

DATE 08/09/2016

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT

000034328

APPLICANT MIKE TODD PHONE 755-4387
ADDRESS 129 NE COLBURN AVE LAKE CITY FL 32055
OWNER DAVID RANDALL MORGAN PHONE 601-529-4755
ADDRESS 220 SW RIVERSIDE AVE FORT WHITE FL 32038
CONTRACTOR MIKE TODD PHONE 755-4387
LOCATION OF PROPERTY 47 S, R 27, L RIVERSIDE AVE, ON RIGHT LOT 5

TYPE DEVELOPMENT SFD, UTILITY ESTIMATED COST OF CONSTRUCTION 161900.00
HEATED FLOOR AREA 2128.00 TOTAL AREA 3238.00 HEIGHT 35.00 STORIES 2
FOUNDATION PIERS WALLS FRAMED ROOF PITCH 5/12 FLOOR WOOD
LAND USE & ZONING ESA-2 MAX. HEIGHT 35
Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 10.00
NO. EX.D.U. 1 FLOOD ZONE AE DEVELOPMENT PERMIT NO. 16-007

PARCEL ID 23-6S-15-00533-000 SUBDIVISION THREE RIVERS ESTATES
LOT 5 BLOCK PHASE UNIT 1 TOTAL ACRES 1.44

CRC006209
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor
EXISTING 16-046-M BS TC N
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident Time/STUP No.

COMMENTS: MINIMUM FLOOR ELEVATION 35.4', NEED ELEVATION CERTIFICATE BEFORE
POWER, 1 FT RISE REC'D, NOC ON FILE, SEE SURVEY FOR ELEVATIONS.

COMBINATION PERMIT WITH 34327 RELOCATION

Check # or Cash 17406

FOR BUILDING & ZONING DEPARTMENT ONLY

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by
Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by
Framing date/app. by Insulation date/app. by
Rough-in plumbing above slab and below wood floor date/app. by Electrical rough-in date/app. by
Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by Pool date/app. by
Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by
Pump pole date/app. by Utility Pole date/app. by M/H tie downs, blocking, electricity and plumbing date/app. by
Reconnection date/app. by RV date/app. by Re-roof date/app. by

BUILDING PERMIT FEE \$ 810.00 CERTIFICATION FEE \$ 16.19 SURCHARGE FEE \$ 16.19
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ 50.00 FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ TOTAL FEE 967.38

INSPECTORS OFFICE

CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO
THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY.
NOTICE: ALL OTHER APPLICABLE STATE OR FEDERAL PERMITS SHALL BE OBTAINED BEFORE COMMENCEMENT OF THIS
PERMITTED DEVELOPMENT.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR
IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY
BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED
WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR
ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN
APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID
WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Emailed for
site plans
8-2-16

Columbia County New Building Permit Application

#967.38 c/c 17406

RES.
CHECKLIST

For Office Use Only Application # 1606-37 Date Received 6/13 By LH Permit # 34328
Zoning Official BKS Date 7-29-16 Flood Zone AE Land Use ESA Zoning ESA-2
FEMA Map # 458C Elevation 34.4' MFE 35.4' River Fish Plans Examiner J.C. Date 8-1-16
Comments See Survey For Elevations / I Rise Letter Rec'd, need finished Const. Elevation letter before power
☒ NOC ☒ DEH ☒ Deed or PA ☒ Site Plan ☐ State Road Info ☒ Well letter ☒ 911 Sheet ☐ F W Comp. letter
☒ Dev Permit # 16-007 ☐ In Floodway ☐ Letter of Auth. from Contractor ☐ F W Comp. letter
☐ Owner Builder Disclosure Statement ☐ Land Owner Affidavit ☐ Ellisville Water ☒ App Fee Paid ☒ Sub VF Form

Septic Permit No. 16-0464-M OR City Water Fax _____
Applicant (Who will sign/pickup the permit) Mike Todd Phone 386-755-4387
Address 129 N. E Colburn Ave Lake City FL 32055
Owners Name David Randell Morgan Phone 601-529-4755
911 Address 220 S. W Riverside Ave Ft White FL 32038
Contractors Name Mike Todd Phone 386-755-4387
Address 129 N E Colburn Ave Lake City FL 32055
Contractor Email mike@miketoddconstruction.com ***Include to get updates on this job.

Fee Simple Owner Name & Address _____
Bonding Co. Name & Address _____
Architect/Engineer Name & Address Keen Engineering/Surveying Inc. 9263 CR 417 Live Oak FL 32060
Mortgage Lenders Name & Address _____

Circle the correct power company FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Duke Energy
Property ID Number 00-00-00-00533-000 Estimated Construction Cost \$229,908.73
Subdivision Name Three River Estates Lot 5 Block _____ Unit 1 Phase _____
Driving Directions from a Major Road 47 to SR 27 go left 5 miles to Riverside Ave, go left. Lot 5 is on Right.

Construction of New House Commercial OR ☒ Residential
Proposed Use/Occupancy Sgl. Fam Number of Existing Dwellings on Property 1
Is the Building Fire Sprinkled? N If Yes, blueprints included _____ Or Explain _____

Circle Proposed ☒ Culvert Permit or Culvert Waiver or D.O.T. Permit or Have an Existing Drive
Actual Distance of Structure from Property Lines - Front _____ Side See Site Plan Side _____ Rear _____
Number of Stories 2 Heated Floor Area 2128 Total Floor Area 3238 Acreage 1.542
Zoning Applications applied for (Site & Development Plan, Special Exception, etc.) Connected to Same

power supply
1st Mike & VM 8-2-16

Columbia County Building Permit Application

CODE: Florida Building Code 2014 and the 2011 National Electrical Code.

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless pursued in good faith or a permit has been issued.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO CONTRACTOR AND AGENT: **YOU ARE HEREBY NOTIFIED** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

David Morgan

Print Owners Name

[Signature]

Owners Signature

****Property owners must sign here before any permit will be issued.**

****If this is an Owner Builder Permit Application then, ONLY the owner can sign the building permit when it is issued.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

[Signature]
Contractor's Signature

Contractor's License Number CGC006209
Columbia County
Competency Card Number 539

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 13 day of June 2016

Personally known [initials] or Produced Identification [initials]

[Signature]
State of Florida Notary Signature (For the Contractor)



Recreation 'Carn' SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER 1606-36 CONTRACTOR Mike Todd PHONE 386-755-4387
 THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL	Print Name <u>Math Burns Electric</u>	Signature <u>Sec Attached</u>	Phone #:
<u>W.C.</u>	License #: <u>EC13006531</u>		<u>386-935-0444</u>
MECHANICAL/	Print Name <u>D.L. Williams Htg & Cooling</u>	Signature <u>D.L. Williams</u>	Phone #:
<u>A/C 13</u>	License #: <u>CAC1816913</u>		<u>386-754-1987</u>
PLUMBING/	Print Name <u>Hometown Plumbing</u>	Signature <u>D.R.</u>	Phone #:
<u>GAS 298</u>	License #: <u>CPC1728890</u>		<u>386-754-6140</u>
ROOFING	Print Name <u>Mike Todd Construction</u>	Signature <u>[Signature]</u>	Phone #:
	License #:		
SHEET METAL	Print Name _____	Signature _____	Phone #:
	License #: _____		
FIRE SYSTEM/	Print Name _____	Signature _____	Phone #:
SPRINKLER	License #: _____		
SOLAR	Print Name _____	Signature _____	Phone #:
	License #: _____		

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON <u>LING</u>	<u>000222</u>	<u>Harold Houston Construction</u>	<u>[Signature]</u>
CONCRETE FINISHER	<u>000310</u>	<u>Parrish Enterprises</u>	<u>[Signature]</u>
FRAMING			
INSULATION			
STUCCO			
DRYWALL			
PLASTER			
CABINET INSTALLER			
PAINTING	<u>1040</u>	<u>John Davis Painting</u>	<u>[Signature]</u>
ACOUSTICAL CEILING			
GLASS			
CERAMIC TILE			
FLOOR COVERING	<u>000710</u>	<u>Brown Vann - Carpet One</u>	<u>[Signature]</u>
ALUM/VINYL SIDING			
GARAGE DOOR			
METAL BLDG ERECTOR			

F. S. 440.103 Building permits; identification of minimum premium policy.—Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

Development Permit
F 023- 16-007

FLOOD ZONE AE BY BS 2-4-2009 FIRM COMMUNITY # 120070 - PANEL # 458-C
FIRM 100 YEAR ELEVATION 34.4' PLAN INCLUDED YES or NO
REQUIRED LOWEST HABITABLE FLOOR ELEVATION 35.4'
IN THE REGULATORY FLOODWAY YES or NO RIVER Itchetucknee
SURVEYOR / ENGINEER NAME Curtis Keen LICENSE NUMBER 23836

DATE THE FINISHED FLOOR ELEVATION CERTIFICATE WAS PROVIDED _____



STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL
SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. 16-0464M
DATE PAID: 8/2/16
FEE PAID: 203.00
RECEIPT #: 1250664

APPLICATION FOR:

☐ New System ☐ Existing System ☐ Holding Tank ☐ Innovative
☐ Repair ☐ Abandonment ☐ Temporary ☒ MODIFICATION

APPLICANT: David MorganAGENT: ROCKY FORD, A & B CONSTRUCTIONTELEPHONE: 386-497-2311MAILING ADDRESS: 546 SW Dortch Street, FT. WHITE, FL, 32038

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3) (m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: 5 BLOCK: na SUB: Three Rivers Estates unit 1 PLATTED: 5/1/02PROPERTY ID #: 00-00-00-00533-000 ZONING: Res I/M OR EQUIVALENT: ☐ Y ☒ NPROPERTY SIZE: 1.54 ACRES WATER SUPPLY: ☒ PRIVATE PUBLIC ☐ <=2000GPD ☐ >2000GPDIS SEWER AVAILABLE AS PER 381.0065, FS? ☐ Y ☒ N DISTANCE TO SEWER: — FTPROPERTY ADDRESS: 218 SW Riverside Ave, Fort White, FL, 32038

DIRECTIONS TO PROPERTY: 47 South, TR on US 27, TL on Riverside (3 Rivers), 5th lot
on the right

BUILDING INFORMATION

☒ RESIDENTIAL ☐ COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	SF Residential	3	2512	ORIGINAL 384 SQ Feet ADDING
2				2128 SQ Feet 3/5R'S
3				

☒ Floor/Equipment Drains ☒ Other (Specify) _____SIGNATURE: Rocky D Ford DATE: 1/26/2016

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number _____

MORGAN

----- PART II - SITEPLAN -----

Scale: 1 inch = 40 feet.

SEE ATTACHED

Notes: _____

Site Plan submitted by: *Rocky D. F...*

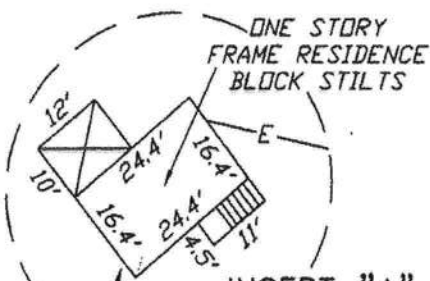
MASTER CONTRACTOR

Plan Approved *[Signature]* Not Approved _____

Date *8/11/16*

By *[Signature]* *Chen...* County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT



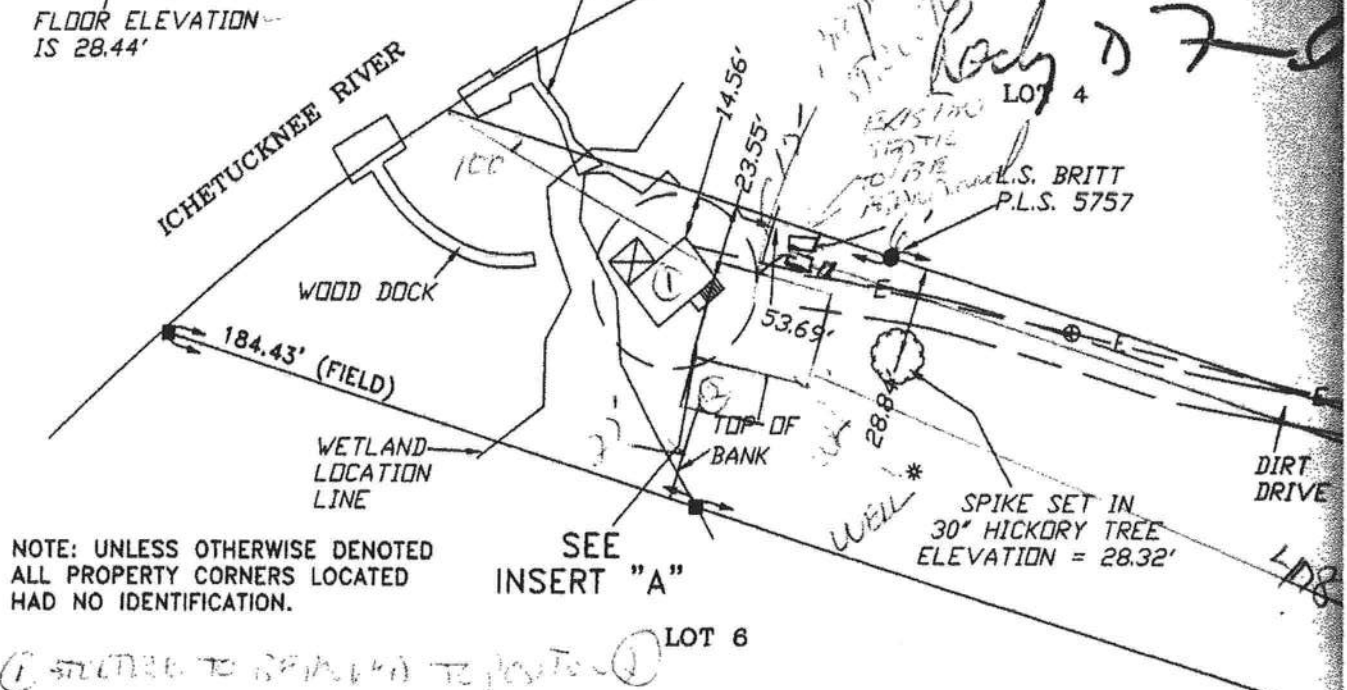
INSERT "A"
SCALE: 1" = 30'

FLOOR ELEVATION
IS 28.44'

DESCRIPTION:
LOT 5 OF "THREE RIVERS ESTATES SECTION NO. 1" AS PER
PLAT THEREOF RECORDED IN PLAT BOOK 3, PAGE 53 OF THE
PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.
TOGETHER WITH AND SUBJECT TO AN EASEMENT FOR INGRESS
AND EGRESS.
(SEE SHEET # 2 FOR DESCRIPTION AND DETAILS)

WOOD DOCK
APPARENT USE
(DECK ELEVATION
IS 20.98')

AUG 01 2016



NOTE: UNLESS OTHERWISE DENOTED
ALL PROPERTY CORNERS LOCATED
HAD NO IDENTIFICATION.

SEE
INSERT "A"

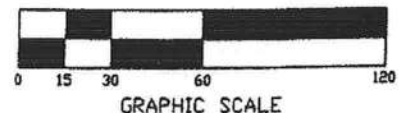
LOT 6

SYMBOL LEGEND:

■ 4"x4" CONCRETE MONUMENT FOUND	⊕ CENTERLINE
□ 4"x4" CONCRETE MONUMENT SET	- - E - - ELECTRIC LINES
● IRON PIPE FOUND	- - X - - WIRE FENCE
○ IRON PIN AND CAP SET	- - O - - CHAIN LINK FENCE
* 'X' CUT IN PAVEMENT	- - D - - WOODEN FENCE
+ CALCULATED PROPERTY CORNER	- - - - SECTION LINE
⊙ NAIL & DISK	(PLAT) AS PER A PLAT OF RECORD
⊕ POWER POLE	(DEED) AS PER A DEED OF RECORD
▲ WATER METER	(CALC.) AS PER CALCULATIONS
⊙ UTILITY BOX	(FIELD) AS PER FIELD MEASUREMENTS
* WELL	P.R.M. PERMANENT REFERENCE MARKER
⊕ SANITARY MANHOLE	P.C.P. PERMANENT CONTROL POINT
+ SIGN POST	



SCALE: 1" = 60'



CERTIFIED TO:

DAVID & BRYAN MORGAN

SURVEYOR'S CERTIFICATE

I HEREBY CERTIFY THAT THIS SURVEY WAS MADE UNDER MY
TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD
IN CHAPTER 65B-07, F.A.C. (F.L.A.C. 65B-07.001) AND THE FLORIDA BOARD

10/29/15
FIELD SURVEY DATE

11/05/15
DRAWING DATE

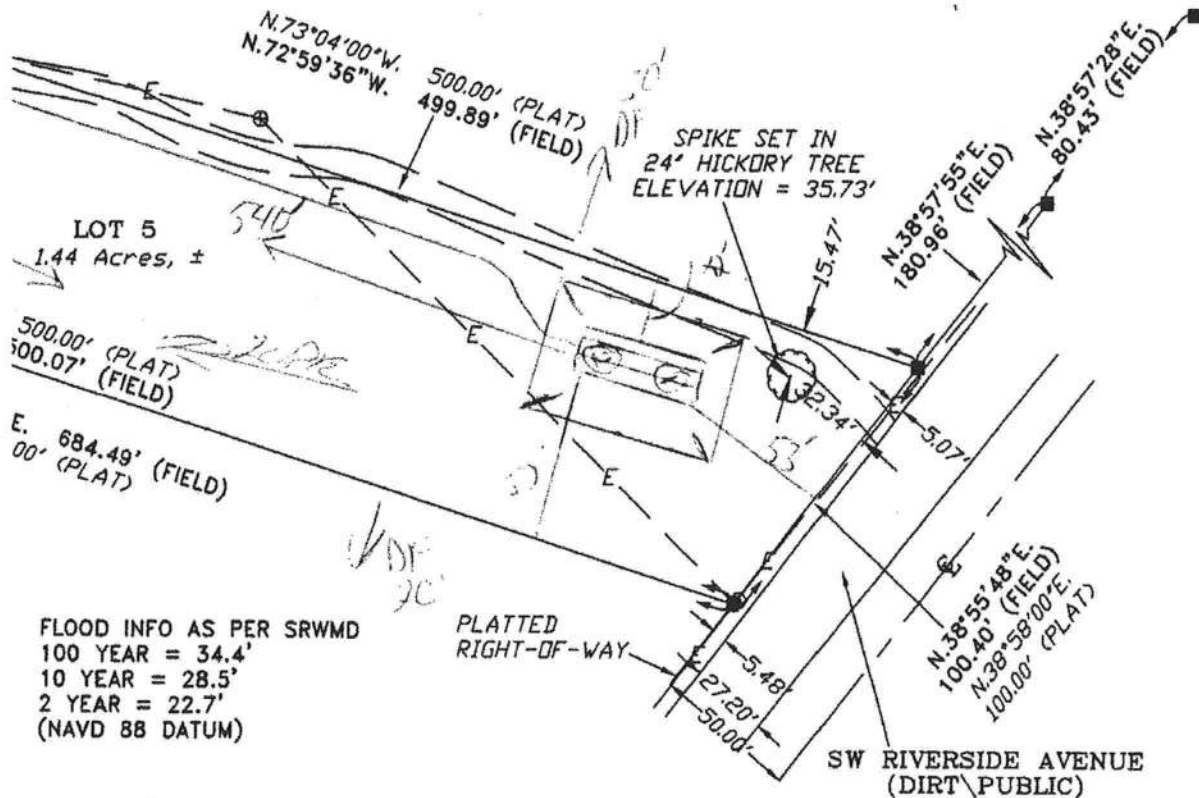
NOTE: UNLESS IT BEARS THE SIGNATURE AND THE ORIGINAL ROLL
HAPPER THIS DRAWING, SKETCH, PLAT OR MAP IS FOR INFORMATION

FIELD BOOK: SEE PAGE(S): FILE

BOUNDARY SURVEY IN SECTION 26, TOWNSHIP 6 SOUTH,
RANGE 15 EAST, COLUMBIA COUNTY, FLORIDA.

SURVEYOR'S NOTES:

1. BOUNDARY BASED ON MONUMENTATION FOUND IN ACCORDANCE WITH THE RETRACEMENT OF THE ORIGINAL SURVEY FOR SAID PLAT OF RECORD.
2. BEARINGS ARE BASED ON SAID PLAT OF RECORD.
3. THIS PARCEL IS IN ZONE "AE" AND IS SUBJECT TO FLOODING. A BASE FLOOD ELEVATION IS ESTABLISHED TO BE 34.4 FEET AS PER FLOOD INSURANCE RATE MAP, DATED 4 FEBRUARY, 2009 FIRM PANEL NO. 12023C0458C. HOWEVER, THE FLOOD INSURANCE RATE MAPS ARE SUBJECT TO CHANGE. (BASED ON NAVD 88 DATUM)
4. THE IMPROVEMENTS, IF ANY, INDICATED ON THIS SURVEY DRAWING ARE AS LOCATED ON DATE OF FIELD SURVEY AS SHOWN HEREDON.
5. IF THEY EXIST, NO UNDERGROUND ENCROACHMENTS AND/OR UTILITIES WERE LOCATED FOR THIS SURVEY EXCEPT AS SHOWN HEREDON.
6. THIS SURVEY WAS COMPLETED WITHOUT THE BENEFIT OF A TITLE COMMITMENT OR A TITLE POLICY.
7. DIMENSIONS SHOWN HEREDON ARE IN FEET AND DECIMAL PARTS THEREOF.
8. THIS SURVEY DOES NOT REFLECT OR DETERMINE OWNERSHIP.
9. THE ADJACENT OWNERSHIP INFORMATION AS SHOWN HEREDON IS BASED ON THE COUNTY PROPERTY APPRAISERS GIS SYSTEM, UNLESS OTHERWISE DENOTED.
10. ALL ELEVATIONS SHOWN HEREDON ARE BASED ON NAVD 88 DATUM.



POSSIBLE CHARGE AND MEETS THE MINIMUM
* PROFESSIONAL SURVEYORS AND MAPPERS
* CERTIFICATION # 5757

L. SCOTT BRITT, P.S.M.
CERTIFICATION # 5757
SEAL OF A FLORIDA LICENSED SURVEYOR AND
PURPOSES ONLY AND IS NOT VALID.



BRITT SURVEYING & MAPPING, LLC

LAND SURVEYORS AND MAPPERS, L.B. # 8016

2086 SW MAIN BLVD, SUITE 112, LAKE CITY, FLORIDA 32025

(386)752-7163 FAX (386)752-5573

www.brittsurvey.com

WORK ORDER # L-23730

ONE FOOT RISE CERTIFICATION

OWNER: David & Bryn Morgan

DESCRIPTION: Lot 5, Three Rivers Estates Section no. 1, Columbia
County, Florida

BASE FLOOD ELEVATION: 34.4'

COMMUNITY PANEL NUMBER: 12023C0458C dated 02/04/2009

PROJECT: 28' x 46' HOME, 28'x46' CONCRETE PAD AT GRADE
24' x 16' EXISTING HOME/10'x12' EXISTING PORCH

FLOOD AREA (isolated) AT BASE FLOOD ELEVATION= 140 ACRES

FILL OF FLOODPLAIN: N/A

LOWEST AVERAGE GROUND ELEVATION AT BUILDING = 24.5'

I hereby certify that the placement of the 28'x46' raised home on columns, 28'x 46' concrete pad at grade, 24'x 16' home/10'x12' proch will increase the Suwannee River floodplain less than one foot at the project location. The average ground elevation at the home is 24.5'. The lowest horizontal member of the home will be required to be set at 12 inches above the 100 year flood minimum.


Curtis E. Keen, PE #23836
EB #3761

Date: 07/26/16

Copy: David Morgan



ONE FOOT RISE CALCULATIONS

OWNER: David & Bryn Morgan

DESCRIPTION: Lot 5, Three Rivers Estates Section no. 1, Columbia County, Florida

BASE FLOOD ELEVATION: 34.4'

COMMUNITY PANEL NUMBER: 12023C0458C dated 02/04/2009

PROJECT: 28' x 46' HOME, 28'x 46' CONCRETE PAD AT GRADE.
24' x 16' EXISTING HOME/10'x12' EXISTING PORCH

FLOOD AREA (isolated) AT BASE FLOOD ELEVATION= 140 ACRES

FILL OF FLOODPLAIN: N/A

LOWEST AVERAGE GROUND ELEVATION AT BUILDING = 24.5'

This project will be located in the staging area of the Ichetucknee River and not the floodway.
No step backwater calculations are required. The calculations are based on the amount of
floodplain volume removed if the foundation is enclosed.

$$\% \text{ FLOODPLAIN AREA REMOVED} = \frac{1,792 \text{ s.f.}/43,560 \text{ s.f.}}{140 \text{ acres}} = 0.0294\%$$

$$\text{FLOODPLAIN LEVEL INCREASE} = \frac{1,792 \text{ s.f.} \times 1.0 \text{ feet}}{140 \text{ ac.} \times 43,560 \text{ s.f.}} = 0.0003 \text{ foot}$$

Curtis E. Keen 7/26/16

Curtis E. Keen, PE #23836
EB #3761

Date: 07/26/16



Columbia County Property Appraiser

updated: 5/5/2016

2015 Tax Year**Parcel:** 00-00-00-00533-000

<< Next Lower Parcel Next Higher Parcel >>

Tax Collector

Tax Estimator

Property Card

Parcel List Generator

Interactive GIS Map

Print

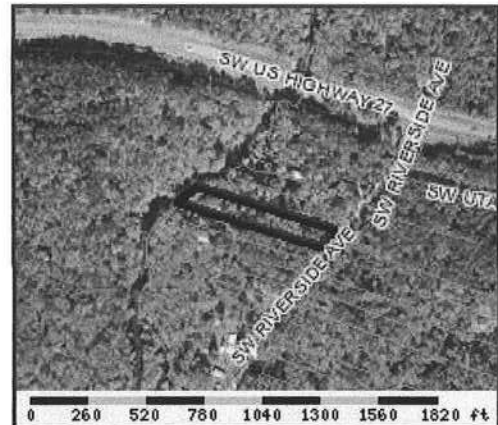
<< Prev

Search Result: 16 of 73

Next >>

Owner & Property Info

Owner's Name	MORGAN DAVID RANDAL &		
Mailing Address	BRYNN PARKER MORGAN 2103 WHITNEY OAKS DR OCEAN SPRINGS, MS 39564		
Site Address	218 SW RIVERSIDE AVE		
Use Desc. (code)	SINGLE FAM (000100)		
Tax District	3 (County)	Neighborhood	100000
Land Area	1.542 ACRES	Market Area	02
Description	NOTE: This description is not to be used as the Legal Description for this parcel in any legal transaction. LOT 5 UNIT 1 THREE RIVERS ESTATES. ORB 327-638, WD 1195- 1190, WD 1282-590,		

**Property & Assessment Values**

2015 Certified Values		
Mkt Land Value	cnt: (0)	\$95,000.00
Ag Land Value	cnt: (1)	\$0.00
Building Value	cnt: (1)	\$12,907.00
XFOB Value	cnt: (1)	\$1,914.00
Total Appraised Value		\$109,821.00
Just Value		\$109,821.00
Class Value		\$0.00
Assessed Value		\$109,821.00
Exempt Value		\$0.00
Total Taxable Value	Cnty: \$109,821 Other: \$109,821 Schl: \$109,821	

2016 Working Values (...Hide Values)		
Mkt Land Value	cnt: (0)	\$95,000.00
Ag Land Value	cnt: (1)	\$0.00
Building Value	cnt: (1)	\$12,504.00
XFOB Value	cnt: (1)	\$1,914.00
Total Appraised Value		\$109,418.00
Just Value		\$109,418.00
Class Value		\$0.00
Assessed Value		\$109,418.00
Exempt Value		\$0.00
Total Taxable Value	Cnty: \$109,418 Other: \$109,418 Schl: \$109,418	

NOTE: 2016 Working Values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Sales History

Show Similar Sales within 1/2 mile

Sale Date	OR Book/Page	OR Code	Vacant / Improved	Qualified Sale	Sale RCode	Sale Price
9/29/2014	1282/590	WD	I	Q	01	\$142,500.00
5/28/2010	1195/1190	WD	I	Q	01	\$141,300.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	SFR PILING (000300)	1960	AVERAGE (05)	384	504	\$12,504.00
Note: All S.F. calculations are based on exterior building dimensions.						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0084	DOCK-RIVER	1993	\$1,914.00	0000330.000	0 x 0 x 0	(000.00)

Neighbor's
Main House

NOTICE OF COMMENCEMENT

Tax Parcel Identification Number:

00-00-00-00533-000

Clerk's Office Stamp

Inst: 201612009778 Date: 06/13/2016 Time: 2:34PM
Page 1 of 1 B: 1316 P: 1912, P.DeWitt Cason, Clerk of Court
Columbia, County, By: BD
Deputy Clerk

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

1. Description of property (legal description): Lot 5 Unit 1 Three Rivers Estates
a) Street (job) Address: 218 S.W. Riverside Ave Ft White FL 32038
2. General description of improvements: Re locate EXISTING Cabin, Build new House
3. Owner Information or Lessee information if the Lessee contracted for the improvements:
a) Name and address: David Morgan 218 S.W. Riverside Ave Ft White FL 32038
b) Name and address of fee simple titleholder (if other than owner) _____
c) Interest in property Owner
4. Contractor Information
a) Name and address: Mike Todd Construction 129 N.E Colburn Ave Lake City FL 32055
b) Telephone No.: 386-755-4387
5. Surety Information (if applicable, a copy of the payment bond is attached):
a) Name and address: _____
b) Amount of Bond: _____
c) Telephone No.: _____
6. Lender
a) Name and address: _____
b) Phone No.: _____
7. Person within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes:
a) Name and address: Mike Todd Construction 129 N.E Colburn Ave Lake City FL 32055
b) Telephone No.: 386-755-4387
8. In addition to himself or herself, Owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
a) Name: Mike Todd OF Mike Todd Construction, Inc.
b) Telephone No.: 386-755-4387
9. Expiration date of Notice of Commencement (the expiration date will be 1 year from the date of recording unless a different date is specified): _____

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

10. [Signature]
Signature of Owner or Lessee, or Owner's or Lessee's Authorized Office/Director/Partner/Manager
Contractor Mike Todd
Printed Name and Signatory's Title/Office

The foregoing instrument was acknowledged before me, a Florida Notary, this 13 day of June, 2016, by:
Mike Todd as Agent for David Morgan
(Name of Person) (Type of Authority) (name of party on behalf of whom instrument was executed)

Personally Known ☒ OR Produced Identification ☐ Type _____

Notary Signature [Signature]

Notary Stamp or Seal



Columbia County Property Appraiser

updated: 5/5/2016

2015 Tax Year

Tax Collector

Tax Estimator

Property Card

Parcel List Generator

Parcel: 00-00-00-00533-000

<< Next Lower Parcel Next Higher Parcel >>

Interactive GIS Map

Print

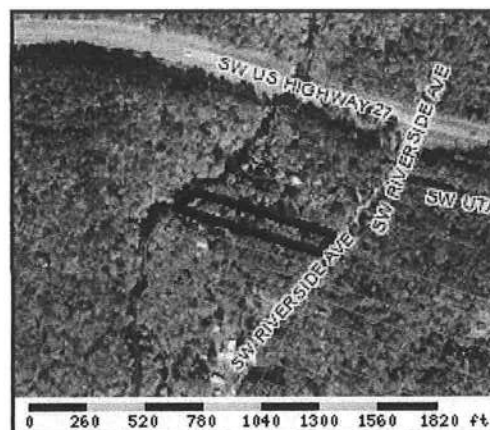
Owner & Property Info

<< Prev

Search Result: 16 of 73

Next >>

Owner's Name	MORGAN DAVID RANDAL &		
Mailing Address	BRYNN PARKER MORGAN 2103 WHITNEY OAKS DR OCEAN SPRINGS, MS 39564		
Site Address	218 SW RIVERSIDE AVE		
Use Desc. (code)	SINGLE FAM (000100)		
Tax District	3 (County)	Neighborhood	100000
Land Area	1.542 ACRES	Market Area	02
Description	NOTE: This description is not to be used as the Legal Description for this parcel in any legal transaction. LOT 5 UNIT 1 THREE RIVERS ESTATES. ORB 327-638, WD 1195- 1190, WD 1282-590.		



Property & Assessment Values

2015 Certified Values		
Mkt Land Value	cnt: (0)	\$95,000.00
Ag Land Value	cnt: (1)	\$0.00
Building Value	cnt: (1)	\$12,907.00
XFOB Value	cnt: (1)	\$1,914.00
Total Appraised Value		\$109,821.00
Just Value		\$109,821.00
Class Value		\$0.00
Assessed Value		\$109,821.00
Exempt Value		\$0.00
Total Taxable Value	Cnty: \$109,821 Other: \$109,821 Schl: \$109,821	

2016 Working Values (...Hide Values)		
Mkt Land Value	cnt: (0)	\$95,000.00
Ag Land Value	cnt: (1)	\$0.00
Building Value	cnt: (1)	\$12,504.00
XFOB Value	cnt: (1)	\$1,914.00
Total Appraised Value		\$109,418.00
Just Value		\$109,418.00
Class Value		\$0.00
Assessed Value		\$109,418.00
Exempt Value		\$0.00
Total Taxable Value	Cnty: \$109,418 Other: \$109,418 Schl: \$109,418	

NOTE: 2016 Working Values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Sales History

Show Similar Sales within 1/2 mile

Sale Date	OR Book/Page	OR Code	Vacant / Improved	Qualified Sale	Sale RCode	Sale Price
9/29/2014	1282/590	WD	I	Q	01	\$142,500.00
5/28/2010	1195/1190	WD	I	Q	01	\$141,300.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	SFR PILING (000300)	1960	AVERAGE (05)	384	504	\$12,504.00
Note: All S.F. calculations are based on <u>exterior</u> building dimensions.						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0084	DOCK-RIVER	1993	\$1,914.00	0000330.000	0 x 0 x 0	(000.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000132	SFR RIVER (MKT)	100 FF - (0000001.542AC)	1.00/1.00/1.00/1.00	\$950.00	\$95,000.00

Columbia County Property Appraiser

updated: 5/5/2016

<< Prev

16 of 73

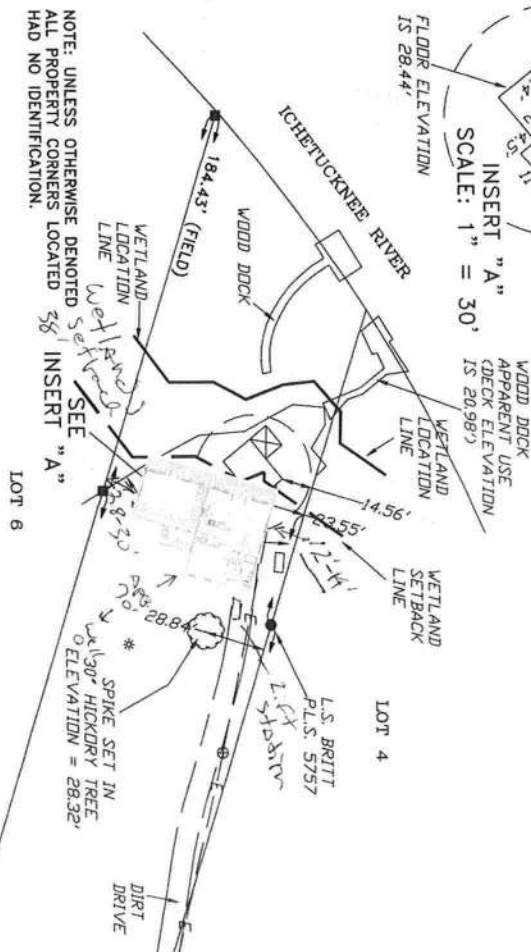
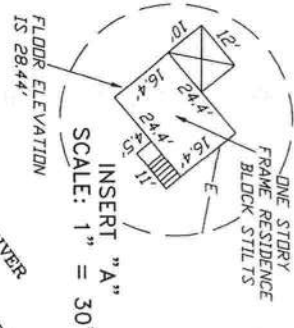
Next >>

DISCLAIMER

This information was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

BOUNDARY SURVEY IN SECTION 26, TOWNSHIP 6 SOUTH,
RANGE 15 EAST, COLUMBIA COUNTY, FLORIDA.

DESCRIPTION:
LOT 5 OF "THREE RIVERS ESTATES SECTION NO. 1" AS PER THE
PLAT THEREOF RECORDED IN PLAT BOOK 3, PAGE 53 OF THE
PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA,
TOGETHER WITH AND SUBJECT TO AN EASEMENT FOR INGRESS
(SEE SHEET # 2 FOR DESCRIPTION AND DETAILS)

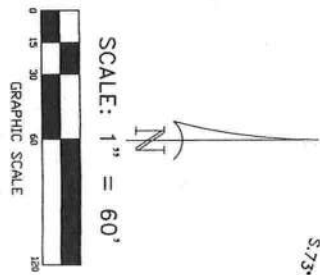


NOTE: UNLESS OTHERWISE DENOTED
ALL PROPERTY CORNERS LOCATED
HAD NO IDENTIFICATION.

LOT 6

SYMBOL LEGEND:

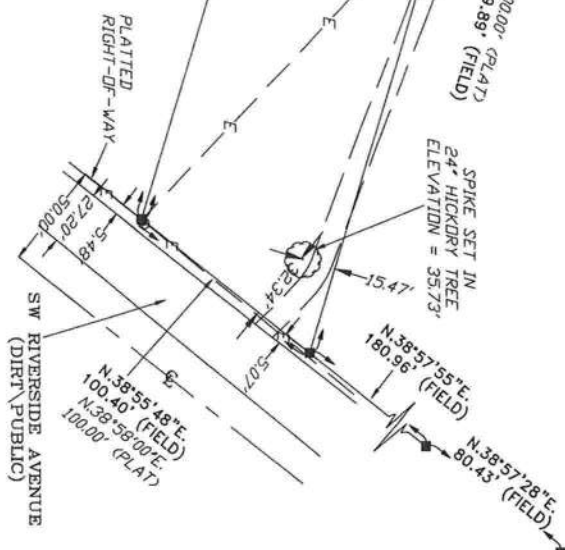
4x4' CONCRETE MONUMENT FOUND	CENTERLINE
4x4' CONCRETE MONUMENT SET	ELECTRIC LINES
IRON PIPE FOUND	WIRE FENCE
IRON PIN AND CAP SET	CHAIN LINK FENCE
1/2" CUT IN PAVEMENT	WOODEN FENCE
+ CALCULATED PROPERTY CORNER	SECTION LINE
+ NAIL & DISK	(PLAT) AS PER A PLAT OF RECORD
+ POWER POLE	(DEED) AS PER A DEED OF RECORD
+ WATER METER	(CALC.) AS PER CALCULATIONS
+ UTILITY BOX	(FIELD) AS PER FIELD MEASUREMENTS
* WELL	P.R.M. PERMANENT REFERENCE MARKER
+ SANITARY MANHOLE	P.C.P. PERMANENT CONTROL POINT
+ SIGN POST	



SCALE: 1" = 60'

FLOOD INFO AS PER SRMWD
100 YEAR = 34.4'
10 YEAR = 28.5'
2 YEAR = 22.7'
(NAVD 88 DATUM)

- SURVEYOR'S NOTES:
1. BOUNDARY BASED ON MONUMENTATION FOUND IN ACCORDANCE WITH THE RETRACEMENT OF THE ORIGINAL SURVEY FOR SAID PLAT OF RECORD.
 2. BEARINGS ARE BASED ON SAID PLAT OF RECORD.
 3. THIS PARCEL IS IN ZONE "AEC" AND IS SUBJECT TO FLOODING. A BASE FLOOD ELEVATION IS ESTABLISHED TO BE 34.4 FEET AS PER FLOOD INSURANCE RATE MAP, DATED 4 FEBRUARY, 2009 FIRM PANEL NO. 12023C0458C. HOWEVER, THE FLOOD INSURANCE RATE MAPS ARE SUBJECT TO CHANGE. (BASED ON NAVD 88 DATUM)
 4. THE IMPROVEMENTS, IF ANY, INDICATED ON THIS SURVEY DRAWING ARE AS LOCATED ON DATE OF FIELD SURVEY AS SHOWN HEREON.
 5. IF THEY EXIST, NO UNDERGROUND ENCROACHMENTS AND/OR UTILITIES WERE LOCATED FOR THIS SURVEY EXCEPT AS SHOWN HEREON.
 6. THIS SURVEY WAS COMPLETED WITHOUT THE BENEFIT OF A TITLE COMMITMENT OR A TITLE POLICY.
 7. DIMENSIONS SHOWN HEREON ARE IN FEET AND DECIMAL PARTS THEREOF.
 8. THIS SURVEY DOES NOT REFLECT OR DETERMINE DIMENSION.
 9. THE ADJACENT DIMENSIONSHIP INFORMATION AS SHOWN HEREON IS BASED ON THE COUNTY PROPERTY APPRAISERS GIS SYSTEM, UNLESS OTHERWISE DENOTED.
 10. ALL ELEVATIONS SHOWN HEREON ARE BASED ON NAVD 88 DATUM.



CERTIFIED TO:

DAVID & BRYAN MORGAN

[Signature]

FIELD BOOK, SEE PAGE(S), FILE

DIRECTOR'S CERTIFICATION

I HEREBY CERTIFY THAT THIS SURVEY HAS BEEN MADE IN ACCORDANCE WITH THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPERS IN CHAPTER 46B, F.S. (FLORIDA STATUTES) AND THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPERS THIS DRAWING, SKETCH, PLAT OR MAP IS FOR INFORMATIONAL PURPOSES ONLY AND IS NOT VALID.

10/29/15

11/05/15

SCOTT BRITT, P.S.

CERTIFICATION # 5757

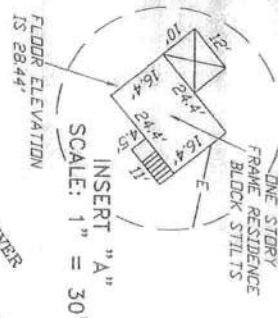


BRITT SURVEYING
& MAPPING, LLC

LAND SURVEYORS AND MAPPERS, L.B. # 8016
2006 SW MAIN BLVD., SUITE 112, LAKE CITY, FLORIDA 32805
(386) 752-7168
www.brittsurveying.com
WORK ORDER # L-23730

BOUNDARY SURVEY IN SECTION 26, TOWNSHIP 6 SOUTH,
RANGE 15 EAST, COLUMBIA COUNTY, FLORIDA.

DESCRIPTION:
LOT 5 OF "THREE RIVERS ESTATES SECTION NO. 1" AS PER THE
PLAT THEREOF RECORDED IN PLAT BOOK 3, PAGE 59 OF THE
PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA
TOGETHER WITH AND SUBJECT TO AN EASEMENT FOR INGRESS
AND EGRESS (SEE SHEET # 2 FOR DESCRIPTION AND DETAILS)



WOOD DOCK
APPROXIMATE USE
DECK ELEVATION
IS 20.98'



SYMBOL LEGEND:

■ 4"x4" CONCRETE MONUMENT FOUND	—E— CENTERLINE
□ 4"x4" CONCRETE MONUMENT SET	—E— ELECTRIC LINES
○ IRON PIPE FOUND	—X— WIRE FENCE
○ IRON PIN AND CAP SET	—O— CHAIN LINK FENCE
× "X" CUT IN PAVEMENT	—O— VIDDEN FENCE
+ CALCULATED PROPERTY CORNER	— — — SECTION LINE
⊙ MAIL & DISK	(PLAT) AS PER A PLAT OF RECORD
⊙ POWER POLE	(DEED) AS PER A DEED OF RECORD
⊙ WATER METER	(CALC.) AS PER CALCULATIONS
⊙ UTILITY BOX	(FIELD) AS PER FIELD MEASUREMENTS
* WELL	P.R.M. PERMANENT REFERENCE MARKER
⊙ SANITARY MANHOLE	P.C.P. PERMANENT CONTROL POINT
+ SIGN POST	

LOT 6

SEE "A"

LOT 4

L.S. BRITT
P.L.S. 5/757

SPINE SET IN
30" HICKORY TREE
ELEVATION = 28.32'

DIRT DRIVE

LOT 5
1.44 Acres, ±

500.00' (PLAT)
500.07' (FIELD)

S.73°04'00"E 684.49' (FIELD)
630.00' (PLAT)

N.73°04'00"W
N.72°59'36"W

500.00' (PLAT)
499.59' (FIELD)

SPINE SET IN
24" HICKORY TREE
ELEVATION = 35.73'

N.35°57'28"E 80.43' (FIELD)
N.35°57'28"E 80.43' (FIELD)

N.35°57'48"E 100.40' (FIELD)
N.35°59'00"E 100.00' (PLAT)

SW RIVERSIDE AVENUE
(DIRTY/PUBLIC)

PLATTED
RIGHT-OF-WAY

500.00'

FLOOD INFO AS PER SRWMD
100 YEAR = 34.4'
10 YEAR = 28.5'
2 YEAR = 22.7'
(NAVD 88 DATUM)



DAVID & BRYAN MORGAN

CERTIFIED TO:

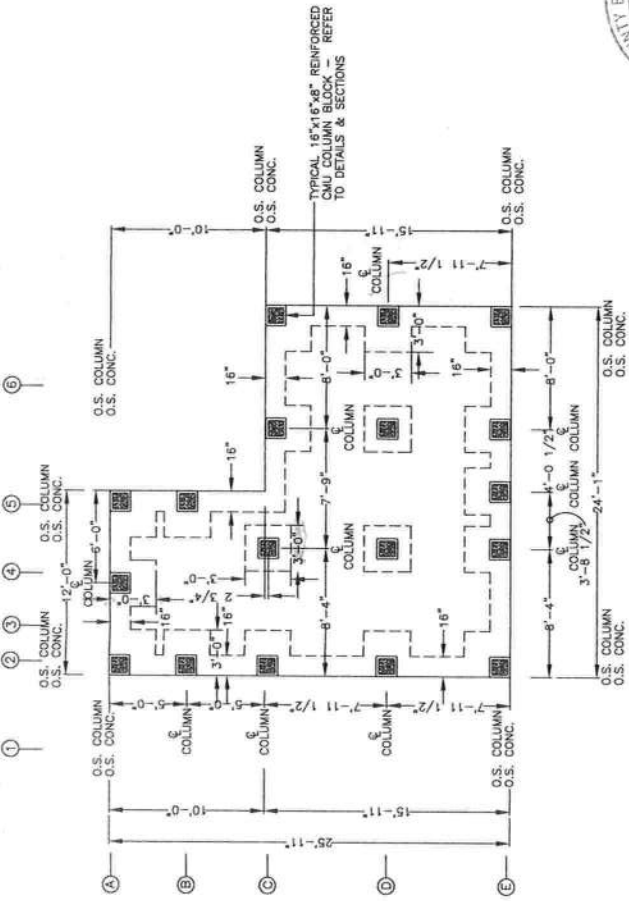
FIELD BOOK, SEE PAGE(S) FILE

I HEREBY CERTIFY THAT THIS SURVEY WAS MADE UNDER MY PERSONAL SUPERVISION AND THAT THE SURVEY
TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS
IN CHAPTER 120, F.S. HAVE BEEN STRICTLY ADHERED TO IN THE CONDUCT OF THIS SURVEY.
10/29/15 11/05/15
FIELD SURVEY DATE SURVEY DATE
L.S. BRITT, P.L.S.
COLUMBIA COUNTY, FLORIDA
CERTIFICATION # 5753



BRITT SURVEYING
& MAPPING, LLC
LAND SURVEYORS AND MAPPERS, L.L.C. # 8016
2086 SW MAIN BLVD., SUITE 110, LAKE CITY, FLORIDA 32085
(386)752-7153 FAX (386)752-5573
WWW.BRITTSURVEYING.COM
WORK ORDER # L-23730

SCALE NOTE:
PLAN VIEW: 1/8"=1'-0"



2 RELOCATED DWELLING FOUNDATION PLAN VIEW
A2.1.0 SCALE: N.T.S.



CERTIFICATION:
THESE PLANS FOR THE MORGAN RESIDENCE WILL COMPLY WITH SECTION 1809 OF THE FLORIDA BUILDING CODE, 2014 EDITION, FOR A 120 MPH WIND LOAD, 3 SECOND GUST, EXPOSURE C, WITH THE INTERNAL PRESSURE OF + 0.18 AND - 0.18 INCLUDED IN THESE LOADS.

COMPONENTS/CLADDING ROOF = - 22.17 PSF
+ 11.86 PSF
COMPONENTS/CLADDING ROOF = - 51.82 PSF
OVERHANG + 11.86 PSF
COMPONENTS/CLADDING/WALLS = - 33.32 PSF
+ 25.22 PSF

Curtis E. Keen 6/24/16
CURTIS E. KEEN, PE #23836

MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA

KEEN ENGINEERING
& SURVEYING, INC.

9263 CR 417
LIVE OAK, FLORIDA 32060
386-362-4787
ENG. LIC. EB 3761

Curtis E. Keen, PE #23836
Certification of Authorization #3761
DATE:

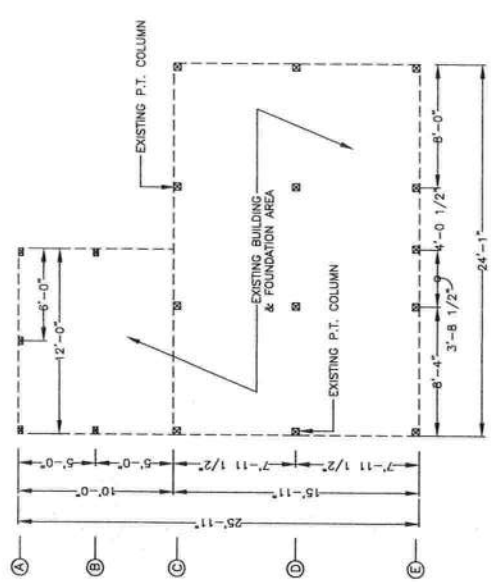
EXISTING & RELOCATED BUILDING FOUNDATION PLAN VIEWS
MISC. NOTES, REFERENCES & INSTRUCTIONS
© 2016 KEEN ENGINEERING & SURVEYING, INC.

PROJECT No.
MORGAN-A2.1.0
SHEET No.
A2.1.0

DATE
02/26/16

REFER TO SHEET No. S1.1.0 FOR CMU COLUMN & FOUNDATION REQUIREMENTS.
NEW COLUMN FINISH FLOOR HEIGHTS OF RELOCATED BUILDING SHALL MATCH EXISTING FLOOR CONSTRUCTION.
A P.T. 2x4 SHALL BE ATTACHED TO THE TOP OF THE CMU PER/ COLUMN W/ 1" A307 ANCHOR BOLT.
EACH REINFORCED CMU COLUMN SHALL BE ATTACHED TO DOUBLE 2x6 BAND JOISTS OR INTERIOR JOISTS W/ 1" MODEL No. PAS1 BY SIMPSON STRONG-TIE OR EQUAL W/ 9"-16d COMMON NAILS AS SPECIFIED BY MANUFACTURER.
DETAILS FOR NEW CONSTRUCTION ARE SIMILAR TO THOSE REQUIRED FOR THIS SPECIFIC RELOCATION.

1 EXISTING DWELLING FOUNDATION PLAN VIEW
A2.1.0 SCALE: N.T.S.



SCALE NOTE:
ELEVATIONS: 1/8"=1'-0"

26 GA. GALV. STEEL ROOFING PANELS - REFER TO REQUIREMENTS

REFER TO ROOF SYSTEM FOR DETAILS, SECTIONS & SPECIFIC REQUIREMENTS

MANUFACTURED ROOF TRUSSES - SEE ROOF SYSTEM

SEE ROOF SYSTEM FOR MECHANICAL DEVICE REQUIRED

VINYL FASCIA SYSTEM OR OTHER OWNER APPROVED MATERIAL/SYSTEMS
INSTALLED PER SELECTED MANUFACTURER'S REQUIREMENTS FOR
THIS SPECIFIC APPLICATION

EXTERIOR FINISH MATERIALS TO BE INSTALLED IN ACCORDANCE WITH THE
MANUFACTURER'S RECOMMENDATIONS FOR THIS APPLICATION

TRIM @ THE DIRECTION OF OWNER

CONDITIONED AREA 1ST LEVEL	1064.00 S.F.
CONDITIONED AREA 2ND LEVEL	1064.00 S.F.
TOTAL CONDITIONED AREA	2128.00 S.F.
PORCH AREA(S)	448.00 S.F.
WALK AREA(S)	40.00 S.F.
SUB LEVEL STAIR & STORAGE	199.50 S.F.
SUB LEVEL PARKING	1110.70 S.F.
TOTAL UNCONDITIONED AREAS	1110.70 S.F.
TOTAL CONSTRUCTION AREA	3238.70 S.F.

FLOOD DATA
100 YR 34.4
10 YR 29.5
NAVD 89 DATUM
EXISTING GRADE = 24.5

1 2 3 4 5 6



TOP OF COLUMN = 35.4 MIN.

1 EXTERIOR FINISH SIDEWALL ELEVATION
A3.0.0/ SCALE 1/8" = 1'-0"

C
D
E

5 1/2:12

SEE GUARDRAIL, STAIR AND LANDING REQUIREMENTS

TOP OF COLUMN = 35.4 MIN.

REINFORCED CMU COLUMN BLOCK - SEE FOUNDATION REQUIREMENTS

AVERAGE GRADE = 24.5

2 EXTERIOR FINISH ENDWALL ELEVATION
A3.0.0/ SCALE 1/8" = 1'-0"

ENCLOSURE NOTE
THE ABOVE ENCLOSED AREA OR UNFINISHED ENCLOSURE IS DESIGNED WITH THE SOLE INTENT OR PURPOSE TO
WITHSTAND EXPOSURE TO THE ELEMENTS, HAVE A LOW FLOOD DAMAGE POTENTIAL, AND PROVIDE LIMITED STORAGE
(VS. GRADE) EQUIPMENT, BICYCLES AND OTHER LOW FLOOD DAMAGE POTENTIAL ITEMS. THIS AREA IS NOT
DESIGNED TO BE AhabITABLE AND IS NOT TO BE USED FOR ANY OTHER PURPOSE. FURTHER, THIS AREA IS
DESIGNED TO BE NON-PARTITIONED AND VOID OF ALL UTILITIES (EXCEPT
ESSENTIAL AND NECESSARY UTILITIES) INCLUDING, BUT NOT LIMITED TO, GAS, ELECTRIC, AND WATER. THIS
AREA SHALL NOT BE TEMPERATURE CONTROLLED.

9263 CR 417
LIVE OAK, FLORIDA 32080
386-362-4787
ENG. LIC. EB 3761

MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA

Curtis E. Keen, PE #23836
Certification of Authorization #3761
DATE:

PROJECT No.
MORGAN-A3.0.0/

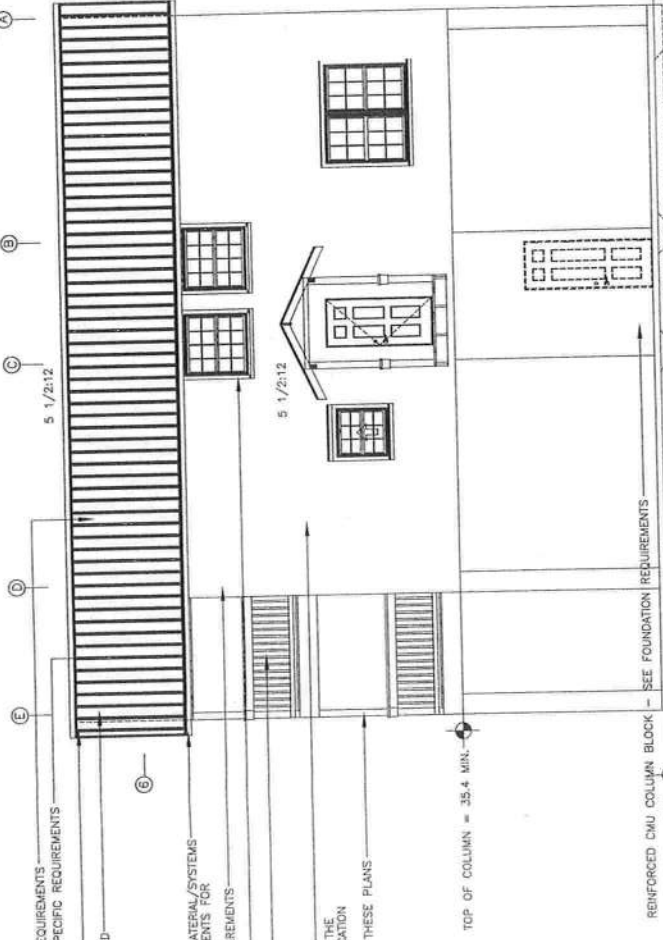
DATE
02/26/16

EXTERIOR FINISH ELEVATIONS
MSC. NOTES, REFERENCES & INSTRUCTIONS

SHEET No.
A3.0.0

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SCALE NOTE:
ELEVATIONS: 1/8" = 1'-0"



1 EXTERIOR FINISH SIDEWALL ELEVATION

A3.1.0 SCALE: 1/8" = 1'-0"

GABLE END FINISH MATERIAL: BOARD & BATTEN BY "JAMES HARDIE PRODUCTS", INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS FOR THIS APPLICATION

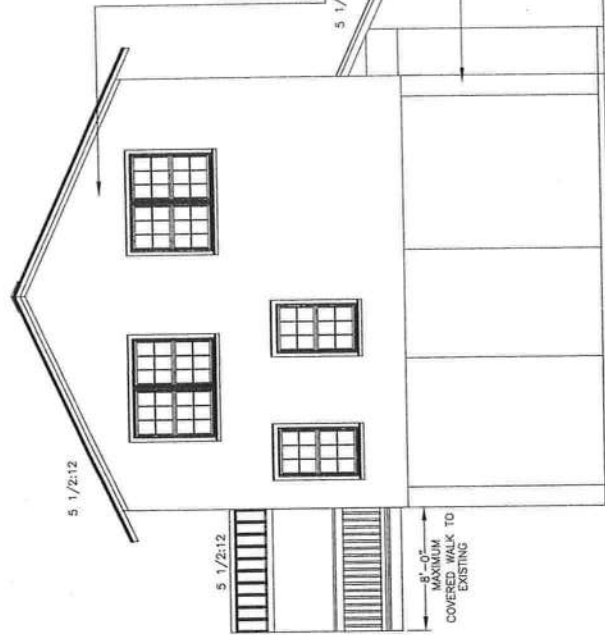
TOP OF COLUMN = 35.4 MIN.

REINFORCED CMU COLUMN BLOCK - SEE FOUNDATION REQUIREMENTS

AVERAGE GRADE = 24.5

CONDITIONED AREA 1ST LEVEL	1064.00 S.F.
CONDITIONED AREA 2ND LEVEL	1064.00 S.F.
TOTAL CONDITIONED AREA	2128.00 S.F.
PORCH AREA(S)	448.00 S.F.
WALK AREA(S)	40.00 S.F.
SUB LEVEL STAIR & STORAGE	199.50 S.F.
SUB LEVEL PARKING	199.50 S.F.
TOTAL UNCONDITIONED AREAS	1110.70 S.F.
TOTAL CONSTRUCTION AREA	3238.70 S.F.

5 1/2:12



2 EXTERIOR FINISH ENDWALL ELEVATION

A3.0.0 SCALE: 1/8" = 1'-0"

EXTERIOR FINISH ELEVATIONS
MISC. NOTES, REFERENCES & INSTRUCTIONS

PROJECT No.
MORGAN-A3.0.0.DWG
SHEET No.
A3.1.0

DATE
02/26/16

Curtis E. Keen, P.E. #23838
Certification of Authorization #3761
DATE:

MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA

KEEN ENGINEERING
& SURVEYING, INC.
9263 CR 417
OAK, FLORIDA 32060
386-182-4787
ENG. LIC. EB 3761

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ADDITIONAL WINDOW NOTES

REFER TO PLAN VIEWS & ELEVATIONS FOR SPECIFIC DESIGNATIONS FOR REQUIRED EGRESS WINDOWS

GRADE FULLY COMPLIANT WINDOW GUARDS ARE REQUIRED
7'-0" ABOVE FINISH FLOOR AND OVER
FINISH FLOOR AND OVER 24" ABOVE FINISH FLOOR AND OVER 7'-0" ABOVE ADJACENT FINISH GRADE FULLY COMPLIANT WINDOW GUARDS ARE REQUIRED

INSTALLATION REQUIREMENTS
FOLLOW FACTORY'S INSTALLATION REQUIREMENTS



MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA

Curtis E. Keen, PE #23836
Certification of Authorization
DATE:

WINDOW & DOOR SCHEDULES
MISC. NOTES, REFERENCES & INSTRUCTIONS

PROJECT No. MORGAN-45.0.0.DWG	DATE 2/26/16
SHEET No. 1	

[illegible]

E	D	C	B	A
INTERIOR - INSUL.	INTERIOR - NON-INSUL.	EXTERIOR - DASHED LINE OPTION - EXTERIOR - INSUL.	EXTERIOR - DASHED LINE OPTION - EXTERIOR - NON-INSUL.	EXTERIOR - INSUL.
3'-0"	3'-0"	3'-0"	3'-0"	3'-0"

ADDITIONAL WINDOW NOTES

REFER TO PLAN VIEWS & ELEVATIONS FOR SPECIFIC DESIGNATIONS FOR REQUIRED EGRESS WINDOWS

GRADE FULLY COMPLIANT WINDOW GUARDS ARE REQUIRED OVER ALL FINISH FLOORS AND OVER 7'-0" ABOVE ADJACENT FINISH GRADE. FULLY COMPLIANT WINDOW GUARDS ARE NOT REQUIRED WHERE INSTALLATION REQUIREMENTS ARE LESS THAN 24".

ADDITIONAL WINDOW NOTES

REFER TO PLAN VIEWS & ELEVATIONS FOR SPECIFIC DESIGNATIONS FOR REQUIRED EGRESS WINDOWS

GRADE FULLY COMPLIANT WINDOW GUARDS ARE REQUIRED 7'-0" ABOVE FINISH FLOOR AND OVER 7'-0" ABOVE ADJACENT FINISH GRADE FULLY COMPLIANT WINDOW GUARDS ARE REQUIRED 6'-0" ABOVE FINISH FLOOR AND OVER 6'-0" ABOVE ADJACENT FINISH GRADE.

FULLY FACTORED INSTALLATION REQUIREMENTS

WINDOWS AND DOORS

ALL WINDOWS OR DOORS SERIES FROM SELECTED MANUFACTURERS SHALL COMPLY WITH FBC 502.3.2.6. SHALL BE CALKED, GASKETED, WEATHER STRIPPED OR OTHER ALTERNATIVE METHOD. SMALL COMPLY WITH FBC 502.3.3

ALL DOORS SHALL COMPLY WITH SCHED ALONG WITH MANUFACTURER'S INSTALLATION PRODUCT APPROVAL. SPECIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL. REQUIREMENTS FBC 1711.5.5 SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO INSTALLATION.

1. ALL GLAZING WITHIN 24 INCHES ARC OF ALL DOORS, WHOSE BOTTOM EDGE IS LESS THAN 24 INCHES ABOVE FINISH FLOOR, ALL GLAZING IN DOORS SHALL BE GLASS, TEMPERED OR ACRYLIC PLASTIC SHEET.

2. MINIMUM FINISH CORRIDOR WIDTH SHALL BE AS NOTED AND SHALL NOT BE REDUCED OR ALTERED BY CONTRACTOR.

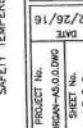
3. ALL WINDOWS, GLASS AND DOORS SHALL COMPLY W/ AANA - NW504 101-1.5.2

4. ALL WINDOWS USED IN CONSTRUCTION OF THIS BUILDING WHICH ARE COVERED BY THE FLORIDA BUILDING COMMISSION CHAPTER 61 020-3.006 RULES SHALL HAVE CURRENT FLORIDA APPROVAL.

5. IF WINDOW SILLS ARE LESS THAN 24 INCHES ABOVE FINISH FLOOR & OVER 7 FEET ABOVE ADJACENT FINISH GRADE FULLY COMPLIANT WINDOW GUARDS SHALL BE REQUIRED.

6. RETENTION OF TREE CANOPIES & RELATED SCHEDULES FOR FULLY COMPLIANT WINDOW & DOOR SIZES AND LOCATIONS

2. MINIMUM FINISH CORRIGED WIDTH SHALL BE AS NOTED AND SHALL NOT BE REDUCED OR INCREASED BY CONTRACTOR.
3. ALL WINDOWS & GLASS AND DOORS SHALL COMPLY W/ AAMA - NW101-1.5.2 WHICH PROVIDES THE MINIMUM REQUIREMENTS FOR WEATHER RESISTANCE.
4. ALL WINDOWS USED IN CONSTRUCTION OF THIS BUILDING WHICH ARE COVERED BY THE FLORIDA BUILDING COMMISSION CHAPTER 61 620-3.006 RULES SHALL HAVE CURRENT FLORIDA APPROVAL.
5. IF WINDOW SILLS ARE LESS THAN 24 INCHES ABOVE FINISH FLOOR & OVER 7 FEET ABOVE ADJACENT FINISH GRADE FULL COMPLIANT WINDOW GUARDS SHALL BE REQUIRED. AIR VENTS & RELATED SCHEDULES FOR FULLY COMPLIANT WINDOWS & DOORS SHALL BE PROVIDED. PROVIDE SIZES AND LOCATIONS.



SCALE NOTE:
PLAN VIEW: 1/8"=1'-0"

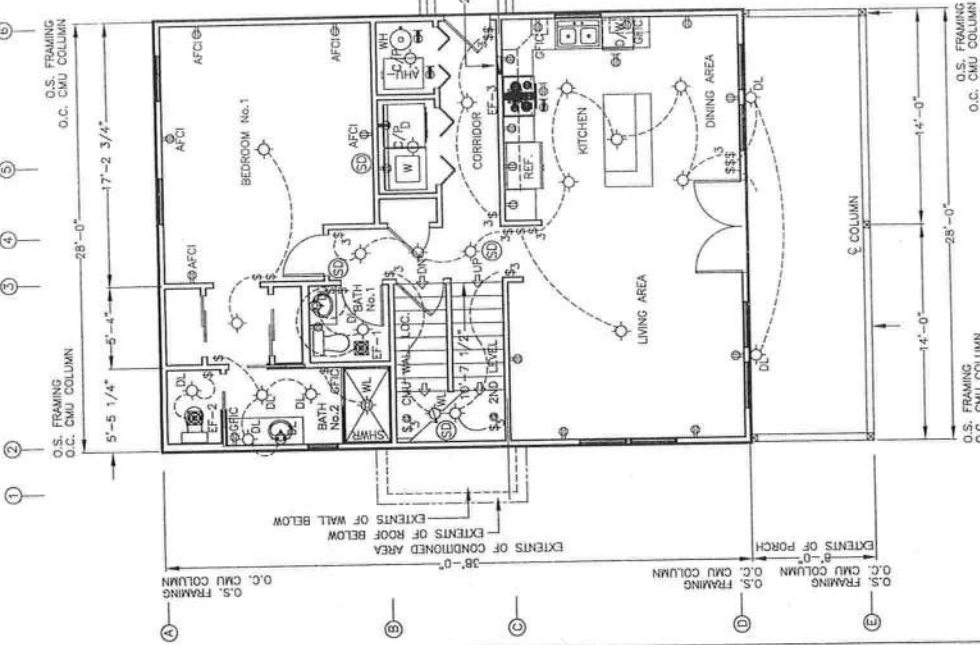
ELECTRICAL FIXTURE SYMBOLS

- FLUORESCENT STRIP FIXTURE, SURFACE MOUNTED
- FLUORESCENT, SURFACE MOUNTED
- OWNER SELECTED FIXTURE - CONTRACTOR INSTALLED
- DROP MOUNTED FIXTURE
- WALL BRACKET MOUNTED INCANDESCENT OR HID FIXTURE
- SURFACE MOUNTED FIXTURE
- RECESSED DOWNLIGHT
- RECESSED DOWNLIGHT OR SURFACE MOUNTED FIXTURE - DAMP LOCATION RATED
- RECESSED DOWNLIGHT OR SURFACE MOUNTED FIXTURE - WET LOCATION RATED
- RECESSED DOWNLIGHT OR SURFACE MOUNTED FIXTURE - FLUORESCENT LAMP
- EXTERIOR FLOOD LIGHTING: LOCATIONS & SWITCHES @ OWNER'S DISCRETION
- GROUND MOUNTED FLOOD OR DOWNLIGHT LOW VOLTAGE SYSTEM LOCATED @ THE DISCRETION OF THE OWNER
- ON DEMAND WATER HEATER: MFR. SIZE, ETC. @ DIR. OF OWNER
- DUPLEX RECEPTACLE, LOCATED @ ALL COUNTER AREAS SHALL BE 43" A.F.F.
- DUPLEX RECEPTACLE, WALL MOUNTED 12" A.F.F. WEATHERPROOF BOX/CONNECTION
- DUPLEX RECEPTACLE W/ GROUND FAULT CIRCUIT INTERRUPTER, WALL MOUNTED
- DUPLEX RECEPTACLE W/ GROUND FAULT CIRCUIT INTERRUPTER, WALL MOUNTED
- EXACT LOCATION AS DETERMINED BY OWNER @ TIME OF INSTALLATION
- 220 RECEPTACLE OR CONNECTION - SEE PLAN VIEWS(S) FOR LOCATIONS
- TELEPHONE OUTLET, WALL MOUNTED @ OWNER'S DISCRETION
- TELEPHONE DATA OUTLET, WALL MOUNTED 18" A.F.F. UNLESS OTHERWISE INDICATED
- NOTE: LOCATE @ OWNER'S DISCRETION
- \$ SINGLE POLE DIMMER SWITCH MOUNTED 48" A.F.F.
- \$ D SINGLE POLE DIMMER SWITCH MOUNTED 48" A.F.F.
- \$ M SINGLE POLE MOTOR RATED TOGGLE SWITCH
- FLUSH MOUNTED, 200A, 240/120 V - @ ELECTRICAL PANEL - TOP @ 6'-6" A.F.F.
- DISCONNECT SWITCH: SIZE AS NOTED ADJACENT TO SYMBOL
- OWNER SELECTED CEILING FAN W/ OPTIONAL LIGHT KIT BY MANUFACTURER
- OWNER SELECTED MANUFACTURER & MODEL - SIZE AS NOTED ON PLANS
- FAN AND OR LIGHT MAY BE SWITCHED - REFER TO PLAN VIEWS(S)
- EF-1 EXHAUST FAN: OWNER APPROVED PROVIDING A MIN. 40 CFM
- EF-2 EXHAUST FAN: OWNER APPROVED PROVIDING A MIN. 30 CFM
- EF-3 EXHAUST FAN: BY OWNER APPROVED STOVE MANUFACTURER
- CAT 5 WIRE @ OWNER SELECTED LOCATIONS
- SPEAKERS & RELATED SOUND DEVICES - LOCATE @ OWNER'S DISCRETION
- TELEVISION COAXIAL OUTLET SHALL BE LOCATED 18" A.F.F.
- NOTE: SEE PLANS FOR LOCATE @ OWNER'S DISCRETION
- SMOKE DETECTOR: CEILING MOUNTED UNLESS OTHERWISE NOTED
- ALL SMOKE DETECTORS SHALL BE 120 VOLT WITH BATTERY BACKUP. OF THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER, INSTALL INSIDE AND NEAR ALL BEDROOMS ON SEPARATE CIRCUIT TO THE MAIN DISTRIBUTION PANEL. ADDITIONAL LOCATIONS SHALL BE DETERMINED BY THE FIRE MARSHALL. REFER TO PLAN VIEWS FOR LOCATIONS

ELECTRICAL NOTES

1. ALL WORK SHALL BE FULLY COMPLIANT WITH THE MOST CURRENT EDITION OF THE NATIONAL ELECTRICAL CODE (NEC).
2. ALL CIRCUITS & EQUIPMENT SHALL BE GROUNDED IN ACCORDANCE WITH THE APPROPRIATE ARTICLES OF THE NEC.
3. WHEN LIGHT FIXTURES ARE INSTALLED IN CLOSETS THEY SHALL BE SURFACE MOUNTED. WHEN LIGHT FIXTURES ARE INSTALLED IN CLOSETS THEY SHALL HAVE COMPLETELY ENCLOSED LAMPS. SURFACE MOUNTED INCANDESCENT FIXTURES SHALL HAVE A MINIMUM CLEARANCE OF 12 INCHES AND ALL OTHER FIXTURES SHALL HAVE A MINIMUM CLEARANCE OF 6 INCHES FROM STORAGE AREAS AS DEFINED BY NEC 410.16(D).
4. ACCESSIBLE DISCONNECTS ADJACENT TO THE WATER HEATER SHALL BE LOCATED WITHIN SIGHT FROM THE WATER HEATER OR IS CAPABLE OF BEING LOCKED.
5. HVAC EQUIPMENT SHALL BE PROVIDED WITH READILY ACCESSIBLE DISCONNECTS ADJACENT TO THE EQUIPMENT. A UNIT SWITCH WITH ALL UNGROUNDED CONDUCTORS SHALL BE PROVIDED AS THE DISCONNECTING MEANS WHERE BREAKERS ARE ALSO PROVIDED BY A READILY ACCESSIBLE CIRCUIT BREAKER.
6. PRIOR TO ENERGIZING THE ELECTRICAL SYSTEM THE INTERRUPTING RATING OF THE MAIN BREAKER MUST BE VERIFIED BY THE ELECTRICAL CONTRACTOR.
7. THE MAIN ELECTRICAL PANEL AND FEEDERS IF NOT SPECIFICALLY DEFINED IN THESE PLANS ARE DESIGNED BY OTHERS.
8. NOT USED.
9. ALL RECEPTACLES INSTALLED IN WET LOCATIONS (EXTERIOR) SHALL BE IN WEATHER RESISTANT ENCLOSURES. THE INTEGRITY OF WHICH IS NOT AFFECTED WHEN AN ATTACHMENT PLUG CAP IS INSERTED OR REMOVED. THE RECEPTACLE ITSELF SHALL ALSO BE LISTED FOR USE IN DAMP AND WET LOCATIONS AS PER MOST CURRENT EDITION OF THE NEC.
10. EXPOSED ELECTRICAL SERVICE CONDUCTORS SHALL BE CONNECTED TO A FIBERGLASS OR THERMOPLASTIC SHEATHING FOR 24 HOUR USE SHALL BE CONNECTED TO A FIBERGLASS OR THERMOPLASTIC SHEATHING.
11. ALL EXPOSED ELECTRICAL SERVICE CONDUCTORS SHALL BE FULLY ENCLOSED IN APPROPRIATELY SIZED NON METALLIC CONDUIT (NMT) PER THE LATEST EDITION OF THE NEC.
12. RELOCATING SWITCHES NOT INDICATED BY LOCATION ON THE PLAN VIEWS(S) SHALL BE LOCATED @ THE DISCRETION OF THE OWNER IN FULL COMPLIANCE WITH ALL APPLICABLE SECTIONS OF THE NEC.
13. ALL SMOKE DETECTORS SHALL BE 120 VOLT (120 V) WITH BATTERY BACKUP. OF THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER. INSTALLATION INSIDE AND NEAR ALL BEDROOMS ON A SEPARATE CIRCUIT TO THE MAIN DISTRIBUTION PANEL. REFER TO PLAN VIEW FOR SUGGESTED LOCATIONS WITH ALL ADJUSTMENTS AND ADDITIONAL LOCATIONS DETERMINED BY LOCAL FIRE MARSHALL.
14. PROVIDE COMBINATION SMOKE/CARBON MONOXIDE DETECTORS WHEN ANY FOSSIL FUEL APPLIANCES ARE INSTALLED. DETECTORS SHALL REFER TO SPECIFIC LOCATIONS AND NOTES ON THE PLAN VIEWS(S) FOR LOCATIONS.
15. RELOCATING SWITCHES NOT INDICATED BY LOCATION ON THE PLAN VIEWS(S) SHALL BE LOCATED @ THE DISCRETION OF THE OWNER IN FULL COMPLIANCE WITH ALL APPLICABLE SECTIONS OF THE NEC.
16. ELECTRICAL CONTRACTOR SHALL REFER TO SPECIFIC LOCATIONS AND NOTES ON THE PLAN VIEWS(S) FOR LOCATIONS.
17. PROVIDE TAMPER RESISTANT RECEPTACLES IN ACCORDANCE WITH SECTION 408.11 OF 2014 NEC.
18. REFER TO ELECTRICAL PLANS, DETAILS, SECTIONS, NOTES AND SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

FIRST LEVEL ELECTRICAL NOTES: ALL LIGHTING, SWITCHING FOR LIGHTING AND RECEPTACLES SHALL BE WET LABLE AND INTERLOCKED TOGETHER. WEATHER PROOF BOXES W/ APPROVED CONNECTIONS WITH GROUND FAULT INTERRUPTER. ALL WIRING SHALL BE INSTALLED IN NMT CONDUIT PER LATEST EDITION OF THE N.E.C.



