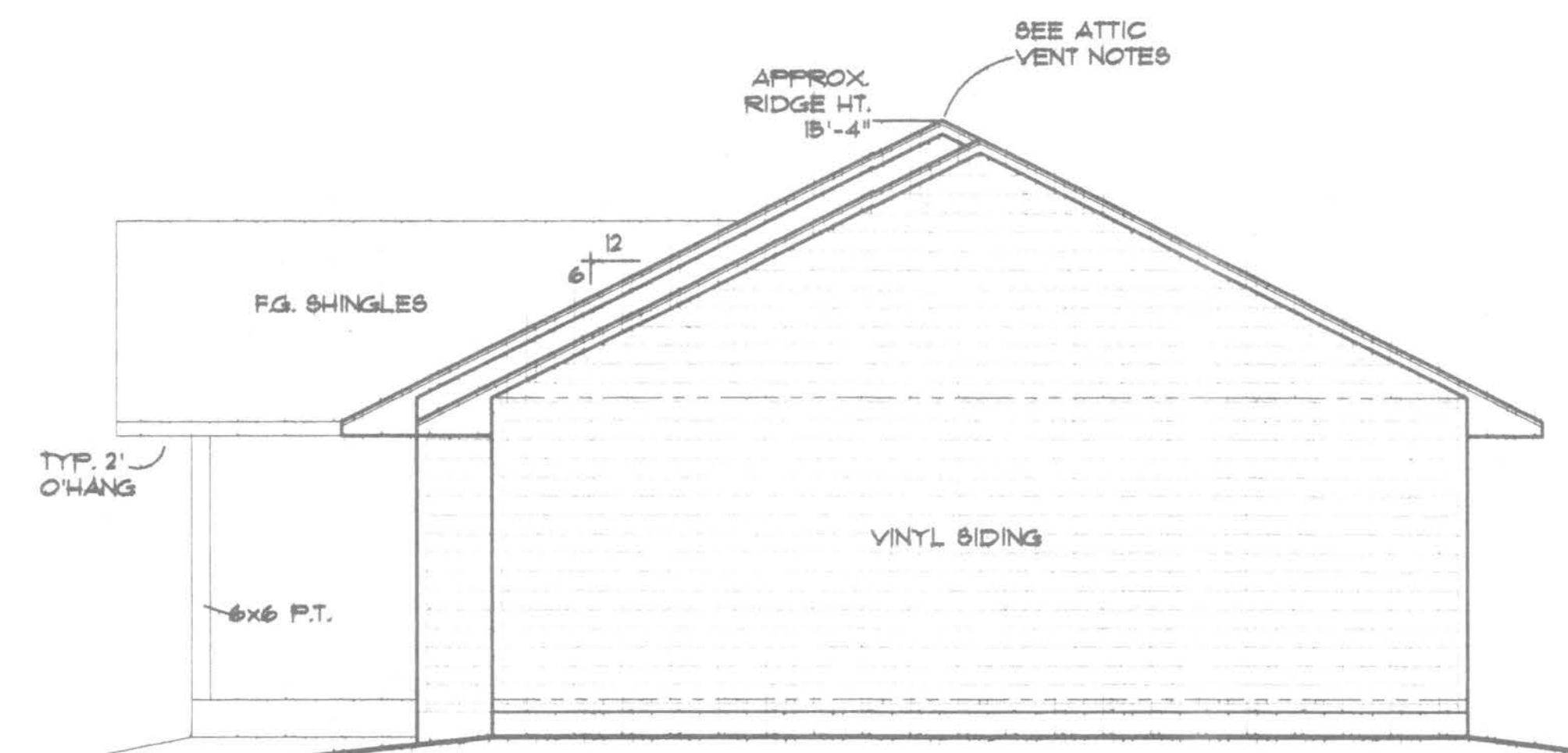
REAR ELEVATION

SCALE: 1/4 IN. = 1 FT.



LEFT ELEVATION

SCALE: 1/4 IN. = 1 FT.



RIGHT ELEVATION

SCALE: 1/4 IN. = 1 FT.

ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with a 1/8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

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GENERAL NOTES

- 1.) See "Wind Load Detail Sheet S-1" and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 4.) Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- 5.) Roof Truss design is the responsibility of the supplier.
- 6.) The Truss Manufacturer shall prepare Shop Drawings indicating Truss placement, Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- 7.) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- 8.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

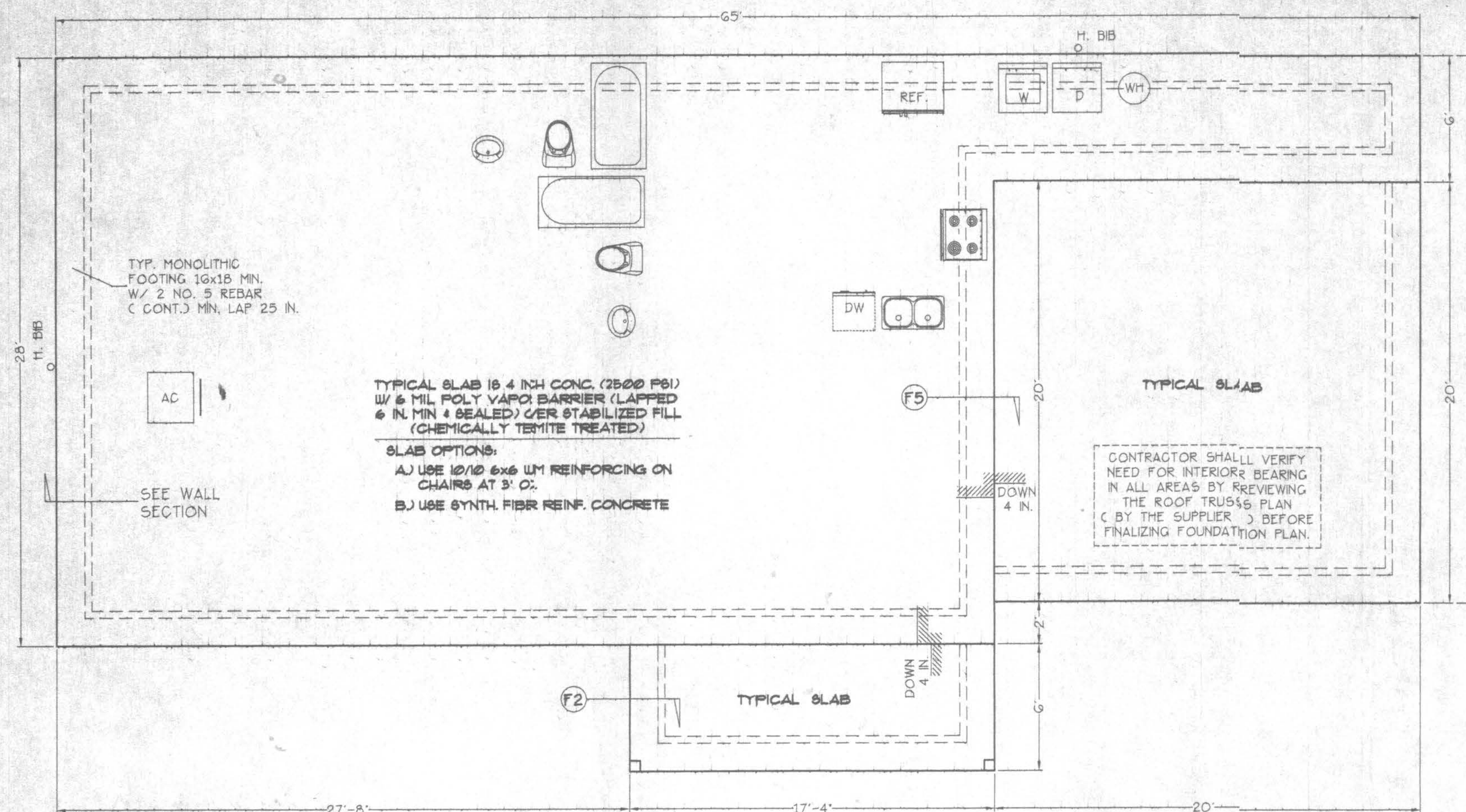
LOTS 1 or 2, 341 ESTATES

Location: Columbia County, FL

Job No.:

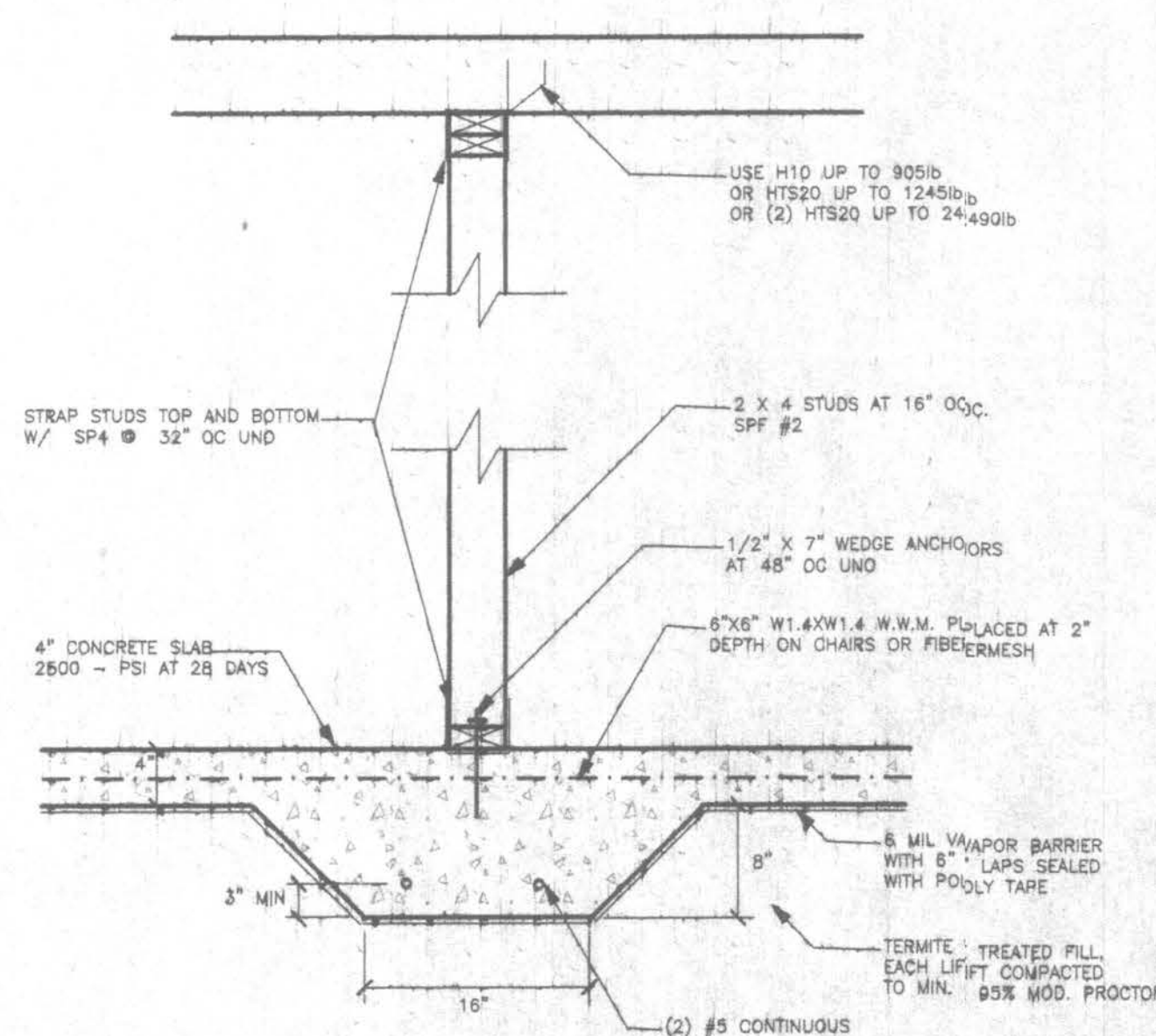
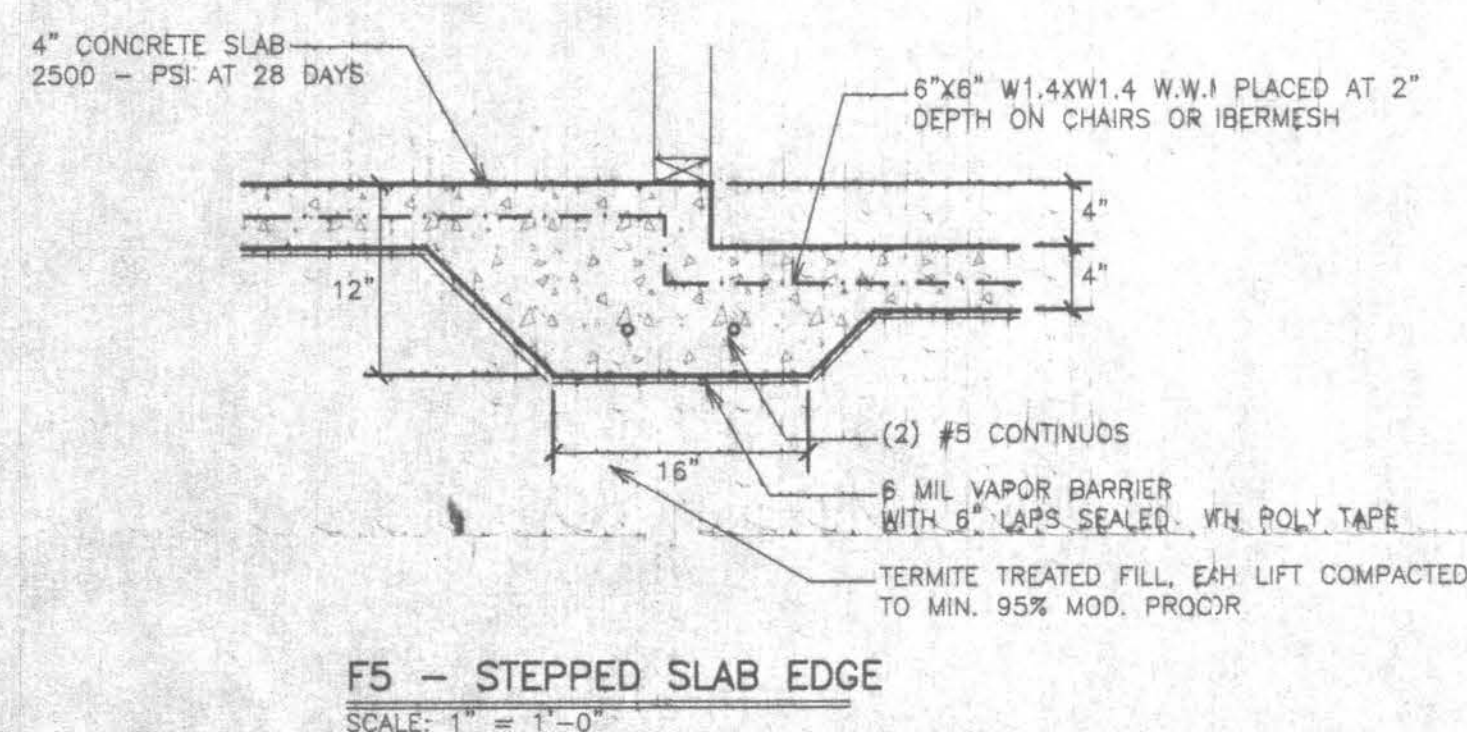
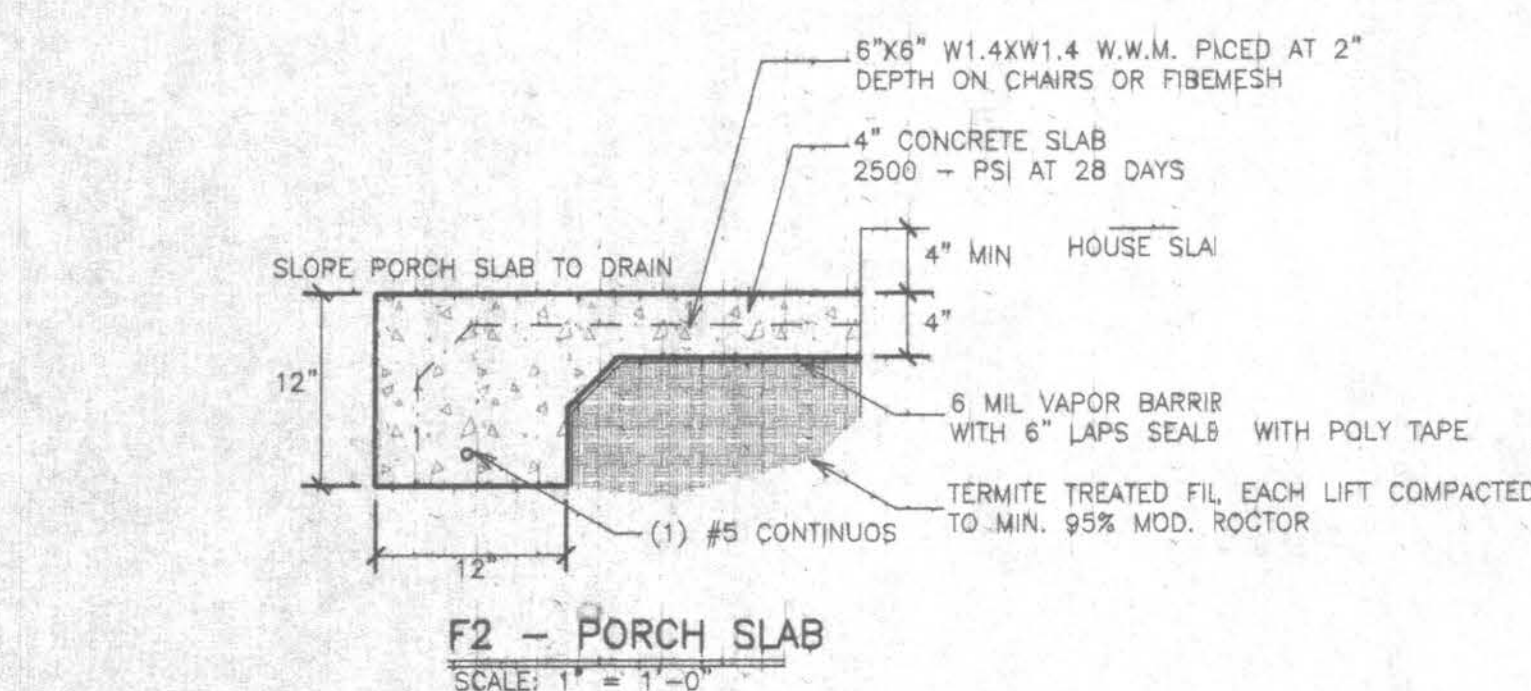
A-2

