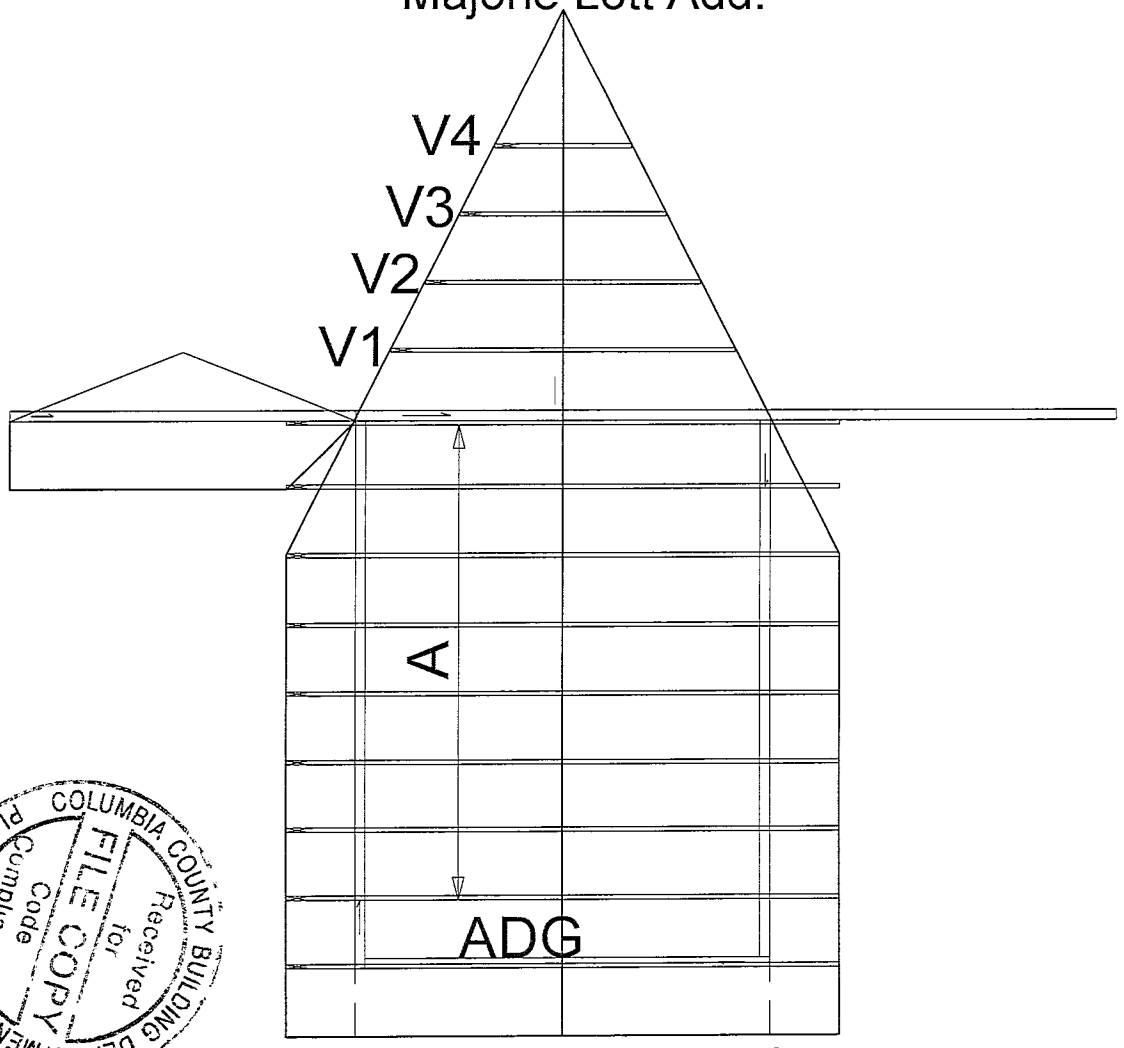
Majorie Lott Add.



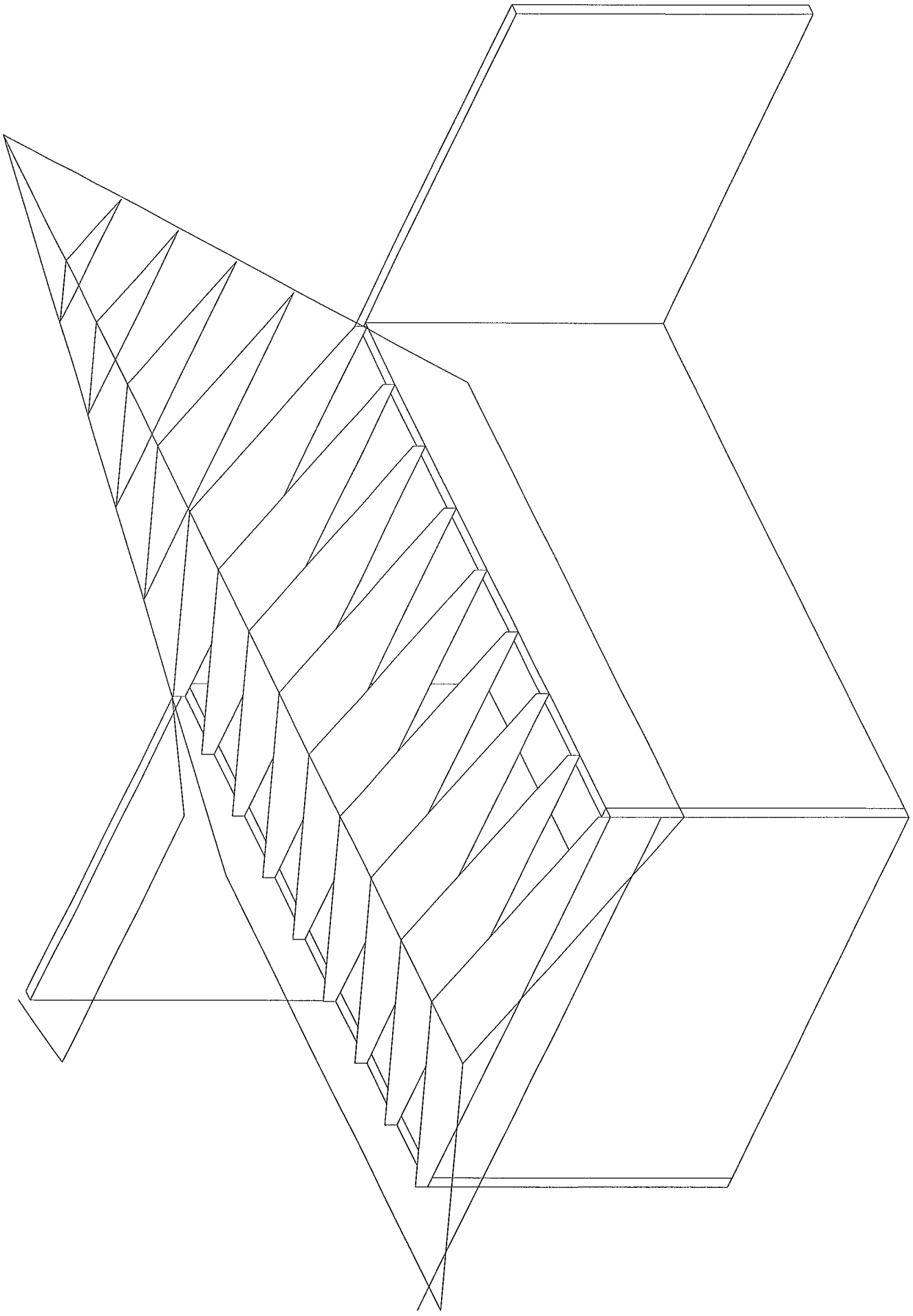
Total Plan Area with OHs = 353 sq.ft
Roof Plane Sheathing Area = 372 sq ft
Total Truss Quantity = 13.