1 FIRST LEVEL ELECTRICAL PLAN VIEW
E2.0.0 SCALE N.T.S.

KEEN ENGINEERING LIVE
& SURVEYING, INC.
9263 CP 417
OKLAHOMA, FLORIDA 32060
386-162-4787
ENG. LIC. EB 3761

MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA

DATE: 02/26/16
PROJECT NO: MORGAN-E2.0.0.DWG
E2.0.0

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**KEEN ENGINEERING
& SURVEYING, INC.**
LIVE OAK, FLORIDA 32080
9263 CR 417
386-362-4787
ENG. LIC. FB 3761

CLOTHES DRIER

9. ALL RECEPTACLES INSTALLED IN WET LOCATIONS (EXTERIOR) SHALL BE IN WEATHER-PROOF (WP) ENCLOSURES WHICH SHALL NOT BE AFFECTED WHEN AN ARC FLASH EVENT OCCURS. IF CAPS ARE INSERTED OR REMOVED, THE RECEPTACLE ITSELF SHALL ALSO BE PROVIDED FOR USE IN DAMP AND WET LOCATIONS AS PER MOST CURRENT EDITION OF THE NEC.
10. EXTERIOR LIGHTING SHALL BE INTENDED FOR 24 HOUR USE SHALL BE CONNECTED TO A SEPARATE CIRCUIT.
11. ALL EXPOSED ELECTRICAL SERVICE CONDUITS SHALL BE FULLY ENCLOSED IN AN APPROPRIATELY SIZED NON METALIC CONDUIT (NMT) PER THE LATEST EDITION OF THE NEC.
12. ALL SWITCHES NOT INDICATED BY LOCATION ON THE PLAN VIEW(S) SHALL BE LOCATED WITHIN THE DIRECTION OF THE OWNER IN FULL COMPLIANCE WITH ALL APPLICABLE SECTIONS OF THE NEC.
13. ALL SMOKE DETECTOR TYPE AND 120 VOLT (120 V) WITH BATTERY BACKUP OF ALL SMOKE DETECTORS SHALL BE INTERLOCKED TOGETHER. INSTALLATION SHALL BE INSIDE AND NEAR TO ALL BEDROOMS ON A SEPARATE CIRCUIT TO THE MAIN DISTRIBUTION PANEL, REFER TO PLAN VIEW FOR SUGGESTED LOCAL FIRE MARSHALL ADJUSTMENTS AND ADDITIONAL ALARM LOGIC. MONOXIDE DETECTORS WHEN ANY FOSSIL FUEL APPLIANCES ARE PROVIDED OR INSTALLED.
14. ELECTRICAL CONTRACTOR SHALL REFER TO SPECIFIC LOCATIONS AND NOTES ON RECEPTACLES INTERRUPTER PANELS TO IDENTIFY GFCI'S.
15. THE PLAN VIEWS FOR GROUND FAULT INTERRUPTER CIRCUITS (GFI) SWITCHES, RECEPTACLES INTERRUPTER SHALL REFER TO SPECIFIC LOCATIONS AND NOTES ON THE PLAN VIEW(S) FOR ARC FAULT CIRCUIT INTERRUPTER (AFCI) SWITCHES, RECEPTACLES AND RELATED ELEMENTS.
17. PROVIDE TAMPER RESISTANT RECEPTACLES IN ACCORDANCE WITH SECTION 406.11 OF 2017 NATIONAL ELECTRICAL PLANS.
18. FOR ADDITIONAL REQUIREMENTS.

CLOTHES DRYER

- DRIVER EXHAUST DUCTS SHALL CONFORM TO THE FOLLOWING:
- M1502.4.0.6. MATERIAL AND SIZE
 - M1502.4.1. MATERIAL AND SIZE
EXHAUST DUCTS SHALL BE CONSTRUCTED OF METAL OF 0.016 INCH THICK. THE EXHAUST DUCT SIZE SHALL BE 4 INCHES NOMINAL IN DIAMETER.
 - M1502.4.2. DUCT INSTALLATION
EXHAUST DUCTS SHALL BE SUPPORTED AT 4 FOOT INTERVALS AND SECURED IN PLACE. EXHAUST DUCTS SHALL BE SECURED TO THE EXHAUST MANIFOLD. EXHAUST DUCTS SHALL END INTO THE ADJOINING DUCT IN THE DIRECTION OF THE FLOW. DUCTS SHALL NOT BE JOINED WITH SCREENS THAT PROTRUDE INTO THE INSIDE OF THE DUCT.
 - M1502.4.3. TRANSITION DUCT
TRANSITION DUCTS SHALL BE A SINGLE LENGTH THAT IS LISTED AND LABELED IN ACCORD WITH UL 2185A. TRANSITION DUCT SHALL BE A MAXIMUM OF 8 FEET IN LENGTH.
 - M1502.4.4. DUCT LENGTH
THE MAXIMUM ALLOWABLE EXHAUST DUCT LENGTH SHALL BE DETERMINED BY ONE OF THE METHODS SPECIFIED IN SECTION M1502.4.4.1 OR M1502.4.4.2
 - M1502.4.4.1. SPECIFIED LENGTH
THE MAXIMUM LENGTH OF EXHAUST DUCT SHALL BE 25 FEET FROM THE CONNECTION TO THE EXHAUST MANIFOLD TO THE OUTLET TERMINAL. SEE TABLE M1502.4.4.1 FOR THE SPECIFIED LENGTH.
 - M1502.4.4.2. TURNED LENGTH
THE MAXIMUM LENGTH OF EXHAUST DUCT FROM DRYER TO THE OUTLET TERMINAL SHALL BE THE SPECIFIED LENGTH PLUS 4 FEET FOR EACH 90 DEGREE TURN.

3303.5.1 INTAKE OPENINGS
MECHANICAL AND GRAVITY OUTDOOR INTAKE OPENINGS SHALL BE LOCATED A MINIMUM 10 FEET FROM ANY HAZARDOUS OR NOXIOUS CONTAMINATE, SUCH AS VENTS AS CHIMNEYS, EXCEPT AS SPECIFIED IN THIS CODE. WHERE SOURCE OF CONTAMINATE IS LOCATED WITHIN PARKING LOTS AND ALLEYS, STREETS, PLUMBING VENTS, LOADING DOCKS, EXCEPT AS SPECIFIED IN THIS CODE, WHERE SOURCE OF CONTAMINATE IS LOCATED WITHIN 10 FEET OF AN AIR INTAKE OPENING THE OPENINGS SHALL BE LOCATED A MINIMUM OF 2 FEET FROM THE CONTAMINATE SOURCE.

- MECHANICAL NOTES

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1. ALL PLUMBING FIXTURES SHALL HAVE SEPARATE SHUT OFF VALVES.
2. WATER HEATER SHALL HAVE SAFETY PAN WITH 3/4 INCH DRAIN TO EXTERIOR, T&P RELIEF VALVE W/ DRAIN TO EXTERIOR, AND A SHUTOFF VALVE WITHIN 3 FEET ON A POTABLE COLD WATER LINE W/ DRAIN TO EXTERIOR, AND A SHUTOFF VALVE WITHIN 3 FEET ON A POTABLE HOT WATER LINE.

4. REFER TO PLUMBING DRAWINGS FOR ADDITIONAL PIPING INSULATION

CW PIPING & RUNOUTS
HW PIPING (2" OR LESS)

- INSULATION SHALL MEET UL-273 FOR FLAME

ALL INSULATION SHALL BE EQUAL TO OWENINGS CORNING FIBERGLASS ASJ/SSL-II WITH ALL SERVICE JACKETS. ANY PIPING LOCATED IN AREAS SUBJECT TO FREEZING SHALL BE HEAT TAPED WITH DOUBLE THE THICKNESS OF THE ABOVE INSULATION. AREAS CONSIDERED SUBJECT TO FREEZING (THE THICKNESS OF THE ABOVE INSULATION AND NON-HEATED AREAS (EXCLUDING CHASES)).



1 ELECTRICAL SCHEMATIC RISER: 1ST LEVEL
E2.4.0 SCALE: NO SCALE

PROJECT No. MORGAN-EA-0.0/DWG	DATE 02/26/16	ELECTRICAL RISER MISC. NOTES, REFERENCES & INSTRUCTIONS	MORGAN RESIDENCE COLUMBIA COUNTY, FLORIDA	KERN ENGINEERING & SURVEYING, INC. 386-362-4787 ENG. LIC. EB 3761
SHEET No. EA.0.0		© 2016 KERN ENGINEERING & SURVEYING, INC.	Curtis E. Kern, PE #29536 Certification of Authorization #3761	
			DATE:	

SCALE NOTE:
PLAN VIEW: 1/8"=1'-0"

SCHEDULE OF REQUIRED FLOOR SYSTEM MECHANICAL FASTENERS

BAND JOISTS/FLOOR GIRDERS
 LOCATION OF ALL CONDITIONED AREA MULTI-PLY FLOOR GIRDERS - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
 FLOOR GIRDERS AS ILLUSTRATED SHALL BE 2" OR 3" 14"x1.75" GANG LAM LVL BY LOUISIANA PACIFIC OR
 EQUAL W/ A MIN. 2250 FB 1.5E
 P.T. 2X SHALL BE ATTACHED TO THE TOP CMU COLUMN COURSE OR CMU FILLED CELL WALL COURSE BENEATH ALL FLOOR
 FLOOR GIRDERS BEARING W/ AN O.A. ANCHOR BOLT
 LOCATION OF ALL UNCONDITIONED AREA SINGLE PLY FLOOR GIRDERS - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
 FLOOR GIRDERS AS ILLUSTRATED SHALL BE 1" 14"x1.75" GANG LAM LVL BY LOUISIANA PACIFIC OR EQUAL W/ A
 MINIMUM 2250 FB 1.5E
PORCH AREA COLUMNS/PIERS
 LOCATION OF ALL PORCH AREA MULTI-PLY FLOOR GIRDERS - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
 FLOOR GIRDERS AS ILLUSTRATED SHALL BE 2" OR 3" 14"x1.75" GANG LAM LVL BY LOUISIANA PACIFIC OR EQUAL W/ A
 MINIMUM 2250 FB 1.5E
 P.T. 2X SHALL BE ATTACHED TO THE TOP CMU COLUMN COURSE OR CMU FILLED CELL WALL COURSE BENEATH ALL PORCH
 FLOOR GIRDERS BEARING W/ AN O.A. ANCHOR BOLT
 AS NOTED ON THE PLAN VIEWS & DETAILED ELSEWHERE THESE PLANS, ATTACHED TO ORDERS W/ A MINIMUM OF
 9" - 16d COMMON NAILS AS SPECIFIED BY MANUFACTURER
 P.T. 2X SHALL BE ATTACHED TO THE TOP CMU COLUMN COURSE BENEATH ALL FLOOR GIRDERS BEARING W/ 1" - A307
 STANDARD ANCHOR BOLT
 P.T. 2X SHALL BE ATTACHED TO LVL BAND BEAM W/ 1" - MODEL No. U210-3 MAX BY SIMPSON STRONG
 TIE OR EQUAL W/ A TOTAL OF 18" - 16d COMMON NAILS & 10-10d COMMON NAILS PER DEVICE
 REFER TO SPECIAL DETAILS AND REQUIREMENTS FOR ADDITIONAL DEVICES ELSEWHERE THESE PLANS
BAND JOISTS (CONDITIONED AREA)
 LOCATION OF ALL CONDITIONED AREA BAND JOISTS - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
 BAND JOISTS AS ILLUSTRATED SHALL BE 1" OR 2" 14"x1.75" GANG LAM LVL BY LOUISIANA PACIFIC OR EQUAL W/ A
 MINIMUM 2250 FB 1.5E - ATTACHED TO TOP COURSE OF CMU COLUMN COURSE W/ 1" - MODEL No. PAS1 BY SIMPSON
 ATTACHED TO GIRDERS W/ A MINIMUM OF 9" - 16d COMMON NAILS PER DEVICE
 ATTACH LVL BAND JOISTS TO CMU COLUMN COURSE WITH 1" - MODEL No. PAS1 BY SIMPSON STRONG-TIE OR EQUAL W/ A
 TOTAL OF 18" - 16d COMMON NAILS & 10-10d COMMON NAILS PER DEVICE
MANUFACTURED FLOOR TRUSSES
 LOCATION OF ALL FLOOR TRUSSES - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
 FLOOR TRUSSES AS ILLUSTRATED SHALL BE ALPINE OR EQUAL W/ AN O.A. ANCHOR BOLT
 O.C. & AS ILLUSTRATED OR NOTED & ATTACHED TO ORDERS W/ 1" - MODEL No. PAS1 BY SIMPSON STRONG-TIE OR EQUAL W/ A
 TOTAL OF 18" - 16d COMMON NAILS & 10-10d COMMON NAILS PER DEVICE
EXTERIOR WALL STUDS
 LOCATION OF ALL EXTERIOR WALL 2X4 OR 2X6 STUDS - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
 EXTERIOR WALL STUDS SHALL BE 2X4 OR 2X6 O.C. TYPICAL ALONG THE LENGTH OF THE EXTERIOR
 WALLS. STUDS SHALL BE FASTENED TO FOUNDATION WITH 1" - MODEL No. PAS1 BY SIMPSON STRONG-TIE OR EQUAL W/ A
 TOTAL OF 18" - 16d COMMON NAILS & 10-10d COMMON NAILS PER DEVICE
EXTERIOR WALL OPENINGS
 LOCATION OF ALL EXTERIOR WALL OPENINGS - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
 A "HEADER" OF 2" - 2X12 No.2 S.Y.P. W/ 1" - 15/16" O.C. CORE SHALL BE INSTALLED AS NOTED THESE PLANS
 36" WIDE OPENINGS REQUIRE A MINIMUM OF 2 CONTINUOUS STUDS EACH SIDE OF OPENING W/ 1" - MODEL No. H6
 BY SIMPSON STRONG-TIE OR EQUAL W/ A TOTAL OF 18" - 16d COMMON NAILS PER DEVICE
 48" WIDE OPENINGS REQUIRE 3 CONTINUOUS STUDS EACH SIDE OF OPENING W/ 1" - MODEL No. H6 BY SIMPSON
 STRONG-TIE OR EQUAL W/ A TOTAL OF 18" - 16d COMMON NAILS PER DEVICE
 72" WIDE OPENINGS REQUIRE 4 CONTINUOUS STUDS EACH SIDE OF OPENING W/ 2" - MODEL No. H6 BY SIMPSON
 STRONG-TIE OR EQUAL W/ A TOTAL OF 18" - 16d COMMON NAILS PER DEVICE
 REFER TO DETAILS FOR ADDITIONAL REQUIREMENTS
PORCH AREA GIRDERS/BAND BEAMS
 LOCATION OF REAR PORCH GIRDERS/BAND BEAMS - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
 PORCH AREA GIRDERS SHALL BE 2" OR 3" 14"x1.75" GANG LAM LVL BY LOUISIANA PACIFIC OR EQUAL W/ A
 MINIMUM 2250 FB 1.5E
 P.T. 2X SHALL BE ATTACHED TO PIER/COLUMNS W/ 1" - MODEL No. PAS1
 BY SIMPSON STRONG-TIE OR EQUAL W/ A TOTAL OF 18" - 16d COMMON NAILS PER DEVICE
PORCH AREA DECK JOISTS
 LOCATION OF PORCH DECK JOISTS - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
 PORCH AREA DECK JOISTS SHALL BE 1" 2X10 No.2 S.Y.P. @ 16" O.C. & AS ILLUSTRATED ON THE PLAN VIEW AND
 SHALL BE ATTACHED TO GIRDERS/BAND BEAMS WITH A MINIMUM OF 9" - 16d COMMON NAILS PER DEVICE
 NOTE: A P.T. 2X SHALL BE ATTACHED TO EACH JOIST, ORDER OR BAND BEAM. ALTERNATE SHALL BE 3" - #12 SCREWS
 W/ A MINIMUM EMBEDMENT OF 2"

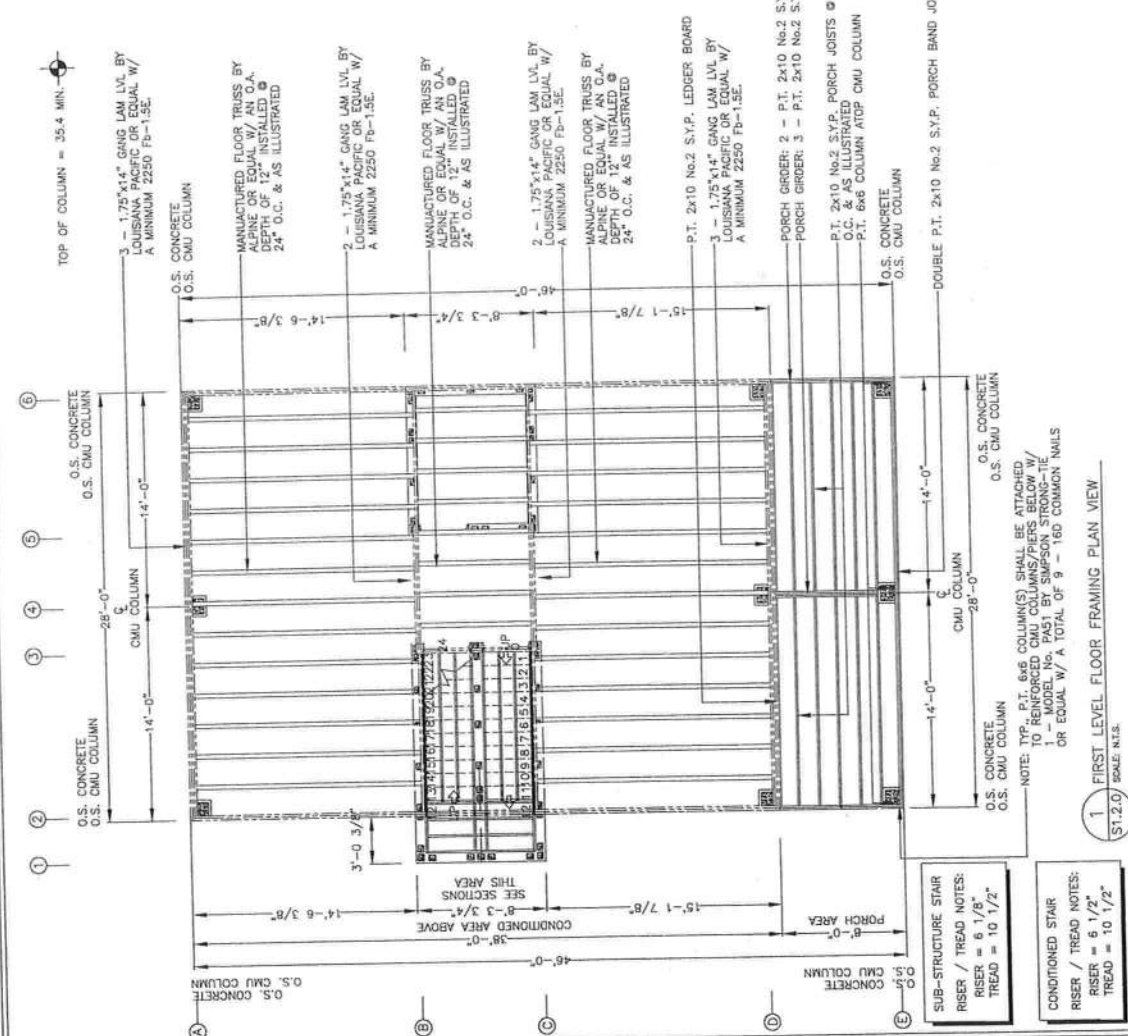
**KEEN ENGINEERING
& SURVEYING, INC.**
 9263 CR 417
 LIVE OAK, FLORIDA 32080
 386-362-4787
 ENG. LIC. EB 5761

**MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA**

Curtis E. Keen, PE #23836
 Certification of Authorization #3761
 DATE

FIRST LEVEL FLOOR FRAMING PLAN VIEW
 MISC. NOTES, REFERENCES & INSTRUCTIONS
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PROJECT No.
 MORGAN-S1.2.0.DWG
 SHEET No.
 S1.2.0



1 FIRST LEVEL FLOOR FRAMING PLAN VIEW
 S1.2.0 SCALE N.T.S.

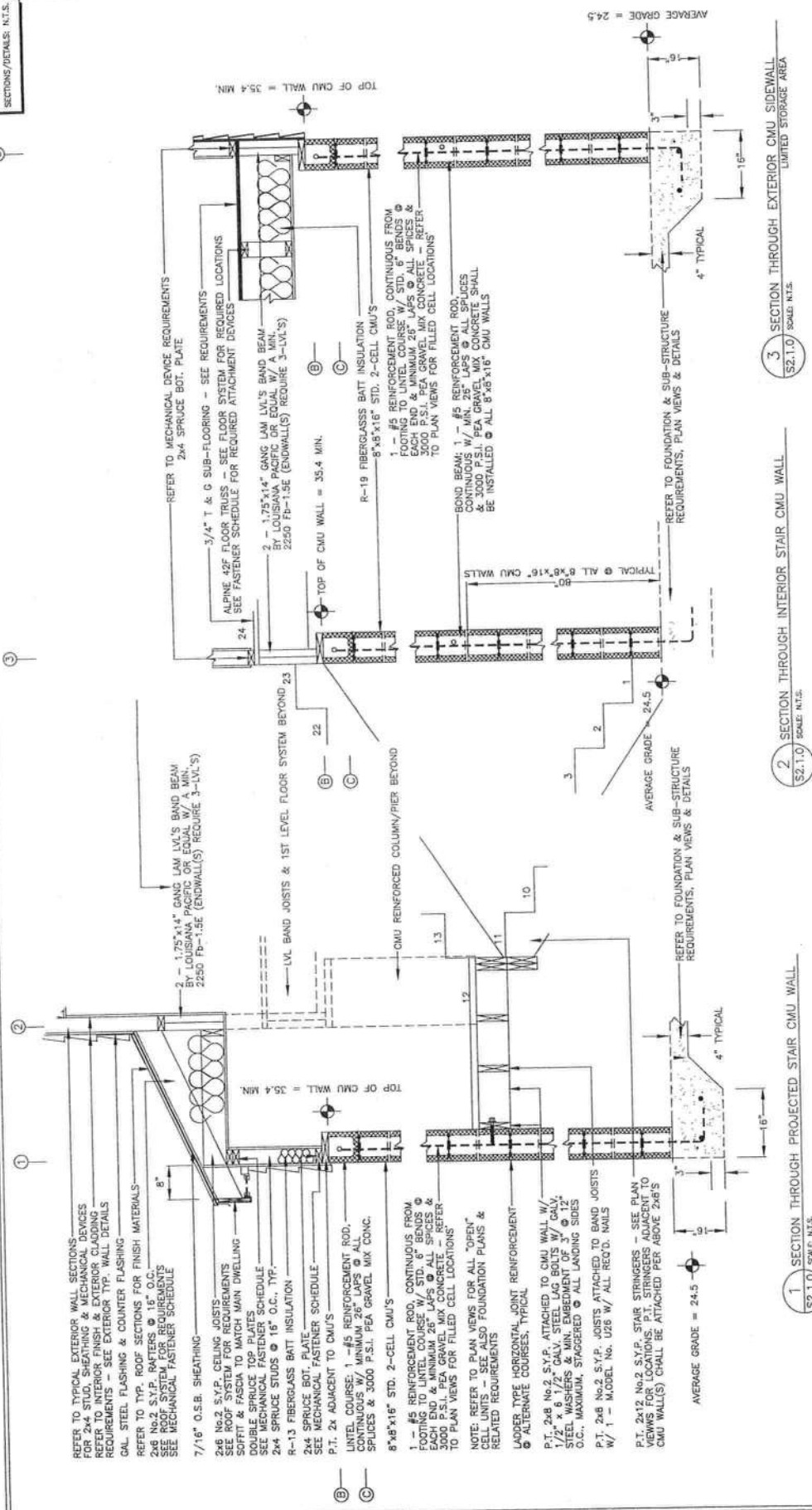
SUB-STRUCTURE STAIR
 RISER / TREAD NOTES:
 RISER = 6 1/8"
 TREAD = 10 1/2"

CONDITIONED STAIR
 RISER / TREAD NOTES:
 RISER = 6 1/2"
 TREAD = 10 1/2"



PROJECT No.	91	REFERENCED EXTERIOR MISC. NOTES, REFERENCE
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SCALE NOTE:
SECTIONS/DETAILS: N.T.S.



PROJECT NO.
WPM-261.0.WPS

DATE
02/26/16

REFERENCED FOUNDATION SECTIONS & DETAILS
MISC. NOTES, FOUNDATIONS & INSTRUCTIONS

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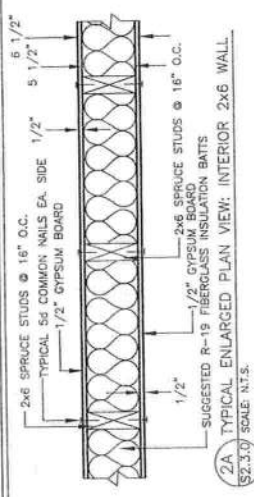
Carl E. Keen, PE #23836
Certification of Authorization #3761
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MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA

KEEN ENGINEERING
& SURVEYING, INC.

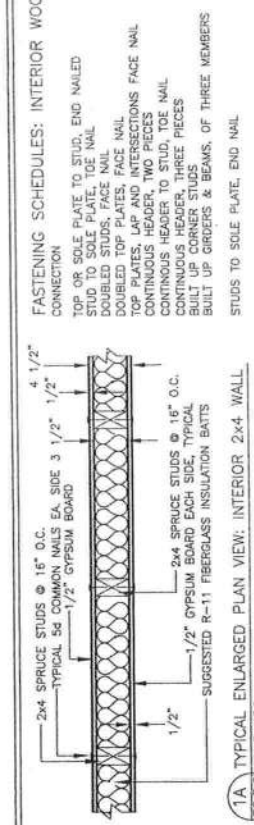
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SCALE NOTE:
SECTIONS/DETAILS: N.T.S.



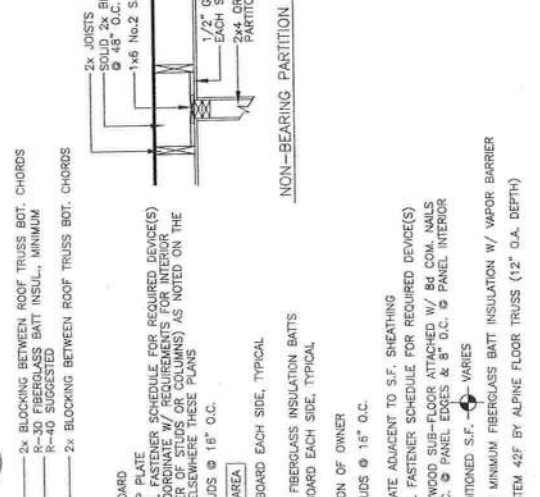
FASTENING SCHEDULES: INTERIOR WOOD FRAMING CONNECTION

FASTENER	NUMBER OR SPACING
16d COMMON	2
16d COMMON	4
16d COMMON	24" O.C.
16d COMMON	18" O.C.
16d COMMON	3-10d COMMON
16d COMMON	16" O.C. ALONG EACH EDGE
16d COMMON	12" O.C. ALONG EACH EDGE
16d COMMON	24" O.C.
20d COMMON	32" O.C. AT TOP & BOTTOM & STAGGERED 2 ENDS & EACH END
16d COMMON	2 EACH END



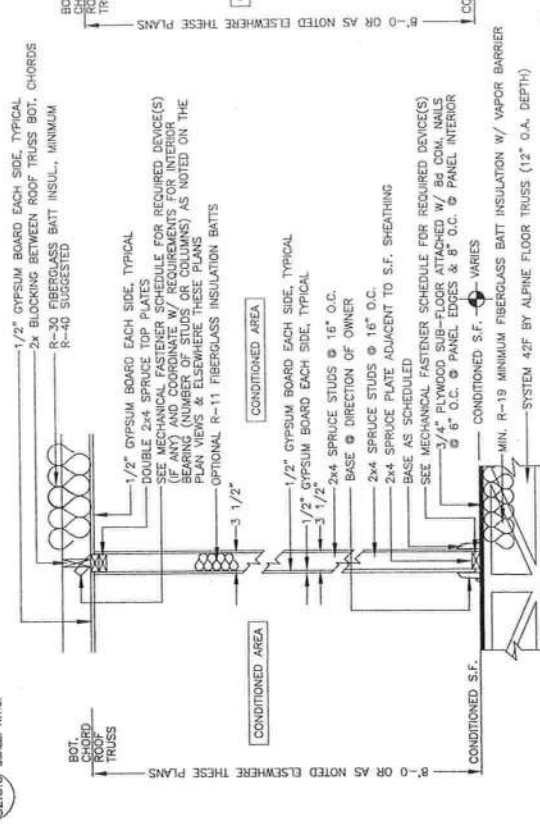
FASTENING SCHEDULES: INTERIOR WOOD FRAMING CONNECTION

FASTENER	NUMBER OR SPACING
16d COMMON	2
16d COMMON	4
16d COMMON	24" O.C.
16d COMMON	18" O.C.
16d COMMON	3-10d COMMON
16d COMMON	16" O.C. ALONG EACH EDGE
16d COMMON	12" O.C. ALONG EACH EDGE
16d COMMON	24" O.C.
20d COMMON	32" O.C. AT TOP & BOTTOM & STAGGERED 2 ENDS & EACH END
16d COMMON	2 EACH END



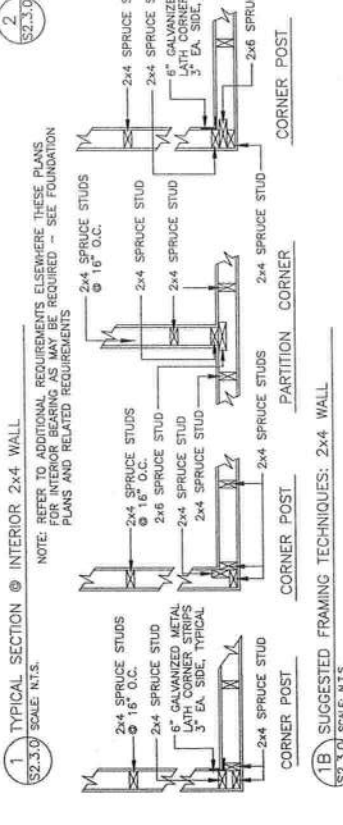
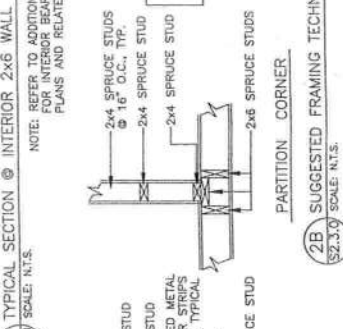
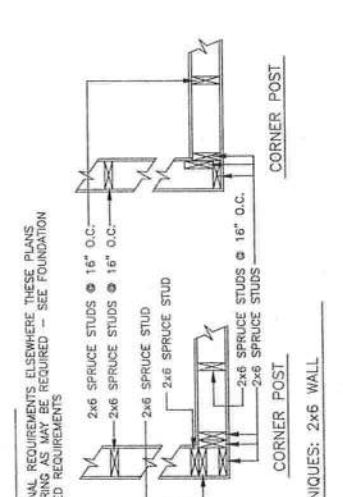
FASTENING SCHEDULES: INTERIOR WOOD FRAMING CONNECTION

FASTENER	NUMBER OR SPACING
16d COMMON	2
16d COMMON	4
16d COMMON	24" O.C.
16d COMMON	18" O.C.
16d COMMON	3-10d COMMON
16d COMMON	16" O.C. ALONG EACH EDGE
16d COMMON	12" O.C. ALONG EACH EDGE
16d COMMON	24" O.C.
20d COMMON	32" O.C. AT TOP & BOTTOM & STAGGERED 2 ENDS & EACH END
16d COMMON	2 EACH END



FASTENING SCHEDULES: INTERIOR WOOD FRAMING CONNECTION

FASTENER	NUMBER OR SPACING
16d COMMON	2
16d COMMON	4
16d COMMON	24" O.C.
16d COMMON	18" O.C.
16d COMMON	3-10d COMMON
16d COMMON	16" O.C. ALONG EACH EDGE
16d COMMON	12" O.C. ALONG EACH EDGE
16d COMMON	24" O.C.
20d COMMON	32" O.C. AT TOP & BOTTOM & STAGGERED 2 ENDS & EACH END
16d COMMON	2 EACH END

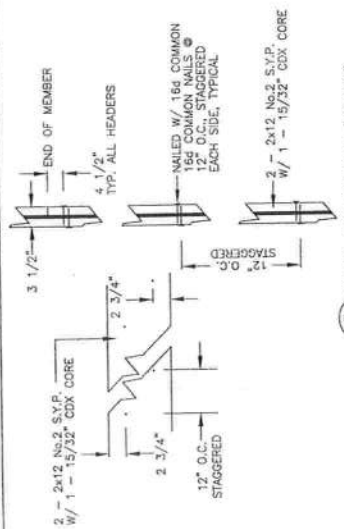


FASTENING SCHEDULES: INTERIOR WOOD FRAMING CONNECTION

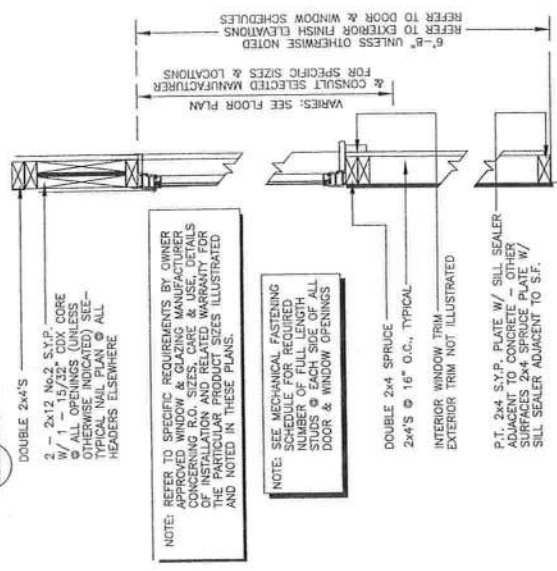
FASTENER	NUMBER OR SPACING
16d COMMON	2
16d COMMON	4
16d COMMON	24" O.C.
16d COMMON	18" O.C.
16d COMMON	3-10d COMMON
16d COMMON	16" O.C. ALONG EACH EDGE
16d COMMON	12" O.C. ALONG EACH EDGE
16d COMMON	24" O.C.
20d COMMON	32" O.C. AT TOP & BOTTOM & STAGGERED 2 ENDS & EACH END
16d COMMON	2 EACH END

PROJECT No. MORGAN-52.3.0.016 SHEET No. 52.3.0	REFERENCED SECTIONS & DETAILS MISC. NOTES, REFERENCES & INSTRUCTIONS DATE: 02/26/16 DATE: 02/26/16	CURTIS E. KEEN, P.E. #23835 CERTIFICATION OF AUTHORIZATION #3761 DATE:	MORGAN RESIDENCE COLUMBIA COUNTY, FLORIDA	KEEN ENGINEERING & SURVEYING, INC. 9263 CR 417 LIVE OAK, FLORIDA 32060 386-362-4787 ENG. LIC. EB 3761
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SCALE NOTE:
SECTIONS/DETAILS: N.T.S.

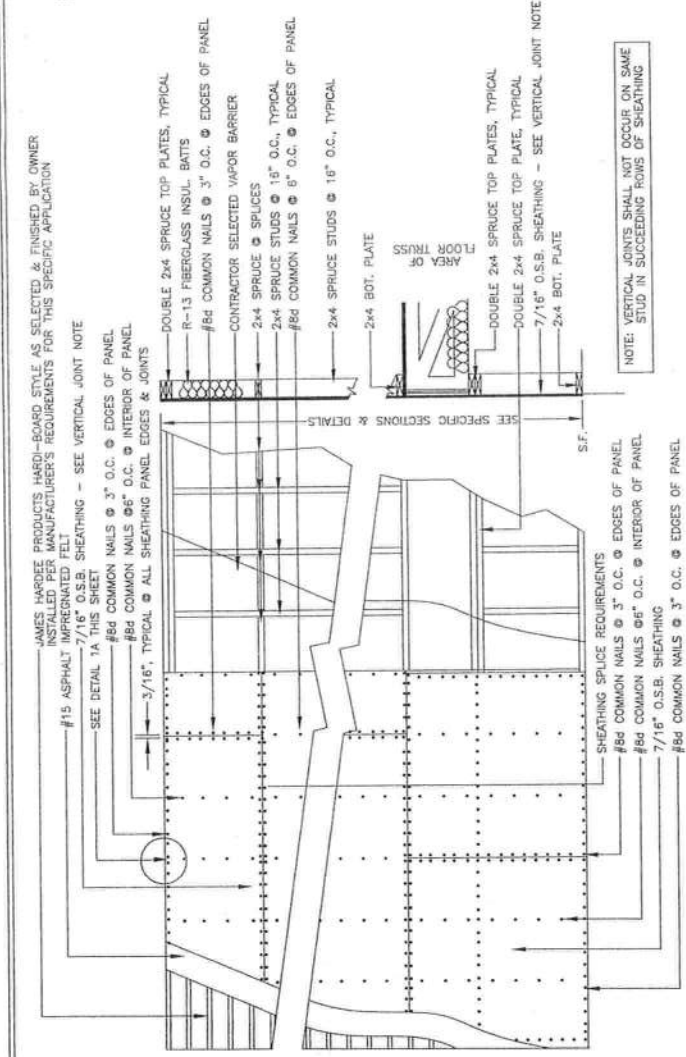


2 TYPICAL EXTERIOR WALL "HEADER"
S2.3.0 SCALE: N.T.S.

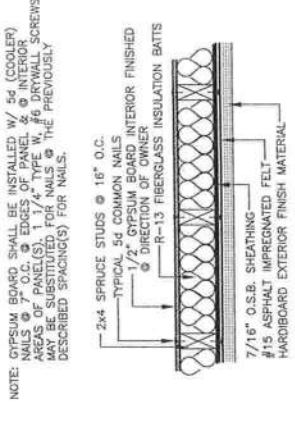


3 TYPICAL SECTION THROUGH EXTERIOR WALL OPENING
S2.1.0 SCALE: N.T.S.

NOTE: CONTRACTOR SHALL PROVIDE ALL APPROVAL SPECIFICATIONS & REQUIREMENTS PER F.B.C. 1715.5 & FULLY COMPLY W/ ALL MANUFACTURER'S REQUIREMENTS FOR OWNER SELECTED MODELS & PROPER INSTALLATION OF WINDOWS & DOORS.



1 SHEATHING DETAIL - ELEVATION VIEW
S2.3.0 SCALE: N.T.S.



1A TOP PLATE DEVICES
S2.2.0 SCALE: N.T.S.

1B TYPICAL ENLARGED PLAN VIEW: EXTERIOR WALL
S2.2.0 SCALE: N.T.S.

PROJECT NO.
MORGAN-S2.4.DWG
SHEET NO.
S2.4.0

91/92/20
3/93

REFERENCES & DETAILS
MISC. NOTES, REFERENCES & INSTRUCTIONS

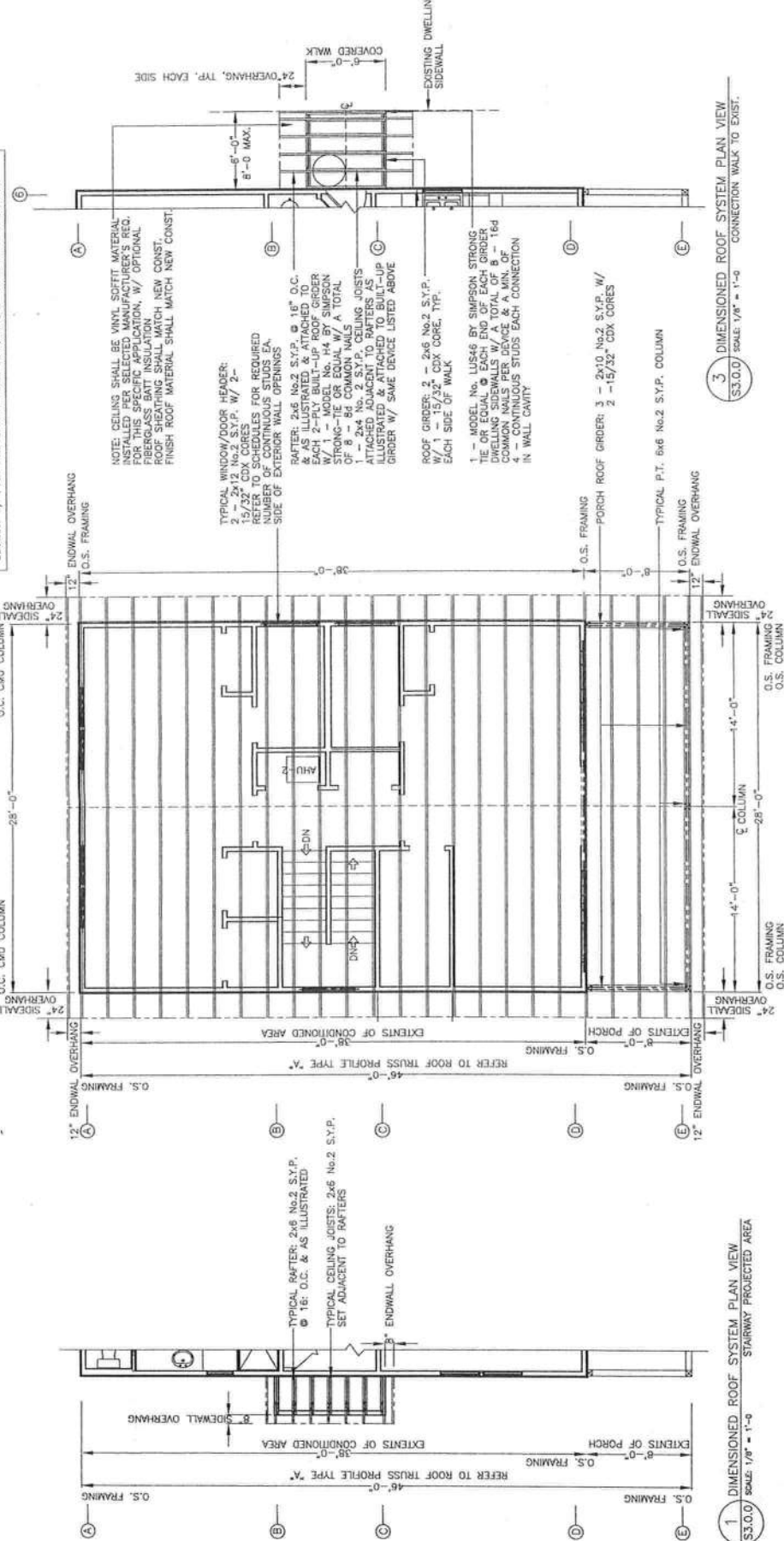
Curtis E. Keen, PE #23836
Certification of Authorization #3761
DATE:

MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA

KEEN ENGINEERING
& SURVEYING, INC.
9263 CR 417
LIVE OAK, FLORIDA 32060
386-362-4787
ENG. LIC. EB 3761

SCALE NOTE:
PLAN VIEW: 1/8"=1'-0"

ATTIC VENTILATION MAIN DWELLING ROOF AREA
THE CONTRACTOR SHALL PROVIDE A MINIMUM OF 10.20 NET SQUARE FEET OF FREE AREA OF COMBINED RIDGE, SOFFIT AND END WALL VENTS COMPLYING W/ SECTION 1203 OF THE F.B.C. SUCH VENTS SHALL BE AS DESCRIBED IN THIS SECTION W/ FULL PROTECTION FROM RAIN, INSECT OR VERMIN INTRUSION.



1 DIMENSIONED ROOF SYSTEM PLAN VIEW
SCALE: 1/8" = 1'-0" STAIRWELL PROJECTED AREA

2 DIMENSIONED ROOF SYSTEM PLAN VIEW
SCALE: 1/8" = 1'-0" MAIN DWELLING ROOF AREA

3 DIMENSIONED ROOF SYSTEM PLAN VIEW
SCALE: 1/8" = 1'-0" CONNECTION WALK TO EXIST.

NOTE: CEILING SHALL BE VINYL SOFFIT MATERIAL
FOR THIS SPECIFIC APPLICATION, W/ OPTIONAL
FIBERGLASS BATT INSULATION, MATCH NEW CONST.
ROOF FINISH SHALL MATCH NEW CONST.
FINISH ROOF MATERIAL SHALL MATCH NEW CONST.

TYPICAL WINDOW/DOOR HEADER:
2 - 2x12 No.2 S.Y.P. W/ 2 -
15/32" CDX CORES FOR REQUIRED
STRENGTH, 16" O.C. & AS ILLUSTRATED
SIDE OF EXTERIOR WALL OPENINGS

BAFFER: 2x6 No.2 S.Y.P. @ 16" O.C.
& AS ILLUSTRATED & ATTACHED TO
EACH 2-PLY BUILT-UP ROOF ORDER
W/ 1 - MODEL No. 14 BY SIMPSON
STRONG-TIE COMMON WALLS
1 - 2x4 No. 2 S.Y.P. CEILING JOISTS
ATTACHED ADJACENT TO BAFFERS AS
ILLUSTRATED & ATTACHED TO BUILT-UP
ORDER W/ SAME DEVICE LISTED ABOVE

ROOF ORDER: 2 - 2x6 No.2 S.Y.P.
W/ 1 - 15/32" CDX CORE, TYP.
1 - MODEL No. LUS46 BY SIMPSON STRONG
TIE OR EQUAL @ EACH END OF EACH ORDER
DWELLING SIDEWALLS W/ A TOTAL OF 8 - 16d
COMMON NAILS PER DEVICE & A MIN. OF
4 - 2x4 CONTINUOUS STUDS EACH CONNECTION
IN WALL CAVITY

ATTIC ACCESS LOCATION(S): IF NOT SPECIFICALLY LOCATED OR NOTED
ON PLAN VIEWS, ATTIC ACCESS SHALL BE IN DIRECTION OF OWNER

PROJECT No. 02/26/16
MORGAN-S3.0.0.DWG
SHEET No. S3.0.0
DATE 02/26/16
MISC. NOTES, REFERENCES & INSTRUCTIONS
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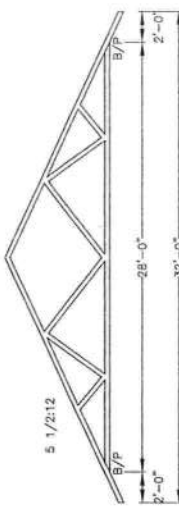
MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA

Curtis E. Keen, PE #23836
Professional Seal
DATE

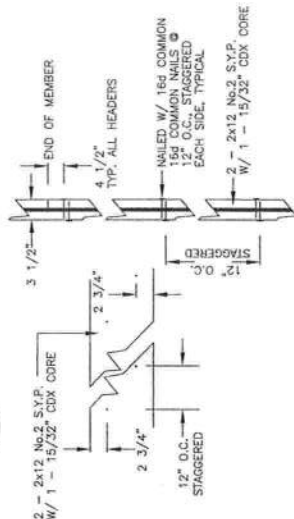
KEEN ENGINEERING
& SURVEYING, INC.
9263 CR 417
LIVE OAK, FLORIDA 32060
386-362-4787
ENG. LIC. EB 3761

NOTE: INTERIOR & GABLE END TRUSSES TO BE DESIGNED AND CERTIFIED BY A PROFESSIONAL ENGINEER. THE DESIGN SHALL BE BASED ON THE LATEST EDITION OF THE FLORIDA BUILDING CODE. THE DRAWINGS (IF ANY) DEPICTED ARE INTENDED FOR INFORMATION REGARDING REQUIRED SPAN, BEARING CAPACITY, AND MEMBER SIZES. THE DESIGNER SHALL BE RESPONSIBLE FOR ACCURATELY REFLECT OR REPRESENT MEMBER, CHORD MEMBER SIZES OR PLATED CONNECTIONS.

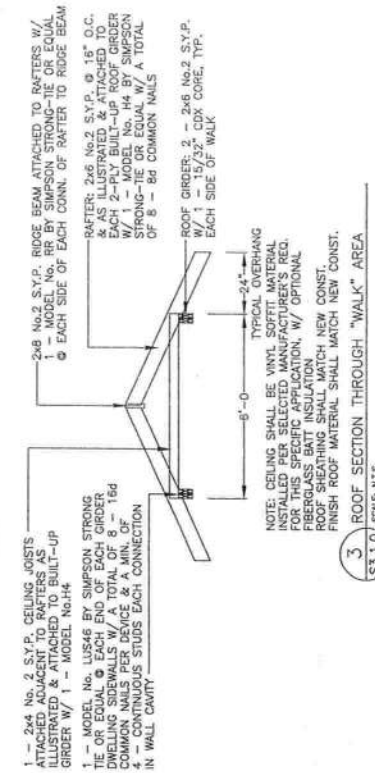
NOTE: MANUFACTURER TO SPECIFY ALL BRACING, HANDLING & INSTALLATION PRACTICES TO ENSURE PROPER STRUCTURAL INTEGRITY OF INSTALLED TRUSSES.



1 SUGGESTED ROOF TRUSS PROFILE TYPE "A"
S3.1.0 SCALE N.T.S.



2 TYPICAL EXTERIOR WALL "HEADER"
S2.3.0 SCALE N.T.S.



3 ROOF SECTION THROUGH "WALK" AREA
S3.1.0 SCALE N.T.S.

SCALE NOTE:
DETAILS/SECTIONS: N.T.S.

SCHEDULE OF REQUIRED ROOF SYSTEM MECHANICAL FASTENERS & DEVICES

TRUSS TYPE "A" AND/OR ALTERNATE "A1"

LOCATION OF MAIN DWELLING (CONDITIONED & REAR PORCH AREAS) MANUFACTURED ROOF TRUSSES: SEE PLAN VIEW FOR LOCATIONS

1 - MODEL No. H4 BY SIMPSON STRONG-TIE OR EQUAL @ EACH BEARING POINT OF ROOF TRUSS TO TOP PLATES, HEADER OR BUILT-UP PORCH GIRDER ATTACHED W/ A TOTAL OF 8 - 8d COMMON NAILS PER DEVICE

LOCATION OF MAIN DWELLING CABLE END (CONDITIONED & REAR PORCH AREAS) MANUFACTURED ROOF TRUSSES: SEE PLAN VIEW

2 - MODEL No. H4 BY SIMPSON STRONG-TIE OR EQUAL @ EACH BEARING POINT OF ROOF TRUSS TO TOP PLATES OR BUILT-UP PORCH GIRDER ATTACHED W/ A TOTAL OF 8 - 8d COMMON NAILS PER DEVICE

REAR PORCH GIRDER: SEE PLAN VIEWS FOR LOCATIONS

LOCATION OF 3-PLY BUILT-UP PORCH GIRDER - SEE PLAN VIEW FOR LOCATIONS

1 - MODEL No. AC6 MAX OR AC6B MAX BY SIMPSON STRONG-TIE OR EQUAL @ EACH BUILT-UP ORDER TO P.T. COLUMN TOP CONNECTION ATTACHED W/ A TOTAL OF 20 - 16d COMMON NAILS (AC6B MAX) OR 28 - 16d COMMON NAILS (AC6 MAX)

1 - MODEL No. U210-3 BY SIMPSON STRONG-TIE OR EQUAL @ EACH BUILT-UP ROOF GIRDER TO DWELLING ENDWALL CONNECTION W/ A TOTAL OF 18 - 16d COMMON NAILS PER DEVICE

PORCH COLUMNS: SEE PLAN VIEWS FOR LOCATIONS

LOCATION OF P.T. 6x6 PORCH COLUMNS - SEE PLAN VIEW FOR LOCATIONS

SEE ABOVE ORDERS FOR REQUIRED ATTACHMENT DEVICE(S)

COVERED WALK ROOF

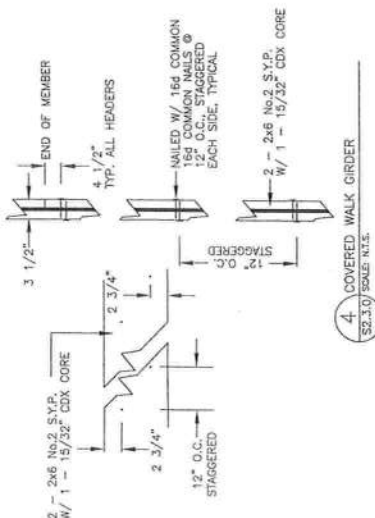
ROOF BEAM SHALL BE 1 - 2x10 No.2 S.Y.P. - SEE ROOF SYSTEM PLAN VIEW FOR LOCATIONS

RAFTERS SHALL BE 2x4 No.2 S.Y.P. - SEE ROOF SYSTEM PLAN VIEW FOR LOCATIONS

CEILING JOISTS SHALL BE 2x4 No.2 S.Y.P. - SEE ROOF SYSTEM PLAN VIEW FOR LOCATIONS

REFER TO DETAILS FOR REQUIRED ATTACHMENT

COORDINATE W/ DETAILS AND OTHER REQUIREMENTS ELSEWHERE THESE PLANS



4 COVERED WALK GIRDER
S2.3.0 SCALE N.T.S.

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products.

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	<i>Dyke</i>	<i>Inswing/outswing steel</i>	<i>FL4904-R5</i> ✓
B. SLIDING			
C. SECTIONAL			
D. ROLL UP			
E. AUTOMATIC			
F. OTHER			
2. WINDOWS			
A. SINGLE HUNG	<i>MT</i>	<i>Vinyl Single Hung</i>	<i>FL12250-R11</i> ✓
B. HORIZONTAL SLIDER			<i>15217</i>
C. CASEMENT			
D. DOUBLE HUNG			
E. FIXED			
F. AWNING			
G. PASS THROUGH			
H. PROJECTED			
I. MULLION			
J. WIND BREAKER			
K. DUAL ACTION			
L. OTHER			
3. PANEL WALL			
A. SIDING	<i>Certaainteed</i>	<i>Fibercement siding</i>	<i>5734-R3</i>
B. SOFFITS			
C. EIFS			
D. STOREFRONTS			
E. CURTAIN WALLS			
F. WALL LOUVER			
G. GLASS BLOCK			
H. MEMBRANE			
I. GREENHOUSE			
J. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES			
B. UNDERLAYMENTS			
C. ROOFING FASTENERS			
D. NON-STRUCTURAL METAL ROOFING	<i>Gulf Coast Supply</i>	<i>26 G.A Tuff Rib</i>	<i>11651.22 R1</i> ✓
E. WOOD SHINGLES AND SHAKES			
F. ROOFING TILES			
G. ROOFING INSULATION			
H. WATERPROOFING			
I. BUILT UP ROOFING ROOF SYSTEMS			
J. MODIFIED BITUMEN			
K. SINGLE PLY ROOF SYSTEMS			
L. ROOFING SLATE			
M. CEMENTS-ADHESIVES COATINGS			



Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
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N. LIQUID APPLIED FINISH OF SYSTEMS			
O. ROOF TILE ADHESIVE			
P. SPRAY APPLIED POLYURETHANE ROOF			
Q. OTHER			
5. SHUTTERS			
A. ACCORDION			
B. BAHAMA			
C. STORM PANELS			
D. COLONIAL			
E. ROLL-UP			
F. EQUIPMENT			
G. OTHERS			
6. SKYLIGHTS			
A. SKYLIGHT			
B. OTHER			
7. STRUCTURAL COMPONENTS			
A. WOOD CONNECTORS/ ANCHORS	Simpson	LSTA-MSTA-ESPY	FL 13872-RIN ✓
B. TRUSS PLATES			
C. ENGINEERED LUMBER			
D. RAILING			
E. COOLERS-FREEZERS			
F. CONCRETE ADMIXTURES			
G. MATERIAL			
H. INSULATION FORMS			
I. PLASTICS			
J. DECK-ROOF			
K. WALL			
L. SHEDS			
M. OTHER			
8. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			
B.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

APPLICANT SIGNATURE

DATE



**COLUMBIA COUNTY BUILDING DEPARTMENT
RESIDENTIAL CHECK LIST**

MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2014 EFFECTIVE 1 JULY 2015
AND THE NATIONAL ELECTRICAL 2011 EFFECTIVE 1 JULY 2015

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT 2014 FLORIDA BUILDING CODES RESIDENTIAL, EFFECTIVE 1 JULY 2015. NATIONAL ELECTRICAL CODE 2011 EFFECTIVE 1 JULY 2015. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609-A THROUGH 1609-C ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES
Revised 7/1/15

**GENERAL REQUIREMENTS:
APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL**

Items to Include-
Each Box shall be
Circled as
Applicable

		Yes	No	N/A
1	Two (2) complete sets of plans containing the following:	✓		
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void	✓		
3	Condition space (Sq. Ft.) <u>2128</u> Total (Sq. Ft.) under roof <u>3238.70</u>	IIIIII	IIIIII	IIII

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL R101.2.1

Site Plan information including:

4	Dimensions of lot or parcel of land	✓		
5	Dimensions of all building set backs	✓		
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	✓		
7	Provide a full legal description of property.	✓		

Wind-load Engineering Summary, calculations and any details are required.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
8	Plans or specifications must show compliance with FBCR Chapter 3	IIII	IIII	IIII
		YES	NO	N/A
9	Basic wind speed (3-second gust), miles per hour	✓		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	✓		
11	Wind importance factor and nature of occupancy	✓		
12	The applicable internal pressure coefficient, Components and Cladding	✓		
13	The design wind pressure in terms of psf (kN/m ²), to be used for the design of exterior component, cladding materials not speciffally designed by the registered design professional.	✓		

Elevations Drawing including:

14	All side views of the structure	✓		
15	Roof pitch	✓		
16	Overhang dimensions and detail with attic ventilation	✓		
17	Location, size and height above roof of chimneys			✓
18	Location and size of skylights with Florida Product Approval			✓
18	Number of stories	✓		
20A	Building height from the established grade to the roofs highest peak	✓		

Floor Plan including:

20	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies	✓		
21	Raised floor surfaces located more than 30 inches above the floor or grade	✓		
22	All exterior and interior shear walls indicated	✓		
23	Shear wall opening shown (Windows, Doors and Garage doors)	✓		
24	Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each bedroom (net clear opening shown) and Show compliance with Section FBC 1405.13.2 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass.	✓		
25	Safety glazing of glass where needed	✓		
26	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 and chapter 24 of FBCR)			✓
27	Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails	✓		
28	Identify accessibility of bathroom (see FBCR SECTION 320)	✓		

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans
(see Florida product approval form)

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable
---	--	--

FBCR 403: Foundation Plans

		YES	NO	N/A
29	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	✓		
30	All posts and/or column footing including size and reinforcing	✓		
31	Any special support required by soil analysis such as piling.			✓
32	Assumed load-bearing value of soil _____ Pound Per Square Foot			✓
33	Location of horizontal and vertical steel, for foundation or walls (include # size and type) For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an grounding electrode system. Per the National Electrical Code article 250.52.3	✓		

FBCR 506: CONCRETE SLAB ON GRADE

34	Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)			✓
35	Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports			✓

FBCR 318: PROTECTION AGAINST TERMITES

36	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or Submit other approved termite protection methods. Protection shall be provided by registered termiticides	✓		
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FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

37	Show all materials making up walls, wall height, and Block size, mortar type	✓		
38	Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement	✓		

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

39	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer	✓		
40	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers			✓
41	Girder type, size and spacing to load bearing walls, stem wall and/or piers	✓		
42	Attachment of joist to girder	✓		
43	Wind load requirements where applicable	✓		
44	Show required under-floor crawl space			✓
45	Show required amount of ventilation opening for under-floor spaces	✓		
46	Show required covering of ventilation opening	✓		
47	Show the required access opening to access to under-floor spaces	✓		
48	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing	✓		
49	Show Draftstopping, Fire caulking and Fire blocking	✓		
50	Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6			✓
51	Provide live and dead load rating of floor framing systems (psf).	✓		

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
		YES	NO	N/A
52	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	✓		
53	Fastener schedule for structural members per table IRC 602.3 are to be shown	✓		
54	Show Wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	✓		
55	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	✓		
56	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FBCB 2308.9.5	✓		
57	Indicate where pressure treated wood will be placed	✓		
58	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	✓		
59	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail	✓		

FBCR :ROOF SYSTEMS:

60	Truss design drawing shall meet section FBCR 802.1.7.1 Wood trusses	✓		
61	Include a layout and truss details, signed and sealed by Florida Professional Engineer	✓		
62	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	✓		
63	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	✓		
64	Provide dead load rating of trusses			

FBCR 802:Conventional Roof Framing Layout

65	Rafter and ridge beams sizes, span, species and spacing			✓
66	Connectors to wall assemblies' include assemblies' resistance to uplift rating			✓
67	Valley framing and support details			✓
68	Provide dead load rating of rafter system			

FBCR 803 ROOF SHEATHING

69	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	✓		
70	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	✓		

ROOF ASSEMBLIES FRC Chapter 9

71	Include all materials which will make up the roof assemblies covering	✓		
72	Submit Florida Product Approval numbers for each component of the roof assemblies covering	✓		

FBCR Energy Conservation R.401

Residential construction shall comply with this code by using the following compliance methods in the Residential buildings compliance methods. **Two of the required forms are to be submitted, R 402-2014 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form R 402-2014, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.**

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
		YES	NO	N/A
73	Show the insulation R value for the following areas of the structure	✓		
74	Attic space	✓		
75	Exterior wall cavity	✓		
76	Crawl space			✓

HVAC information

77	Submit two copies of a Manual J sizing equipment or equivalent computation study	✓		
78	Exhaust fans shown in bathrooms Mechanical exhaust capacity of 50 cfm intermittent or 20 cfm continuous required	✓		
79	Show clothes dryer route and total run of exhaust duct	✓		

Plumbing Fixture layout shown

80	All fixtures waste water lines shall be shown on the foundation plan	✓		
81	Show the location of water heater	✓		

Private Potable Water

82	Pump motor horse power	✓		
83	Reservoir pressure tank gallon capacity	✓		
84	Rating of cycle stop valve if used	✓		

Electrical layout shown including

85	Show Switches, receptacles outlets, lighting fixtures and Ceiling fans	✓		
86	Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected by Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A	✓		
87	Show the location of smoke detectors & Carbon monoxide detectors	✓		
88	Show service panel, sub-panel, location(s) and total ampere ratings	✓		
89	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3	✓		
90	Appliances and HVAC equipment and disconnects	✓		
91	Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed Combination arc-fault circuit interrupter , Protection device. NEC 210.12A	✓		

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable
--	--

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

		YES	NO	N/A
92	Building Permit Application A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted. There is a \$15.00 application fee. The completed application with attached documents and application fee can be mailed.	✓		
93	Parcel Number The parcel number (Tax ID number) from the Property Appraisers Office (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com	✓		
94	Environmental Health Permit or Sewer Tap Approval A copy of a approved Columbia County Environmental Health (386) 758-1058	✓		
95	City of Lake City A City Water and/or Sewer letter. Call 386-752-2031			✓
96	Toilet facilities shall be provided for all construction sites			✓
97	Town of Fort White (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.			✓
98	Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations	✓		
99	CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required.	✓		
100	A Flood development permit is also required for AE, Floodway & AH. Development permit cost is \$50.00			
101	Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. County Public Works Dept. determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required.	✓		
102	911 Address: An application for a 911 address must be applied for and received through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125.	✓		

Disclosure Statement for Owner Builders *If you as the applicant will be acting as an owner/builder under section 489.103(7) of the Florida Statutes, submit the required owner builder disclosure statement form.*

Notice Of Commencement

A notice of commencement form **recorded** in the Columbia County Clerk Office is required to be filed with the building department Before Any Inspections can be preformed.

Section R101.2.1 of the Florida Building Code Residential:

The provisions of Chapter 1, Florida Building Code shall govern the administration and enforcement of the Florida Building Code, Residential.



Manual S Compliance Report Entire House

Job: 1
Date: Jun 06, 2016
By: Derick Williams
Plan: 1

Project Information

For: Morgan, Mike Todd Const
Lake City, FL

Cooling Equipment

Design Conditions

Outdoor design DB:	92.0°F	Sensible gain:	16963	Btuh	Entering coil DB:	75.0°F
Outdoor design WB:	76.3°F	Latent gain:	3533	Btuh	Entering coil WB:	62.5°F
Indoor design DB:	75.0°F	Total gain:	20496	Btuh		
Indoor RH:	50%	Estimated airflow:	776	cfm		

Manufacturer's Performance Data at Actual Design Conditions

Equipment type:	Split ASHP		
Manufacturer:	Generic	Model:	SEER 14.0, HSPF 8.1
Actual airflow:	776	cfm	
Sensible capacity:	0	Btuh	0% of load
Latent capacity:	0	Btuh	0% of load
Total capacity:	0	Btuh	0% of load SHR: 0%

Heating Equipment

Design Conditions

Outdoor design DB:	33.4°F	Heat loss:	18314	Btuh	Entering coil DB:	68.0°F
Indoor design DB:	68.0°F					



Manufacturer's Performance Data at Actual Design Conditions

Equipment type:	Split ASHP		
Manufacturer:	Generic	Model:	SEER 14.0, HSPF 8.1
Actual airflow:	776	cfm	
Output capacity:	24000	Btuh	131% of load
Supplemental heat required:	0	Btuh	

Capacity balance: 23 °F
Economic balance: -99 °F

Backup equipment type:	Elec strip		
Manufacturer:		Model:	
Actual airflow:	776	cfm	
Output capacity:	5.4	kW	100% of load Temp. rise: 0 °F

The above equipment was selected in accordance with ACCA Manual S.



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Residential Plans Examiner Review Form for HVAC System Design (Loads, Equipment, Ducts)

Form
RPER 1
15 Mar 09

Columbia Header Information

Contractor: _____

Mechanical license: _____

Building plan #: 1

Home address (Street or Lot#, Block, Subdivision): _____, Entire House

REQUIRED ATTACHMENTS

Manual J1 Form (and supporting worksheets):
or MJ1AE Form* (and supporting worksheets):
OEM performance data (heating, cooling, blower):
Manual D Friction Rate Worksheet:
Duct distribution sketch:

ATTACHED

Yes	☐	No	☐
Yes	☐	No	☐
Yes	☐	No	☐
Yes	☐	No	☐
Yes	☐	No	☐

HVAC LOAD CALCULATION (IRC M1401.3)

Design Conditions

Winter Design Conditions

Outdoor temperature: 33 °F
Indoor temperature: 68 °F
Total heat loss: 18314 Btuh

Summer Design Conditions

Outdoor temperature: 92 °F
Indoor temperature: 75 °F
Grains difference: 47 gr/lb @ 50% RH
Sensible heat gain: 17488 Btuh
Latent heat gain: 3642 Btuh
Total heat gain: 21130 Btuh

Building Construction Information

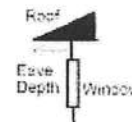
Building

Orientation: Front Door faces North
North, East, West, South, Northeast, Northwest, Southeast, Southwest

Number of bedrooms: 1
Conditioned floor area: 1968 ft²
Number of occupants: 4

Windows

Eave overhang depth: 2.0 ft
Internal shade: blinds
Blinds, drapes, etc.
Number of skylights: 0



HVAC EQUIPMENT SELECTION (IRC M1401.3)

Heating Equipment Data

Equipment type: Split ASHP
Furnace, Heat pump, Boiler, etc.
Model: Generic
SEER 14.0, HSPF 8.1
Heating output capacity: 0 Btuh
Heat pumps - capacity at winter design outdoor conditions
Aux. heating output capacity: 18314 Btuh

Cooling Equipment Data

Equipment type: Split ASHP
Air Conditioner, Heat pump, etc.
Model: Generic
SEER 14.0, HSPF 8.1+
Total cooling capacity: 0 Btuh
Sensible cooling capacity: 0 Btuh
Latent cooling capacity: 0 Btuh

Blower Data

Heating cfm: 776
Cooling cfm: 776
Static pressure: 0.50 in H2O
Fan's rated external static pressure for design airflow

HVAC DUCT DISTRIBUTION SYSTEM DESIGN (IRC M1601.1)

Design airflow: 776 cfm	Longest supply duct: 290 ft	Duct Materials Used
Equipment design ESP: 0.50 in H2O	Longest return duct: 0 ft	Trunk duct: Round flex vinyl
Total device pressure losses: 0 in H2O	Total effective length (TEL): 290 ft	Branch duct: Round flex vinyl
Available static pressure (ASP): 0.50 in H2O	Friction rate: 0.173 in/100ft Friction Rate = ASP ÷ (TEL × 100)	

I declare the load calculation, equipment, equipment selection and duct design were rigorously performed based on the building plan listed above. I understand the claims made on these forms will be subject to review and verification.

Contractor's printed name: _____

Contractor's signature: _____

Date: _____

Reserved for County, Town Municipality or Authority having jurisdiction use.

*Home qualifies for MJ1AE Form based on Abridged Edition Checklist



TRANE

Load Short Form Entire House

Job: 1
Date: Jun 06, 2016
By: Derick Williams
Plan: 1

Project Information

For: Morgan, Mike Todd Const
Lake City, Fl

Design Information

	Htg	Clg	Infiltration	Simplified
Outside db (°F)	33	92	Method	Average
Inside db (°F)	68	75	Construction quality	
Design TD (°F)	35	17	Fireplaces	
Daily range	-	M		
Inside humidity (%)	50	50		
Moisture difference (gr/lb)	29	47		

0

HEATING EQUIPMENT

Make	Generic
Trade	
Model	SEER 14.0, HSPF 8.1
AHRI ref	
Efficiency	8.1 HSPF
Heating input	
Heating output	23881 Btuh @ 47°F
Temperature rise	28 °F
Actual air flow	776 cfm
Air flow factor	0.042 cfm/Btuh
Static pressure	0.50 in H2O
Space thermostat	

COOLING EQUIPMENT

Make	Generic
Trade	
Cond	SEER 14.0, HSPF 8.1
Coil	
AHRI ref	
Efficiency	12.2 EER, 14 SEER
Sensible cooling	16800 Btuh
Latent cooling	7200 Btuh
Total cooling	24000 Btuh
Actual air flow	776 cfm
Air flow factor	0.046 cfm/Btuh
Static pressure	0.50 in H2O
Load sensible heat ratio	0.83

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
br1	238	2342	1150	99	53
living	210	2729	3993	116	183
mech	45	497	200	21	9
utility	72	96	519	4	24
toilet	20	464	185	20	8
bath1	55	711	655	30	30
bath2	36	48	22	2	1
hall	54	363	148	15	7
hall2	44	58	27	2	1
kitchen	210	2364	2940	100	134
loft2	304	3390	2812	144	129
clos2	50	280	123	12	6
clos6	20	209	88	9	4
clos5	20	15	11	1	1
loft1	340	3514	2870	149	131
bath3	50	391	502	17	23

Calculations approved by ACCA to meet all requirements of Manual J 8th Ed.



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bath4	50	391	502	17	23
utilty	25	19	14	1	1
clos4	40	224	98	9	5
clos3	30	168	74	7	3
stor	25	19	14	1	1
hall3	30	22	16	1	1
Entire House	1968	18314	16963	776	776
Other equip loads		0	0		
Equip. @ 0.97 RSM			16454		
Latent cooling			3533		
TOTALS	1968	18314	19987	776	776

Calculations approved by ACCA to meet all requirements of Manual J 8th Ed.



