BUILDING CODE: FBC 2020(7TH EDITION)

CONCRETE NOTES:

1. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 318,'BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE' AND ACI 301,' SPECIFICATION FOR STRUCTURAL CONCRETE BUILDINGS'

2. CONCRETE MATERIAL SHALL COMPLY WITH:
   A.CEMENT-ASTM C150 TYPE I
   B.AGGREGATE-ASTM C33 MAXIMUM AGGREGATE SIZE 3/4'
   C.WATER- POTABLE
   D.AIR ENTRAINING ADMIXTURE-ASTM C260
   E.WATER-REDUCING ADMIXTURE-ASTM C494,INCLUDING SUPER PLASTICIZERS
   F. FLY ASH- ASTMC618, CLASS C

3. CONCRETE SHALL DEVELOP THE MAXIMUM 28 DAY COMPRESSIVE STRENGTH(fc),(EXCEPT AS NOTED ON DRAWINGS)

TYPE OF CONSTRUCTION	COMPRESSIVE STRENGTH(fc)
A.FOOTINGS,FOUNDATIONS,WALLS	3000PSI
B.INTERIOR SLABS	3000PSI
C. EXTERIOR SLABS,WALLS AND CURBS(AIR ENTRAINED CONCRETE)	3000PSI

   CONCRETE PROPERTIES SHALL BE ESTABLISHS ON THE BASIS OF FIELD EXPERIENCE AND/OR TRIALMIXTURE IN ACCORDANCE WITH ACI-318-11 SECTIONS 5.2 AND 5.3 WHEN FLY ASH UTILIZED IN THE MIX. MIX SHALL CONTAIN A WATER REDUCER. FLY ASH SHALL BE ADDED AT THE RATE OF NOT MORE THAN 100 POUNDS PER CUBIC YARD AND CEMENT SHALL BE REDUCED BY NOT MORE THAN 15 PERCENT BY WEIGHT.

4. PRORPORTION AND DESIGN MIXES TO RESULT IN CONCRETE SLUMP AT POINT OF PLACEMENT OF NOT MORE THAN 4 INCHES.

5. USE OF AIR ENTRAINING ADMIXTURE IN EXTERIOR EXPOSED CONCRETE TO 7 PERCENT ENTRAINED AIR.

6. READY MIX CONCRETE SHALL COMPLY WITH THE REQUIREMENTS OF ASTM C94

7. SAMPLINGSS & TESTING FOR QUALITY CONTROL DURING THE PLACEMENT OF CONCRETE SHALL INCLUDE THE FOLLOWING, WITH ALL SAMPLING AND TESTING BEING DONE AT A COMMERCIAL TESTING LABORATOTY. THE TESTING DOES NOT RELIEVE CONTRACTOR OF RESPONSIBILITY OF PROVIDING CONCRETE IN COMPLIANCE WITH SPECIFICATIONS:
   A.SAMPLING-ASTM C172
   B.SLUMP-ASTM C143, ONE TEST FOR EACH SET OF COMPRESSIVE STRENGTH TEST SPECIMENS.
   C.COMPRESSIVE TEST SPECIMEN-ASTM C31, ONE SET OF 4 CYLINDERS FOR EACH 50 CUBIC YARD OR FRACTION THEREOF WITH A MINIMUM OF ONE SET FOR EACH DAYS PLACEMENT.
   D.AIR CONTENT- ASTM C231,ONE TEST FOR EACH SET OF COMPRESSIVE STRENGTH SPECIMEN.
   E. COMPRESSIVE STRENGTH TEST-ASTM C39, TEST ONE SPECIMEN AT 7 DAYS AND TWO SPECIMENS AT 28 DAYS AND ONE SPECIMEN RETAINED IN RESERVE FOR LATER TESTING IF REQUIRED. TEST RESULTS SHALL BE REPORTED IN WRITING TO ARCHITECT,ENGINEER,CONTRACTOR AND CONCRETE PRODUCER ON THE SAME DAY TEST ARE MADE.

8. NO ALUMINIUM ITEMS SHALL BE EMBEDED IN ANY CONCRETE CHLORIDES IN ANY FORM OR CONCENTRATION SHALL NOT BE ADDED TO ANY CONCRETE.