PlanName
Created 07-17-2013

JOB DESCRIPTION Restoration Specialists
/ Majorie Lott add
ADDRESS
JOB #: Q-110
DESIGNER Curt V Burlingame
SALESMAN Curt V Burlingame

JOB NO
Q-110

PAGE NO
1 OF 1



ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
 Document ID 1UY1487-Z0219133408

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
07/19/2013

Truss Fabricator	Anderson Truss Company
Job Identification	13-207--Restoration Specialists Majorie Lott add. -- Lake City, FL
Truss Count	6
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Res/TPI-2007(STD)
Engineering Software	Alpine Software,Version 12.03.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Walter P Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: 12015EC1-GBLLETIN-GABRST10-VAL16010-

#	Ref	Description	Drawing#	Date
1	81613--A	12' Common	13200001	07/19/13
2	81614--ADG	12' Gable	13200002	07/19/13
3	81615-V1	9'11"6 Valley	13200003	07/19/13
4	81616-V2	7'11"6 Valley	13200004	07/19/13
5	81617-V3	5'11"6 Valley	13200005	07/19/13
6	81618-V4	3'11"6 Valley	13200006	07/19/13

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-207--Restoration Specialists Majorie Lott add -- Lake City, FL - A 12' Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP_#3__12A

Lumber grades designated with '12A" use design values approved 1/5/2012 by ALSC

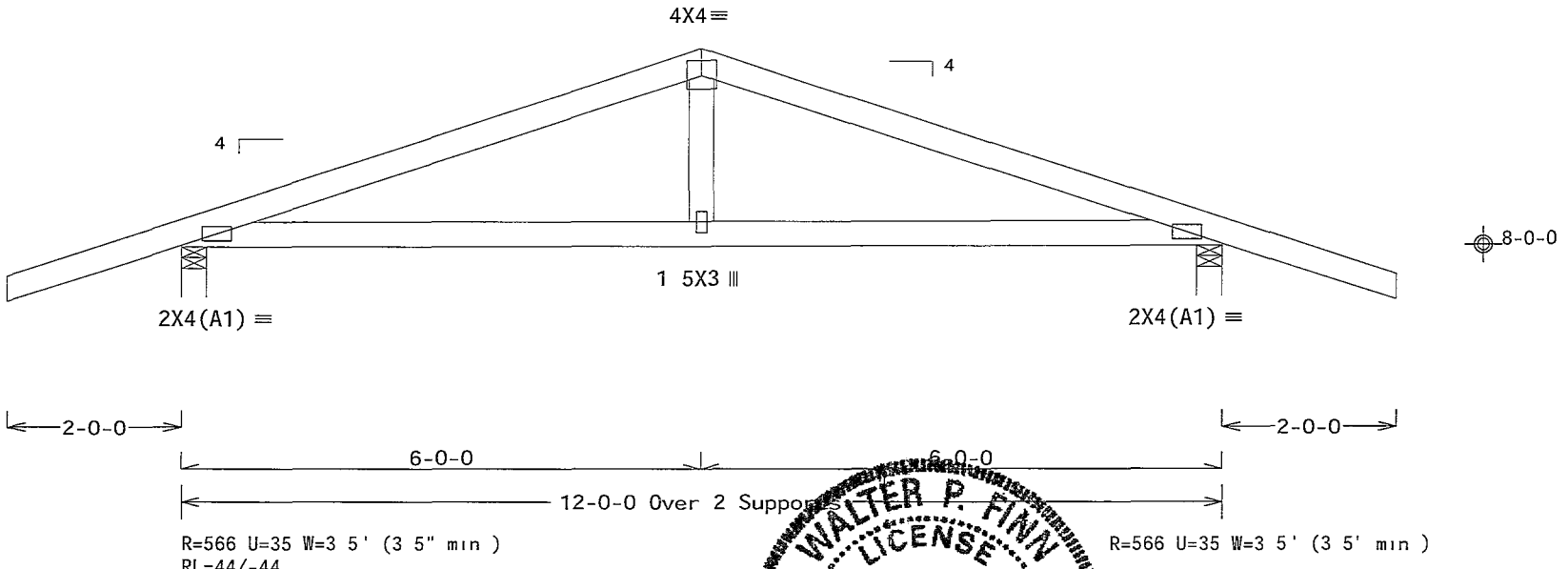
This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

07/19/2013

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) ★12 03.04.03.26.14 07W 8 FL/-/3/-/-/R/- Scale =.5"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling sh pping install ng and brac ng Re and follow the latest ed tion of BCSI (Building Component Safety Informat on by TPI and WTCA) for practices prior to performing these functions Installers shall provide tempo ary bracing per Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom c shall have a properly attached r g d ceiling Locat ons shown for permanent lateral restra nt of shall have bracing nstalled per BCSI sect ons B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from th s de any failure to bu ld the truss in conformance with ANSI/TPI 1 or for handling sh pp ng nstallat on brac ng of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Deta ls unless noted otherw se Refer to drawings 160A-Z for standard plate posit ons A seal on th s draw ng or cover page list ng this draw ng indicates acceptance of professional eng neer ng responsibility solely for the design shown The suitab lity and use of th s design for any structure s the responsibility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

TC LL	20 0 PSF	REF	R487-- 81613
TC DL	7.0 PSF	DATE	07/19/13
BC DL	10 0 PSF	DRW	HCUSR487 13200001
BC LL	0 0 PSF	HC-ENG	SSB/AP
TOT LD	37 0 PSF	SEQN-	310172
DUR FAC.	1.25		
SPACING	24 0"	JREF-	1UY1487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-207--Restoration Specialists Majorie Lott add -- Lake City, FL - ADG 12 Gable)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A
Stack Chord SC1 2x4 SP_#1_12A Stack Chord SC2 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

