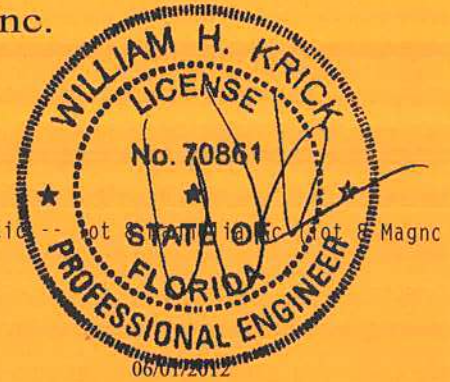


73

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1UML9114Z0101155343



Truss Fabricator: Anderson Truss Company
Job Identification: 12-110--Stanley Crawford Construc Kuhn / Garage with attic
Truss Count: 2
Model Code: Florida Building Code
Truss Criteria: FBC2010Res/TPI-2007(STD)
Engineering Software: Alpine Software, Version 10.03.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
- Address: the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

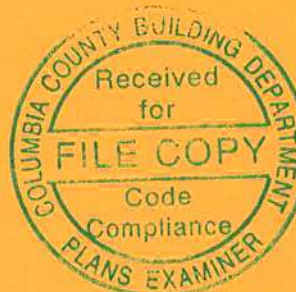
William H. Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: 12015EC1-GBLLETIN-GABRST10-

#	Ref	Description	Drawing#	Date
1	64196--ATGE		12153001	06/01/12
2	64197--AT		12153002	06/01/12

ALPINE



ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Page 1 of 1 Document ID: IUML9114Z0101155343

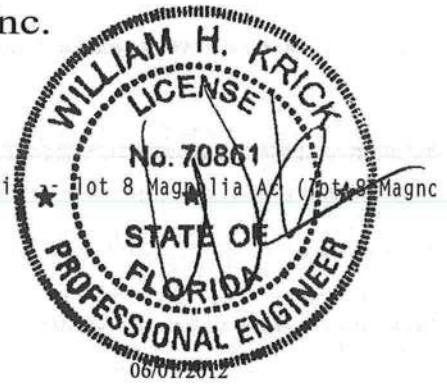
Truss Fabricator: Anderson Truss Company
Job Identification: 12-110--Stanley Crawford Construc Kuhn / Garage with att
Truss Count: 1
Model Code: Florida Building Code
Truss Criteria: FBC2010Res/TPI-2007(STD)
Engineering Software: Alpine Software, Version 10.03.
Structural Engineer of Record:
Address:
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Revised Trusses

#	Ref	Description	Drawing#	Date
1	64197-	AT	12153002	06/01/12



-Truss Design Engineer-
William H. Krick

1950 Marley Drive
Haines City, FL 33844

ALPINE

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, GCpi (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

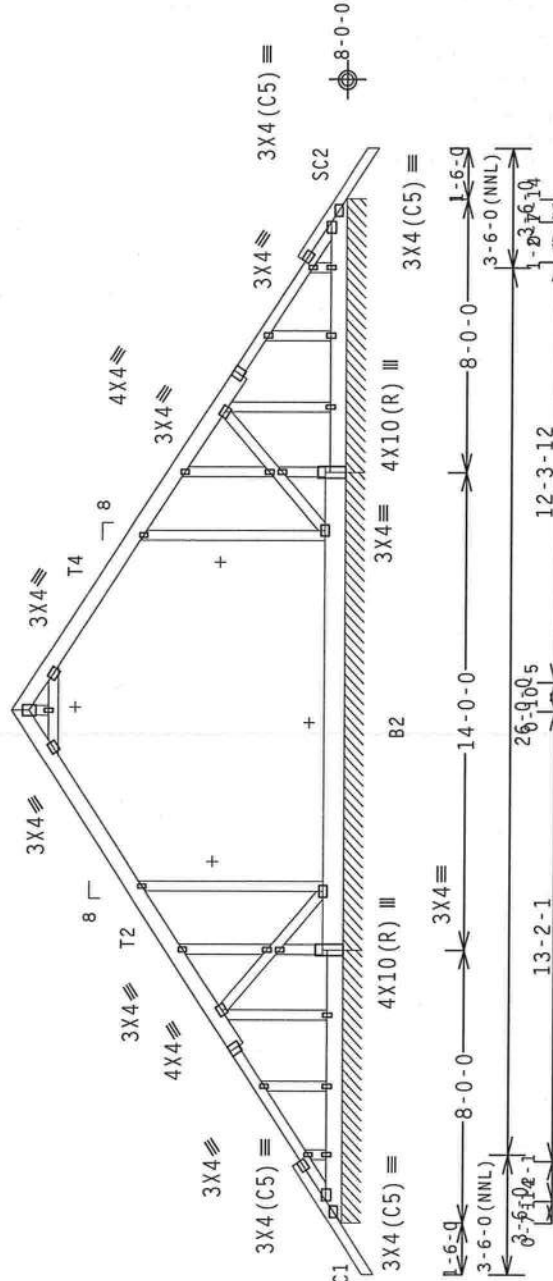
See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

+ MEMBER TO BE Laterally Braced For Out Of Plane Wind Loads. 4X5 (R) III
Bracing System To Be Designed And Furnished By
Others.



$R=135$ PLF $U=3$ PLF $W=840-0$

Design Crit: FBC2010Res/TPI-2007(STD)

BCZOTs/PI-200/
FT/RT=10%(0%)/0(0)

FL 1-141-1-1R1-

No. 70861

STATE OF



06/01/2012

ALPINE
NW Building Components Group Inc.

