



THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C. STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

(END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)		
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT	
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT	
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT	

# **GENERAL NOTES:**

RUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS, TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4. FB = 85KSI. WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT.
AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

#### **BUILDER'S RESPONSIBILITY:**

HE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, RUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

# **ROOF SYSTEM DESIGN:**

DESIGN PRESSURES.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR. IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING, THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

**DESIGN CRITERIA & LOADS:** 

CODE FOR DESIGN LOADS

(BUILDER MUST FIELD VERIFY)

(BUILDER MUST FIELD VERIFY)

ENCLOSURE CLASSIFICATION ENCLOSED

C&C DESIGN PRESSURES SEE TABLE

SOIL BEARING CAPACITY 1500 PSF

FLORIDA BUILDING CODE RESIDENTIAL

ASCE 7-16

7-45 DEGREES

40 PSF LIVE LOAD

30 PSF LIVE LOAD

20 PSF LIVE LOAD

16 PSF LIVE LOAD

12 PSF LIVE LOAD

COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C)

+22.6(Vasd) -25.5(Vasd)

+21.7(Vasd) -24.1(Vasd)

+42.6(Vult) -46.2(Vult)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

THIS BUILDING IS NOT IN THE FLOOD ZONE

+25.6(Vasd) -27.8(Vasd) +25.6(Vasd) -34.2(Vasd)

END 4' FROM ALL OUTSIDE CORNER

+42.6(Vult) -57(Vult)

**BUILDING CODE** 

WINDLOADS

BASIC WIND SPEED

(ASCE 7-16, 3S GUST

TOPOGRAPHIC FACTOR

INTERNAL PRESSURE

MEAN ROOF HEIGHT

FLOOR LOADING

ROOMS OTHER THAN

SLEEPING ROOM

SLEEPING ROOMS **ROOF LOADING** 

FLAT OR < 4:12

4:12 TO < 12:12

12:12 & GREATER

FLOOD ZONE

WIND AREA (FT2)

9x7 GARAGE DOOR

16x7 GARAGE DOOR

WIND EXPOSURE

RISK CATEGORY

COEFFICIENT

ROOF ANGLE

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution.

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portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

CERTIFICATION: I hereby certify that I have

examined this plan, and that the applicable

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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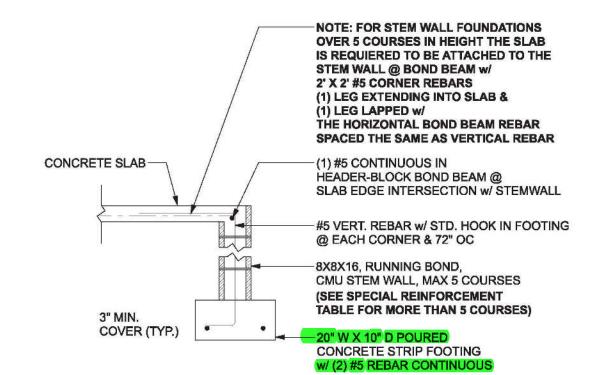


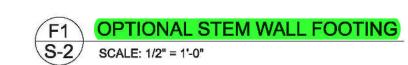
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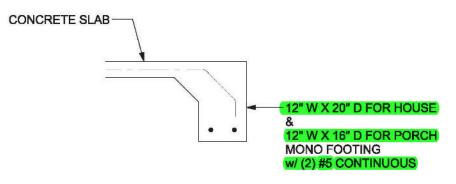
Mark Disosway P.E. 163 SW Midtown Place Suite 103 Lake City, Florida 32025 386.754.5419 disoswaydesign@gmail.com