9. REINFORCING STEEL:
   A. REINFORCING BARS-ASTM A615 GRADE 60, EXCEPT TIES & STIRRUPS MAY BE GRADE 40 DEFORMED.
   B. WELDED WIRE FABRIC-ASTM A 185
   C. ACCESSORIES AND SUPPORTS FOR REINFORCEMENT- COMPLY WITH CRSI RECOMMENDATIONS.
   D. BARS MARKED CONTINOUS AND ALL VERTICAL STEEL SHALL BE LAPPED OR EMBEDDED TO DEVELOP THE FULL TENSILE CAPACITY OF THE BAR. LAPS SHALL BE CLASS'B' UNLESS SHOWN OTHERWISE SPLICE TOP BARS NEAR MIDSPAN AND SPLICE BOTTOM BARS OVER SUPPORTS.

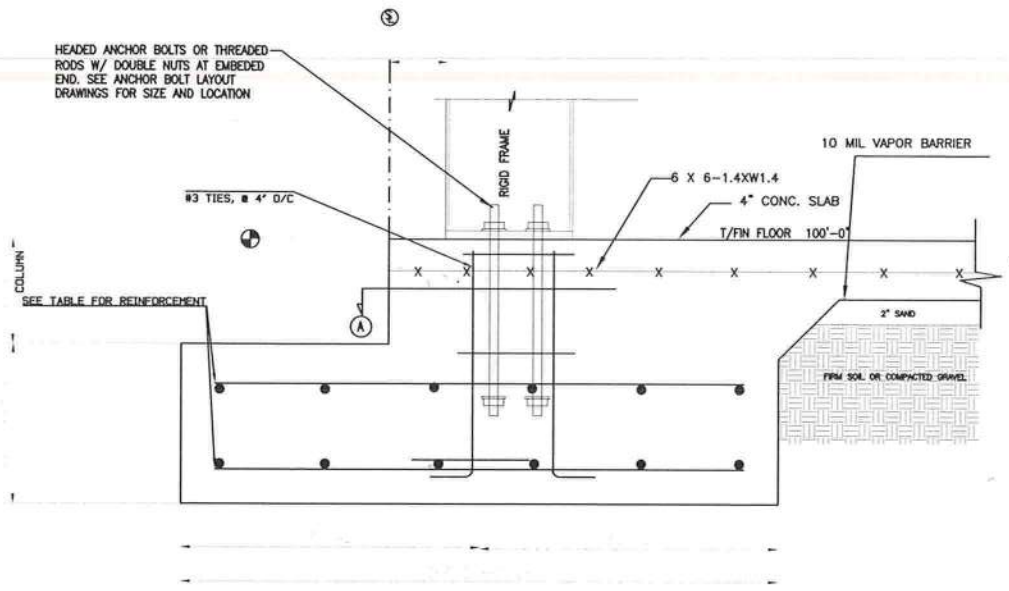
0. CONCRETE WORK EXECUTION:
   A.CONSTRUCT FORM TO CORRECT SIZE,SHAPE, ALIGNMENT,ELEVATION AND POSITION, AND SUPPORT VERTICAL AND LATERAL LOADS.
   B.POSITION,SUPPORT AND SECURE REINFORCEMENT AGAINST PLACEMENT.
   MINIMUM CONCRETE COVER FOR REINFORCEMENT SHALL BE UNLESS NOTED OTHERWISE ON THE DRAWINGS

CAST AGAINST & EXPOSED EARTH	3 INCHES
EXPOSED TO EARTH OR WATER	2 INCHES
NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH	1/2 INCHES

   C.PLACE CONCRETE IN COMPLIANCE WITH ACI 304'RECOMMEND PRACTICE FOR MEASURING,MIXING,TRANSPORTING AND PLACING CONCRETE'
   D.CONSOLIDATE PLACED CONCRETE USING MECHANICAL VIBRATING EQUIPMENT SO THAT CONCRETE IS WORKED AROUND REINFORCEMENTS,EMBEDE ITEMS AND IN TO FORMS.
   E.ALL CONCRETE IS REINFORCED UNLESS SPECIFICALLY NOTED AS UNREINFORCED. REINFORCED CONCRETE NOT OTHERWISE INDICATED WITH THE SAME REINFORCEMENT AS OTHER SIMILAR SECTIONS.
   F.PROTECT CONCRETE WORK FROM PHYSICAL DAMAGE OR REDUCED STRENGTH DUE TO WEATHER EXTREMES, IN COLD WEATHER. COMPLY WITH ACI 306; IN HOT WEATHER COMPLY WITH ACI 305.
   G.IN CONCERNS OF GRADE BEAMS AND WALLS , PROVIDE CORNER REINFORCEMENT (LAP TWO FEET EACH DIRECTION) IN COURSE FACE, MATCHING SIZE AND SPACING OF HORIZONTAL REINFORCEMENT. PROVIDE THREE # 4 SUPORT BARS WHERE REQUIRED FOR SUPPORT OR CORNER BARS.
   H. AT OPENINGS IN WALLS ADD ONE #5 BAR(OPENING DIMENSION PLUS 60 BAR DIAMETERS) FOR EACH FACE AND EACH SIDE OF OPENING AND ONE #5 BAR BY 5'-0' DIAGONALLY FOR EACH FACE AND EACH CORNER OF OPENING.
   I.PROVIDE ONE #5 BAR DIAGONALLY AT EACH FACE OF ALL STEPS IN GRADE BEAMS AND FOUNDATION WALLS.
   J.LAP REINFORCEMENTS AS FOLLOWS, EXCEPT AS NOTED OTHERWISE ON THE DRAWING: #4 BARS-12',#5 BARS-15',#6 BARS-19',#6 BARS-26'.
   K.PROVIDE CONSTRUCTION JOINTS IN FOOTINGS, GRADE BEAMS AND WALLS AS SHOWN ON DWG'S.
   L.STEEL TROWEL FINISH ALL INTERIOR CONCRETE SLABS, BROOM FINISH ALL EXTERIOR CONCRETE SLABS.
   M. REMOVE FINS AND PROJECTIONS, REPAIR AND PATCH DEFECTIVE AREAS WITH CEMENT GROUT IMMEDIATELY AFTER REMOVAL.