FL COA #0 278

06/01/2012

	:T2,	T4	2x6	SP	M-26:
Top chord	2x4	SP	#1		
Bot chord	2x6	SP	#2		
Webs	2x4	SP	#3		

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.17" due to live load and 0.36" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

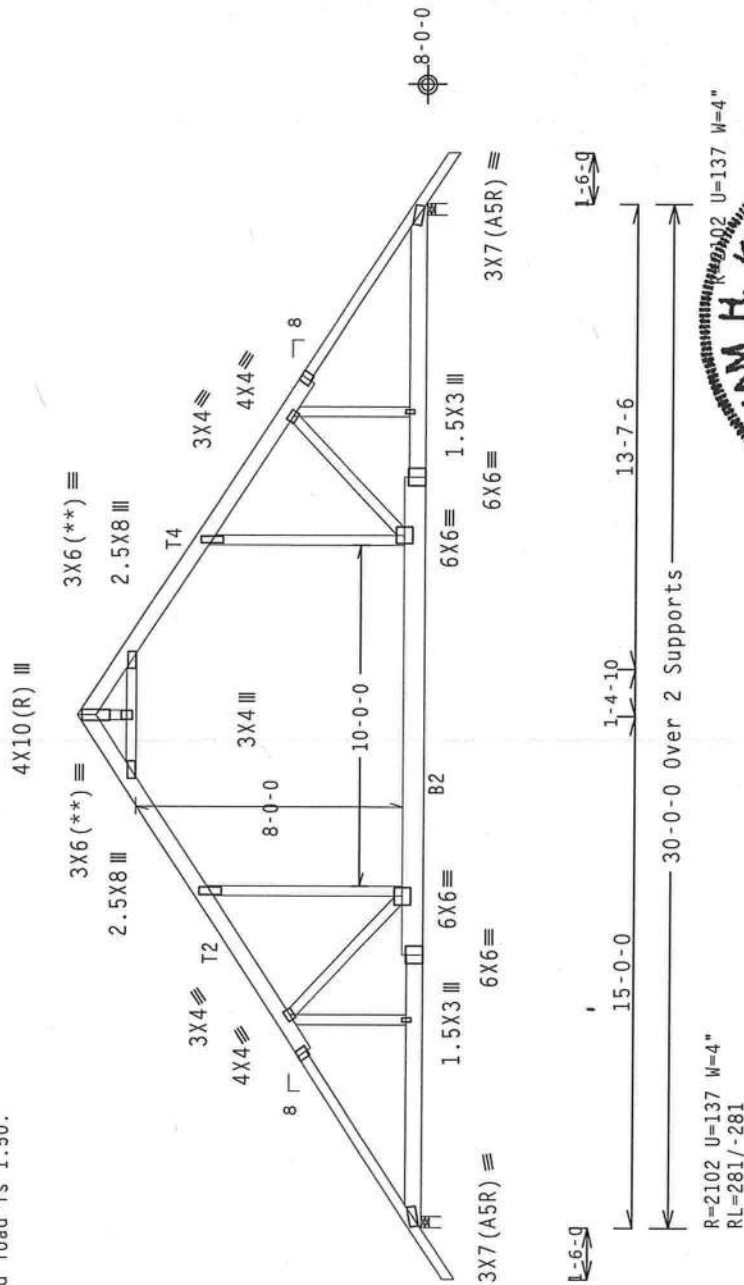
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, Exp C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 6Cpi (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 10-0-0 to 20-0-0.



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

Scale = .1875" / Ft.

IC LL	20.0	PSF	REF	R9114-	64197
TC DL	10.0	PSF	DATE	06/01/12	
BC DL	10.0	PSF	DRW	HCUSR9114	12153002
BC LL	0.0	PSF	HC-ENG	JB/WHK	
TOT.LD.	40.0	PSF	SEQN-	4864	REV
DUR.FAC.	1.25				
SPACING	24.0"		JREF-	IUML9114Z01	

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

[illegible]

ITW Building Components Group Inc. (INCBG) shall not be responsible for any deviation from the design shown on drawings or specifications. INCBG shall not be responsible for any failure in manufacturing or assembly of components or parts, or for any failure in packaging of units. Apply plates to each face of truss and position as shown above and on the details. Details, unless noted otherwise. Refer to Drawings 160A-2 For standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of Professional engineering certification by the building official. The engineer's name, title, address, phone number, fax number, e-mail address, and professional seal must appear on the drawing. This job's general notes pages: ITW-BGCS; www.itwbgcs.com; TPI; www.tpiinc.com; NTCN; www.ntccsa.org; and; www.ntccsa.org

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

ASCE 7-10: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

ASCE 7-10: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, $K_{zt} = 1.00$

□: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, $K_{zt} = 1.00$

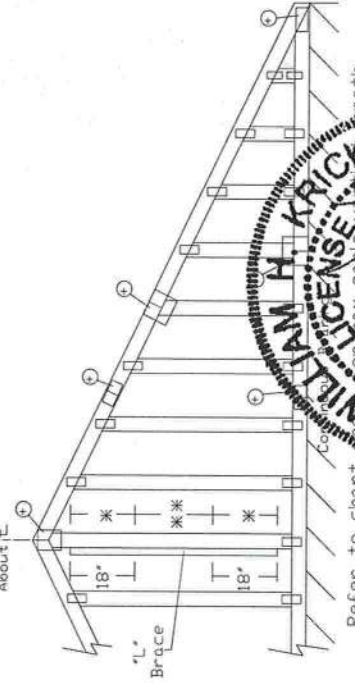
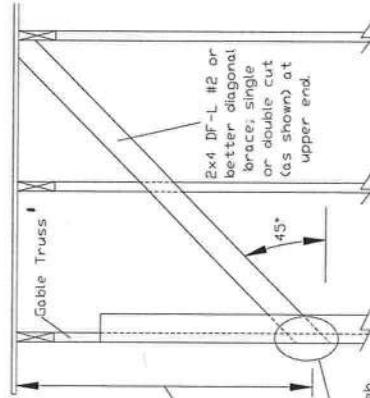
□r: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

[illegible]

Diagonal brace option:
vertical length may be
doubled when diagonal
brace is used. Connect
diagonal brace for 339
at each end. Max web
total length is 14'

Vertical length shown
in table above.

Connect diagonal at midpoint of vertical web



Refer to chart above for details.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE
INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCISI (Building Component Safety Information, by TPI and WCA) for practices for performing these functions. Installers shall provide temporary bracing for all trusses until they are properly attached to the structural sheathing and bracing. All webs shall have a properly attached rigid collar. Attached collars shall be made of webs shall have bracing installed per BCISI sections B3, B7 or B10, as applicable. Apply plate to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions.

Building Components Group Inc.

Earth City, MO 63045

Bracing Group Species and Grades:

Group A:

- Spruce-Pine-Fir**
 - #1 / #2 Standard
 - #3 Stud Standard
- Douglas Fir-Larch**
 - #3
 - Stud
 - Standard
- Southern Pine*****
 - #3
 - Stud
 - Standard

Group B:

- Hem-Fir**
 - #1 & Btr
 - #1
- Southern Pine*****
 - #1
 - #2

1x4 Braces shall be SRB (Stress-Rated Board).

*****For 1x4 Sd. Pine use only Industrial S5 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.**

1x4 Braces shall be SRB (Stress-Rated Braced)

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So: Pine lumber design values based on

the ALSC January, 2012 rule
Attach "L" braces with 10d (0.128"x3.0" min) bolts.