Bottom chord checked for 10 00 psf non-concurrent live load

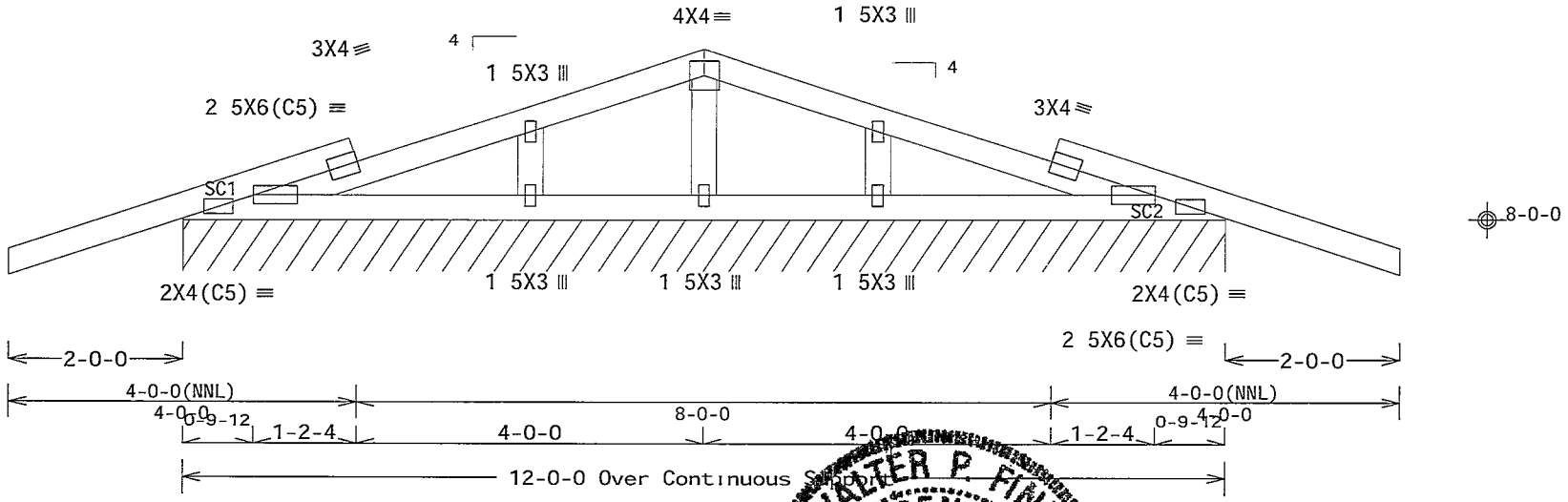
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6



R=94 PLF U=6 PLF W=12-0-0
RL=4/-4 PLF

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 2.03.04.0826.14 DT: FL/-/3/-/-/R/- Scale =.5"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

07/19/2013

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care in fabricating handling shipping nstalling and bracing Re to ans follow the latest ed t on of BCSI (Building Component Safety Information by TPI and WTCA) for practices prior to performing these functions Installers shall provide temporary bracing per B Unless noted otherwise top chord shall have properly attached structural sheathing and bottom ch shall have a properly attached r gid ce l ng Locations shown for permanent lateral restraint of shall have brac ng installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any dev at on from this des any failure to build the trusses in conformance with ANSI/TPI 1 or for handl ng shipp ng nstallat on & brac ng of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Deta ls unless noted otherwise Refer to draw ngs 160A-Z for standard plate posit ons A seal on th s drawing or cover page l sting this draw ng ind cates acceptance of professional eng neer ng respons bility solely for the design shown The suitab lity and use of th s design for any structure s the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more informat on see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

TC LL	20.0 PSF	REF	R487-- 81614
TC DL	7.0 PSF	DATE	07/19/13
BC DL	10.0 PSF	DRW	HCUSR487 13200002
BC LL	0.0 PSF	HC-ENG	SSB/AP
TOT LD	37.0 PSF	SEQN-	310173
DUR FAC.	1.25		
SPACING	24 0"	JREF-	1UY1487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-207--Restoration Specialists Majorie Lott add -- Lake City, FL - V1 9 11 6 Valley)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with '12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

MWFRS loads based on trusses located at least 7 50 ft from roof edge

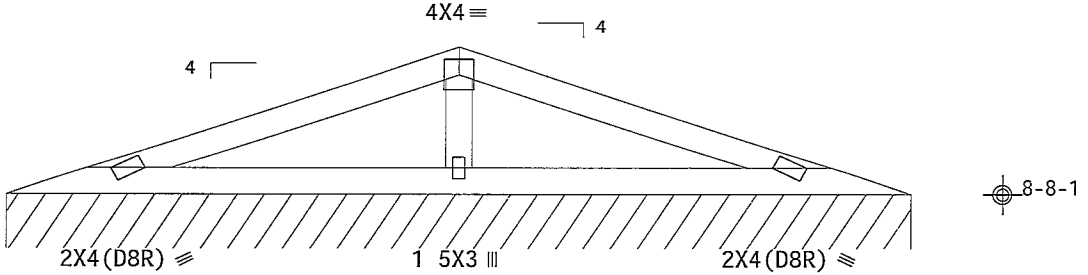
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

See DWG VAL160100212 for valley details



R=75 PLF U=1 PLF W=9-11-6

Design Crit. FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12 03 04 05 06 14 07 FL/-/3/-/-/R/- Scale =.5"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER 07/19/2013

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care in fabr cat ng handling shipping install ng and brac ng Re m follow the latest edition of BCSI (Build ng Component Safety Informat on by TPI and WTCA) for practices prior to perform ng these funct ons Installers shall prov de temporary brac ng per Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom c shall have a properly attached rigid ceiling Locations shown for permanent lateral restra nt of shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Build ng Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from th s des any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pping nstallation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this drawing or cover page l sting th s drawing indicates acceptance of professional eng neering respons bil ty solely for the des gn shown The su tab l ty and use of this des gn for any structure is the responsibility of the Bu lding Designer per ANSI/TPI 1 Sec 2 For more in"rmat on see Th s job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

TC LL	20 0 PSF	REF	R487-- 81615
TC DL	7 0 PSF	DATE	07/19/13
BC DL	10 0 PSF	DRW	HCUSR487 13200003
BC LL	0 0 PSF	HC-ENG	SSB/AP
TOT LD	37.0 PSF	SEQN-	310174
DUR FAC.	1 25		
SPACING	24.0"	JREF-	1UY1487_Z02