TRANE

Duct System Summary

Entire House

Job: 1
Date: Jun 06, 2016
By: Derick Williams
Plan: 1

Project Information

For: Morgan, Mike Todd Const
Lake City, Fl

	Heating	Cooling
External static pressure	0.50 in H2O	0.50 in H2O
Pressure losses	0 in H2O	0 in H2O
Available static pressure	0.50 in H2O	0.50 in H2O
Supply / return available pressure	0.250 / 0.250 in H2O	0.250 / 0.250 in H2O
Lowest friction rate	0.173 in/100ft	0.173 in/100ft
Actual air flow	776 cfm	776 cfm
Total effective length (TEL)		290 ft

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	H x W (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
bath1	h 711	30	30	0.314	4.0	0x0	VIFx	29.0	130.0	st6
bath2	h 48	2	1	0.379	4.0	0x0	VIFx	22.0	110.0	st6
bath3	c 502	17	23	0.209	4.0	0x0	VIFx	39.0	200.0	st4
bath4	c 502	17	23	0.258	4.0	0x0	VIFx	34.0	160.0	st4
br1	h 2342	99	53	0.286	6.0	0x0	VIFx	25.0	150.0	st6
clos2	h 280	12	6	0.190	4.0	0x0	VIFx	43.0	220.0	st4
clos3	h 168	7	3	0.177	4.0	0x0	VIFx	43.0	240.0	st4
clos4	h 224	9	5	0.254	4.0	0x0	VIFx	37.0	160.0	st5
clos5	h 15	1	1	0.252	4.0	0x0	VIFx	38.0	160.0	st5
clos6	h 209	9	4	0.246	4.0	0x0	VIFx	43.0	160.0	st5
hall	h 363	15	7	0.243	4.0	0x0	VIFx	26.0	180.0	st6A
hall2	h 58	2	1	0.294	4.0	0x0	VIFx	30.0	140.0	st2
hall3	h 22	1	1	0.237	4.0	0x0	VIFx	31.0	180.0	st4
kitchen	c 2940	100	134	0.287	7.0	0x0	VIFx	34.1	140.0	st2
living	c 3993	116	183	0.286	8.0	0x0	VIFx	34.7	140.0	st2
loft1	h 3514	149	131	0.235	7.0	0x0	VIFx	43.0	170.0	st5
loft2	h 3390	144	129	0.173	7.0	0x0	VIFx	39.5	250.0	st4
mech	h 497	21	9	0.286	4.0	0x0	VIFx	35.0	140.0	st2
stor	h 19	1	1	0.215	4.0	0x0	VIFx	32.0	200.0	st4
toilet	h 464	20	8	0.220	4.0	0x0	VIFx	37.0	190.0	st6A
utility	c 519	4	24	0.292	4.0	0x0	VIFx	31.0	140.0	st2
utility	h 19	1	1	0.267	4.0	0x0	VIFx	27.0	160.0	st4



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Project Summary Entire House

Job: 1
Date: Jun 06, 2016
By: Derick Williams
Plan: 1

Project Information

For: Morgan, Mike Todd Const
Lake City, FL

Notes:

Design Information

Weather: Gainesville Regional AP, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	68 °F
Design TD	35 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	47 gr/lb

Heating Summary

Structure	18314 Btuh
Ducts	0 Btuh
Central vent (0 cfm)	0 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	18314 Btuh

Sensible Cooling Equipment Load Sizing

Structure	16963 Btuh
Ducts	0 Btuh
Central vent (0 cfm)	0 Btuh
Blower	0 Btuh

Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	16454 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft ²)	1968	1968
Volume (ft ³)	19680	19680
Air changes/hour	0.50	0.26
Equiv. AVF (cfm)	164	85

Latent Cooling Equipment Load Sizing

Structure	3533 Btuh
Ducts	0 Btuh
Central vent (0 cfm)	0 Btuh
Equipment latent load	3533 Btuh
Equipment total load	19987 Btuh
Req. total capacity at 0.70 SHR	2.0 ton

Heating Equipment Summary

Make	Generic
Trade	
Model	SEER 14.0, HSPF 8.1
AHRI ref	
Efficiency	8.1 HSPF
Heating input	
Heating output	23881 Btuh @ 47°F
Temperature rise	28 °F
Actual air flow	776 cfm
Air flow factor	0.042 cfm/Btuh
Static pressure	0.50 in H2O
Space thermostat	

Cooling Equipment Summary

Make	Generic
Trade	
Cond	SEER 14.0, HSPF 8.1
Coil	
AHRI ref	
Efficiency	12.2 EER, 14 SEER
Sensible cooling	16800 Btuh
Latent cooling	7200 Btuh
Total cooling	24000 Btuh
Actual air flow	776 cfm
Air flow factor	0.046 cfm/Btuh
Static pressure	0.50 in H2O
Load sensible heat ratio	0.83

Calculations approved by ACCA to meet all requirements of Manual J 8th Ed.



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Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)	Duct Material	Trunk
st2	Peak AVF	609	677	0.173	485	16.0	0 x 0	VinIFlx	st1
st6	Peak AVF	166	99	0.220	477	8.0	0 x 0	VinIFlx	st1
st1	Peak AVF	776	776	0.173	556	16.0	0 x 0	VinIFlx	
st6A	Peak AVF	35	15	0.220	401	4.0	0 x 0	VinIFlx	st6
st3	Peak AVF	366	326	0.173	466	12.0	0 x 0	VinIFlx	st2
st4	Peak AVF	198	185	0.173	568	8.0	0 x 0	VinIFlx	st3
st5	Peak AVF	168	140	0.235	481	8.0	0 x 0	VinIFlx	st3

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	776	776	0	0	0	0	0x 0		VIFx	

FLORIDA BUILDING CODE, ENERGY CONSERVATION

Residential Building Thermal Envelope Approach

FORM R402-2014

Climate Zone ☐

Scope: Compliance with Section R402.1.1 of the *Florida Building Code, Energy Conservation*, shall be demonstrated by the use of Form R402 for single- and multiple-family residences of three stories or less in height, additions to existing residential buildings, alterations, renovations, and building systems in existing buildings, as applicable. To comply, a building must meet or exceed all of the energy efficiency requirements on Table R402A and all applicable mandatory requirements summarized in Table R402B of this form. If a building does not comply with this method, or by the UA Alternative method, it may still comply under Section R405 of the *Florida Building Code, Energy Conservation*.

PROJECT NAME:
AND ADDRESS:

BUILDER:

OWNER:

DAVID F. BRYN MORGAN

PERMITTING OFFICE:

JURISDICTION NUMBER:

PERMIT NUMBER:

General Instructions:

1. Fill in all the applicable spaces of the "To Be Installed" column on Table R402A with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
2. Complete page 1 based on the "To Be Installed" column information.
3. Read the requirements of Table R402B and check each box to indicate your intent to comply with all applicable items.
4. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

1. New construction, addition, or existing building	1. <u>NEW</u>	
2. Single-family detached or multiple-family attached	2. <u>SF</u>	
3. If multiple-family, number of units covered by this submission	3. <u>NO</u>	
4. Is this a worst case? (yes/no)	4. <u>NO</u>	
5. Conditioned floor area (sq. ft.)	5. <u>1,064 per floor</u>	
6. Windows, type and area		
a) U-factor:	6a. <u>0.65</u>	
b) Solar Heat Gain Coefficient (SHGC)	6b. <u>0.25</u>	
c) Area	6c. <u>135</u>	
7. Skylights		
a) U-factor:	7a. <u>-</u>	
b) Solar Heat Gain Coefficient (SHGC)	7b. <u>-</u>	
8. Floor type, area or perimeter, and insulation:		
a) Slab-on-grade (R-value)	8a. <u>-</u>	
b) Wood, raised (R-value)	8b. <u>1064</u>	
c) Wood, common (R-value)	8c. <u>-</u>	
d) Concrete, raised (R-value)	8d. <u>-</u>	
e) Concrete, common (R-value)	8e. <u>-</u>	
9. Wall type and insulation:		
a) Exterior:		
1. Wood frame (Insulation R-value)	9a1. <u>13</u>	
2. Masonry (Insulation R-value)	9a2. <u>-</u>	
b) Adjacent:		
1. Wood frame (Insulation R-value)	9b1. <u>-</u>	
2. Masonry (Insulation R-value)	9b2. <u>-</u>	
10. Ceiling type and insulation		
a) Attic (Insulation R-value)	10a. <u>30</u>	
b) Single assembly (Insulation R-value)	10b. <u>-</u>	
11. Air distribution system:		
a) Duct location, insulation	11a. <u>ATTIC</u>	
b) AHU location	11b. <u>CONDITIONED</u>	
c) Total duct leakage. Test report attached.	11c. <u>cfm/100 s.f.</u> Yes ☐ No ☐	
12. Cooling system:		
a) type	12a. <u>CENTRAL</u>	
b) efficiency	12b. <u>SEER 14.0</u>	
13. Heating system:		
a) type	13a. <u>HEAT PUMP</u>	
b) efficiency:	13b. <u>8.2</u>	
14. HVAC sizing calculation: attached	14. Yes ☐ No ☒	
15. Water heating system:		
a) type	15a. <u>ELECTRIC</u>	
b) efficiency	15b. <u>0.90</u>	



I hereby certify that the plans and specifications covered by this form are in compliance with the *Florida Building Code, Energy Conservation*.
 PREPARED BY: Curtis Kien Date: 5/10/16
 I hereby certify that this building is in compliance with the *Florida Building Code, Energy Conservation*.
 OWNER/AGENT: _____ Date: _____

Review of plans and specifications covered by this form indicate compliance with the *Florida Building Code, Energy Conservation*. Before construction is complete, this building will be inspected for compliance in accordance with Section 553.908, F.S.
 CODE OFFICIAL: _____
 Date: _____

TABLE R402B MANDATORY REQUIREMENTS			
Component	Section	Summary of Requirement(s)	Check
Air leakage	R402.4	To be caulked, gasketed, weatherstripped or otherwise sealed per Table R402.4.1.1. Recessed lighting: IC-rated as having ≤ 2.0 cfm tested to ASTM E 283. Windows and doors: 0.3 cfm/sq.ft (swinging doors: 0.5 cfm/sf) when tested to NFRC 400 or AAMA/WDMA/CSA 101/I.S. 2/A440. Fireplaces: Tight-fitting flue dampers & outdoor combustion air.	✓
Programmable thermostat	R403.1.2	Where forced-air furnace is primary system, a programmable thermostat is required.	✓
Air distribution system	R403.2.2 R403.2.4	Ducts shall be tested to Section 803 of the RESNET standards by an energy rater certified in accordance with Section 553.99, <i>Florida Statutes</i> , or as authorized by <i>Florida Statutes</i> . Air handling units are not allowed in attics.	✓
Water heaters	R403.4	Comply with efficiencies in Table C404.2. Hot water pipes insulated to $\geq R-3$ to kitchen outlets, other cases. Circulating systems to have an automatic or accessible manual OFF switch. Heat trap required for vertical pipe risers.	✓
Swimming pools & spas	R403.9	Spas and heated pools must have vapor-retardant covers or a liquid cover or other means proven to reduce heat loss except if 70% of heat from site-recovered energy. Off/timer switch required. Gas heaters minimum thermal efficiency is 82%. Heat pump pool heaters minimum COP is 4.0.	✓
Cooling/heating equipment	R403.6	Sizing calculation performed & attached. Special occasion cooling or heating capacity requires separate system or variable capacity system.	
Lighting equipment	R404.1	At least 75% of permanently installed lighting fixtures shall be high-efficacy lamps.	✓



Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: 755227 - MIKE TODD CONST. - MORGAN RES.

MiTek USA, Inc.

6904 Parke East Blvd.
Tampa, FL 33610-4115

Site Information:

Customer Info: Mike Todd Const. Project Name: 755227 Model: Morgan Res.
Lot/Block: Subdivision:
Address: 218 SW Riverside Ave.
City: Columbia Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: Unknown at time of seals License #: Unknown at time of seals
Address: Unknown at time of seals
City: Unknown at time of seals State: Unknown at time of seals

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2014/TPI2007 Design Program: MiTek 20/20 7.6
Wind Code: ASCE 7-10 Wind Speed: 130 mph
Roof Load: 37.0 psf Floor Load: 55.0 psf

This package includes 9 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T8884698	F01	6/20/016
2	T8884699	F02	6/20/016
3	T8884700	F03	6/20/016
4	T8884701	KW1	6/20/016
5	T8884702	T01	6/20/016
6	T8884703	T01G	6/20/016
7	T8884704	T01GG	6/20/016
8	T8884705	T02	6/20/016
9	T8884706	T02G	6/20/016



The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Albani, Thomas
My license renewal date for the state of Florida is February 28, 2017.

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.



Thomas A. Albani PE No. 39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 20, 2016

Job 755227	Truss F01	Truss Type FLOOR	Qty 18	Ply 1	MIKE TODD CONST. - MORGAN RES.	T8884698
Builders FirstSource, Lake City, FL 32055						Job Reference (optional)

7.640 s Apr 19 2016 MiTek Industries, Inc. Mon Jun 20 18:37:23 2016 Page 1
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0-1-8

1-3-0

1-10-0

0-1-8

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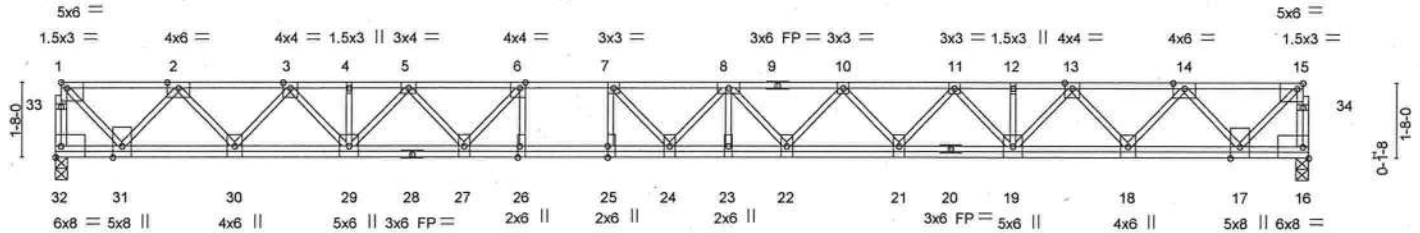


Plate Offsets (X,Y) =	[1:Edge,0-1-8], [6:0-1-8,Edge], [15:0-1-8,Edge], [25:0-3-0,0-0-0], [26:0-3-0,Edge]
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LOADING (psf)	SPACING-	1-7-2	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.45	Vert(LL)	-0.45	24	>731	360	MT20
TCDL 10.0	Lumber DOL	1.00	BC 0.48	Vert(TL)	-0.71	24	>468	240	244/190
BCLL 0.0	Rep Stress Incr	YES	WB 0.71	Horz(TL)	0.07	16	n/a	n/a	
BCDL 5.0	Code FBC2014/TPI2007		(Matrix)						
									Weight: 195 lb FT = 11%F, 11%E

LUMBER-

TOP CHORD 2x4 SP M 31(flat)
BOT CHORD 2x4 SP M 31(flat)
WEBS 2x4 SP No.3(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 32=1209/0-3-4, 16=1209/0-3-4

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 32-33=-1198/0, 1-33=-1197/0, 16-34=-1197/0, 15-34=-1196/0, 1-2=-1040/0, 2-3=-2710/0, 3-4=-4042/0, 4-5=-4042/0, 5-6=-4929/0, 6-7=-5463/0, 7-8=-5642/0, 8-9=-5478/0, 9-10=-5478/0, 10-11=-4949/0, 11-12=-4035/0, 12-13=-4035/0, 13-14=-2712/0, 14-15=-1040/0
BOT CHORD 30-31=0/1974, 29-30=0/3431, 28-29=0/4567, 27-28=0/4567, 26-27=0/5463, 25-26=0/5463, 24-25=0/5463, 23-24=0/5669, 22-23=0/5669, 21-22=0/5298, 20-21=0/4580, 19-20=0/4580, 18-19=0/3428, 17-18=0/1974
WEBS 6-26=-85/592, 7-25=-563/128, 1-31=0/1491, 2-31=-1447/0, 2-30=0/1140, 3-30=-1117/0, 3-29=0/922, 5-29=-790/0, 5-27=0/621, 6-27=-1006/0, 15-17=0/1490, 14-17=-1448/0, 14-18=0/1142, 13-18=-1111/0, 13-19=0/914, 11-19=-820/0, 11-21=0/573, 10-21=-540/0, 10-22=0/305, 8-22=-312/0, 7-24=-303/588

NOTES- (5)

- Unbalanced floor live loads have been considered for this design.
- All plates are 3x6 MT20 unless otherwise indicated.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST. - MORGAN RES.	T8884699
755227	F02	Floor	5	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

7.640 s Apr 19 2016 MiTek Industries, Inc. Mon Jun 20 18:37:24 2016 Page 1
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0-1-8

1-3-0

1-10-0

0-8-0
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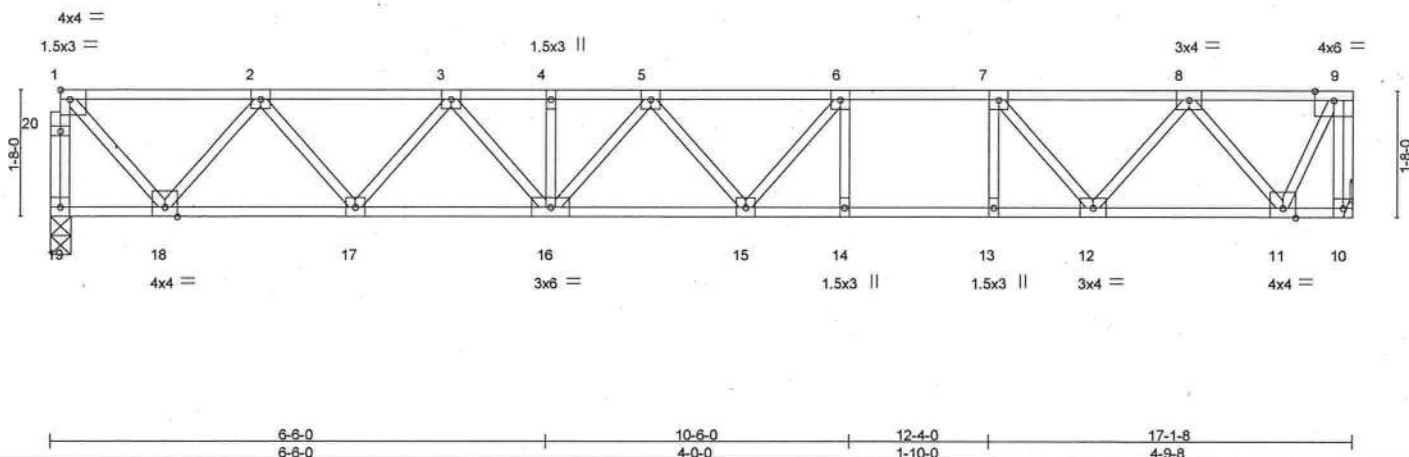


Plate Offsets (X,Y)-- [1:Edge,0-1-8]		6-6-0		10-6-0		12-4-0		17-1-8	
		6-6-0		4-0-0		1-10-0		4-9-8	
LOADING (psf)	SPACING-	1-7-2	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.34	Vert(LL)	-0.18 14-15	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.00	BC 0.60	Vert(TL)	-0.28 14-15	>736	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.41	Horz(TL)	0.03 10	n/a	n/a		
BCDL 5.0	Code FBC2014/TPI2007		(Matrix)					Weight: 98 lb	FT = 11%F, 11%E

LUMBER-

TOP CHORD 2x4 SP M 31(flat)
BOT CHORD 2x4 SP M 31(flat)
WEBS 2x4 SP No.3(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 19=735/0-3-4, 10=740/Mechanical

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 19-20=-730/0, 1-20=-730/0, 9-10=-751/0, 1-2=-582/0, 2-3=-1420/0, 3-4=-1899/0, 4-5=-1899/0, 5-6=-1899/0,
6-7=-1783/0, 7-8=-1259/0, 8-9=-348/0
BOT CHORD 17-18=0/1096, 16-17=0/1724, 15-16=0/2056, 14-15=0/1783, 13-14=0/1783, 12-13=0/1783, 11-12=0/863
WEBS 6-14=-323/0, 7-13=0/348, 1-18=0/852, 2-18=-816/0, 2-17=0/515, 3-17=-483/0, 3-16=0/270, 6-15=-49/416,
9-11=0/761, 8-11=-817/0, 8-12=0/629, 7-12=-814/0

NOTES- (7)

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x3 MT20 unless otherwise indicated.
- 3) Refer to girder(s) for truss to truss connections.
- 4) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 38610

Job 755227	Truss F03	Truss Type Floor Girder	Qty 1	Ply 1	MIKE TODD CONST. - MORGAN RES.	T8884700
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Builders FirstSource, Lake City, FL 32055

7.840 s Apr 19 2016 MiTek Industries, Inc. Mon Jun 20 18:37:25 2016 Page 1
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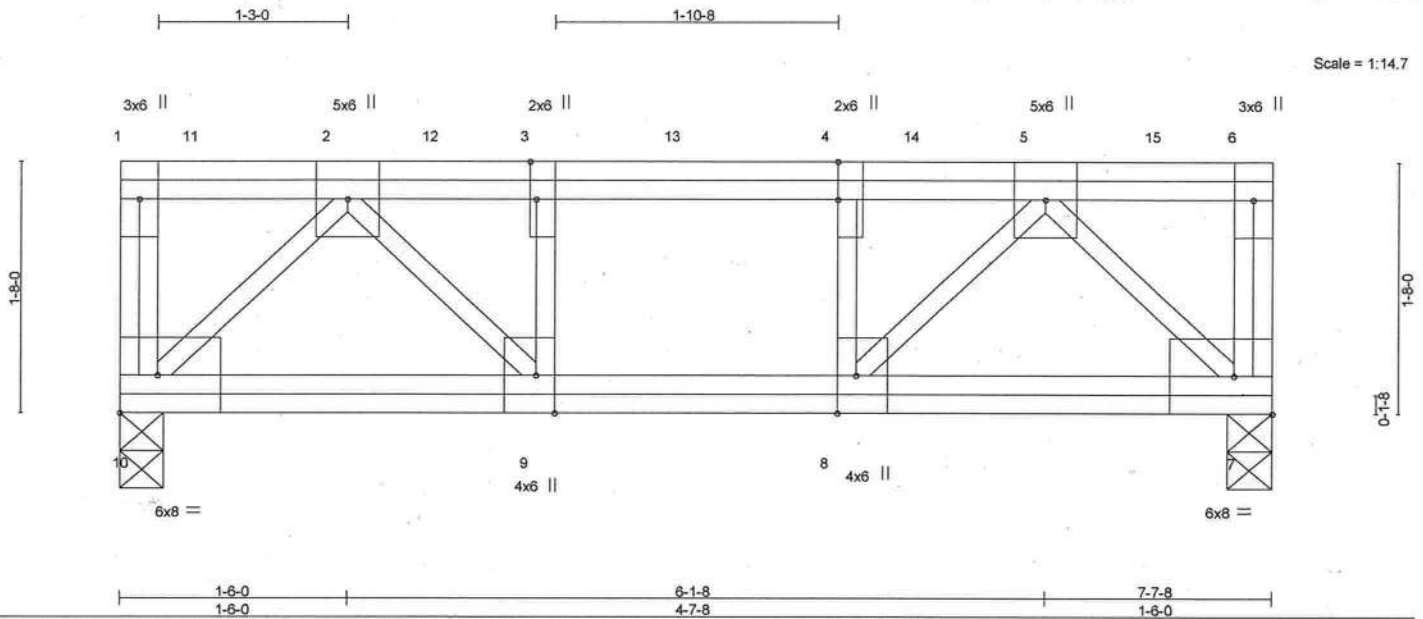


Plate Offsets (X,Y)– [3:0-3-0,Edge], [4:0-3-0,Edge], [8:0-3-0,Edge], [9:0-3-0,Edge]									
LOADING (psf)		SPACING- 1-7-2		CSI.		DEFL. in (loc) l/defl L/d		PLATES GRIP	
TCLL	40.0	Plate Grip DOL	1.00	TC	0.22	Vert(LL)	-0.02 8-9 >999 360	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.17	Vert(TL)	-0.03 8-9 >999 240		
BCLL	0.0	Rep Stress Incr	NO	WB	0.57	Horz(TL)	0.01 7 n/a n/a		
BCDL	5.0	Code FBC2014/TPI2007		(Matrix)				Weight: 66 lb	FT = 11%F, 11%E

LUMBER-
TOP CHORD 2x4 SP M 31(flat)
BOT CHORD 2x4 SP M 31(flat)
WEBS 2x4 SP No.3(flat)

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 10=2033/0-3-8, 7=1994/0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-10=-423/0, 6-7=-378/0, 2-12=-2353/0, 3-12=-2353/0, 3-13=-2353/0, 4-13=-2353/0, 4-14=-2353/0, 5-14=-2353/0
BOT CHORD 9-10=0/1544, 8-9=0/2353, 7-8=0/1550
WEBS 5-7=-2225/0, 2-10=-2217/0, 5-8=0/1196, 2-9=0/1203, 3-9=-864/0, 4-8=-862/0

NOTES- (6)

- Unbalanced floor live loads have been considered for this design.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 727 lb down at 0-7-0, 727 lb down at 2-2-2, 676 lb down at 3-9-4, and 727 lb down at 5-4-6, and 727 lb down at 6-11-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

- Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 7-10=-8, 1-6=-80
Concentrated Loads (lb)
Vert: 11=-676(F) 12=-676(F) 13=-676(F) 14=-676(F) 15=-676(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 36610

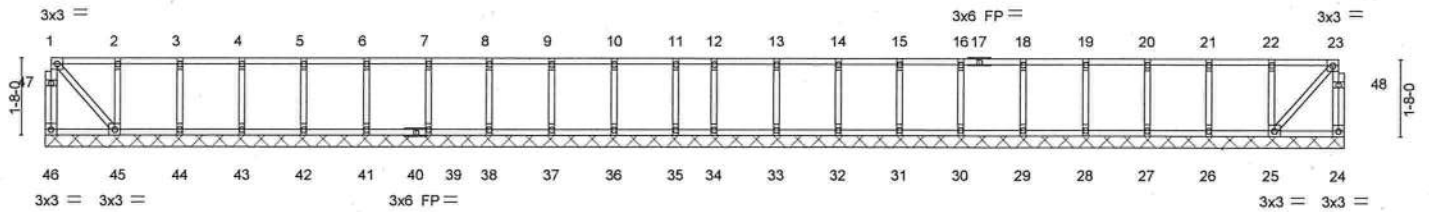
Job 755227	Truss KW1	Truss Type GABLE	Qty 2	Ply 1	MIKE TODD CONST. - MORGAN RES. T8884701
Builders FirstSource, Lake City, FL 32055					

7.640 s Apr 19 2016 MiTek Industries, Inc. Mon Jun 20 18:37:25 2016 Page 1
ID:AwD7fSKXwlnKYwWX1sWuKPzYL70-D7qhv8qPEyug9dmiEurHdHDzwAw_uks_yXKLo?z4JnO

0-1-8

0-1-8

Scale: 1/4"=1'



1-6-12	2-10-12	4-2-12	5-6-12	6-10-12	8-2-12	9-6-12	10-10-12	12-2-12	13-6-12	14-4-12	15-8-12	17-0-12	18-4-12	19-8-12	21-0-12	22-4-12	23-8-12	25-0-12	26-4-12	27-11-8
1-6-12	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	0-10-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-6-12
LOADING (psf)	SPACING- 2-0-0					CSI.		DEFL.		in (loc)		l/defl		L/d		PLATES		GRIP		
TCLL 40.0	Plate Grip DOL 1.00					TC 0.04		Vert(LL) n/a		-		n/a		999		MT20		244/190		
TCDL 10.0	Lumber DOL 1.00					BC 0.00		Vert(TL) n/a		-		n/a		999						
BCLL 0.0	Rep Stress Incr YES					WB 0.04		Horz(TL) -0.00		25		n/a		n/a						
BCDL 5.0	Code FBC2014/TPI2007					(Matrix)												Weight: 138 lb		FT = 11%F, 11%E

LUMBER-

TOP CHORD 2x4 SP M 31(flat)
BOT CHORD 2x4 SP M 31(flat)
WEBS 2x4 SP No.3(flat)
OTHERS 2x4 SP No.3(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 45-46,24-25.

REACTIONS.

All bearings 27-11-8.
(lb) - Max Grav All reactions 250 lb or less at joint(s) 46, 24, 45, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 44, 43, 42, 41, 39, 38, 37, 36, 35

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (7)

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Gable requires continuous bottom chord bearing.
- 3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 4) Gable studs spaced at 1-4-0 oc.
- 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 36610

Job 755227	Truss T01	Truss Type COMMON	Qty 18	Ply 1	MIKE TODD CONST. - MORGAN RES.	T8884702
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Builders FirstSource, Lake City, FL 32055

7.640 s Apr 19 2016 MiTek Industries, Inc. Mon Jun 20 18:37:27 2016 Page 1

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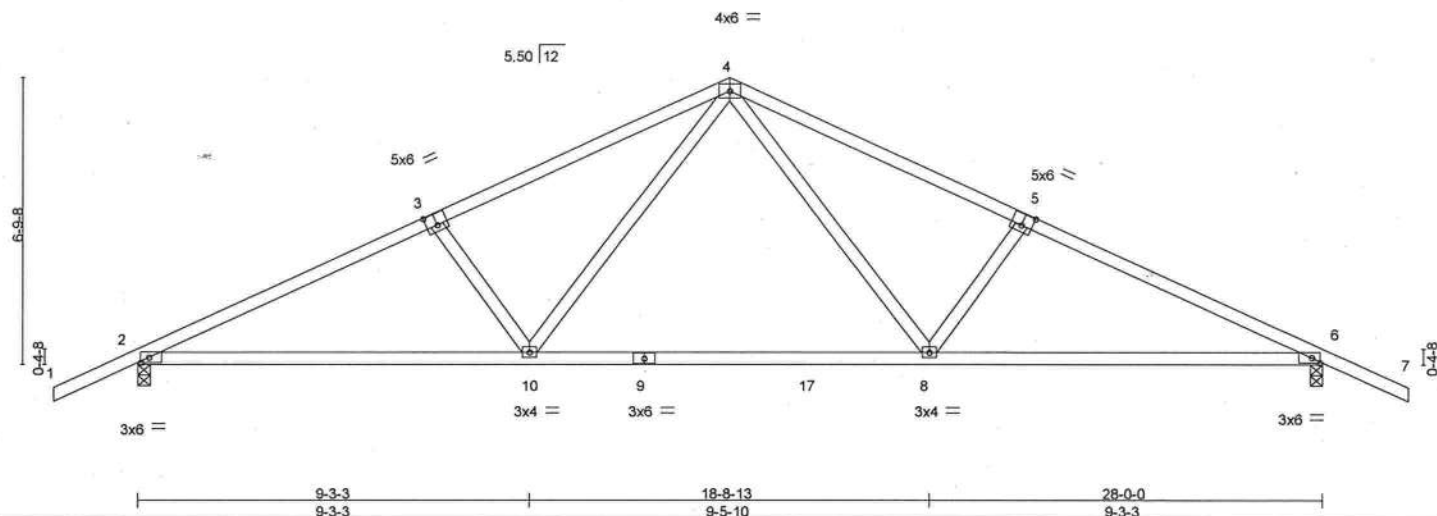
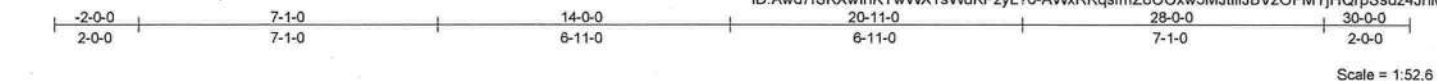


Plate Offsets (X,Y)- [2:0-2-6,0-1-8], [3:0-3-0,0-3-4], [5:0-3-0,0-3-4], [6:0-2-6,0-1-8]											
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc)		l/defl L/d		PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.25		TC	0.54	Vert(LL)	-0.32 8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL 1.25		BC	0.85	Vert(TL)	-0.61 8-10	>554	180		
BCLL	0.0 *	Rep Stress Incr YES		WB	0.40	Horz(TL)	0.08 6	n/a	n/a		
BCDL	10.0	Code FBC2014/TPI2007		(Matrix-M)						Weight: 129 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-7-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-8-8 oc bracing.

REACTIONS. (lb/size) 2=1144/0-3-8, 6=1144/0-3-8
Max Horz 2=154(LC 17)
Max Uplift 2=460(LC 12), 6=460(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1863/1007, 3-4=-1657/969, 4-5=-1657/969, 5-6=-1863/1007
BOT CHORD 2-10=-767/1631, 9-10=-376/1079, 9-17=-376/1079, 8-17=-376/1079, 6-8=-773/1631
WEBS 4-8=-316/623, 5-8=-371/375, 4-10=-316/625, 3-10=-371/375

NOTES- (7)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 460 lb uplift at joint 2 and 460 lb uplift at joint 6.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 36610

Job 755227	Truss T01G	Truss Type GABLE	Qty 1	Ply 1	MIKE TODD CONST. - MORGAN RES.	T8884703
Builders FirstSource, Lake City, FL 32055						Job Reference (optional)

7.640 s Apr 19 2016 MiTek Industries, Inc. Mon Jun 20 18:37:28 2016 Page 1
ID: Awd7fSKXwlnKYwWX1sWuKPzYL70-eIVqY9sHXIGF04VHw0O_EwrQ2NwA54eQfVZ?PKz4JnL

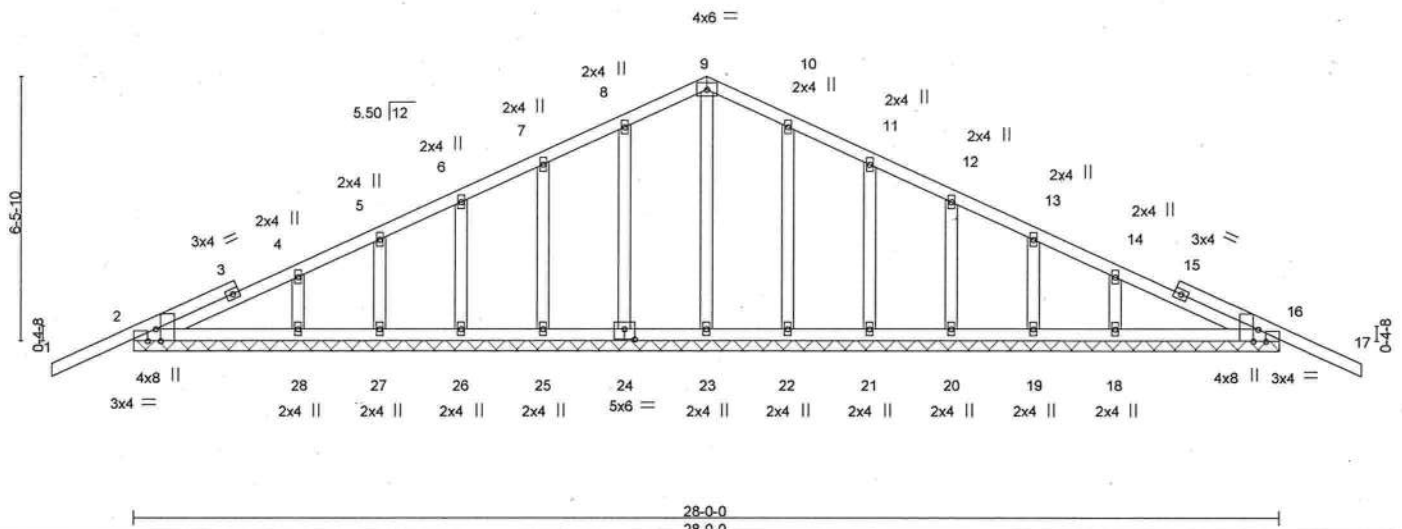


Plate Offsets (X,Y)-- [2:0-3-8,Edge], [2:0-2-4,Edge], [16:0-3-8,Edge], [16:0-2-4,Edge], [24:0-3-0,0-3-0]												
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc)		l/defl L/d		PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.25	TC	0.30	Vert(LL)	-0.02	17	n/r	120	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.10	Vert(TL)	-0.02	17	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.10	Horz(TL)	0.01	16	n/a	n/a		
BCDL	10.0	Code FBC2014/TPI2007		(Matrix)							Weight: 156 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 28-0-0.
(lb) - Max Horz 2=147(LC 12)
Max Uplift All uplift 100 lb or less at joint(s) 24, 25, 26, 27, 22, 21, 20, 19 except 2=105(LC 8), 16=123(LC 13), 28=113(LC 12), 18=118(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 23, 24, 25, 26, 27, 28, 22, 21, 20, 19, 18 except 2=267(LC 1), 16=267(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 8-9=-81/252, 9-10=-81/252

- NOTES-** (10)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 25, 26, 27, 22, 21, 20, 19 except (jt=lb) 2=105, 16=123, 28=113, 18=118.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MI TEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 36610

Job 755227	Truss T01GG	Truss Type GABLE	Qty 1	Ply 1	MIKE TODD CONST. - MORGAN RES.	T8884704
Builders FirstSource, Lake City, FL 32055						Job Reference (optional)

7.640 s Apr 19 2016 MiTek Industries, Inc. Mon Jun 20 18:37:30 2016 Page 1
ID: Awd7fSKXwlnKYwWX1sVWuKPzYL70-a5dazruY3UWzF0ef1RQSKLxmXBcEZ_?j6p26TDz4JnJ

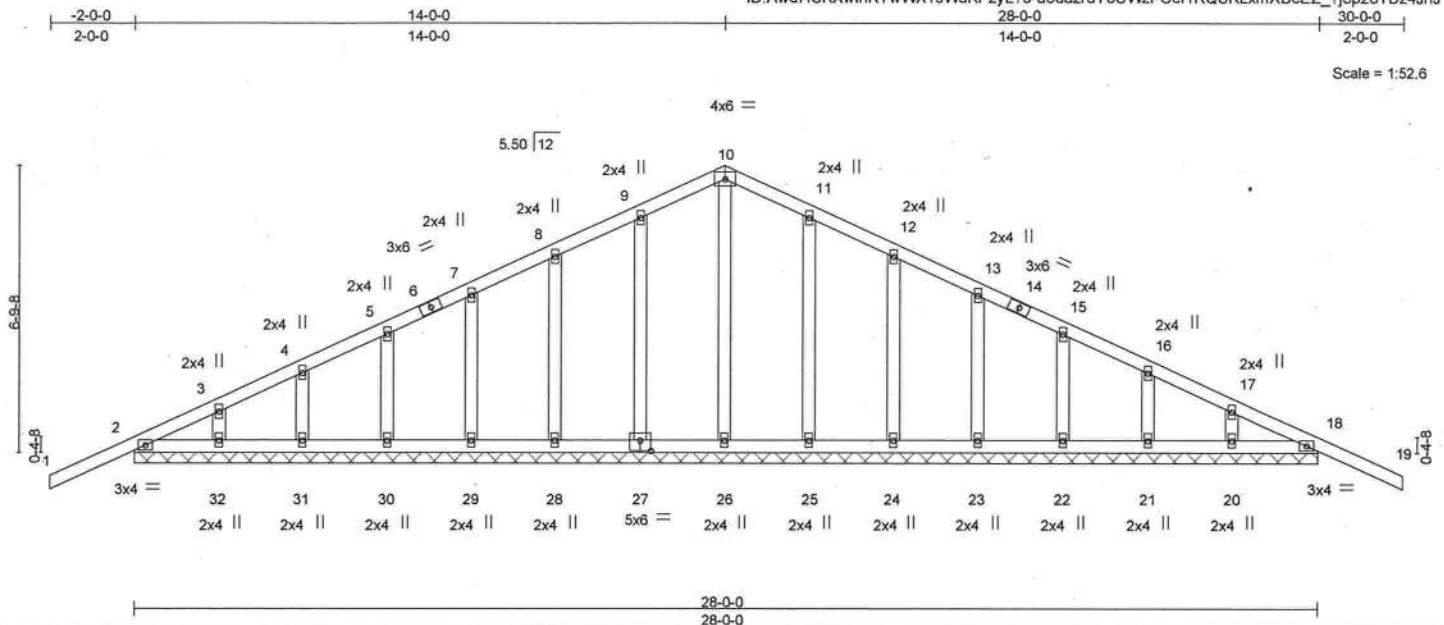


Plate Offsets (X,Y) [27'-0"-3'-0"-3'-0"]							
LOADING (psf)	SPACING-	2'-0"-0"	CSI.	DEFL.	in (loc)	I/defl	L/d
TCLL 20.0	Plate Grip DOL	1.25	TC 0.30	Vert(LL)	-0.02	19	n/r
TCDL 7.0	Lumber DOL	1.25	BC 0.13	Vert(TL)	-0.04	19	n/r
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.11	Horz(TL)	0.01	18	n/a
BCDL 10.0	Code FBC2014/TPI2007		(Matrix)				
				Weight: 158 lb FT = 20%			

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS. All bearings 28'-0"-0".
(lb) - Max Horz 2=154(LC 13)
Max Uplift All uplift 100 lb or less at joint(s) 27, 28, 29, 30, 32, 25, 24, 23, 22, 20 except 2=117(LC 8),
31=103(LC 12), 21=103(LC 13), 18=112(LC 9)
Max Grav All reactions 250 lb or less at joint(s) 2, 26, 27, 28, 29, 30, 31, 32, 25, 24, 23, 22, 21, 20, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 9-10=95/268, 10-11=95/268

- NOTES-** (10)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - 4) Gable requires continuous bottom chord bearing.
 - 5) Gable studs spaced at 2'-0" oc.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 27, 28, 29, 30, 32, 25, 24, 23, 22, 20 except (jt=lb) 2=117, 31=103, 21=103, 18=112.
 - 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 36610

Job 755227	Truss T02	Truss Type COMMON	Qty 3	Ply 1	MIKE TODD CONST. - MORGAN RES.	T8884705
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Builders FirstSource, Lake City, FL 32055

7,640 s Apr 19 2016 MiTek Industries, Inc. Mon Jun 20 18:37:31 2016 Page 1
ID:Aw77SKXwlnKYwWX1sWuKPzyl?0-2HByABvAqoeqtYDsb9yhsYTpwamBIOzsLTnf?fz4Jnl

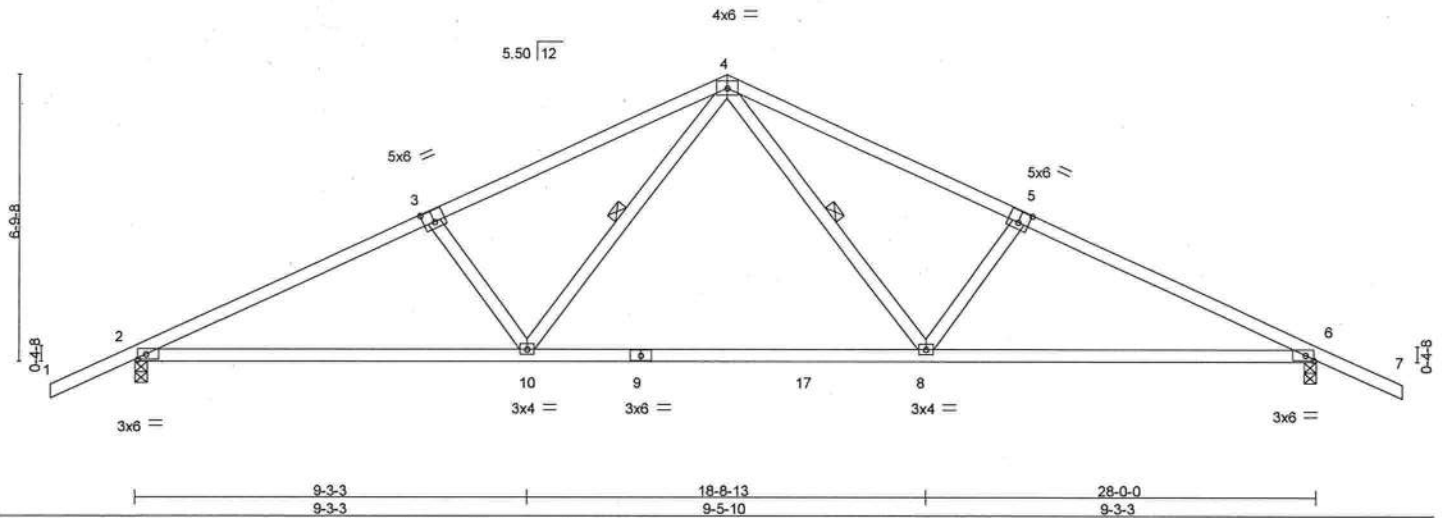
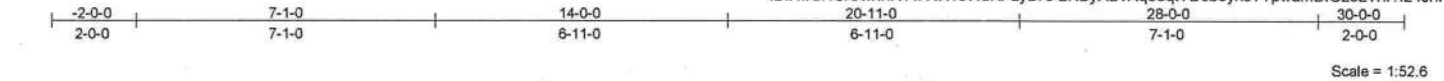


Plate Offsets (X,Y)-- [2:0-2-6,0-1-8], [3:0-3-0,0-3-4], [5:0-3-0,0-3-4], [6:0-2-6,0-1-8]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.71	Vert(LL)	0.38	8-10	>894	240	MT20
TCDL 7.0	Lumber DOL	1.25	BC 0.85	Vert(TL)	-0.61	8-10	>554	180	244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.32	Horz(TL)	0.08	6	n/a	n/a	
BCDL 10.0	Code FBC2014/TPI2007		(Matrix-M)						
								Weight: 129 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-7-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-10-9 oc bracing.
WEBS 1 Row at midpt 4-8, 4-10

REACTIONS. (lb/size) 2=1144/0-3-8, 6=1144/0-3-8
Max Horz 2=154(LC 12)
Max Uplift 2=561(LC 9), 6=561(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1863/2159, 3-4=-1657/2120, 4-5=-1657/2119, 5-6=-1863/2159
BOT CHORD 2-10=-1817/1631, 9-10=-1072/1079, 9-17=-1072/1079, 8-17=-1072/1079, 6-8=-1823/1631
WEBS 4-8=-910/623, 5-8=-371/382, 4-10=-910/625, 3-10=-371/382

NOTES- (7)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl.; GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=561, 6=561.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MI TEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



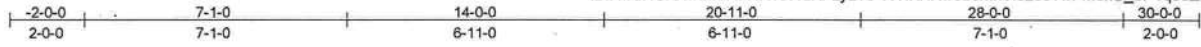
6904 Parke East Blvd.
Tampa, FL 36610

Job 755227	Truss T02G	Truss Type COMMON STRUCTURAL GA	Qty 1	Ply 1	MIKE TODD CONST. - MORGAN RES.	T8884706
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Builders FirstSource, Lake City, FL 32055

7.640 s Apr 19 2016 MITek Industries, Inc. Mon Jun 20 18:37:32 2016 Page 1

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Scale = 1:59.9

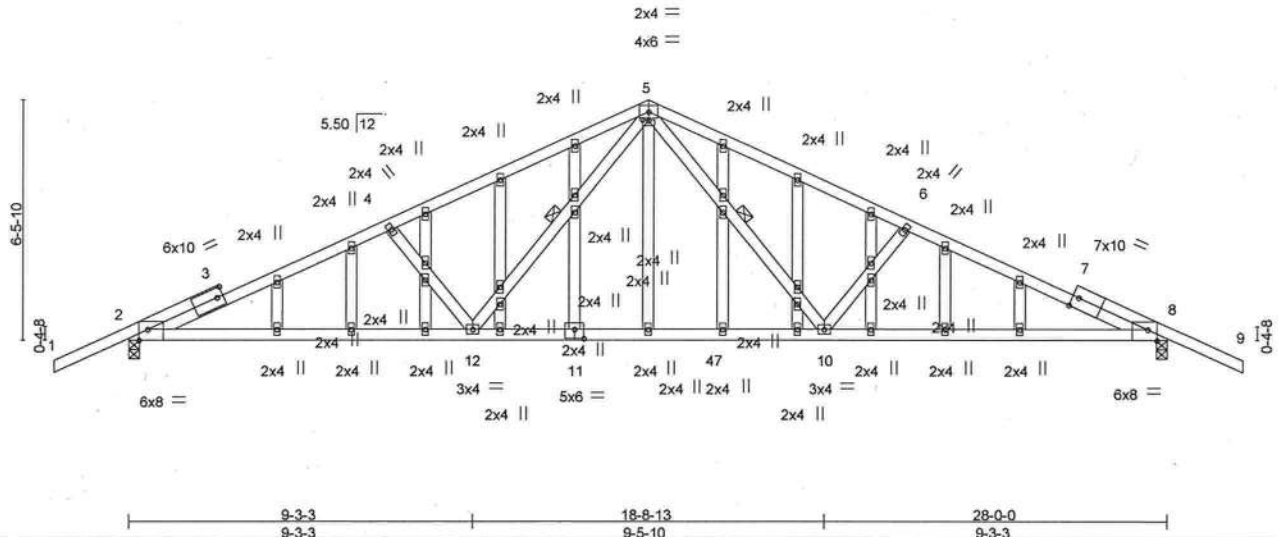


Plate Offsets (X,Y)-- [2:0-2-15,Edge], [5:0-2-0,0-0-4], [7:0-2-0,Edge], [8:0-2-15,Edge], [11:0-3-0,0-3-0]		9-3-3		18-8-13		28-0-0	
		9-3-3		9-5-10		9-3-3	
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d
TCLL 20.0	Plate Grip DOL	1.25	TC 0.91	Vert(LL)	0.41 10-12	>802	240
TCDL 7.0	Lumber DOL	1.25	BC 0.85	Vert(TL)	-0.64 10-12	>523	180
BCDL 0.0 *	Rep Stress Incr	YES	WB 0.34	Horz(TL)	0.07 8	n/a	n/a
BCDL 10.0	Code FBC2014/TPI2007		(Matrix-M)				
				Weight: 188 lb		FT = 20%	

LUMBER-
TOP CHORD 2x4 SP No.1 *Except*
1-3,7-9: 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-5-12 oc bracing.
WEBS 1 Row at midpt 5-10, 5-12

REACTIONS. (lb/size) 2=1141/0-3-8, 8=1141/0-3-8
Max Horz 2=147(LC 12)
Max Uplift 2=559(LC 8), 8=559(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1969/2335, 3-4=-1991/2390, 4-5=-1791/2301, 5-6=-1791/2301, 6-7=-1991/2390,
7-8=-1969/2335
BOT CHORD 2-12=-2071/1819, 11-12=-1149/1127, 11-47=-1149/1127, 10-47=-1149/1127,
8-10=-2092/1819
WEBS 5-10=-1043/694, 6-10=-447/503, 5-12=-1043/699, 4-12=-447/503

- NOTES-** (9)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=559, 8=559.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

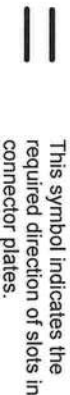
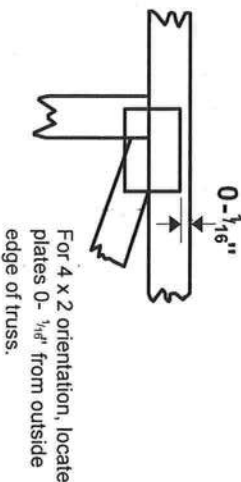
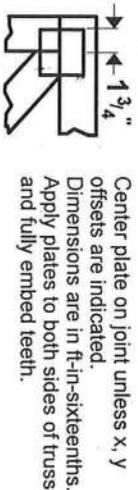
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 36610

Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

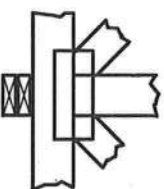
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



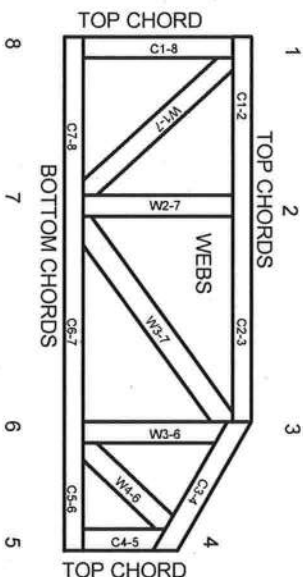
BEARING



Industry Standards:

ANSI/TP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 1 section 6.3 These truss designs rely on lumber values established by others.

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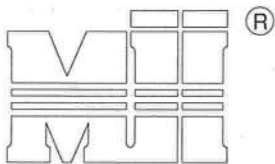


MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and ware at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.



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Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern

T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.

Note: Nail along entire length of T-Brace / I-Brace
(On Two-Ply's Nail to Both Plies)

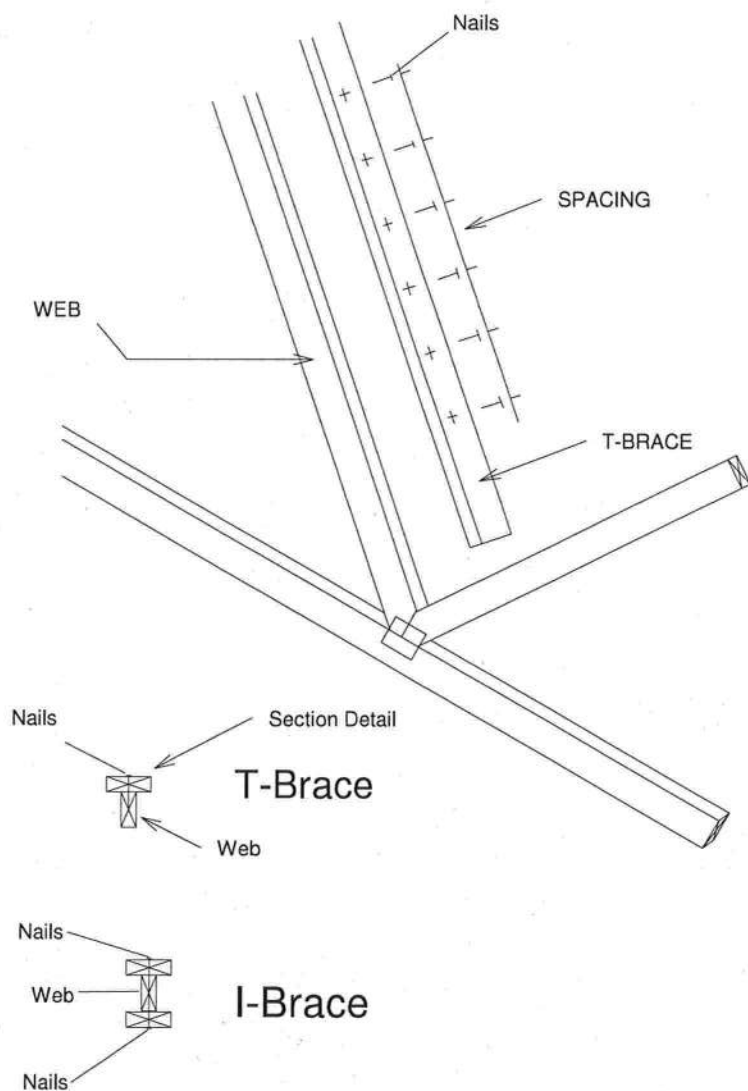
Brace Size
for One-Ply TrussSpecified Continuous
Rows of Lateral Bracing

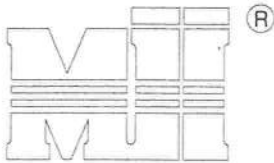
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size
for Two-Ply TrussSpecified Continuous
Rows of Lateral Bracing

Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.

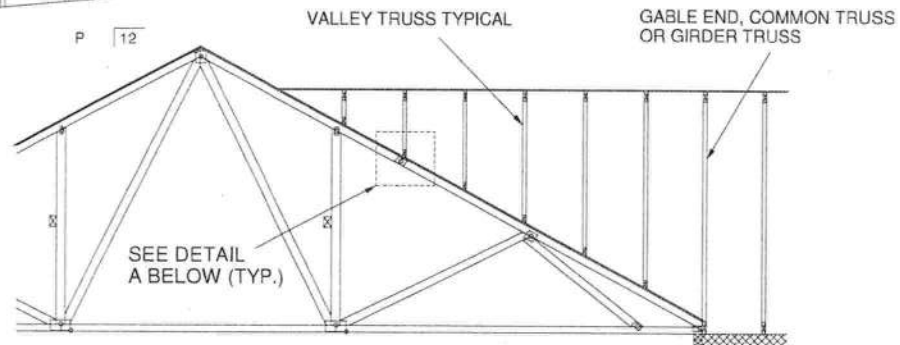
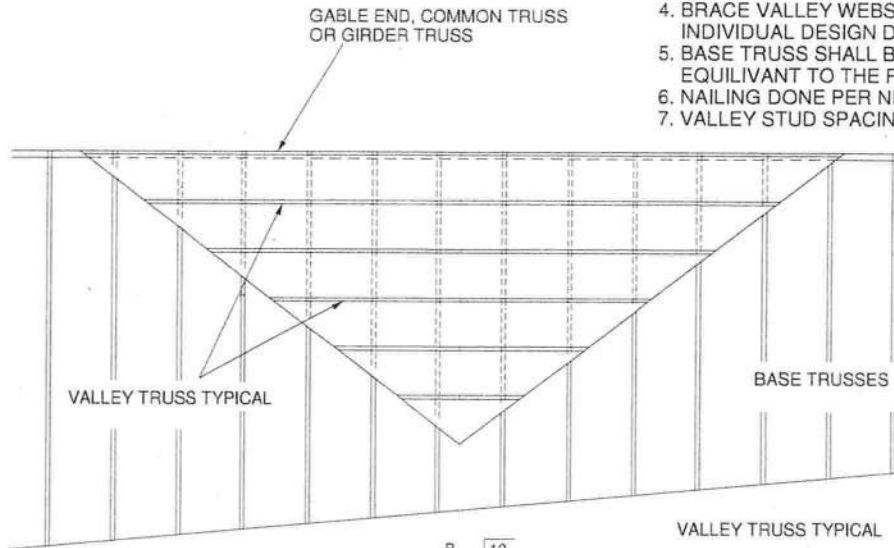




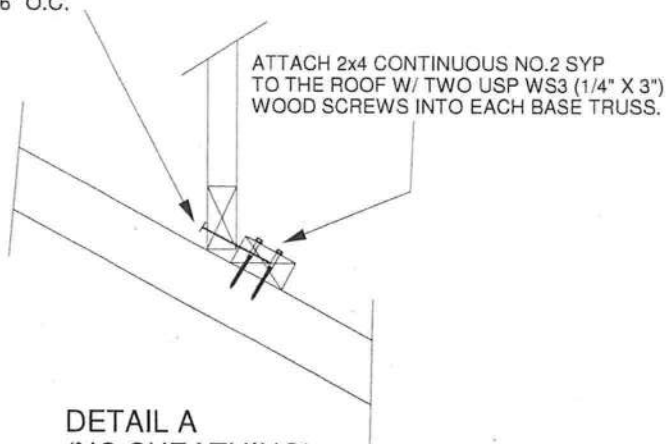
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GENERAL SPECIFICATIONS

1. NAIL SIZE = 3" X 0.131" = 10d
2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT
DO NOT USE DRYWALL OR DECKING TYPE SCREW
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
6. NAILING DONE PER NDS - 01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.

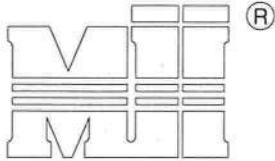


SECURE VALLEY TRUSS
W/ ONE ROW OF 10d
NAILS 6" O.C.



DETAIL A
(NO SHEATHING)
N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES



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Page 1 of 1

NOTES:

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.)
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

EXAMPLE:

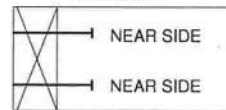
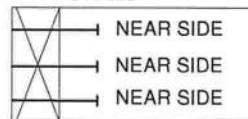
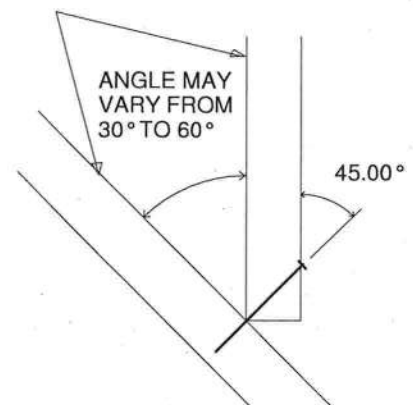
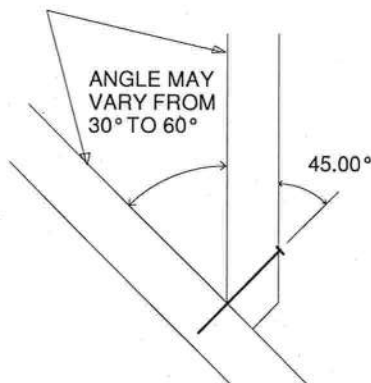
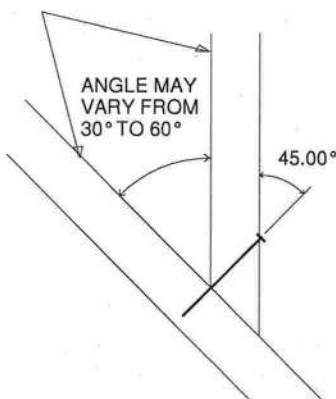
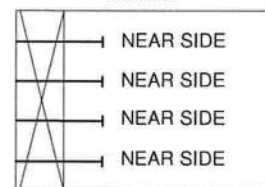
(3) - 16d NAILS (.162" diam. x 3.5") WITH SPF SPECIES BOTTOM CHORD

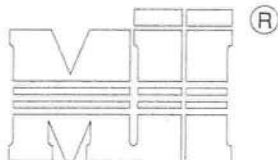
For load duration increase of 1.15:

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

SIDE VIEW
(2x3)
2 NAILSSIDE VIEW
(2x4)
3 NAILSSIDE VIEW
(2x6)
4 NAILS

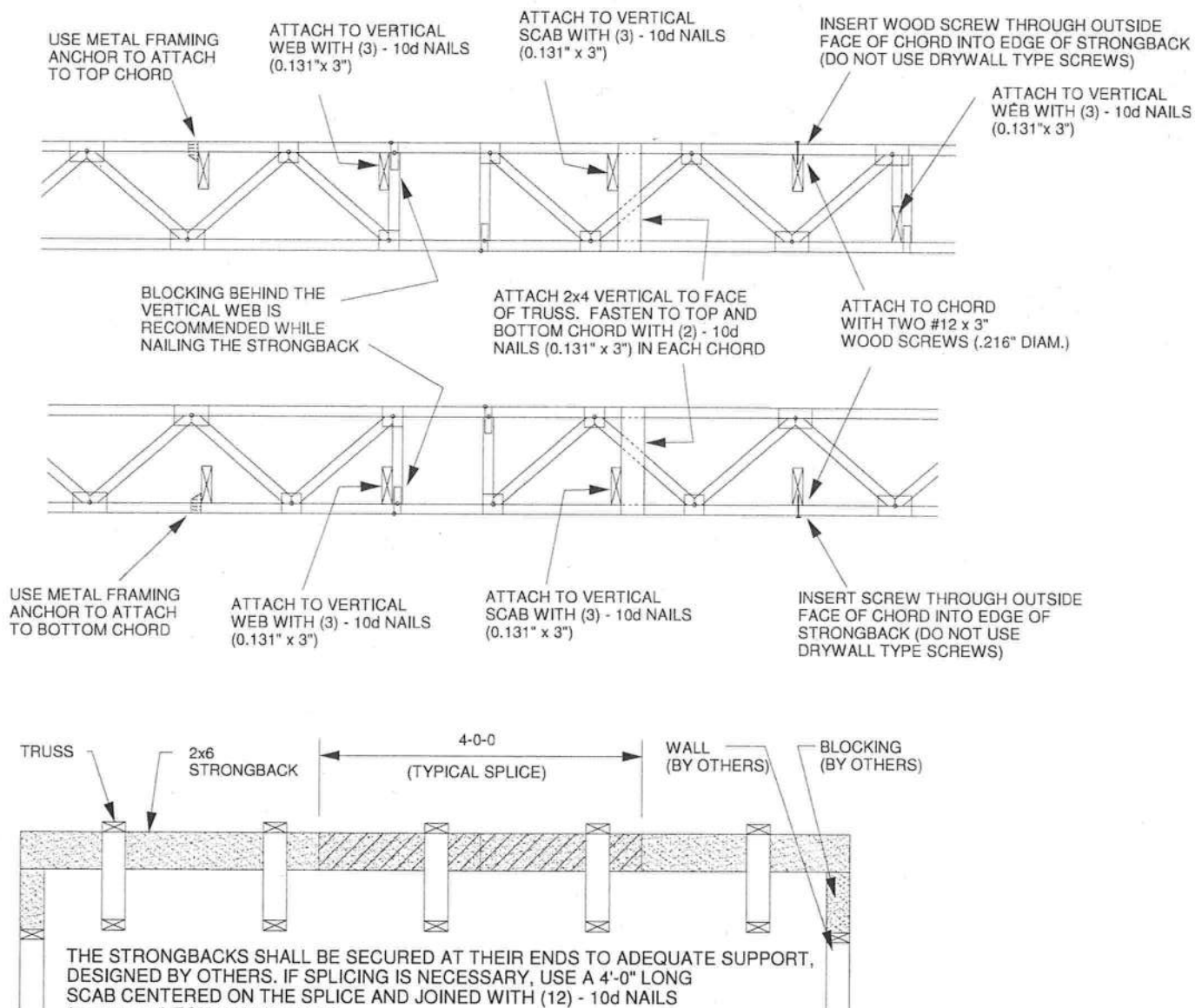


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TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.



THE STRONGBACKS SHALL BE SECURED AT THEIR ENDS TO ADEQUATE SUPPORT, DESIGNED BY OTHERS. IF SPLICING IS NECESSARY, USE A 4'-0" LONG SCAB CENTERED ON THE SPLICE AND JOINED WITH (12) - 10d NAILS (0.131" x 3") EQUALLY SPACED.

ALTERNATE METHOD OF SPLICING:

OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d NAILS (0.131" x 3") STAGGERED AND EQUALLY SPACED.

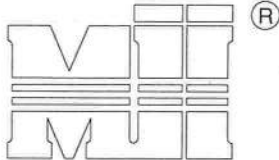
(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)

FEBRUARY 14, 2012

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10

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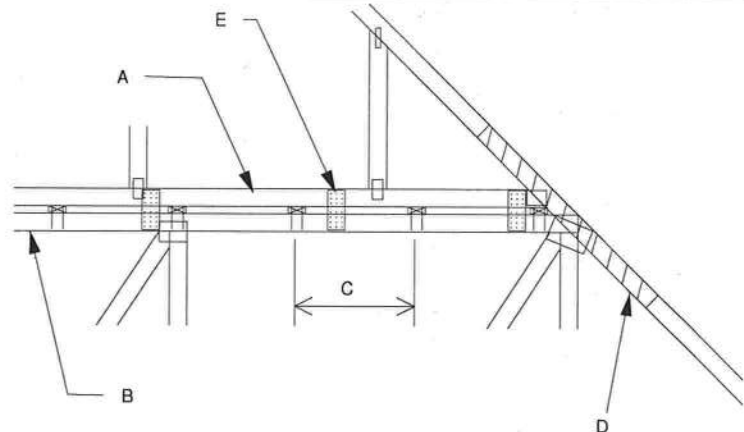


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MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24" O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

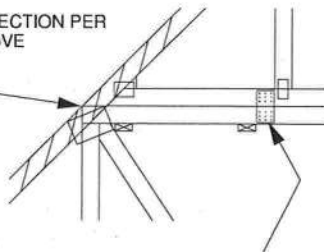
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED.
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.
- D - 2 X $\frac{1}{2}$ " X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



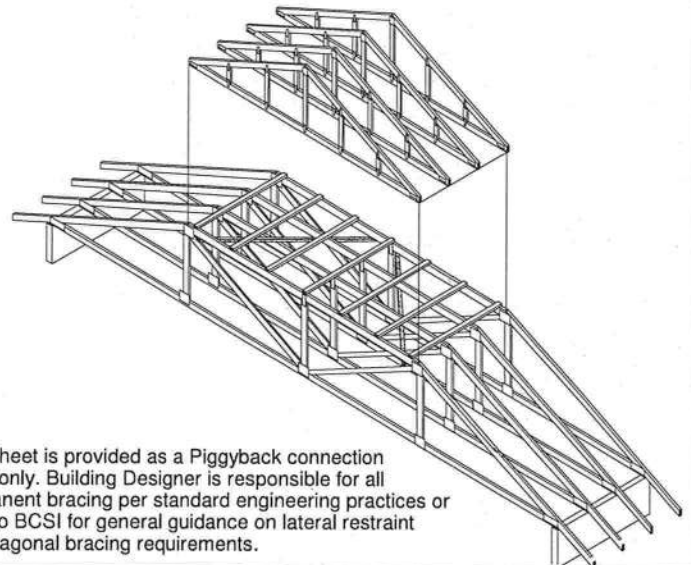
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER
NOTE D ABOVE

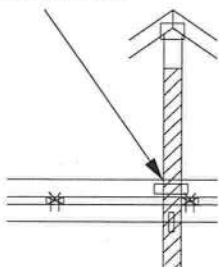


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



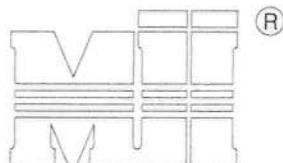
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED
TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x $\frac{1}{2}$ " x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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Typical 2x4 L-Brace Nailed To
2x Verticals W/10d Nails, 6" o.c.

Vertical Stud

SECTION B-B

TRUSS GEOMETRY AND CONDITIONS
SHOWN ARE FOR ILLUSTRATION ONLY.

12

Varies to Common Truss

SEE INDIVIDUAL MITTEK ENGINEERING
DRAWINGS FOR DESIGN CRITERIA

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST
TWO TRUSSES AS NOTED. TOENAIL BLOCKING
TO TRUSSES WITH (2) - 10d NAILS AT EACH END.
ATTACH DIAGONAL BRACE TO BLOCKING WITH
(5) - 10d COMMON WIRE NAILS.

(4) - 8d NAILS MINIMUM, PLYWOOD
SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

1'-3"
Max.

(2) - 10d

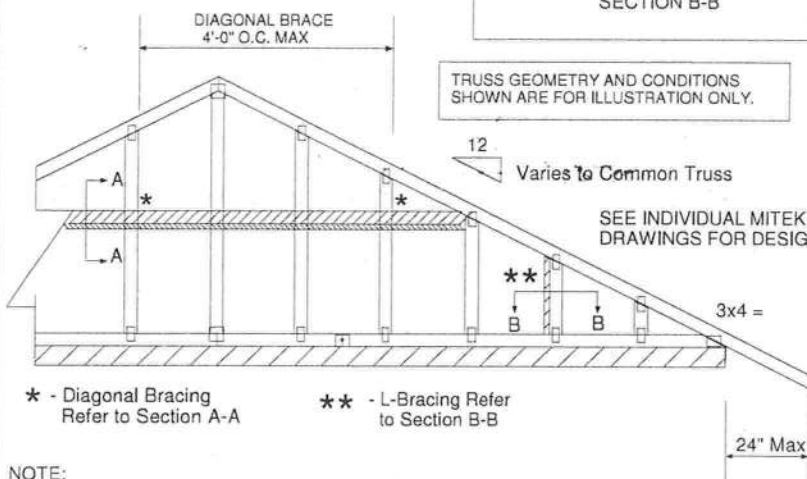
Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C.
ATTACHED TO VERTICAL WITH (4) - 16d
COMMON WIRE NAILS AND ATTACHED
TO BLOCKING WITH (5) - 10d COMMONS.

Diag. Brace
at 1/3 points
if needed

End Wall

HORIZONTAL BRACE
(SEE SECTION A-A)



* - Diagonal Bracing
Refer to Section A-A

** - L-Bracing Refer
to Section B-B

NOTE:

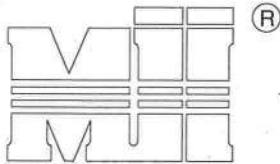
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS 1/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SPF Std/Stud	12" O.C.	4-0-7	4-3-2	6-0-4	8-0-15	12-1-6
2x4 SPF Std/Stud	16" O.C.	3-7-0	3-8-4	5-2-10	7-1-15	10-8-15
2x4 SPF Std/Stud	24" O.C.	2-11-1	3-0-2	4-3-2	5-10-3	8-9-4

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d common wire nails 8in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length.

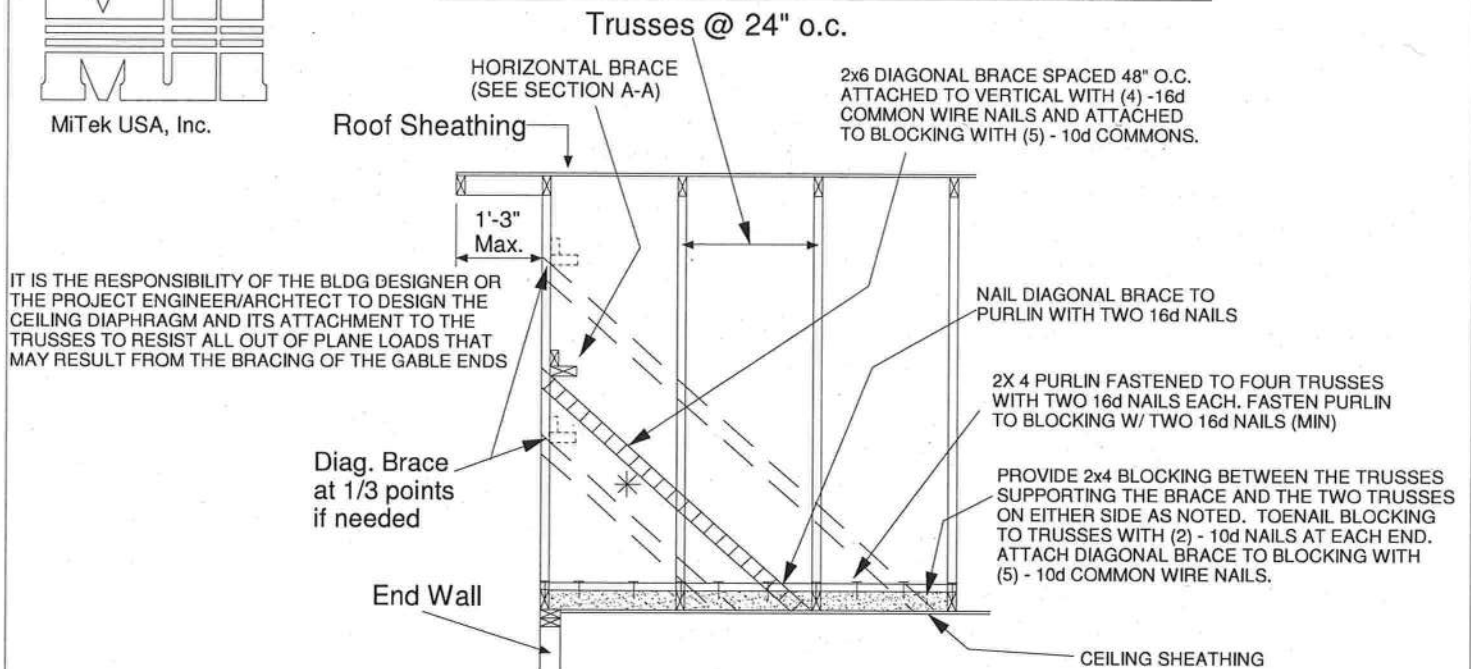
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

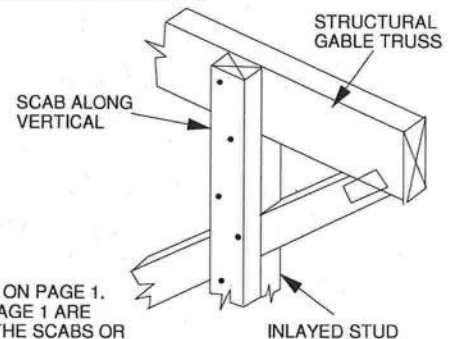
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:

METHOD 1 : ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

METHOD 2 : ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (.131" X 3") NAILS SPACED 6" O.C.
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)



MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1. ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.