* For (1) "L" brace: space nails at 2' o.c.

in 18" end zones and 4" o.c. between zones.

*For (2) "L" braces: space nails at 3" o.c.

in 18" end zones and 6" o.c. between zones.

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	25X4

+ Refer to common truss design for peck, splice, and heel plates.

- + Refer to common truss design for peak, splice, and heel plates.

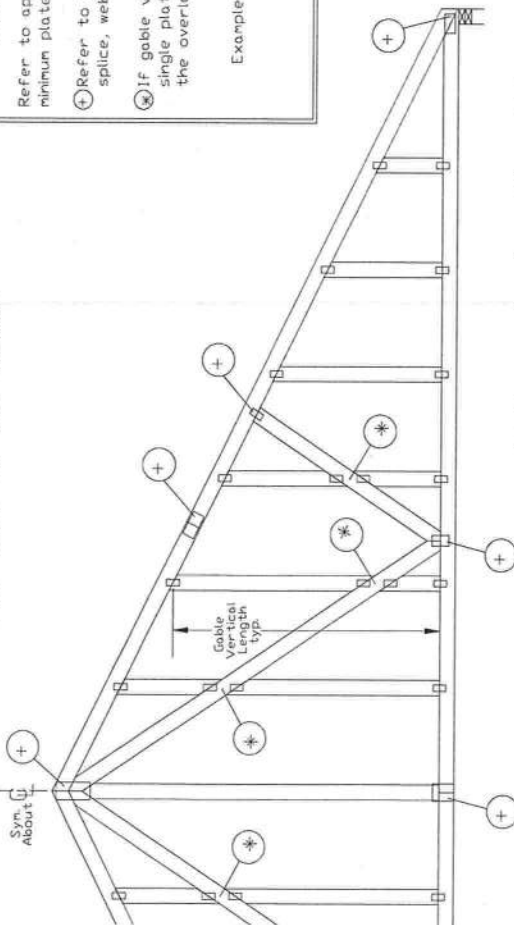
Refer to the Building Designer for conditions not addressed by this detail.

REF	ASCE7-10-GABI2015
DATE	2/14/12
DRWG	A12015FNC100212

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

06/01/2012

Gable Detail For Let-in Verticals

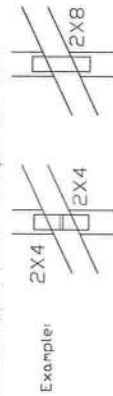


Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊙ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:

- 10d Common (0.148"x 3.1/2" min) Nails at 4" o.c. plus
- (4) nails in the top and bottom chords.

Toenailed Nails:

- 10d Common (0.148"x 3.1/2" min) Toenails at 4" o.c. plus
- (4) toenails in the top and bottom chords.

This detail to be used with the appropriate ITW gable detail for ASCE wind load.

ASCE 7-98 Gable Detail Drawings

A13015980109, A12015980109, A11015980109, A10015980109,
A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 Gable Detail Drawings

A13015020109, A12015020109, A11015020109, A10015020109,
A13030020109, A12030020109, A11030020109, A10030020109

ASCE 7-05 Gable Detail Drawings

A13015050109, A12015050109, A11015050109, A10015050109,
A13030050109, A12030050109, A11030050109, A10030050109

ASCE 7-10 Gable Detail Drawings

A11515ENC100212, A12015ENC100212, A14015ENC100212,
A18015ENC100212, A20015ENC100212, A20015ENC100212,
A11530ENC100212, A12030ENC100212, A14030ENC100212,
A18030ENC100212, A20030ENC100212, A20030ENC100212

See appropriate ITW gable detail for maximum gable truss height.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety) Information, by TPI and VITA for best practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Trusses shall be braced in accordance with the attached structural sheathing and bottom chord shall have a properly attached diagonal brace. Trusses shall be braced in accordance with each face of truss and position as shown above and on the Joint Details. Apply plate. Refer to drawings 160A-Z for standard plate positions.

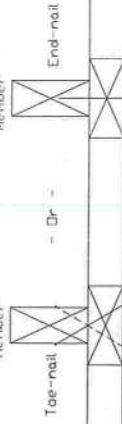
ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing. Failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation, bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this framing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. ITWBCS www.itwbcsg.com, TPI www.tpinet.org, VITA www.vita-usa.com, IBC www.iccsafe.org



Building Components Group Inc.

Earth City, MO 63045

T Reinforcement Attachment Detail
T Reinforcing Member



To convert from "L" to "T" reinforcing members, multiply "L" increase by length (based on appropriate ITW gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. SP #3

"T" Reinforcing Member Size = 2x4

"T" Brace Increase (From Above) = 30% = 1.30

(1) 2x4 "L" Brace Length = 8' 7"

Maximum "T" Reinforced Gable Vertical Length

1.30 x 8' 7" = 11' 2"

REF LET-IN VERT
DATE 2/16/12
DRWG GBLLETIN0212

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"



ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

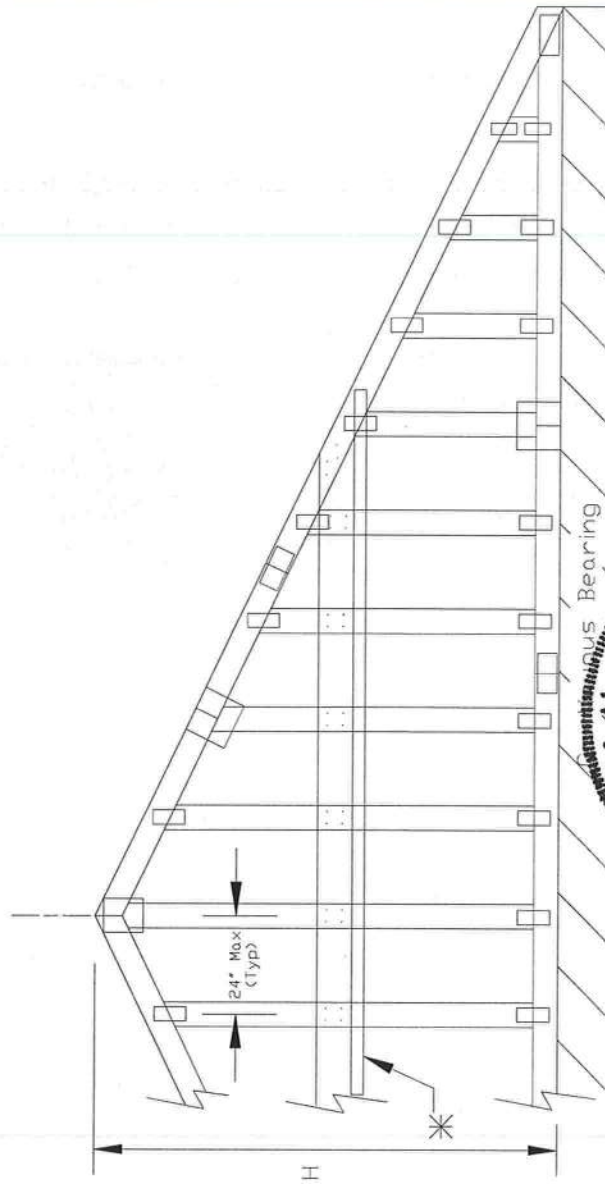
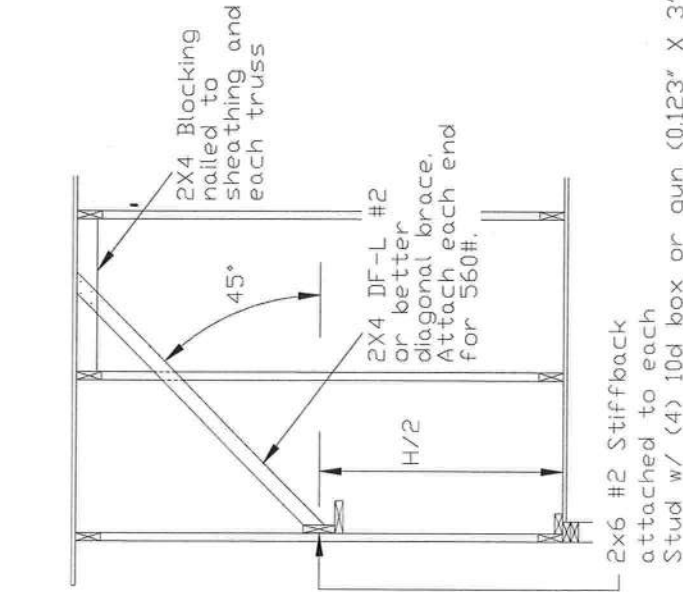
Nails: 10d box or gun (0.128" x 3", min) nails.

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback
to roof diaphragm every 6'0" (see detail below or
refer to DRWG A12030ENC10).

H Greater than 7'6" to 12'0" max: provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC10).

*Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.



WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
 IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE
 INSTALLER

[illegible]

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1, Sec.2. For more information see: www.bobst.com; this job's general notes page and these web sites: www.VBCC.com; www.itwbuildingcomponents.com; www.ansi/tpi.com

Earth City, MO 63045

06/01/2012



The seal is circular with a double border. The outer border contains the text "PROFESSIONAL ENGINEER" at the top and "STATE OF FLORIDA" at the bottom. Inside the seal, the text "No. 70861" is on the left and "06/01/2012" is on the right. In the center, there is a five-pointed star. A diagonal line runs from the top left to the bottom right, passing through the star. At the bottom, there is a small rectangular box containing the text "to one" and "of this".

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212

Evaluation Report "5V Crimp" & "Millennium-V" Metal Roof Assembly

Manufacturer:
Millennium Metals, Inc.
10200 Eastport Road
Jacksonville, FL 32218
(877) 358-7663

for

Florida Product Approval
FL 5211.1 R3

Florida Building Code 2010
Per Rule 9N-3

Method: 1 - D

Category: Roofing

Sub - Category: Metal Roofing

Product:	"5V Crimp" & "Millennium-V"
Material:	Steel
Panel Thickness:	26 gauge
Panel Width:	24"
Support:	Wood Deck

Prepared by:

James L. Buckner, P.E., S.E.C.B.
Florida Professional Engineer # 31242
Florida Evaluation ANE ID: 1916
Project Manager: Diana Galloway
Report No. 12-117-5V-S6W-ER
Date: 2 / 6 / 12

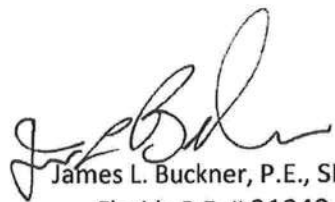
Contents:

Evaluation Report

Pages 1 - 8

CBUCK, Inc.

1399 N. Killian Drive, Suite 4, West Palm Beach, Florida 33403
Phone: (561)491-9927 Fax: (561)491-9928 Website: www.cbuckinc.net


James L. Buckner, P.E., SECB
Florida P.E. # 31242
2/17/12

Manufacturer: Millennium Metals

Product Name: "5V Crimp" & "Millennium-V"

Product Category: Roofing

Product Sub-Category: Metal Roofing

Compliance Method: State Product Approval Rule 9N-3.005 (1) (d)

Product/System Description: "5V Crimp" & "Millennium-V"
26 gauge Steel roof panel mechanically attached to Plywood Deck with screws.

Product Assembly as Evaluated: Refer to Page 4 of this report for product assembly components/materials & standards:

1. Roof Panel
2. Fasteners
3. Underlayment
4. Insulation (Optional)

Support: **Type:**
Wood Deck
(Design of support and its attachment to support framing is outside the scope of this evaluation.)

- Description:**
- 15/32 (min.) or 19/32" (min.) or greater plywood,
 - or Wood plank (min. specific gravity of 0.42)

Slope: Minimum slope shall be in accordance with manufacturer's recommendations, FBC Section 1507.4.2 and applicable code sections.

Performance: Wind Uplift Resistance:
• Design Uplift Pressure:
(Refer to "Table A" attachment details herein)