FILE: OG-021	RESIDENCE By Blake Const.	SHEET: 2 OF 4
DATE: 4-4-06		CAD FILE: OG021
DRAWN: T A D		REV:
CHECK: T A D		REV:
PREPARED BY: TIM DELBENE Drafting + Technical Services 192 SW Sagewood Gln. Lake City, FL 32024 Phone (386) 755-5891		



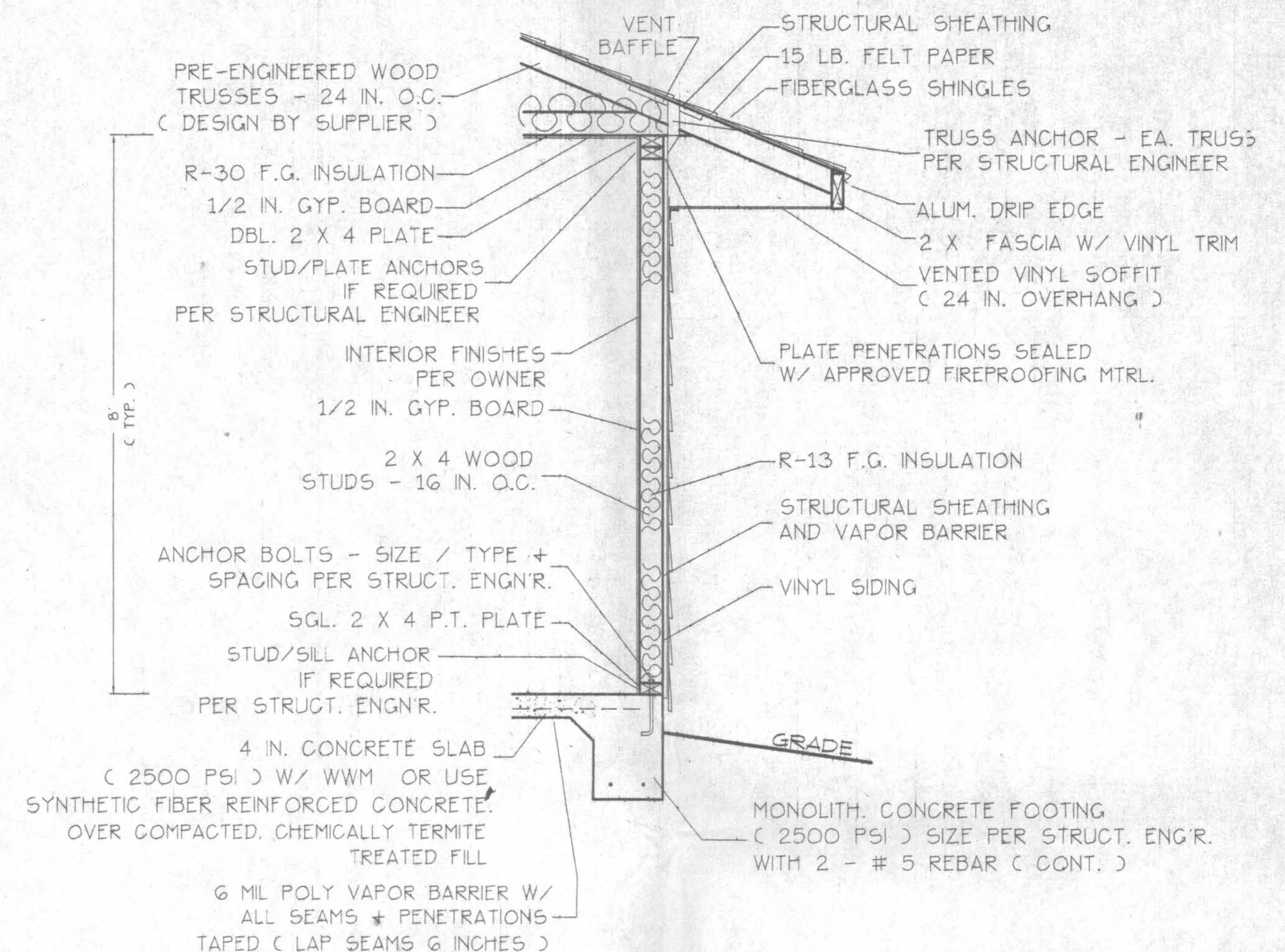
FOUNDATION PLAN

SCALE: 1/4 IN. = 1 FT.



F4 - INTERIOR BEARING FOOTING

VERIFY NEED FOR INTERIOR BEARING W/ TRUSS PLAN (BY SUPPLIER)



WALL SECTION NOTES:

- This Typical Wall Section is for Estimating purposes only.
- All data shown in this Wall Section shall be subject to review and final input by the Structural Engineer.
- See Sheet S-1 for Engineer's structural Wall Section.