STRUCTURAL GABLE TRUSS

AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

NOTE : THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAVED STUDS. TRUSSES WITHOUT INLAVED STUDS ARE NOT ADDRESSED HERE.

STANDARD GABLE TRUSS

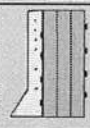
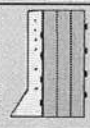
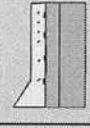
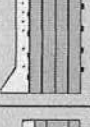
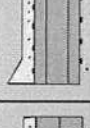
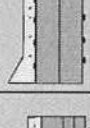
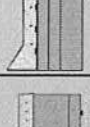
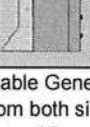
Multiple-Member Connections for Side-Loaded Beams with Concentrated Loads

Table 1: Maximum Concentrated Load Applied to Beam – Bolt or Nail Connection (lbs)⁽¹⁾

Beam Configuration		Hanger Type	$\frac{1}{2}$ " ϕ or $\frac{3}{4}$ " ϕ Bolts ⁽³⁾⁽⁴⁾					16d (0.148" x 3 $\frac{1}{4}$ ") Nails			
			Bolt Diameter ⁽²⁾	# Bolts				# Nails			
				2	4	6	8	6	12	18	24
3-ply 1 $\frac{1}{4}$ " (5 $\frac{1}{4}$ " Beam)		FACE	0.5	2,100	4,200	6,300	8,400	2,125 ⁽⁵⁾	4,250 ⁽⁵⁾	6,370 ⁽⁵⁾	8,495 ⁽⁵⁾
			0.75	3,060	6,120	9,180	12,240				
		TOP	0.5	760	1,525	2,285	3,050	1,060 ⁽⁶⁾	2,125 ⁽⁶⁾	3,185 ⁽⁶⁾	4,250 ⁽⁶⁾
			0.75	930	1,860	2,790	3,720				
3 $\frac{1}{2}$ " + 1 $\frac{1}{4}$ " (5 $\frac{1}{4}$ " Beam)		FACE	---	---	---	---	---	---	---	---	---
			---	---	---	---	---				
		TOP	0.5	1,050	2,100	3,150	4,200	1,060	2,125	3,185	4,250
			0.75	1,530	3,060	4,590	6,120				
3 $\frac{1}{2}$ " + 1 $\frac{1}{4}$ " (5 $\frac{1}{4}$ " Beam)		FACE / TOP	0.5	2,100	4,200	6,300	8,400	2,125 ⁽⁵⁾	4,250 ⁽⁵⁾	6,370 ⁽⁵⁾	8,495 ⁽⁵⁾
			0.75	3,060	6,120	9,180	12,240				
4-ply 1 $\frac{1}{4}$ " (7" Beam)		FACE	0.5	1,400	2,800	4,200	5,600	---	---	---	---
			0.75	2,040	4,080	6,120	8,160				
		TOP	0.5	675	1,355	2,030	2,710	---	---	---	---
			0.75	825	1,655	2,480	3,305				
2-ply 1 $\frac{1}{4}$ " + 3 $\frac{1}{2}$ " (7" Beam)		FACE	0.5	2,800	5,600	8,400	11,200	2,830 ⁽⁵⁾	5,665 ⁽⁵⁾	8,495 ⁽⁵⁾	11,330 ⁽⁵⁾
			0.75	5,025	10,050	15,070	20,095				
		TOP	0.5	935	1,865	2,800	3,735	945 ⁽⁶⁾	1,890 ⁽⁶⁾	2,830 ⁽⁶⁾	3,775 ⁽⁶⁾
			0.75	1,360	2,720	4,080	5,440				
2-ply 3 $\frac{1}{2}$ " (7" Beam)		FACE / TOP	0.5	1,720	3,440	5,160	6,880	---	---	---	---
			0.75	2,480	4,960	7,440	9,920				
2-ply 1 $\frac{1}{4}$ " + 3 $\frac{1}{2}$ " (7" Beam)		FACE / TOP	0.5	1,015	2,030	3,050	4,065	---	---	---	---
			0.75	1,240	2,480	3,720	4,960				
2-ply 1 $\frac{1}{4}$ " + 3 $\frac{1}{2}$ " (7" Beam)		FACE	0.5	1,720	3,440	5,160	6,880	---	---	---	---
			0.75	2,480	4,960	7,440	9,920				
		TOP	0.5	675	1,355	2,030	2,710	---	---	---	---
			0.75	825	1,655	2,480	3,305				
5 $\frac{1}{4}$ " + 1 $\frac{1}{4}$ " (7" Beam)		FACE / TOP	0.5	2,800	5,600	8,400	11,200	2,830 ⁽⁵⁾	5,665 ⁽⁵⁾	8,495 ⁽⁵⁾	11,330 ⁽⁵⁾
			0.75	5,025	10,050	15,070	20,095				

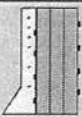
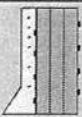
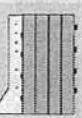
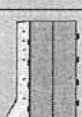
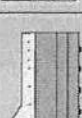
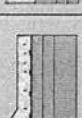
1. See page 4 for table General Notes, connection details and beam depth limitations.
2. Washers required. Bolt holes to be $\frac{9}{16}$ " maximum for $\frac{1}{2}$ " bolts, $\frac{13}{16}$ " maximum for $\frac{3}{4}$ " bolts.
3. Minimum end distance for bolts is 6".
4. Drilling bolt holes reduces the section of the beam and thus the load capacity of the supporting member. Shear and moment capacity of the beam must be checked at the location of each row of bolts.
5. Number of required nails shown must be installed from side opposite hanger.
6. Number of required nails shown must be installed from hanger side. Additionally, install half the number of required nails from side opposite hanger.

Table 2A: Maximum Concentrated Load Applied to Beam – Wood Screw Connection (lbs)⁽¹⁾

Beam Configuration		Hanger Type	Wood Screw Length	Fastener Type							
				USP WS				Simpson SDS			
				# Screws				# Screws			
				2	4	6	8	2	4	6	8
3-ply 1¼" (5¼" Beam)		FACE ⁽³⁾	3½"	1,435	2,870	4,305	5,740	2,040	4,080	6,120	8,160
		TOP ⁽²⁾	3½"	720	1,435	2,155	2,870	1,020	2,040	3,060	4,080
3½"+1¼" (5¼" Beam)		FACE	3½"	---	---	---	---	---	---	---	---
		TOP ⁽⁴⁾	3½"	720	1,435	2,155	2,870	1,020	2,040	3,060	4,080
3½"+1¼" (5¼" Beam)		FACE / TOP ⁽³⁾	3½"	1,435	2,870	4,305	5,740	2,040	4,080	6,120	8,160
4-ply 1¼" (7" Beam)		FACE ⁽²⁾	6" ⁽⁵⁾	1,055	2,110	3,165	4,220	1,360	2,720	4,080	5,440
		TOP ⁽²⁾	6" ⁽⁵⁾	705	1,405	2,110	2,810	905	1,815	2,720	3,625
2-ply 1¼" + 3½" (7" Beam)		FACE ⁽³⁾	3½"	1,915	3,825	5,740	7,655	2,720	5,440	8,160	10,880
			6" ⁽⁵⁾	2,110	4,220	6,325	8,435	2,720	5,440	8,160	10,880
		TOP ⁽²⁾	3½"	640	1,275	1,915	2,550	905	1,815	2,720	3,625
			6" ⁽⁵⁾	705	1,405	2,110	2,810	905	1,815	2,720	3,625
2-ply 3½" (7" Beam)		FACE / TOP ⁽²⁾	6" ⁽⁵⁾	1,055	2,110	3,165	4,220	930	1,860	2,785	3,715
2-ply 1¼" + 3½" (7" Beam)		FACE / TOP ⁽³⁾	6" ⁽⁵⁾	1,055	2,110	3,165	4,220	1,360	2,720	4,080	5,440
2-ply 1¼" + 3½" (7" Beam)		FACE ⁽⁴⁾	6" ⁽⁵⁾	1,055	2,110	3,165	4,220	930	1,860	2,785	3,715
		TOP ⁽⁴⁾	6" ⁽⁵⁾	705	1,405	2,110	2,810	905	1,815	2,720	3,625
5¼" + 1¼" (7" Beam)		FACE / TOP ⁽³⁾	3½"	1,915	3,825	5,740	7,655	2,720	5,440	8,160	10,880

1. See page 4 for table General Notes, connection details and beam depth limitations.
2. Install screws from both sides of beam.
3. Install screws from side opposite hanger only.
4. Install screws from hanger side only.
5. 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

Table 2B: Maximum Concentrated Load Applied to Beam – Wood Screw Connection (lbs)⁽¹⁾

Beam Configuration		Hanger Type	Wood Screw Length	Connection Name			
				TrussLok			
				# Screws			
				2	4	6	8
3-ply 1 $\frac{3}{4}$ " (5 $\frac{1}{4}$ " Beam)		FACE ⁽³⁾	3 $\frac{3}{8}$ "	1,600	3,205	4,805	6,410
		TOP ⁽²⁾	3 $\frac{3}{8}$ "	800	1,600	2,405	3,205
3 $\frac{1}{2}$ " + 1 $\frac{3}{4}$ " (5 $\frac{1}{4}$ " Beam)		FACE	3 $\frac{3}{8}$ "	---	---	---	---
		TOP ⁽⁴⁾	3 $\frac{3}{8}$ "	800	1,600	2,405	3,205
3 $\frac{1}{2}$ " + 1 $\frac{3}{4}$ " (5 $\frac{1}{4}$ " Beam)		FACE / TOP ⁽³⁾	3 $\frac{3}{8}$ "	1,600	3,205	4,805	6,410
4-ply 1 $\frac{3}{4}$ " (7" Beam)		FACE ⁽²⁾	5", 6 $\frac{3}{4}$ "	1,160	2,320	3,480	4,640
		TOP ⁽²⁾	5", 6 $\frac{3}{4}$ "	775	1,545	2,320	3,095
2-ply 1 $\frac{3}{4}$ " + 3 $\frac{1}{2}$ " (7" Beam)		FACE ⁽³⁾	3 $\frac{3}{8}$ "	2,135	4,270	6,410	8,545
			5", 6 $\frac{3}{4}$ "	2,320	4,640	6,960	9,280
		TOP ⁽²⁾	3 $\frac{3}{8}$ "	710	1,425	2,135	2,850
			5", 6 $\frac{3}{4}$ "	775	1,545	2,320	3,095
2-ply 3 $\frac{1}{2}$ " (7" Beam)		FACE / TOP ⁽²⁾	5", 6 $\frac{3}{4}$ "	1,160	2,320	3,480	4,640
2-ply 1 $\frac{3}{4}$ " + 3 $\frac{1}{2}$ " (7" Beam)		FACE / TOP ⁽³⁾	5", 6 $\frac{3}{4}$ "	1,160	2,320	3,480	4,640
2-ply 1 $\frac{3}{4}$ " + 3 $\frac{1}{2}$ " (7" Beam)		FACE ⁽⁴⁾	5", 6 $\frac{3}{4}$ "	1,160	2,320	3,480	4,640
		TOP ⁽⁴⁾	5", 6 $\frac{3}{4}$ "	1,160	2,320	3,480	4,640
5 $\frac{1}{4}$ " + 1 $\frac{3}{4}$ " (7" Beam)		FACE / TOP ⁽³⁾	5", 6 $\frac{3}{4}$ "	2,320	4,640	6,960	9,280

1. See page 4 for table General Notes, connection details and beam depth limitations.
2. Install screws from both sides of beam.
3. Install screws from side opposite hanger only.
4. Install screws from hanger side only.

Table 1, 2A, 2B General Notes

- Connections are based on NDS[®] 2005 or manufacturer's code report.
- All plies must be the same material and grade.
- Values are for 100% duration of load. Increase 15% for snow load or 25% for non-snow roof load conditions, where code allows.
- Rotational effects should be considered for 7" wide beams loaded from one side only.
- Capacities shown for face mount hanger conditions are based on 16d common (0.162" x 3 1/2") nails installed in the hanger. Other nails used for face mount hanger installations invalidate the capacities in these tables.
- Verify adequacy of beam with all loads applied by using iLevel software or other methods.
- See the iLevel Trus Joist[®] Beams, Headers, and Columns Specifier's Guide (#TJ-9000) for required connections for uniform side loads.

Bolt or Wood Screw Connection Details

- 1/2" bolts - 9/16" ϕ maximum holes
- 3/4" bolts - 13/16" ϕ maximum holes
- Lead holes not required for wood screws

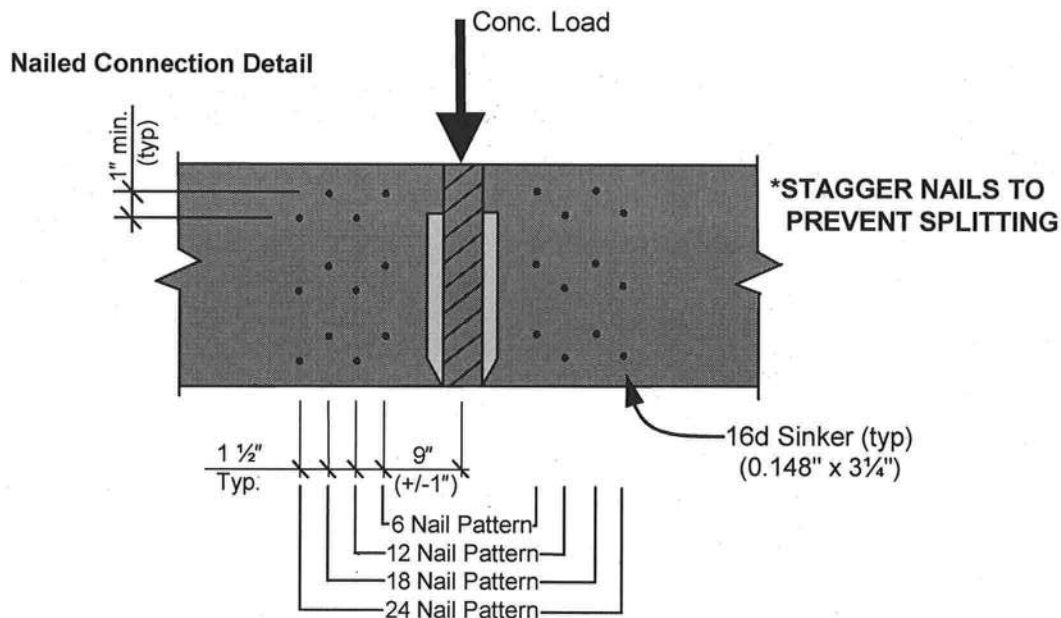
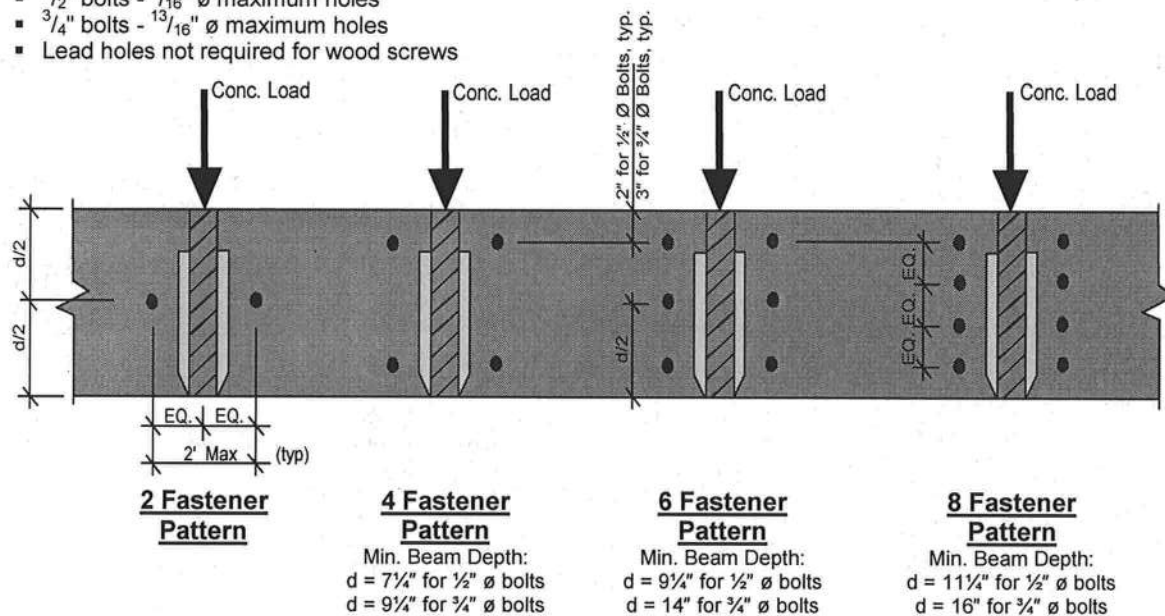


Table 3: Allowable Capacities of 5/4" Beams with Bolt Connections

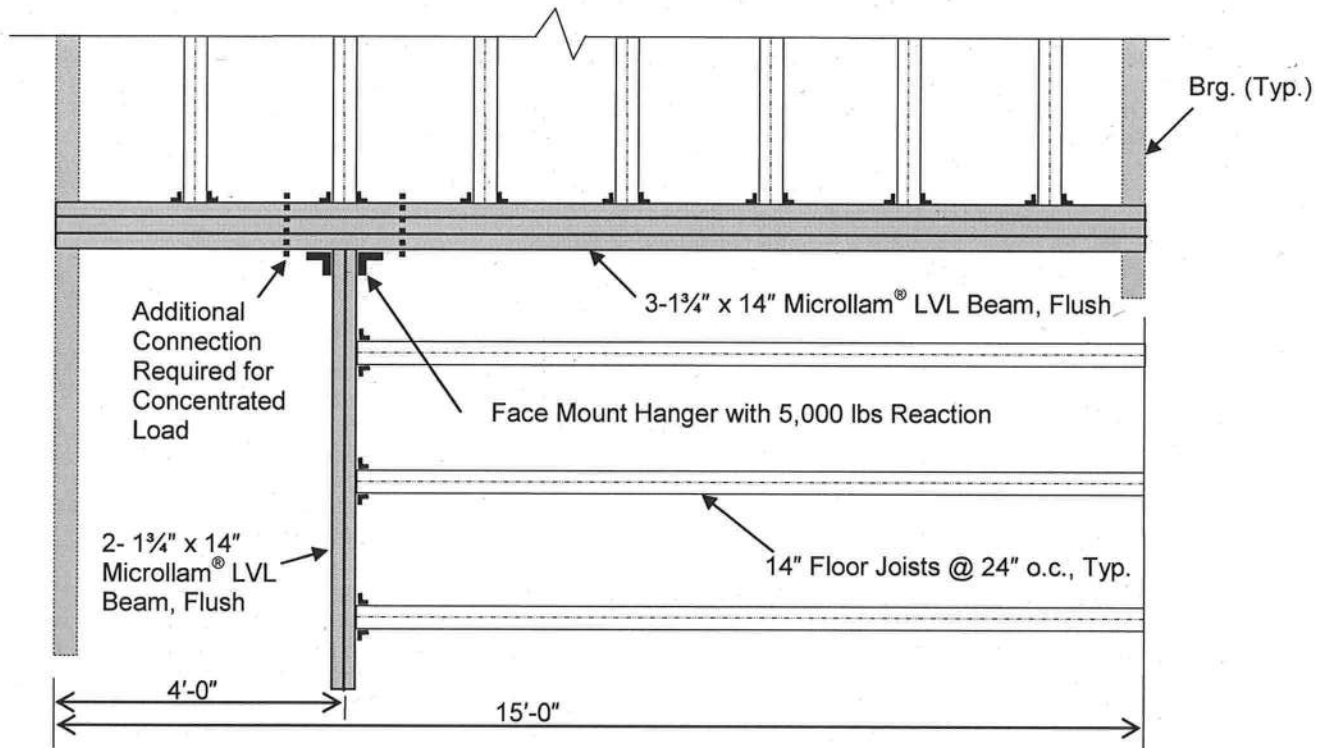
		Bolt Dia.	Conn. Loc.	Beam Depth									
				7¼"	9¼"	9½"	11¼"	11½"	14"	16"	18"	20"	24"
TimberStrand® LSL	Max Shear (lbs)	½"	< 5d		6,235	6,545	8,755	9,560	12,345	15,005			
			≥ 5d		10,150	10,500	12,950	13,825	16,800	19,600			
		¾"	< 5d		3,995	4,260	6,210	6,940	9,510	12,015			
			≥ 5d		8,750	9,100	11,550	12,425	15,400	18,200			
	Max Moment (ft-lb)	½"	2		10,605	15,295	15,685	23,905	32,760	42,265			
			4		9,355	13,470	13,730	20,915	28,725	37,250			
			6		9,350	13,470	13,730	20,910	28,725	37,245			
			8				13,510	20,575	28,275	36,685			
		¾"	2		10,600	15,290	15,685	23,900	32,755	42,260			
			4		9,900	14,215	14,195	21,490	29,020	37,225			
			6						29,015	37,220			
			8						36,655				
Microllam® LVL	Max Shear (lbs)	½"	< 5d	2,745	4,445	4,665	6,240	6,810	8,795	10,690	12,610	14,545	18,440
			≥ 5d	5,235	7,230	7,480	9,225	9,850	11,970	13,965	15,960	17,955	21,945
		¾"	< 5d	1,455	2,845	3,035	4,425	4,945	6,775	8,560	10,390	12,250	16,040
			≥ 5d	4,240	6,235	6,485	8,230	8,855	10,975	12,970	14,965	16,960	20,950
	Max Moment (ft-lb)	½"	2	9,960	16,220	17,105	23,990	26,730	36,385	46,670	58,130	70,740	99,375
			4	9,025	14,305	15,065	21,000	23,385	31,905	41,130	51,535	63,100	89,670
			6		14,305	15,060	20,995	23,385	31,905	41,125	51,530	63,100	89,670
			8				20,660	23,010	31,405	40,510	50,795	62,250	88,590
		¾"	2	9,950	16,210	17,100	23,985	26,725	36,380	46,665	58,125	70,740	99,370
			4		15,145	15,895	21,710	24,030	32,235	41,105	51,120	62,285	88,015
			6						32,230	41,100	51,115	62,280	88,010
			8						40,475	50,335	61,335	86,745	
Parallam® PSL	Max Shear (lbs)	½"	< 5d		4,520	4,745	6,350	6,930	8,950	10,880	12,830		
			≥ 5d		7,360	7,615	9,390	10,025	12,180	14,210	16,240		
		¾"	< 5d		2,895	3,090	4,505	5,030	6,895	8,710	10,575		
			≥ 5d		6,345	6,600	8,375	9,010	11,165	13,195	15,225		
	Max Moment (ft-lb)	½"	2		18,090	19,080	26,760	29,815	40,740	52,430	65,495		
			4		15,960	16,805	23,420	26,085	35,725	46,205	58,065		
			6		15,955	16,800	23,420	26,080	35,725	46,205	58,060		
			8				23,045	25,665	35,165	45,510	57,235		
		¾"	2		18,080	19,070	26,755	29,810	40,735	52,425	65,490		
			4		16,890	17,730	24,215	26,805	36,095	46,180	57,600		
			6						36,085	46,170	57,595		
			8						45,470	56,715			

Table 3 General Notes

- Connection location refers to distance from face of bearing to centerline of connection, where d is the beam depth. Less than 5d refers to connections located less than five times the depth of the beam from the face of bearing.
- Shear reduction has been taken in accordance with NDS 3.4.3.3.
- All values shown are for 5/4" widths. Multiply by 1.33 for 7" wide beams.
- 9 1/4" and 11 1/4" TimberStrand® LSL are 1.3E grade, all other depths are 1.55E grade.

Connection Design Example

Given the floor framing shown below, specify the necessary connections for the a 3-ply Microllam® LVL beam supporting a 2-ply Microllam® LVL beam on one side and floor joists from the other side. The floor joists apply a 500 plf load to the side of the beam while the 2-ply beam reaction is 5,000 lbs. See the following page for the solution.



Partial Framing Plan

Solution:

- Use the iLevel Trus Joist[®] Beams, Headers, and Columns Specifier's Guide (#TJ-9000) to select a connection for the uniform loading. Multiple options exist that meet or exceed the 500 plf requirement:
 - 2 rows of ½" diameter bolts at 16" on-center (570 plf)
 - 2 rows of USP WS35 wood screws at 16" on-center (540 plf) on each side of beam
 - 2 rows of SDS ¼" x 3½" at 24" on-center (510 plf) on each side of beam
 - 2 rows of 5" TrussLok wood screws at 24" on-center (500 plf)

- Determine what additional connection is needed for the concentrated load using the information in this Technical Bulletin:

1. Using Table 1, 2A or 2B of this bulletin, locate the section for a 3-ply 1¼" member and face mount hanger. Again, multiple options exist that exceed the 5,000 lb load requirement:
 - ½" diameter bolts in a 6-bolt connection (6,300 lbs)
 - 16d (0.148" x 3¼") sinker nails in an 18-nail connection (6,370 lbs)
 - 3½" WS wood screws in an 8-screw connection (5,740 lbs)
 - 3½" SDS wood screws in a 6-screw connection (6,120 lbs)
 - 3¾" TrussLok wood screws in an 8-screw connection (6,410 lbs)

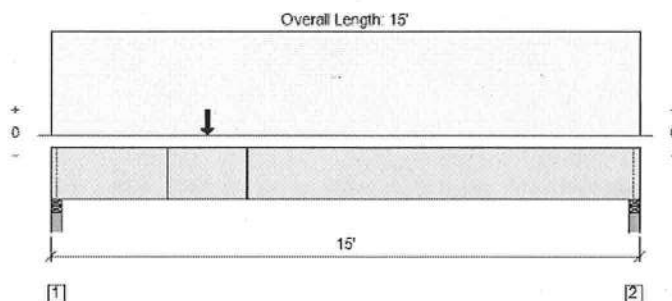
Note that the allowable applied concentrated load is dependent upon the type of hanger used and 16d common nails must be used for face mount hanger installation to achieve the capacities shown.

2. If the bolted connection option is chosen, the 3-ply beam must be checked for the effect of the bolt holes from the 6-bolt connection. This task may be accomplished by utilizing the location analysis feature in iLevel software. Per the detail on page 4, the bolt holes will be located approximately 1' to either side of the concentrated load location. After inputting the span and load information, choose the two locations corresponding to the concentrated load location (±) 1'. Upon completion of the analysis, compare the calculated shear and moment for each of the two locations against the reduced allowable shear and moment capacities of the beam for the bolt pattern selected using Table 3 on page 5 of this bulletin.

See the Forte[®] software output on the following page for results of this analysis. The 3-ply Microllam[®] LVL beam works for the given loading. Refer to the highlighted values for the results at both bolt locations (1' on either side of the load). The highest calculated shear at either location is 6035 lbs. The highest calculated moment is 25,215 ft-lbs. The connection is less than 5d (48" < 14" * 5 = 70") from the end of the member. From Table 3 of this bulletin, the reduced allowable shear for the 3-ply 14" Microllam[®] LVL beam with the 6 bolt pattern located less than 5d from the end of the member is 8,795 lbs while the reduced allowable moment is 31,905 ft-lbs. Therefore, the beam is acceptable with the 6 bolt connection.

- Select the preferred options for both the uniform load connection along the length of the beam and the concentrated load connection at the hanger location and specify both on the drawings including information on fastener type, size, spacing and installation pattern as appropriate.

3 PIECE(S) 1 3/4" x 14" 1.9E Microllam® LVL PASSED



All Dimensions are Horizontal; Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF
Member Reaction (lbs)	7595 @ 2"	7809	Passed (97%)	--
Shear (lbs)	6837 @ 1' 5 1/2"	13965	Passed (49%)	1.0
Moment (Ft-lbs)	25215 @ 4' 11 7/8"	36387	Passed (69%)	1.0
Live Load Defl. (in)	0.356 @ 7' 1 13/16"	0.489	Passed (L/494)	--
Total Load Defl. (in)	0.456 @ 7' 1 7/8"	0.733	Passed (L/386)	--

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Design results assume a fully braced condition where all compression edges (top and bottom) are properly braced to provide lateral stability.
- Bracing (Lu): All compression edges (top and bottom) must be braced at 13' 15/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Total Bearing	Available Bearing	Required Bearing	Support Reactions (lbs) Dead / Floor / Roof / Snow	Accessories
1 - Stud wall - Spruce Pine Fir	3.50"	3.50"	3.40"	1641 / 5955 / 0 / 0	Blocking
2 - Stud wall - Spruce Pine Fir	3.50"	3.50"	2.33"	1164 / 4045 / 0 / 0	Blocking

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform (PSF)	0 to 15'	1'	100.0	400.0	0.0	0.0	10' Floor
2 - Point (lb)	4'	N/A	1000	4000	0	0	

Location Analysis	Shear (lbs) Actual / Allowed / LDF	Moment (Ft - lbs) Actual / Allowed / LDF	Comments
1 - 3'	6035 / 13965 / 1.0	19186 / 36387 / 1.0	BOLTS LEFT OF HANGER
2 - 5'	39 / 12569 / 0.9	25215 / 36387 / 1.0	BOLTS RIGHT OF HANGER

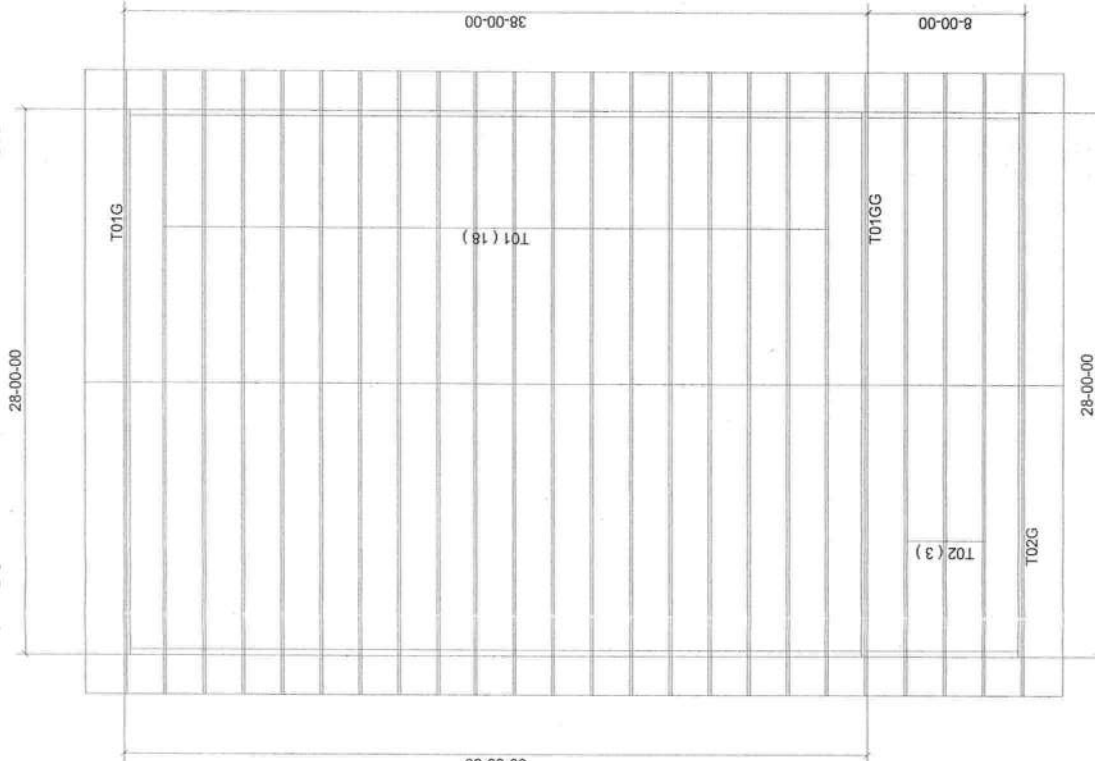
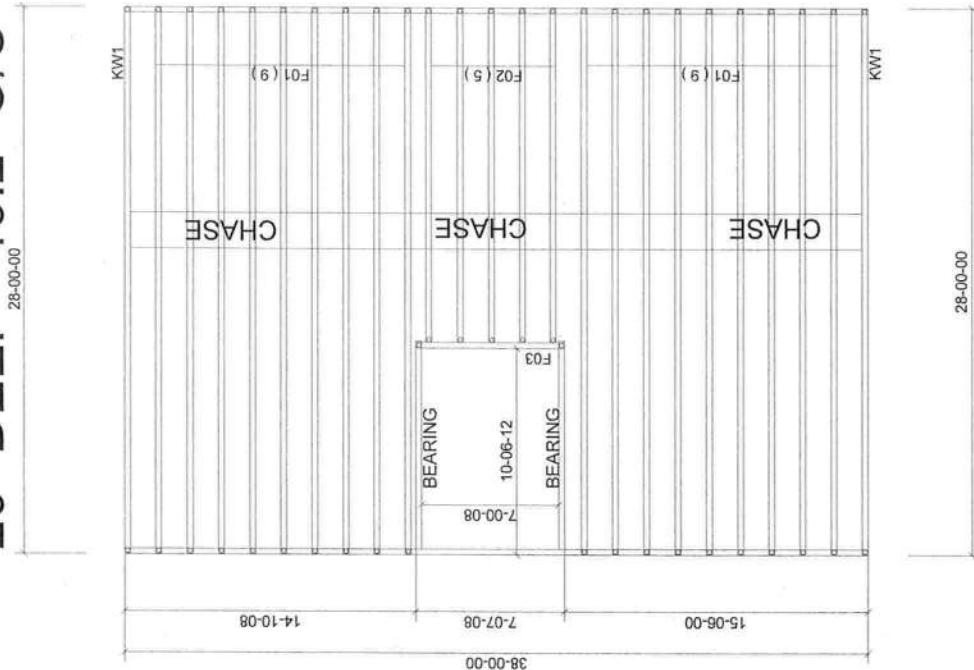
Forte™ Software Operator	Job Notes

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Page 1 of 1

20" DEEP - 19.2" O/C

5.5/12 PITCH - 24" O/H



NOTES:

- 1) REFER TO 20" DEEP RAFTERS FOR ALL DIMENSIONS AND SPACING. RAFTERS TO BE SPACED AT 19.2" O/C FOR PERMANENT BEARING.
- 2) ALL RAFTERS INCLUDING TRUSSES MUST BE SPACED AT 19.2" O/C FOR PERMANENT BEARING. RAFTERS MUST BE SPACED AT 19.2" O/C FOR PERMANENT BEARING.
- 3) ALL RAFTERS ARE TO BE CONCENTRICALLY PLACED AT 19.2" O/C.
- 4) ALL RAFTERS ARE TO BE SPACED AT 19.2" O/C.
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- 20) ALL RAFTERS ARE TO BE SPACED AT 19.2" O/C.



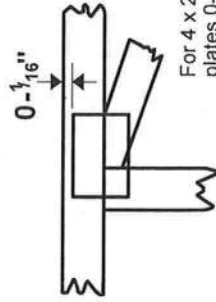
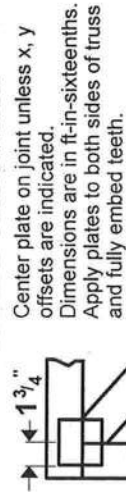
Builders FirstSource
 Jacksonville
 PHONE: 904-772-8000 FAX: 904-772-8073
 Tampa
 PHONE: 813-427-8031 FAX: 813-428-8954
 Freeport
 PHONE: 850-875-4541 FAX: 850-875-4875

MIKE TODD CONST.	
TRADE:	MORGAN HOUSE
DATE:	2-2-16
BY:	KLH
PROJECT:	755227
LOCATION:	755227

MITEK PLATE APPROVAL #'s 2197.2 - 2197.4, WEYERHAUSER PRODUCT #'s 1630.2 - 1630.10

Symbols

PLATE LOCATION AND ORIENTATION



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

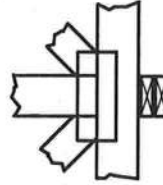
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



BEARING

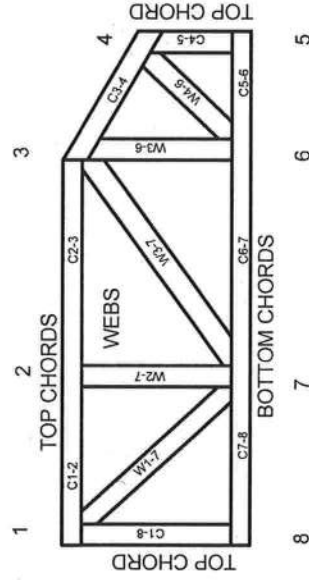


Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS
ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor 1 bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: 755227 - MIKE TODD CONST. - MORGAN RES.

MiTek USA, Inc.

6904 Parke East Blvd.
Tampa, FL 33610-4115

Site Information:

Customer Info: Mike Todd Const. Project Name: 755227 Model: Morgan Res.
Lot/Block: Subdivision:
Address: 218 SW Riverside Ave.
City: Columbia Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: Unknown at time of seals License #: Unknown at time of seals
Address: Unknown at time of seals
City: Unknown at time of seals State: Unknown at time of seals

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2014/TPI2007 Design Program: MiTek 20/20 7.6
Wind Code: ASCE 7-10 Wind Speed: 130 mph
Roof Load: 37.0 psf Floor Load: 55.0 psf

This package includes 15 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T10408273	F01	2/9/17
2	T10408274	F02	2/9/17
3	T10408275	F03	2/9/17
4	T10408276	F04	2/9/17
5	T10408277	F05	2/9/17
6	T10408278	F06	2/9/17
7	T10408279	F07	2/9/17
8	T10408280	F08	2/9/17
9	T10408281	F09	2/9/17
10	T10408282	KW1	2/9/17
11	T10408283	T01	2/9/17
12	T10408284	T01G	2/9/17
13	T10408285	T01GG	2/9/17
14	T10408286	T02	2/9/17
15	T10408287	T02G	2/9/17

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2019.

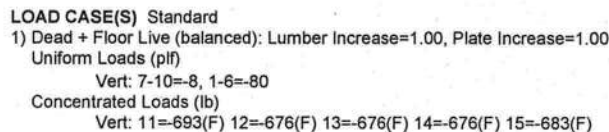
IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek. Any project specific information included is for MiTek's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Joaquin Velez PE No. 68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 9, 2017

7.640 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 09 16:09:54 2017 Page 1
ID:Awd7fSKXwlnKYwWX1sWuKPzYL70-LthNyKSvFADrytg0rBFi59gvPBGLCezKpqlIC?zmqjR



Job 755227	Truss F02	Truss Type Floor	Qty 5	Ply 1	MIKE TODD CONST. - MORGAN RES.	T10408274
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.640 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 09 16:09:54 2017 Page 1
ID:AwD7fSKXwlnKYwWX1sWuKPzYL70-LthNyKSvFADRYtgOrBFi59gtXB9XChfKpqlIC7zmjR

0-1-8

1-3-0

1-10-0

0-8-0
Scale = 1:29.2

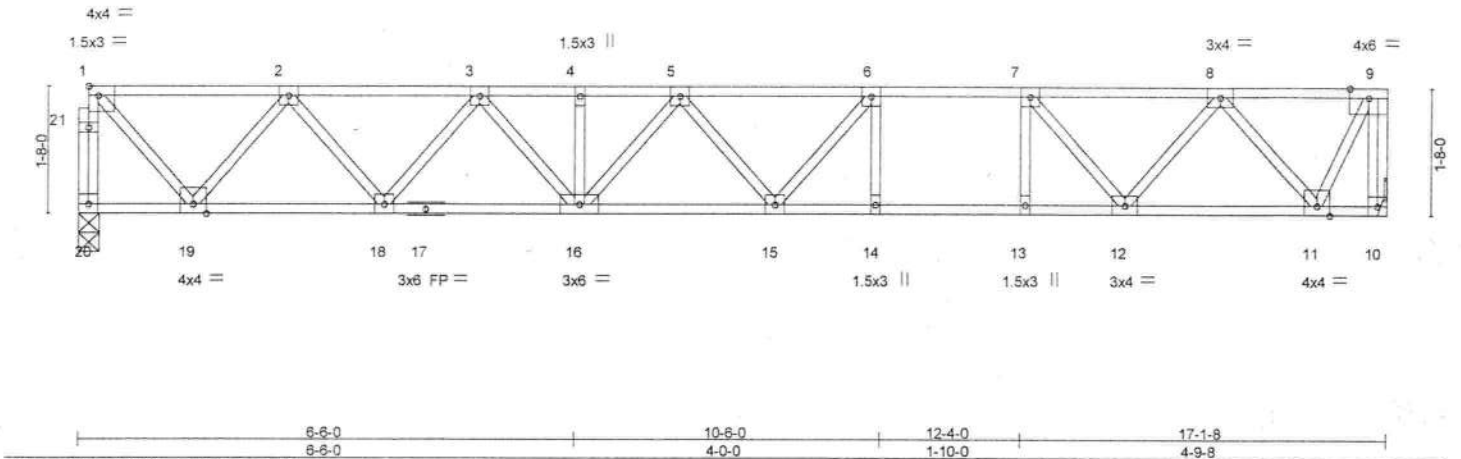


Plate Offsets (X,Y)-- [1:Edge,0-1-8]		6-6-0		10-6-0		12-4-0		17-1-8	
		8-6-0		4-0-0		1-10-0		4-9-8	
LOADING (psf)	SPACING-	1-7-2	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.34	Vert(LL)	-0.18 14-15	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.00	BC 0.60	Vert(TL)	-0.28 14-15	>736	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.41	Horz(TL)	0.03 10	n/a	n/a		
BCDL 5.0	Code FBC2014/TPI2007		(Matrix)						
				Weight: 98 lb				FT = 11%F, 11%E	

LUMBER-

TOP CHORD 2x4 SP M 31(flat)
BOT CHORD 2x4 SP M 31(flat)
WEBS 2x4 SP No.3(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 20=735/0-3-4, 10=740/Mechanical

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 20-21=-730/0, 1-21=-730/0, 9-10=-751/0, 1-2=-582/0, 2-3=-1420/0, 3-4=-1899/0, 4-5=-1899/0, 5-6=-1899/0,
6-7=-1783/0, 7-8=-1259/0, 8-9=-348/0
BOT CHORD 18-19=0/1096, 17-18=0/1724, 16-17=0/1724, 15-16=0/2056, 14-15=0/1783, 13-14=0/1783, 12-13=0/1783, 11-12=0/863
WEBS 6-14=-323/0, 7-13=0/348, 1-19=0/852, 2-19=-816/0, 2-18=0/515, 3-18=-483/0, 3-16=0/270, 6-15=-49/416,
9-11=0/761, 8-11=-817/0, 8-12=0/629, 7-12=-814/0

NOTES- (7)

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x3 MT20 unless otherwise indicated.
- 3) Refer to girder(s) for truss to truss connections.
- 4) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job 755227	Truss F04	Truss Type FLOOR	Qty 9	Ply 1	MIKE TODD CONST. - MORGAN RES. Job Reference (optional)	T10408276
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Builders FirstSource, Lake City, FL 32055

7.640 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 09 16:09:55 2017 Page 1
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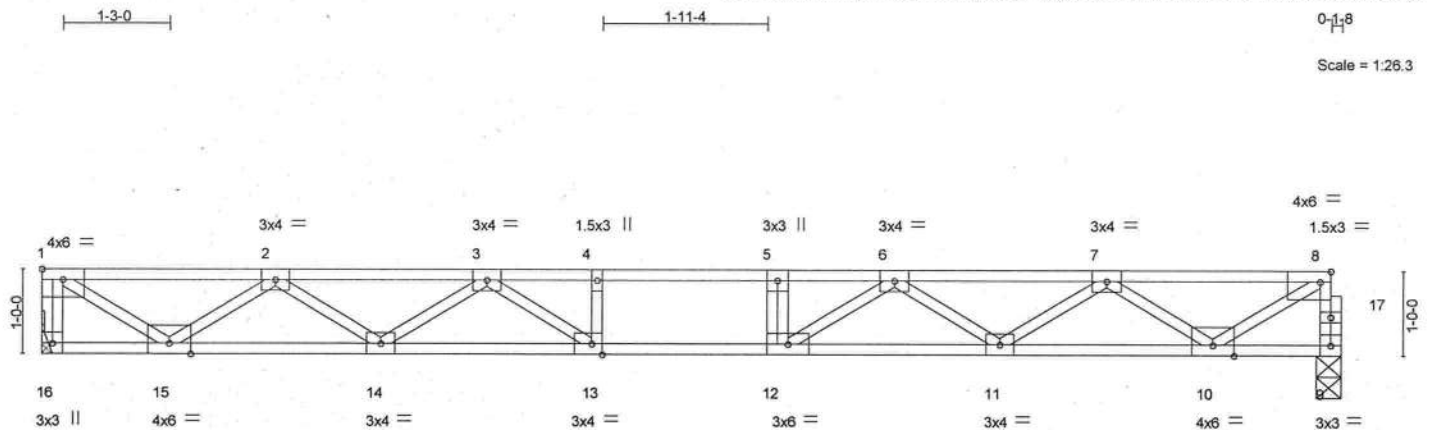


Plate Offsets (X,Y)-- [1:Edge,0-1-8], [8:0-1-8,Edge], [13:0-1-8,Edge]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.57	Vert(LL)	-0.22 12-13	>837	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.00	BC 0.45	Vert(TL)	-0.34 12-13	>535	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.63	Horz(TL)	0.05 9	n/a	n/a		
BCDL 5.0	Code FBC2014/TPI2007		(Matrix)						
								Weight: 75 lb	FT = 11%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.2(flat)
BOT CHORD 2x4 SP M 31(flat)
WEBS 2x4 SP No.3(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 16=828/Mechanical, 9=822/0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-16=-820/0, 9-17=-816/0, 8-17=-814/0, 1-2=-1123/0, 2-3=-2681/0, 3-4=-3504/0, 4-5=-3504/0, 5-6=-3504/0, 6-7=-2682/0, 7-8=-1125/0
BOT CHORD 14-15=0/2117, 13-14=0/3209, 12-13=0/3504, 11-12=0/3210, 10-11=0/2115
WEBS 8-10=0/1283, 1-15=0/1332, 7-10=-1208/0, 2-15=-1213/0, 7-11=0/692, 2-14=0/688, 6-11=-644/0, 3-14=-645/0, 6-12=0/641, 3-13=0/642

NOTES- (6)

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Refer to girder(s) for truss to truss connections.
- 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss designs.
- 6) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job 755227	Truss F06	Truss Type FLOOR	Qty 9	Ply 1	MIKE TODD CONST. - MORGAN RES. Job Reference (optional)	T10408278
Builders FirstSource, Lake City, FL 32055		7.640 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 09 16:09:56 2017 Page 1 ID: Awd7fSKXwnKYwWX1sWuKPzYL70-HGp8N0T9nnU9BAqOychAAaICV7vSgY3dH8nrGtmqjP				

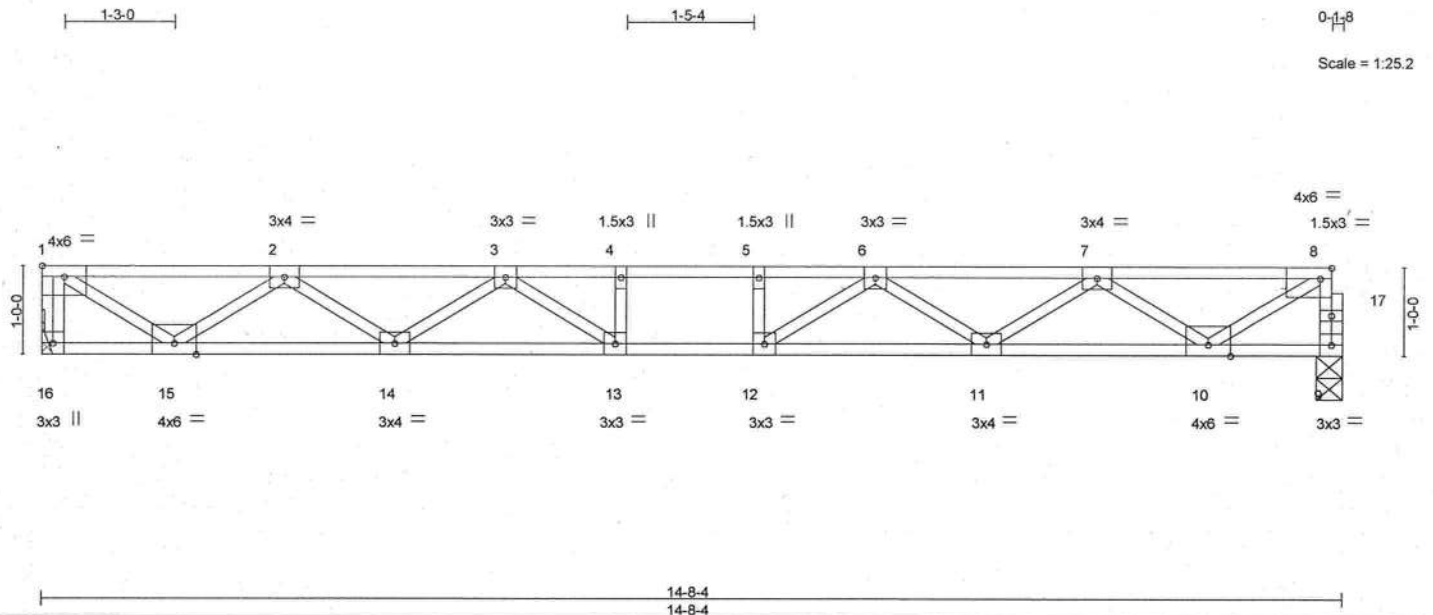


Plate Offsets (X,Y)-- [1:Edge,0-1-8], [8:0-1-8,Edge]									
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.43	Vert(LL)	-0.18 12-13	>945	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.00	BC 0.38	Vert(TL)	-0.29 12-13	>606	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.60	Horz(TL)	0.04 9	n/a	n/a		
BCDL 5.0	Code FBC2014/TPI2007		(Matrix)					Weight: 72 lb	FT = 11%F, 11%E

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP M 31(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3(flat)	

REACTIONS. (lb/size) 16=794/Mechanical, 9=788/0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	1-16=-786/0, 9-17=-782/0, 8-17=-780/0, 1-2=-1069/0, 2-3=-2532/0, 3-4=-3233/0, 4-5=-3233/0, 5-6=-3233/0, 6-7=-2531/0, 7-8=-1072/0
BOT CHORD	14-15=0/2015, 13-14=0/3011, 12-13=0/3233, 11-12=0/3012, 10-11=0/2012
WEBS	8-10=0/1221, 1-15=0/1267, 7-10=-1148/0, 2-15=-1154/0, 7-11=0/633, 2-14=0/632, 6-11=-587/0, 3-14=-585/0, 6-12=-54/527, 3-13=-53/527

NOTES- (6)

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Refer to girder(s) for truss to truss connections.
- 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.
- 6) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MIITek

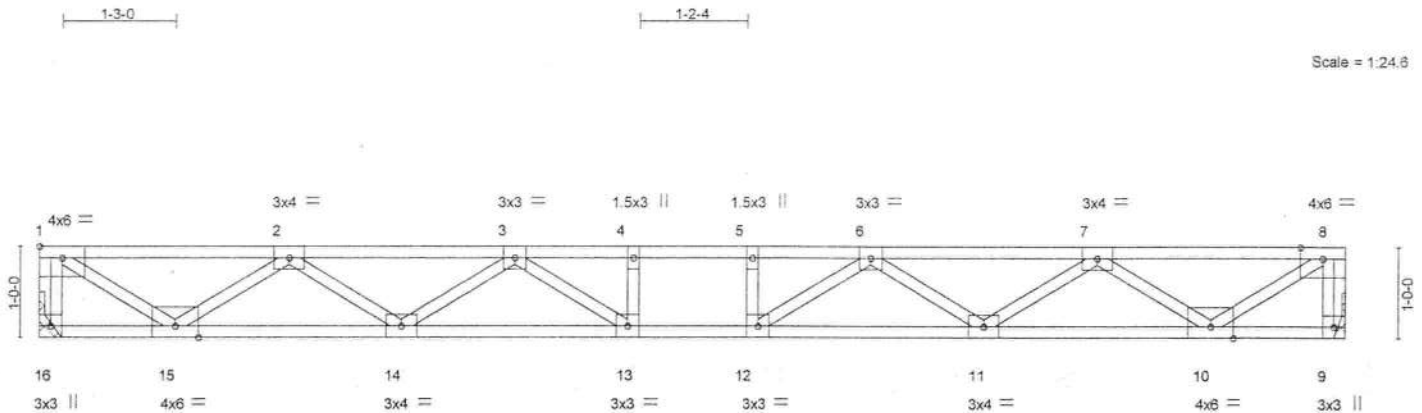
6904 Parke East Blvd.
Tampa, FL 38610

Job 755227	Truss F07	Truss Type FLOOR	Qty 4	Ply 1	MIKE TODD CONST. - MORGAN RES. T10408279
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.540 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 09 16:09:57 2017 Page 1
ID:AwD7fSKXwlnKYwWX1sWuKPzyL70-mSNWaMUnY5cOpKbWJoPjnINKPEwP7VnVoXPcKzmjQ

Scale = 1:24.8



1-8-0	4-0-0	10-5-4	12-11-4	14-5-4
1-8-0	2-6-0	6-5-4	2-6-0	1-8-0
Plate Offsets (X,Y)-- [1:Edge,0-1-8]				
LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES GRIP
TCLL 40.0	2-0-0	TC 0.43	in (loc) l/defl L/d	MT20 244/190
TCOL 10.0	Plate Grip DOL 1.00	BC 0.37	Vert(LL) -0.17 12-13 >992 360	
BCLL 0.0	Lumber DOL 1.00	WB 0.59	Vert(TL) -0.27 12-13 >635 240	
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.04 9 n/a n/a	
	Code FBC2014/TPI2007			Weight: 71 lb FT = 11%F, 11%E

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP M 31(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3(flat)	

REACTIONS. (lb/size) 16=780/Mechanical, 9=780/Mechanical

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	1-16=-772/0, 8-9=-772/0, 1-2=-1048/0, 2-3=-2472/0, 3-4=-3127/0, 4-5=-3127/0, 5-6=-3127/0, 6-7=-2472/0, 7-8=-1048/0
BOT CHORD	14-15=0/1973, 13-14=0/2932, 12-13=0/3127, 11-12=0/2932, 10-11=0/1973
WEBS	8-10=0/1242, 1-15=0/1242, 7-10=-1130/0, 2-15=-1130/0, 7-11=0/609, 2-14=0/609, 6-11=-562/0, 3-14=-562/0, 6-12=-72/483, 3-13=-72/483

NOTES- (5)

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Refer to girder(s) for truss to truss connections.
- 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek

6904 Parke East Blvd.
Tampa, FL 36610

Job 755227	Truss F08	Truss Type FLOOR	Qty 4	Ply 1	MIKE TODD CONST. - MORGAN RES. Job Reference (optional)	T10408280
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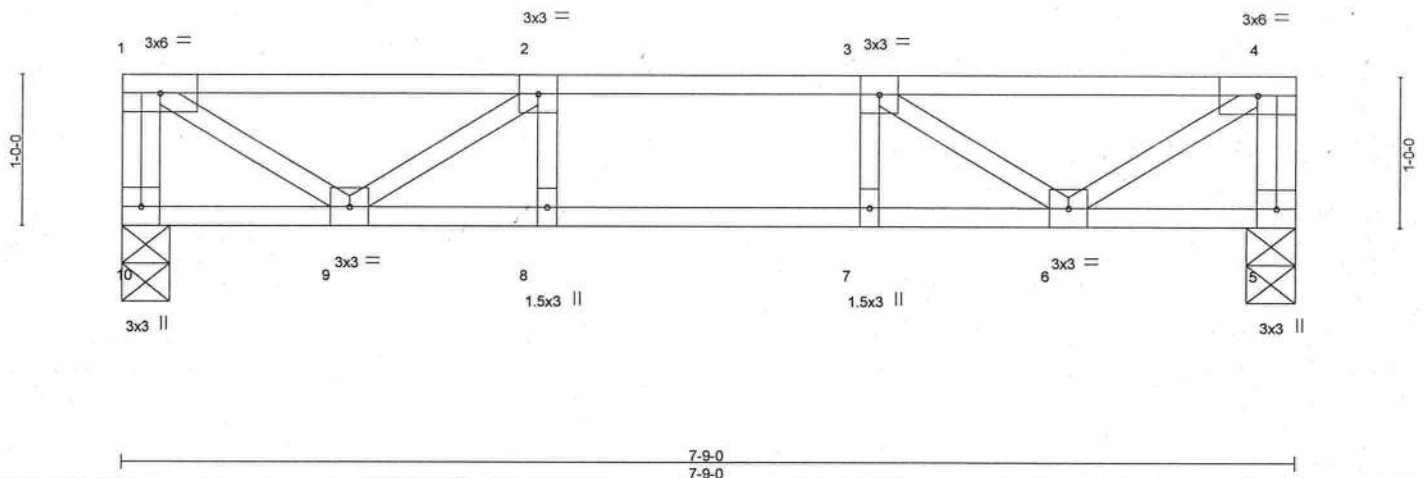
Builders FirstSource, Lake City, FL 32055

7.640 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 09 16:09:57 2017 Page 1

ID:Awd7fSKXwInKYwWX1sVWuKPzyL70-mSNWaMUnY5c0pKPbWJoPjnIMSPCZP4hnVoXPoKzmjO

1-3-0 2-0-0

Scale = 1:14.7



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.49	Vert(LL)	-0.04	8	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.00	BC 0.45	Vert(TL)	-0.05	8	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.26	Horz(TL)	0.01	5	n/a	n/a		
BCDL 5.0	Code FBC2014/TPI2007		(Matrix)						Weight: 38 lb	FT = 11%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.2(flat)
BOT CHORD 2x4 SP No.2(flat)
WEBS 2x4 SP No.3(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 10=413/0-3-13, 5=413/0-3-13

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-10=-406/0, 4-5=-406/0, 1-2=-460/0, 2-3=-879/0, 3-4=-460/0
BOT CHORD 8-9=0/879, 7-8=0/879, 6-7=0/879
WEBS 4-6=0/546, 1-9=0/546, 3-6=-503/0, 2-9=-503/0

NOTES- (4)

- Unbalanced floor live loads have been considered for this design.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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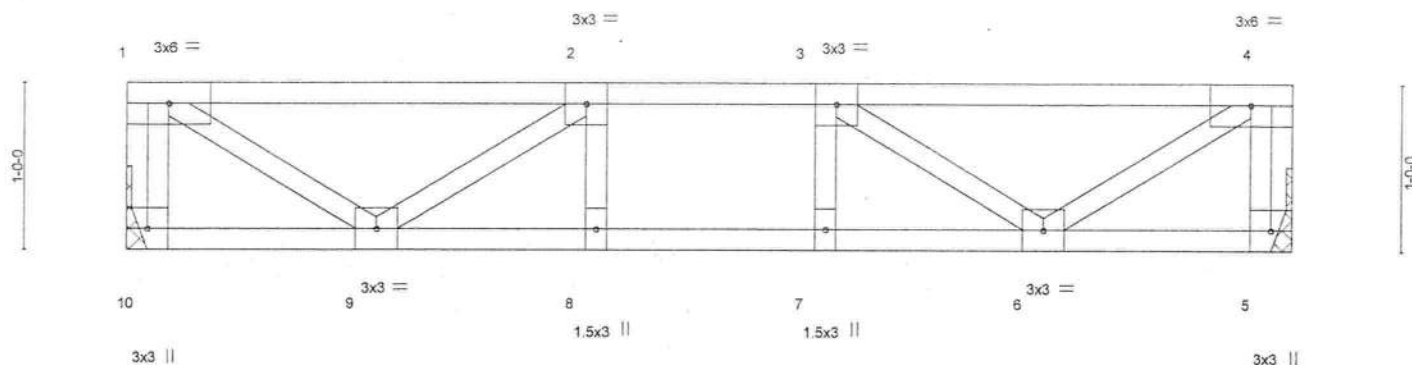
Job 755227	Truss F09	Truss Type FLOOR	Qty 4	Ply 1	MIKE TODD CONST. - MORGAN RES. Job Reference (optional)	T10408281
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Builders FirstSource, Lake City, FL 32055

7,540 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 09 16:09:57 2017 Page 1
ID:Awdf7fSKXwlnKYwWX1sWuKPzyl70-mSNWamUnY5c0pKPbWJoPjnlMYPEJP5GnVoXPoKzmqjO

1-3-0 1-3-0

Scale = 1:13.4



LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCCL	40.0	Plate Grip DOL	1.00	TC	0.48	Vert(LL)	-0.03	MT20		244/190	
TCDL	10.0	Lumber DOL	1.00	BC	0.38	Vert(TL)	-0.03				
BCCL	0.0	Rep Stress Incr	YES	WB	0.22	Horz(TL)	0.01				
BCDL	5.0	Code FBC2014/TPI2007		(Matrix)							
										Weight: 36 lb	FT = 11%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.2(flat)
BOT CHORD 2x4 SP No.2(flat)
WEBS 2x4 SP No.3(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 10=371/Mechanical, 5=371/Mechanical

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-10=-365/0, 4-5=-365/0, 1-2=-394/0, 2-3=-746/0, 3-4=-394/0
BOT CHORD 8-9=0/746, 7-8=0/746, 6-7=0/746
WEBS 4-6=0/467, 1-9=0/467, 3-6=-423/0, 2-9=-423/0

NOTES- (5)

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Refer to girder(s) for truss to truss connections.
- 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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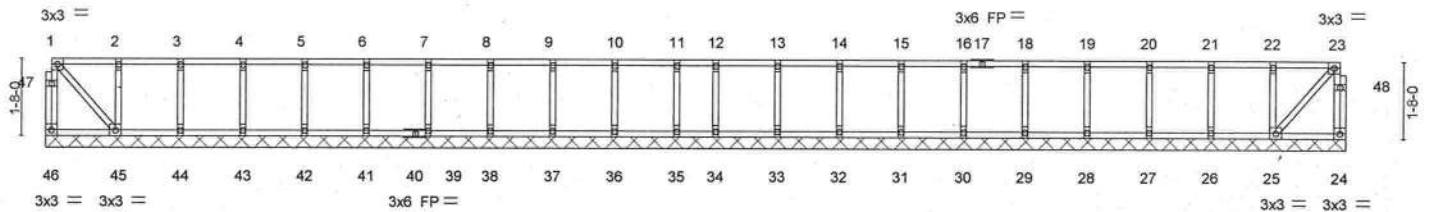
Job 755227	Truss KW1	Truss Type GABLE	Qty 2	Ply 1	MIKE TODD CONST. - MORGAN RES. Job Reference (optional) 7.640 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 09 16:09:58 2017 Page 1 ID:AwD7fSKXwlnKYwWX1sWuKPzYL70-EexuohVPJPktQU_n40JeG?qeCpfq8bRwkSGyLmzmqjN	T10408282
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Builders FirstSource, Lake City, FL 32055

0-1/8

0-1/8

Scale: 1/4"=1'



1-6-12	2-10-12	4-2-12	5-6-12	6-10-12	8-2-12	9-6-12	10-10-12	12-2-12	13-6-12	14-4-12	15-8-12	17-0-12	18-4-12	19-8-12	21-0-12	22-4-12	23-8-12	25-0-12	26-4-12	27-11-8
1-6-12	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	0-10-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-6-12
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP											
TCLL 40.0	Plate Grip DOL 1.00		TC 0.04	Vert(LL) n/a	-	n/a	999	MT20	244/190											
TCDL 10.0	Lumber DOL 1.00		BC 0.00	Vert(TL) n/a	-	n/a	999													
BCLL 0.0	Rep Stress Incr YES		WB 0.04	Horz(TL) -0.00	25	n/a	n/a													
BCDL 5.0	Code FBC2014/TPI2007		(Matrix)																	
										Weight: 138 lb FT = 11%F, 11%E										

LUMBER-	BRACING-																			
TOP CHORD 2x4 SP M 31 (flat)	TOP CHORD	Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals.																		
BOT CHORD 2x4 SP M 31 (flat)	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 45-46,24-25.																		
WEBS 2x4 SP No.3 (flat)																				
OTHERS 2x4 SP No.3 (flat)																				

REACTIONS. All bearings 27-11-8.
(lb) - Max Grav All reactions 250 lb or less at joint(s) 46, 24, 45, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 44, 43, 42, 41, 39, 38, 37, 36, 35

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-** (7)
- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
 - 2) Gable requires continuous bottom chord bearing.
 - 3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 4) Gable studs spaced at 1-4-0 oc.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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Job 755227	Truss T01	Truss Type COMMON	Qty 18	Ply 1	MIKE TODD CONST. - MORGAN RES.	T10408283
Builders FirstSource, Lake City, FL 32055						Job Reference (optional) 7.640 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 09 16:09:59 2017 Page 1 ID:AwD7fSKXwlnKYwVWX1sWuKPzYL70-irVG71V14isk2aYzekqtoCNh1Costy24z60WtCzmjM
						Scale = 1:52.8

Plate Offsets (X,Y) - [2:0-2-6,0-1-8], [3:0-3-0,0-3-4], [5:0-3-0,0-3-4], [6:0-2-6,0-1-8]					
LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.54	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.85	Vert(LL) -0.32 8-10 >999 240		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.40	Vert(TL) -0.61 8-10 >554 180		
BCDL 10.0	Rep Stress Incr YES	(Matrix-M)	Horz(TL) 0.08 6 n/a n/a		
	Code FBC2014/TPI2007			Weight: 129 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 3-7-11 oc purlins.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 6-8-8 oc bracing.
WEBS 2x4 SP No.3	
REACTIONS. (lb/size) 2=1144/0-3-8, 6=1144/0-3-8	
Max Horz 2=154(LC 17)	
Max Uplift 2=460(LC 12), 6=460(LC 13)	

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1863/1007, 3-4=-1657/969, 4-5=-1657/969, 5-6=-1863/1007
BOT CHORD 2-10=-767/1631, 9-10=-376/1079, 9-17=-376/1079, 8-17=-376/1079, 6-8=-773/1631
WEBS 4-8=-316/623, 5-8=-371/375, 4-10=-316/625, 3-10=-371/375

- NOTES-** (7)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 460 lb uplift at joint 2 and 460 lb uplift at joint 6.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

Job 755227	Truss T01G	Truss Type GABLE	Qty 1	Ply 1	MIKE TODD CONST. - MORGAN RES.	T10408284
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Builders FirstSource, Lake City, FL 32055

7.640 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 09 16:10:00 2017 Page 1
ID: Awd7fSKXwInKYwWX1sWuKPzYL70-A12eDNWfr0_bgo79BRM6LQwwacKncUzDBmI3PezmqjL

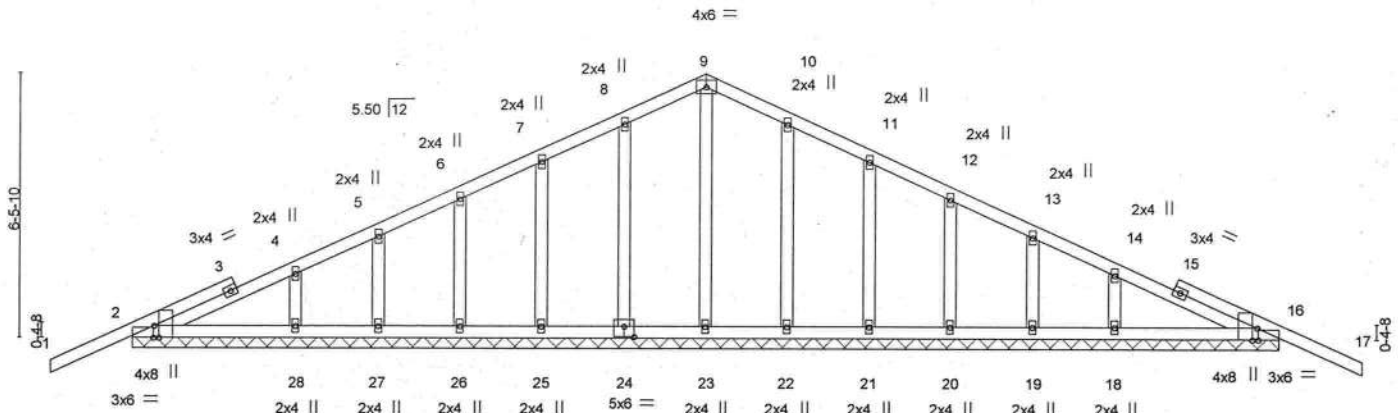
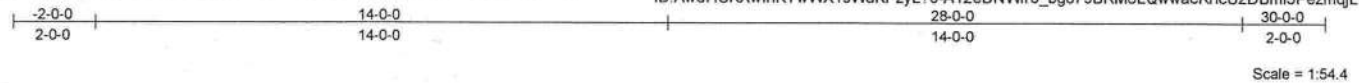


Plate Offsets (X,Y)--	[2:0-3-8,Edge], [2:0-0-4,Edge], [16:0-3-8,Edge], [16:0-0-4,Edge], [24:0-3-0-0-3-0]
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LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.30	Vert(LL)	-0.02	17	n/r	120	MT20
TCDL 7.0	Lumber DOL	1.25	BC 0.10	Vert(TL)	-0.02	17	n/r	120	244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.10	Horz(TL)	0.01	16	n/a	n/a	
BCDL 10.0	Code FBC2014/TPI2007		(Matrix)						
								Weight: 156 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 28-0-0.
(lb) - Max Horz 2=147(LC 12)
Max Uplift All uplift 100 lb or less at joint(s) 24, 25, 26, 27, 22, 21, 20, 19 except 2=105(LC 8), 16=123(LC 13), 28=113(LC 12), 18=118(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 23, 24, 25, 26, 27, 28, 22, 21, 20, 19, 18 except 2=267(LC 1), 16=267(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 8-9=81/252, 9-10=81/252

- NOTES-** (10)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 25, 26, 27, 22, 21, 20, 19 except (jt=lb) 2=105, 16=123, 28=113, 18=118.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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6904 Parke East Blvd.
Tampa, FL 36610

Job 755227	Truss T01GG	Truss Type GABLE	Qty 1	Ply 1	MIKE TODD CONST. - MORGAN RES.	T10408285
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

7.840 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 09 16:10:01 2017 Page 1
ID:Awdf7fSKXwlnKYwWX1sWuKPzYL70-eDc1QjXlcK6SHyIMISLtdSSK0gcLx4MQQVcx5zmqjK

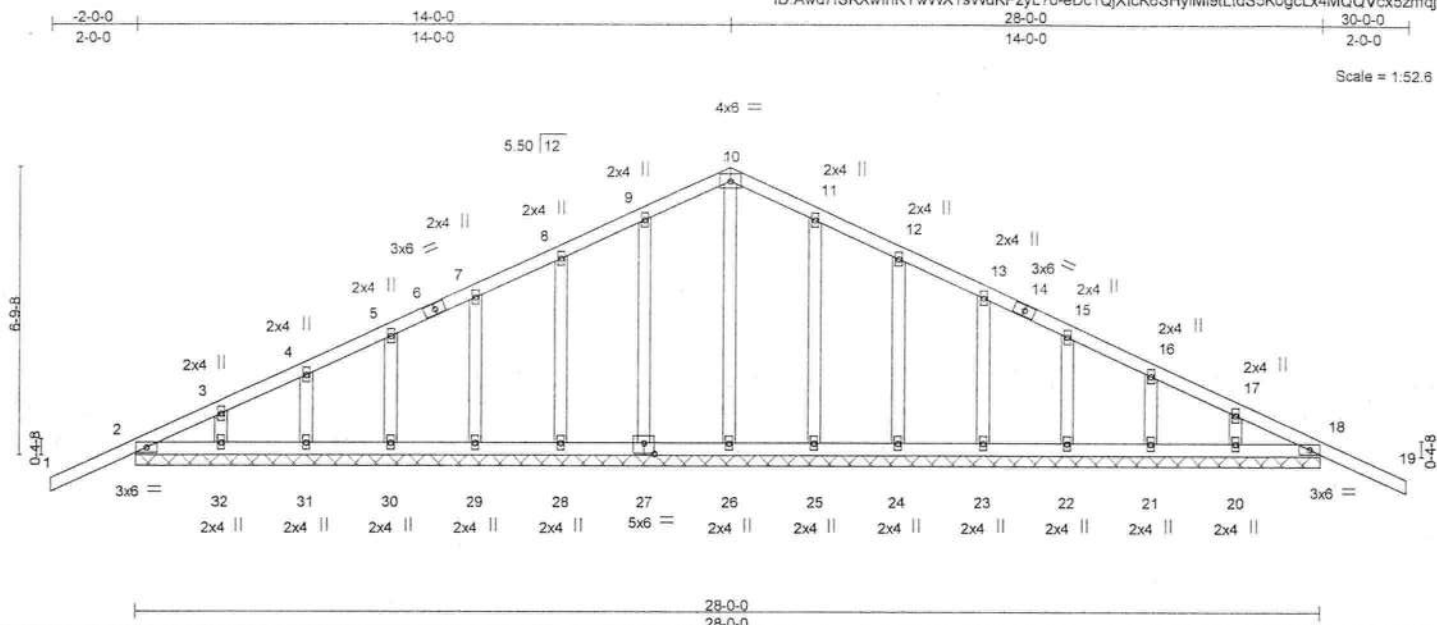


Plate Offsets (X,Y)-- [27:0-3-0-0-3-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.30	Vert(LL)	-0.02	19	n/r	120	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.13	Vert(TL)	-0.04	19	n/r	120	
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.11	Horz(TL)	0.01	18	n/a	n/a	
BCDL 10.0	Code FBC2014/TPI2007		(Matrix)						
Weight: 158 lb									FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 28-0-0.