(13-207--Restoration Specialists Majorie Lott add -- Lake City, FL - V2 7'11'6 Valley)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

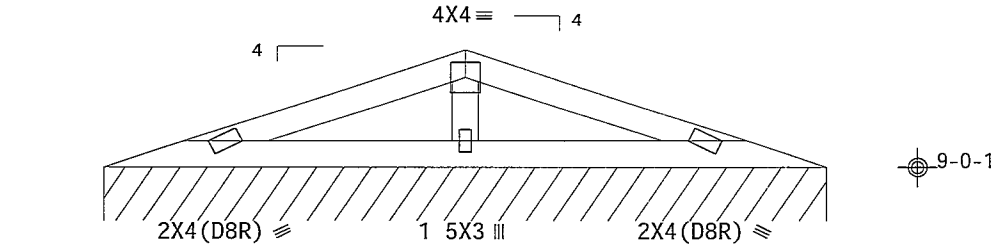
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

See DWG VAL160100212 for valley details



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP Wave

FT/RT=10%(0%)/0(0)

12.03.04 06.26.14

FL/-/3/-/-/R/-

Scale =.5"/Ft

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

07/19/2013

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from th s desi any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipp ng installation & bracing of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing indicates acceptance of professional eng neering responsibility solely for the design shown The suitability and use of th s design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

TC LL	20.0 PSF	REF	R487-- 81616
TC DL	7.0 PSF	DATE	07/19/13
BC DL	10.0 PSF	DRW	HCUSR487 13200004
BC LL	0 0 PSF	HC-ENG	SSB/AP
TOT LD.	37.0 PSF	SEQN-	310175
DUR.FAC	1 25		
SPACING	24.0"	JREF-	1UY1487_Z02

(13-207--Restoration Specialists Majorie Lott add -- Lake City, FL - V3 5'11 6 Valley)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A' use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

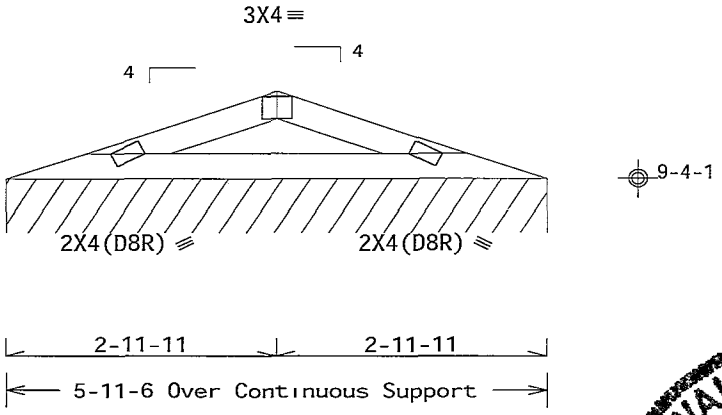
See DWG VAL160100212 for valley details

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=75 PLF U=0 PLF W=5-11-6

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
07/19/2013

Design Crit FBC2010Res/TPI-2007(STD FT/RT=10%(0%)/0(0) 2 03 04 0526 14 CT FL/-/3/-/-/R/- Scale =.5"/Ft.

PLT TYP Wave

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from th s des gn any fa lure to build the truss n conformance with ANSI/TPI 1 or for handl ng shipp ng nstallat on & bracing of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Details unless noted otherwise Refer to drawings 160A-Z for standard plate posit ons A seal on th s drawing or cover page listing this drawing indicates acceptance of profess onal eng neer ng respons bility solely for the design shown The suitabil ty and use of th s des gn for any structure s the respons bility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more informat on see This job s general notes page ITW BCG www twbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

TC LL	20 0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT LD	37 0 PSF
DUR FAC.	1.25
SPACING	24 0"

REF	R487-- 81617
DATE	07/19/13
DRW	HCUSR487 13200005
HC-ENG	SSB/AP
SEQN-	310176
JREF-	1UY1487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-207--Restoration Specialists Majorie Lott add -- Lake City, FL - V4 3'11 6 Valley)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with 12A" use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

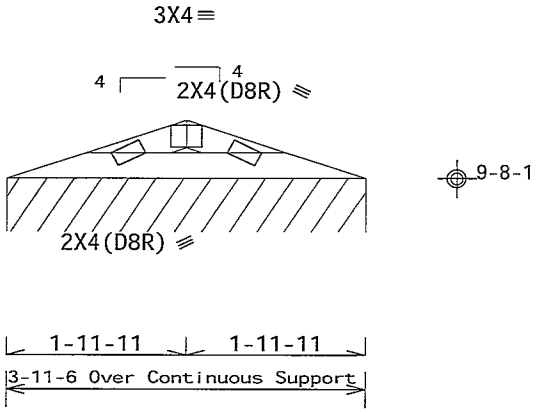
See DWG VAL160100212 for valley details

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=74 PLF U=0 PLF W=3-11-6

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
07/19/2013

Design Crit FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)
12 03 04 0326 14 GT1

PLT TYP Wave

FL/-/3/-/-/R/-

Scale = 5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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TC LL	20 0 PSF	REF	R487-- 81618
TC DL	7.0 PSF	DATE	07/19/13
BC DL	10 0 PSF	DRW	HCUSR487 13200006
BC LL	0 0 PSF	HC-ENG	SSB/AP
TOT LD	37.0 PSF	SEQN-	310177
DUR FAC.	1.25		
SPACING	24.0"	JREF-	1UY1487_Z02