Refer to Table A

- Performance Standards:** The product described herein has demonstrated compliance with:
- UL580-06 – *Test for Uplift Resistance of Roof Assemblies*
 - UL 1897-04 – *Uplift test for roof covering systems*
 - TAS 125-03 – *Standard Requirements for Metal Roofing Systems*
- Standards Equivalency:** The UL 580-94 & UL 1897-98 standard version used to test the evaluated product assembly is equivalent with the prescribed standards in UL 580-06 & UL 1897-04 adopted by the Florida Building Code 2010.
- Code Compliance:** The product described herein has demonstrated compliance with Florida Building Code 2010, Section 1504.3.2.
- Evaluation Report Scope:** This product evaluation is limited to compliance with the structural requirements of the Florida Building Code, as related to the scope section to Florida Product Approval Rule 9N-3.001.
- Limitations and Conditions of Use:**
- Scope of "Limitations and Conditions of Use" for this evaluation:
This evaluation report for "Optional Statewide Approval" contains technical documentation, specifications and installation method(s) which include "Limitations and Conditions of Use" throughout the report in accordance with Rule 9N-3.005. Per Rule 9N-3.004, the Florida Building Commission is the authority to approve products under "Optional Statewide Approval".
 - Option for application outside "Limitations and Conditions of Use"
Rule 9N-3.005(1)(e) allows engineering analysis for "project specific approval by the local authorities having jurisdiction in accordance with the alternate methods and materials authorized in the Code". Any modification of the product as evaluated in this report and approved by the Florida Building Commission is outside the scope of this evaluation and will be the responsibility of others.
 - Design of support system is outside the scope of this report.
 - Fire Classification is outside the scope of Rule 9N-3, and is therefore not included in this evaluation.
 - This evaluation report does not evaluate the use of this product for use in the High Velocity Hurricane Zone code section. (Dade & Broward Counties)
- Quality Assurance:** The manufacturer has demonstrated compliance of roof panel products in accordance with the Florida Building Code and Rule 9N-3.0005 (3) for manufacturing under a quality assurance program audited by an approved quality assurance entity through **Keystone Certifications, Inc.** (FBC Organization #: QUA 1824).

**Components/Materials
(by Manufacturer):**

Roof Panel: "5V Crimp" & "Millennium-V"
Material: Steel
Thickness: 26 gauge (min.)
Panel Width: 24" (max.) Coverage
rib Height: 7/16"
Yield Strength: 40 ksi min.
Corrosion Resistance: In compliance with FBC Section 1507.4.3:

- ASTM A792 coated, or
- ASTM A653 G90 galvanized steel

Fastener:

Type: Hex-Head Wood Screw with WSW
Size : #9 x 1"
Corrosion Resistance: Per FBC Section 1506.6 and 1507.4.4
Standard: Per ANSI/ASME B18.6.4

Underlayment:

Per roofing manufacturer's guidelines in compliance with FBC Section 1507.4.5

**Components/Materials
(by Others):**

Insulation (Optional):

Type: Rigid Insulation Board
Thickness: 3" (max.)
Properties:
Density: 2.25 pcf (lbs/ft³) min.
Or Compressive Strength: 20 psi min.

Insulation shall comply with FBC Section 1508. When insulation is incorporated, fastener length shall conform to penetrate thru bottom of support a minimum of 3/16".

Installation:**Installation Method:**

(Refer to drawings on Pages 6-8 of this report.)

- Fastener spacing: **Refer to Table "A" Below**
(along the length of the panel)
- Row Spacing: **Refer to Table "A" Below**
(along the row, across the panel profile)
- Rib Interlock: Lapped
- Minimum fastener penetration thru bottom of support, 3/16".
- For panel construction at the end of panels, refer to manufacturer's instructions and any site specific design.

TABLE "A"		
	METHOD 1:	METHOD 2:
Design Pressure:	- 112.5 PSF	- 113.75 PSF
Deck Thickness:	19/32"	15/32"
Row Spacing:	12"	12"
Fastener Spacing:	12"	6"
Fastener Location:	Corrugation Peaks	Corrugation Peaks & between ribs
Refer to Drawing #:	A-1 & A-2	B-1 & B-2

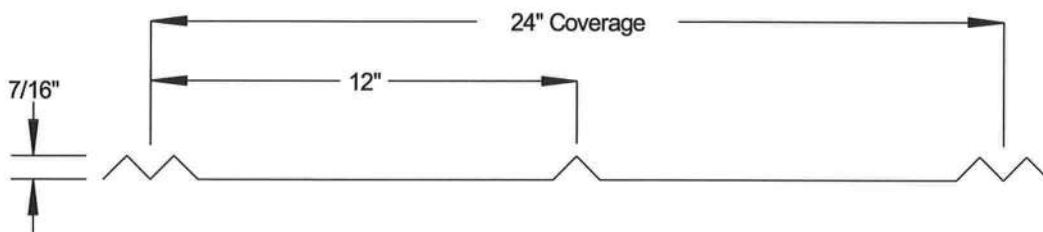
Install the "5V Crimp" roof panel assembly in compliance with the installation method listed in this report and applicable code sections of FBC 2010. The installation method described herein is in accordance with the scope of this evaluation report. Refer to manufacturer's installation instructions as a supplemental guide for attachment.

Referenced Data:

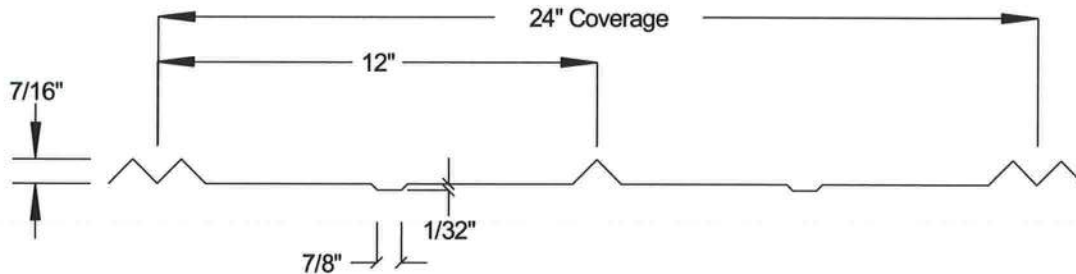
1. UL580-94 & UL 1897-98 Uplift Test
By Hurricane Test Laboratory, LLC (FBC Organization #TST ID: 1527) Report #:
0232-0801-00, Report Date: 9/8/00, Specimens #2-4
0232-0910-05, Report Date: 11/18/05, Specimen #1
2. Quality Assurance
By Keystone Certifications, Inc. (QUA ID: 1824)
Millennium Metals Licensee # 423
3. Equivalency of Test Standard Certification
By James L. Buckner, P.E. @ CBUCK Engineering
(FBC Organization # ANE 1916)
4. Certification of Independence
By James L. Buckner, P.E. @ CBUCK Engineering
(FBC Organization # ANE 1916)

Installation Method Millennium Metals "5V Crimp" & "Millennium-V" Roof Panel Attached to Wood Deck