(lb) - Max Horz 2=154(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 27, 28, 29, 30, 32, 25, 24, 23, 22, 20 except 2=117(LC 8),

31=103(LC 12), 21=103(LC 13), 18=112(LC 9)

Max Grav All reactions 250 lb or less at joint(s) 2, 26, 27, 28, 29, 30, 31, 32, 25, 24, 23, 22, 21, 20, 18

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 9-10=-95/268, 10-11=-95/268

NOTES- (10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 27, 28, 29, 30, 32, 25, 24, 23, 22, 20 except (jt=lb) 2=117, 31=103, 21=103, 18=112.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

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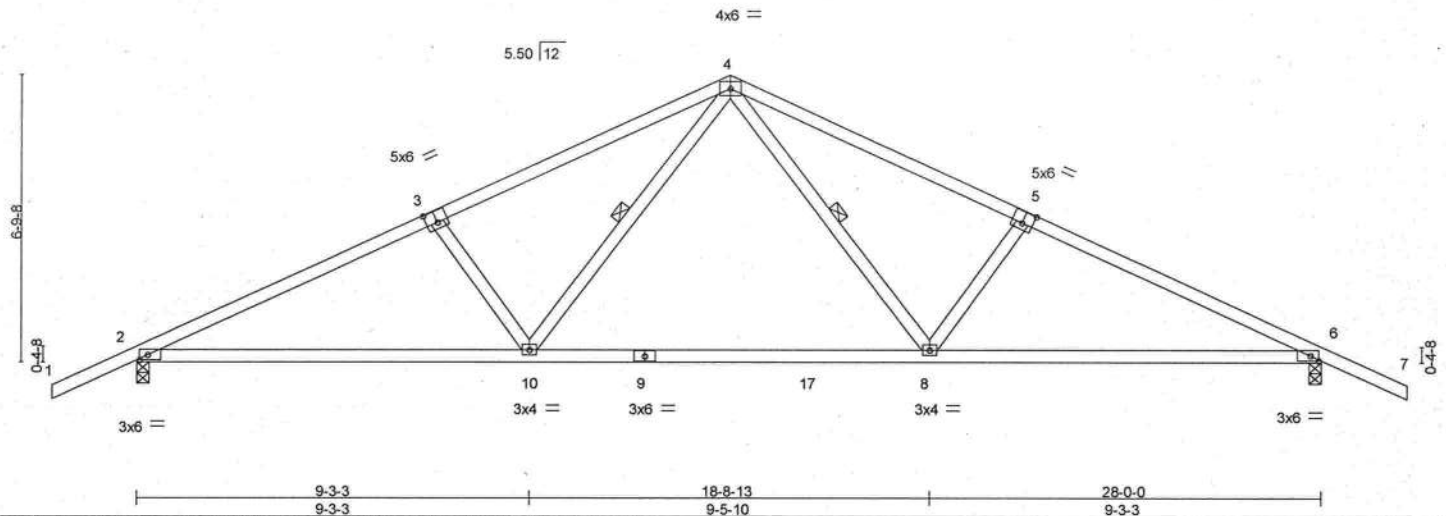
ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component

Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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Tampa, FL 36610

Job 755227	Truss T02	Truss Type COMMON	Qty 3	Ply 1	MIKE TODD CONST. - MORGAN RES.	T10408286
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 7.640 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 09 16:10:02 2017 Page 1	
-2-0-0 2-0-0			7-1-0 7-1-0			14-0-0 6-11-0
20-11-0 6-11-0			28-0-0 7-1-0			30-0-0 2-0-0
ID: Awd7fSKXwlnKYwWX1sWuKPzYL70-6QAPe3YwNdEJv5HYJsOaQr78iQzZ4L2Wf4EAUXzmqJ						
Scale = 1:52.6						



LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL	20.0	Plate Grip DOL	1.25	TC	0.71	in (loc)	l/defl	MT20		244/190	
TCDL	7.0	Lumber DOL	1.25	BC	0.85	0.38 8-10	>894				
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.32	-0.61 8-10	>554				
BCDL	10.0	Code FBC2014/TPI2007		(Matrix-M)		0.08 6	n/a				
								Weight: 129 lb FT = 20%			

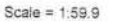
LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 3-7-11 oc purlins.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 3-10-9 oc bracing.
WEBS	2x4 SP No.3	WEBS	1 Row at midpt 4-8, 4-10

REACTIONS. (lb/size) 2=1144/0-3-8, 6=1144/0-3-8
Max Horz 2=-154(LC 17)
Max Uplift 2=-561(LC 9), 6=-561(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1863/2159, 3-4=-1657/2120, 4-5=-1657/2119, 5-6=-1863/2159
BOT CHORD 2-10=-1817/1631, 9-10=-1072/1079, 9-17=-1072/1079, 8-17=-1072/1079, 6-8=-1823/1631
WEBS 4-8=-910/623, 5-8=-371/382, 4-10=-910/625, 3-10=-371/382

- NOTES-** (7)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=561, 6=561.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

7.640 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 09 16:10:03 2017 Page 1
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NOTES- (9)

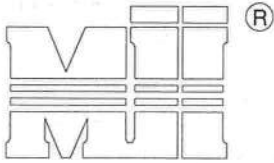
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDF=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=559, 8=559.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

August 10, 2010

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

ST - T-BRACE 2

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Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern

T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

Brace Size for One-Ply Truss

Specified Continuous Rows of Lateral Bracing

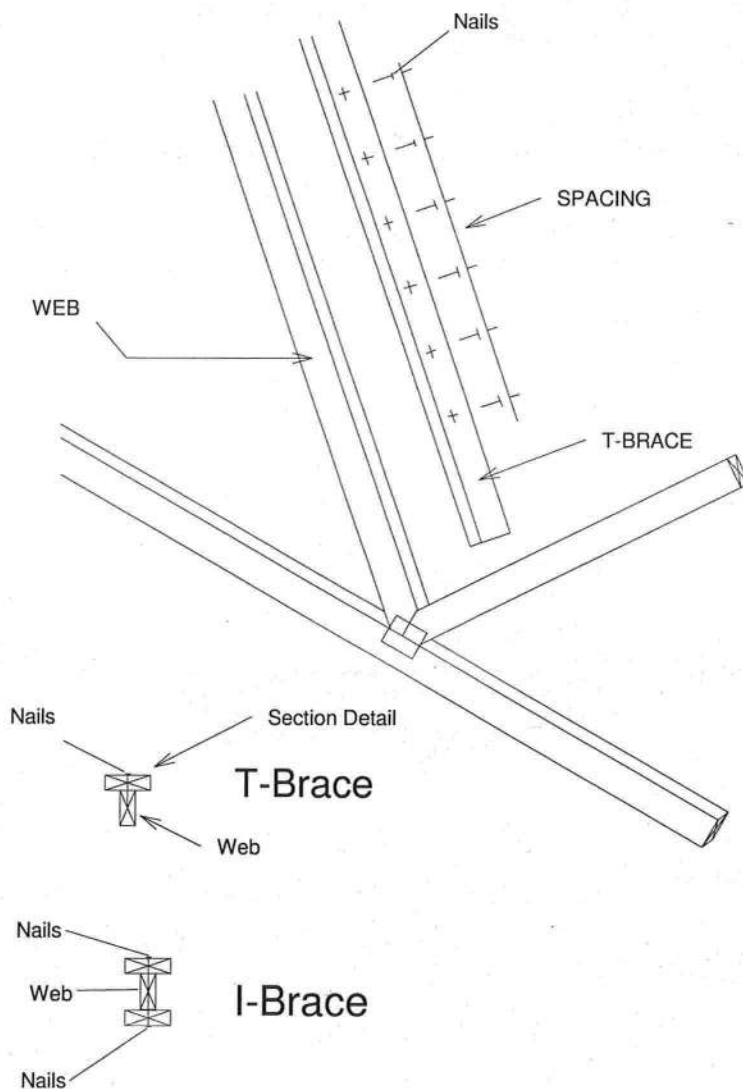
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

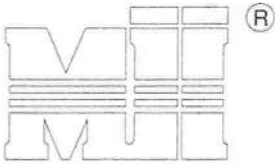
Brace Size for Two-Ply Truss

Specified Continuous Rows of Lateral Bracing

Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.





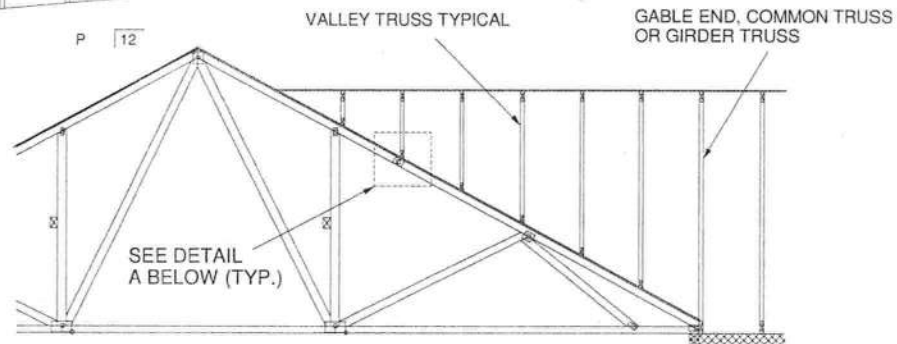
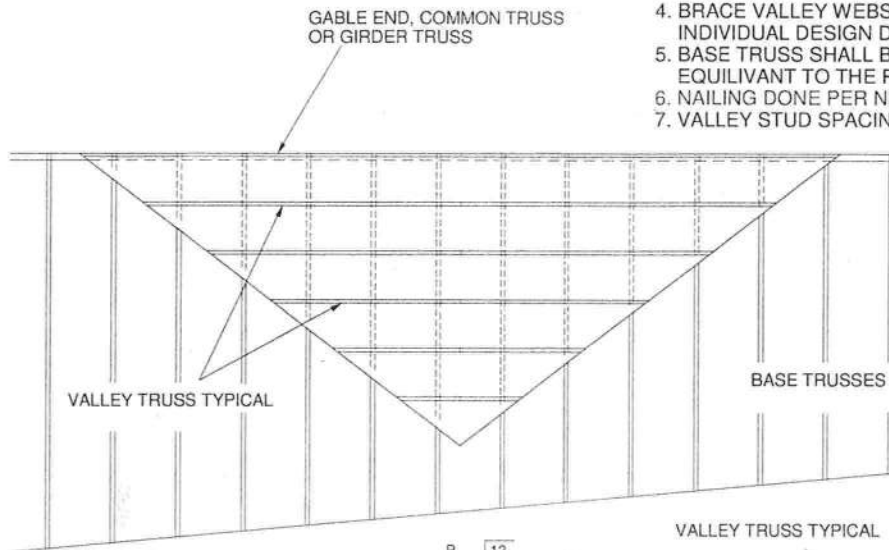
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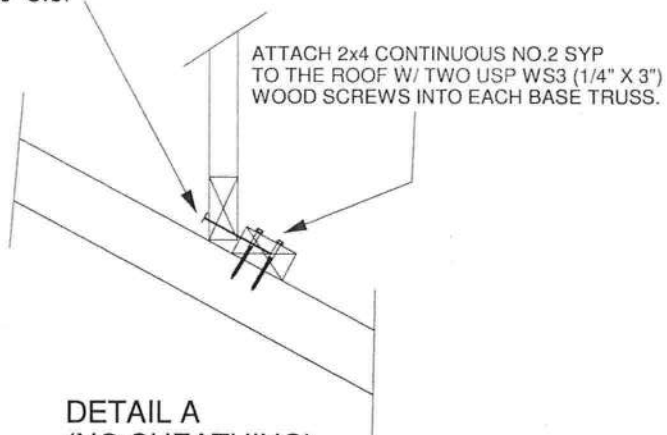
Page 1 of 1

GENERAL SPECIFICATIONS

1. NAIL SIZE = 3" X 0.131" = 10d
2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT
DO NOT USE DRYWALL OR DECKING TYPE SCREW
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
6. NAILING DONE PER NDS - 01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.

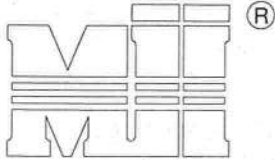


SECURE VALLEY TRUSS
W/ ONE ROW OF 10d
NAILS 6" O.C.



DETAIL A
(NO SHEATHING)
N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES



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Page 1 of 1

NOTES:

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.)
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

EXAMPLE:

(3) - 16d NAILS (.162" diam. x 3.5") WITH SPF SPECIES BOTTOM CHORD

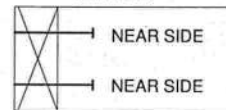
For load duration increase of 1.15:

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

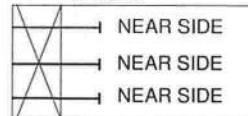
THIS DETAIL APPLICABLE TO THE
THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR
ILLUSTRATION PURPOSES ONLY

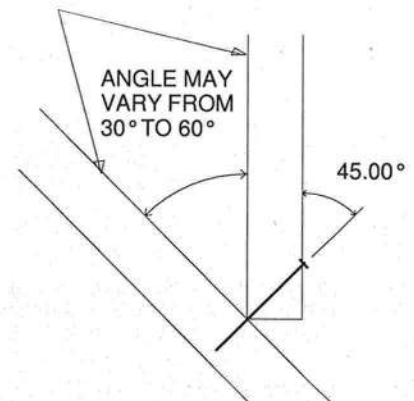
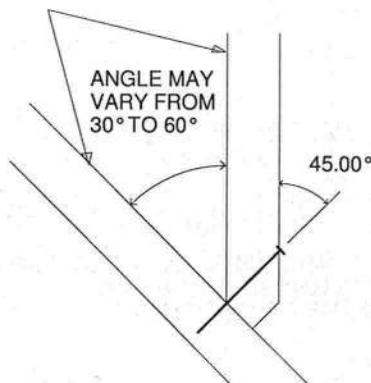
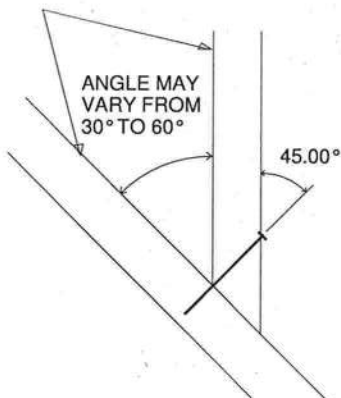
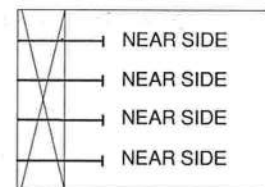
SIDE VIEW
(2x3)
2 NAILS

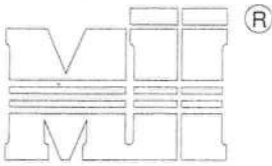


SIDE VIEW
(2x4)
3 NAILS



SIDE VIEW
(2x6)
4 NAILS



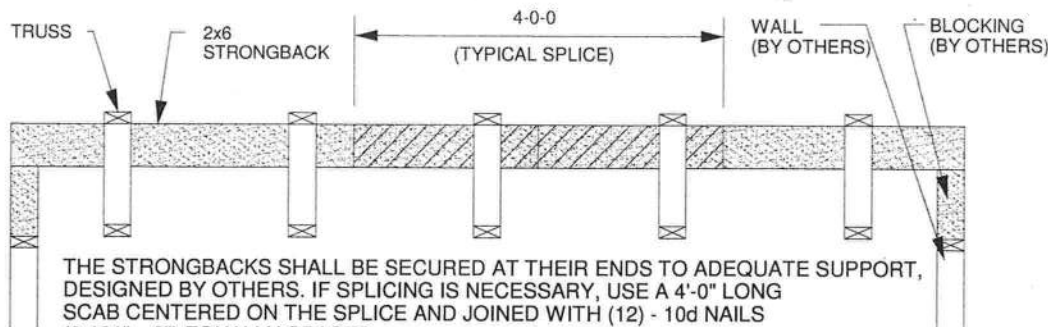
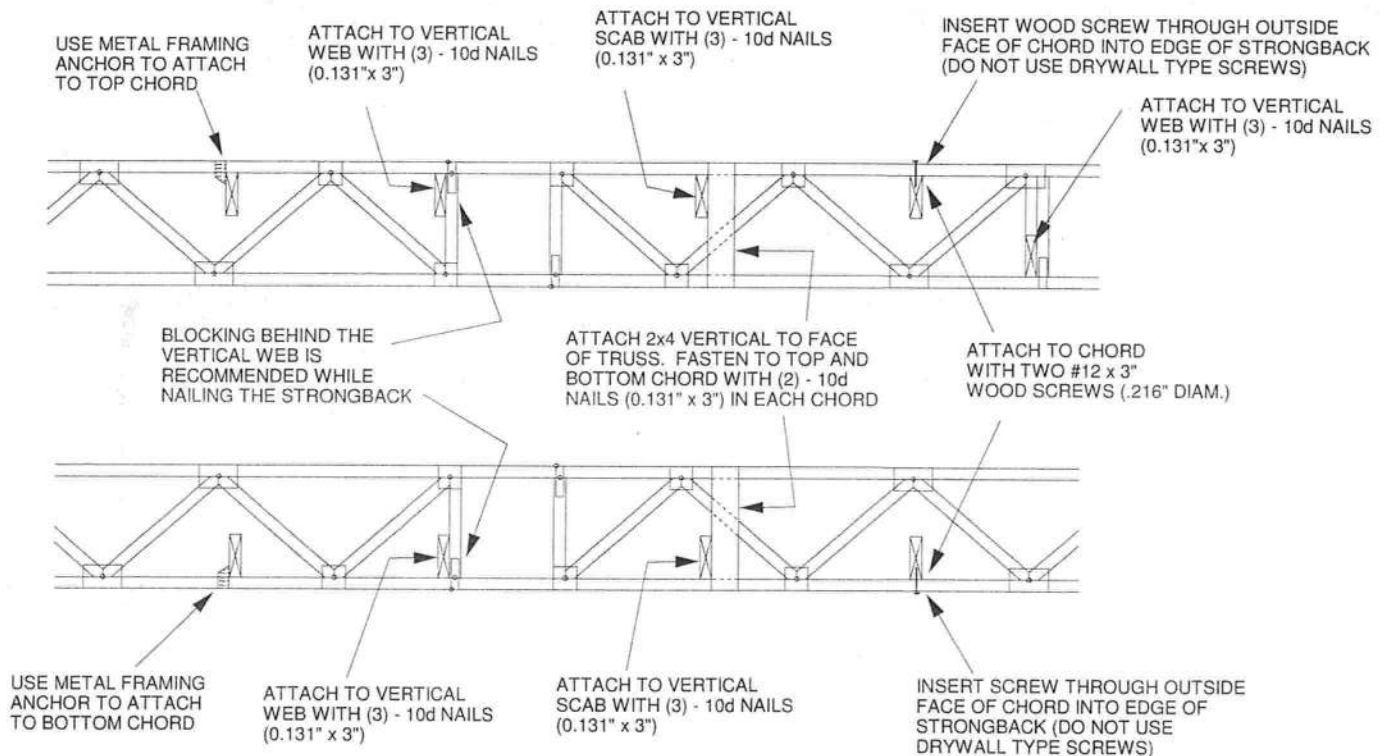


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TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.



THE STRONGBACKS SHALL BE SECURED AT THEIR ENDS TO ADEQUATE SUPPORT, DESIGNED BY OTHERS. IF SPLICING IS NECESSARY, USE A 4'-0" LONG SCAB CENTERED ON THE SPLICE AND JOINED WITH (12) - 10d NAILS (0.131" x 3") EQUALLY SPACED.

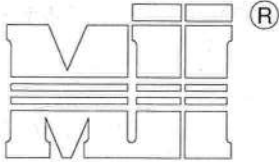
ALTERNATE METHOD OF SPLICING:
OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d NAILS (0.131" x 3") STAGGERED AND EQUALLY SPACED.
(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)

FEBRUARY 14, 2012

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10

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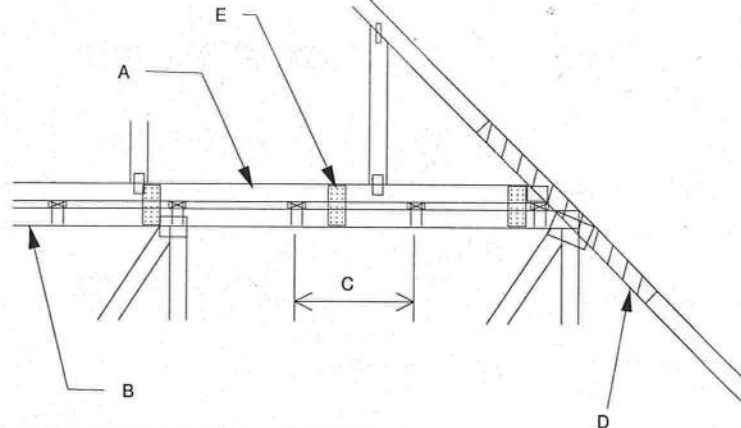


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MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24" O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

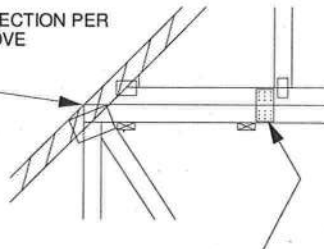
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED.
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.
- D - 2 X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



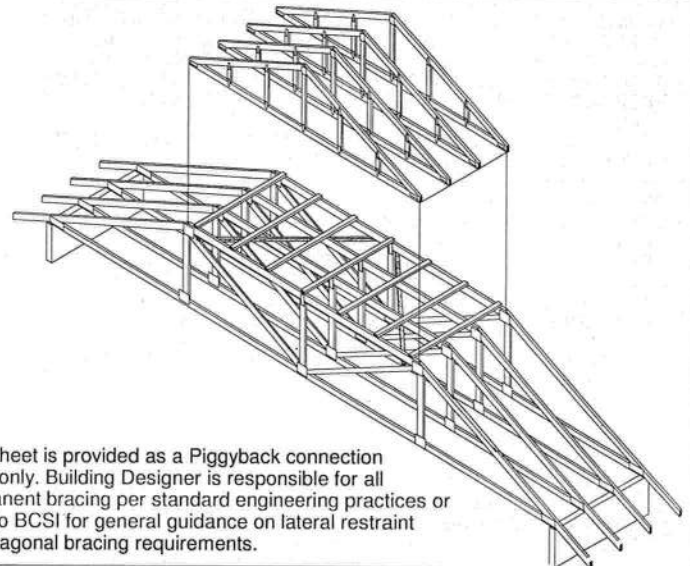
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER
NOTE D ABOVE

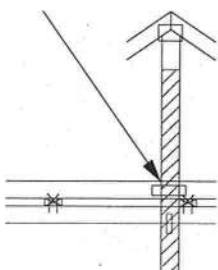


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



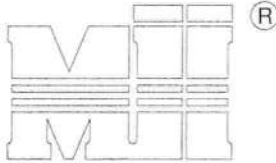
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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Typical $\frac{1}{2}$ " x 4" L-Brace Nailed To
2x Verticals w/10d Nails, 6" o.c.

Vertical Stud

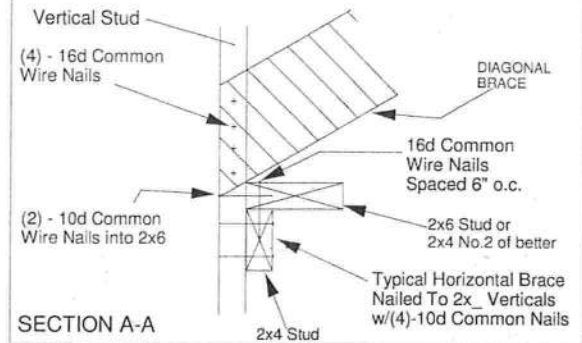
SECTION B-B

TRUSS GEOMETRY AND CONDITIONS
SHOWN ARE FOR ILLUSTRATION ONLY.

12

Varies to Common Truss

SEE INDIVIDUAL MITEK ENGINEERING
DRAWINGS FOR DESIGN CRITERIA

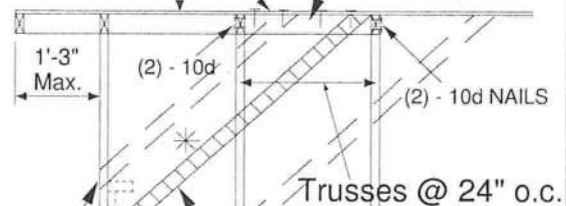


SECTION A-A

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d COMMON WIRE NAILS.

(4) - 8d NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing



Diag. Brace
at 1/3 points
if needed

End Wall

HORIZONTAL BRACE
(SEE SECTION A-A)

NOTE:

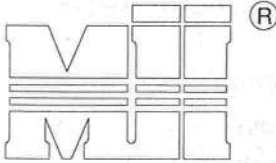
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS $L/240$.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SPF Std/Stud	12" O.C.	4-0-7	4-3-2	6-0-4	8-0-15	12-1-6
2x4 SPF Std/Stud	16" O.C.	3-7-0	3-8-4	5-2-10	7-1-15	10-8-15
2x4 SPF Std/Stud	24" O.C.	2-11-1	3-0-2	4-3-2	5-10-3	8-9-4

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d common wire nails 8in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD

Trusses @ 24" o.c.

HORIZONTAL BRACE
(SEE SECTION A-A)

Roof Sheathing

2x6 DIAGONAL BRACE SPACED 48" O.C.
ATTACHED TO VERTICAL WITH (4) - 16d
COMMON WIRE NAILS AND ATTACHED
TO BLOCKING WITH (5) - 10d COMMONS.1'-3"
Max.

IT IS THE RESPONSIBILITY OF THE BLDG DESIGNER OR
THE PROJECT ENGINEER/ARCHITECT TO DESIGN THE
CEILING DIAPHRAGM AND ITS ATTACHMENT TO THE
TRUSSES TO RESIST ALL OUT OF PLANE LOADS THAT
MAY RESULT FROM THE BRACING OF THE GABLE ENDS

Diag. Brace
at 1/3 points
if neededNAIL DIAGONAL BRACE TO
PURLIN WITH TWO 16d NAILS2X 4 PURLIN FASTENED TO FOUR TRUSSES
WITH TWO 16d NAILS EACH. FASTEN PURLIN
TO BLOCKING W/ TWO 16d NAILS (MIN)

PROVIDE 2x4 BLOCKING BETWEEN THE TRUSSES
SUPPORTING THE BRACE AND THE TWO TRUSSES
ON EITHER SIDE AS NOTED. TOENAIL BLOCKING
TO TRUSSES WITH (2) - 10d NAILS AT EACH END.
ATTACH DIAGONAL BRACE TO BLOCKING WITH
(5) - 10d COMMON WIRE NAILS.

End Wall

CEILING SHEATHING

BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

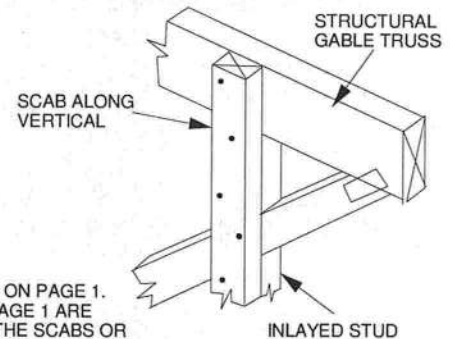
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:

METHOD 1 : ATTACH A MATCHING GABLE TRUSS TO THE INSIDE
FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE
FOLLOWING NAILING SCHEDULE.

METHOD 2 : ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL
MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING
NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE
AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL
MEMBERS WITH ONE ROW OF 10d (.131" X 3") NAILS SPACED 6" O.C.
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL
MEMBERS WITH TWO ROWS OF 10d (.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)



MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1.
ALL BRACING METHODS SHOWN ON PAGE 1 ARE
VALID AND ARE TO BE FASTENED TO THE SCABS OR
VERTICAL STUDS OF THE STANDARD GABLE TRUSS
ON THE INTERIOR SIDE OF THE STRUCTURE.

STRUCTURAL
GABLE TRUSS

AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST
BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM
CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN
IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

NOTE : THIS DETAIL IS TO BE USED ONLY FOR
STRUCTURAL GABLES WITH INLAYED
STUDS. TRUSSES WITHOUT INLAYED
STUDS ARE NOT ADDRESSED HERE.

STANDARD
GABLE TRUSS

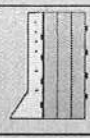
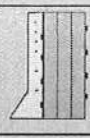
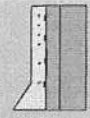
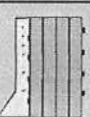
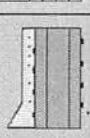
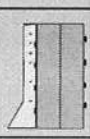
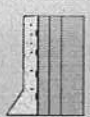
Multiple-Member Connections for Side-Loaded Beams with Concentrated Loads

Table 1: Maximum Concentrated Load Applied to Beam – Bolt or Nail Connection (lbs)⁽¹⁾

Beam Configuration		Hanger Type	½"ø or ¾"ø Bolts ⁽³⁾⁽⁴⁾					16d (0.148" x 3¼") Nails			
			Bolt Diameter ⁽²⁾	# Bolts				# Nails			
				2	4	6	8	6	12	18	24
3-ply 1¼" (5¼" Beam)		FACE	0.5	2,100	4,200	6,300	8,400	2,125 ⁽⁵⁾	4,250 ⁽⁵⁾	6,370 ⁽⁵⁾	8,495 ⁽⁵⁾
			0.75	3,060	6,120	9,180	12,240				
		TOP	0.5	760	1,525	2,285	3,050	1,060 ⁽⁶⁾	2,125 ⁽⁶⁾	3,185 ⁽⁶⁾	4,250 ⁽⁶⁾
			0.75	930	1,860	2,790	3,720				
3½"+1¼" (5¼" Beam)		FACE	---	---	---	---	---	---	---	---	---
			---	---	---	---	---				
		TOP	0.5	1,050	2,100	3,150	4,200	1,060	2,125	3,185	4,250
			0.75	1,530	3,060	4,590	6,120				
3½"+1¼" (5¼" Beam)		FACE / TOP	0.5	2,100	4,200	6,300	8,400	2,125 ⁽⁵⁾	4,250 ⁽⁵⁾	6,370 ⁽⁵⁾	8,495 ⁽⁵⁾
			0.75	3,060	6,120	9,180	12,240				
4-ply 1¼" (7" Beam)		FACE	0.5	1,400	2,800	4,200	5,600	---	---	---	---
			0.75	2,040	4,080	6,120	8,160				
		TOP	0.5	675	1,355	2,030	2,710	---	---	---	---
			0.75	825	1,655	2,480	3,305				
2-ply 1¼" + 3½" (7" Beam)		FACE	0.5	2,800	5,600	8,400	11,200	2,830 ⁽⁵⁾	5,665 ⁽⁵⁾	8,495 ⁽⁵⁾	11,330 ⁽⁵⁾
			0.75	5,025	10,050	15,070	20,095				
		TOP	0.5	935	1,865	2,800	3,735	945 ⁽⁶⁾	1,890 ⁽⁶⁾	2,830 ⁽⁶⁾	3,775 ⁽⁶⁾
			0.75	1,360	2,720	4,080	5,440				
2-ply 3½" (7" Beam)		FACE / TOP	0.5	1,720	3,440	5,160	6,880	---	---	---	---
			0.75	2,480	4,960	7,440	9,920				
2-ply 1¼" + 3½" (7" Beam)		FACE / TOP	0.5	1,015	2,030	3,050	4,065	---	---	---	---
			0.75	1,240	2,480	3,720	4,960				
2-ply 1¼" + 3½" (7" Beam)		FACE	0.5	1,720	3,440	5,160	6,880	---	---	---	---
			0.75	2,480	4,960	7,440	9,920				
		TOP	0.5	675	1,355	2,030	2,710	---	---	---	---
			0.75	825	1,655	2,480	3,305				
5¼" + 1¼" (7" Beam)		FACE / TOP	0.5	2,800	5,600	8,400	11,200	2,830 ⁽⁵⁾	5,665 ⁽⁵⁾	8,495 ⁽⁵⁾	11,330 ⁽⁵⁾
			0.75	5,025	10,050	15,070	20,095				

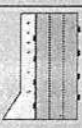
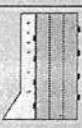
1. See page 4 for table General Notes, connection details and beam depth limitations.
2. Washers required. Bolt holes to be 9/16" maximum for 1/2" bolts, 13/16" maximum for 3/4" bolts.
3. Minimum end distance for bolts is 6".
4. Drilling bolt holes reduces the section of the beam and thus the load capacity of the supporting member. Shear and moment capacity of the beam must be checked at the location of each row of bolts.
5. Number of required nails shown must be installed from side opposite hanger.
6. Number of required nails shown must be installed from hanger side. Additionally, install half the number of required nails from side opposite hanger.

Table 2A: Maximum Concentrated Load Applied to Beam – Wood Screw Connection (lbs)⁽¹⁾

Beam Configuration		Hanger Type	Wood Screw Length	Fastener Type							
				USP WS				Simpson SDS			
				# Screws				# Screws			
				2	4	6	8	2	4	6	8
3-ply 1¼" (5¼" Beam)		FACE ⁽³⁾	3½"	1,435	2,870	4,305	5,740	2,040	4,080	6,120	8,160
		TOP ⁽²⁾	3½"	720	1,435	2,155	2,870	1,020	2,040	3,060	4,080
3½" + 1¼" (5¼" Beam)		FACE	3½"	---	---	---	---	---	---	---	---
		TOP ⁽⁴⁾	3½"	720	1,435	2,155	2,870	1,020	2,040	3,060	4,080
3½" + 1¼" (5¼" Beam)		FACE / TOP ⁽³⁾	3½"	1,435	2,870	4,305	5,740	2,040	4,080	6,120	8,160
4-ply 1¼" (7" Beam)		FACE ⁽²⁾	6" ⁽⁵⁾	1,055	2,110	3,165	4,220	1,360	2,720	4,080	5,440
		TOP ⁽²⁾	6" ⁽⁵⁾	705	1,405	2,110	2,810	905	1,815	2,720	3,625
2-ply 1¼" + 3½" (7" Beam)		FACE ⁽³⁾	3½"	1,915	3,825	5,740	7,655	2,720	5,440	8,160	10,880
			6" ⁽⁵⁾	2,110	4,220	6,325	8,435	2,720	5,440	8,160	10,880
		TOP ⁽²⁾	3½"	640	1,275	1,915	2,550	905	1,815	2,720	3,625
			6" ⁽⁵⁾	705	1,405	2,110	2,810	905	1,815	2,720	3,625
2-ply 3½" (7" Beam)		FACE / TOP ⁽²⁾	6" ⁽⁵⁾	1,055	2,110	3,165	4,220	930	1,860	2,785	3,715
2-ply 1¼" + 3½" (7" Beam)		FACE / TOP ⁽³⁾	6" ⁽⁵⁾	1,055	2,110	3,165	4,220	1,360	2,720	4,080	5,440
2-ply 1¼" + 3½" (7" Beam)		FACE ⁽⁴⁾	6" ⁽⁵⁾	1,055	2,110	3,165	4,220	930	1,860	2,785	3,715
		TOP ⁽⁴⁾	6" ⁽⁵⁾	705	1,405	2,110	2,810	905	1,815	2,720	3,625
5¼" + 1¼" (7" Beam)		FACE / TOP ⁽³⁾	3½"	1,915	3,825	5,740	7,655	2,720	5,440	8,160	10,880

1. See page 4 for table General Notes, connection details and beam depth limitations.
2. Install screws from both sides of beam.
3. Install screws from side opposite hanger only.
4. Install screws from hanger side only.
5. 6" SDS or WS screws can be used with Parallam[®] PSL and Microllam[®] LVL, but are not recommended for TimberStrand[®] LSL.

Table 2B: Maximum Concentrated Load Applied to Beam – Wood Screw Connection (lbs)⁽¹⁾

Beam Configuration		Hanger Type	Wood Screw Length	Connection Name			
				TrussLok			
				# Screws			
				2	4	6	8
3-ply 1 $\frac{1}{4}$ " (5 $\frac{1}{4}$ " Beam)		FACE ⁽³⁾	3 $\frac{3}{8}$ "	1,600	3,205	4,805	6,410
		TOP ⁽²⁾	3 $\frac{3}{8}$ "	800	1,600	2,405	3,205
3 $\frac{1}{2}$ " + 1 $\frac{3}{4}$ " (5 $\frac{1}{4}$ " Beam)		FACE	3 $\frac{3}{8}$ "	---	---	---	---
		TOP ⁽⁴⁾	3 $\frac{3}{8}$ "	800	1,600	2,405	3,205
3 $\frac{1}{2}$ " + 1 $\frac{3}{4}$ " (5 $\frac{1}{4}$ " Beam)		FACE / TOP ⁽³⁾	3 $\frac{3}{8}$ "	1,600	3,205	4,805	6,410
4-ply 1 $\frac{1}{4}$ " (7" Beam)		FACE ⁽²⁾	5", 6 $\frac{3}{4}$ "	1,160	2,320	3,480	4,640
		TOP ⁽²⁾	5", 6 $\frac{3}{4}$ "	775	1,545	2,320	3,095
2-ply 1 $\frac{1}{4}$ " + 3 $\frac{1}{2}$ " (7" Beam)		FACE ⁽³⁾	3 $\frac{3}{8}$ "	2,135	4,270	6,410	8,545
			5", 6 $\frac{3}{4}$ "	2,320	4,640	6,960	9,280
		TOP ⁽²⁾	3 $\frac{3}{8}$ "	710	1,425	2,135	2,850
			5", 6 $\frac{3}{4}$ "	775	1,545	2,320	3,095
2-ply 3 $\frac{1}{2}$ " (7" Beam)		FACE / TOP ⁽²⁾	5", 6 $\frac{3}{4}$ "	1,160	2,320	3,480	4,640
2-ply 1 $\frac{1}{4}$ " + 3 $\frac{1}{2}$ " (7" Beam)		FACE / TOP ⁽³⁾	5", 6 $\frac{3}{4}$ "	1,160	2,320	3,480	4,640
2-ply 1 $\frac{1}{4}$ " + 3 $\frac{1}{2}$ " (7" Beam)		FACE ⁽⁴⁾	5", 6 $\frac{3}{4}$ "	1,160	2,320	3,480	4,640
		TOP ⁽⁴⁾	5", 6 $\frac{3}{4}$ "	1,160	2,320	3,480	4,640
5 $\frac{1}{4}$ " + 1 $\frac{3}{4}$ " (7" Beam)		FACE / TOP ⁽³⁾	5", 6 $\frac{3}{4}$ "	2,320	4,640	6,960	9,280

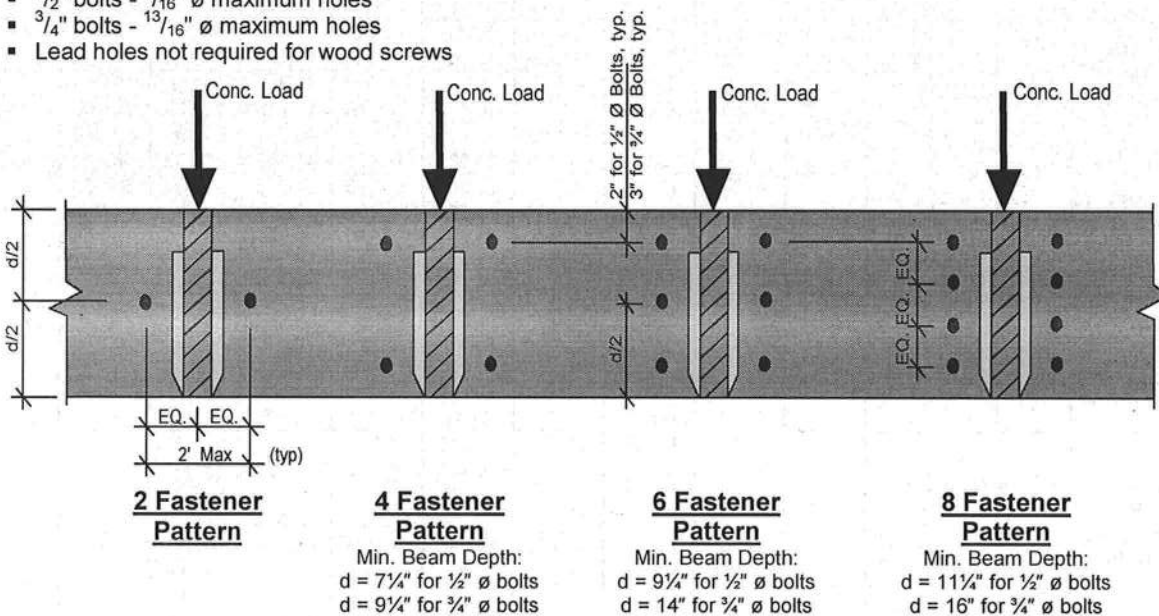
1. See page 4 for table General Notes, connection details and beam depth limitations.
2. Install screws from both sides of beam.
3. Install screws from side opposite hanger only.
4. Install screws from hanger side only.

Table 1, 2A, 2B General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- All plies must be the same material and grade.
- Values are for 100% duration of load. Increase 15% for snow load or 25% for non-snow roof load conditions, where code allows.
- Rotational effects should be considered for 7" wide beams loaded from one side only.
- Capacities shown for face mount hanger conditions are based on 16d common (0.162" x 3½") nails installed in the hanger. Other nails used for face mount hanger installations invalidate the capacities in these tables.
- Verify adequacy of beam with all loads applied by using iLevel software or other methods.
- See the iLevel Trus Joist® Beams, Headers, and Columns Specifier's Guide (#TJ-9000) for required connections for uniform side loads.

Bolt or Wood Screw Connection Details

- ½" bolts - 9/16" Ø maximum holes
- ¾" bolts - 13/16" Ø maximum holes
- Lead holes not required for wood screws



Nailed Connection Detail

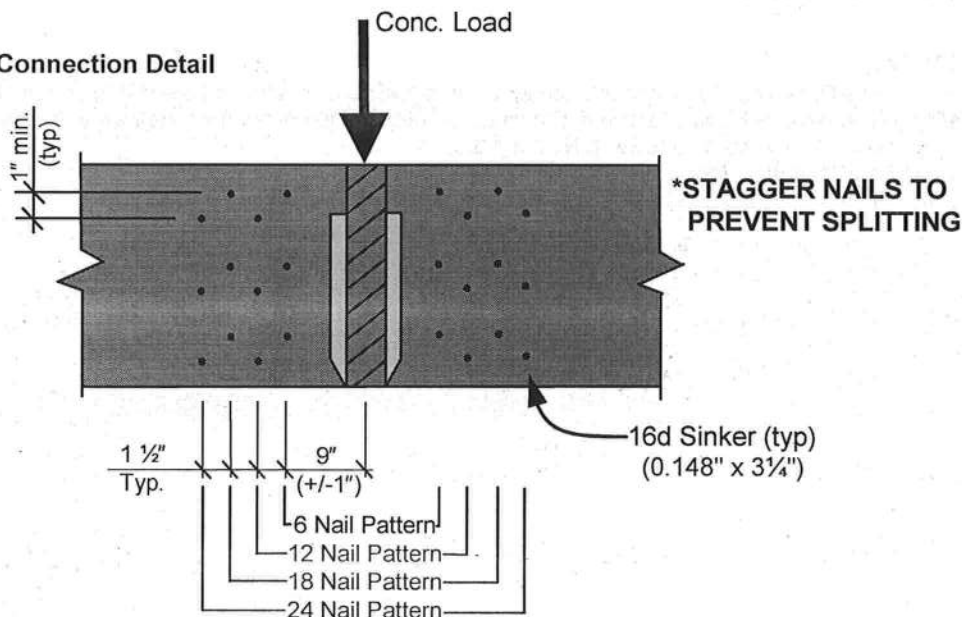


Table 3: Allowable Capacities of 5¼" Beams with Bolt Connections

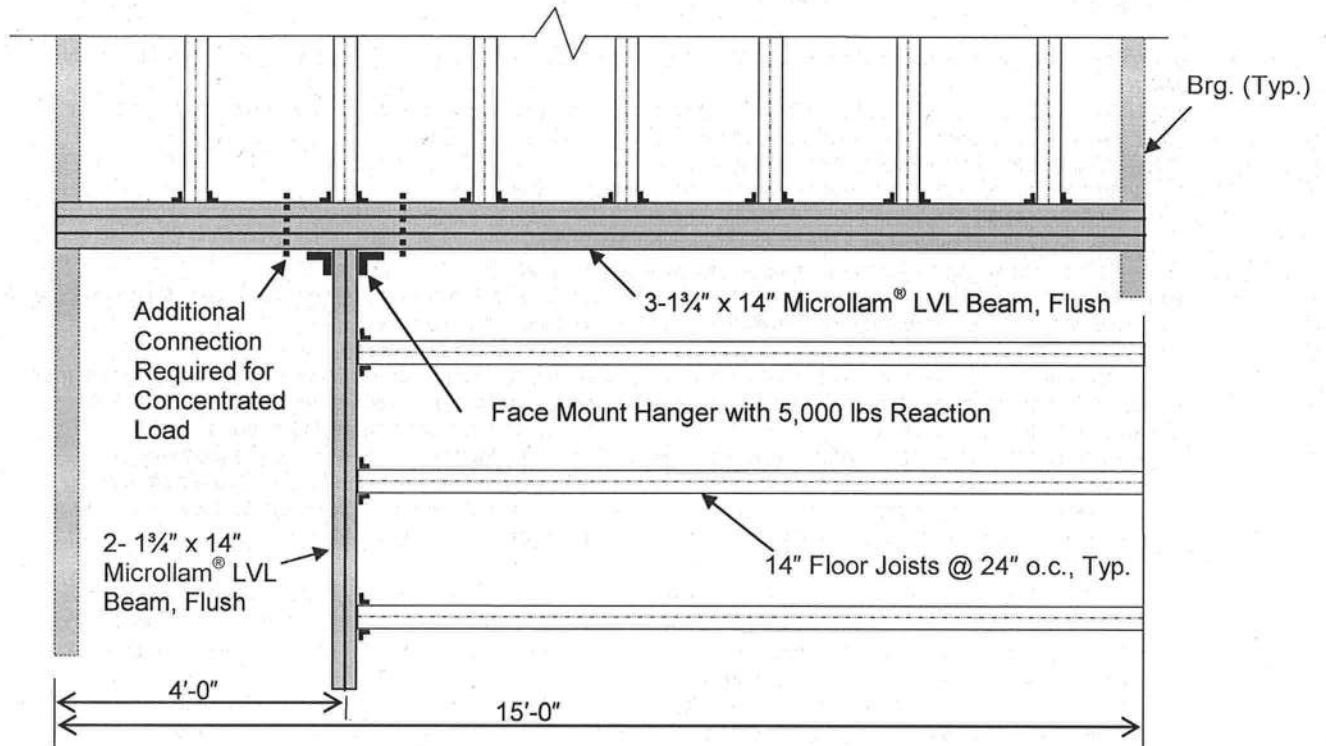
		Bolt Dia.	Conn. Loc.	Beam Depth									
				7¼"	9¼"	9½"	11¼"	11½"	14"	16"	18"	20"	24"
TimberStrand® LSL	Max Shear (lbs)	½"	< 5d		6,235	6,545	8,755	9,560	12,345	15,005			
			≥ 5d		10,150	10,500	12,950	13,825	16,800	19,600			
		¾"	< 5d		3,995	4,260	6,210	6,940	9,510	12,015			
			≥ 5d		8,750	9,100	11,550	12,425	15,400	18,200			
	Max Moment (ft-lb)	½"	2		10,605	15,295	15,685	23,905	32,760	42,265			
			4		9,355	13,470	13,730	20,915	28,725	37,250			
			6		9,350	13,470	13,730	20,910	28,725	37,245			
			8				13,510	20,575	28,275	36,685			
		¾"	2		10,600	15,290	15,685	23,900	32,755	42,260			
			4		9,900	14,215	14,195	21,490	29,020	37,225			
			6						29,015	37,220			
			8						36,655				
Microllam® LVL	Max Shear (lbs)	½"	< 5d	2,745	4,445	4,665	6,240	6,810	8,795	10,690	12,610	14,545	18,440
			≥ 5d	5,235	7,230	7,480	9,225	9,850	11,970	13,965	15,960	17,955	21,945
		¾"	< 5d	1,455	2,845	3,035	4,425	4,945	6,775	8,560	10,390	12,250	16,040
			≥ 5d	4,240	6,235	6,485	8,230	8,855	10,975	12,970	14,965	16,960	20,950
	Max Moment (ft-lb)	½"	2	9,960	16,220	17,105	23,990	26,730	36,385	46,670	58,130	70,740	99,375
			4	9,025	14,305	15,065	21,000	23,385	31,905	41,130	51,535	63,100	89,670
			6		14,305	15,060	20,995	23,385	31,905	41,125	51,530	63,100	89,670
			8				20,660	23,010	31,405	40,510	50,795	62,250	88,590
		¾"	2	9,950	16,210	17,100	23,985	26,725	36,380	46,665	58,125	70,740	99,370
			4		15,145	15,895	21,710	24,030	32,235	41,105	51,120	62,285	88,015
			6						32,230	41,100	51,115	62,280	88,010
			8						40,475	50,335	61,335	86,745	
Parallam® PSL	Max Shear (lbs)	½"	< 5d		4,520	4,745	6,350	6,930	8,950	10,880	12,830		
			≥ 5d		7,360	7,615	9,390	10,025	12,180	14,210	16,240		
		¾"	< 5d		2,895	3,090	4,505	5,030	6,895	8,710	10,575		
			≥ 5d		6,345	6,600	8,375	9,010	11,165	13,195	15,225		
	Max Moment (ft-lb)	½"	2		18,090	19,080	26,760	29,815	40,740	52,430	65,495		
			4		15,960	16,805	23,420	26,085	35,725	46,205	58,065		
			6		15,955	16,800	23,420	26,080	35,725	46,205	58,060		
			8				23,045	25,665	35,165	45,510	57,235		
		¾"	2		18,080	19,070	26,755	29,810	40,735	52,425	65,490		
			4		16,890	17,730	24,215	26,805	36,095	46,180	57,600		
			6						36,085	46,170	57,595		
			8						45,470	56,715			

Table 3 General Notes

- Connection location refers to distance from face of bearing to centerline of connection, where d is the beam depth. Less than 5d refers to connections located less than five times the depth of the beam from the face of bearing.
- Shear reduction has been taken in accordance with NDS 3.4.3.3.
- All values shown are for 5¼" widths. Multiply by 1.33 for 7" wide beams.
- 9¼" and 11¼" TimberStrand® LSL are 1.3E grade, all other depths are 1.55E grade.

Connection Design Example

Given the floor framing shown below, specify the necessary connections for the a 3-ply Microllam® LVL beam supporting a 2-ply Microllam® LVL beam on one side and floor joists from the other side. The floor joists apply a 500 plf load to the side of the beam while the 2-ply beam reaction is 5,000 lbs. See the following page for the solution.



Partial Framing Plan

Solution:

- Use the iLevel Trus Joist[®] Beams, Headers, and Columns Specifier's Guide (#TJ-9000) to select a connection for the uniform loading. Multiple options exist that meet or exceed the 500 plf requirement:
 - 2 rows of ½" diameter bolts at 16" on-center (570 plf)
 - 2 rows of USP WS35 wood screws at 16" on-center (540 plf) on each side of beam
 - 2 rows of SDS ¼" x 3½" at 24" on-center (510 plf) on each side of beam
 - 2 rows of 5" TrussLok wood screws at 24" on-center (500 plf)

- Determine what additional connection is needed for the concentrated load using the information in this Technical Bulletin:

1. Using Table 1, 2A or 2B of this bulletin, locate the section for a 3-ply 1¼" member and face mount hanger. Again, multiple options exist that exceed the 5,000 lb load requirement:
 - ½" diameter bolts in a 6-bolt connection (6,300 lbs)
 - 16d (0.148" x 3¼") sinker nails in an 18-nail connection (6,370 lbs)
 - 3½" WS wood screws in an 8-screw connection (5,740 lbs)
 - 3½" SDS wood screws in a 6-screw connection (6,120 lbs)
 - 3½" TrussLok wood screws in an 8-screw connection (6,410 lbs)

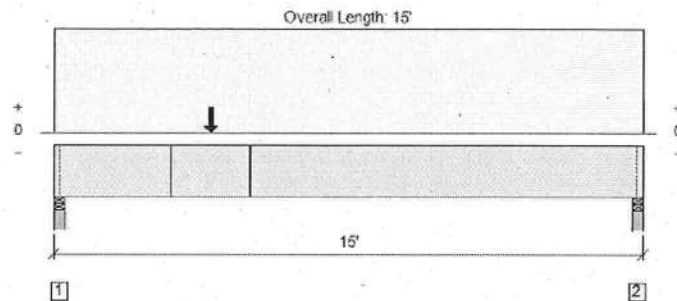
Note that the allowable applied concentrated load is dependent upon the type of hanger used and 16d common nails must be used for face mount hanger installation to achieve the capacities shown.

2. If the bolted connection option is chosen, the 3-ply beam must be checked for the effect of the bolt holes from the 6-bolt connection. This task may be accomplished by utilizing the location analysis feature in iLevel software. Per the detail on page 4, the bolt holes will be located approximately 1' to either side of the concentrated load location. After inputting the span and load information, choose the two locations corresponding to the concentrated load location (±) 1'. Upon completion of the analysis, compare the calculated shear and moment for each of the two locations against the reduced allowable shear and moment capacities of the beam for the bolt pattern selected using Table 3 on page 5 of this bulletin.

See the Forte[®] software output on the following page for results of this analysis. The 3-ply Microllam[®] LVL beam works for the given loading. Refer to the highlighted values for the results at both bolt locations (1' on either side of the load). The highest calculated shear at either location is 6035 lbs. The highest calculated moment is 25,215 ft-lbs. The connection is less than 5d (48" < 14" * 5 = 70") from the end of the member. From Table 3 of this bulletin, the reduced allowable shear for the 3-ply 14" Microllam[®] LVL beam with the 6 bolt pattern located less than 5d from the end of the member is 8,795 lbs while the reduced allowable moment is 31,905 ft-lbs. Therefore, the beam is acceptable with the 6 bolt connection.

- Select the preferred options for both the uniform load connection along the length of the beam and the concentrated load connection at the hanger location and specify both on the drawings including information on fastener type, size, spacing and installation pattern as appropriate.

3 PIECE(S) 1 3/4" x 14" 1.9E Microllam® LVL PASSED



All Dimensions are Horizontal; Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF
Member Reaction (lbs)	7595 @ 2"	7809	Passed (97%)	--
Shear (lbs)	6837 @ 1' 5 1/2"	13965	Passed (49%)	1.0
Moment (Ft-lbs)	25215 @ 4' 11 7/8"	36387	Passed (69%)	1.0
Live Load Defl. (in)	0.356 @ 7' 1 13/16"	0.489	Passed (L/494)	--
Total Load Defl. (in)	0.456 @ 7' 1 7/8"	0.733	Passed (L/386)	--

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Design results assume a fully braced condition where all compression edges (top and bottom) are properly braced to provide lateral stability.
- Bracing (Lu): All compression edges (top and bottom) must be braced at 13' 15/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Total Bearing	Available Bearing	Required Bearing	Support Reactions (lbs) Dead / Floor / Roof / Snow	Accessories
1 - Stud wall - Spruce Pine Fir	3.50"	3.50"	3.40"	1641 / 5955 / 0 / 0	Blocking
2 - Stud wall - Spruce Pine Fir	3.50"	3.50"	2.33"	1164 / 4045 / 0 / 0	Blocking

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 15'	1'	100.0	400.0	0.0	0.0	10' Floor
2 - Point(lb)	4'	N/A	1000	4000	0	0	

Location Analysis	Shear (lbs) Actual / Allowed / LDF	Moment (Ft - lbs) Actual / Allowed / LDF	Comments
1 - 3'	6035 / 13965 / 1.0	19186 / 36387 / 1.0	BOLTS LEFT OF HANGER
2 - 5'	39 / 12569 / 0.9	25215 / 36387 / 1.0	BOLTS RIGHT OF HANGER

Forte™ Software Operator	Job Notes

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Page 1 of 1

Alternative Fall Protection Plans – Planes

After conducting a JHA, if the qualified person is able to demonstrate that conventional fall protection measures are infeasible (3) or otherwise impractical (4) to a particular worker or the entire crew, an employer may develop and implement an alternative fall protection plan in compliance with residential construction fall protection under 29 CFR 1926.501(b)(13).

The fall protection plan's alternative measures must apply to sufficiently trained and experienced workers (5), and the plan must meet the requirements of 29 CFR 1926.502(k) and be site-specific. The use of alternative measures shall be used in conjunction with conventional fall protection systems (6), and the use of alternative methods shall be as limited as possible.

Ground Assembly

Pro-assemble a truss system on the ground. Fully laterally restrain and diagonally brace, per BCS1-B1* and BCS1-B2*. The bottom chord and web member planes. Completely brace, per BCS1-B1* and BCS1-B2*, or sheath the top chord plane, for adequate stability. Lift and set in place. This pre-assembled section may then be used as an attachment point for personal fall restraint anchorage.



Definitions

(1) Under 29 CFR 1926.503(a)(2), a qualified person is one who should have knowledge, and be able to provide training to others, in the following areas: "the nature of fall hazards in the work area; the correct procedures for erecting, maintaining, dismantling, and inspecting the fall protection systems to be used; the use and operation of guardrail systems, personal fall arrest systems, safety net systems, warning line systems, personal monitoring systems, controlled access zones, and other protection to be used; the role of each employee in the safety monitoring system when this system is used; the limitations on the use of mechanical equipment during the performance of roofing work on low-sloped roofs; the correct procedures for the handling and storage of equipment and materials and the erection of overhead protection; and, the role of employees in fall protection plans." (2) Commentary E5.4.2.2 of ANSI/ASSE Z359.2-2007 states, "The impact of fall forces on beams, columns and their supports other than anchorages are not addressed by this standard."

(3) Under 29 CFR 1926.500(b), infeasible means "that it is impossible to perform the construction work using a conventional fall protection system (i.e., guardrail system, safety net system, or personal fall arrest system) or that it is technologically impossible to use any one of these systems to provide fall protection."

(4) 29 CFR 1926.501(b)(12)(ii) states, "There is a presumption that it is infeasible and will not create a greater hazard to implement at least one level of fall protection systems. Accordingly, the employer has the burden of establishing that it is infeasible to implement a fall protection plan which complies with 1926.502(k) for a particular workplace situation, in lieu of implementing any of those systems." (5) 29 CFR 1926.503(a)(1) states, "The employer program shall enable each employee to recognize the hazards of falling and shall train each employee in the procedures to be followed in order to minimize these hazards."

(6) Under 29 CFR 1926.500(b), conventional fall protection systems are: "guardrail system, safety net system, or personal fall arrest system."



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Alternatives de Protección Contra Caídas

Después de realizar una evaluación JHA, si la persona calificada es capaz de demostrar que las medidas convencionales de protección contra caídas son inrealizables (3) o presentan un mayor peligro (4) para un trabajador en particular o a la cuadrilla entera, un empleador puede implementar un plan de protección contra caídas alternativo por escrito en cumplimiento con 29 CFR 1926.501(b)(13) para protección contra caídas en construcción residencial.

Las medidas alternativas del plan de protección contra caídas deberán aplicarse a trabajadores suficientemente capacitados y experimentados (5), y el plan deberá cumplir los requisitos de 29 CFR 1926.502(k) y ser específicas para cada sitio. El uso de medidas alternativas se utilizará en conjunción con sistemas de protección contra caídas (6), y el uso de métodos alternativos será lo más limitado posible.

Montaje en el terreno

Realice el montaje previo de un sistema de entramado en el terreno. Fully laterally restrain and diagonally brace, per BCS1-B1* and BCS1-B2*. The bottom chord and web member planes. Completely brace, per BCS1-B1* and BCS1-B2*, or sheath the top chord plane, for adequate stability. Lift and set in place. This pre-assembled section may then be used as an attachment point for anchorage of restriction personal contra caídas.

Definiciones

(1) Bajo 29 CFR 1926.503(a)(2), una persona calificada es aquella que debe tener conocimientos y aptitud para proporcionar capacitación a otros trabajadores en las siguientes áreas: "la naturaleza de los peligros de caída en el área de trabajo; los procedimientos correctos para la construcción, el mantenimiento, el desmontaje e inspección de los sistemas de protección contra caídas a utilizar; el uso y la operación de sistemas de rampas de protección, sistemas de líneas de advertencia, sistemas de monitoreo de seguridad, zonas de acceso controlado y otra protección a utilizar; el rol de cada empleado en el sistema de monitoreo de seguridad cuando se utiliza este sistema; las limitaciones en el uso de equipo mecánico durante la realización de trabajo en techos de baja pendiente; los procedimientos correctos para el manejo y almacenamiento de equipo y materiales; y la construcción de protección superior; y la función de los empleados en planes de protección contra caídas."

(2) El comentario E5.4.2.2 de ANSI/ASSE Z359.2-2007 establece, "El impacto de las fuerzas de caída sobre vigas, columnas y sus soportes no están abordados por este estándar."

(3) Según 29 CFR 1926.500(b), infeasible significa "que es imposible llevar a cabo el trabajo de construcción usando un sistema convencional de protección contra caídas (por ejemplo, sistemas de rampas de protección, sistema de red de seguridad o sistema de protección personal para detención de caídas), o que es tecnológicamente imposible usar cualquier uno de estos sistemas para proporcionar protección contra caídas."

(4) 29 CFR 1926.501(b)(12)(ii) establece, "Existe una presunción de que es realizable y que no se creará un peligro mayor al implementar al menos uno de los sistemas de protección contra caídas [indicados]. Por consiguiente, el empleador tiene la responsabilidad de establecer que se es apropiado implementar un plan de protección contra caídas que se adhiera a 1926.502(k) para una situación particular en el lugar de trabajo."

(5) 29 CFR 1926.503(a)(1) establece, "El programa del empleador permitirá a cada empleado poder reconocer los peligros de caídas y capacitará a cada empleado en los procedimientos a seguir para minimizar estos peligros."

(6) Según 29 CFR 1926.500(b), los sistemas convencionales de protección contra caídas son: "sistema de rampas de protección, sistema de detención de caídas o sistema de protección personal para detención de caídas."

*Contact the component manufacturer for more information or consult a Professional Engineer for assistance.



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B11 Fall Protection & Trusses

Protección de Caída y Trusses

Regulations on fall protection and erection/installation of trusses in residential construction are contained in OSHA's Fall Protection Standard, 29 CFR 1926 Subpart M (the Standard). Section 1926.501(b)(13) of the Standard states in part: "Each employee engaged in residential construction activities 6 feet or more above lower levels shall be protected by guardrail systems, scot-folding, a safety net system or a personal fall arrest system."

Las regulaciones sobre protección contra caídas y levantamiento/instalación de entramados en construcción residencial vienen contenidos en la Regulación de Protección contra Caídas de OSHA, 29 CFR 1926 Subparte M (el Estándar). La sección 1926.501(b)(13) de la regulación establece en parte: "Cada empleado que participe en actividades de construcción residencial a 6 pies o más por encima de niveles inferiores deberán estar protegidos por sistemas de rampas de protección, andamiaje, un sistema de red de seguridad o un sistema de protección personal para detención de caídas."

SBCA has created an alternate fall protection plan template, complete with links throughout the document to the applicable steps to follow in its online step-by-step approach to erecting an initial truss system under OSHA's fall protection requirements. Visit sbcindustry.com/b11



Truss Systems

⚠ DANGER Any part of an inadequately braced or sheathed roof or floor truss system used as an anchorage point for any type of personal fall arrest system is dangerous and will increase the risk of serious injury or death.

⚠ WARNING

Roof and floor trusses that are not properly braced per BCS1 or sheathed are not able to resist lateral impact loads associated with falls. A falling worker attached to an inadequately braced set of trusses could cause all the previously set trusses to collapse in a domino effect.

☑ Refer to BCS1-B1* and BCS1-B2 for recommendations on proper bracing of trusses.



⚠ DANGER Do not walk on unbraced trusses.

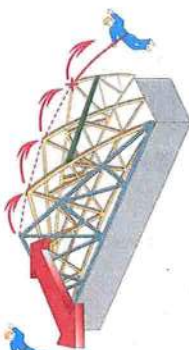
⚠ DANGER Do not stand on Truss overhangs until Structural Sheathing has been applied to the Truss and overhangs.



Consulte BCS1-B1* sobre recomendaciones para levantar correcto de los entramados.

⚠ PELIGRO No camine sobre entramados no apuntalados.

⚠ PELIGRO No se pare en el braguero voladizo hasta revestimiento estructural se ha aplicado a la armadura y voladizos.



- ⚠ DANGER** Do not walk on trusses or gable end frames lying flat, they do not have the structural strength to support a worker safely while oriented flat.



⚠ PELIGRO! No camine sobre entramados ni estructuras de muro pirón dispuestos horizontalmente ya que no poseen la resistencia estructural para poder soportar a un trabajador con seguridad con orientación horizontal.

Site-Specific Job Hazard Assessment

Fall protection and safety measures are jobsite and building specific. The appropriate fall protection method must be determined through a site-specific job hazard assessment (JHA) conducted by a qualified person (1) who can design, install, and use fall protection systems and is authorized to correct any problems. The JHA is intended to assist in identifying risks and identify the least hazardous way to install trusses for a particular job.

Fall hazards identified in the JHA shall be addressed with conventional methods whenever possible, including guardrails, scaffolding, safety nets, personal fall arrest systems or catch platforms.

Employers must consider whether it is safer to design and install a safe work platform/system around a hazard.

- ⚠ WARNING** All fall protection solutions come with their own inherent hazards during use.

- ⚠ CAUTION** Addressing fall protection hazards may be obvious, but other hazards must also be considered when choosing the appropriate site-specific fall protection systems. This includes:

1. electrical hazards, including power lines;
2. projectile hazards while using pneumatic nail guns;
3. tripping hazards from cords and bracing materials; and,
4. lower level hazards, such as wall bracing, which some fall protection systems do not protect against.

Evaluación de Peligros de Trabajos en Sitios Específicos

La protección contra caídas y las medidas de seguridad son específicas para cada sitio de trabajo o edificio. El método apropiado para la protección contra caídas deberá determinarse siguiendo una evaluación de peligros en el trabajo (JHA) específicos, realizados por una persona que esté calificada (1) que pueda diseñar, instalar y utilizar sistemas de protección para caídas, y que esté autorizada para corregir cualquier problema. La JHA tiene el propósito de ayudar en la detección de riesgos e identificar la forma menos peligrosa para instalar entramados para un trabajo en particular.

Los peligros sobre de caídas identificados en la JHA serán tratados con métodos convencionales cuando sea posible como, por ejemplo, rampas de protección, andamiaje, redes de seguridad, sistema de protección personal para detención de caídas o plataformas de agarre.

Los empleados deberán considerar si es seguro diseñar e instalar una plataforma/sistema de trabajo seguro alrededor de un peligro.

⚠ ADVERTENCIA! Todas las soluciones de protección contra caídas vienen con sus propios peligros durante el uso.

⚠ PRECAUCIÓN! La solución de peligros para la protección contra caídas puede ser obvia, pero deberán considerarse otros peligros al elegir los sistemas de protección contra caídas apropiados para un sitio específico. Esto incluye:

1. peligros de carácter eléctrico, como líneas de conducción eléctrica;
2. peligros de objetos lanzados violentamente por el uso de engrapadoras neumáticas tipo pistola;
3. peligros de tropiezos por cables y materiales de punta lamiendo; y,
4. peligros de niveles bajos, como un apuntalamiento de pared, contra lo que no protegen los sistemas de protección contra caídas.

Fall Protection Equipment Installation

- ⚠ DANGER** Individual trusses alone are NOT designed to SUPPORT fall protection equipment.

The Contractor is responsible for the construction means, methods, techniques, sequences, procedures, programs, and safety in connection with the receipt, storage, handling, installation, restraining, and bracing of trusses.

- ☑ Refer to ANSI/ASSE Z359.2-2007, Minimum Requirements for a Comprehensive Managed Fall Protection Program, for guidance in meeting minimum fall protection equipment installation and use requirements. This standard refers to equipment only, and does not apply to the underlying structure to which the equipment is attached (2)

- ⚠ CAUTION!** While the equipment itself may resist the forces generated by a falling worker, it is up to a qualified person to determine whether the building's structural system to which the fall protection equipment is attached meets or exceeds this standard as well.

Scaffolding

Use of interior or exterior scaffolding as a fall arrest system is permitted, but installation and use must adhere to the Standard requirements in 29 CFR 1926.500.



Guard Rails

Use of guard rails along the perimeter of the work area as a fall arrest system is permitted, but installation and use must adhere to the Standard requirements in 29 CFR 1926.500.

Roof Peak Anchors

- ⚠ WARNING** Completely brace, per BCSI-B1* and BCSI-B2* or sheath trusses, before installing a roof anchor for use as personal fall restraint system. Installation and use must adhere to the Standard requirements in 29 CFR 1926.500.



- ⚠ WARNING** It is always safest to sheath a section (e.g. 3 or more trusses) of the roof system before installing a roof anchor for use as a personal fall restraint system. Installation and use must adhere to the Standard requirements in 29 CFR 1926.500.

Instalación de Equipo de Protección Contra Caídas

⚠ PELIGRO! Los entramados individuales por sí solos NO están diseñados para SOPORTAR equipo de protección contra caídas.

El contratista será responsable de los medios de construcción, los métodos, las técnicas, las secuencias, los procedimientos, los programas y la seguridad en lo que se refiere a recepción, almacenamiento, manejo, instalación, restricción y apuntalamiento de entramados.

Consulte ANSI/ASSE Z359.2-2007, Minimum Requirements for a Comprehensive Managed Fall Protection Program (Requisitos mínimos para un programa completo de protección contra caídas) si desea información para el cumplimiento de los requisitos mínimos para la instalación y uso de equipo de protección contra caídas. Esta norma se refiere a equipo solamente, y no es aplicable a la estructura subyacente a la que esté acoplado el equipo. (2)

⚠ PRECAUCIÓN! Aunque el equipo por sí mismo puede resistir las fuerzas generadas por la caída de un trabajador, es responsabilidad de una persona calificada determinar si el sistema estructural del edificio al que se acopla el equipo de protección contra caídas cumple o supera esta regulación también.

Andamiaje

Está permitido el uso de andamiaje interior o exterior como sistema para detención de caídas, pero la instalación y el uso deberá adherirse a los requisitos regulatorios descritos en 29 CFR 1926.500.

Rampas de Protección

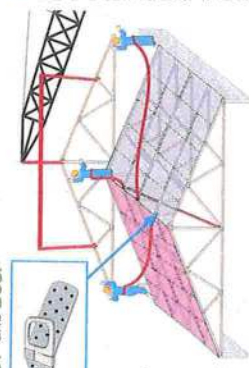
Se permite el uso de rampas de protección a lo largo del perímetro del área de trabajo como sistema para detención de caídas, pero la instalación y el uso deberán adherirse a los requisitos de la regulación que se encuentran en 29 CFR 1926.500.

Anclajes de Tejado

⚠ ADVERTENCIA! Apuntale completamente, según BCSI-B1* y BCSI-B2* o forre los entramados, antes de instalar un anclaje de tejado para su uso como sistema personal de restricción de caídas. La instalación y el uso deberá adherirse a los requisitos regulatorios que se indican en 29 CFR 1926.500.

⚠ ADVERTENCIA!

Siempre es más seguro forrar una sección (por ejemplo, 3 o más entramados) del sistema de tejado antes de instalar un anclaje de tejado para uso como sistema personal de restricción de caídas. La instalación y el uso deberá adherirse a los requisitos regulatorios que se indican en 29 CFR 1926.500.



Field Assembly and Other Special Conditions Ensamblaje de Campo y Otras Condiciones Especiales

Certain sizes or shapes of trusses require some assembly at the jobsite. Refer to the TDDs for specific instructions on assembly methods, unless the construction documents indicate otherwise.

Ciertos tamaños o formas de trusses requieren algún ensamblaje en la obra. Refiera a los dibujos del diseño de truss para instrucciones específicas sobre ensamblaje de montaje, a menos que los documentos de construcción indiquen lo contrario.

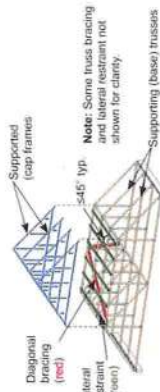
The contractor is responsible for proper field assembly.

El contratista es responsable para el ensamblaje de campo apropiado.

Piggyback Trusses Trusses de Piggyback

The supporting (base) trusses shall be completely installed with all truss member AND permanent building stability bracing, as required, BEFORE installing the supported (cap) frames.

Los trusses de apoyo serán completamente instalados con todos los miembros de truss y el bracing permanente requerido antes de la construcción como es requeriendo ANTES de instalar los armazones ("cap") soportados.



WARNING Adequately restrain and brace the flat portion of the supporting trusses to prevent buckling.

ADVERTENCIA Restringir y amarrar adecuadamente la parte plana de los trusses soportantes para prevenir el fallar.

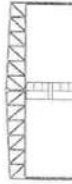
For more information on how to permanently laterally restrain and diagonally brace piggyback truss assemblies refer to BCSI-B3.

Para más información sobre como restringir lateralmente permanentemente y amarrar diagonalmente sistemas de trusses de piggyback referre al BCSI-B3.

Field Spliced Trusses Trusses Empalmados En Campo

Splicing can be performed on the ground before installation or the truss sections can be supported by temporary shoring after being hoisted into place and the splices installed from a safe working surface.

Empalmado puede ser hecho en el suelo antes de la instalación o las secciones de trusses pueden ser soportados por "shoring" temporal después de que sean levantados en lugar y los empalmes son instalados de una superficie de trabajo segura.



Contact the component manufacturer for more information or consult a registered design professional for assistance.

Para ver a non-printing PDF of this document, visit abnindustry.com/b2.

This document summarizes the information provided in Section B2 of the 2013 Edition (updated March 2015) of Building Component Safety Information BCSI - Guide to Good Practice for Handling, Installing, Restraint & Bracing of Metal Plate Connected Wood Trusses. Copyright © 2014 - 2016 Structural Building Components Association and Trade Plate Institute. All Rights Reserved. This guide or any part thereof may not be reproduced in any form without the written permission of the publishers. Printed in the United States of America.

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Valley Set Frame Installation Instalación de Armazón de Conjunto de Valle

The top chords of the supporting trusses must be braced properly. This can be accomplished by attaching structural sheathing or with rows of lateral restraint, spaced no more than the maximum on center spacing specified on the TDD, and diagonal bracing.

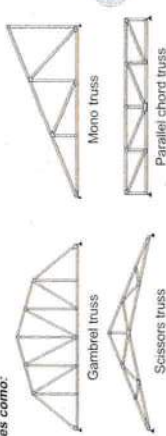
Las cuerdas superiores de los trusses de soporte tienen que ser amarrados apropiadamente. Esto puede ser hecho por sujetar el enablado estructural o con filas de restricción lateral, espaciados no más que el máximo en el espaciamiento del centro que es especificado en el dibujo del diseño de truss, y amosre diagonal.



Valley sets as over-framing

This document applies to all sloped and flat chord trusses built with the wide-face of the lumber oriented vertically. Examples include:

Este documento aplica a todos trusses de cuerda pendiente y plana construidos con la cara-ancha de la madera orientada verticalmente, tales como:



For parallel chord trusses manufactured with 3x2 or 4x2 lumber, see BCSI-B7.