Gable Stud Reinforcement Detail

ASCE 7-10 110 mph Wind Speed 15 Mean Height Enclosed Exposure C Kzt = 1.00

Or 100 mph Wind Speed 15 Mean Height Partially Enclosed Exposure C Kzt 1.00
Or 100 mph Wind Speed 15 Mean Height Enclosed Exposure D Kzt 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 L Brace * Group A	(1) 1x4 L Brace * Group B	(1) 2x4 L Brace * Group A	(1) 2x4 L Brace * Group B	(2) 2x4 L Brace ** Group A	(2) 2x4 L Brace ** Group B	(1) 2x6 L Brace * Group A	(1) 2x6 L Brace * Group B	(2) 2x6 L Brace ** Group A	(2) 2x6 L Brace ** Group B
24" OC	SPF HF	#1 / #2	4-10	8-2	8-6	9-8"	10	11-6"	12-0	14-0	14-0"	14-0	14-0
		#3	4-7	7-9	8-3	9-7"	9-11	11-5	11-10"	14-0	14-0	14-0"	14-0
		Stud	4-7	8-1"	8-4	9-7	9-11"	11-5	11-10"	14-0"	14-0	14-0"	14-0
		Standard	4-7	8-1"	8-4	9-7"	9-11	11-5"	11-10"	14-0"	14-0"	14-0"	14-0
	SP DFL	#1	4-11"	8-3	8-7	9-9"	10-1"	11-7"	12	14-0"	14-0"	14-0	14-0
		#2	4-10	8-2	8-6	9-8"	10-1	11-6"	12-0	14-0"	14-0"	14-0"	14-0
		#3	4-7"	6-11	7-4	9-3"	9-10	1-5"	11-10"	14-0	14-0	14-0	14-0
		Stud	4-7	6-11	7-4	9-3"	9-10	1-5	11-10	14-0"	14-0"	14-0	14-0
		Standard	4-7	6-0	6-4	8-0"	8-6	0-0"	11	12-6	13-5	14-0	14-0
16" OC	SPF HF	#1 / #2	5-6	9-5	9-9	11-1	11-6	3-2"	13-9	14-0	14-0	14-0	14-0
		#3	5-3"	9-3	9-9	10-11	11-4	13-0	13-7"	14-0	14-0"	14-0"	14-0
		Stud	5-3	9-3	9-7	10-11	11-4"	13-0	13-7"	14-0	14-0"	14-0	14-0
		Standard	5-3	9-3"	9-7"	10-11"	11-4	13-0"	13-7"	14-0"	14-0"	14-0"	14-0
	SP DFL	#1	5-8	9-5"	9-10	11-2"	11-7	13-3"	13-10"	14-0"	14-0"	14-0"	14-0"
		#2	5-6"	9-5"	9-9	11-1"	11-6"	13-2	13-9"	14-0	14-0	14-0	14-0
		#3	5-3"	8-6"	9-0"	10-11"	11-4"	13-0	13-7"	14-0"	14-0"	14-0"	14-0
		Stud	5-3	8-6	9-0"	10-11"	11-4"	13-0"	13-7"	14-0"	14-0"	14-0	14-0
		Standard	5-3"	7-4"	7-10"	9-9"	10-5"	13-0	13-7"	14-0"	14-0"	14-0	14-0"
12" OC	SPF HF	#1 / #2	6-1	10-4	10-8"	12-2"	12-8"	13-2"	14-0	14-0"	14-0"	14-0"	14-0
		#3	5-9"	10-2"	10-7	12-0"	12-6"	14-0"	14-0"	14-0	14-0	14-0"	14-0
		Stud	5-9	10-2	10-7	12-0	12-6	4-0	14-0	14-0	14-0	14-0"	14-0
		Standard	5-9	10-2	10-7	12-0"	12-6"	14-0"	14-0	14-0"	14-0	14-0	14-0
	SP DFL	#1	6-2	10-5	10-9"	12-3	12-9"	14-0"	14-0	14-0	14-0	14-0"	14-0
		#2	6-1	10-4	10-8	12-2	12-8	14-0	14-0	14-0	14-0	14-0"	14-0
		#3	5-9	9-9	10-5	12-0"	12-6	14-0	14-0	14-0	14-0"	14-0	14-0
		Stud	5-9"	9-9	10-5	12-0	12-6"	14-0"	14-0	14-0	14-0	14-0	14-0
		Standard	5-9	8-6	9-0"	11-3"	12-1	14-0	14-0	14-0"	14-0"	14-0	14-0

Bracing Group Species and Grades

Group A

Spruce Pine Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B

Hem Fir
#1 & Btr
#1

Douglas Fir Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress Rated Board)

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load)

Gable end supports load from 4-0" outlookers with 2-0 overhang or 12" plywood overhang

So. Pine lumber design values based on the ALSC January 2012 ruling

Attach L braces with 10d (0.128"x3.0" min) nails:
* For (1) L brace: space nails at 2" o.c. 18" end zones and 4" o.c. between ends
* For (2) L braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones

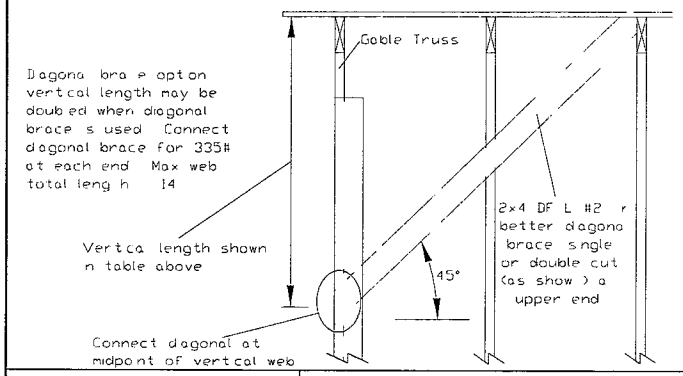
L bracing must be a minimum of 80% of web member length

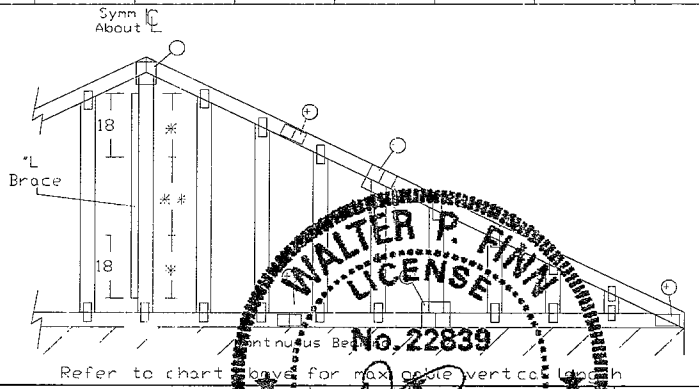
Gable Vertical Plate Sizes

Vertical Length	No. Splice
Less than 4'-0"	1.5X4 or 2X3
Greater than 4'-0" but less than 11'-6"	2X4
Greather than 11'-6"	2.5X4

+ Refer to common truss design for peak splice and heel plates

Refer to the Building Designer for conditions not addressed by this detail





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Building Components Group Inc.