JOB NUMBER: 211063 **S-1** OF 3 SHEETS

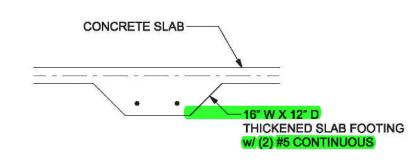


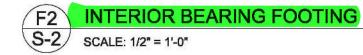


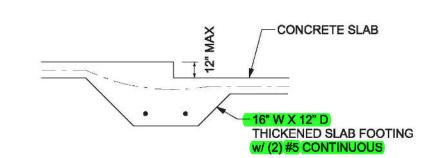




F1 MONOLITHIC S-2 SCALE: 1/2" = 1'-0" MONOLITHIC FOOTING









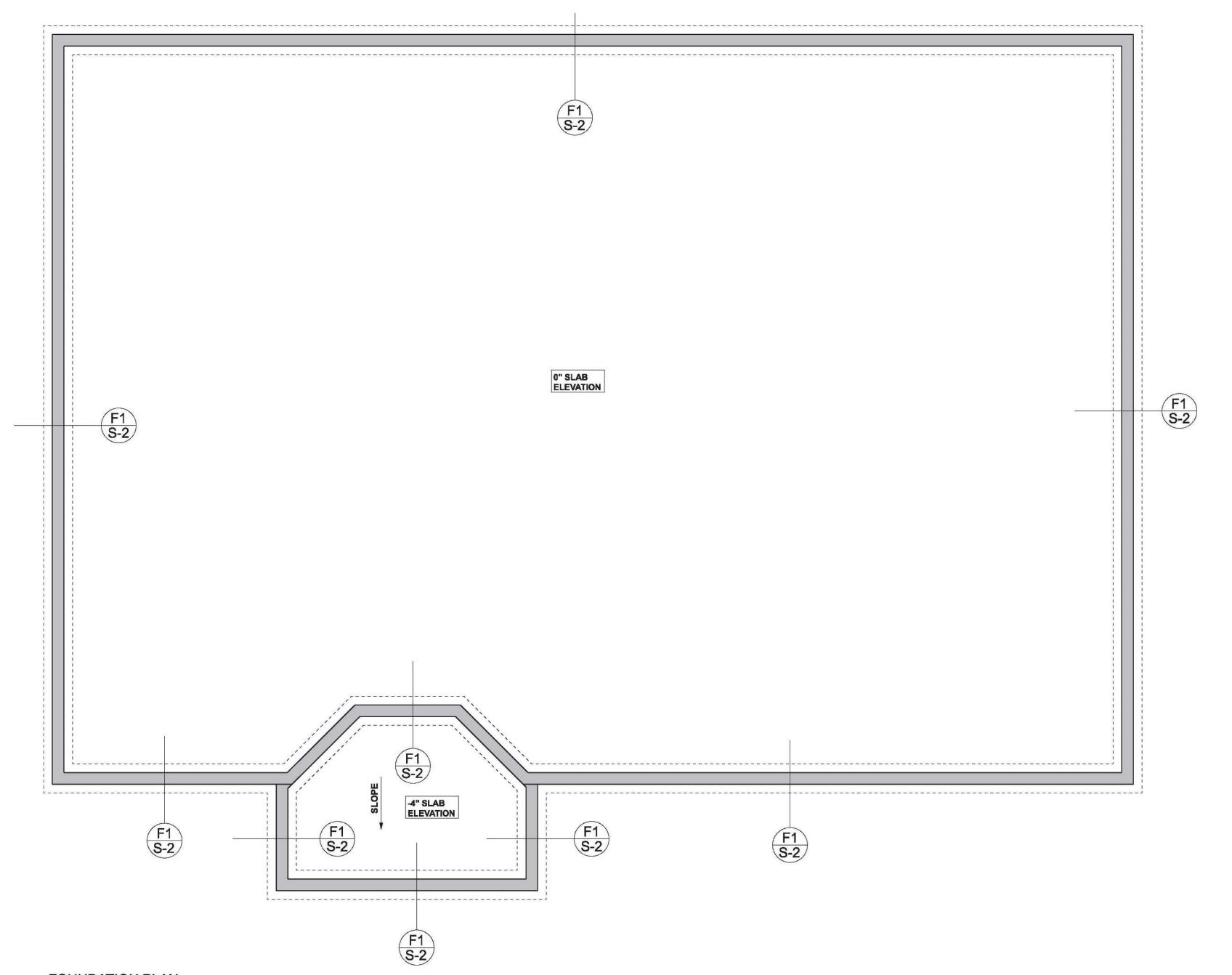
footing and toward the t side of the v vertically or	ssumes 40 ksi fo bent 24" into the ension side of th vall). If the wall i a horizontal bor CMU may be us	e reinforce ne CMU w s over 8' h nd beam w	d slab at th all (away fr nigh, add Di rith 1#5 con	e top. The volume the soil urowall ladd at income at records at re	vertical stee pressure, w ler reinforce nid height. I	I is to be pla within 2" of the ment at 16" For higher p	aced ne exterio 'OC
STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTIC FOR	AL REINFORG 8" CMU STEM (INCHES O.C.	CEMENT WALL	VERTICA FOR 12	AL REINFORC 2" CMU STEM INCHES O.C.)	WALL
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96