Para los trusses de cuerdas paralelas fabricados con madera 3x2 o 4x2, vea el resumen BCSI-B7.



For trusses spaced more than 2'-0" on-center, see BCSI-B10.

Para los trusses espaciados más de 2 pies en el centro, vea el Resumen BCSI-B10.

B2 Truss Installation & Temporary Restraint/Bracing Instalación de Trusses & Restricción/Arriostre Temporal

For Trusses Up to 2'-0" On-Center and 80'-0" in Length Para Trusses Hasta 2 Pies En Centro y Hasta 80 Pies De Longitud

WARNING Spans over 60' require more complex temporary installation restraint/bracing. Consult a registered design professional.

ADVERTENCIA! Los tramos más de 60 pies requieren restricción/arriostre de instalación temporal más complejo. Consulte a un profesional registrado de diseño.

DANGER Disregarding handling, installing, restraining and bracing safety recommendations is the principal cause of truss erection/installation accidents.

PELIGRO El no seguir las recomendaciones de manejo, instalación, restricción y arriostre es la causa principal de los accidentes durante la erección/instalación de trusses.

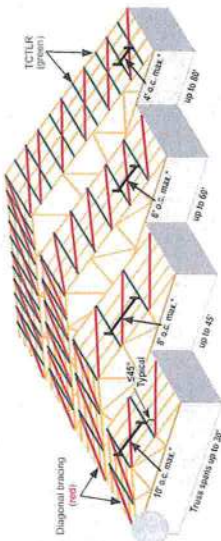


NOTICE

Lateral restraint is NOT adequate without diagonal bracing. La Restricción Lateral NO es adecuada sin el Arriostre Diagonal.

- Always diagonally brace for safety! (Siempre arriostre diagonalmente para seguridad!)

Maximum Spacing For Top Chord Temporary Lateral Restraint (TCLTR) El Espaciamiento Máximo Para La Restricción Lateral Temporal (TCLTR)



The graphic at left shows the maximum on-center spacing (see - at left) of TCLTR based on truss span from the table in Step 2 on page 2.

- Ground bracing not shown for clarity.
- Apply diagonal bracing or structural sheathing immediately. For spans over 60' applying structural sheathing immediately is the preferred method.

El dibujo a la izquierda muestra el espaciamiento máximo en el centro (vea - a la izquierda) del TCLTR basado en los tramos de trusses de la tabla en el Paso 2 en la página 2.

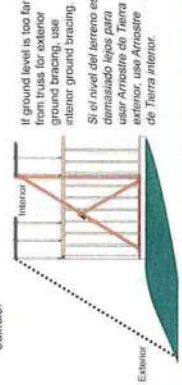
- No se muestra el arriostre de tierra para claridad.
- Aplicar inmediatamente el Arriostre Diagonal o el Enablado Estructural (structural sheathing). Para tramos más de 60 pies el método preferido es enblarlos inmediatamente.

Check These Items Before Starting Truss Erection/Installation and Correct as Needed Revise Estos Puntos Antes De Empezar La Ección/Instalación y Corríjalos Cuando Es Necesario

- Building dimensions match the construction documents. Las dimensiones del edificio concuerdan con los documentos de construcción.
- Bearing supports (e.g., walls, columns, headers, beams, etc.) are accurately and securely installed, plumb and properly braced. Los soportes que sostienen cargas (ej., paredes, columnas, vigas de cabecera, vigas, etcétera) son instalados seguramente y con precisión, y son nivelados y amarrados apropiadamente.
- Hangers, tie-downs, restraint and bracing materials are on site and accessible. Los colgadores (hangers), soportes de anclaje (tie-downs) y materiales de restricción y arriostre están accesibles en la obra.
- Erection/installation crew is aware of installation plan and lateral restraint/diagonal bracing requirements. El personal de erección/instalación es consciente del plan de instalación y los requisitos de restricción/arriostre.
- Multi-ply trusses, including girders, are correctly fastened together prior to lifting into place. Los trusses de varias capas, incluyendo travesseros, son fijados juntos correctamente antes de levantarlos en lugar.
- Any truss damage is reported to truss manufacturer. Refer to BCSI-B5. Do not install damaged trusses unless instructed to do so by the building designer, truss designer or truss manufacturer. Alguien daño a los trusses ha sido reportado al fabricante de trusses. Vea el resumen BCSI-B5. No instale trusses dañados a menos que se dieren el diseñador del edificio, el diseñador del truss o el fabricante del truss.

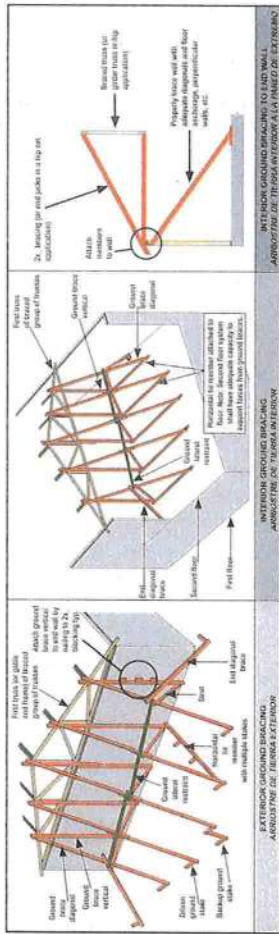
- Trusses are the correct dimension. Los trusses son la dimensión correcta.
- Tops of bearing supports are flat, level and at the correct elevation. La parte superior de los soportes de cojinete son planas, niveladas y a la elevación correcta.
- Jobsite is clean and neat, and free of obstructions. La obra está limpia, ordenada y sin obstrucciones.

- Ground bracing procedure for first truss is based on site and building configuration. El procedimiento de Arriostre de Tierra para el primer truss es basado en el terreno y la configuración del edificio.

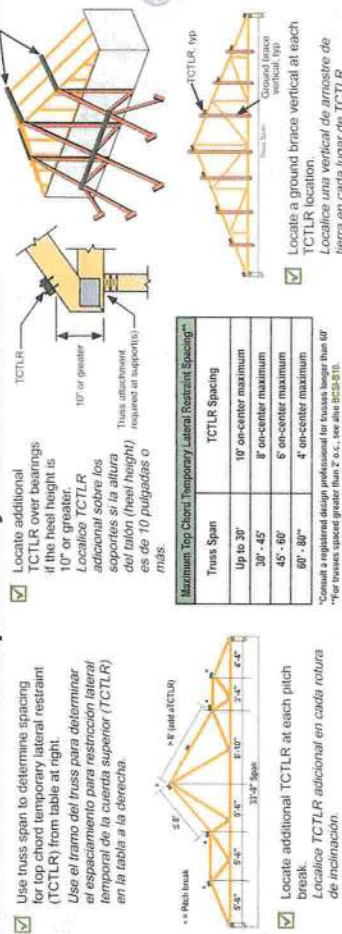


Steps to Setting Trusses Pasos Para El Montaje De Trusses

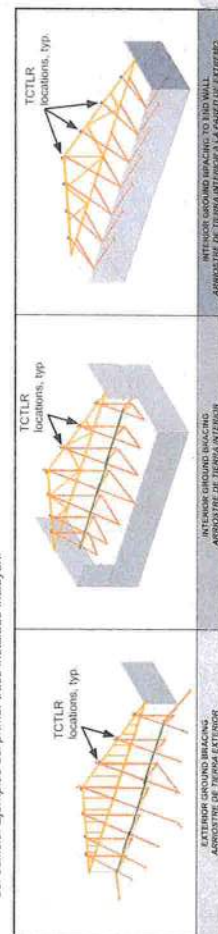
1. Establish Ground Bracing Procedure: Exterior or Interior Establezca el Procedimiento de Arriostre de Tierra: Exterior o Interior



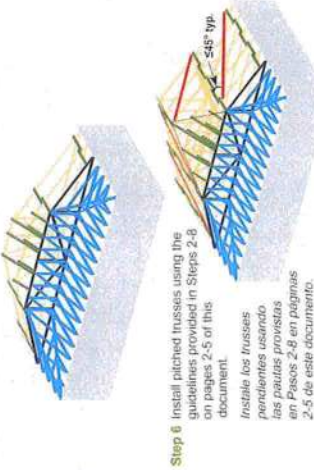
2. Determine the Locations for TCTLR and Ground Braces Determine las Ubicaciones para TCTLR y los Arriostres de Tierra



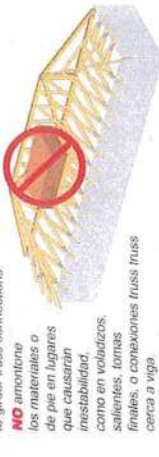
3. Set First Truss and Fasten Securely to Ground Braces Coloque el Primer Truss y Fíjelo Seguramente a los Arriostres de Tierra



CAUTION First truss must be attached securely to all bearings and to all required ground braces prior to removing the hoisting supports.
PRECAUCIÓN El primer truss tiene que ser sujeción seguramente a todos soportes y a todas arriostres de tierra requeridos, antes de quitar los soportes de levantamiento.



DO NOT stack materials or stand at locations that will cause instability, such as on cantilevers, overhangs, end jacks, or near truss-to-ground truss connections.



Long Span Truss Installation Instalación de Trusses de Tramos Largos

DANGER Trusses with clear spans of 60 feet or greater are extremely dangerous to install and require more detailed safety and handling measures than shorter span trusses.
PELIGRO Trusses de 60 pies de largo o más son extremadamente peligrosas a instalar y requieren medidas de seguridad y manejo más detalladas que trusses de tramos más cortos.

Before Starting Antes de Empezar

- Hire a registered design professional to design the temporary restraint/bracing, the permanent individual truss member restraint/bracing and to supervise the erection process. This is required by some building codes.
- Contrate un Profesional Registrado de Diseño para proveer un plan de restricción/arriostre temporal de los permanentes miembros individuales de braguero y para supervisar el proceso de erección. Esto es el requisito en algunos códigos de construcción.
- Develop a safe, effective truss installation method and inform all crew members of their roles.
- Elabore un método seguro y efectivo de la instalación de trusses e informe todos miembros del personal de sus papeles.
- Use personnel who have experience installing trusses 60' and greater in span.
- Use instaladores quienes tienen experiencia en instalar trusses de 60 pies o más de tramo.
- Ensure that the walls and supporting structure are stable and adequately restrained and braced.
- Asegúrese que las paredes y la estructura soportante son estables y adecuadamente restringidas y arriostreadas.
- Have all necessary lifting equipment and building materials on hand.
- Make sure the crane operator understands the special hoisting requirements of long span trusses (see BCSI-B1).
- Tenga todo el equipo de levantar y los materiales de construcción necesarios a la mano. Asegúrese que el operador de grúa entiende los requisitos especiales de levantar los trusses de tramos largos (vea BCSI-B1).

Tips For Safe and Efficient Installations Consejos para Instalaciones Seguras y Eficientes

- Assemble the first five trusses with all structural sheathing, restraint and bracing, either as the trusses are installed or built as a unit on the ground and lifted into place.
- Ensamble los primeros cinco trusses con todo el entablado estructural, restricción y arriostre, cualquier cuando los trusses son instalados o son construidos como una unidad en la tierra y levantados en lugar.
- Keep trusses straight during hoisting to minimize bending out-of-plane.
- Mantenga los trusses derechos durante el levantamiento para minimizar el doblado fuera-de-plano.
- Add a temporary center support to provide greater stability and leave in place until all permanent building stability bracing is installed.
- Agregue un soporte temporal en el centro para proveer más estabilidad y sueldado en lugar hasta que todo el arriostre de estabilidad permanente de construcción es instalado.



- Apply structural sheathing to the top chord as the trusses are being installed.
- Aplique el entablado estructural a la cuerda superior mientras los trusses están instalados.



- Install all permanent individual truss member restraint and bracing immediately.
- Instale todo el arriostre y restricción permanente de miembros de trusses individuales inmediatamente.

Alternate Installation Method: Build it On the Ground and Lift it Into Place

Método Alternativo de Instalación: Construirlo en la Tierra y Levantarlo en Lugar

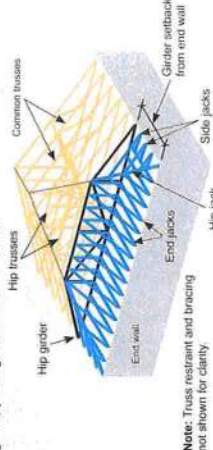
- ✓ Ensure level bearing, set, position, plumb and properly restrain and brace the trusses as modules on the ground. Asegúrese que el soporte es nivelado, colocar, posicionar, nivelar y restringir y arriostrar apropiadamente el grupo de trusses en la tierra.
- ✓ Apply structural sheathing for stability. Begin at the heel and alternate 4'x8' and 4'x4' sheets up to the peak. Aplique el entablado estructural para estabilidad. Empezar en el talón y alternar láminas de 4 pies x 8 pies y 4 pies x 4 pies hasta el pico.
- ✓ Be sure to install all top chord, web member and bottom chord lateral restraint and bracing prior to lifting the module. Asegúrese de instalar toda la restricción y arrioste de la cuerda superior, el miembro secundario, y la cuerda inferior antes de levantar el módulo.
- ⚠ **CAUTION** Additional restraint and bracing may be required to safely lift units into place. **¡CUIDADO!** La restricción y arrioste adicional pueden ser requeridos para levantar los módulos en sitio sin peligro.
- ✓ Be sure to get proper guidance from a registered design professional to insure modules are designed and installed safely and properly. Asegúrese de obtener consejos apropiados por un profesional registrado de diseño para asegurar que los módulos son diseñados y instalados apropiadamente y sin peligro.

Hip Set Assembly and Bracing

Ensamblaje y Arrioste del Grupo de Cadera

Permanently connecting the end jacks to the end wall and girder as early in the installation process as possible dramatically increases the stability of the hip girder and the safety of the structure.

Conectando permanentemente los gatos extremos a la pared extremo y el travieso tan temprano en el proceso de instalación como sea posible aumenta drásticamente la estabilidad del travieso de cadera (hip girder) y la seguridad de la estructura.



Note: Truss restraint and bracing not shown for clarity.

Use the following sequence:

Use la siguiente secuencia:

- Step 1** Position the hip girder on the bearing walls at the specified end wall setback and permanently attach to supports.

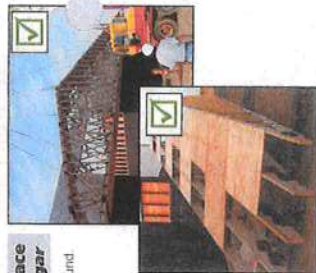
Posicione el travieso de cadera en las paredes de soporte al "setback" especificado de la pared extremo y sujete a los soportes permanentemente.

NOTE: Whenever possible, connect multi-ply girder trusses together and install the end jack hangers (if required) prior to erection/ installation. All piles of a multi-ply girder shall be attached per the fastening schedule on the TDD before attaching any framing members or applying any loads.

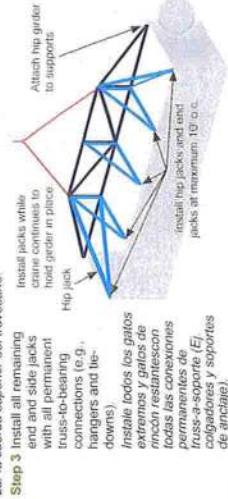
Siempre que sea posible, conecte los trusses de travieso de varias capas juntos y instale los colgadores de gato extremo (si es requerido) antes de la erección/instalación. Todas las capas de un travieso de varias capas sujetarán por el horario de fijar en el dibujo del diseño de truss antes de sujetar algunos miembros de arrioste o de aplicar algunas cargas.

- Step 2** Install hip jacks, and end jacks at maximum 10' on-center while crane continues to hold girder in place.

Instale los gatos de cadera y los gatos extremos a 10 pies máximo en centro mientras la grúa continúa mantener el travieso en lugar. La conexión a la top chord es importante as it helps to stabilize the top chord of the girder.



Sujete los trusses de la cuerda superior y la cuerda inferior del truss de travieso. La conexión por encima es importante porque ayuda a estabilizar la cuerda superior del travieso.



- Step 4** Install the next hip truss with 2x4 short member temporary lateral restraints. Attach each short member temporary lateral restraint to the top chord of the hip girder and adjacent hip truss with two nails at each connection. The short member temporary lateral restraints should be long enough to extend at least 1 1/2' past the top chord of each truss. Place short member temporary lateral restraints at pitch breaks and at intervals along the top chords in accordance with Step 2 on page 2.



Instale el próximo truss de cadera con 2x4 restricciones temporales laterales. Conecte cada miembro corto temporal a la restricción lateral temporal del miembro corto a la cuerda superior del travieso de cadera y al truss de cadera adyacente con dos clavos a cada conexión. Los miembros cortos temporales deben ser bastante largo para extender a un mínimo de 1 1/2' pasado de la cuerda superior de cada truss. Pese la restricción lateral temporal de los miembros cortos a roturas del pendiente y a intervalos por las cuerdas superiores según el Paso 2 en la página 2.

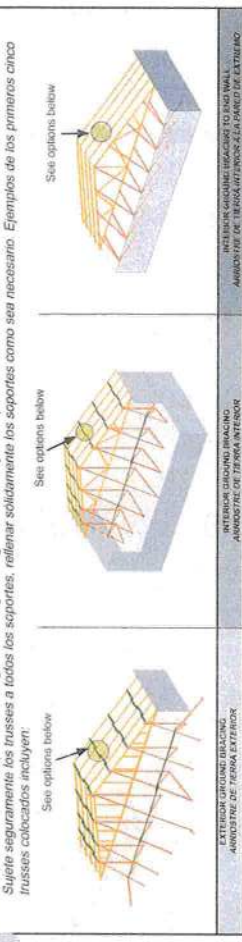
Step 5 Install remaining hip trusses. For the flat portion of each hip truss, use short member temporary lateral restraints that are at least double the length of the first set of short member temporary lateral restraints (see Figure on next page). For the sloped chords of the trusses, install short member temporary lateral restraints according to one of the three options in Step 4 on page 3.

Instale los restantes de los trusses de cadera. Para la parte plana de cada truss de cadera use restricciones laterales temporales de miembros cortos que son por lo menos dobles a la longitud del primer conjunto de restricciones laterales temporales de miembros cortos (ver la figura en la página siguiente). Para las cuerdas pendientes de los trusses, instale las restricciones laterales temporales de miembros cortos según uno de las tres opciones en Paso 4 en la página 3.

4. Set Trusses 2, 3, 4 & 5 with TCTLR in Line with Ground Bracing

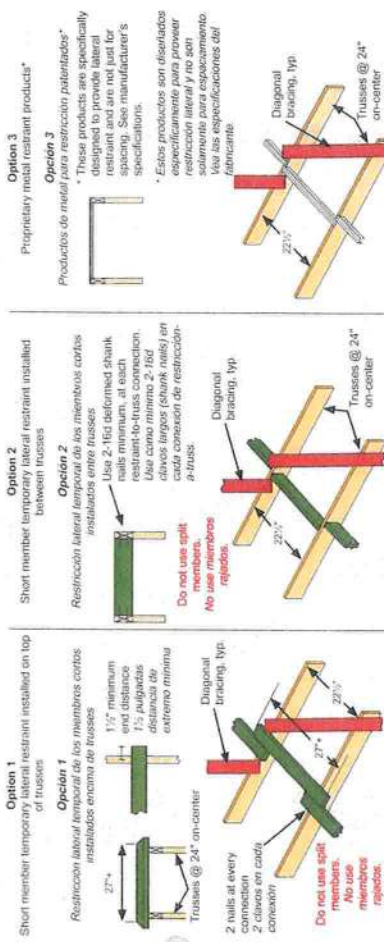
4. Coloque los Trusses 2, 3, 4 y 5 con TCTLR en Línea con los Arriostres de Tierra

Attach trusses securely at all bearings, shimmed bearings as necessary. Examples of first five trusses set include: Sujete seguramente los trusses a todos los soportes, rellenar solamente los soportes como sea necesario. Ejemplos de los primeros cinco trusses colocados incluyen:



NOTICE The following three (3) Short Member Temporary Lateral Restraint options require that the diagonal bracing be installed continuously. See figure for Option B in Step 8 on page 5.

Las siguientes tres opciones para instalar la Restricción Lateral Temporal de los Miembros Cortos requieren que el arrioste diagonal esté instalado continuamente. Vea la figura de Opción B en el Paso 8 en la página 5.



CAUTION Each truss must be attached securely at all bearing locations and all TCTLR installed before removing the hoisting supports.

¡CUIDADO! Cada truss tiene que ser sujetado seguramente en todas ubicaciones de soportes y todas las TCTLR instaladas antes de quitar los soportes de levantar.

LATERAL RESTRAINT/BRACING MATERIAL AND CONNECTIONS

CONEXIONES Y MATERIALES DE RESTRICCIÓN/ARRIOSTRE LATERAL

- ✓ Minimum size of bracing and lateral restraint material is 2x4 stress-graded lumber or approved proprietary metal restraint/bracing, unless otherwise specified by the building designer. El tamaño del material de restricción lateral y arrioste debe ser por lo menos 2x4 madera graduada por esfuerzo o restricción/arrioste de metal patentado aprobado, a menos que especifique el diseñador del edificio.
- ✓ All bracing and lateral restraint members must be connected to each truss with at least 2 nails (see minimum sizes below), except for the short member restraints shown in Step 4, Option 2 (see above), which require 2-16d deformed-shank (i.e., ring- or screw-shank) nails. Todos los miembros de restricción lateral y arrioste tienen que ser conectados a cada truss con un mínimo de 2 clavos (ver los tamaños mínimos de clavos abajo). Excepto para las restricciones de miembros cortos mostrados en el Paso 4, Opción 2 (vea arriba), cuales requieren 2-16d clavos con largos deformados (E). Largos de anillos o tornillos.
- ✓ Drive nails flush. If the temporary restraint and bracing is to be removed when the permanent bracing is installed, use double-headed nails for ease of removal. Clave los clavos al raso. Si la restricción y arrioste temporal será quitado al instalació del arrioste permanente, use clavos de dos cabezas para quitarlos más fácilmente.



5. Install Top Chord Diagonal Bracing

7. Install Bottom Chord Lateral Restraint and Diagonal Bracing

Attach diagonal bracing to the first five trusses. Examples of diagonal bracing on first five trusses include:

Coloque el arriostre diagonal a los primeros cinco trusses. Ejemplos de arriostre diagonal en los primeros cinco trusses incluyen:

See Short Member Temporary Lateral Restraint options page 3.

See Short Member Temporary Lateral Restraint options page 3.

See Short Member Temporary Lateral Restraint options page 3.

EXTERIOR GROUND BRACING - ARRIOSTRE DE TIERRA EXTERIOR

INTERIOR GROUND BRACING - ARRIOSTRE DE TIERRA INTERIOR

Bottom chord temporary lateral restraint and diagonal bracing stabilizes the bottom chord plane during installation and helps maintain proper spacing of the trusses.

La restricción lateral temporal y el arriostre diagonal de la cuerda inferior estabilizan el plano de la cuerda inferior durante la instalación y asiste en mantener el espaciado apropiado de los trusses.

Install rows of temporary lateral restraint at 15' on-center maximum. Remove, if desired, after the permanent ceiling diaphragm is in place.

Instale filas de restricción lateral temporal a 15 pies en el centro como máximo. Quételes, si así lo desea, después que el diafragma permanente del techo esté colocado.

Install rows of permanent lateral restraint at 10' on-center maximum. Required spacing may be less; check the TDD and/or with the building designer.

Instale filas de restricción lateral permanente a 10 pies en el centro como máximo. El espaciado especificado puede ser menor; verifique el dibujo del diseño de truss con el diseñador del edificio.

Install diagonal bracing at intervals of 20' maximum along the run of trusses.

Instale el arriostre diagonal a intervalos de 20 pies máximo a lo largo de la línea de trusses.

CAUTION Do not remove ground bracing until all top chord, bottom chord and web lateral restraint and bracing is installed on at least the first five trusses.

CAUTELA No remueva el arriostre de tierra hasta que toda la restricción lateral y arriostre de la cuerda superior, la cuerda inferior y los miembros secundarios estén instalados en por lo menos los primeros cinco trusses.

6. Install Web Member Diagonal Bracing

8. Repeat Steps 4 Through 7 with Groups of Four Trusses Using Option A or B

Temporary web member diagonal bracing acts with the top chord and bottom chord temporary lateral restraint and diagonal bracing to form triangulation perpendicular to the plane of the truss and prevents trusses from leaning or dominating.

El arriostre diagonal temporal de los miembros secundarios trabaja con la restricción lateral y el arriostre diagonales temporales de la cuerda superior e inferior para formar una triangulación perpendicular al plano del truss y evita que los trusses se inclinen o caigan como domos.

Install diagonal bracing at about 45° to the horizontal on web members (verticals whenever possible) located at or near rows of bottom chord lateral restraint. Web diagonal bracing must extend from the top chord to the bottom chord. Repeat at the intervals shown in the figure below.

Instale el arriostre diagonal a aproximadamente 45 grados en los miembros secundarios (verticales cuando sea posible) colocados en o cerca de las filas de restricción lateral de la cuerda inferior. Arriostre diagonal para los miembros secundarios tiene que extender de la cuerda superior a la cuerda inferior. Repita a los intervalos mostrados en la figura a la derecha.

NOTICE The requirements for web permanent individual truss member restraint are specified on the truss design drawing (TDD). Refer to BCSI-B3 for more information.

Los requisitos para la restricción permanente de miembros individuales de truss para miembros secundarios son especificados en el dibujo del diseño de truss. Vea el resumen BCSI-B3 para más información.

Note: Same spacing as bottom chord lateral restraint

Note: Some chord and web members not shown for clarity.

Option A: Install the next four trusses using short member temporary lateral restraint Options 1 - 3 per Step 4 on page 3. Add long-length (minimum 2x4x12) continuous lateral restraint (CLR) to tie trusses together between the CLR at least 2 trusses. Install diagonal bracing at intervals of 20' maximum along the run of trusses (see Figure below).

Instale los siguientes cuatro trusses utilizando las opciones 1-3 por el Paso 4 en la página 3 de la restricción lateral de miembros cortos. Añada la restricción lateral continua (CLR) de longitud-largo (mínimo 2x4x12) para atar junto los trusses. Solaparse las puntas del CLR a mínima de dos trusses. Instale el arriostre diagonal cada 20 pies máximo a lo largo de la línea de trusses (vea la Figura abajo).

NOTICE The maximum diagonal brace spacing provided in this Option assumes ground bracing is properly installed and in place. El espaciado máximo de arriostre diagonal en esta Opción asume que el arriostre de tierra está en sitio y es instalado correctamente.

Diagonal bracing required at each end of building between each row of TCLR, and every 10 truss spaces (20' max.).

Arriostre diagonal requerido en cada extremo del edificio entre cada fila de TCLR, y cada 10 trusses espacios (20' max.).

Note: Ground Bracing not shown for clarity.

Spacing between rows of TCLR is dependent on truss span and pitch breaks (see Step 2 on page 2).

Temporarily Diagonal Bracing repeated entire length of building.

ENSURE THAT ALL TRUSSES ARE PROPERLY DIAGONALLY BRACED. ASEGÚRESE QUE TODOS LOS TRUSSES ESTÉN ARRIOSTRADOS DIAGONALMENTE APROPIADAMENTE.

- Apply structural sheathing early and often.
- DO NOT wait until all trusses are set to apply structural sheathing.
- Aplique el entablado estructural temprano y con frecuencia.
- NO ESPERE hasta que todos los trusses estén colocados para aplicar el entablado estructural.
- Remove only as much top chord temporary lateral restraint and diagonal bracing as necessary to nail down the next sheet of structural sheathing. DO NOT erode truss design load with construction loads. (SEE BCSI-B4)
- Quite solo tanta restricción lateral temporal y arriostre diagonal de la cuerda superior como sea necesario para clavar la siguiente hoja de entablado estructural. NO EXCEDA la carga de diseño de truss con cargas de construcción. (VEA BCSI-B4)

NOTICE Mono trusses, deep flat trusses and other types of trusses with deep ends also require temporary lateral restraint and diagonal bracing on the vertical web member at the deep end of the truss.

Los trusses de una sola pendiente, trusses planos y profundos y otros tipos de trusses con extremos profundos también requieren restricción lateral temporal y arriostre diagonal en los miembros secundarios largos al parte profundo del truss.

Delivery

		Sold To: Mike Todd Const.		AppWrt Job No. 970576 OnLine Job No. 576 P.T. Sales No		Ship Date 1/20/2016 Quote Account PO	
				Req'd Engineering		Req'd Layouts	
Plant: Lake City Truss 2525 E Duval Street Lake City, FL 386-755-6894		SubDvsn: MORGAN HOUSE, FL Lot: N/A Model/Elev: CUSTOM / GABLE Options Shipment Roof		Job Contacts Site Office Name TIM Phone Fax			
Sales Rep KIMBER HOLLOWAY Sales Area Dist Center Designer Holloway, Kim		Directions					

	Truss ID	Quantity	Type	Slope (Depth)	Left	Right	Stub	Height Weight/Ply	Total Weight
	F04 15-3-12	9	FLOOR	Depth: 12"					
	F05 15-0-12	4	FLOOR	Depth: 12"					
	F06 14-8-4	9	FLOOR	Depth: 12"					
	F07 14-5-4	4	FLOOR	Depth: 12"					
	F08 7-9-0	4	FLOOR	Depth: 12"					
	F09 7-0-0	4	FLOOR	Depth: 12"					
Total Trusses:		34							

Miscellaneous Items:	Quantity	Item	Description
	44	LUS46	4x Hanger (LUS46)

	Trusses	Jacks	Beams	Hangers
Piggy Backs	0	0	0	44
Valleys	0	0	0	0
Floors	34	Trim-It	0	Misc
				0

Delivery Person Delivered By: _____ Date: _____ Remarks:	On Site Receiver Received By: _____ Date: _____ Remarks:
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Delivery

		Sold To: Mike Todd Const.		AppWrt Job No. 970576		Ship Date 1/20/2016	
				OnLine Job No. 576		Quote	
				P.T.		Account	
				Sales No		PO	
Plant: Lake City Truss 2525 E Duval Street Lake City, FL 386-755-6894		SubDvsn: MORGAN HOUSE, FL Lot: N/A Model/Elev: CUSTOM / GABLE Options Shipment Roof		Req'd Engineering		Req'd Layouts	
Sales Rep KIMBER HOLLOWAY Sales Area Dist Center Designer Holloway, Kim		Directions		Job Contacts Name TIM Phone Fax		Site Office	

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	F04 15-3-12	9	FLOOR	Depth: 12"					
	F05 15-0-12	4	FLOOR	Depth: 12"					
	F06 14-8-4	9	FLOOR	Depth: 12"					
	F07 14-5-4	4	FLOOR	Depth: 12"					
	F08 7-9-0	4	FLOOR	Depth: 12"					
	F09 7-0-0	4	FLOOR	Depth: 12"					
Total Trusses:		34							

Miscellaneous Items:	Quantity	Item	Description
	44	LUS46	4x Hanger (LUS46)

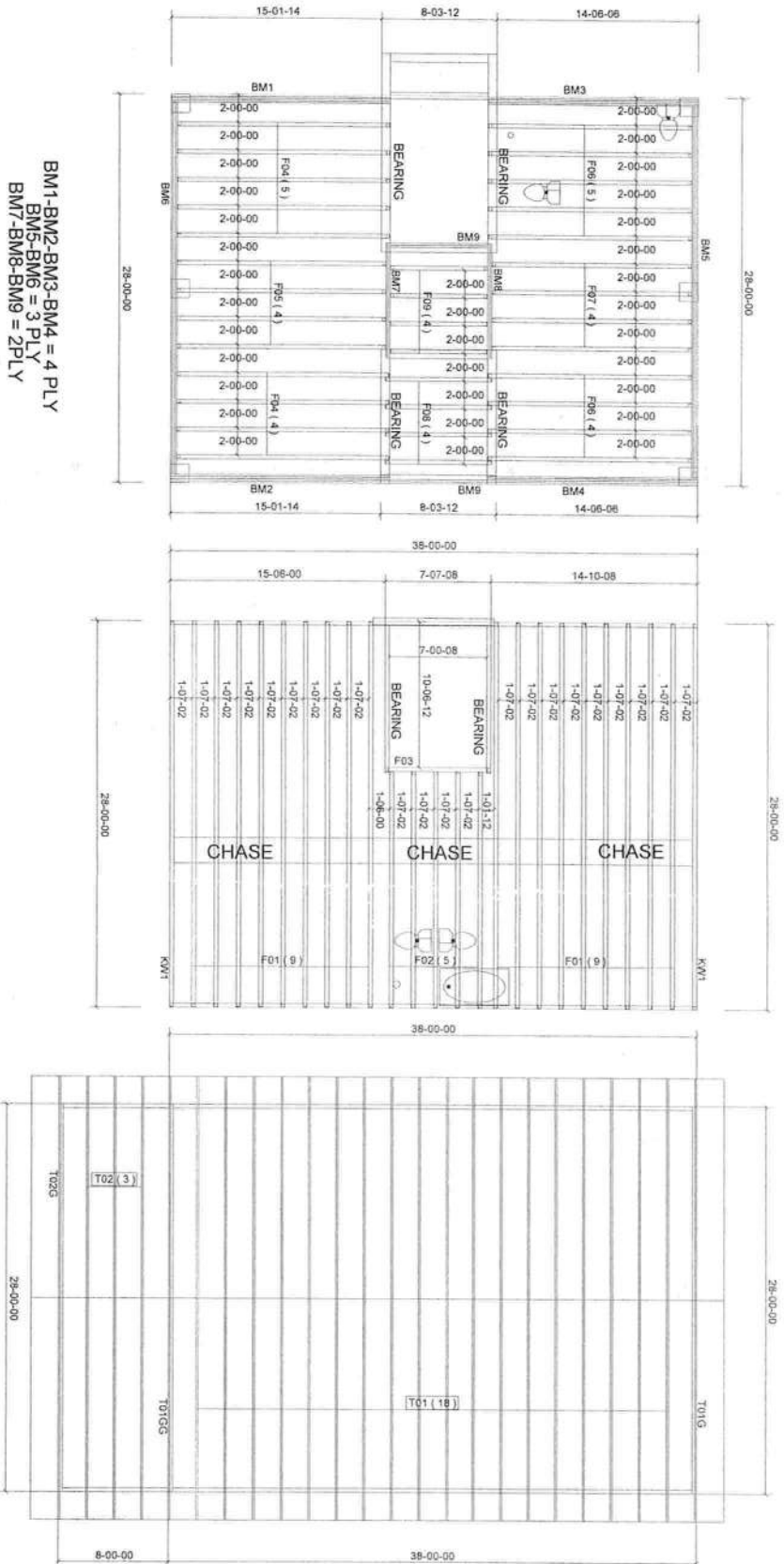
	Trusses	Jacks	Beams	Hangers
Piggy Backs	0	0	0	44
Others	0	0	0	0
Valleys	0	34	0	0

Delivery Person Delivered By: _____ Date: _____ Remarks: _____	On Site Receiver Received By: _____ Date: _____ Remarks: _____
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14" DEEP BEAMS
12" DEEP FLOORS
24" O/C

20" DEEP FLOOR
19.2" O/C

5.5/12 PITCH
24" O/H



BM1-BM2-BM3-BM4 = 4 PLY
BM5-BM6 = 3 PLY
BM7-BM8-BM9 = 2 PLY

MITEK PLATE APPROVAL #'S 2197.2 - 2197.4, WEYERHAUSER PRODUCT #'S 1630.2 - 1630.10

BEARING HEIGHT SCHEDULE

	8' 1-1/2"
	9' 1-1/2"
	10' 1-1/2"
	11' 1-1/2"
	12' 1-1/2"

NOTES:

- 1) ALL BEARING HEIGHTS ARE TO BE MEASURED TO THE TOP OF THE BEARING SURFACE.
- 2) ALL BEARING HEIGHTS ARE TO BE MEASURED TO THE TOP OF THE BEARING SURFACE.
- 3) ALL BEARING HEIGHTS ARE TO BE MEASURED TO THE TOP OF THE BEARING SURFACE.
- 4) ALL BEARING HEIGHTS ARE TO BE MEASURED TO THE TOP OF THE BEARING SURFACE.
- 5) ALL BEARING HEIGHTS ARE TO BE MEASURED TO THE TOP OF THE BEARING SURFACE.
- 6) ALL BEARING HEIGHTS ARE TO BE MEASURED TO THE TOP OF THE BEARING SURFACE.
- 7) ALL BEARING HEIGHTS ARE TO BE MEASURED TO THE TOP OF THE BEARING SURFACE.
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- 9) ALL BEARING HEIGHTS ARE TO BE MEASURED TO THE TOP OF THE BEARING SURFACE.
- 10) ALL BEARING HEIGHTS ARE TO BE MEASURED TO THE TOP OF THE BEARING SURFACE.



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Freeport
MORGAN RES.

MIKE TODD CONST.
MORGAN RES.

12-29-16	KLH	755227
12-29-16	KLH	755227

14" DEEP BEAMS
12" DEEP FLOORS
24" O/C

20" DEEP FLOOR
19.2" O/C

5.5/12 PITCH
24" O/H

BEARING HEIGHT SCHEDULE

	8' 1-1/8"
	9' 1-1/8"
	10' 1-1/8"
	11' 1-1/8"
	12' 1-1/8"

NOTES:

- 1) REFER TO HD 91 (RECOMMENDATIONS FOR HANDING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEER DRAWINGS FOR PERMANENT BRACING REQUIRED.
- 2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMES) MUST BE COMPLETELY DICKED OR REFER TO DETAIL 103 FOR ALTERNATE BRACING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TRUSSES ARE DESIGNED FOR 2 % SLOPE. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
- 5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
- 6) 5/42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) DEWATERAGE/INTEL (DRI) TO BE FURNISHED BY BUILDER.



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PHONE: 904-772-6100 FAX: 904-772-1973

PHONE: 813-621-0631 FAX: 813-620-0956

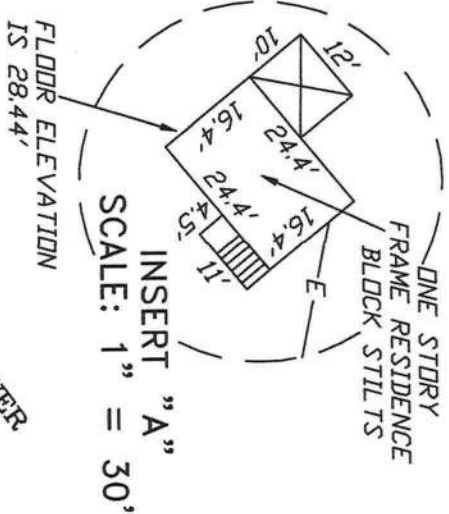
BUILDER
MIKE TODD CONST.
REAL ESTATE
MORGAN RES.

DATE:	OWNER:	DESIGNED BY:
12-29-16	KLH	Original Drawing
ISSUED BY:	REVIEW BY:	DATE:
		7552227

MITEK PLATE APPROVAL #'s 2197.2 - 2197.4, WEYERHAUSER PRODUCT #'s 1630.2 - 1630.10

BOUNDARY SURVEY IN SECTION 26, TOWNSHIP 6 SOUTH,
RANGE 15 EAST, COLUMBIA COUNTY, FLORIDA.

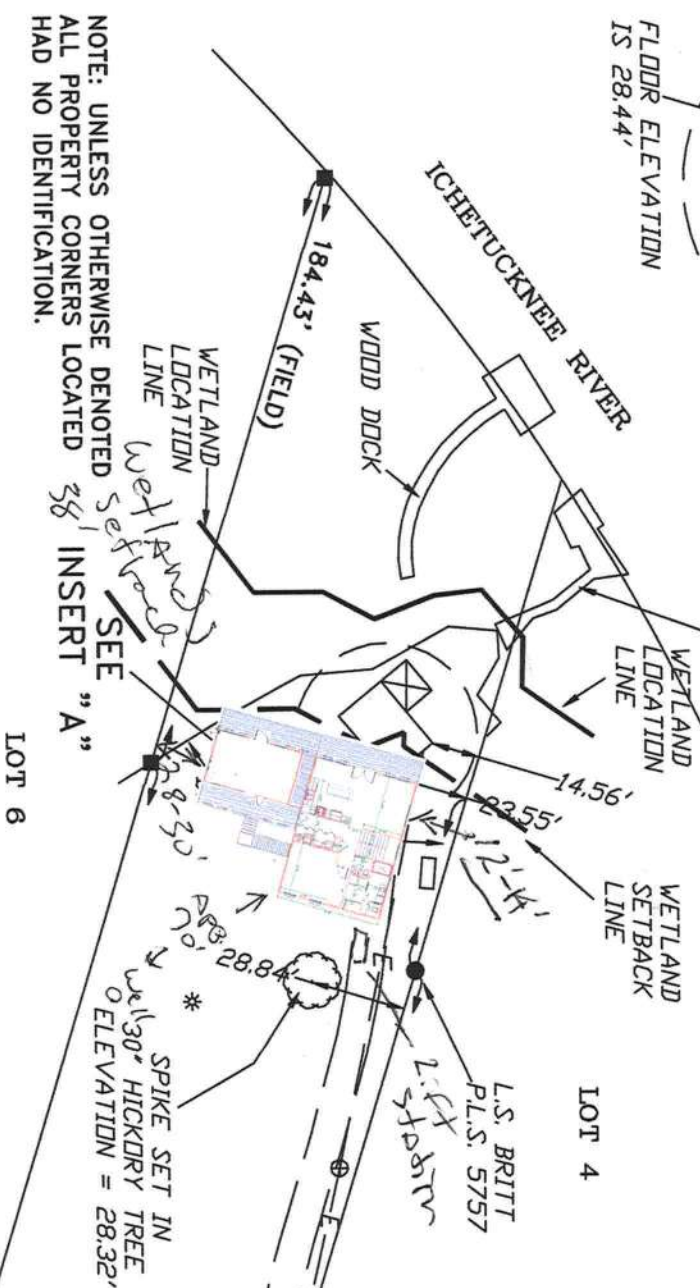
DESCRIPTION:
LOT 5 OF THREE RIVERS ESTATES SECTION NO. 1 AS PER THE
PLAT THEREOF RECORDED IN PLAT BOOK 3, PAGE 53 OF THE
PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA,
TOGETHER WITH AND SUBJECT TO AN EASEMENT FOR INGRESS
AND EGRESS.
(SEE SHEET # 2 FOR DESCRIPTION AND DETAILS)



SCALE: 1" = 30'

FLOOD ELEVATION
IS 28.44'

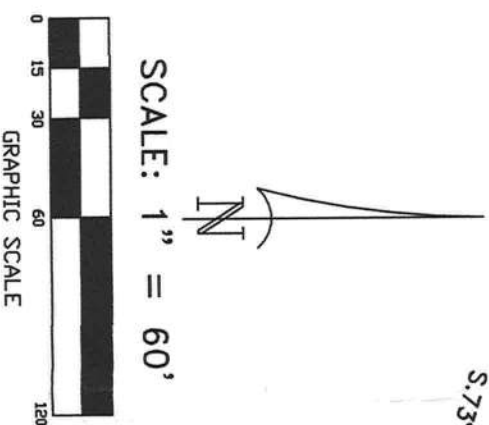
WOOD DOCK
APPARENT USE
DECK ELEVATION
IS 20.98'



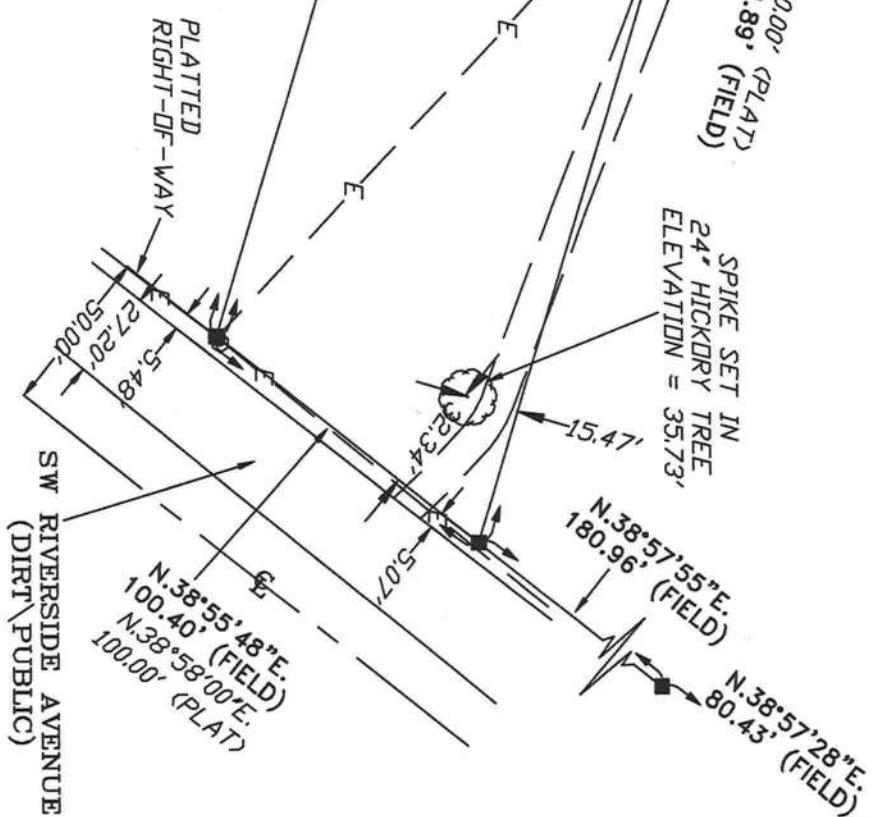
- SURVEYOR'S NOTES:
1. BOUNDARY BASED ON MONUMENTATION FOUND IN ACCORDANCE WITH THE RETRACEMENT OF THE ORIGINAL SURVEY FOR SAID PLAT OF RECORD.
 2. BEARINGS ARE BASED ON SAID PLAT OF RECORD.
 3. THIS PARCEL IS IN ZONE "AE" AND IS SUBJECT TO FLOODING. A BASE FLOOD ELEVATION IS ESTABLISHED TO BE 34.4 FEET AS PER FLOOD INSURANCE RATE MAP, DATED 4 FEBRUARY, 2009 FIRM PANEL NO. 12023C0458C. HOWEVER, THE FLOOD INSURANCE RATE MAPS ARE SUBJECT TO CHANGE. (BASED ON NAVD 88 DATUM)
 4. THE IMPROVEMENTS, IF ANY, INDICATED ON THIS SURVEY DRAWING ARE AS LOCATED ON DATE OF FIELD SURVEY AS SHOWN HEREON.
 5. IF THEY EXIST, NO UNDERGROUND ENCROACHMENTS AND/OR UTILITIES WERE LOCATED FOR THIS SURVEY EXCEPT AS SHOWN HEREON.
 6. THIS SURVEY WAS COMPLETED WITHOUT THE BENEFIT OF A TITLE COMMITMENT OR A TITLE POLICY.
 7. DIMENSIONS SHOWN HEREON ARE IN FEET AND DECIMAL PARTS THEREOF.
 8. THIS SURVEY DOES NOT REFLECT OR DETERMINE OWNERSHIP.
 9. THE ADJACENT OWNERSHIP INFORMATION AS SHOWN HEREON IS BASED ON THE COUNTY PROPERTY APPRAISERS GIS SYSTEM, UNLESS OTHERWISE DENOTED.
 10. ALL ELEVATIONS SHOWN HEREON ARE BASED ON NAVD 88 DATUM.

SYMBOL LEGEND:

4"x4" CONCRETE MONUMENT FOUND	E	CENTERLINE
4"x4" CONCRETE MONUMENT SET	-E--	ELECTRIC LINES
IRON PIPE FOUND	-X--	WIRE FENCE
IRON PIN AND CAP SET	-O--	CHAIN LINK FENCE
1/2" CUT IN PAVEMENT	-D--	WOODEN FENCE
CALCULATED PROPERTY CORNER	---	SECTION LINE
MAIL & DISK	(PLAT)	AS PER A PLAT OF RECORD
POWER POLE	(DEED)	AS PER A DEED OF RECORD
WATER METER	(CALC.)	AS PER CALCULATIONS
UTILITY BOX	(FIELD)	AS PER FIELD MEASUREMENTS
WELL	P.R.M.	PERMANENT REFERENCE MARKER
SANITARY MANHOLE	P.C.P.	PERMANENT CONTROL POINT
SIGN POST		



FLOOD INFO AS PER SRWMD
100 YEAR = 34.4'
10 YEAR = 28.5'
2 YEAR = 22.7'
(NAVD 88 DATUM)



CERTIFIED TO:

DAVID & BRYAN MORGAN

David Morgan

FIELD BOOK: SEE PAGE(S): FILE

SURVEYOR'S CERTIFICATION

I HEREBY CERTIFY THAT THIS SURVEY WAS MADE UNDER MY RESPONSIBLE CHARGE AND MEETS THE MINIMUM
TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS
IN CHAPTER RULES, REGULATIONS AND STANDARDS FOR THE PRACTICE OF SURVEYING, REVISION 7/1/2010, 1301.0001.

10/29/15
FIELD SURVEY DATE
11/05/15
DRAWING DATE

L. SCOTT BRITT, P.S.M.
CERTIFICATION # 5757

NOTE: UNLESS IT BEARS THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND
MAPPER THIS DRAWING, SKETCH, PLAT OR MAP IS FOR INFORMATIONAL PURPOSES ONLY AND IS NOT VALID.



BRITT SURVEYING
& MAPPING, LLC

LAND SURVEYORS AND MAPPERS, L.B. # 8016
2086 SW MAIN BLVD, SUITE 112, LAKE CITY, FLORIDA 32025
(386) 752-7163 FAX (386) 752-5573
www.brittsurvey.com
WORK ORDER # L-23730

28-00-00

28-00-00

28-00-00

28-00-00

BEARING HEIGHT SCHEDULE	
	8' 1-1/8"
	9' 1-1/8"
	10' 1-1/8"
	11' 1-1/8"
	12' 1-1/8"

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

- 2) ALL T05655, INCLUDING T05655 UNDER VALLEY FRAMING, MUST BE COMPLETELY DELETED OR REFER TO DETAIL V05 FOR ALTERNATE BRACING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL T05655 ARE DESIGNED FOR 2 O.C. MAXIMUM SPACING. UNLESS OTHERWISE NOTED.
- 5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING. UNLESS OTHERWISE NOTED.
- 6) S742 T05655 MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) BEAM/ADVERT/ULTE (R/R) TO BE FURNISHED BY BUILDER.



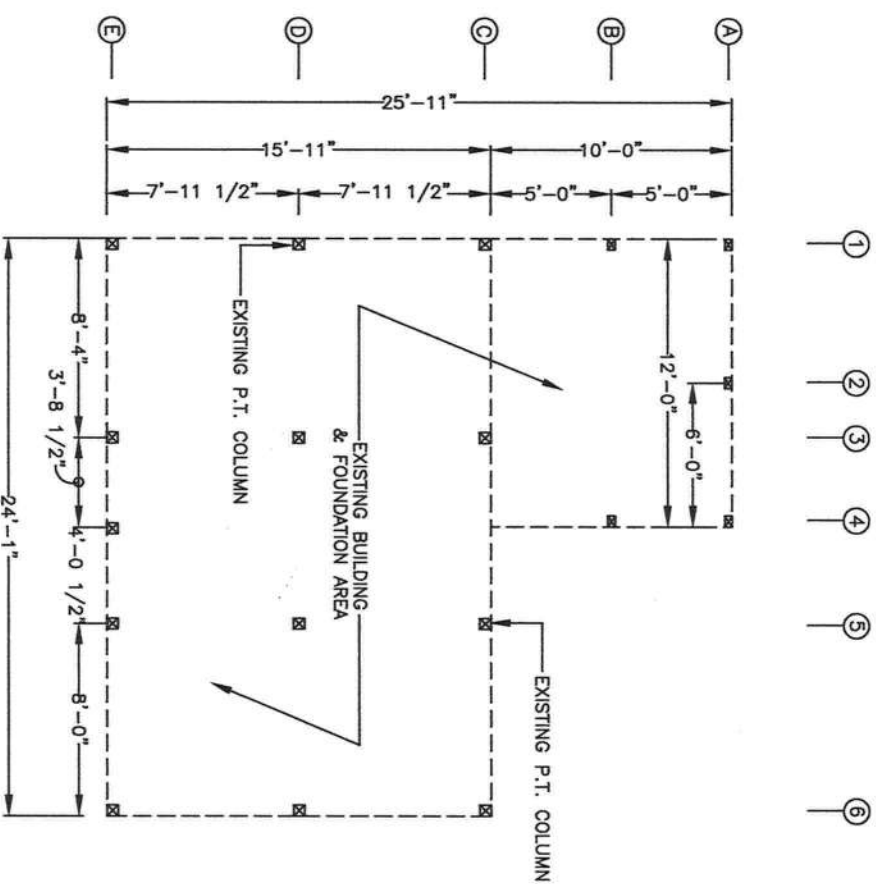
Freeport
PHONE: 850-835-4541 FAX: 850-835-6835

LEGAL ADDRESS:
MORGAN HOUSE

DATE:	2-2-16	PERSON:		TEL. NO.:	755227
Tel. No.:		NAME:	KLH	Ext. No.:	
		Original Reference #:	755227	Total Amt:	755227

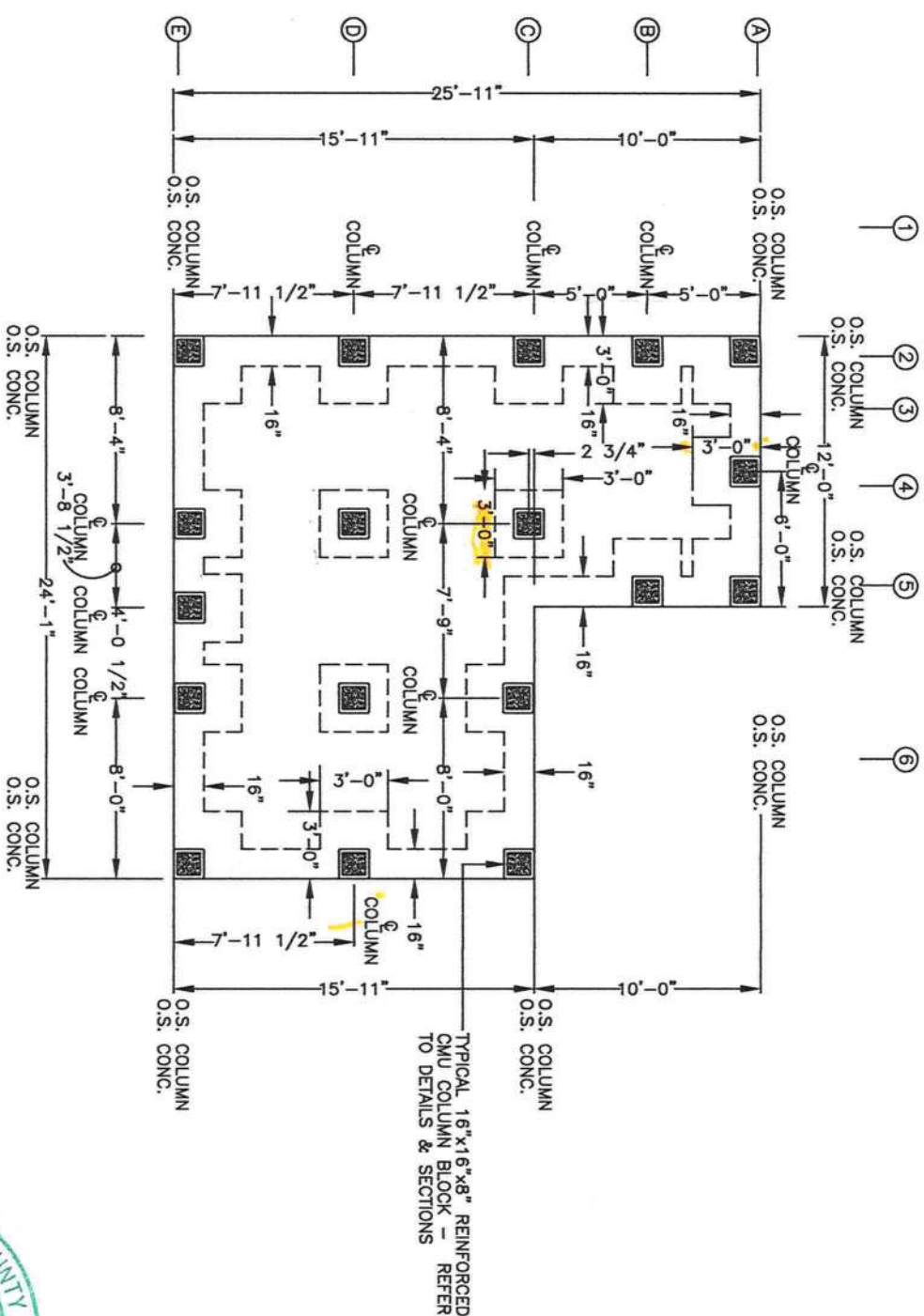
MITEK PLATE APPROVAL #'s 2197.2 - 2197.4, WEYERHAUSER PRODUCT #'s 1630.2 - 1630.10

SCALE NOTE:
PLAN VIEW: 1/8"=1'-0"



1 EXISTING DWELLING FOUNDATION PLAN VIEW
A2.1.0 SCALE: N.T.S.

REFER TO SHEET No. S1.1.0 FOR CMU COLUMN & FOUNDATION REQUIREMENTS.
HEIGHT OF NEW COLUMN: FINISH FLOOR HEIGHTS OF RELOCATED BUILDING SHALL MATCH ADJACENT NEW CONSTRUCTION
A P.T. 2x SHALL BE ATTACHED TO THE TOP OF THE CMU PIER/COLUMN w/ 1- A507 ANCHOR BOLT
EACH REINFORCED CMU COLUMN SHALL BE ATTACHED TO DOUBLE 2x8 BAND JOISTS OR INTERIOR JOISTS w/ 1-MODEL No. PAST BY SIMPSON STRONG-TIE OR EQUAL w/ 9-16d COMMON NAILS AS SPECIFIED BY MANUFACTURER
DETAILS FOR NEW CONSTRUCTION ARE SIMILAR TO THOSE REQUIRED FOR THIS SPECIFIC RELOCATION.



2 RELOCATED DWELLING FOUNDATION PLAN VIEW
A2.1.0 SCALE: N.T.S.

TYPICAL 16"x16"x8" REINFORCED CMU COLUMN BLOCK - REFER TO DETAILS & SECTIONS



CERTIFICATION:

THESE PLANS FOR THE MORGAN RESIDENCE WILL COMPLY WITH SECTION 1609 OF THE FLORIDA BUILDING CODE, 2014 EDITION FOR A 120 MPH WIND LOAD, 3 SECOND GUST, EXPOSURE C, WITH THE INTERNAL PRESSURE OF + 0.18 AND - 0.18 INCLUDED IN THESE LOADS.

COMPONENTS/CLADDING ROOF = - 22.17 PSF
+ 11.86 PSF
COMPONENTS/CLADDING ROOF = - 51.82 PSF
OVERHANG + 11.86 PSF
COMPONENTS/CLADDING/WALLS = - 33.32 PSF
+ 25.22 PSF

Curtis E. Keen 6/29/16
CURTIS E. KEEN, PE #23836

PROJECT No.
MORGAN-A2.1.0.DWG
SHEET No.
A2.1.0
DATE
02/26/16

EXISTING & RELOCATED BUILDING FOUNDATION PLAN VIEWS
MISC. NOTES, REFERENCES & INSTRUCTIONS
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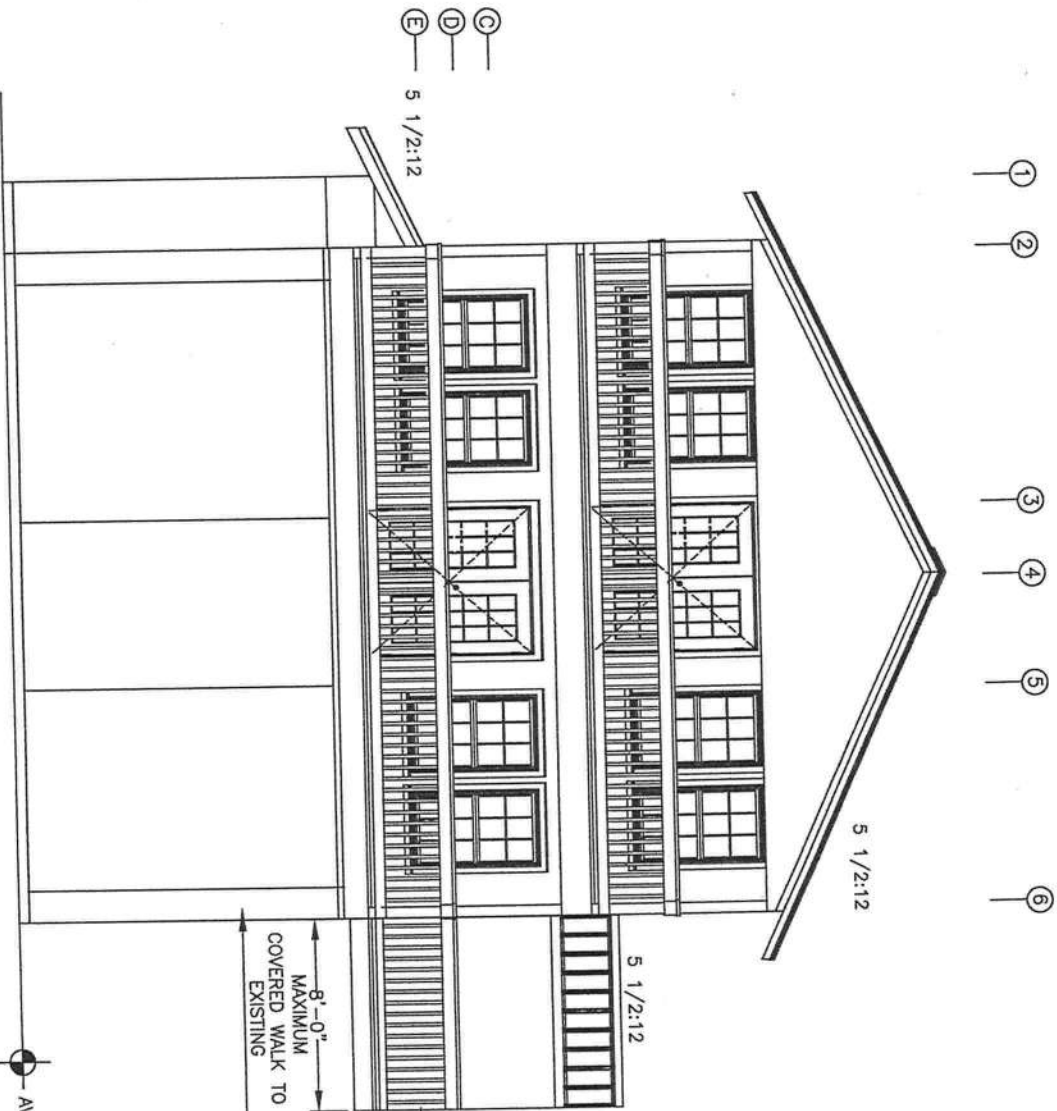
Curtis E. Keen, PE #23836
Certification of Authorization #3761
DATE:

MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA

KEEN ENGINEERING & SURVEYING, INC.
9263 CR 417
LIVE OAK, FLORIDA 32060
386-362-4787
ENG. LIC. EB 3761

CONDITIONED AREA 1ST LEVEL	1064.00 S.F.
CONDITIONED AREA 2ND LEVEL	1064.00 S.F.
TOTAL CONDITIONED AREA	2128.00 S.F.
PORCH AREA(S)	448.00 S.F.
WALK AREA(S)	40.00 S.F.
SUB LEVEL STAIR & STORAGE	199.50 S.F.
SUB LEVEL PARKING	199.50 S.F.
TOTAL UNCONDITIONED AREAS	1110.70 S.F.
TOTAL CONSTRUCTION AREA	3238.70 S.F.

FLOOD DATA
100 YR 34.4
10 YR 28.5
NAVD 88 DATUM
EXISTING GRADE = 24.5



2 EXTERIOR FINISH ENDWALL ELEVATION
A3.0.0 SCALE: 1/8" = 1'-0

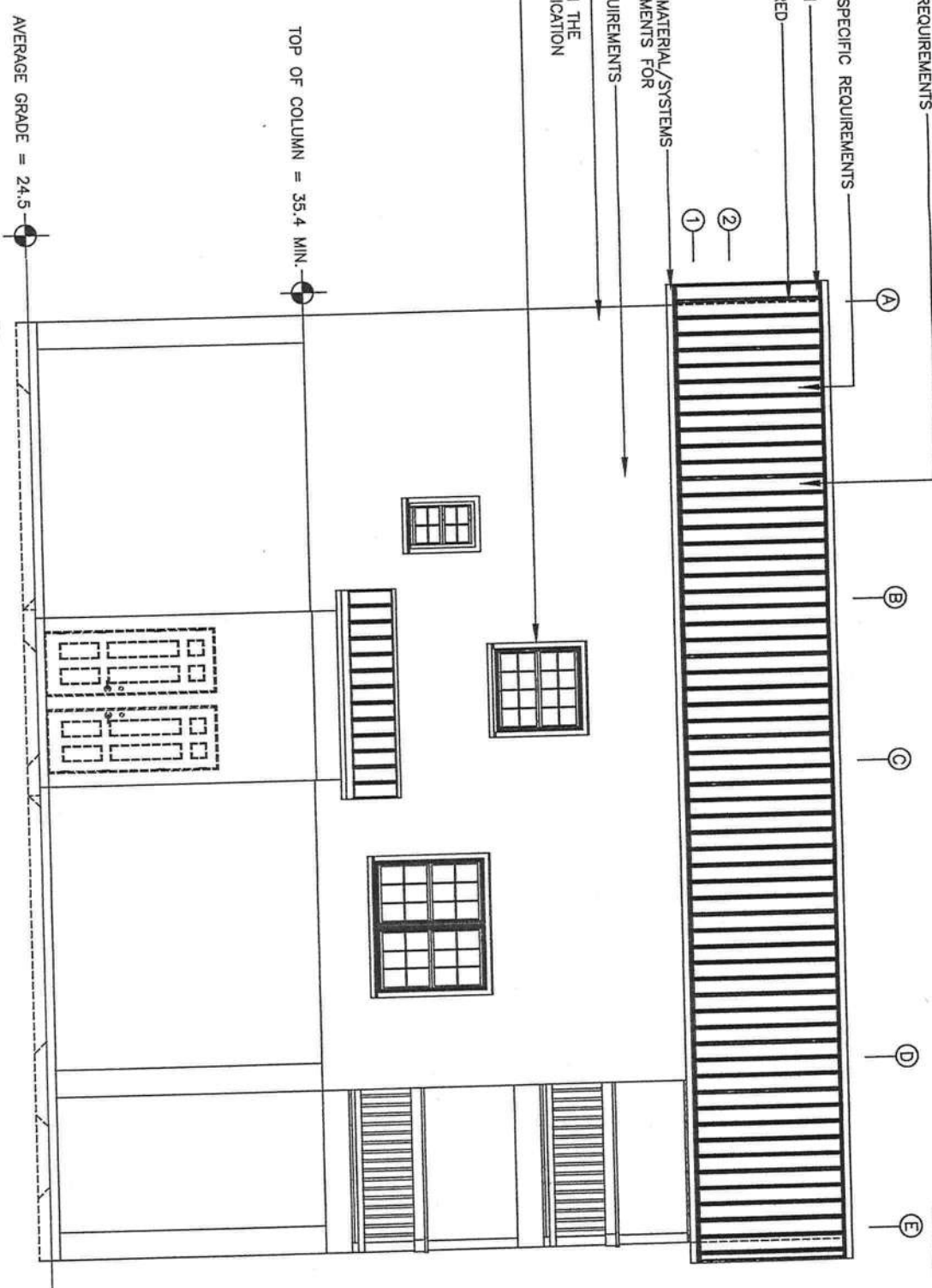
PROJECT No.
MORGAN-A3.0.0.DWG
SHEET No.
A3.0.0
DATE
02/26/16
EXTERIOR FINISH ELEVATIONS
MISC. NOTES, REFERENCES & INSTRUCTIONS
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Curtis E. Keen, PE #23836
Certification of Authorization #3761
DATE:

MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA

KEEN ENGINEERING & SURVEYING, INC.
9263 CR 417
OAK, FLORIDA 32060
386-362-4787
ENG. LIC. EB 3761

26 GA. GALV. STEEL ROOFING PANELS - REFER TO REQUIREMENTS
REFER TO ROOF SYSTEM FOR DETAILS, SECTIONS & SPECIFIC REQUIREMENTS
MANUFACTURED ROOF TRUSSES - SEE ROOF SYSTEM
SEE ROOF SYSTEM FOR MECHANICAL DEVICE REQUIRED
VINYL FASCIA SYSTEM OR OTHER OWNER APPROVED MATERIAL/SYSTEMS
INSTALLED PER SELECTED MANUFACTURER'S REQUIREMENTS FOR THIS SPECIFIC APPLICATION
2x4 FRAMING - SEE DETAILS & SECTIONS FOR REQUIREMENTS
EXTERIOR FINISH MATERIAL: LAP SIDING BY "JAMES HARDIE PRODUCTS", INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS FOR THIS APPLICATION
TRIM @ THE DIRECTION OF OWNER



1 EXTERIOR FINISH SIDEWALL ELEVATION
A3.0.0 SCALE: 1/8" = 1'-0

ENCLOSURE NOTE
THE ABOVE ENCLOSED AREA OR UNFINISHED ENCLOSURE IS DESIGNED WITH THE SOLE INTENT OR PURPOSE TO WITHSTAND EXPOSURE TO THE ELEMENTS, HAVE A LOW FLOOD DAMAGE POTENTIAL, AND PROVIDE LIMITED STORAGE (e.g. GARDEN EQUIPMENT, BICYCLES AND OTHER LOW DAMAGE ITEMS). FURTHER, THIS AREA IS OF A FLOOD RESISTANT MATERIAL, NON-PARTITIONED AND VOID OF ALL UTILITIES (EXCEPT ESSENTIAL AND BASIC LIGHTING CONSISTING OF GFCI SWITCHES AND FIXTURES INSTALLED WITH NON METALLIC CONDUIT) AND PROVIDED WITH OPENINGS TO PRECLUDE HYDROSTATIC LOADING AND ALLOW VENTILATION. THIS AREA SHALL NOT BE TEMPERATURE CONTROLLED.

SCALE NOTE:
ELEVATIONS: 1/8"=1'-0

CONDITIONED AREA 1ST LEVEL	1064.00 S.F.
CONDITIONED AREA 2ND LEVEL	1064.00 S.F.
TOTAL CONDITIONED AREA	2128.00 S.F.
PORCH AREA(S)	448.00 S.F.
WALK AREA(S)	40.00 S.F.
SUB LEVEL STAIR & STORAGE	199.50 S.F.
SUB LEVEL PARKING	199.50 S.F.
TOTAL UNCONDITIONED AREAS	1110.70 S.F.
TOTAL CONSTRUCTION AREA	3238.70 S.F.

26 GA. GALV. STEEL ROOFING PANELS - REFER TO REQUIREMENTS
REFER TO ROOF SYSTEM FOR DETAILS, SECTIONS & SPECIFIC REQUIREMENTS
MANUFACTURED ROOF TRUSSES - SEE ROOF SYSTEM
SEE ROOF SYSTEM FOR MECHANICAL DEVICE REQUIRED

VINYL FASCIA SYSTEM OR OTHER OWNER APPROVED MATERIAL/SYSTEMS
INSTALLED PER SELECTED MANUFACTURER'S REQUIREMENTS FOR
THIS SPECIFIC APPLICATION
2x4 FRAMING - SEE DETAILS & SECTIONS FOR REQUIREMENTS
TRIM @ THE DIRECTION OF OWNER
SEE GUARD RAIL REQUIREMENTS

EXTERIOR FINISH MATERIAL: LAP SIDING BY "JAMES
HARDIE PRODUCTS", INSTALLED IN ACCORDANCE WITH THE
MANUFACTURER'S RECOMMENDATIONS FOR THIS APPLICATION
P.T. 6x6 COLUMN - SEE REQUIREMENTS ELSEWHERE THESE PLANS

TOP OF COLUMN = 35.4 MIN.

REINFORCED CMU COLUMN BLOCK - SEE FOUNDATION REQUIREMENTS

AVERAGE GRADE = 24.5

GABLE END FINISH MATERIAL: BOARD & BATTEN BY "JAMES
HARDIE PRODUCTS", INSTALLED IN ACCORDANCE WITH THE
MANUFACTURER'S RECOMMENDATIONS FOR THIS APPLICATION

TOP OF COLUMN = 35.4 MIN.