DESIGN WALL SECTION

NON-STRUCTURAL DATA

SCALE: 3/4 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32058, 388-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

LOTS 1 or 2, 341 ESTATES

Location: Columbia County, FL

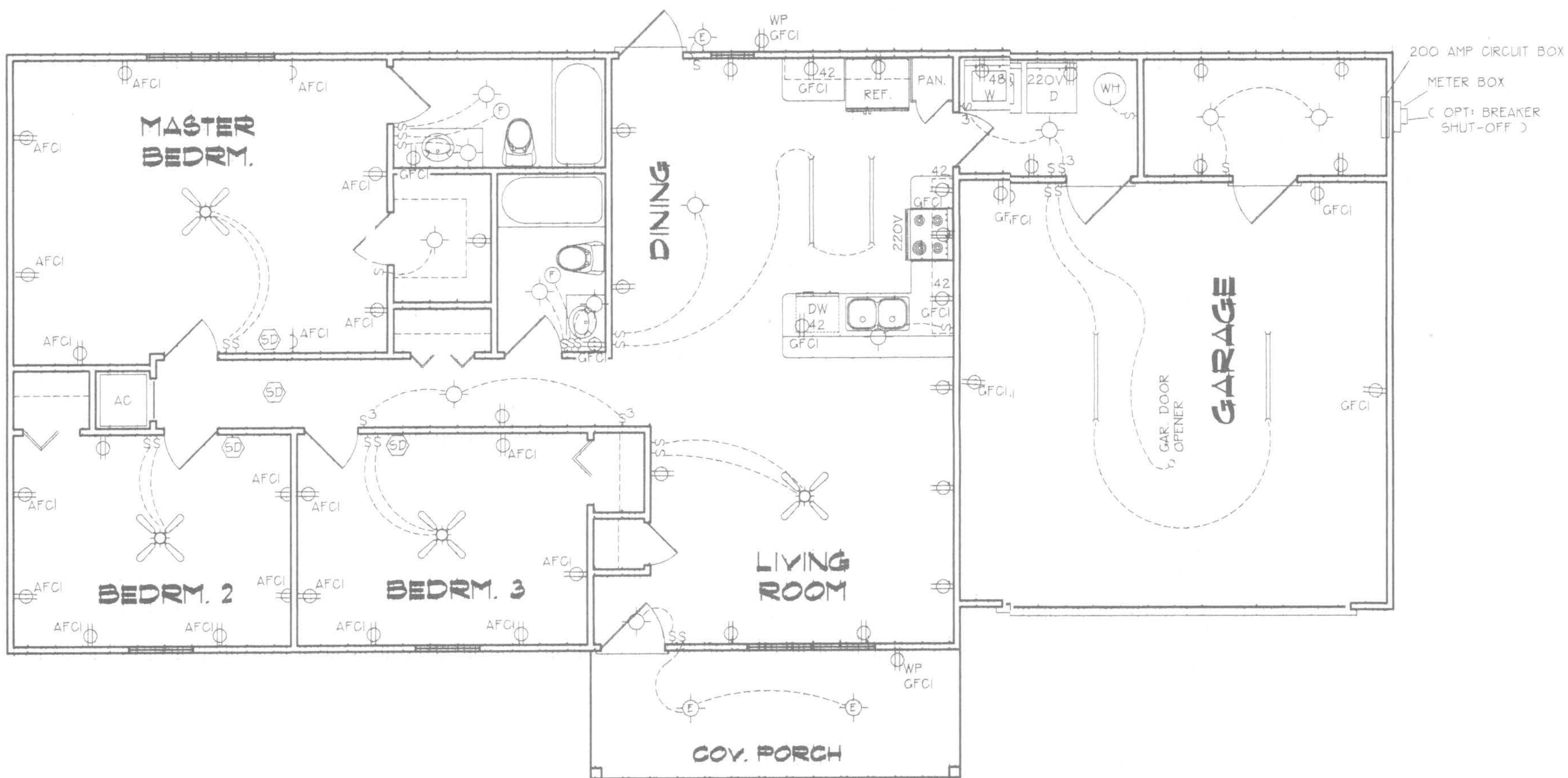
Job No.:

A-3

FILE: OG-021	RESIDENCE By Blake Const.	SHEET 3 OF 4
DATE: 4-4-06		CAD FILE: OG021
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	192 SW Seagwood Cir. Lake City, FL 32024 Phone: (386) 755-5891	REV:

ELECTRICAL PLAN NOTES

- WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, + IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- ELECTRICAL CONTR. SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.



ELECTRICAL SYMBOL LEGEND

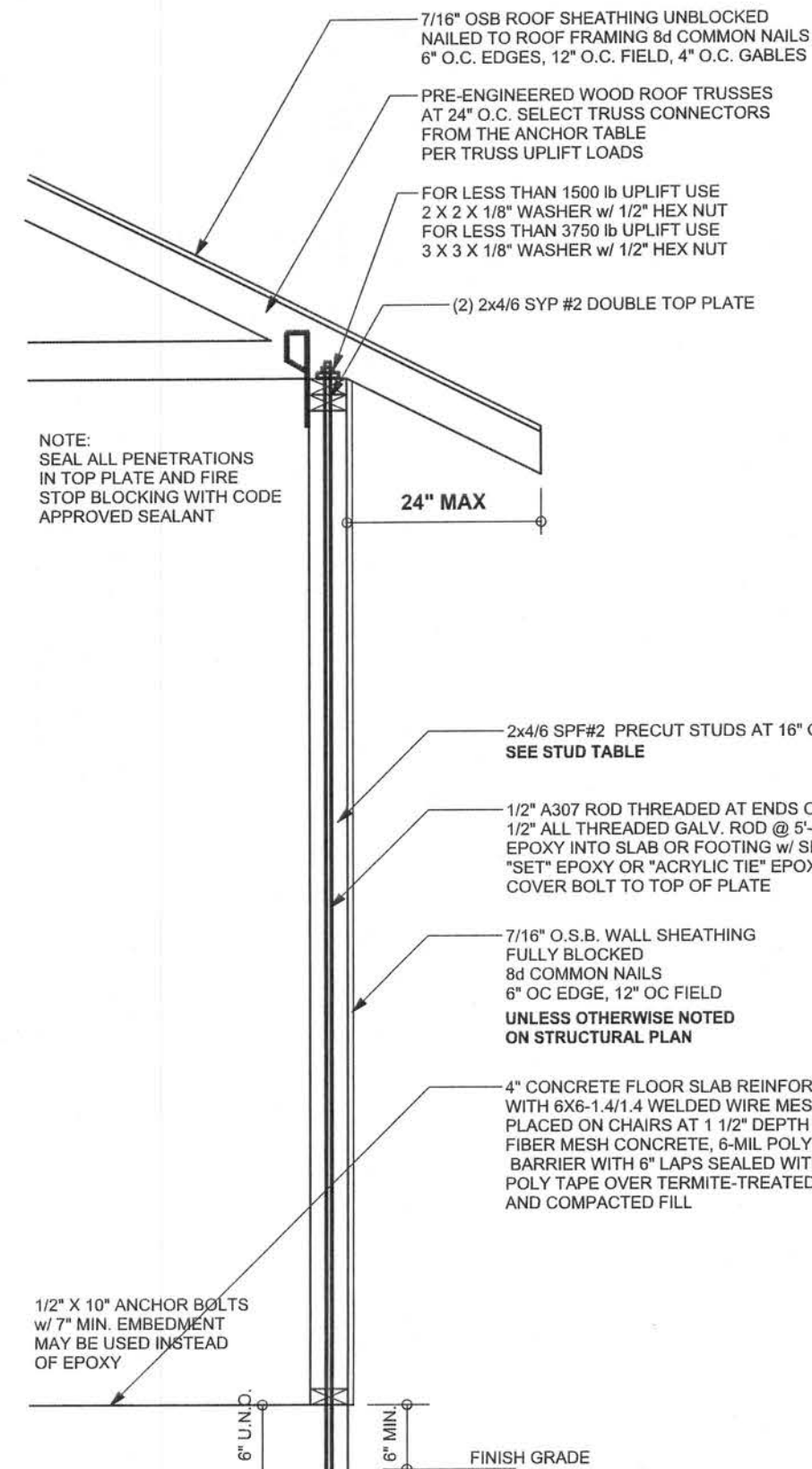
- = FLOURESCENT LIGHTING FIXTURE.
- = CEILING LIGHT FIXTURE
- = EXTERIOR LIGHTING FIXTURE
- = LIGHT SWITCH.
- = THREE-WAY SWITCH.
- = 110 V. DUPLEX OUTLET.
- = SPECIAL HEIGHT 110 V. DUPLEX OUTLET
- = GROUNDED OUTLET
- = 220 VOLT OUTLET (4 WIRE)
- = FAN LOCATION (CEILING)
- = FAN LOCATION (EXHAUST)
- = SMOKE DETECTOR