HEIGHT (FEET)	BACKFILL HEIGHT	FOR 8" CMU STEMWALL (INCHES O.C.)		FOR 12" CMU STEMWALL (INCHES O.C.)			
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

	AND MATERIALS FOR THIS PROJECT
	REQUIREMENTS OF "SPECIFICATION ES" (ACI 530.1/ASCE 6/TMS 602).
THE CONTRACTOR AND MA	ASON MUST IMMEDIATELY, BEFORE
	ENGINEER OF ANY CONFLICTS
	THESE DESIGN DRAWINGS.
	30.1-02 MUST BE APPROVED BY
THE ENGINEER IN WRITING	l.

	ACI530.1-02 Section	Specific Requirements		
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi		
2.1	Mortar	ASTM C 270, Type N, UNO		
2.2	Grout	ASTM C 476, admixtures require approval		
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block		
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"		
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)		
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS		
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS		
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.		
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.		

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BOTTOM OF EXTERIO	R FOOTINGS	SHALL BE A MIN	IIMUM OF
15. BELOM ONDISTO	KRED SOIL O	R ENGINEERED	FILL
	$\sim$		



**FOUNDATION PLAN** SCALE: 1/4" = 1'-0"

**FOUNDATION NOTES** 

FN - 1
DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS
FOR ACTUAL DIMENSIONS, RECESSES IN SLAB,
STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR
MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR
DIMENSION ERRORS ON THIS PLAN.

CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING
FN - 2 IN ALL AREAS BY REVIEWINGTHE ROOF TRUSS PLAN
(BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN

FN - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/

6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS

@ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL
POLY VAPOR BARRIER W/ 6" LAPS SEALED W/
POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL
(ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT
METHOD CAN BE USED INSTEAD)

LIMITATION: This design is valid for one building, at specified location.

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permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering compty with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