REINFORCED CMU COLUMN BLOCK - SEE FOUNDATION REQUIREMENTS

AVERAGE GRADE = 24.5

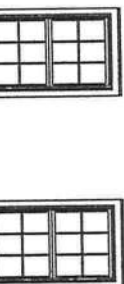
SCALE NOTE:
ELEVATIONS: 1/8"=1'-0"

1 EXTERIOR FINISH SIDEWALL ELEVATION
A3.1.0 SCALE: 1/8" = 1'-0

A
C

5 1/2:12

5 1/2:12



8'-0"
MAXIMUM
COVERED WALK TO
EXISTING

5 1/2:12

2 EXTERIOR FINISH ENDWALL ELEVATION
A3.0.0 SCALE: 1/8" = 1'-0

PROJECT No.
MORGAN-A3.0.0.DWG
SHEET No.
A3.1.0
DATE
02/26/16

EXTERIOR FINISH ELEVATIONS
MISC. NOTES, REFERENCES & INSTRUCTIONS
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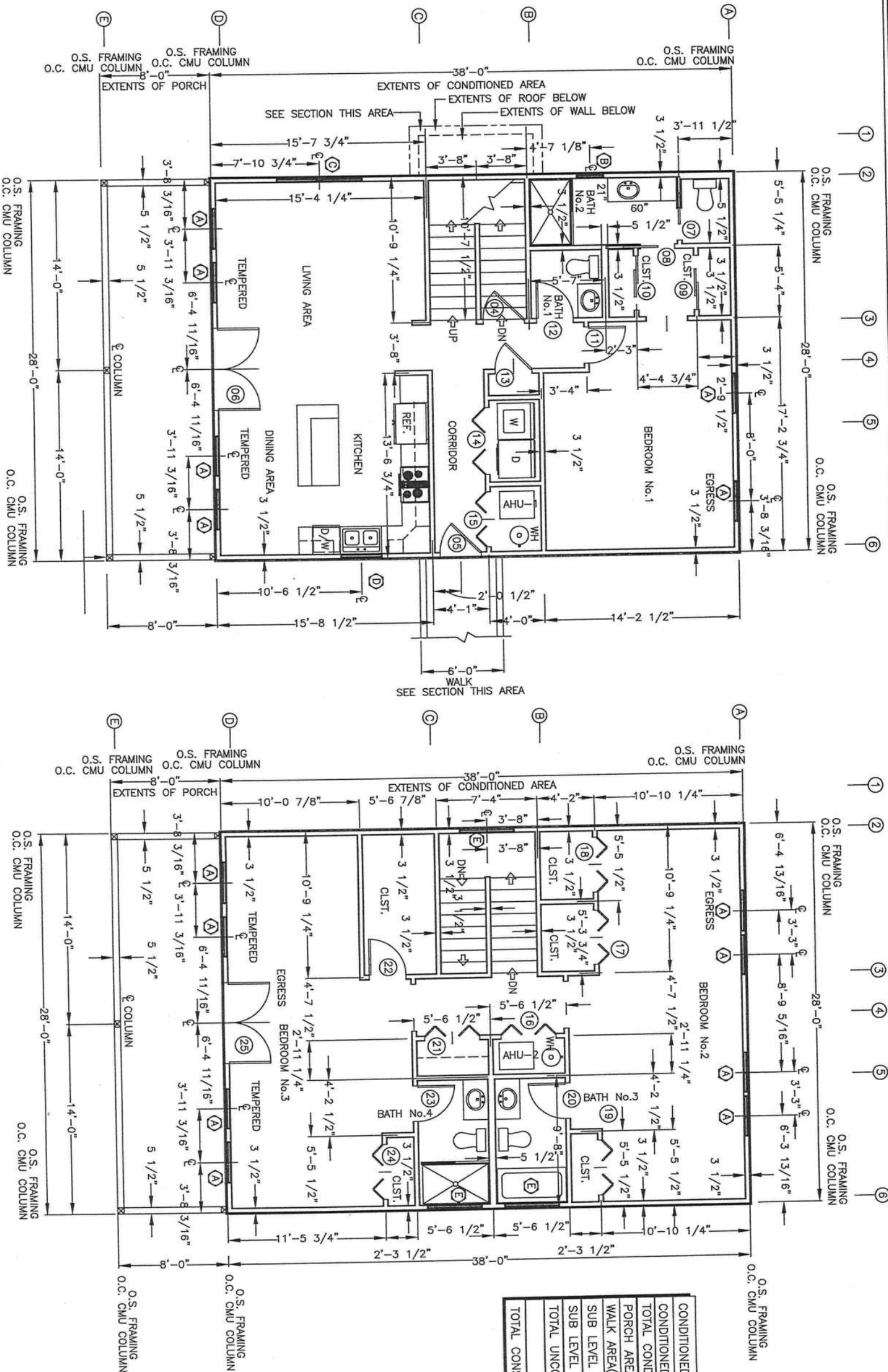
Curtis E. Keen, PE #23836
Certification of Authorization #5761
DATE:

MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA

KEEN ENGINEERING & SURVEYING, INC.
9263 CR 417
LIVE OAK, FLORIDA 32060
386-362-4787
ENG. LIC. EB 5761

SCALE NOTE:
PLAN VIEW: 1/8"=1'-0"

CONDITIONED AREA 1ST LEVEL	1064.00 S.F.
CONDITIONED AREA 2ND LEVEL	1064.00 S.F.
TOTAL CONDITIONED AREA	2128.00 S.F.
PORCH AREA(S)	448.00 S.F.
WALK AREA(S)	40.00 S.F.
SUB LEVEL STAIR & STORAGE	199.50 S.F.
SUB LEVEL PARKING	199.50 S.F.
TOTAL UNCONDITIONED AREAS	1110.70 S.F.
TOTAL CONSTRUCTION AREA	3238.70 S.F.



2 FIRST LEVEL PLAN VIEW
A2.0.0 SCALE: N.T.S.

3 SECOND LEVEL PLAN VIEW
A2.0.0 SCALE: N.T.S.

PROJECT No.
MORGAN-A2.0.0.DWG
SHEET No.
A2.0.0
DATE
02/26/16

FIRST & SECOND LEVELS DIMENSIONED PLAN VIEWS
MISC. NOTES, REFERENCES & INSTRUCTIONS
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COLUMBIA COUNTY, FLORIDA

KEEN ENGINEERING & SURVEYING, INC.
9263 CR 417
OAK, FLORIDA 32060
386-362-4787
ENG. LIC. EB 3761

WINDOW										RATING			FRAME				DETAILS				
CATEGORY																					
TYPE																					
WIDTH																					
HEIGHT																					
HORIZONTAL SLIDER																					
FIXED																					
SINGLE HUNG																					
DOUBLE HUNG																					
HOLLOW METAL																					
SOLID CORE WOOD																					
HOLLOW CORE WOOD																					
STILE AND RAIL WOOD																					
ALUMINUM																					
VINYL																					
VINYL / ALUMINUM																					
OBSCURRED GLASS																					
TEMPERED GLASS																					
WIRED GLASS (RATED)																					
TYPE																					
HOLLOW METAL WELDED																					
HOLLOW METAL KNOCKDOWN																					
WOOD																					
ALUMINUM																					
VINYL / ALUMINUM																					
HEAD																					
JAMB																					
SILL																					
TYPE																					
A										SOME LOCATIONS REQUIRE TEMPERED GLAZING											
B																					
C																					
D																					
E																					

REMARKS																			
A 3'-0"										EXTERIOR - INSUL. - DIVIDED LITE OPTION - CONTRACTOR SHALL PROVIDE APPROVAL SPECIFICATIONS REQUIREMENTS PER F.B.C. 1715.5 & MFG'S. INSTALLATION REQUIREMENTS - SOME LOCATIONS REQUIRE TEMPERED GLAZING									
B 2'-0"										EXTERIOR - INSUL. - DIVIDED LITE OPTION - CONTRACTOR SHALL PROVIDE APPROVAL SPECIFICATIONS REQUIREMENTS PER F.B.C. 1715.5 & MFG'S. INSTALLATION REQUIREMENTS - SECURED GLASS OPTION									
C 6'-0"										EXTERIOR - INSUL. - DIVIDED LITE OPTION - CONTRACTOR SHALL PROVIDE APPROVAL SPECIFICATIONS REQUIREMENTS PER F.B.C. 1715.5 & MFG'S. INSTALLATION REQUIREMENTS									
D 3'-0"										EXTERIOR - INSUL. - DIVIDED LITE OPTION - CONTRACTOR SHALL PROVIDE APPROVAL SPECIFICATIONS REQUIREMENTS PER F.B.C. 1715.5 & MFG'S. INSTALLATION REQUIREMENTS									
E 4'-0"										EXTERIOR - INSUL. - DIVIDED LITE OPTION - CONTRACTOR SHALL PROVIDE APPROVAL SPECIFICATIONS REQUIREMENTS PER F.B.C. 1715.5 & MFG'S. INSTALLATION REQUIREMENTS - SELECTED LOCATIONS W/ OBSCURED GLASS OPTION									

ADDITIONAL WINDOW NOTES

REFER TO PLAN VIEWS & ELEVATIONS FOR SPECIFIC DESIGNATIONS FOR REQUIRED EGRESS WINDOWS

IF WINDOW SILLS ARE LESS THAN 24" ABOVE FINISH FLOOR AND OVER 7'-0" ABOVE ADJACENT FINISH GRADE FULLY COMPLIANT WINDOW GUARDS ARE REQUIRED

CONTRACTOR SHALL PROVIDE ALL APPROVAL SPECIFICATIONS & REQUIREMENTS PER F.B.C. 1715.5 & MANUFACTURER'S INSTALLATION REQUIREMENTS

DOOR										RATING			FRAME				DETAILS				THRESHOLD				HDWR			REMARKS																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																								
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DOOR NUMBER										TYPE			WIDTH				HEIGHT 6'-8"				HEIGHT 8'-0				1 3/8" THICK				INSUL. METAL				SOLID CORE WOOD				HOLLOW CORE WOOD				STILE AND RAIL WOOD				ALUMINUM				TEMPERED GLASS				WIRED GLASS				FINISH				20 MINUTE LABEL				C LABEL 3/4 HR				B LABEL 1 HR				TYPE				HOLLOW METAL WELDED				HOLLOW METAL KNOCKDOWN				WOOD				ALUMINUM				HEAD				JAMB				SILL				FINISH				THRESHOLD/ACC				1/4" UNDERCUT				3/4" UNDERCUT				SMOKE SEALS				DOOR NUMBER				AUTOMATIC CLOSURE DEVICE				DOOR VIEWER				HANDLES LEVER TYPE				LOCKSETS																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
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SCALE NOTE:
PLAN VIEW: 1/8"=1'-0"

ELECTRICAL FIXTURE SYMBOLS

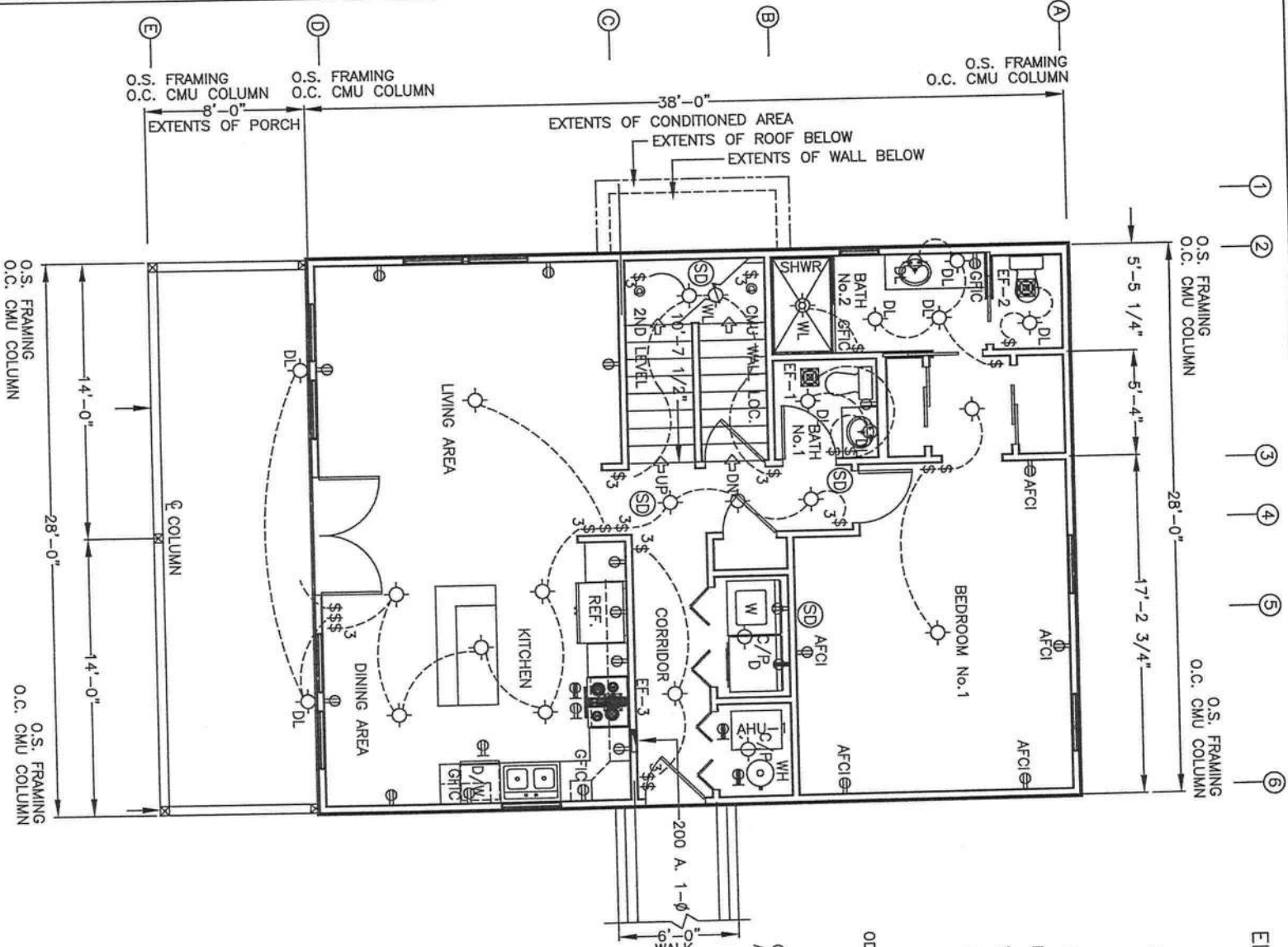
- FLUORESCENT STRIP FIXTURE, SURFACE MOUNTED
- FLUORESCENT, SURFACE MOUNTED
- OWNER SELECTED FIXTURE - CONTRACTOR INSTALLED
- DROP MOUNTED FIXTURE
- WALL BRACKET MOUNTED INCANDESCENT OR HID FIXTURE
- SURFACE MOUNTED FIXTURE
- RECESSED DOWNLIGHT
- RECESSED DOWNLIGHT OR SURFACE MOUNTED FIXTURE - DAMP LOCATION RATED
- RECESSED DOWNLIGHT OR SURFACE MOUNTED FIXTURE - WET LOCATION RATED
- RECESSED DOWNLIGHT OR SURFACE MOUNTED FIXTURE - FLUORESCENT LAMP
- RECESSED DOWNLIGHT OR SURFACE MOUNTED FIXTURE - OWNER'S DIRECTION
- EXTERIOR FLOOD LIGHTING: LOCATIONS & SWITCHES @ OWNER'S DIRECTION
- GROUND MOUNTED FLOOD OR DIRECTIONAL DOWNLIGHT LOW VOLTAGE SYSTEM LOCATED @ THE DIRECTION OF THE OWNER
- ON DEMAND WATER HEATER: MFR., SIZE, ETC. @ DIR. OF OWNER
- DUPLEX RECEPTACLE, WALL MOUNTED 12" A.F.F. UNLESS OTHERWISE NOTED
- NOTE: RECEPTACLES LOCATED @ ALL COUNTER AREAS SHALL BE 43" A.F.F. UNLESS OTHERWISE NOTED
- DUPLEX RECEPTACLE, WALL MOUNTED 12" A.F.F. WEATHERPROOF BOX/CONNECTION
- DUPLEX RECEPTACLE W/ GROUND FAULT CIRCUIT INTERRUPTER, WALL MOUNTED
- DUPLEX RECEPTACLE W/ ARC FAULT CIRCUIT INTERRUPTER, WALL MOUNTED
- DUPLEX RECEPTACLE W/ GROUND FAULT CIRCUIT INTERRUPTER, FLOOR MOUNTED
- EXACT LOCATION AS DETERMINED BY OWNER @ TIME OF INSTALLATION
- 220 RECEPTACLE OR CONNECTION - SEE PLAN VIEW(S) FOR LOCATIONS
- TELEPHONE OUTLET WALL MOUNTED 18" A.F.F.
- NOTE: LOCATE @ OWNER'S DIRECTION
- TELEPHONE DATA OUTLET, WALL MOUNTED 18" A.F.F. UNLESS OTHERWISE INDICATED
- NOTE: LOCATE @ OWNER'S DIRECTION
- SINGLE POLE TOGGLE SWITCH MOUNTED @ 48" A.F.F.
- SINGLE POLE DIMMER SWITCH WALL MOUNTED 48" A.F.F.
- THREE-WAY TOGGLE SWITCH
- SINGLE POLE MOTOR RATED TOGGLE SWITCH
- FLUSH MOUNTED, 200A., 240/120 1 - Ø ELECTRICAL PANEL - TOP @ 6'-6" A.F.F.
- DISCONNECT SWITCH: SIZE AS NOTED ADJACENT TO SYMBOL
- OWNER SELECTED CEILING FAN W/ OPTIONAL LIGHT KIT BY MANUFACTURER
- OWNER SELECTED MANUFACTURER & MODEL - SIZE AS NOTED ON PLANS
- FAN AND OR LIGHT MAY BE SWITCHED - REFER TO PLAN VIEW(S)
- EXHAUST FAN: OWNER APPROVED PROVIDING A MIN. 40 CFM
- EF-1 EXHAUST FAN: OWNER APPROVED PROVIDING A MIN. 30 CFM
- EF-2 EXHAUST FAN: OWNER APPROVED PROVIDING A MIN. 30 CFM
- EF-3 EXHAUST FAN: BY OWNER APPROVED STOVE MANUFACTURER
- CAT 5 WIRE @ OWNER SELECTED LOCATIONS
- SPEAKERS & RELATED SOUND DEVICES - LOCATE @ OWNER'S DIRECTION
- TELEVISION COAXIAL OUTLET SHALL BE LOCATED 18" A.F.F.
- NOTE: SEE PLAN VIEW & LOCATE ADDITIONAL @ OWNER'S DIRECTION
- SMOKE DETECTOR: CEILING MOUNTED UNLESS OTHERWISE NOTED
- ALL SMOKE DETECTORS SHALL BE 120 VOLT WITH BATTERY BACKUP, OF THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER, INSTALL INSIDE AND NEAR ALL BEDROOMS ON SEPARATE CIRCUIT TO THE MAIN DISTRIBUTION PANEL. ADDITIONAL LOCATIONS MAY BE DETERMINED BY THE FIRE MARSHALL. REFER TO PLAN VIEWS FOR LOCATIONS

FIRST LEVEL ELECTRICAL NOTES: ALL LIGHTING, SWITCHING FOR LIGHTING AND RECEPTACLES SHALL BE WET LABE AND IN APPROPRIATE WEATHER PROOF BOXES W/ APPROVED CONNECTIONS WITH GROUND FAULT INTERRUPTER CIRCUITRY.
ALL WIRING SHALL BE INSTALLED IN NMT CONDUIT PER LATEST EDITION OF THE N.E.C.

ELECTRICAL NOTES

- ALL WORK SHALL BE FULLY COMPLIANT WITH THE MOST CURRENT EDITION OF THE NATIONAL ELECTRICAL CODE (NEC).
- ALL CIRCUITS & EQUIPMENT SHALL BE GROUNDED IN ACCORDANCE WITH THE APPROPRIATE ARTICLES OF THE NEC.
- WHEN LIGHT FIXTURES ARE INSTALLED IN CLOSETS THEY SHALL BE SURFACE MOUNTED OR RECESSED. INCANDESCENT FIXTURES SHALL HAVE COMPLETELY ENCLOSED LAMPS. SURFACE MOUNTED INCANDESCENT FIXTURES SHALL HAVE A MINIMUM CLEARANCE OF 12 INCHES AND ALL OTHER FIXTURES SHALL HAVE A MINIMUM CLEARANCE OF 6 INCHES FROM "STORAGE AREAS" AS DEFINED BY NEC 410-B(9).
- WHEN WATER HEATERS ARE INSTALLED THEY SHALL BE PROVIDED WITH READILY ACCESSIBLE DISCONNECTS ADJACENT TO THE WATER HEATERS SERVED. THE BRANCH CIRCUIT SWITCH OR CIRCUIT BREAKER SHALL BE PERMITTED TO SERVE IS WITHIN SIGHT FROM THE WATER HEATER OR IS CAPABLE OF BEING LOCKED IN THE OPEN POSITION.
- HVAC EQUIPMENT SHALL BE PROVIDED WITH READILY ACCESSIBLE DISCONNECTS ADJACENT TO THE EQUIPMENT SERVED. A UNIT SWITCH WITH A MARKED "OFF" THAT IS A PART OF THE HVAC EQUIPMENT AND DISCONNECTS ALL UNGROUNDED CONDUCTORS SHALL BE PERMITTED AS THE DISCONNECTING MEANS WHERE OTHER DISCONNECTING ARE ALSO PROVIDED BY A READILY ACCESSIBLE CIRCUIT BREAKER.
- PRIOR TO ENERGIZING THE ELECTRICAL SYSTEM THE INTERRUPTING RATING OF THE MAIN BREAKER MUST BE VERIFIED AS BEING IN COMPLIANCE WITH SECTION 110-9 OF THE NEC BY ELECTRICAL CONTRACTOR
- THE MAIN ELECTRICAL PANEL AND FEEDERS IF NOT SPECIFICALLY DEFINED IN THESE PLANS ARE DESIGNED BY OTHERS.
- NOT USED
- ALL RECEPTACLES INSTALLED IN WET LOCATIONS (EXTERIOR) SHALL BE IN WEATHER PROOF (WP) ENCLOSURES. THE INTEGRITY OF WHICH IS NOT AFFECTED WHEN AN ATTACHMENT PLUG CAP IS INSERTED AND REMOVED. THE RECEPTACLE ITSELF SHALL ALSO BE LISTED FOR USE IN DAMP AND WET LOCATIONS AS PER MOST CURRENT EDITION OF THE NEC.
- EXTERIOR LIGHTS NOT INTENDED FOR 24 HOUR USE SHALL BE CONNECTED TO A PHOTOCELL OR TIMER
- ALL EXPOSED ELECTRICAL SERVICE CONDUCTORS SHALL BE FULLY ENCLOSED IN APPROPRIATELY SIZED NON METALLIC CONDUIT (NMT) PER THE LATEST EDITION OF THE NEC.
- ALL LIGHTING SWITCHES NOT INDICATED BY LOCATION ON THE PLAN VIEW(S) SHALL BE LOCATED @ THE DIRECTION OF THE OWNER IN FULL COMPLIANCE WITH ALL APPLICABLE SECTIONS OF THE NEC.
- ALL SMOKE DETECTORS SHALL BE 120 VOLT (120 V.) WITH BATTERY BACKUP OF THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER. INSTALLATION SHALL BE INSIDE AND NEAR ALL BEDROOMS ON A SEPARATE CIRCUIT TO THE MAIN DISTRIBUTION PANEL. REFER TO PLAN VIEW FOR SUGGESTED LOCATIONS WITH ALL ADJUSTMENTS AND ADDITIONAL LOCATIONS DETERMINED BY LOCAL FIRE MARSHALL.
- PROVIDE COMBINATION SMOKE/CARBON MONOXIDE DETECTORS WHEN ANY FOSSIL FUEL APPLIANCES ARE PROVIDED OR INSTALLED.
- ELECTRICAL CONTRACTOR SHALL REFER TO SPECIFIC LOCATIONS AND NOTES ON THE PLAN VIEWS FOR GROUND FAULT INTERRUPTER CIRCUITS (GFI) SWITCHES, RECEPTACLES AND RELATED ELEMENTS.
- ELECTRICAL CONTRACTOR SHALL REFER TO SPECIFIC LOCATIONS AND NOTES ON THE PLAN VIEWS(S) FOR ARC FAULT CIRCUIT INTERRUPTER (AFCI) SWITCHES, RECEPTACLES AND RELATED ELEMENTS.
- PROVIDE TAMPER RESISTANT RECEPTACLES IN ACCORDANCE WITH SECTION 406.11 OF 2014 NEC
- REFER TO ELECTRICAL PLANS, DETAILS, SECTIONS, NOTES AND SPECIFICATIONS FOR FOR ADDITIONAL REQUIREMENTS.

1 FIRST LEVEL ELECTRICAL PLAN VIEW
E2.0.0 SCALE: N.T.S.



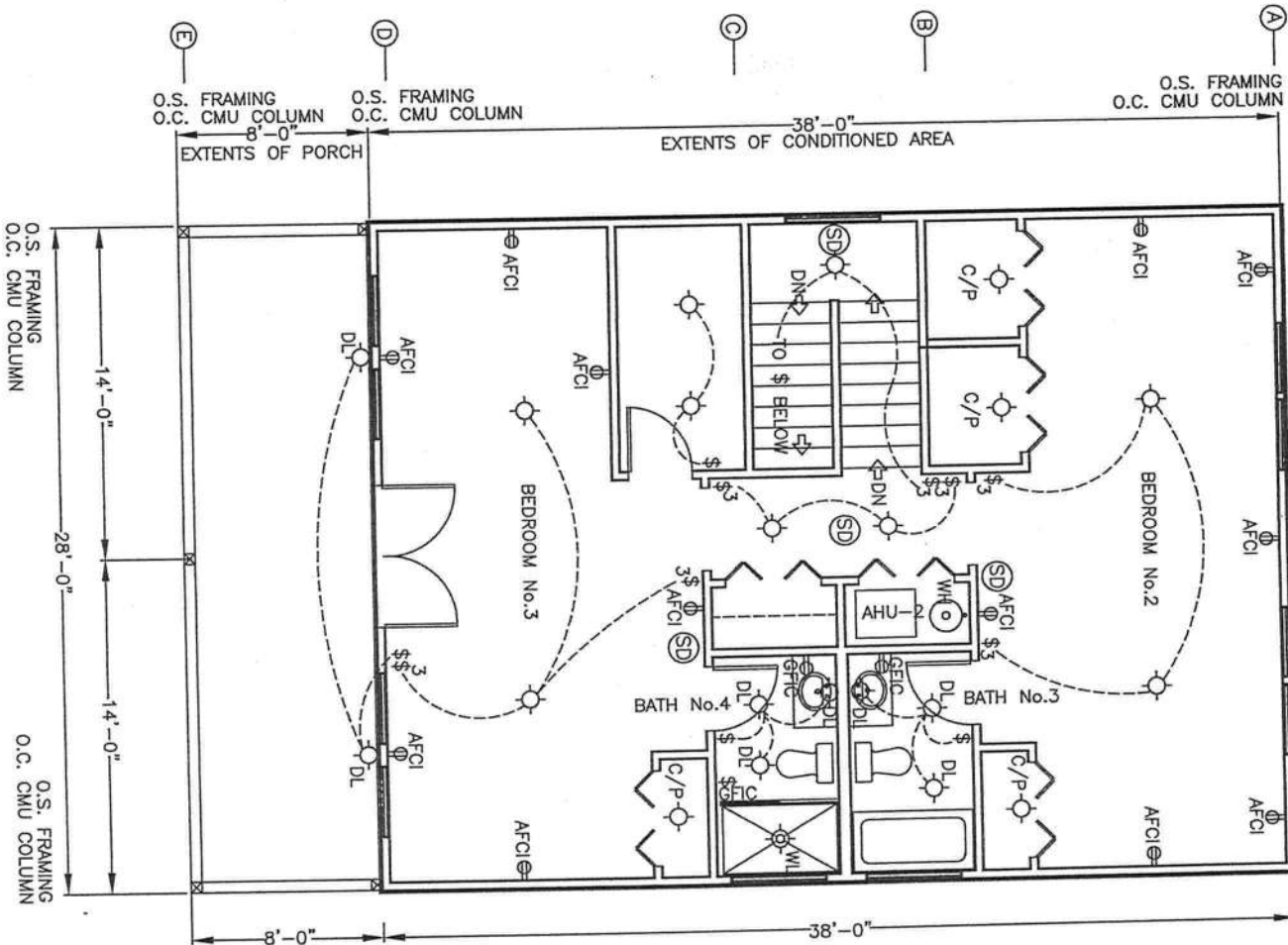
SCALE NOTE:
PLAN VIEW: 1/8"=1'-0"

ELECTRICAL FIXTURE SYMBOLS

- FLUORESCENT STRIP FIXTURE, SURFACE MOUNTED
- FLUORESCENT, SURFACE MOUNTED
- OWNER SELECTED FIXTURE - CONTRACTOR INSTALLED
- DROP MOUNTED FIXTURE
- WALL BRACKET MOUNTED INCANDESCENT OR HID FIXTURE
- SURFACE MOUNTED FIXTURE
- RECESSED DOWNLIGHT
- RECESSED DOWNLIGHT OR SURFACE MOUNTED FIXTURE - DAMP LOCATION RATED
- RECESSED DOWNLIGHT OR SURFACE MOUNTED FIXTURE - WET LOCATION RATED
- RECESSED DOWNLIGHT OR SURFACE MOUNTED FIXTURE - FLUORESCENT LAMP
- RECESSED DOWNLIGHT OR SURFACE MOUNTED FIXTURE - OWNER'S DIRECTION
- EXTERIOR FLOOD LIGHTING: LOCATIONS & SWITCHES @ OWNER'S DIRECTION
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- NOTE: LOCATE @ OWNER'S DIRECTION
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- FAN AND/OR LIGHT MAY BE SWITCHED - REFER TO PLAN VIEWS(S)
- EXHAUST FAN: OWNER APPROVED PROVIDING A MIN. 40 CFM
- EF-1 EXHAUST FAN: OWNER APPROVED PROVIDING A MIN. 30 CFM
- EF-2 EXHAUST FAN: OWNER APPROVED PROVIDING A MIN. 30 CFM
- EF-3 EXHAUST FAN: BY OWNER APPROVED STOVE MANUFACTURER
- CAT 5 WIRE @ OWNER SELECTED LOCATIONS
- SPEAKERS & RELATED SOUND DEVICES - LOCATE @ OWNER'S DIRECTION
- TELEVISION COAXIAL OUTLET SHALL BE LOCATED 18" A.F.F.
- NOTE: SEE PLAN VIEW & LOCATE ADDITIONAL @ OWNER'S DIRECTION
- SMOKE DETECTOR: CEILING MOUNTED UNLESS OTHERWISE NOTED
- ALL SMOKE DETECTORS SHALL BE 120 VOLT WITH BATTERY BACKUP. OF
- THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER. INSTALL
- INSIDE AND NEAR ALL BEDROOMS ON SEPARATE CIRCUIT TO THE MAIN
- DISTRIBUTION PANEL. ADDITIONAL LOCATIONS MAY BE DETERMINED BY THE
- FIRE MARSHALL. REFER TO PLAN VIEWS FOR LOCATIONS
- FIRST LEVEL ELECTRICAL NOTES: ALL LIGHTING, SWITCHING FOR LIGHTING AND
- RECEPTACLES SHALL BE WET LABLE AND IN APPROPRIATE WEATHER PROOF
- BOXES W/ APPROVED CONNECTIONS WITH GROUND FAULT INTERRUPTER
- CIRCUITRY.
- ALL WIRING SHALL BE INSTALLED IN NMT CONDUIT PER LATEST EDITION OF
- THE N.E.C.

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- WHEN WATER HEATERS ARE INSTALLED THEY SHALL BE PROVIDED WITH READILY ACCESSIBLE DISCONNECTS ADJACENT TO THE WATER HEATERS SERVED. THE BRANCH CIRCUIT SWITCH OR CIRCUIT BREAKER SHALL BE PERMITTED TO BE LOCATED WITHIN SIGHT FROM THE WATER HEATER OR IS CAPABLE OF BEING LOCKED IN THE OPEN POSITION.
- HVAC EQUIPMENT SHALL BE PROVIDED WITH READILY ACCESSIBLE DISCONNECTS ADJACENT TO THE EQUIPMENT SERVED. A UNIT SWITCH WITH A MARKED "OFF" THAT IS A PART OF THE HVAC EQUIPMENT AND DISCONNECTS ALL UNGROUNDED CONDUCTORS SHALL BE PERMITTED AS THE DISCONNECTING MEANS WHERE OTHER DISCONNECTING ARE ALSO PROVIDED BY A READILY ACCESSIBLE CIRCUIT BREAKER.
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- THE MAIN ELECTRICAL PANEL AND FEEDERS IF NOT SPECIFICALLY DEFINED IN THESE PLANS ARE DESIGNED BY OTHERS.
- NOT USED
- ALL RECEPTACLES INSTALLED IN WET LOCATIONS (EXTERIOR) SHALL BE IN WEATHER PROOF (WP) ENCLOSURES. THE INTEGRITY OF WHICH IS NOT AFFECTED WHEN AN ATTACHMENT PLUG CAP IS INSERTED OR REMOVED. THE RECEPTACLE ITSELF SHALL ALSO BE LISTED FOR USE IN DAMP AND WET LOCATIONS AS PER MOST CURRENT EDITION OF THE NEC.
- EXTERIOR LIGHTS NOT INTENDED FOR 24 HOUR USE SHALL BE CONNECTED TO A PHOTOCELL OR TIMER.
- ALL EXPOSED ELECTRICAL SERVICE CONDUCTORS SHALL BE FULLY ENCLOSED IN APPROPRIATELY SIZED NON METALLIC CONDUIT (NMT) PER THE LATEST EDITION OF THE NEC.
- ALL LIGHTING SWITCHES NOT INDICATED BY LOCATION ON THE PLAN VIEWS(S) SHALL BE LOCATED @ THE DIRECTION OF THE OWNER IN FULL COMPLIANCE WITH ALL APPLICABLE SECTIONS OF THE NEC.
- ALL SMOKE DETECTORS SHALL BE 120 VOLT (120 V.) WITH BATTERY BACKUP OF THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER. INSTALLATION SHALL BE INSIDE AND NEAR ALL BEDROOMS ON A SEPARATE CIRCUIT TO THE MAIN DISTRIBUTION PANEL. REFER TO PLAN VIEW FOR SUGGESTED LOCATIONS WITH ALL ADJUSTMENTS AND ADDITIONAL LOCATIONS DETERMINED BY LOCAL FIRE MARSHALL.
- PROVIDE COMBINATION SMOKE/CARBON MONOXIDE DETECTORS WHEN ANY FOSSIL FUEL APPLIANCES ARE PROVIDED OR INSTALLED.
- ELECTRICAL CONTRACTOR SHALL REFER TO SPECIFIC LOCATIONS AND NOTES ON THE PLAN VIEWS FOR GROUND FAULT INTERRUPTER CIRCUITS (GFI) SWITCHES, RECEPTACLES AND RELATED ELEMENTS.
- ELECTRICAL CONTRACTOR SHALL REFER TO SPECIFIC LOCATIONS AND NOTES ON THE PLAN VIEWS(S) FOR ARC FAULT CIRCUIT INTERRUPTER (AFCI) SWITCHES, RECEPTACLES AND RELATED ELEMENTS.
- PROVIDE TAMPER RESISTANT RECEPTACLES IN ACCORDANCE WITH SECTION 406.11 OF 2014 NEC
- REFER TO ELECTRICAL PLANS, DETAILS, SECTIONS, NOTES AND SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

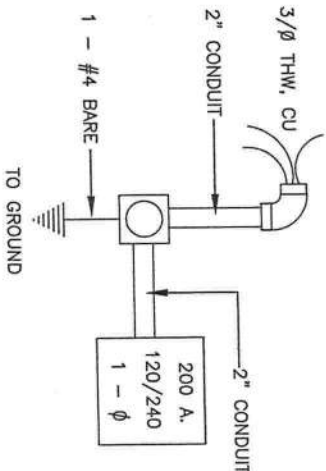


3 SECOND LEVEL PLAN VIEW

E3.0.0

ELECTRICAL NOTES

1. ALL WORK SHALL BE FULLY COMPLIANT WITH THE MOST CURRENT EDITION OF THE NATIONAL ELECTRICAL CODE (NEC).
2. ALL CIRCUITS & EQUIPMENT SHALL BE GROUNDED IN ACCORDANCE WITH THE APPROPRIATE ARTICLES OF THE NEC.
3. WHEN LIGHT FIXTURES ARE INSTALLED IN CLOSETS THEY SHALL BE SURFACE MOUNTED OR RECESSED. INCANDESCENT FIXTURES SHALL HAVE COMPLETELY ENCLOSED LAMPS. SURFACE MOUNTED INCANDESCENT FIXTURES SHALL HAVE A MINIMUM CLEARANCE OF 12 INCHES AND ALL OTHER FIXTURES SHALL HAVE A MINIMUM CLEARANCE OF 6 INCHES FROM "STORAGE AREAS" AS DEFINED BY NEC 410-B(9).
4. WHEN WATER HEATERS ARE INSTALLED THEY SHALL BE PROVIDED WITH READILY ACCESSIBLE DISCONNECTS ADJACENT TO THE WATER HEATERS SERVED. THE BRANCH CIRCUIT SWITCH OR CIRCUIT BREAKER SHALL BE PERMITTED TO SERVE IS WITHIN SIGHT FROM THE WATER HEATER OR IS CAPABLE OF BEING LOCKED IN THE OPEN POSITION.
5. HVAC EQUIPMENT SHALL BE PROVIDED WITH READILY ACCESSIBLE DISCONNECTS ADJACENT TO THE EQUIPMENT SERVED. A UNIT SWITCH WITH A MARKED "OFF" THAT IS A PART OF THE HVAC EQUIPMENT AND DISCONNECTS ALL UNGROUNDED CONDUCTORS SHALL BE PERMITTED AS THE DISCONNECTING MEANS WHERE OTHER DISCONNECTING ARE ALSO PROVIDED BY A READILY ACCESSIBLE CIRCUIT BREAKER.
6. PRIOR TO ENERGIZING THE ELECTRICAL SYSTEM THE INTERRUPTING RATING OF THE MAIN BREAKER MUST BE VERIFIED AS BEING IN COMPLIANCE WITH SECTION 110-9 OF THE NEC BY ELECTRICAL CONTRACTOR
7. THE MAIN ELECTRICAL PANEL AND FEEDERS IF NOT SPECIFICALLY DEFINED IN THESE PLANS ARE DESIGNED BY OTHERS.
8. NOT USED
9. ALL RECEPTACLES INSTALLED IN WET LOCATIONS (EXTERIOR) SHALL BE IN WEATHER PROOF (WP) ENCLOSURES. THE INTEGRITY OF WHICH IS NOT AFFECTED WHEN AN ATTACHMENT PLUG CAP IS INSERTED OR REMOVED. THE RECEPTACLE ITSELF SHALL ALSO BE LISTED FOR USE IN DAMP AND WET LOCATIONS AS PER MOST CURRENT EDITION OF THE NEC.
10. EXTERIOR LIGHTS NOT INTENDED FOR 24 HOUR USE SHALL BE CONNECTED TO A PHOTOCELL OR TIMER.
11. ALL EXPOSED ELECTRICAL SERVICE CONDUCTORS SHALL BE FULLY ENCLOSED IN APPROPRIATELY SIZED NON METALIC CONDUIT (NMT) PER THE LATEST EDITION OF THE NEC.
12. ALL LIGHTING SWITCHES NOT INDICATED BY LOCATION ON THE PLAN VIEW(S) SHALL BE LOCATED @ THE DIRECTION OF THE OWNER IN FULL COMPLIANCE WITH ALL APPLICABLE SECTIONS OF THE NEC.
13. ALL SMOKE DETECTORS SHALL BE 120 VOLT (120 V.) WITH BATTERY BACKUP OF THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER. INSTALLATION SHALL BE INSIDE AND NEAR ALL BEDROOMS ON A SEPERATE CIRCUIT TO THE MAIN DISTRIBUTION PANEL. REFER TO PLAN VIEW FOR SUGGESTED LOCATIONS WITH ALL ADJUSTMENTS AND ADDITIONAL LOCATIONS DETERMINED BY LOCAL FIRE MARSHALL.
14. PROVIDE COMBINATION SMOKE/CARBON MONOXIDE DETECTORS WHEN ANY FOSSIL FUEL APPLIANCES ARE PROVIDED OR INSTALLED.
15. ELECTRICAL CONTRACTOR SHALL REFER TO SPECIFIC LOCATIONS AND NOTES ON THE PLAN VIEWS FOR GROUND FAULT INTERRUPTER CIRCUITS (GFCI) SWITCHES, RECEPTACLES AND RELATED ELEMENTS.
16. ELECTRICAL CONTRACTOR SHALL REFER TO SPECIFIC LOCATIONS AND NOTES ON THE PLAN VIEW(S) FOR ARC FAULT CIRCUIT INTERRUPTER (AFCI) SWITCHES, RECEPTACLES AND RELATED ELEMENTS.
17. PROVIDE TAMPER RESISTANT RECEPTACLES IN ACCORDANCE WOTH SECTION 406.11 OF 2014 NEC
18. REFER TO ELECTRICAL PLANS, DETAILS, SECTIONS, NOTES AND SPECIFICATIONS FOR FOR ADDITIONAL REQUIREMENTS.



1 ELECTRICAL SCHEMATIC RISER: 1ST LEVEL
E2.4.0 SCALE: NO SCALE

CLOTHES DRYER

- M5102.2 GENERAL CLOTHES DRYERS SHALL BE EXHAUSTED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS & REQUIREMENTS
- M1502.3 TERMINATION EXHAUST DUCTS SHALL TERMINATE ON THE OUTSIDE OF THE BUILDING. EXHAUST DUCT TERMINATION SHALL BE IN ACCORDANCE WITH DRYER MANUFACTURER'S INSTRUCTIONS. THE DUCT SHALL TERMINATE NOT LESS THAN 3 FEET IN ANY DIRECTION FROM OPENING INTO BUILDING. EXHAUST DUCT TERMINATIONS SHALL BE EQUIPPED WITH A BACKDRAFT DAMPER. SCREENS SHALL NOT BE INSTALLED AT THE DUCT TERMINATION
- M1502.4 DRYER EXHAUST DUCTS DRYER EXHAUST DUCTS SHALL CONFORM TO THE REQUIREMENTS OF SECTIONS M1502.4.1 THROUGH M1502.4.6.
- M1502.4.1 MATERIAL AND SIZE EXHAUST DUCTS SHALL HAVE A SMOOTH INTERIOR FINISH AND SHALL BE CONSTRUCTED OF METAL OF 0.016 INCH THICK. THE EXHAUST DUCT SIZE SHALL BE 4 INCHES NOMINAL IN DIAMETER.
- M1502.4.2 DUCT INSTALLATION EXHAUST DUCTS SHALL BE SUPPORTED AT 4 FOOT INTERVALS AND SECURED IN PLACE. THE INSERT END OF DUCT SHALL EXT END INTO THE ADJOINING DUCT IN THE DIRECTION OF THE AIRFLOW. DUCTS SHALL NOT BE JOINED WITH SCREENS THAT PROTRUDE INTO THE INSIDE OF THE DUCT.
- M1502.4.3 TRANSITION DUCT TRANSITION DUCTS SHALL BE A SINGLE LENGTH THAT IS LISTED AND LABELED IN ACCORD WITH UL 2158A. TRANSITION DUCT SHALL BE A MAXIMUM OF 8 FEET IN LENGTH.
- M1502.4.4 DUCT LENGTH M1502.4.4 DUCT LENGTH SHALL BE DETERMINED BY ONE OF THE METHODS SPECIFIED IN SECTION M1502.4.4.1 OR M1502.4.4.2
- M1502.4.4.1 SPECIFIED LENGTH M1502.4.4.1 SPECIFIED LENGTH OF THE EXHAUST DUCT SHALL BE 25 FEET FROM THE CONNECTION TO THE TRANSITION DUCT FROM DRYER TO THE OUTLET TERMINAL. SEE TABLE M1502.4.4.1 FOR REDUCED LENGTH DUE TO TURNS.
- INTAKE OPENINGS
- R303.5.1 INTAKE OPENINGS MECHANICAL AND GRAVITY OUTDOOR INTAKE OPENINGS SHALL BE LOCATED A MINIMUM 10 FEET FROM ANY HAZARDOUS OR NOXIOUS CONTAMINATE, SUCH AS VENTS AS CHIMNEYS, PLUMBING VENTS, STREETS, ALLEYS, PARKING LOTS AND LOADING DOCKS, EXCEPT AS OTHERWISE SPECIFIED IN THIS CODE. WHERE SOURCE OF CONTAMINATE IS LOCATED WITHIN 10 FEET OF AN AIR INTAKE OPENING THE OPENINGS SHALL BE LOCATED A MINIMUM OF 2 FEET BELOW THE CONTAMINATE SOURCE.
- R303.6 OUTSIDE OPENING PROTECTION AIR EXHAUST AND INTAKE OPENINGS THAT TERMINATE OUTDOORS SHALL BE PROTECTED WITH CORROSION RESISTANT SCREENS, LOUVERS OR GRILLES HAVING A MINIMUM OPENING OF 1/4 INCH AND A MAXIMUM OPENING OF 1/2 INCH IN ANY DIMENSION. OPENING SHALL BE PROTECTED AGAINST LOCAL WEATHER CONDITIONS. OUTDOOR AND EXHAUST INTAKE OPENINGS SHALL MEET THE PROVISIONS FOR EXTERIOR WALL OPENING PROTECTIVES IN ACCORDANCE WITH THIS CODE.
- WINDOW AND OTHER OPENINGS FOR THE PURPOSE OF LIGHT AND VENTILATION IN THE EXTERIOR WALLS ACCESSIBLE TO RODENTS BY THE WAY OF EXPOSED PIPES, WIRES, CONDUITS AND OTHER APURTENANCES SHALL BE COVERED WITH WIRE MESH (0.035 INCH) THICK OR SOLID METAL GUARDS (0.024 INCH) THICK OR HEAVIER. GUARDS SHALL BE FITTED AROUND THESE FIXTURES. IN ADDITION THEY SHALL BE FASTENED SECURELY TO AND SHALL EXTEND PERPENDICULAR FROM THE EXTERIOR WALL FOR A MINIMUM 12 INCHES BEYOND EITHER SIDE OF APURTENANCES.
- MECHANICAL NOTES
1. ALL SUPPLY REGISTERS ARE ADJUSTABLE. EXCEPT WHERE OTHERWISE SPECIFIED
2. INTERIOR DOORS SHALL BE UNDERCUT ABOVE FINISH FLOOR FOR AN AIR RETURN AND OR NOTED IN SCHEDULES OR PLAN VIEWS ELSEWHERE THESE PLANS
3. BATHROOM VENT FANS SHALL PROVIDE A MINIMUM OF 50 CFM OF VENTILATION REFER TO ELECTRICAL SCHEDULES FOR ADDITIONAL MINIMUM CFM REQUIRED
4. HVAC EQUIPMENT SHALL BE EQUIPPED WITH OUTSIDE FRESH AIR INTAKE TO PROVIDE A MINIMUM OF 20 CFM PER PERSON
5. ALL VENTS SHALL BE DUCTED TO THE EXTERIOR OF DWELLING
6. ALL DUCT SYSTEM COMPONENTS INSTALLED IN THE ATTIC AREA W/ INSULATION SHALL HAVE A MINIMUM R-VALUE OF R-8.0
7. ALL HVAC COMPONENTS INSTALLED ON SITE SHALL BE INSTALLED, TESTED & BALANCED IN ACCORDANCE WITH SECTION 503.2.9.1 BY HVAC CONTRACTOR
8. INSTALLED HVAC EQUIPMENT SHALL MEET THE MINIMUM EFFICIENCIES OF TABLES 503.2.2 (1) THROUGH (8)

PLUMBING NOTES

1. ALL PLUMBING FIXTURES SHALL HAVE SEPERATE SHUT OFF VALVES
2. WATER HEATER SHALL HAVE SAFETY PAN WITH 3/4 INCH DRAIN TO EXTERIOR, T&P RELIEF VALVE W/ DRAIN TO EXTERIOR, AND A SHUTOFF VALVE WITHIN 3 FEET ON A POTABLE COLD WATER LINE W/ DRAIN TO EXTERIOR, AND A SHUTOFF VALVE WITHIN 3 FEET ON A POTABLE COLD WATER LINE
3. POTABLE WATER SUPPLY LINES SHALL BE PEX, CPVC OR COPPER. WHEN PEX SUPPLY LINES ARE INSTALLED THE MAXIMUM WATER HEATER TEMPERATURE SETTING IS 180°. THE PEX PIPE SHALL BE INSTALLED TO COMPLY WITH THE MANUFACTURER'S LIMITATIONS AND SPECIFIC REQUIREMENTS FOR THIS APPLICATION
4. WATER CLOSETS AVERAGE USAGE SHALL NOT EXCEED 1.6 GALLONS PER FLUSH
5. SHOWER STALLS SHALL BE COVERED WITH NON-ASBOBANT MATERIAL TO A MINIMUM HEIGHT OF 72 INCHES ABOVE THE FINISHED FLOOR
6. SHOWERS SHALL BE CONTROLLED BY AN APPROVED MIXING VALVE WITH A MAXIMUM WATER OUTLET TEMPERATURE 120° F.
7. AIR ADMITTANCE VALVES SHALL CONFORM TO ASSE 1051. THE VALVES SHALL BE LOCATED A MINIMUM OF 4 INCHES ABOVE THE HORIZONTAL DRAIN OR FIXTURE DRAIN BEING VENTED AND MUST BE INSTALLED IN WELL VENTILATED SPACES OR BE PROVIDED WITH VENTILATED ACCESS.
8. WATER HAMMER ARRESTORS SHALL BE INSTALLED WHERE QUICK CLOSING VALVES ARE UTILIZED (i.e. DISHWASHERS, CLOTHING WASHERS, ICE MAKERS AND OTHER QUICK CLOSING DEVICES W/ SOLENOID VALVES). ARRESTORS SHALL BE INSTALLED IN ACCORD WITH MANUFACTURER'S SPECIFICATIONS
9. SINKS 2.2 GALS AND LAVS 2.2 GALS PER MINUTE MAXIMUM @ 60 PSI
10. SHOWER HEADS SHALL NOT USE MORE THAN 2.5 GALS/MIN @ 80 PSI PER ANSI STD A112.181M
11. AN APPROVED THERMAL EXPANSION DEVICE SHALL BE INSTALLED IN THE WATER SUPPLY SYSTEM IN ACCORDANCE WITH MANUFACTURER'S INSTALLATION INSTRUCTIONS. SUCH A DEVICE IS REQUIRED WHEN BACK FLOW PREVENTORS, PRESSURE REDUCING VALVES, CHECK VALVES OR A SYSTEM WHICH MAY PREVENT PRESSURE RELIEF
12. AN ACCESSIBLE SHUT OFF VALVE SHALL BE PROVIDED AHEAD OF THE FIRST OUTLET OR BRANCH CONNECTED TO THE SERVICE OR DISTRIBUTION PIPE.
13. CHANGE IN DIRECTION OF SCHEDULE 40 DWV-PVC AND ABS DRAINAGE PIPING SHALL BE MADE BY THE APPROPRIATE USE OF 45° (0.785 RADUS) WTES. QUARTER BENDS, OR LONG SWEEP QUARTER BENDS, ONE-SIXTH, ONE-EIGHTH, ONE-SIXTEENTH, OR BY A COMBINATION OF THESE OR EQUIVALEANT FITTINGS. SINGLE AND DOUBLE SANITARY TEES AND QUARTER BENDS MAY BE USED IN DRAINAGE LINES ONLY WHERE THE DIRECTION OF FLOW IS FROM THE HORIZONTAL TO THE VERTICAL.
14. SHORT SWEEPS NOT LESS THAN 3 INCHES DIAMETER MAY BE USED IN SOIL AND WASTE LINES WHERE THE CHANGE IN DIRECTION OF FLOW IS FROM THE HORIZONTAL TO THE VERTICAL AND MAY BE FOR MAKING NECESSARY OFFSETS BETWEEN THE CEILING AND THE NEXT FLOOR ABOVE.
15. REFER TO PLUMBING DRAWINGS FOR ADDITIONAL SPECIFICATIONS & REQUIREMENTS
- WATER HEATER NOTES
1. WATER HEATER SHALL BE PROVIDED WITH A COLD WATER "DIP" TUBE WITH A HOLE AT THE TOP OR A VACUUE RELIEF VALVE INSTALLED IN THE COLD WATER SUPPLY LINE ABOVE THE TOP OF THE WATER HEATER TANK; BOTTOM FED WATER HEATERS SHALL HAVE A VACUUE RELIEF VALVE COMPLYING WITH ANSI Z21.22 INSTALLED.
2. WATER HEATERS SHALL BE PROVIDED WITH A TEMPERATURE AND PRESSURE RELIEF VALVE COMPLYING W. ANSI Z21.22 INSTALLED IN THE SHELL OF THE WATER HEATER TANK. THE VALVE SHALL BE ACTUATED BY THE WATER IN THE TOP 6 INCHES OF THE TANK AND SHALL HAVE A TEMPERATURE RATING OF NOT MORE THAN 210°F AND A PRESSURE SETTING NOT EXCEEDING THE TANKS RATED WORKING PRESSURE OR 150 PSI WHICHEVER IS LESS.
3. WATER HEATERS SHALL BE EQUIPPED WITH AN ENERGY CUTOFF DEVICE THAT WILL CUT OFF THE SUPPLY OF HEAT ENERGY TO THE WATER TANK BEFORE THE TEMPERATURE OF THE WATER IN THE TANK EXCEEDS 210°F.
4. REFER TO PLUMBING DRAWINGS FOR ADDITIONAL SPECIFICATIONS & REQUIREMENTS
- PIPING INSULATION
- ALL PIPING SHALL BE INSULATED AS FOLLOWS:
- | | |
|------------------------|--------------|
| CW PIPING & RUNOUTS | 1/2" THICK |
| HW PIPING (2" OR LESS) | 1" THICK |
| HW PIPING (OVER 2") | 1 1/2" THICK |
| HW RETURN PIPING | 1/2" THICK |
- ALL INSULATION SHALL MEET UL 273 FOR FLAME & SMOKE STANDARDS. INSULATION SHALL BE EQUAL TO OWENINGS CORNING FIBERGLASS ASJ/SSL-II WITH ALL SERVICE JACKET. ANY PIPING LOCATED IN AREAS SUBJECT TO FREEZING SHALL BE HEAT TAPED WITH DOUBLE THE THICKNESS OF THE ABOVE INSULATION. AREAS CONSIDERED SUBJECT TO FREEZING INCLUDE ATTIC AND NON-HEATED AREAS (EXCLUDING CHASES).

PROJECT No.
MORGAN-EA.0.0.DWG
SHEET No.
E4.0.0

DATE
02/26/16

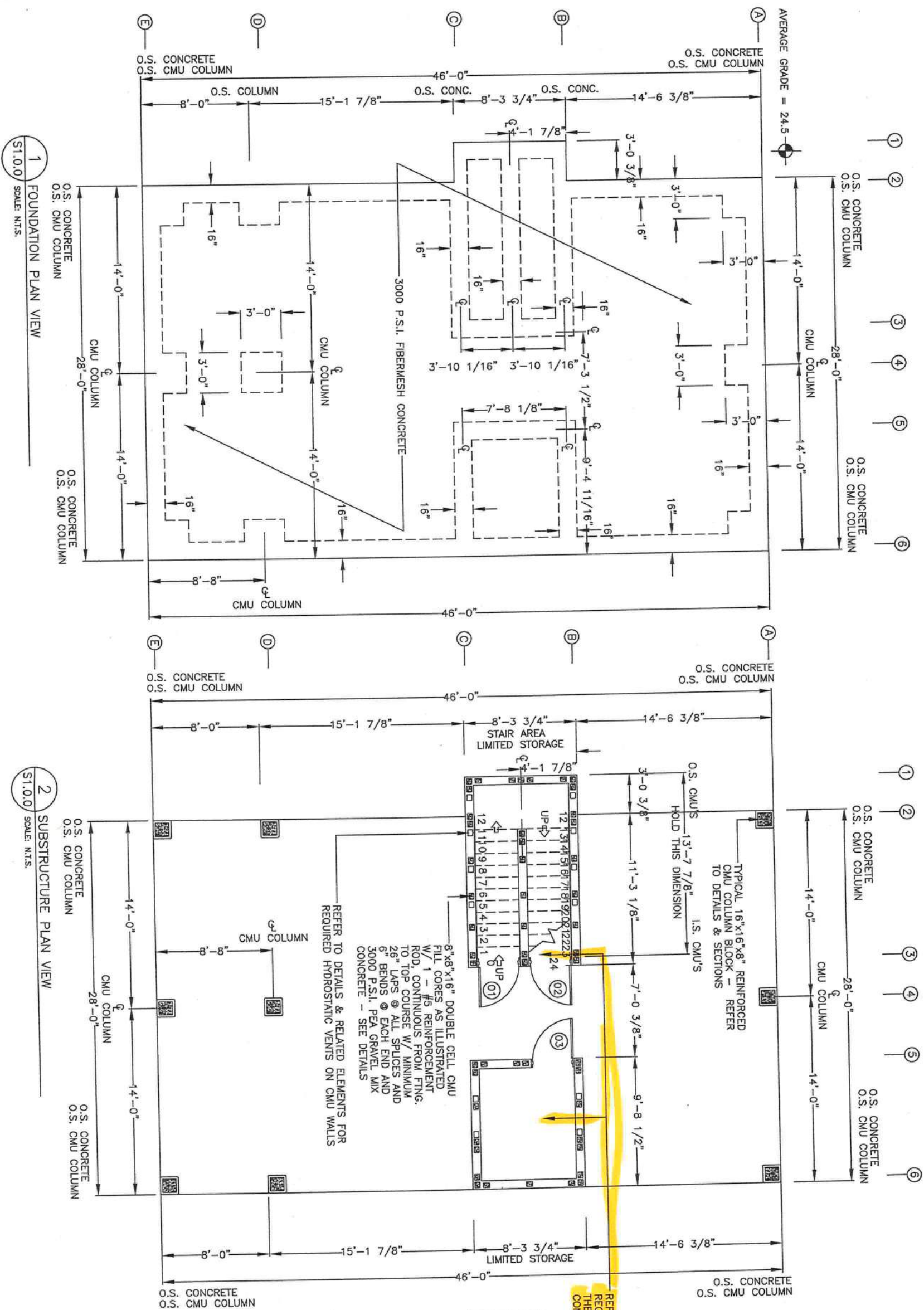
ELECTRICAL RISER
MISC. NOTES, REFERENCES & INSTRUCTIONS
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Curtis E. Keen, PE #23836
Certification of Authorization #3761
DATE:

MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA

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OAK, FLORIDA 32060
386-362-4787
ENG. LIC. EB 3761

SCALE NOTE:
PLAN VIEW: 1/8"=1'-0"



SCALE NOTE:
SECTIONS/DETAILS: N.T.S.

CONCRETE & RELATED REQUIREMENTS

CONCRETE

1. CONCRETE CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE AMERICAN CONCRETE INSTITUTE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE ACI 318," & "MANUAL CONCRETE PRACTICE, PART 1 ACI 305 & ACI 306," & MANUAL OF CONCRETE PRACTICE, PART 1 ACI 305 & 306" LATEST EDITION
2. CEMENT FOR CONCRETE SHALL MEET THE REQUIREMENTS OF ASTM C 150
3. AGGREGATES FOR CONCRETE SHALL MEET THE REQUIREMENTS OF ASTM C 33
4. WATER FOR CONCRETE SHALL BE POTABLE WATER
5. OPTIONAL: TEST CONCRETE FOR COMPRESSION WITH 1 SET OF 3 CYLINDERS FOR EACH 50 CUBIC YARDS OF CONCRETE PLACED ON A GIVEN DAY. BREAK 1 CYLINDER @ 7 DAYS AND THE OTHERS @ 28 DAYS. TESTING WILL BE PAID FOR BY OWNER.
6. CONCRETE SHALL HAVE STRENGTHS AND CHARACTERISTICS AS INDICATED ELSEWHERE THESE PLANS
7. SAWED JOINTS MUST BE SAWED WITHIN 24 HOURS OF PLACEMENT OF CONCRETE
8. REINFORCING STEEL SHALL MEET THE REQUIREMENTS OF ASTM A615 OR 60 UNLESS OTHERWISE NOTED
9. NOT USED
10. NOT USED
11. SLAB REINFORCING SHALL BE IN TOP 1/2 OF SLAB OR AS ILLUSTRATED
12. VIBRATE OR SCREEN ALL CONCRETE THOROUGHLY INTO PLACE
13. MINIMUM COVER OF REINFORCEMENT SHALL BE AS REQUIRED BY CODE
14. MOIST CURE CONCRETE FOR 7 DAYS AFTER PLACING
15. PROVIDE VAPOR BARRIER OF POLYETHYLENE UNDER SLAB(S)
16. PLACE CONTROL JOINTS IN SLAB TO PROVIDE MAXIMUM SLAB SIZE OF 600 SQUARE FEET
17. CONCRETE TEMPERATURE SHALL NOT EXCEED 90 DEGREES F DURING PLACEMENT
18. CONCRETE SHALL NOT BE ALLOWED TO FREE FALL MORE THAN 60 INCHES
19. CONCRETE SHALL BE PLACED IN A MANNER TO PREVENT SEGREGATION
20. AREAS TO RECEIVE CONCRETE SHALL BE CLEAR OF ANY DEBRIS AND SHALL HAVE REINFORCING STEEL PROPERLY POSITIONED PRIOR TO CONCRETE PLACEMENT
21. FOR LOCATION OF CONTROL OR CONSTRUCTION JOINTS OTHER THAN THOSE ILLUSTRATED VERIFY W/ ENGINEER
22. NOT USED
23. ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A 307
24. ANCHOR BOLTS AND DOWELS SHALL BE SET IN SUCH A MANNER THAT THEIR FULL EMBEDDED LENGTH SHALL BE COVERED WITH CONCRETE
25. LAP SPLICES SHALL BE 40 BAR DIAMETERS OR AS SHOWN OR NOTED ELSEWHERE THESE PLANS
26. DETAILING, FABRICATION AND PLACEMENT OF REINFORCEMENT STEEL SHALL CONFORM TO CURRENT CRSI AND ACI SPECIFICATIONS
27. REINFORCING STEEL SHALL BE FREE OF LOOSE RUST, MIL SCALE AND COATINGS THAT WOULD REDUCE OR DESTROY BOND
28. REINFORCING BARS SHALL NOT BE REDUCED IN SECTION, KINKED OR BENT OTHER THAN INDICATED
29. NOT USED
30. SUPPORT REINFORCING STEEL IN CHAIRS
31. KEEP ONE SET OF CONCRETE CYLINDERS ON SITE AT ALL TIMES TO MAKE SAMPLES IN CASE CONCRETE CHARACTER CHANGES

REINFORCING STEEL

1. REINFORCING STEEL SHALL BE #5 UNLESS OTHERWISE NOTED
2. ALL REINFORCING STEEL SHALL BE A MINIMUM OF GRADE 40 AND IDENTIFIED IN ACCORDANCE W/ ASTM A615, A616, A617 OR A 706
3. SPLICES SHALL BE LAP SPLICES W/ A MINIMUM OF 25" FOR #5 BARS
4. FOR MINIMUM COVER OVER REINFORCEMENT - SEE DETAILS & SECTIONS ELSEWHERE THESE PLANS
5. ALL REINFORCEMENT IN CMU'S SHALL EXTEND A MINIMUM OF 6" INTO ALL FOOTINGS W/ A 6" STANDARD BEND

METAL ACCESSORIES

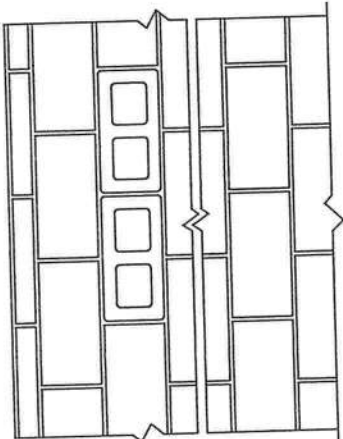
1. ALL JOINT REINFORCEMENT & ANCHOR TIES SHALL CONFORM TO ASTM A36 & A366 AS REQUIRED
2. LONGITUDINAL WIRES OF JOINT REINFORCEMENT SHALL BE FULLY EMBEDDED IN MORTAR OR GROUT W/ A MINIMUM COVER OF 5/8" WHEN EXPOSED TO EARTH OR WEATHER AND A MINIMUM OF 1/2" WHEN NOT EXPOSED TO EARTH OR WEATHER
3. METAL ACCESSORIES USED IN EXTERIOR WALL CONSTRUCTION SHALLBE GALVANIZED IN ACCORDANCE W/ ASTM A153
4. CLASS B-2
5. METAL ACCESSORIES USED IN INTERIOR WALL CONSTRUCTION SHALL BE MILD GALVANIZED IN ACCORDANCE W/ ASTM A641, CLASS 1

FILL COMPACTION

1. PRIOR TO GRADING OPERATIONS ALL SOIL, ORGANIC LITTER AND FILL SHALL BE STRIPPED FROM BUILDING AREA
2. COMPACTION SHALL NOT BE LESS THAN 98% OF THE STANDARD PROCTOR DENSITY
3. ALL FILL MATERIAL SHALL BE INORGANIC W/ NOT MORE THAN 30% BY WEIGHT FINER THAN 200 U.S. STANDARD SIEVE CONFORMING TO
 - A. LIQUID LIMIT, LW.....30, MAXIMUM
 - B. ELASTICITY, LW.....15, MAXIMUM
 - C. DRY UNIT WEIGHT.....100 LBS. PER CU. FT.
4. ALL FILL MATERIAL SHALL BE UNIFORMLY PLACED @ OPTIMUM MOISTURE CONTENT IN 6" UNIFORM LAYERS AND COMPACTED TO A DENSITY OF 98% OF THE STANDARD PROCTOR IN ACCORDANCE W/ ASTM D698T
5. FOOTINGS EXCAVATIONS SHALL BE INSPECTED PRIOR TO PLACING ANY CONCRETE TO ENSURE THAT FOOTINGS REST UPON SOUND EARTH
6. ALL SUBGRADES MUST BE LEVEL, SMOOTH AND UNIFORMLY COMPACTED
7. SUB GRADE MUST BE ACCURATE WITHIN 1/4" OF THE DESIGNATED LEVEL
8. ANY WALL WHICH IS TO RECEIVE BACK FILL ON BOTH SIDES SHALL HAVE THE BACK FILL PLACED SIMULTANEOUSLY ON BOTH SIDES IN EVEN LAYERS AS PREVIOUSLY DESCRIBED SO AS NOT TO APPLY UNEVEN LOADS

GENERAL

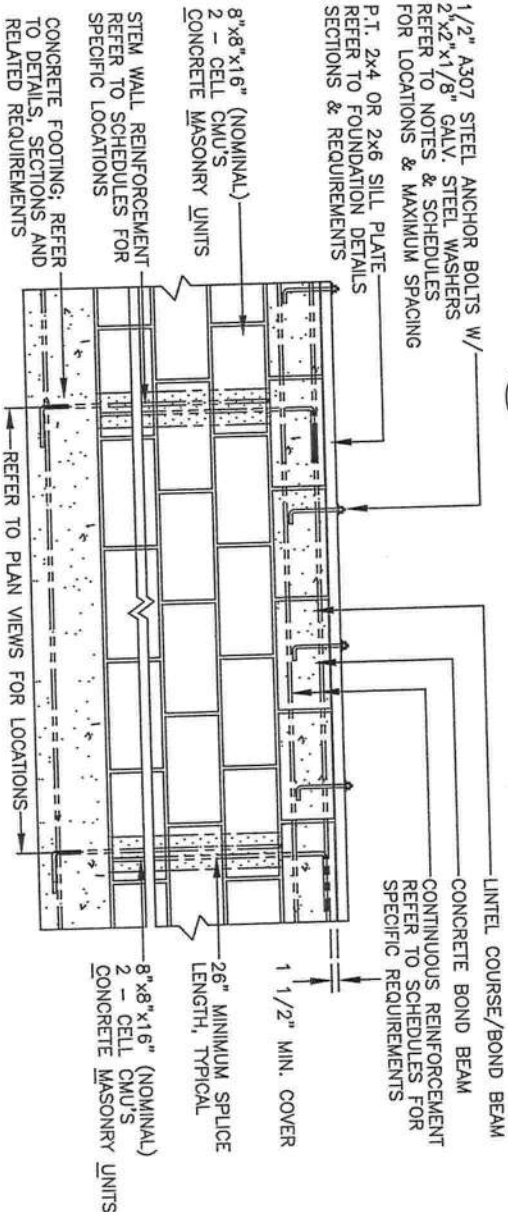
1. FOOTINGS SHALL BE LEVEL OR STEPPED AS INDICATED ON PLAN VIEWS & DETAILS OR SECTIONS
2. SOIL, WASTE PIPES OR BUILDING DRAINS PASSING UNDER A FOOTING OR THROUGH A FOUNDATION SHALL BE PROVIDED W/ A RELIEVING ARCH OR AN IRON PIPE SLEEVE A MINIMUM OF 2 - PIPE SIZES GREATER THAN THE PIPE PASSING THROUGH
3. STEM WALLS SHALL EXTEND NO GREATER THAN 3 FEET ABOVE THE FINISH GRADE AND CONSTRUCTED W/ THE PREVIOUSLY DESCRIBED MASONRY UNITS
4. ALL STATE AND LOCAL CODES SHALL BE COMPLIED WITH BY THE CONTRACTOR
5. 2000 P.S.F. SOILD BEARING PRESSURE SHALL BE OBTAINED UNDER ALL FOOTINGS & SLABS



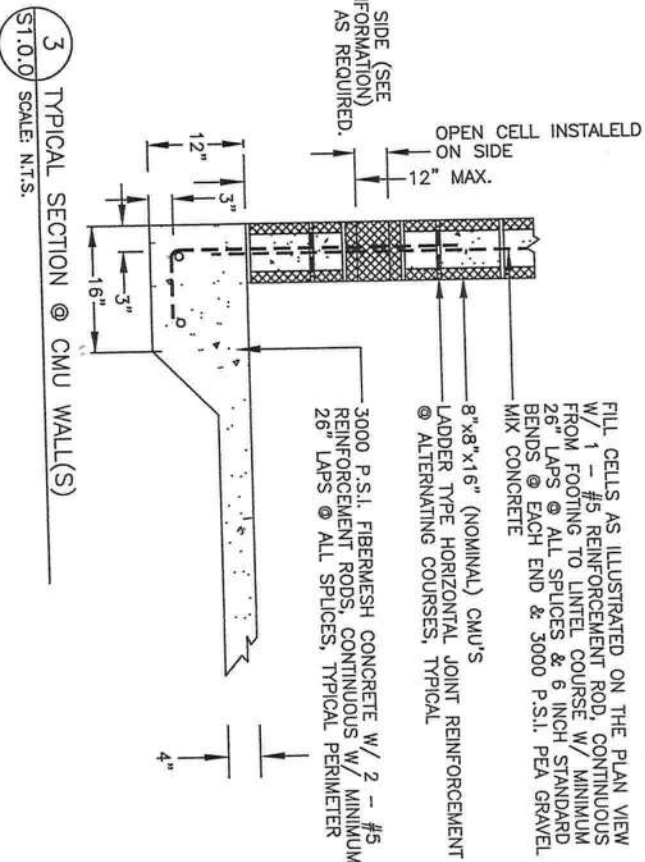
TYPICAL CMU; REFER TO PLAN VIEWS FOR REINFORCEMENT & SPECIFIC LOCATIONS

TYPICAL TWO CELL CMU INSTALLED ON SIDE W/ INSECT & VERMIN PROOF SCREENING TO PROVIDE THE MINIMAL HYDROSTATIC OPENINGS. REFER TO NOTES & REQUIREMENTS THESE OPENINGS SHALL NOT BE BLOCKED OR OBSTRUCTED IN ANY MANNER.

1 TYPICAL ELEVATION @ CMU WALL(S)
S1.1.0 SCALE: N.T.S.



2 TYPICAL SECTION @ CMU WALL(S)
S1.0.0 SCALE: N.T.S.

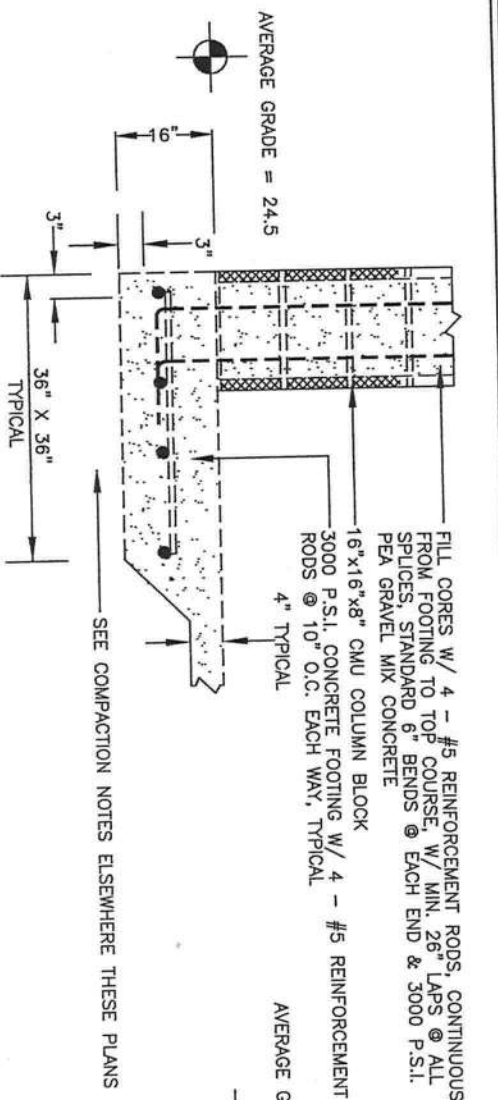


NOTE: INSTALL 3 - CMU'S ON SIDE (SEE ELEVATIONS FOR ADDITIONAL INFORMATION) & PROVIDE INSECT SCREENING AS REQUIRED.

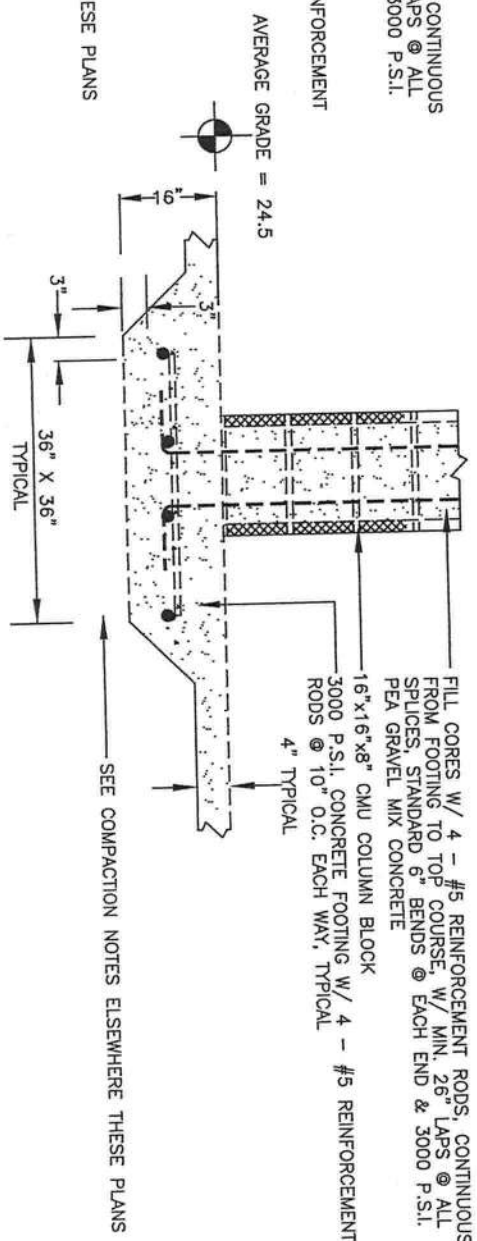
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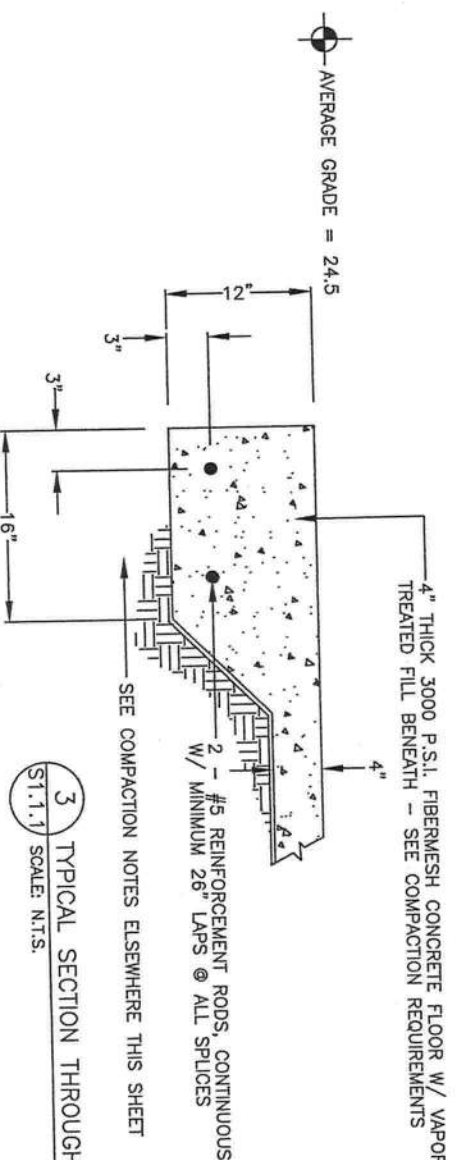
SCALE NOTE:
SECTIONS/DETAILS: N.T.S.