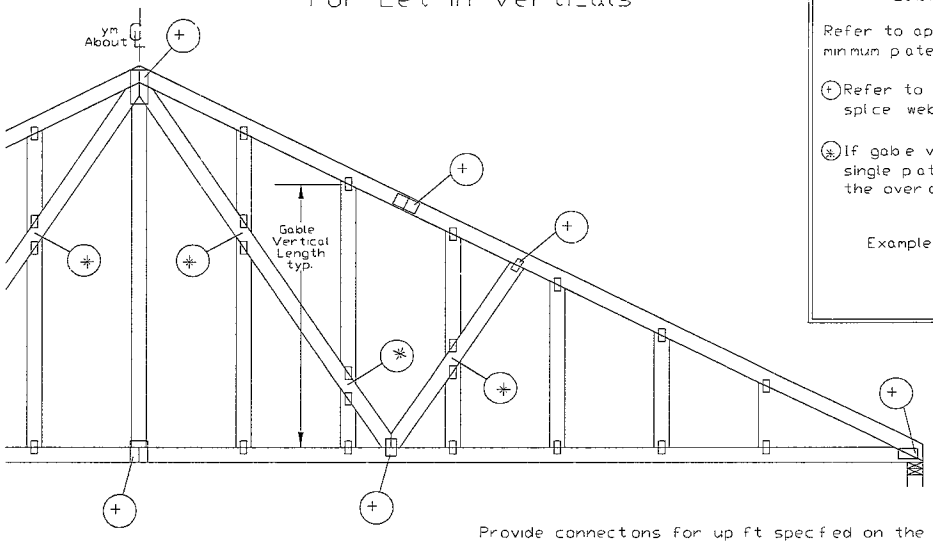
Earth City, MO 63045

MAX TOT LD 60 PSF

MAX SPACING 24.0

REF ASCE7-10-GAB12015
DATE 2/14/12
DRWG A12015ENC100212

Gable Detail
For Let-in Verticals



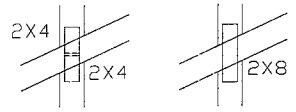
Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical stud

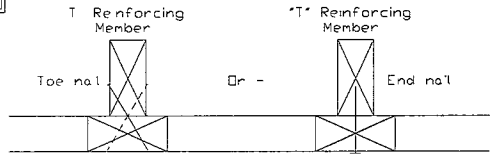
(+) Refer to Engineered truss design or peak splice web and heel plates

(*) If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web

Example



"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members multiply T increase by length (based on appropriate ITW gable detail)

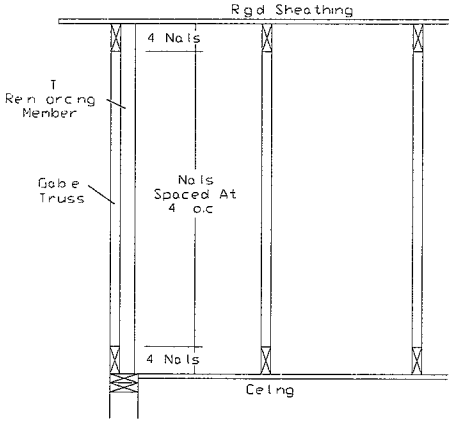
Maximum allowable T reinforced gable vertical length is 14' from top to bottom chord

"T" reinforcing member material must match size species and grade of the "L" reinforcing member

Web Length Increase w/ T Brace

"T" Reinf Mbr Size	"T" Increase
2x4	30 %
2x6	20 %

Example
- ASCE 7-10 Wind Speed = 120 mph
- Mean Roof Height 30 ft Kzt = 1.00
- Gable Vertical - 24"oc SP #3
- "T" Reinforcing Member Size - 2x4
- "T" Brace Increase (From Above) = 30% = 1.30
- (1) 2x4 "L" Brace Length = 8' 7"
- Maximum T Reinforced Gable Vertical Length
  1.30 x 8' 7" - 11' 2"



Provide connections for uplift specified on the engineered truss design

Attach each T reinforcing member with

End Driven Nails
- 10d Common (0.148" x 3" min) Nails at 4" o.c. plus
- (4) nails in the top and bottom chords

Toenailed Nails
- 10d Common (0.148"x3" min) Toenails at 4" o.c. plus
- (4) toenails in the top and bottom chords

This detail to be used with the appropriate ITW gable detail for ASCE wind load

ASCE 7-98 Gable Detail Drawings
A1015980109 A12015980109 A11015980109 A10015980109
A13030980109 A12030980109 A11030980109 A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109 A12015020109 A11015020109 A10015020109 A14015020109
A13030020109 A12030020109 A11030020109 A10030020109 A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109 A12015050109 A11015050109 A10015050109 [illegible]
A13030050109 A12030050109 A11030050109 A10030050109 A14030050109

ASCE 7-10 Gable Detail Drawing
A11515ENC100212 A12015ENC100212 A14015ENC100212 [illegible]
A18015ENC100212 A20015ENC100212 A20015PEM100212
A11530ENC100212 A12030ENC100212 A14030ENC100212 A16030ENC100212
A18030ENC100212 A20030ENC100212 A22030ENC100212 A20030PEM100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length

WALTER P. FINN
LICENSE
No. 22839
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
07/19/2013

Jul 19 '13

ITW Building Components Group

Earth City MO 63045

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ITWBCG: www.itwbcg.com TPI: www.tpinst.org WTCA: www.sbcindustry.org, ICC: www.iccsafe.org

MAX TOT LD	60 PSF
DUR FAC	ANY
MAX SPACING	24.0"

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212

ASCE 7-10 120 mph, 30' Mean Height, Closed Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph 30ft Mean Hgt ASCE 7-10 Enclosed Exp C or
100 mph 30ft Mean Hgt ASCE 7-10, Enclosed Exp D or
100 mph 30ft Mean Hgt ASCE 7 10, Part Enclosed Exp C
Kzt - 100 Wind TC DL-50 psf, Wind BC DL=50 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

See Engineers sealed design referencing this detail for lumber plates, and other information not shown on this detail