Profile Drawings



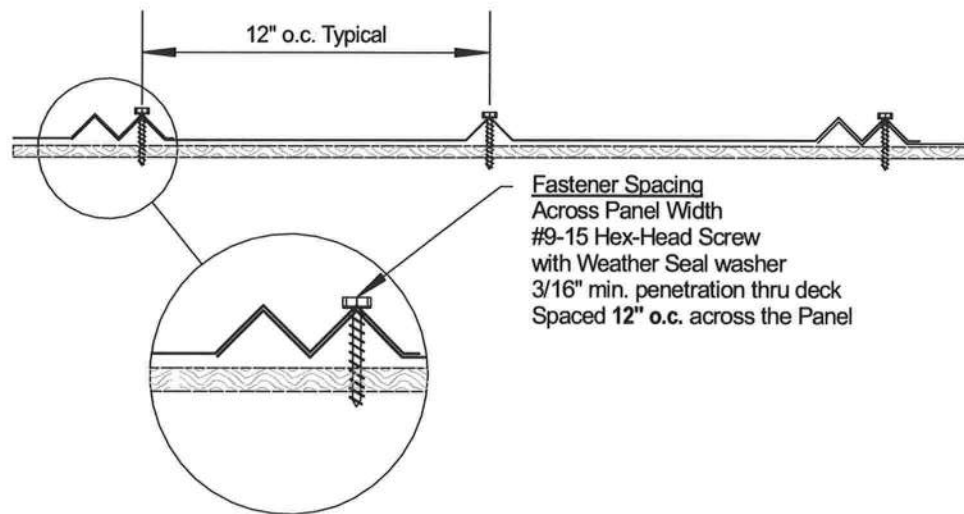
"5V-Crimp" Typical Panel Profile View



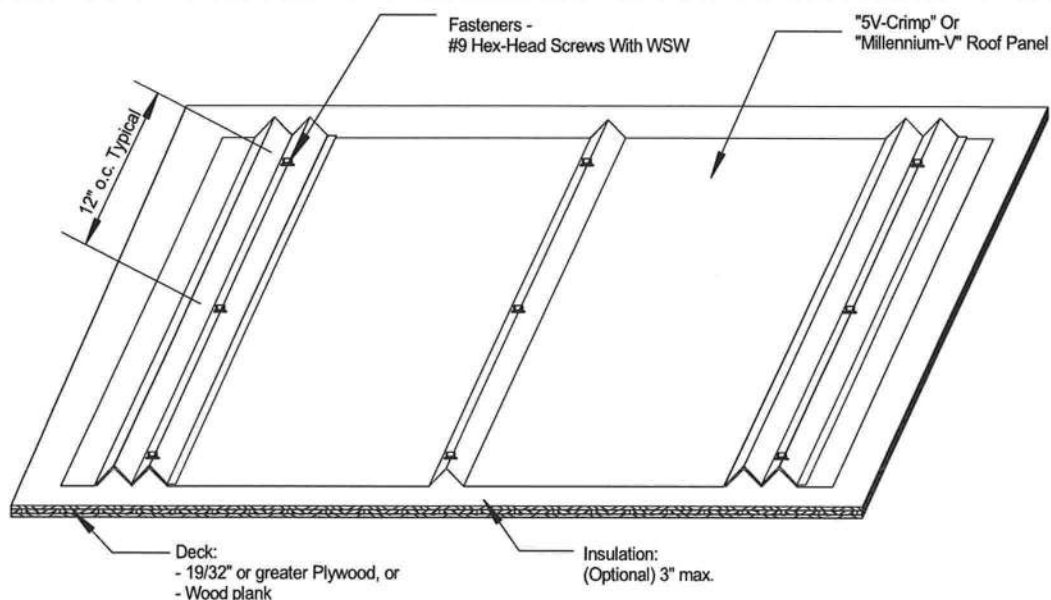
"Millennium-V" Typical Panel Profile View

TABLE "A"		
	METHOD 1:	METHOD 2:
Design Pressure:	- 112.5 PSF	- 113.75 PSF
Deck Thickness:	19/32"	15/32"
Row Spacing:	12"	12"
Fastener Spacing:	12"	6"
Fastener Location:	Corrugation Peaks	Corrugation Peaks & between ribs
Refer to Drawing #:	A-1 & A-2	B-1 & B-2

Installation Method Millennium Metals "5V Crimp" & "Millennium-V" Roof Panel Attached to Wood Deck

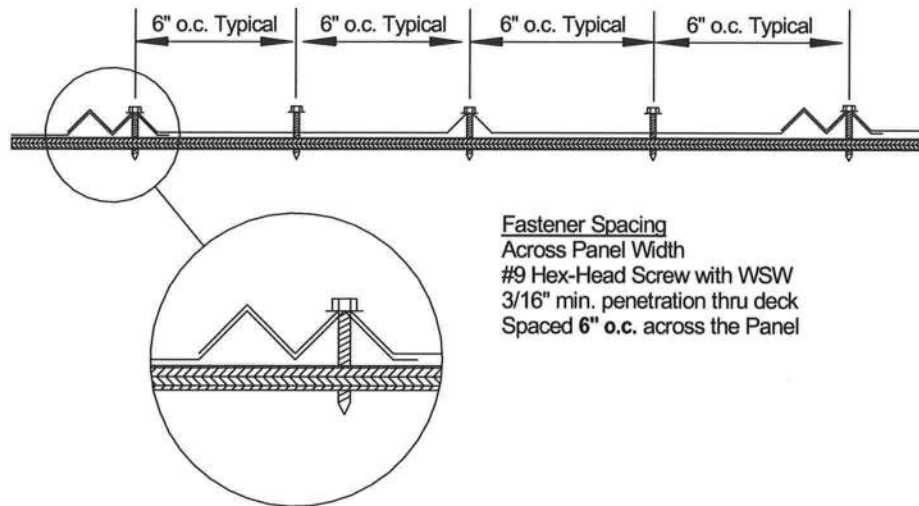


Drawing A-1
Typical Assembly Profile View (METHOD 1)