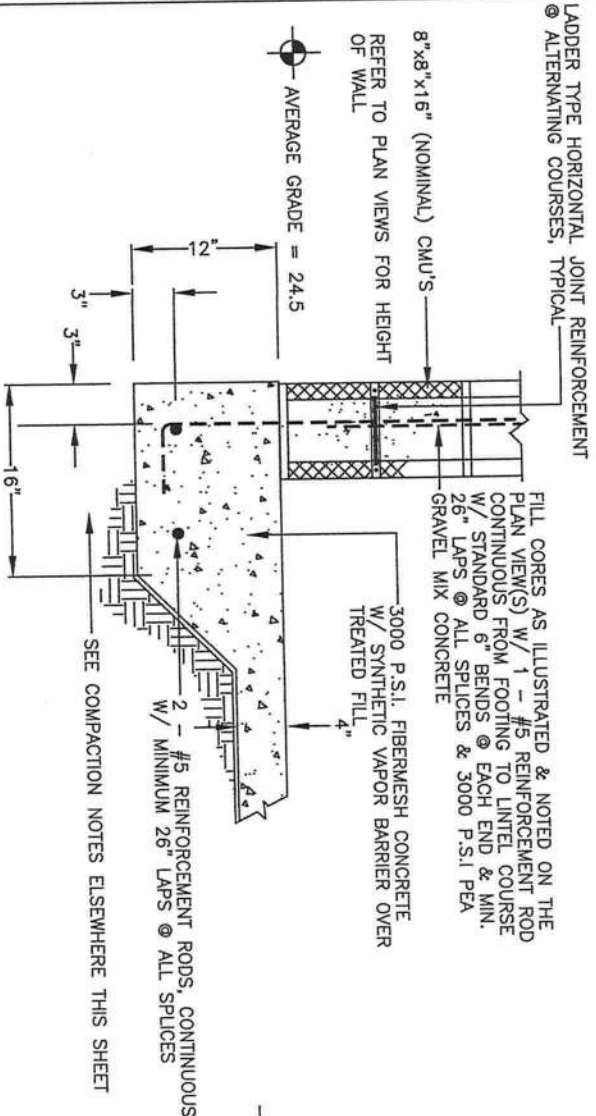
1 TYPICAL SECTION THROUGH EXTERIOR PIERS/ COLUMNS
S1.1.1 SCALE: N.T.S.



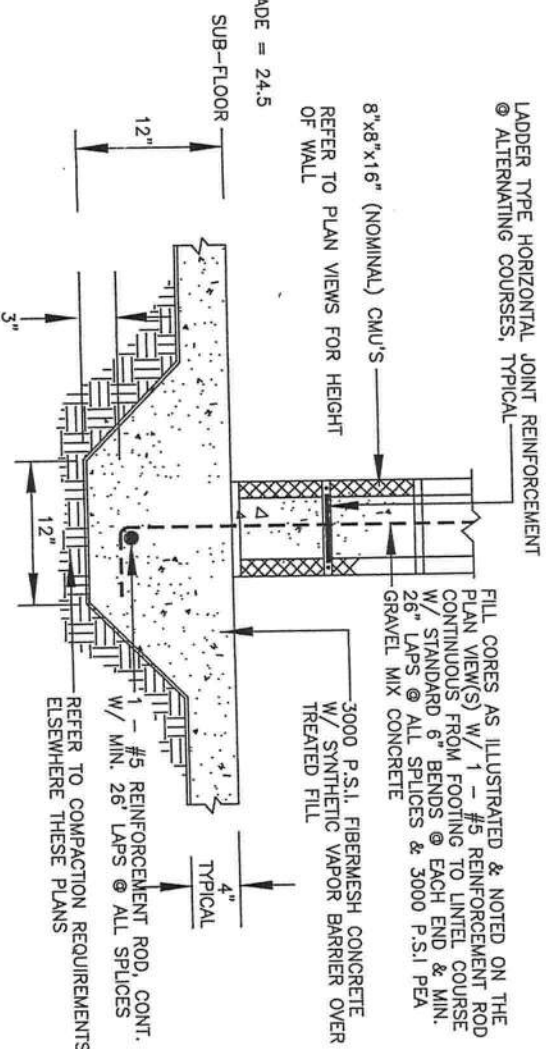
2 TYPICAL SECTION THROUGH INTERIOR PIERS/ COLUMNS
S1.1.1 SCALE: N.T.S.



3 TYPICAL SECTION THROUGH PERIMETER
S1.1.1 SCALE: N.T.S.



4 TYPICAL SECTION THROUGH EXTERIOR PIERS/ COLUMNS
S1.1.1 SCALE: N.T.S.



5 TYPICAL SECTION @ INTERIOR BEARING
S1.1.1 SCALE: N.T.S.

PROJECT No.
MORGAN-S1.1.1.DWG
SHEET No.
S1.1.1
DATE
02/26/16

REFERENCED FOUNDATION SECTIONS & DETAILS
MISC. NOTES, REFERENCES & INSTRUCTIONS
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Curtis E. Keen, PE #23836
Certification of Authorization #3761
DATE:

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SCALE NOTE:
PLAN VIEW: 1/8"=1'-0

SCHEDULE OF REQUIRED FLOOR SYSTEM MECHANICAL FASTENERS

BAND JOISTS/FLOOR GIRDERS

LOCATION OF ALL CONDITIONED AREA MULT-PLY FLOOR GIRDERS - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
FLOOR GIRDERS AS ILLUSTRATED SHALL BE 2 - OR - 3 - 14"x1.75" GANG LAM LVL BY LOUISIANA PACIFIC OR
EQUAL W/ A MIN. 2250 Fb 1.5E.
LOCATION OF ALL CONDITIONED AREA BAND JOISTS - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
BAND JOISTS AS ILLUSTRATED SHALL BE 1 - OR 2 - 14"x1.75" GANG LAM LVL BY LOUISIANA PACIFIC OR EQUAL WITH
A MINIMUM 2250 Fb 1.5E - ATTACHED TO 2x STUDS BENEATH W/ 1 - MODEL No. CS18 COILED STRAP BY SIMPSON
STRONG-TIE OR EQUAL W/ A MINIMUM LENGTH EACH SIDE OF LVL JOISTS TO STUD ABOVE AND BENEATH OF 9" (O.A.
LENGTH OD 34") AND SHALL BE INSTALLED @ ALL CORNERS 16" FROM ALL CORNERS & 48" O.C. TO COINCIDE W/ SP4
DEVICE @ TOP PLATES ABOVE AND BENEATH GIRDER W/ A TOTAL OF 18 - 10d COMMON NAILS

MANUFACTURED FLOOR TRUSSES

LOCATION OF ALL FLOOR JOISTS - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
LOCATION OF ALL FLOOR TRUSSES - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
FLOOR TRUSSES AS ILLUSTRATED SHALL BE ALPINE SYSTEM 42F W/ AN OVERALL DEPTH OF 12" INSTALLED @ 24"
O.C. & AS ILLUSTRATED OR NOTED & ATTACHED TO GIRDERS OR BAND JOISTS W/ 1 - MODEL No. IT412 BY
SIMPSON STRONG-TIE OR EQUAL @ EACH END OF EACH FLOOR TRUSS TO GIRDER OR BAND JOISTS CONNECTIONS
W/ A TOTAL OF 6 - 10d COMMON NAILS & 2 - 10d x 1 1/2" NAILS PER DEVICE
1 - MODEL No. H4 BY SIMPSON STRONG-TIE OR EQUAL SHALL BE ATTACHED TO EACH FLOOR TRUSS TO DOUBLE TOP
PLATES CONNECTIONS W/ A TOTAL OF 8 - 8d COMMON NAILS PER DEVICE

EXTERIOR WALL STUDS

LOCATION OF ALL EXTERIOR WALL 2x4 OR 2x6 STUDS - SEE FLOOR PLAN VIEW FOR LOCATIONS
EXTERIOR WALL STUDS SHALL BE 2x4 OR 2x6 SPRUCE @ 16" O.C., TYPICAL ALONG THE LENGTH OF THE EXTERIOR
WALLS. STUDS SHALL BE ATTACHED TO BOT. 2x PLATES AS DESCRIBED OR NOTED ELSEWHERE THESE PLANS AND
REQUIRE 1 - MODEL No. CS18 COILED STRAP BY SIMPSON STRONG-TIE OR EQUAL 34 INCHES LONG INSTALLED
PER ABOVE DIRECTIONS

EXTERIOR WALL OPENINGS

LOCATION OF ALL EXTERIOR 2x4 WALL OPENINGS - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
A "HEADER" OF 2 - 2x12 S.Y.P. W/ 1 - 15/32" CDX CORE SHALL BE INSTALLED AS NOTED THESE PLANS
36" WIDE OPENINGS REQUIRE A MINIMUM OF 2 CONTINUOUS STUDS EACH SIDE OF OPENINGS W/ 1 - MODEL No. H6
BY SIMPSON STRONG-TIE OR EQUAL @ EACH SIDE OF OPENINGS W/ A TOTAL OF 16 - 8d COMMON NAILS PER DEVICE
48" WIDE OPENINGS REQUIRE 2 - CONTINUOUS STUDS EACH SIDE OF OPENING W/ 1 - MODEL No. H6 BY SIMPSON
STRONG-TIE OR EQUAL @ EACH SIDE OF OPENINGS W/ A TOTAL OF 16 - 8d COMMON NAILS PER DEVICE.
72" WIDE OPENINGS REQUIRE 4 - CONTINUOUS STUDS EACH SIDE OF OPENING W/ 2 - MODEL No. H6 BY SIMPSON
STRONG-TIE OR EQUAL @ EACH SIDE OF OPENINGS W/ A TOTAL OF 16 - 8d COMMON NAILS PER DEVICE.

REFER TO DETAILS FOR ADDITIONAL REQUIREMENTS

PORCH AREA GIRDERS/BAND BEAMS

LOCATION OF PORCH GIRDERS/BAND BEAMS - SEE FLOOR SYSTEM FOR LOCATIONS
PORCH BAND BEAMS SHALL BE DOUBLE P.T. 2x10 No.2 S.Y.P. ATTACHED TO P.T. 6x6 COLUMNS W/ 1 - MODEL No. CCO3-
6SDS2.5 BY SIMPSON STRONG-TIE OR EQUAL @ CONNECTION OF 3-PLY GIRDER TO BAND BEAM LOCATIONS INSTALLED W/
A TOTAL OF 30 - 1 1/4" x 2 1/2" SDS SCREWS BY SIMPSON STRONG-TIE OR EQUAL.
CORNER PORCH BAND BEAMS SHALL BE ATTACHED TO P.T. 6x6 COLUMN W/ 1 - MODEL No. EPC46 BY SIMPSON STRONG-
TIE OR EQUAL W/ A TOTAL OF 14 - 16d COMMON NAILS
3 - PLY GIRDER SHALL BE ATTACHED TO DWELLING LVL GIRDER W/ 1 - MODEL No. U210-3 BY SIMPSON STRONG-TIE
OR EQUAL W/ A TOTAL OF 18 - 16d COMMON NAILS PER DEVICE.

DOUBLE P.T. BAND JOISTS SHALL BE ATTACHED TO LVL BAND BEAMS W/ 1 - MODEL No. LUS210-2 BY SIMPSON STRONG-
TIE OR EQUAL W/ A TOTAL OF 14 - 16d COMMON NAILS PER DEVICE

PORCH AREA DECK JOISTS

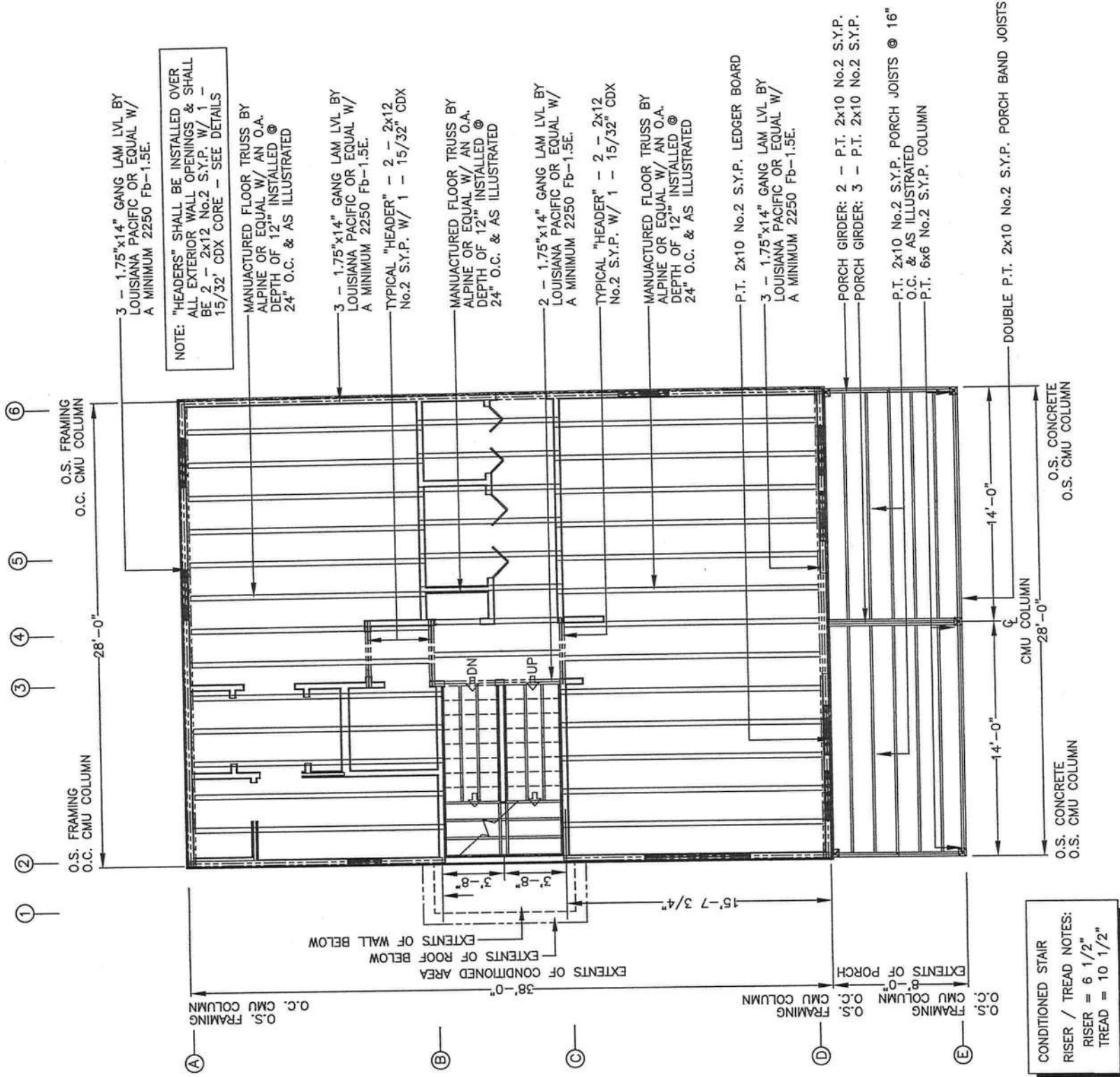
LOCATION OF PORCH DECK JOISTS - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
PORCH AREA DECK JOISTS SHALL BE P.T. 2x10 No.2 S.Y.P. @ 16" O.C. & AS ILLUSTRATED ON THE PLAN VIEW AND
SHALL BE ATTACHED TO GIRDERS AND BAND JOISTS W/ 1 - MODEL No. U210 BY SIMPSON STRONG-TIE OR EQUAL
@ EACH END OF EACH JOIST W/ A TOTAL OF 10 - 10d COMMON NAILS & 6 - 10d x 1 1/2" NAILS PER DEVICE
NOTE: A P.T. 2x8 No.2 S.Y.P. LEDGER BOARD IS REQUIRED TO BE ATTACHED TO MAIN DWELLING BAND JOISTS W/
#10d COMMON NAILS @ 12" O.C. STAGGERED - REFER TO PLAN VIEWS FOR LOCATIONS

PORCH DECKING

LOCATION OF REAR PORCH DECKING - SEE FLOOR SYSTEM PLAN VIEW FOR LOCATIONS
PORCH DECKING SHALL BE P.T. 2x6 No.2 S.Y.P. ATTACHED TO JOISTS, GIRDERS & BAND BEAMS W/ 3-10d ANNULAR
RING NAILS PER EACH CONNECTION TO EACH JOIST, GIRDER OR BAND BEAM. ALTERNATE SHALL BE 3 - #12 SCREWS
W/ A MINIMUM EMBEDMENT OF 2"

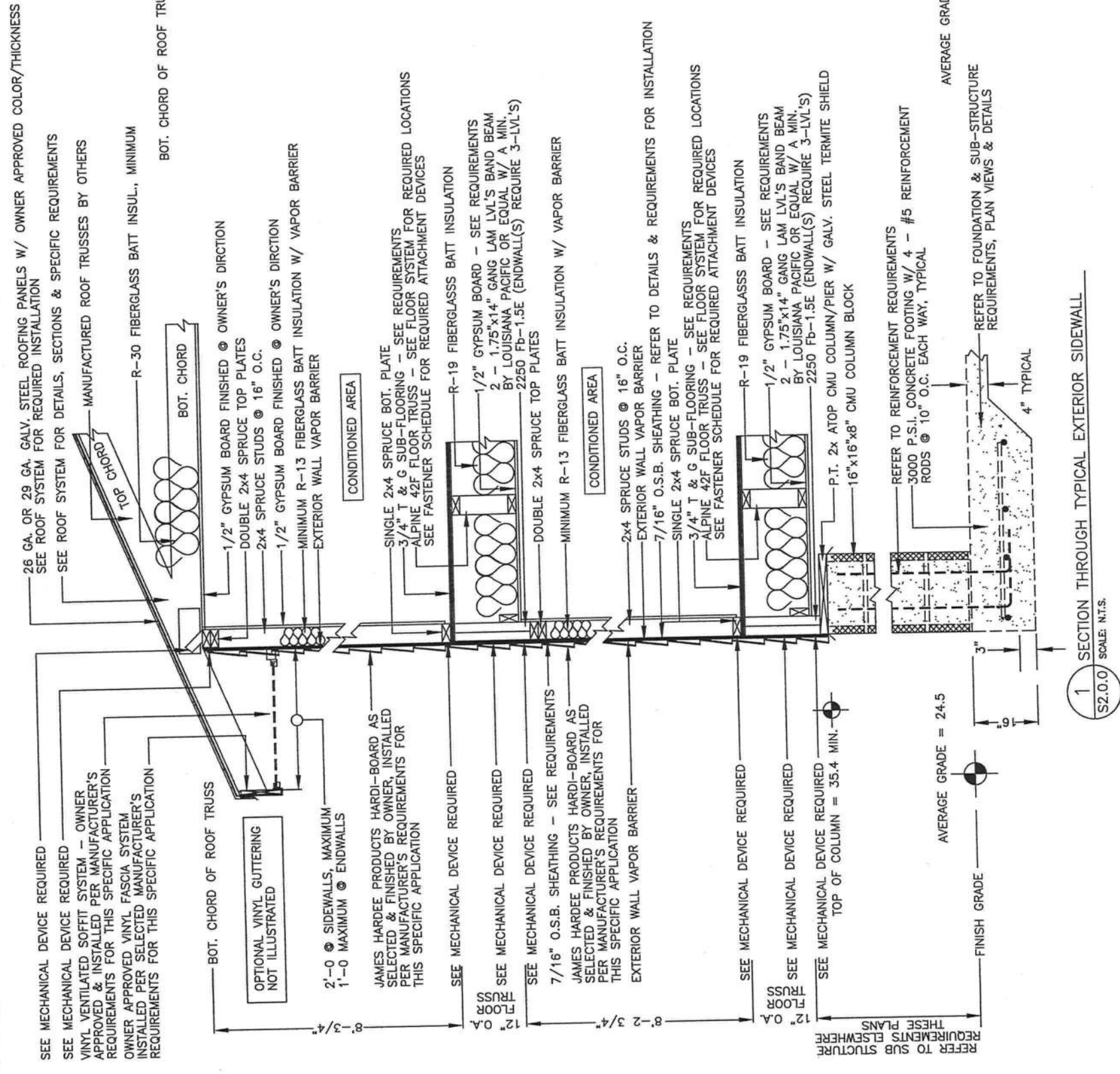
PORCH AREA P.T. WOOD COLUMNS

SEE ABOVE DEVICES - SEE ALSO SECTIONS 7 DETAILS
STAIR REQUIREMENTS
REFER TO DETAILS & SECTIONS ELSEWHERE THESE PLANS FOR SPECIFIC DEVICES & INSTALLATION



1 SECOND LEVEL FLOOR FRAMING PLAN VIEW
S1.3.0 SCALE: N.T.S.

SCALE NOTE:
SECTIONS/DETAILS: N.T.S.



1 SECTION THROUGH TYPICAL EXTERIOR SIDEWALL

2 SECTION THROUGH TYPICAL EXTERIOR ENDWALL
S2.0.0 SCALE: N.T.S.

PROJECT No. MORGAN-S2.0.0.DWG	DATE 02/26/16	REFERENCED EXTERIOR WALL SECTIONS & DETAILS MISC. NOTES, REFERENCES & INSTRUCTIONS
SHEET No. S2.0.0		

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SCALE NOTE:
SECTIONS/DETAILS: N.T.S.

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REFER TO TYPICAL EXTERIOR WALL SECTIONS
FOR 2x4 STUD, SHEATHING & MECHANICAL DEVICES
REFER TO INTERIOR FINISH & EXTERIOR CLADDING
REQUIREMENTS - SEE EXTERIOR TYP. WALL DETAILS
GAL. STEEL FLASHING & COUNTER FLASHING
REFER TO TYP. ROOF SECTIONS FOR FINISH MATERIALS
2x6 No.2 S.Y.P. RAFTERS @ 16" O.C.
SEE ROOF SYSTEM FOR REQUIREMENTS
SEE MECHANICAL FASTENER SCHEDULE

7/16" O.S.B. SHEATHING

2x6 No.2 S.Y.P. CEILING JOISTS
SEE ROOF SYSTEM FOR REQUIREMENTS
SOFFIT & FASCIA TO MATCH MAIN DWELLING
DOUBLE SPRUCE TOP PLATES
SEE MECHANICAL FASTENER SCHEDULE
2x4 SPRUCE STUDS @ 16" O.C., TYP.

R-13 FIBERGLASS BATT INSULATION

2x4 SPRUCE BOT. PLATE

SEE MECHANICAL FASTENER SCHEDULE

P.T. 2x ADJACENT TO CMU'S

LINTEL COURSE: 1 - #5 REINFORCEMENT ROD,
CONTINUOUS W/ MINIMUM 26" LAPS @ ALL
SPICES & 3000 P.S.I. PEA GRAVEL MIX CONC.

8"x8"x16" STD. 2-CELL CMU'S
1 - #5 REINFORCEMENT ROD, CONTINUOUS FROM
FOOTING TO LINTEL COURSE W/ STD. 6" BENDS @
EACH END & MINIMUM 26" LAPS @ ALL SPICES &
3000 P.S.I. PEA GRAVEL MIX CONCRETE - REFER
TO PLAN VIEWS FOR FILLED CELL LOCATIONS

NOTE: REFER TO PLAN VIEWS FOR ALL "OPEN"
CELL UNITS - SEE ALSO FOUNDATION PLANS &
RELATED REQUIREMENTS

LADDER TYPE HORIZONTAL JOINT REINFORCEMENT
@ ALTERNATE COURSES, TYPICAL

P.T. 2x8 No.2 S.Y.P. ATTACHED TO CMU WALL W/
1/2" x 6 1/2" GALV. STEEL LAG BOLTS W/ GALV.
STEEL WASHERS & MIN. EMBEDMENT OF 3" @ 12"
O.C., MAXIMUM, STAGGERED @ ALL LANDING SIDES

P.T. 2x8 No.2 S.Y.P. JOISTS ATTACHED TO BAND JOISTS
W/ 1 - MODEL No. U26 W/ ALL REQ'D. NAILS

P.T. 2x12 No.2 S.Y.P. STAIR STRINGERS - SEE PLAN
VIEWS FOR LOCATIONS. P.T. STRINGERS ADJACENT TO
CMU WALL(S) SHALL BE ATTACHED PER ABOVE 2x8'S

AVERAGE GRADE = 24.5

4" TYPICAL

16"

1 SECTION THROUGH PROJECTED STAIR CMU WALL

S2.1.0 SCALE: N.T.S.

REFER TO MECHANICAL DEVICE REQUIREMENTS
2x4 SPRUCE BOT. PLATE

3/4" T & G SUB-FLOORING - SEE REQUIREMENTS

ALPINE 42F FLOOR TRUSS - SEE FLOOR SYSTEM FOR REQUIRED LOCATIONS
SEE FASTENER SCHEDULE FOR REQUIRED ATTACHMENT DEVICES

2 - 1.75"x14" GANG LAM LVL'S BAND BEAM
BY LOUISIANA PACIFIC OR EQUAL W/ A MIN.
2250 Fb-1.5E (ENDWALL(S) REQUIRE 3-LVL'S)

24

TOP OF CMU WALL = 35.4 MIN.

R-19 FIBERGLASS BATT INSULATION

8"x8"x16" STD. 2-CELL CMU'S

1 - #5 REINFORCEMENT ROD, CONTINUOUS FROM
FOOTING TO LINTEL COURSE W/ STD. 6" BENDS @
EACH END & MINIMUM 26" LAPS @ ALL SPICES &
3000 P.S.I. PEA GRAVEL MIX CONCRETE - REFER
TO PLAN VIEWS FOR FILLED CELL LOCATIONS

BOND BEAM: 1 - #5 REINFORCEMENT ROD,
CONTINUOUS W/ MIN. 26" LAPS @ ALL SPICES
& 3000 P.S.I. PEA GRAVEL MIX CONCRETE SHALL
BE INSTALLED @ ALL 8"x8"x16" CMU WALLS

80"

TYPICAL @ ALL 8"x8"x16" CMU WALLS

1

2

3

AVERAGE GRADE = 24.5

REFER TO FOUNDATION & SUB-STRUCTURE
REQUIREMENTS, PLAN VIEWS & DETAILS

4" TYPICAL

16"

3

2

1

REFER TO FOUNDATION & SUB-STRUCTURE
REQUIREMENTS, PLAN VIEWS & DETAILS

4" TYPICAL

16"

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REFER TO FOUNDATION & SUB-STRUCTURE
REQUIREMENTS, PLAN VIEWS & DETAILS

4" TYPICAL

16"

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REFER TO FOUNDATION & SUB-STRUCTURE
REQUIREMENTS, PLAN VIEWS & DETAILS

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REQUIREMENTS, PLAN VIEWS & DETAILS

4" TYPICAL

16"

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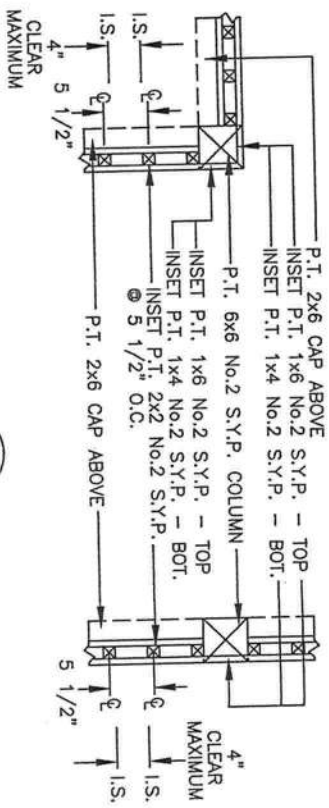
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1

REFER TO FOUNDATION & SUB-STRUCTURE

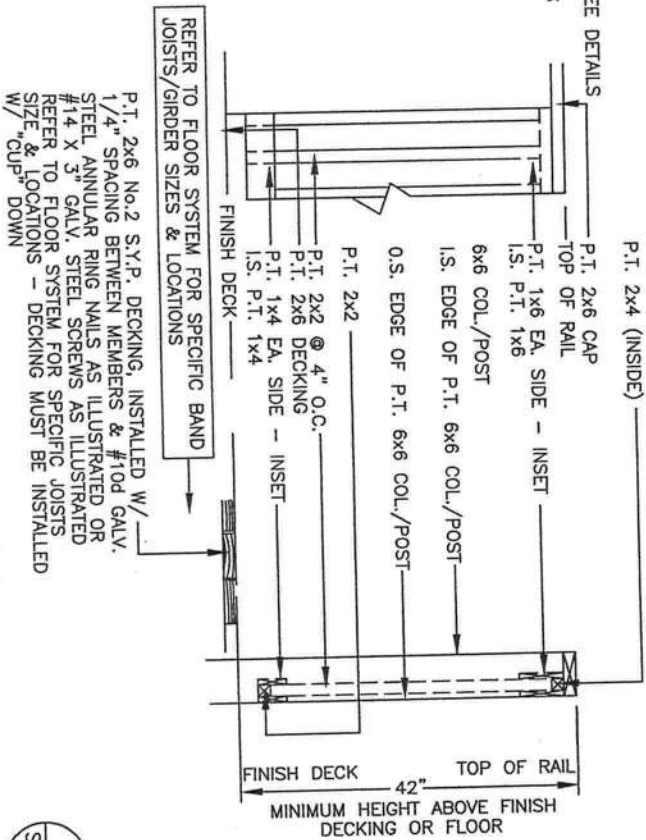
SCALE NOTE:
SECTIONS/DETAILS: N.T.S.

THIS HEIGHT ABOVE FINISH GRADE REQUIRES A GUARD RAILING SYSTEM
NOTE: ALTERNATE MANUFACTURED RAIL SYSTEM INSTALLED PER SELECTED
MANUFACTURER'S REQUIREMENTS FOR THIS SPECIFIC APPLICATION
INCLUDING ALL ATTACHMENTS & DEVICES & INSTALLED TO ENSURE
A FULLY COMPLIANT GUARD RAILING SYSTEM

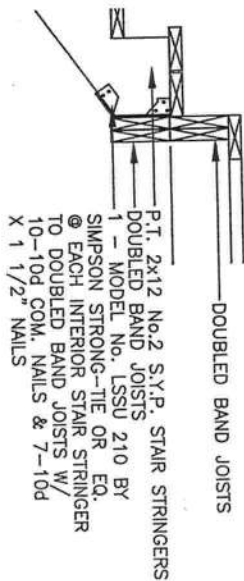


3 CORNER POSTS: PLAN VIEW
S2.2.0 SCALE: N.T.S.

4 IN-LINE POSTS: PLAN VIEW
S2.2.0 SCALE: N.T.S.



2 TYPICAL SECTION THROUGH GUARD RAIL
S2.2.0 SCALE: N.T.S.

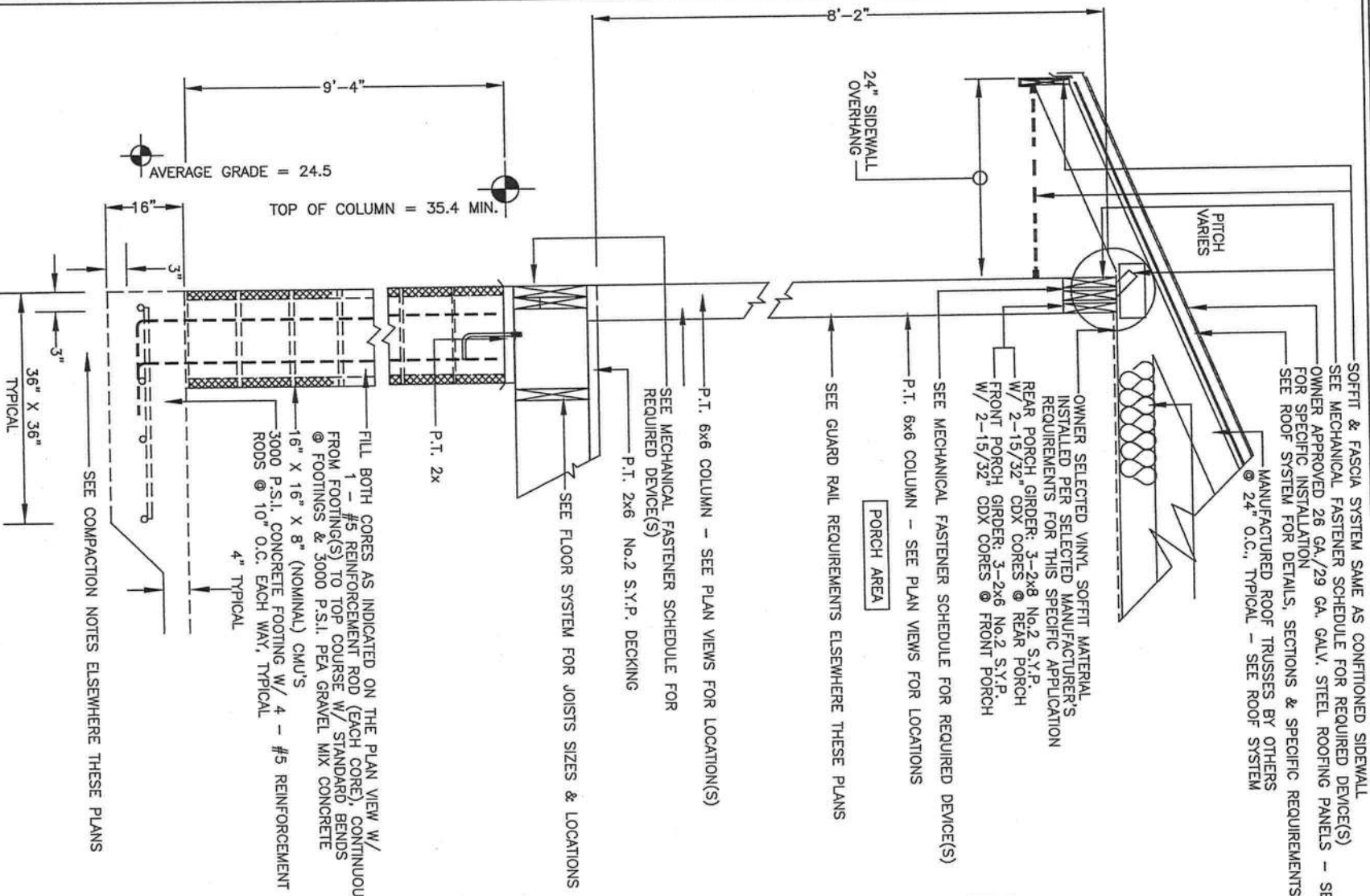
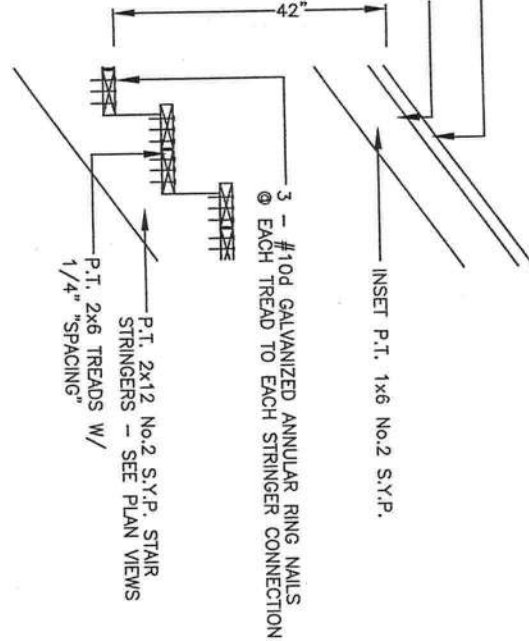


6 SUB STRUCTURE STAIR LANDING
S2.2.0 SCALE: N.T.S. SIMILAR @ CONDITIONED STAIR ABOVE

CONDITIONED STAIR
RISER / TREAD NOTES:
RISER = 6 1/2"
TREAD = 10 1/2"

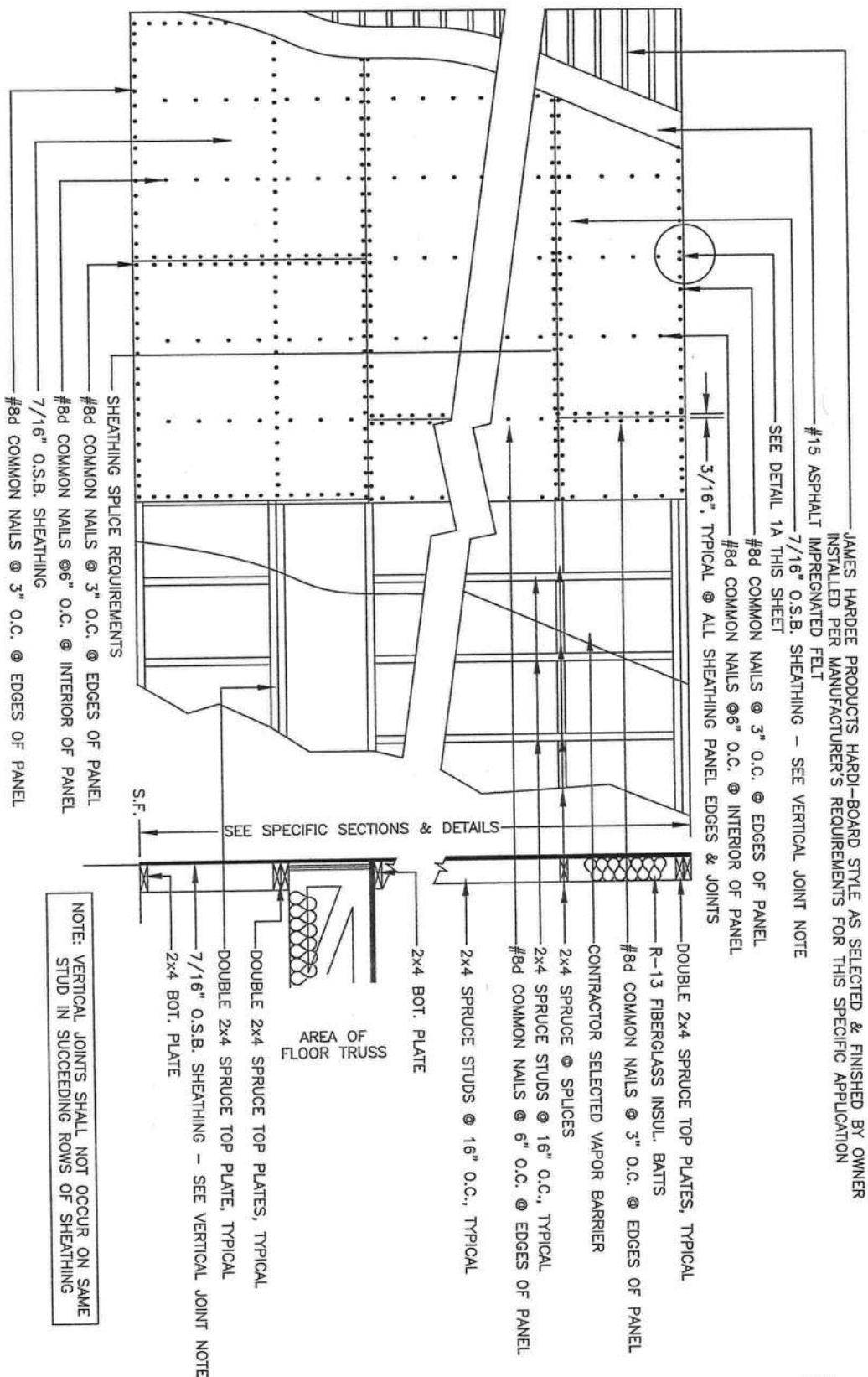
SUB-STRUCTURE STAIR
RISER / TREAD NOTES:
RISER = 6 1/8"
TREAD = 10 1/2"

5 SUB STRUCTURE STAIR DETAILS
S2.2.0 SCALE: N.T.S. SIMILAR @ CONDITIONED STAIR ABOVE

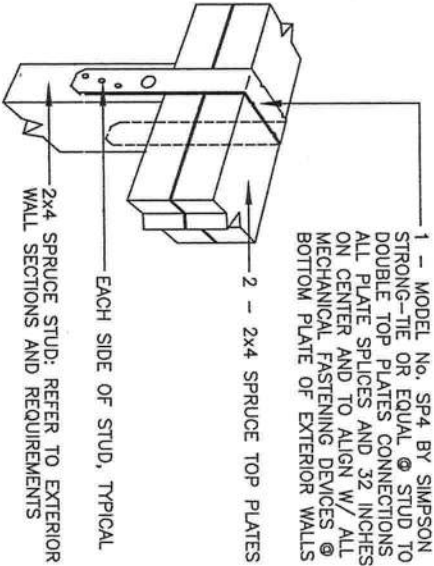


1 SECTION THROUGH REAR PORCH SIDEWALL
S2.2.0 SCALE: N.T.S.

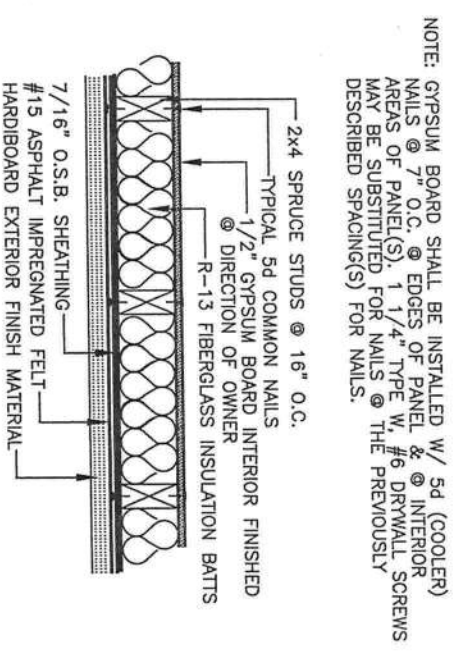
SCALE NOTE:
SECTIONS/DETAILS: N.T.S.



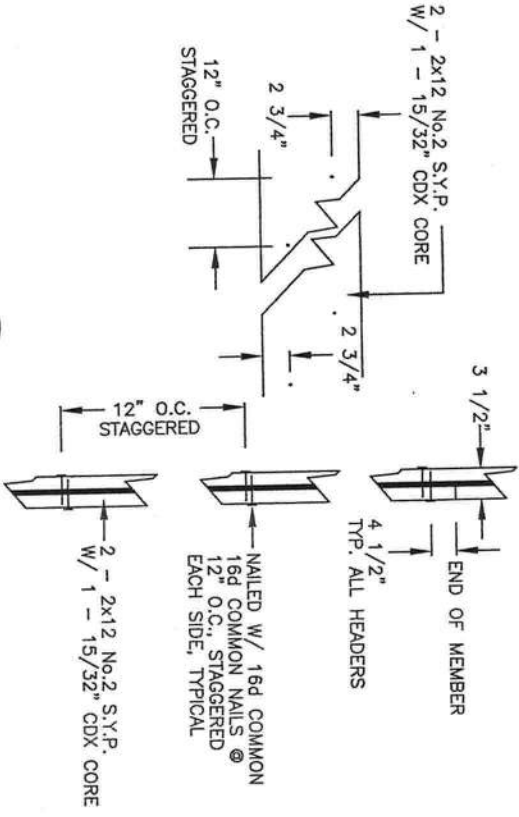
1 SHEATHING DETAIL - ELEVATION VIEW
S2.3.0 SCALE: N.T.S.



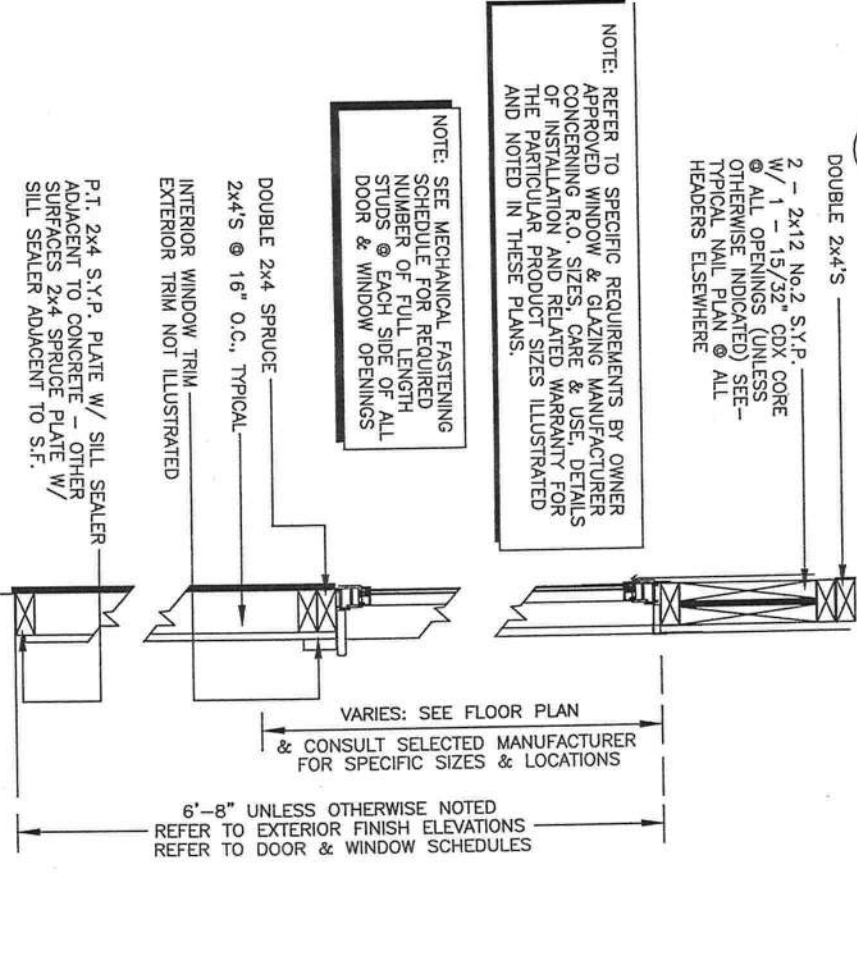
1A TOP PLATE DEVICES
S2.2.0 SCALE: N.T.S.



1B TYPICAL ENLARGED PLAN VIEW: EXTERIOR WALL
S2.2.0 SCALE: N.T.S.

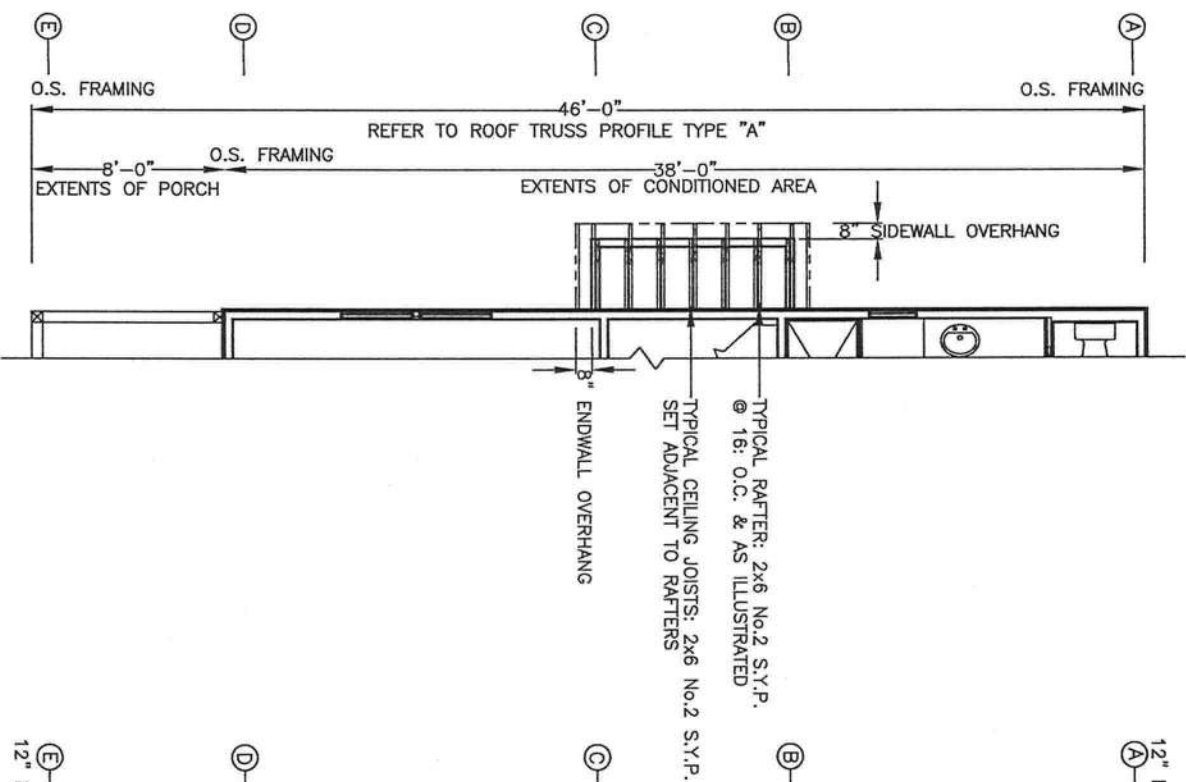


2 TYPICAL EXTERIOR WALL "HEADER"
S2.3.0 SCALE: N.T.S.



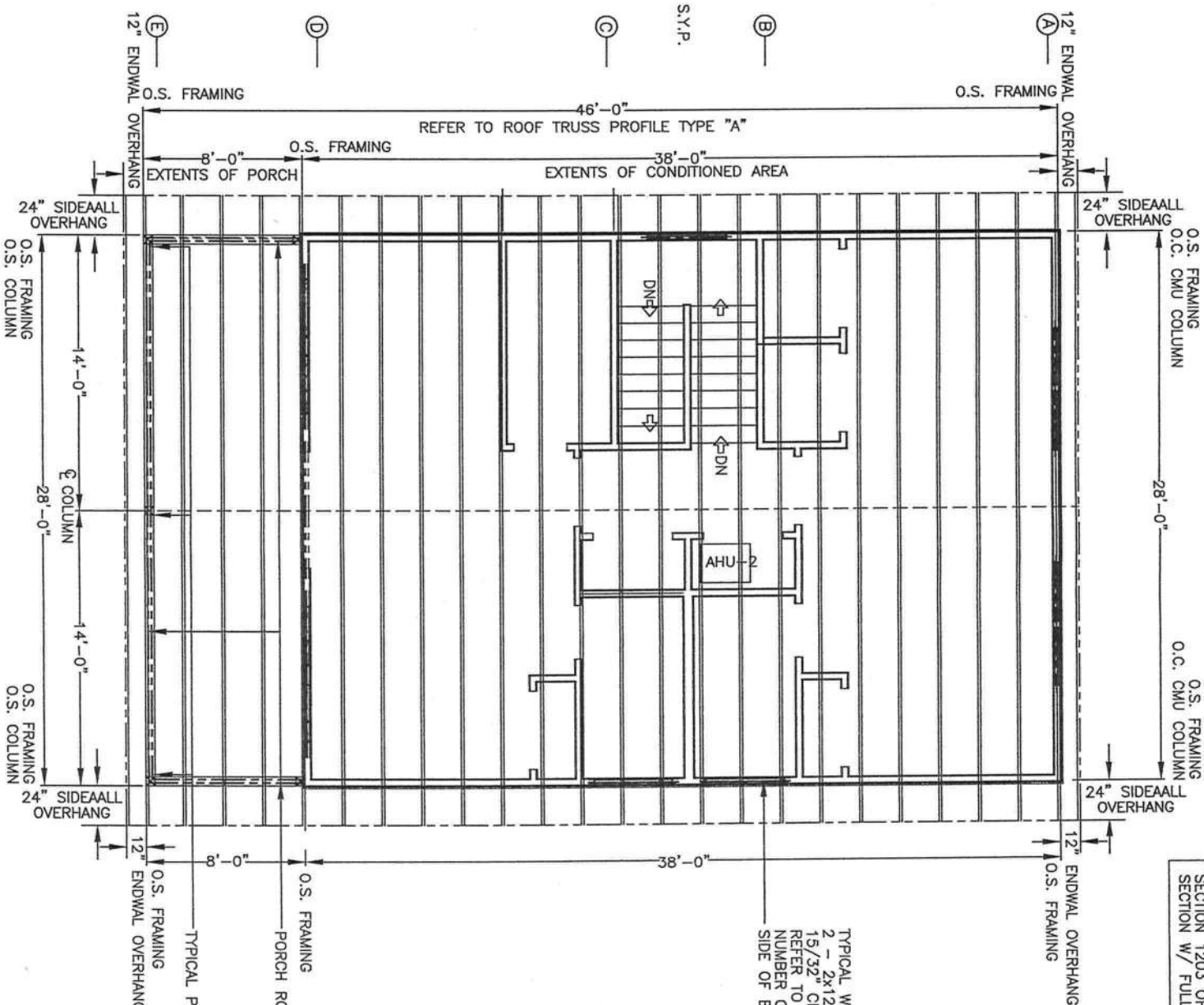
3 TYPICAL SECTION THROUGH EXTERIOR WALL OPENING
S2.1.0 SCALE: N.T.S. SEE PLAN VIEWS FOR APPLICABLE LOCATIONS

1 2



1 DIMENSIONED ROOF SYSTEM PLAN VIEW
STAIRWAY PROJECTED AREA
SCALE: 1/8" = 1'-0"

1 2 3 4 5 6



2 DIMENSIONED ROOF SYSTEM PLAN VIEW
MAIN DWELLING ROOF AREA
SCALE: 1/8" = 1'-0"

SCALE NOTE:
PLAN VIEW: 1/8"=1'-0"

ATTIC VENTILATION MAIN DWELLING ROOF AREA
THE CONTRACTOR SHALL PROVIDE A MINIMUM OF 10.20 NET SQUARE FEET OF FREE AREA OF COMBINED RIDGE, SOFFIT AND END WALL VENTS COMPLYING W/ SECTION 1203 OF THE F.B.C. SUCH VENTS SHALL BE AS DESCRIBED IN THIS SECTION W/ FULL PROTECTION FROM RAIN, INSECT OR VERMIN INTRUSION.

NOTE: CEILING SHALL BE VINYL SOFFIT MATERIAL INSTALLED PER SELECTED MANUFACTURER'S REQ. FOR THIS SPECIFIC APPLICATION, W/ OPTIONAL FIBERGLASS BATT INSULATION. ROOF SHEATHING SHALL MATCH NEW CONST. FINISH ROOF MATERIAL SHALL MATCH NEW CONST.

TYPICAL WINDOW/DOOR HEADER:
2 - 2x12 No.2 S.Y.P. W/ 2 - 15/32" CDX CORES
REFER TO SCHEDULES FOR REQUIRED NUMBER OF CONTINUOUS STUDS EA. SIDE OF EXTERIOR WALL OPENINGS

RAFTER: 2x6 No.2 S.Y.P. @ 16" O.C. & AS ILLUSTRATED & ATTACHED TO EACH 2-PLY BUILT-UP ROOF GIRDER W/ 1 - MODEL No. H4 BY SIMPSON STRONG-TIE OR EQUAL W/ A TOTAL OF 8 - 8d COMMON NAILS
1 - 2x4 No.2 S.Y.P. CEILING JOISTS ATTACHED ADJACENT TO RAFTERS AS ILLUSTRATED & ATTACHED TO BUILT-UP GIRDER W/ SAME DEVICE LISTED ABOVE

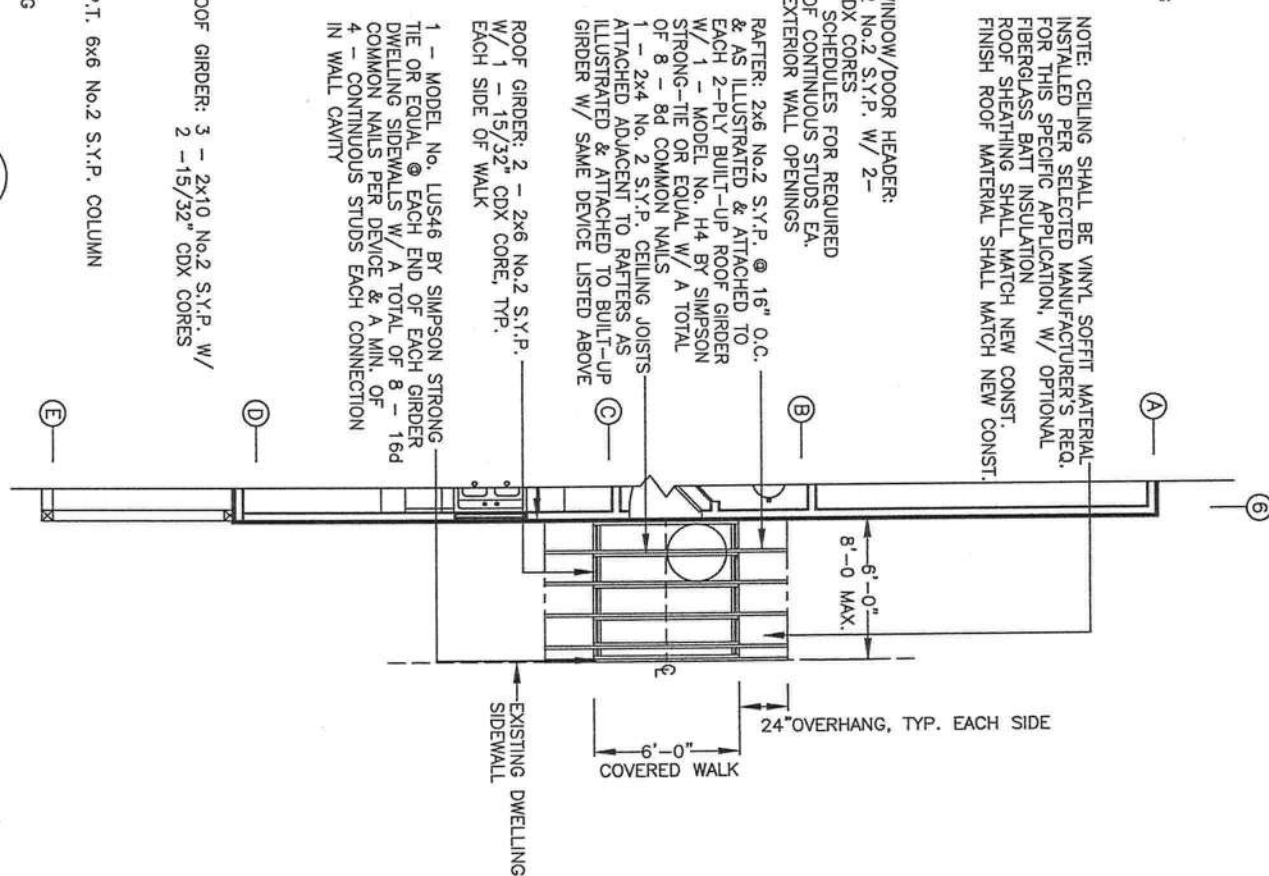
ROOF GIRDER: 2 - 2x6 No.2 S.Y.P. W/ 1 - 15/32" CDX CORE, TYP. EACH SIDE OF WALK

1 - MODEL No. LUS46 BY SIMPSON STRONG-TIE OR EQUAL @ EACH END OF EACH GIRDER DWELLING SIDEWALLS W/ A TOTAL OF 8 - 16d COMMON NAILS PER DEVICE & A MIN. OF 4 - CONTINUOUS STUDS EACH CONNECTION IN WALL CAVITY

PORCH ROOF GIRDER: 3 - 2x10 No.2 S.Y.P. W/ 2 - 15/32" CDX CORES

TYPICAL P.T. 6x6 No.2 S.Y.P. COLUMN

3 DIMENSIONED ROOF SYSTEM PLAN VIEW
CONNECTION WALK TO EXIST.
SCALE: 1/8" = 1'-0"



ATTIC ACCESS LOCATION(S): IF NOT SPECIFICALLY LOCATED OR NOTED ON PLAN VIEWS, ATTIC ACCESS SHALL BE @ DIRECTION OF OWNER

PROJECT No.
MORGAN-S3.0.0.DWG
SHEET No.
S3.0.0
DATE
02/26/16
DIMENSIONED ROOF SYSTEM PLAN VIEWS
MISC. NOTES, REFERENCES & INSTRUCTIONS
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Curtis E. Keen, PE #23836
Certification of Authorization #3761
DATE:

MORGAN RESIDENCE
COLUMBIA COUNTY, FLORIDA

KEEN ENGINEERING & SURVEYING, INC.
9263 CR 417
OAK, FLORIDA 32060
386-362-4787
ENG. LIC. EB 3761

1 - 2x4 No. 2 S.Y.P. CEILING JOISTS
 ATTACHED ADJACENT TO RAFTERS AS
 ILLUSTRATED & ATTACHED TO BUILT-UP
 GIRDER W/ 1 - MODEL No.H4

2x8 No.2 S.Y.P. RIDGE BEAM ATTACHED TO RAFTERS W/
 1 - MODEL No. RR BY SIMPSON STRONG-TIE OR EQUAL
 @ EACH SIDE OF EACH CONN. OF RAFTER TO RIDGE BEAM

RAFTER: 2x6 No.2 S.Y.P. @ 16" O.C.
 & AS ILLUSTRATED & ATTACHED TO
 EACH 2-PLY BUILT-UP ROOF GIRDER
 W/ 1 - MODEL No. H4 BY SIMPSON
 STRONG-TIE OR EQUAL W/ A TOTAL
 OF 8 - 8d COMMON NAILS

ROOF GIRDER: 2 - 2x6 No.2 S.Y.P.
 W/ 1 - 15/32" CDX CORE, TYP.
 EACH SIDE OF WALK

TYPICAL OVERHANG

NOTE: CEILING SHALL BE VINYL SOFFIT MATERIAL,
 INSTALLED PER SELECTED MANUFACTURER'S REQ.
 FOR THIS SPECIFIC APPLICATION, W/ OPTIONAL
 FIBERGLASS BATT INSULATION
 ROOF SHEATHING SHALL MATCH NEW CONST.
 FINISH ROOF MATERIAL SHALL MATCH NEW CONST.

SCHEDULE OF REQUIRED ROOF SYSTEM MECHANICAL FASTENERS & DEVICES
 TRUSS TYPE "A" AND/OR ALTERNATE "A1"
 LOCATION OF MAIN DWELLING (CONDITIONED & REAR PORCH AREAS) MANUFACTURED ROOF TRUSSES: SEE PLAN VIEW FOR LOCATIONS
 1 - MODEL NO. H4 BY SIMPSON STRONG-TIE OR EQUAL @ EACH BEARING POINT OF ROOF TRUSS TO TOP PLATES, HEADER OR
 BUILT-UP PORCH GIRDER ATTACHED W/ A TOTAL OF 8 - 8d COMMON NAILS PER DEVICE
 LOCATION OF MAIN DWELLING GABLE END (CONDITIONED & PORCH AREAS) MANUFACTURED ROOF TRUSSES: SEE PLAN VIEW
 2 - MODEL NO. H4 BY SIMPSON STRONG-TIE OR EQUAL @ EACH BEARING POINT OF ROOF TRUSS TO TOP PLATES OR BUILT-UP
 PORCH GIRDER ATTACHED W/ A TOTAL OF 8 - 8d COMMON NAILS PER DEVICE
 REAR PORCH GIRDERS: SEE PLAN VIEWS FOR LOCATIONS
 LOCATION OF 3-PLY BUILT-UP PORCH GIRDERS - SEE PLAN VIEW FOR LOCATIONS
 BUILT-UP PORCH GIRDER SHALL BE 3 - P.T. 2x10 No.2 S.Y.P. W/ 2 - 15/32" CDX CORES
 1 - MODEL NO. AC6 MAX. OR AC66 MAX BY SIMPSON STRONG-TIE OR EQUAL @ EACH BUILT-UP GIRDER TO P.T. COLUMN TOP
 CONNECTION ATTACHED W/ A TOTAL OF 20 - 16d COMMON NAILS (AC6 MAX) OR 28 = 16d COMMON NAILS (AC6 MAX)
 1 - MODEL NO. U210-3 BY SIMPSON STRONG-TIE OR EQUAL @ EACH BUILT-UP ROOF GIRDER TO DWELLING ENDWALL CONNECTION
 W/ A TOTAL OF 18 - 16d COMMON NAILS PER DEVICE.
 PORCH COLUMNS: SEE PLAN VIEWS FOR LOCATIONS
 LOCATION OF P.T. 6x6 PORCH COLUMNS - SEE PLAN VIEW FOR LOCATIONS
 REAR PORCH COLUMNS SHALL BE P.T. 6x6 No.2 S.Y.P. AS ILLUSTRATED
 SEE ABOVE GIRDERS FOR REQUIRED ATTACHMENT DEVICE(S)
 COVERED WALK ROOF
 BRIDGE BEAM SHALL BE 1 - 2x10 No.2 S.Y.P. - SEE ROOF SYSTEM PLAN VIEW FOR LOCATIONS
 REFER TO DETAILS FOR REQUIRED DEVICES
 RAFTERS SHALL BE 2x6 No.2 S.Y.P. - SEE ROOF SYSTEM PLAN VIEW FOR LOCATIONS
 REFER TO DETAILS FOR REQUIRED DEVICES
 CEILING JOISTS SHALL BE 2x4 No.2 S.Y.P. - SEE ROOF SYSTEM PLAN VIEW FOR LOCATIONS
 REFER TO DETAILS FOR REQUIRED ATTACHMENT
 COORDINATE W/ DETAILS AND OTHER REQUIREMENTS ELSEWHERE THESE PLANS

2 - 2x6 No.2 S.Y.P.
W/ 1 - 15/32" CDX CORE

2 3/4"

12" O.C. -
STAGGERED

3 1/2"

END OF MEMBER

4 1/2"

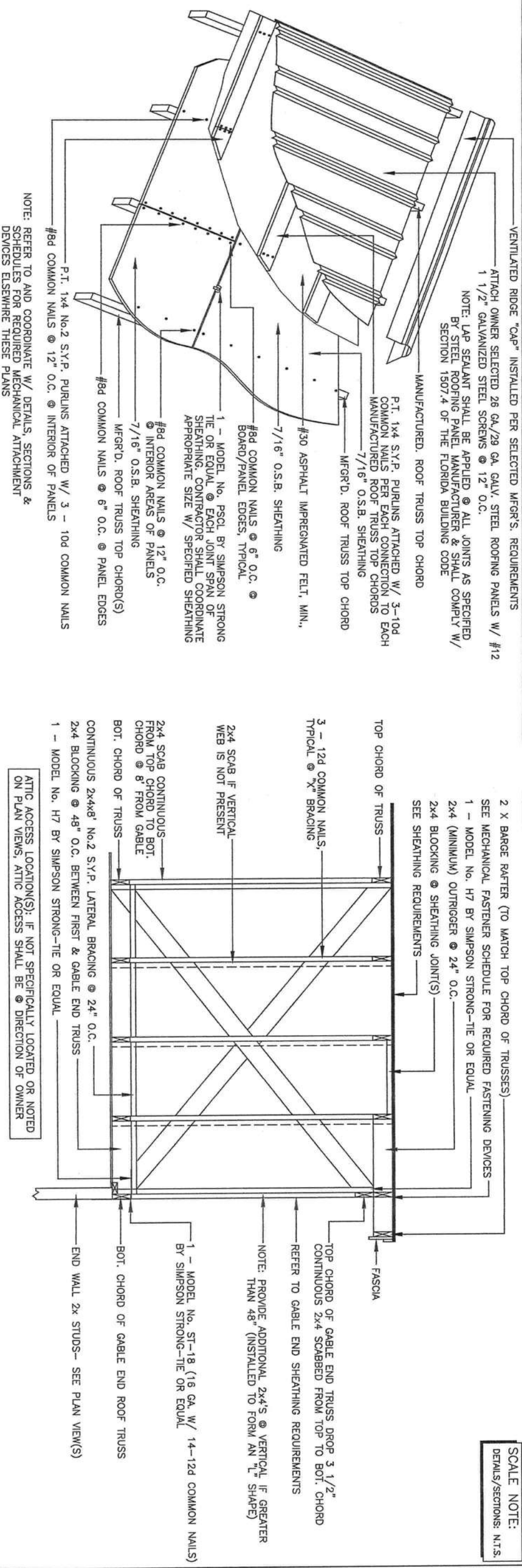
TYP. ALL HEADERS

12" O.C. STAGGERED

2 - 2x6 No.2 S.Y.P.
W/ 1 - 15/32" CDX CORE

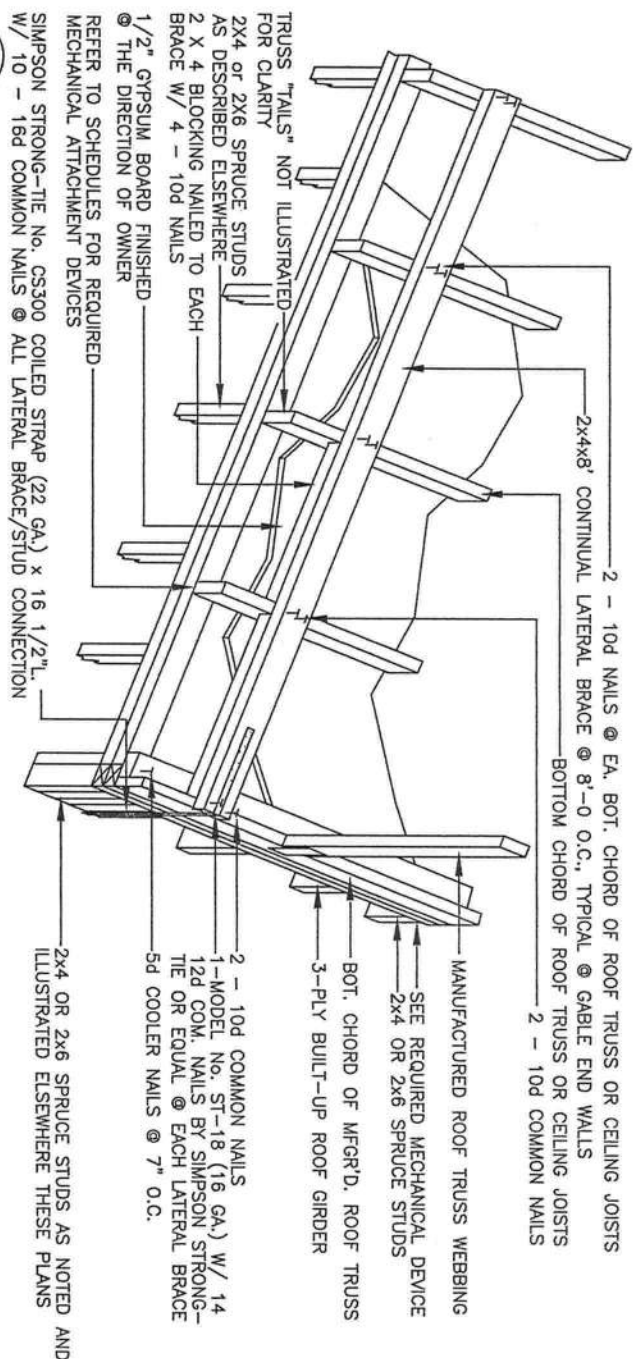
NAILED W/ 16d COMMON
16d COMMON NAILS @
12" O.C., STAGGERED
EACH SIDE, TYPICAL

SCALE NOTE:
DETAILS/SECTIONS: N.T.S.



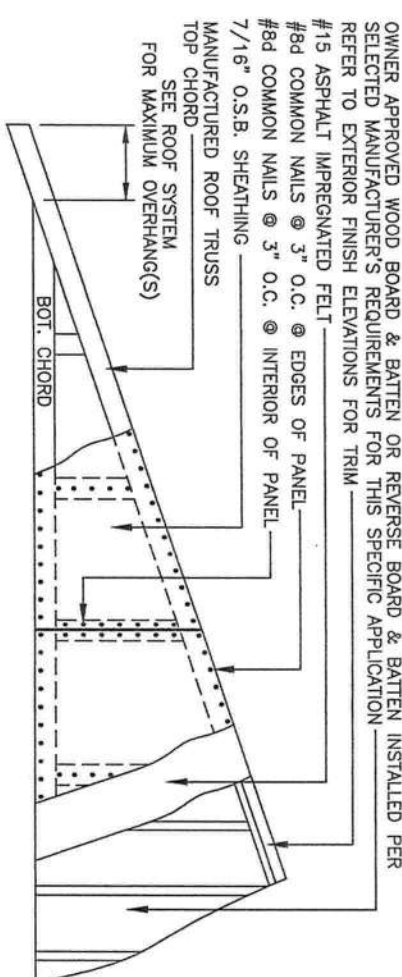
1 ROOFING PANELS & SHEATHING CONNECTIONS TO TRUSSES

S3.2.0 SCALE: N.T.S.



3 GABLE ENDWALL FRAMING REQUIREMENTS

SCALE: N.T.S.



4 GABLE ENDWALL SHEATHING REQUIREMENTS

SCALE: N.T.S.

PROJECT No.	02/26/16
MORGAN-S3.2.0.DWG	
SHEET No.	
S3.2.0	
DATE	
REFERENCED ROOF DETAILS & SECTIONS MISC. NOTES, REFERENCES & INSTRUCTIONS	
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