MARK DISOSWAY P.E. 53915

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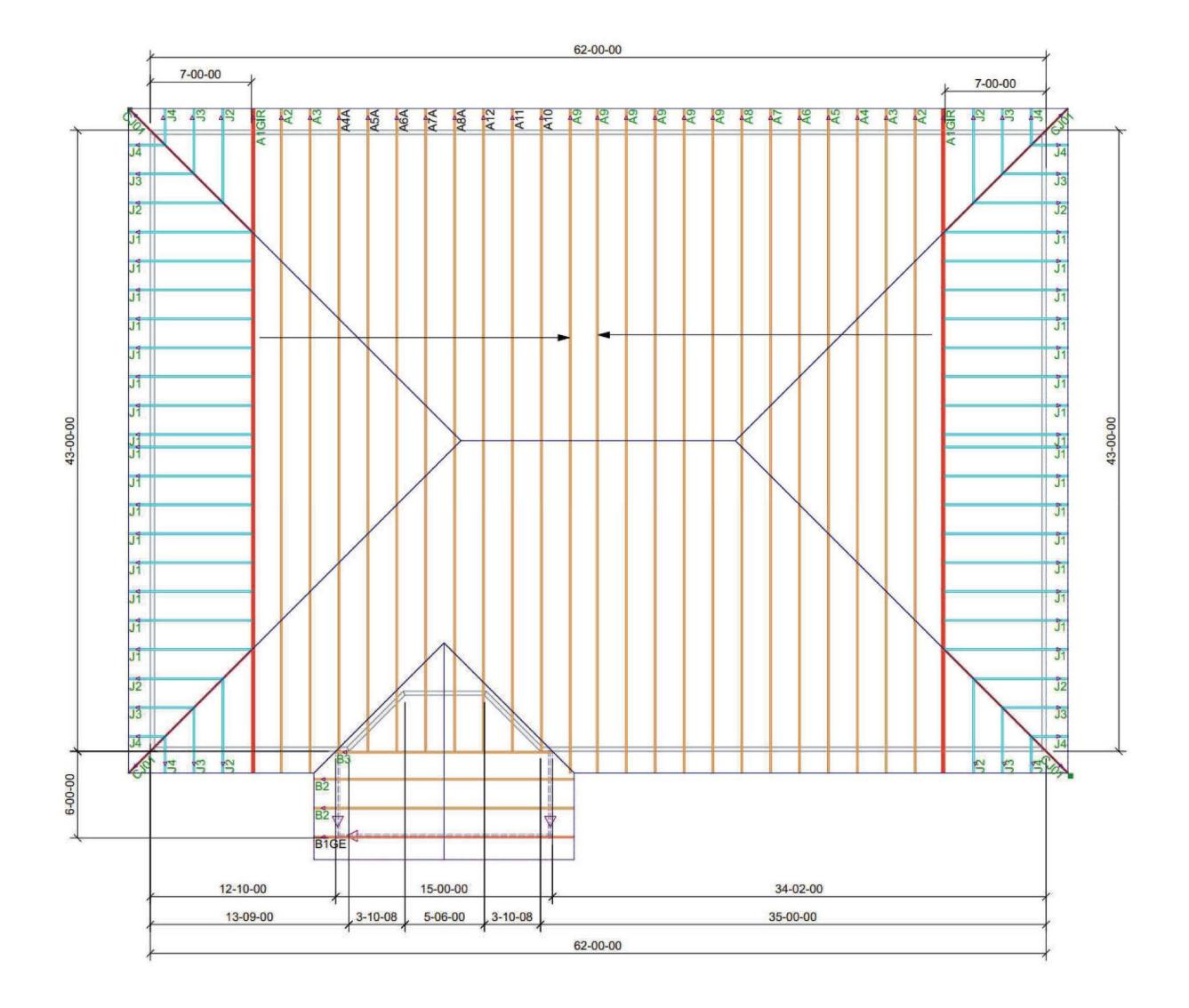