CONSULT OWNER PER LOCATION + SWITCHING OF ANY ADDITIONAL EXTERIOR SECURITY AND ACCENT LIGHTING.

ELECTRICAL PLAN
NOT TO SCALE

A-4

FILE: OG-021	RESIDENCE By Blake Const.	SHEET: 4 OF 4
DATE: 4-4-06		CAD FILE: OG021
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	142 SW Sagewood Cir. Lake City, FL 32024 Phone (386) 755-5891	REV:



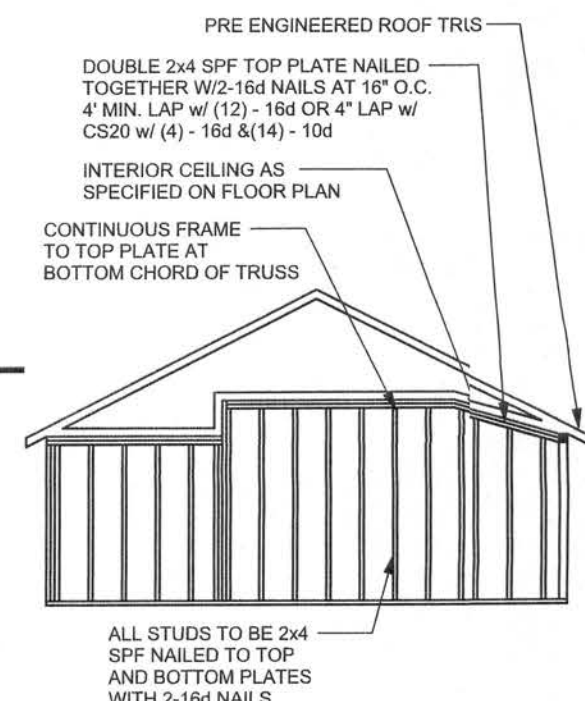
ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

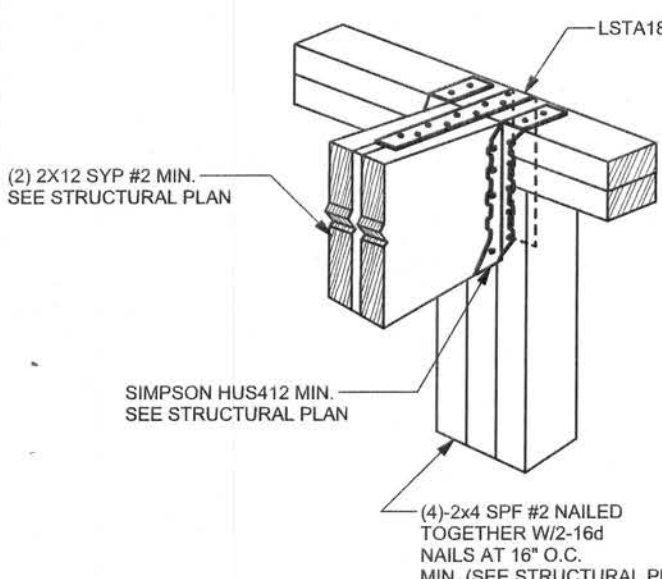
(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B,
EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS
RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B
STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING
LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.
EXAMPLE: 16" O.C. X 0.85 = 13" O.C.



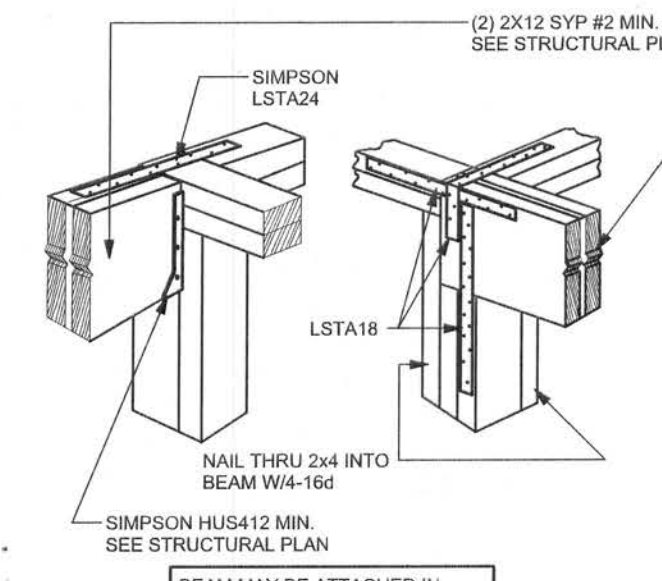
CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.



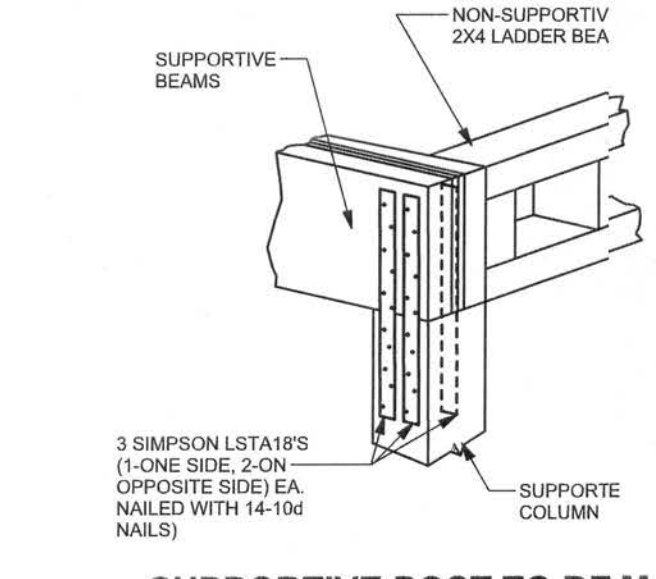
BEAM MID-WALL CONNECTION DETAIL

SCALE: N.T.S.



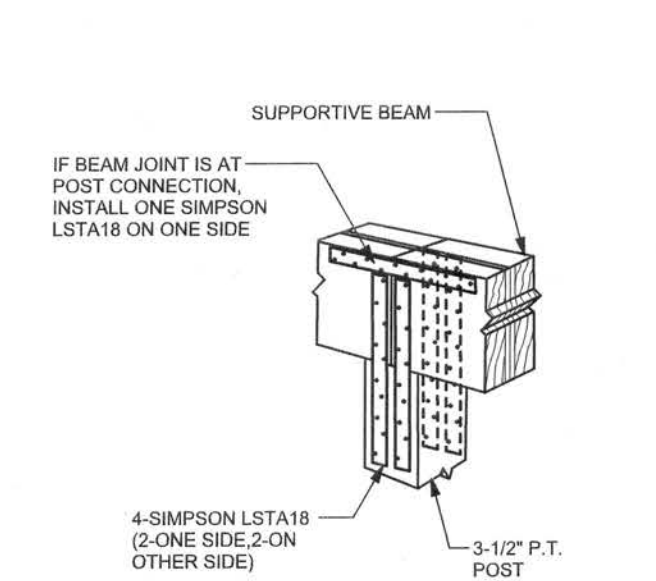
BEAM CORNER CONNECTION DETAIL

SCALE: N.T.S.