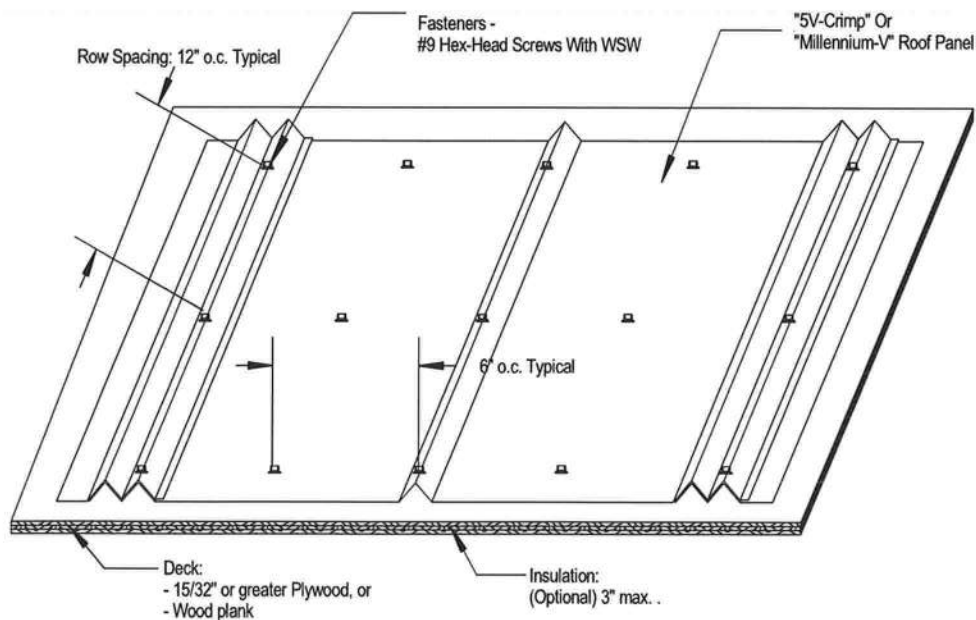


Drawing A-2
Typical Assembly Isometric View (METHOD 1)

Installation Method Millennium Metals "5V Crimp" & "Millennium-V" Roof Panel Attached to Wood Deck



Drawing B-1
Typical Assembly Profile View (METHOD 2)



Drawing B-2
Typical Assembly Isometric View (METHOD 2)

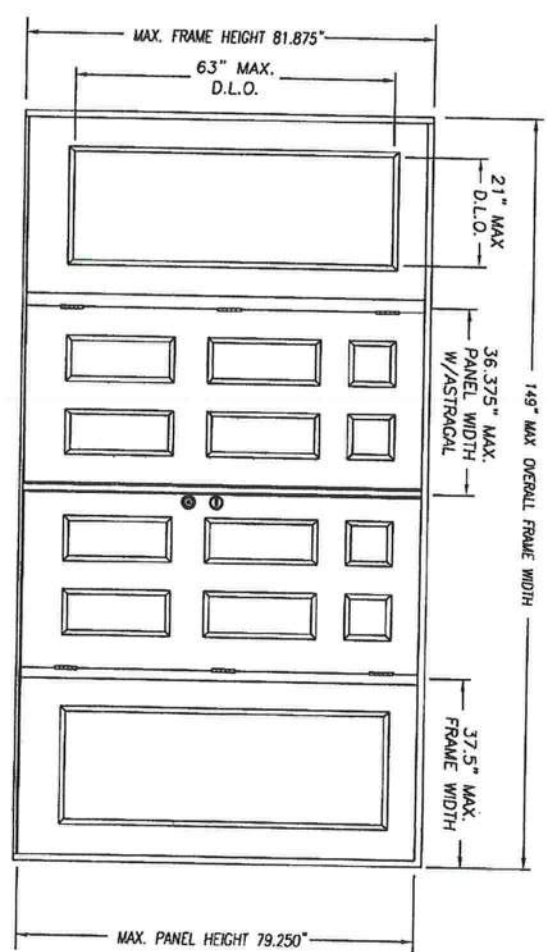
6' doors



SIDE-HINGED WOOD-EDGE STEEL DOOR UNIT
6'-8" DOUBLE DOOR WITH / WITHOUT SIDELITES

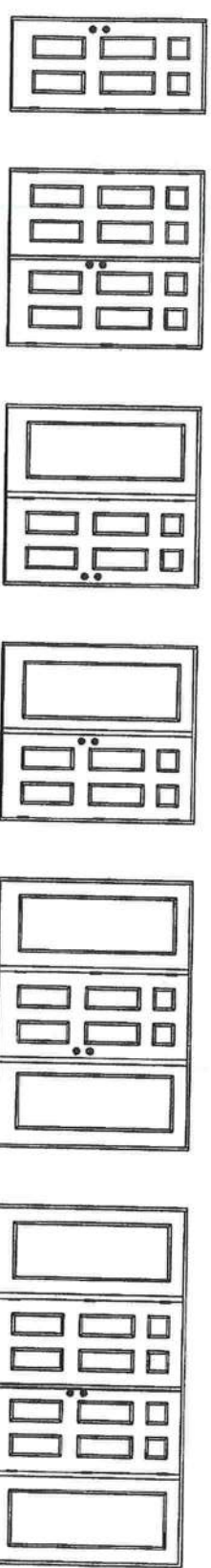
GENERAL NOTES

1. EVALUATED FOR USE IN LOCATIONS ADHERING TO THE FLOOR BUILDING CODE AND WHERE PRESSURE REQUIREMENTS AS DETERMINED BY ASCE 7, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, DOES NOT EXCEED THE DESIGN PRESSURES LISTED
2. HURRICANE PROTECTIVE SYSTEM (SHUTTERS) IS NOT REQUIRED ON OPAQUE PANELS, BUT IS REQUIRED ON GLAZED SIDELITES
3. POLYURETHANE CORE FLAME SPREAD INDEX OF 50 AND SMOKE DEVELOPED INDEX OF 80 PER ASTM E84
4. PLASTICS TESTING OF LITE FRAME MATERIAL:
TEST DESCRIPTION DESIGNATION RESULT
SELF IGNITION TEMP ASTM D1929 680 °F > 650 °F
RATE OF BURNING ASTM D635 1.10 IN/MIN
SMOKE DENSITY ASTM D2843 69.6%
TENSILE STRENGTH* ASTM D638 -7.48% DIFF
* COMPARATIVE TENSILE STRENGTH AFTER WEATHERING 4500 HOURS XENON ARC METHOD 1



DOUBLE INSULATING UNIT W/SIDELITES

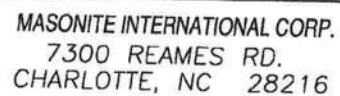
Attention to M/M
Certification No. NT006110
Reviewed By: [Signature]
Date Reviewed: 8/10/05