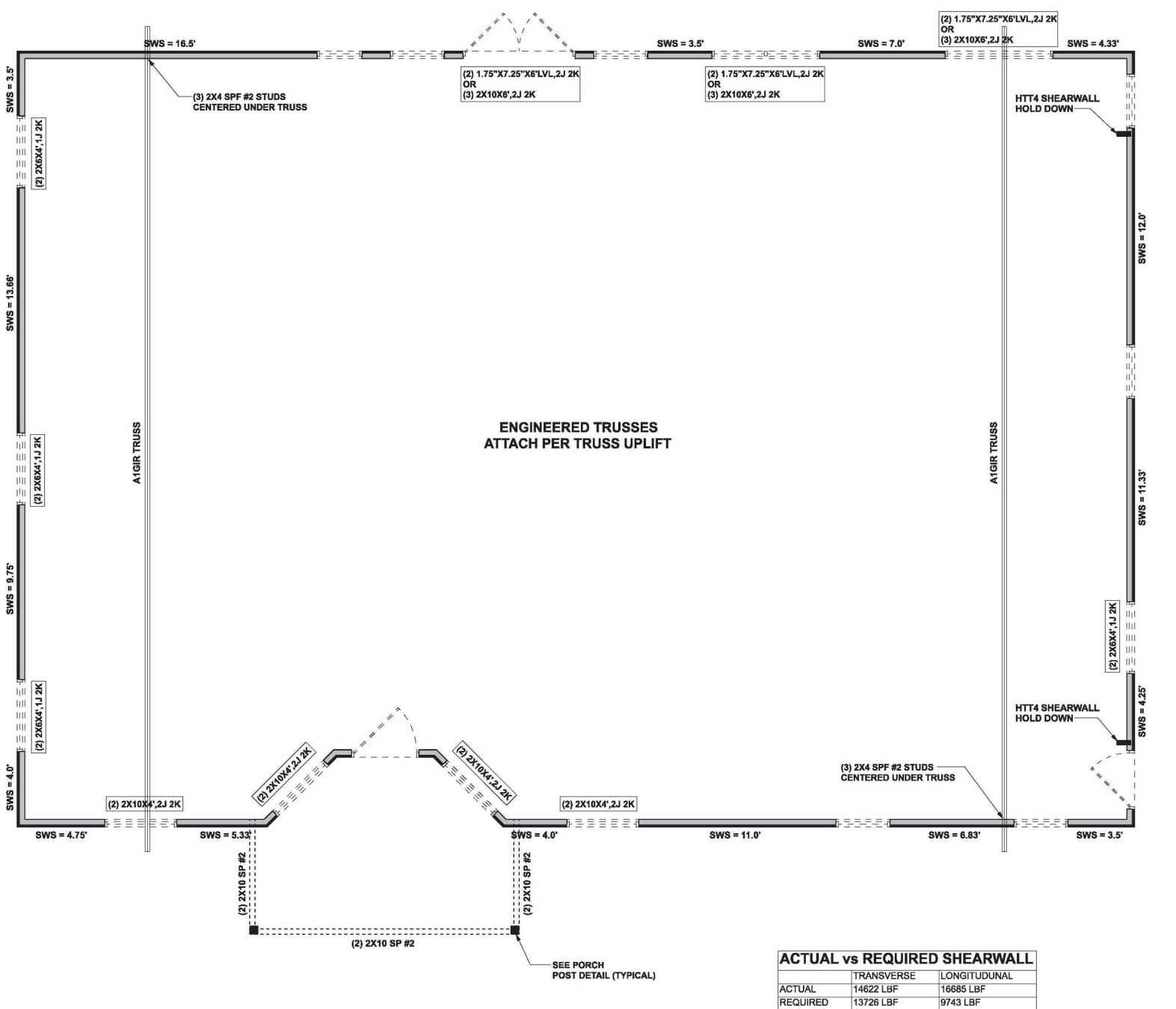
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> JOB NUMBER: 211063 **S-2**

> > OF 3 SHEETS





### STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

### STRUCTURAL PLAN NOTES

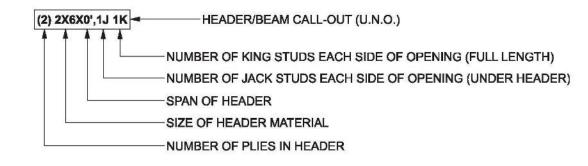
SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

UNLESS NOTED OTHERWISE (MINIMUM REQUIERMENTS) ***SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS***			
BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)		
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)		
HEADERS (STRAPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)		
JACK STUDS UNDER GIRDER TRUS	S USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD		

# HEADER LEGEND



CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. MAYO TRUSS CO. JOB #0821-027 Mervin & Kathleen Dale Res.

PROJECT ADDRESS:
3645 SW Herlong Street
Fort White, FL 32038

DIMENSIONS:
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JOB NUMBER: 211063 **S-3** 

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