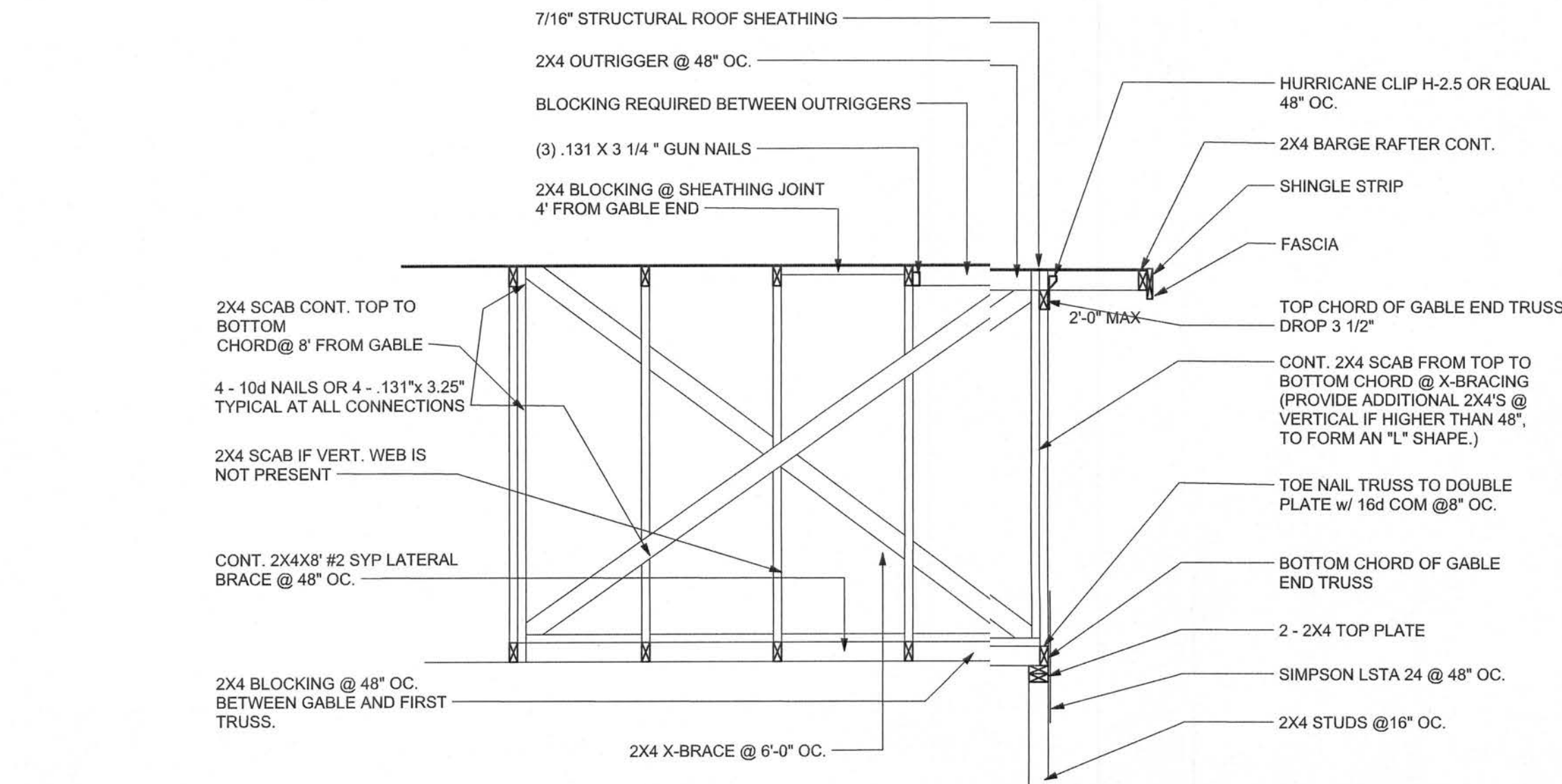
SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM

SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.



TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS
MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 2900	< 2490	2 - HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2050	< 1785	LG72	14 - 16d	14 - 16d	
HEAVY GIRDER TIEDOWNS*			TO FOUNDATION		
< 3965	< 3330	MG7		22 - 10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*			TO STUDS		
< 435	< 435	SSP DOUBLE TOP PLATE	3 - 10d		4 - 10d
< 455	< 420	SSP SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 - 10d		8 - 10d
< 825	< 800	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14 - 10d		
< 1235	< 1235	LSTA21	16 - 10d		
< 1030	< 1030	CS20	16-8d		
< 1705	< 1705	CS16	26-8d		
STUD ANCHORS*			TO STUDS	TO FOUNDATION	
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT131	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

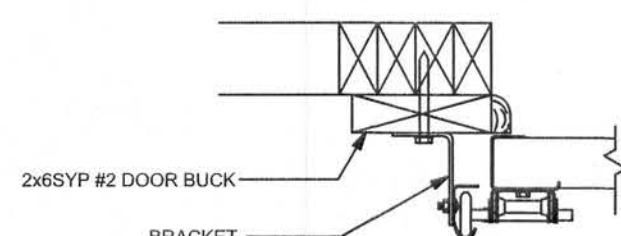
GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT
EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG
SCREWS w/ 1" WASHER LAG SCREWS MAY BE
COUNTERSUNK. HORIZONTAL JAMBS DO NOT
TRANSFER LOAD. CENTER LAG SCREWS OR
STAGGER 16d NAILS OR (2) ROWS OF .131" x 3 1/4"
GN PER TABLE BELOW:

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131" x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL

SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE
FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND
PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR
ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER
AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDERS
RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO
SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR
INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR
REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT
CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET
GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS
VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC
IN W1.4 CONFORMING TO ASTM A188. LOTS LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED
MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 5'.

FIBER CONCRETE SLAB: CONCRETE SLAB ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.
FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD.
PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER
TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN
ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH /
WIDTH RATIO OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 10 FT. DO NOT
CUT WYM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO
OWNER AND CONTRACTORS APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS
BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS);
UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-98, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, FB = 2400, E = 1800000 UNO. SUPPLIER MAY SUPPLY AN
ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16" OSB SHEATHING, UNLOCKED,
APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES
STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6" O.C. PANEL EDGES, 12" O.C. INTERMEDIATE
MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" O.C. UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS,
AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE
SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE
TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION
INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO
LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH
3/4" BOLTS TO BE 2" x 2" x 9/64", WITH 7/8" BOLTS TO BE 2" x 2" x 9/64", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST
REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

**THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE
SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.**

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND
BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004
REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU
BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL
THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS
DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,
TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL
BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION
R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN
TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS
THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE
COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS
MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN
PROFESSIONAL, FOR CORRECT APPLICATION OF FBC 2001 REQUIRED
LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO
REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF
SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL
BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF
DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT
RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE
TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES
RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED
TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;
MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT
ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10%
SLOPE AND UNOBSERVED UPLIFT FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (R2)		
	10	100	
1	19.9	21.8	18.1 -18.1
2	19.9	25.5	18.1 -21.8
2 Othg		40.8	40.8
3	19.9	25.5	18.1 -21.8
3 Othg		68.3	-42.4
4	21.8	23.6	18.5 -20.4
5	21.8	29.1	18.5 -22.6
Doors & Windows Worst Case (Zone 5, 10 R2)		21.8	-29.1
8x7 Garage Door		19.5	-22.9
16x7 Garage Door		18.5	-21.0

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

SOFTWARE
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway,
PE No3915, POB 868, Lake City, FL
32056386-754-5419

DIMENSIONS:
Standard dimensions supercede scaled
dimensions. Refer all questions to
Mark Disoway, P.E. for resolution.
Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portion of the plan, relating to wind engineering
comply with section R301.2.1, Florida building
code residential 2004, to the best of my
knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISOWAY
P.E. 53915

SEAL

Blake Construction

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-01, BCSI-02, & BCSI-03. BCSI-02, & BCSI-03 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

THREADED ROD LEGEND

- INDICATES LOCATION OF:
1ST FLOOR 1/2" A307 ALL THREADED ROD
- INDICATES LOCATION OF:
2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER LEGEND

- (2) 2X12X0'.1J 1K ← HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	36.2'	52.0'
LONGITUDINAL	35.3'	77.0'

WINDLOAD ENGINEER: Mark Disoway,
P.E. No. 3816, P.O. Box 868, Lake City, FL
32056-368-754-5419

DIMENSIONS:
Stated dimensions supersede scaled
dimensions. Refer all questions to
Mark Disoway, P.E. for resolution.
Do not proceed without clarification.