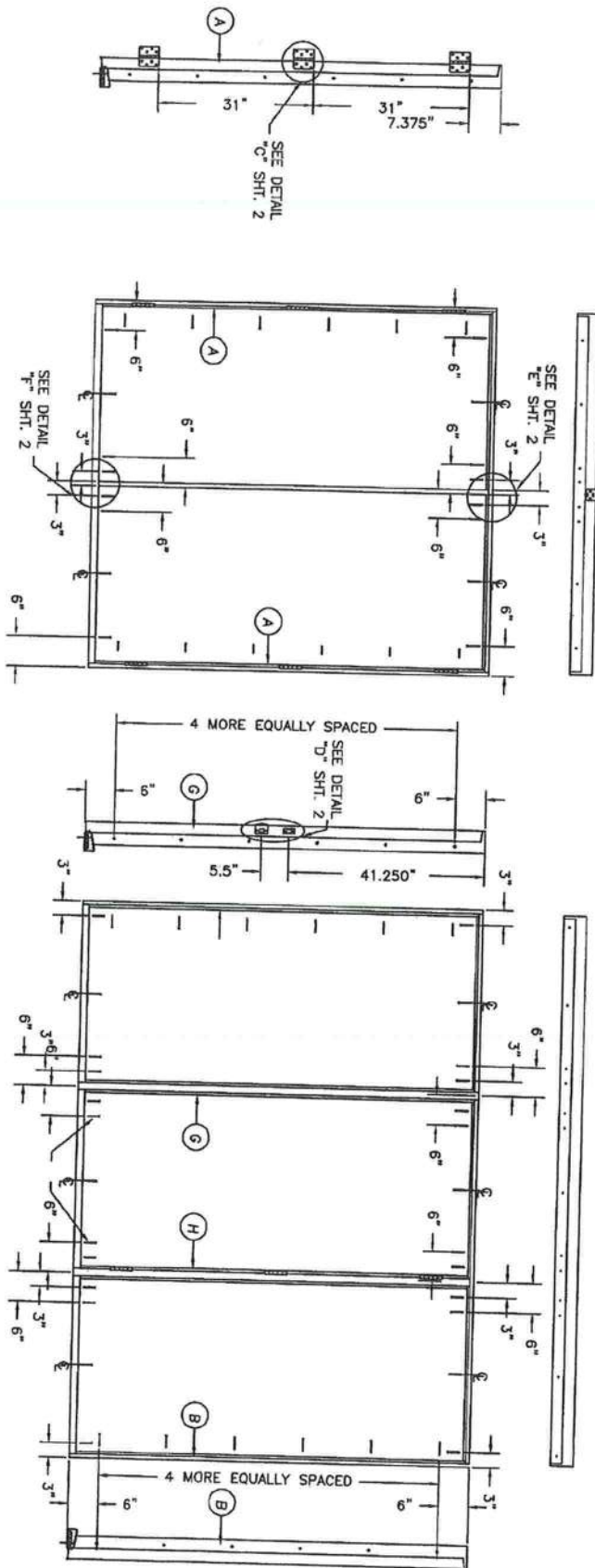


SHEET #	DESCRIPTION
1	TYPICAL ELEVATIONS & GENERAL NOTES
2	ANCHORING LOCATIONS & DETAILS
3	ANCHORING LOCATIONS & DETAILS

DESIGN PRESSURE RATING		WHERE WATER INFILTRATION PERFORMANCE IS REQUIRED TO BE 15% OF DESIGN PRESSURE	
CONFIG	MAX WIDTH	INSULATING	OUTSULATING
X	37.5"	+76.0 / -76.0	+76.0 / -76.0
XX	74"	+55.0 / -55.0	+55.0 / -55.0
XX or XO	75"	+55.0 / -55.0	+55.0 / -55.0
OXXO	112.5"	+55.0 / -55.0	+55.0 / -55.0
OXXO	149"	+55.0 / -55.0	+55.0 / -55.0

DATE: 7/11/05	SCALE: N.T.S.	DMG. BY: SMS	CHK. BY:	DRAWING NO.:	DMG-44-FL0128-05
SHEET 1 of 3					
PRODUCT: "EXTERIOR DOOR PRODUCT" DOUBLE 6'8" OPAQUE WOOD-EDGE STEEL DOOR		PART OR ASSEMBLY: TYPICAL ELEVATIONS & GENERAL NOTES		MASONITE INTERNATIONAL CORP. 7300 REAMES RD. CHARLOTTE, NC 28216	
NO.	DATE	BY	REVISIONS		

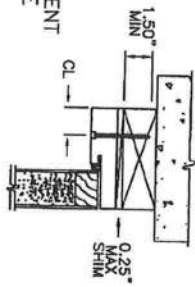




ATTACHMENT DETAIL

1. ANCHOR ANALYSIS FOR LOADING CONDITIONS PREPARED, SIGNED AND SEALED BY HAROLD E. RUPP, PE (FLORIDA #15935) WITH THE LOWEST (LEAST) FASTENER RATING FROM THE DIFFERENT FASTENERS BEING CONSIDERED FOR USE. JAMB, HEAD, AND THRESHOLD FASTENERS ANALYZED FOR THIS UNIT INCLUDE #10 WOOD SCREWS OR 3/16" TAPCONS. A PHYSICAL SHIM MUST BE PLACED IN SHIM SPACE AT EACH ANCHOR LOCATION.
2. THE WOOD SCREW SINGLE SHEAR DESIGN VALUES COME FROM ANS/A&P&A NDA FOR SOUTHERN PINE LUMBER AND ACHIEVEMENT OF 1-1/2" MINIMUM EMBEDMENT. THE TAPCON MUST ACHIEVE MINIMUM EMBEDMENT OF 1-1/4".
3. WOOD BUCKS BY OTHERS MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO STRUCTURE.
4. MINIMUM DESIGN VALUE STRENGTH OF ANCHORS 171 LBS.

TYPICAL ANCHOR INSTALLATION



HARDWARE SCHEDULE

1.	KWIKSET OR SCHLEGEL ANSI/BHMA GRADE 3 OR BETTER CYLINDRICAL AND DEADLOCK HARDWARE TO BE INSTALLED AT 5-1/2" CENTERLINE.
2.	4" X 4" FULL MORTISE BUTT HINGES

Certification No.: NT000110
 Prepared By: [Signature]
 Date Released: 8/10/05

Approved by: [Signature]

PRODUCT: "EXTERIOR DOOR PRODUCT" 6'-8" WOOD-EDGE STEEL OPAQUE DOUBLE DOOR UNIT		MASONITE INTERNATIONAL CORP. 7300 REAMES RD. CHARLOTTE, NC 28216	
PART OR ASSEMBLY: ANCHORING LOCATIONS & DETAILS			
DATE: 7/11/05	SCALE: N.T.S.	NO.	DATE
DWG. BY: SMS	CHK. BY:		
DWG-M4-FL0128-05			
SHEET 3 OF 3		REVISIONS	