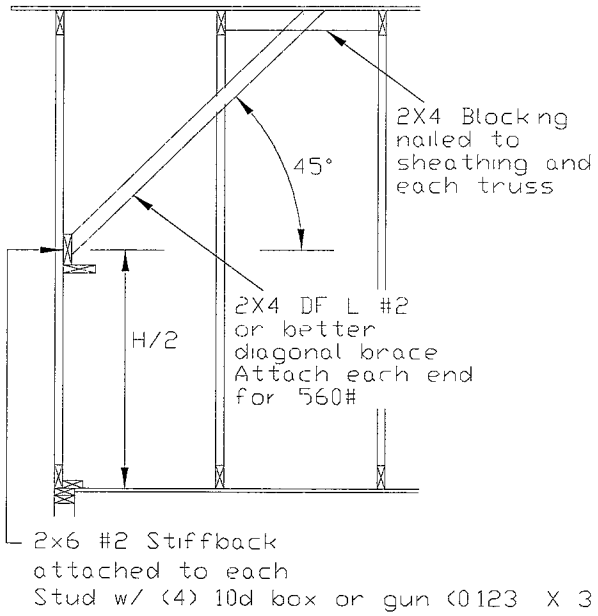
Nails 10d box or gun (0128 x3 mn) nails

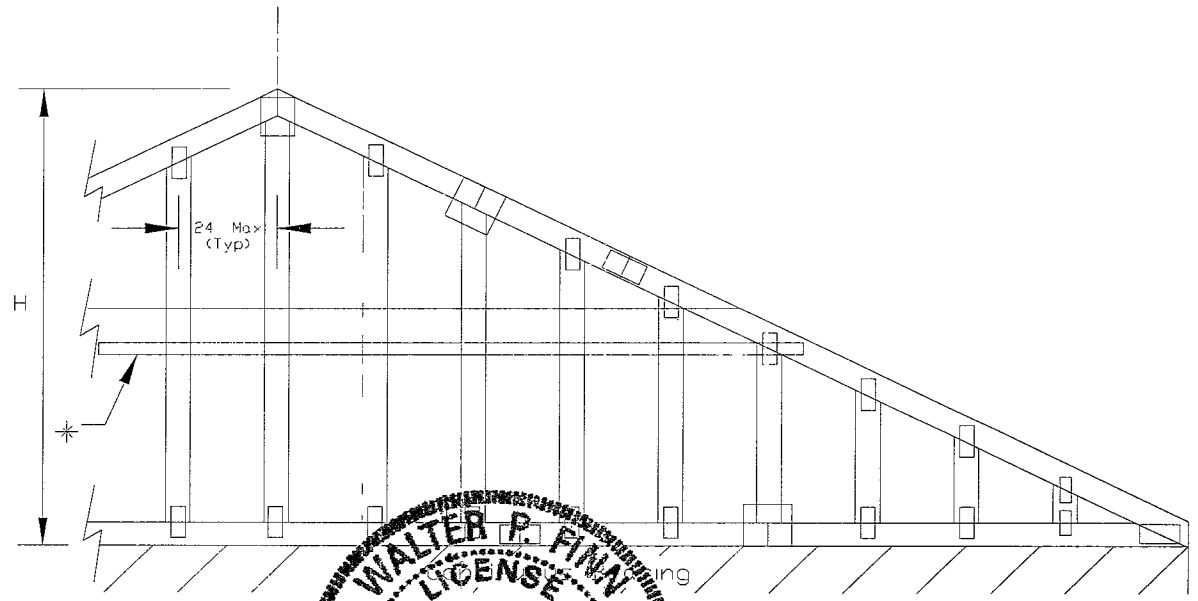
H Less than 46 no stud bracing required

H Greater than 46 to 76 in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 60 (see detail below or refer to DRWG A12030ENC10)

H Greater than 76 to 120 max
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 40 (see detail below or refer to DRWG A12030ENC10)

✳ Optional 2x L-reinforcement attached to stiffback with 0d box or gun (0128 x 3 mn) nails @ 6 oc





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ITWBCG www.twbcg.com TPI www.tpinst.org WTCA: www.sbcindustry.org ICC www.iccsafe.org

MAX TOT LD 60 PSF

MAX SPACING

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212

Valley Detail - ASCE 7-10 160 mph 30' Mean Height, Enclosed, Exp C Kzt=1.00

Top Chord 2x4 SP #2N SPF #1/#2 DF-L #2 or better
Bot Chord 2x4 SP #2N or SPF #1/#2 or better
Webs 2x4 SP #3 SPF #1/#2 DF-L #2 or better

** Attach each valley to every supporting truss with (2) 16d box (0.135 x 3.5") nails toe-nailed for
ASCE 7-10 160 mph 30' Mean Height, Enclosed Building Exp C Wind TC DL=5 psf Kzt = 1.00
Or
ASCE 7-10 140 mph 30' Mean Height Enclosed Building Exp D Wind TC DL=5 psf Kzt = 1.00

Unless specified on engineer's sealed design apply 1x4 T brace 80% length of web same species and SRB grade or better attached with 8d box (0.113 x 2.5") nails at 6" o.c. or continuous lateral bracing equally spaced for vertical valley webs greater than 7'-9"

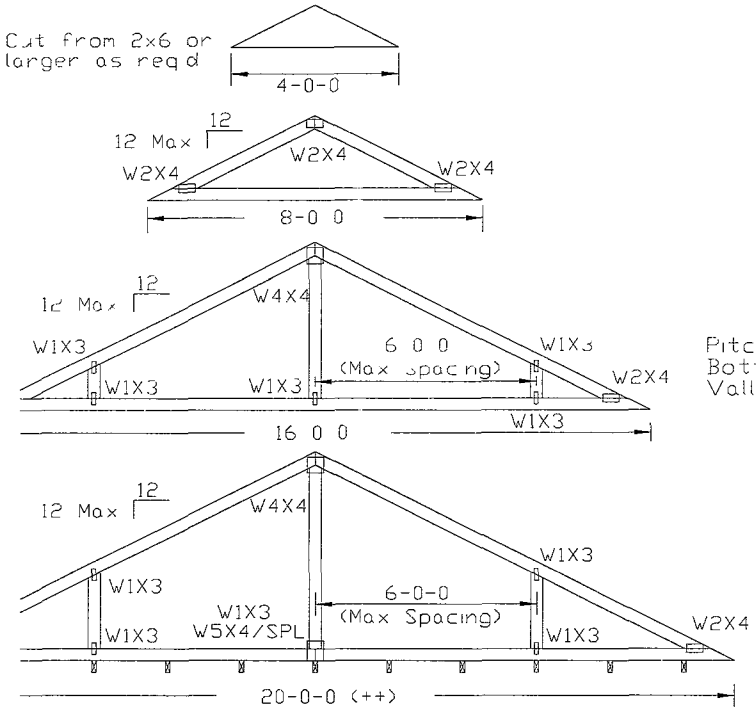
or verticals over 10'-0" tall apply (2) 1x4 T braces 80% length of web same species and SRB grade or better attached with 8d box (0.113 x 2.5") nails @ 6" o.c.