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code residential 2004, to the best of my
knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISOWAY
P.E. 53915

676
SEAL

Blake Construction

Spec House
Lot 1 341 Estates S/D

ADDRESS:
Lot 1 341 Estates S/D
Columbia County, Florida

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
fax: (386) 269 - 4871

PRINTED DATE:
June 07, 2006

DRAWN BY: STRUCTURAL BY:
David Disoway

FINALS DATE:
07 Jun / 06

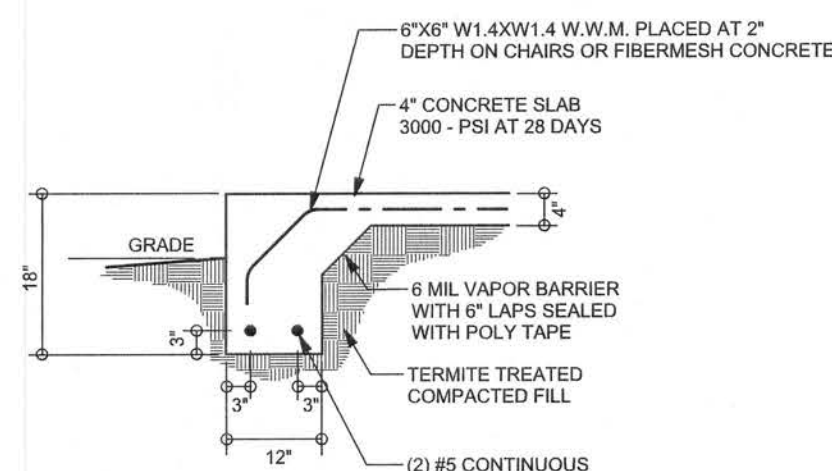
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DRAWING NUMBER

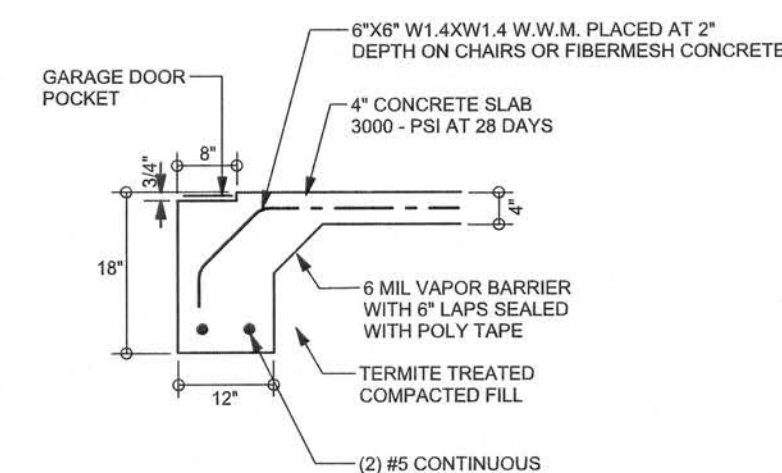
S-2

OF 2 SHEETS

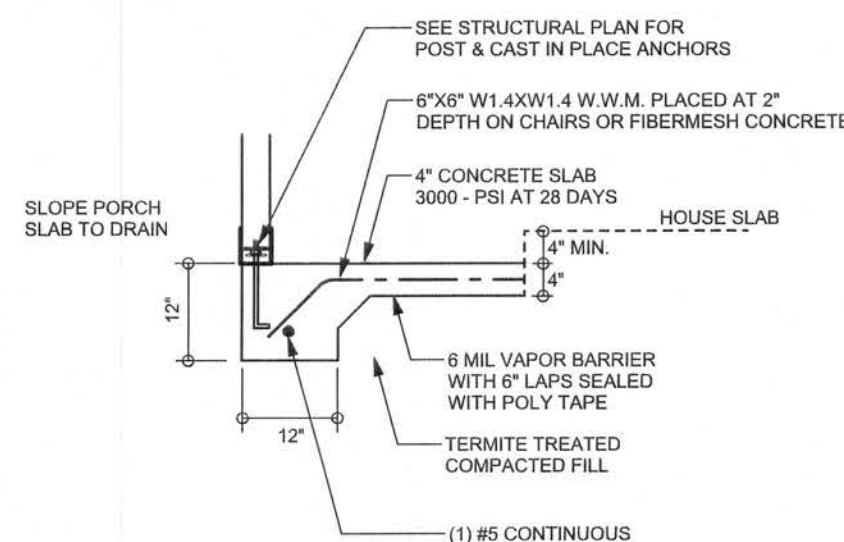
CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. BUILDERS FIRST SOURCE
JOB #L157307



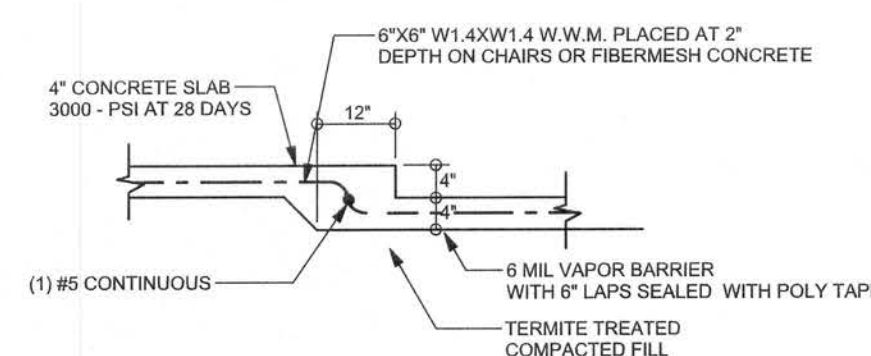
F1
S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



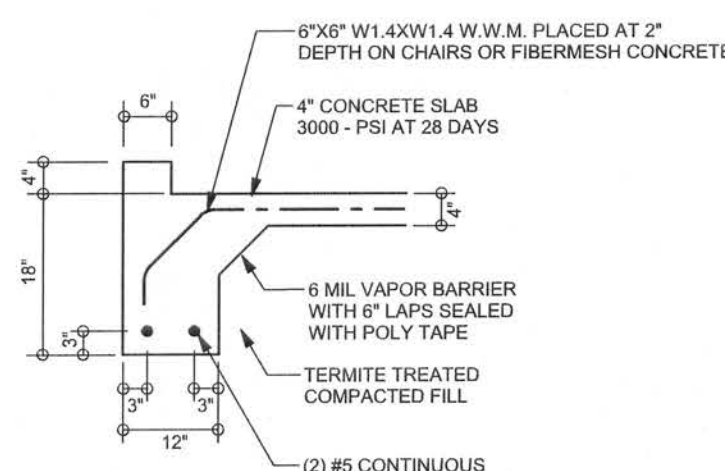
F4
S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



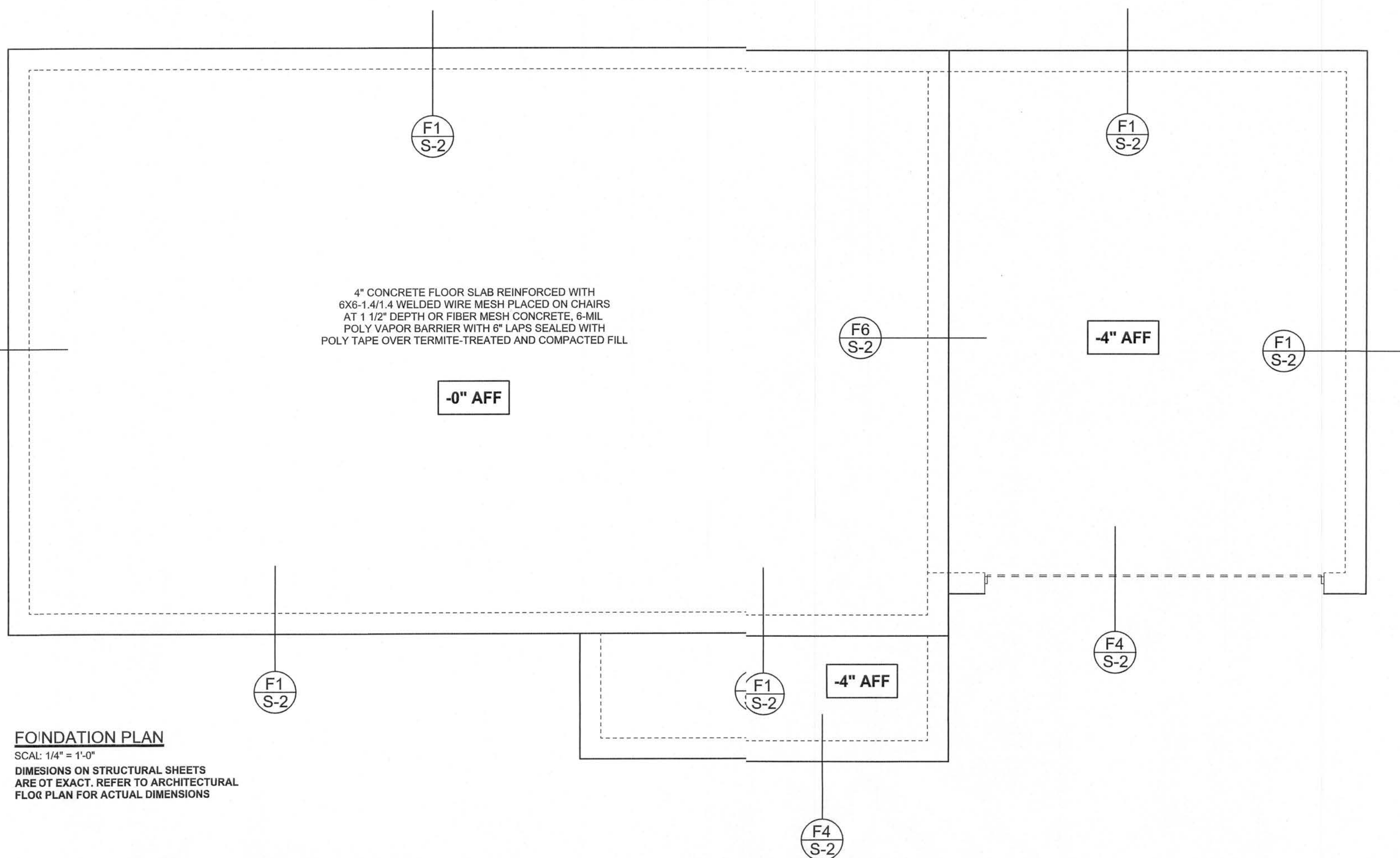
F5
S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



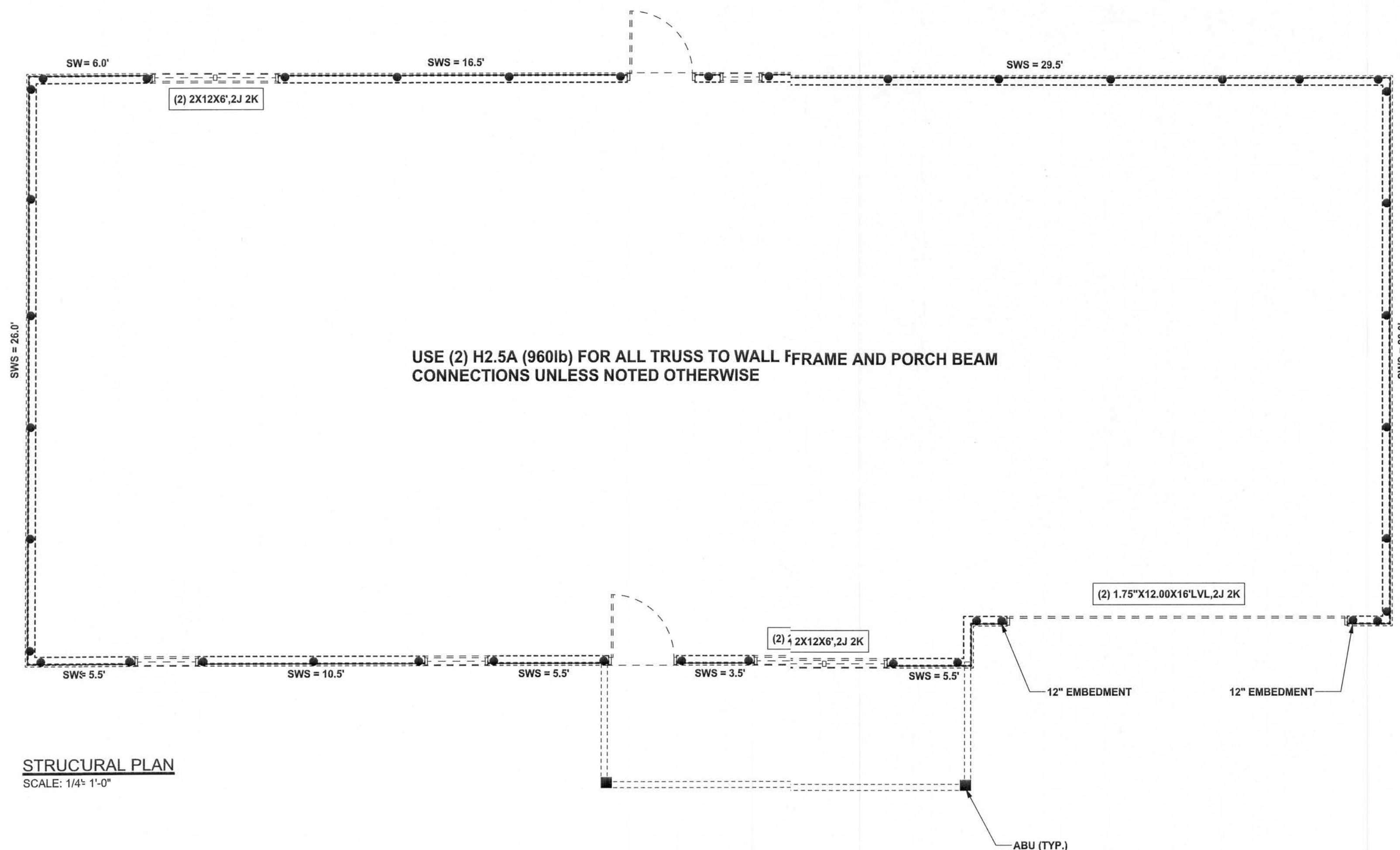
F6
S-2 TYPICAL NON - BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F8
S-2 GARAGE CURB FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

USE (2) H2.5A (960lb) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM
CONNECTIONS UNLESS NOTED OTHERWISE