Top chord of truss beneath valley set must be braced with properly attached rated sheathing applied prior to valley truss installation
Or
Purlins at 24" o.c. or as otherwise specified on Engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design

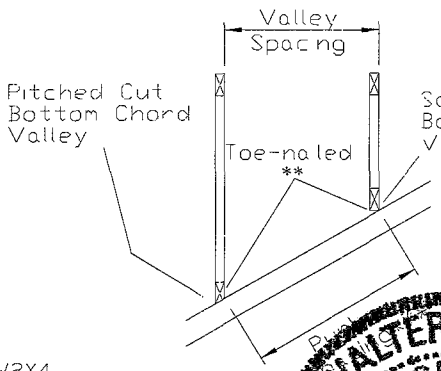
*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord

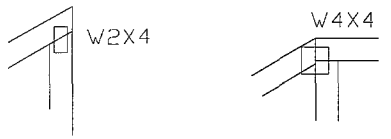
++ Larger spans may be built as long as the vertical height does not exceed 14'-0"

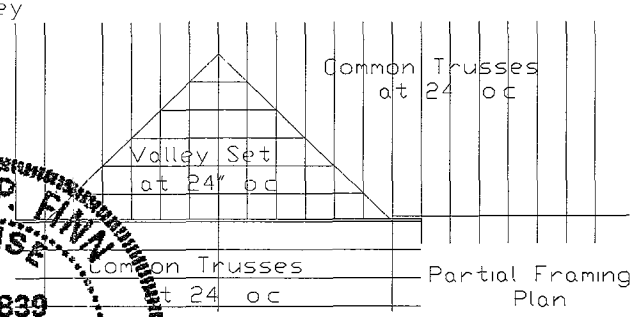
Bottom chord may be square or pitched cut as shown



Supporting trusses at 24" o.c. maximum spacing







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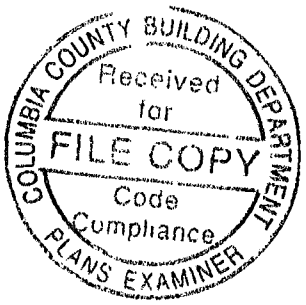
TC LL	30	30	40 PSF	REF VALLEY DETAIL
TC DL	20	15	7 PSF	DATE 2/14/12
BC DL	10	10	10 PSF	DRWG VAL160100212
BC LL	0	0	0 PSF	
TOT LD	60	55	57 PSF	
DURFAC	1.25/1.33	1.15	1.15	
SPACING		24.0		

Wind Load Requirements: Lott Residence
142 SW Arvid Glen, Lake City, Florida

Design Parameters and General Description

Code compliance:	Florida Building Code, 2010 edition; ASCE/SEI 7-10
Risk category:	II
Ultimate design wind speed:	120 mph
Wind directionality factor, K_d :	0.85
Exposure category:	B
Topographic factor, K_{zt} :	1.0
Gust effect factor, G:	0.85
Enclosure classification:	enclosed (by definition)
Internal pressure coeff., GC_{pi} :	±0.18
Number of stories:	one
Plan dimensions:	12.00 ft x 16.17 ft
Exterior walls:	wood frame
Roof slope:	4:12
Roof type:	gable
Eave height:	<10 ft
Mean roof height:	<15 f.
Roof overhang:	2.00 ft max.



Drawings

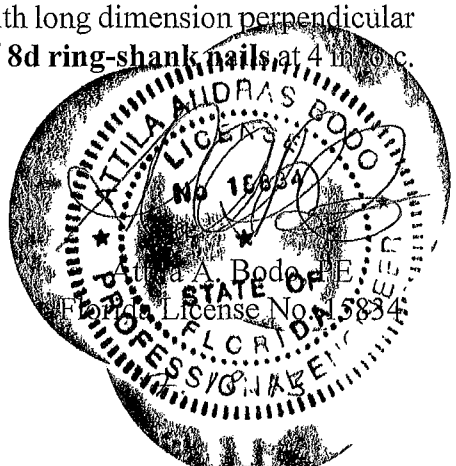
See drawings for additional details. In case of conflict, the requirements of these calculations govern.

Roof Framing

Pre-engineered wood roof trusses at 24" o.c. (nominal). See calculations by *Anderson Truss Company, Lake City, Florida*, dated July 18, 2013, for details.

Roof Sheathing

Minimum 7/16" Exposure 1 wood structural panels. Install with long dimension perpendicular to framing and staggered end joints. Fasten to roof framing w/ **8d ring-shank nails** at 4 in. o.c. on edges and 8 in. o.c. at intermediate framing. Provide blocking at 48 in. max. o.c. in first two framing spaces at gable end. Blocking shall be full depth of truss chords.



Attila A. Bodo, P.E.
Florida License No. 15834

Maximum Wind Pressures

Velocity pressure exposure coefficient, K_z :	0.70
Velocity pressure, q_z:	21.9 psf

Components and cladding:

roof external pressure coefficient, GC_p :	-2.6 (maximum)
roof design wind pressure, p :	-61.0 psf
wall external pressure coefficient, GC_p :	-1.4 (maximum)
wall design wind pressure, p :	-34.7 psf

MWFRS:

maximum design wind pressure on roof:	-27.4 psf
total design wind pressure on walls:	31.6 psf

Exterior Walls

use: 2x4 SPF No. 2 grade or better studs at 16" o.c. maximum

Shearwall Sheathing

Minimum 15/32" plywood or 7/16" OSB, sheathing grade; attach all edges to framing with 8d common nails at 4 in. o.c. Attach to intermediate framing with 8d common nails at 12 in. o.c. Sheathing shall be applied to outside face of **all exterior frame walls**.

Headers

Provide headers in accordance with Section 2308 of the *Florida Building Code, 2010 edition.*

Foundations (sizes based on wind load requirements only)

Thickened edge: 20" thick x 12" wide; reinforce with (2) #5 continuous bars. Lap bars 25". Provide (2) 25"x25" corner bars at each corner.

Connector Schedule

To Connect	To	No.	Product Code[1]	Fastener	Uplift Capacity, lb
truss A	top plates	1	H5	(4+4) 8dx1½" common nails	265
truss ADG	top plates	1	LTP4	(6+6) 8dx1½" common nails	575
outrigger	truss	1	A35	(6+6) 8dx1½" common nails	340

To Connect	To	No.	Product Code[1]	Fastener	Uplift Capacity, lb
top plates	stud	1[2]	SSP	(3+4) 10d common nails	435
stud	bottom plate[4]	1[2]	SSP	(4+1) 10d common nails	420
header	header stud(s)	1[2]	MSTA15	(6+6) 10d common nails	1020
header stud(s)	bottom plate[4]	1[2]	SPH4	(10) 10dx1½" common nails	1065
bottom plate[4]	concrete	1[3]	--	1/2"dia x10" anchor bolt w/ 3"x3"x1/8" washers	>2200

Notes:

1. Product codes refer to connector hardware as manufactured by Simpson Strong-Tie Company, Inc , Pleasanton, CA Other manufacturers' products of equal or higher capacity may be substituted.
2 Use one connector on every stud
3. Connector spacing: within 6" of each end of each plate, within 6" of corners, and at 32" o c maximum
4 All metal hardware and fasteners in contact with pressure-treated wood shall be corrosion-resistant.
5 See truss engineering for truss-to-truss connections
6. Unless noted otherwise, all nails to be common wire nails with the following diameters:
   a 8d 0 131 in
   b 10d 0 148 in.
   c 16d 0 162 in
7 Connections not otherwise specified herein or shown on the drawings shall be in accordance with the 2010 Florida Building Code