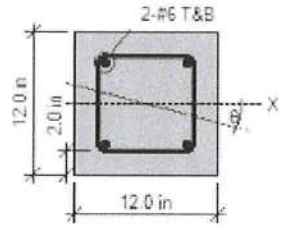
FOUNDATION GENERAL NOTES:

1. THE ALLOWABLE SOIL BEARING CAPACITY IS ASSUMED TO BE A MINIMUM OF 1500 PSF, THE DESIGN ENGINEER SHALL BE NOTIFIED IMMEDIATELY IF ANY CONDITIONS ARE DIFFERENT THAN THESE ASSUMED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE SOIL CONDITIOINS BEFORE CONSTRUCTION, AND IF UNUSUAL CONDITIONS ARE ENCOUNTERED, NOTIFY THE ENGINEER BEFORE CONSTRUCTION BEGINS, THE CONTRACTOR SHALL NOT PROCEED WITH THE WORK IF GROUNDWATER IS ENCOUNTERED DURING EXCAVATION, AND SHALL NOTIFY THE GEOTECHNICAL ENGINEER IMMEDIATELY.
2. COMPACTED BACKFIL SHALL BE USED IN THE EXCAVATION AFTER THE CONCRETE IS SET.
3. REFER TO METAL BUILDING MANUFATURER'S ANCHOR BOLT PLAN FOR ANCHOR BOLTSIZES AND PLACEMENT. ALL BOLTS AT COLUMNS SHALL BE HEADED BOLTS OR THREADED RODS WITH DOUBLE NUTS ATTACHED AT THE END.
4. CONCRETE MONOLITHIC POUR IS ACCEPTABLE.
5. DUE TO LACK OF SPECIFIC GEO-TECHNICAL INFORMATION, SLAB HAS BEEN DESIGNED USING SUBGRADE MODULUS OF 120 and LIVE LOAD OF 250 PSF. SEALING ENGINEER IS NOT RESPONSIBLE FOR DIFFERENTIAL SETTLEMENT, CRACKING, OR OTHER FUTURE DEFECTS RESULTING FROM UNREPORTED CONDITIONS MITIGATING THE ABOVE ASSUMPTIONS



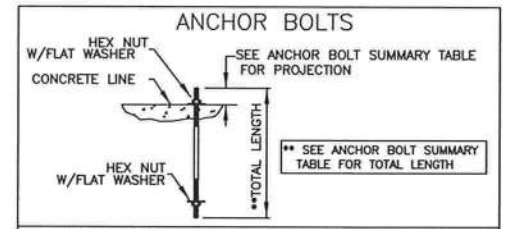
TYP. SECTION THRU EXTERIOR ENDWALL/ SIDEWALL COL. FOOTING

N.T.S.



FOR F1/F2

SECTION A-A



ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Length. (in)	Proj (in)
16	EW COL	3/4"	A307	12.0	1.5
24	RF COL	3/4"	A307	18.0	2.5

Columbia County Building Department
Reviewed for
File Copy
Code Compliance
Plans Examiner

ASHOKBHAI A. PATEL
LICENSE
No. 73663
STATE OF FLORIDA
PROFESSIONAL ENGINEER
05-18-2023

ISSUE	DET	CHK	DATE

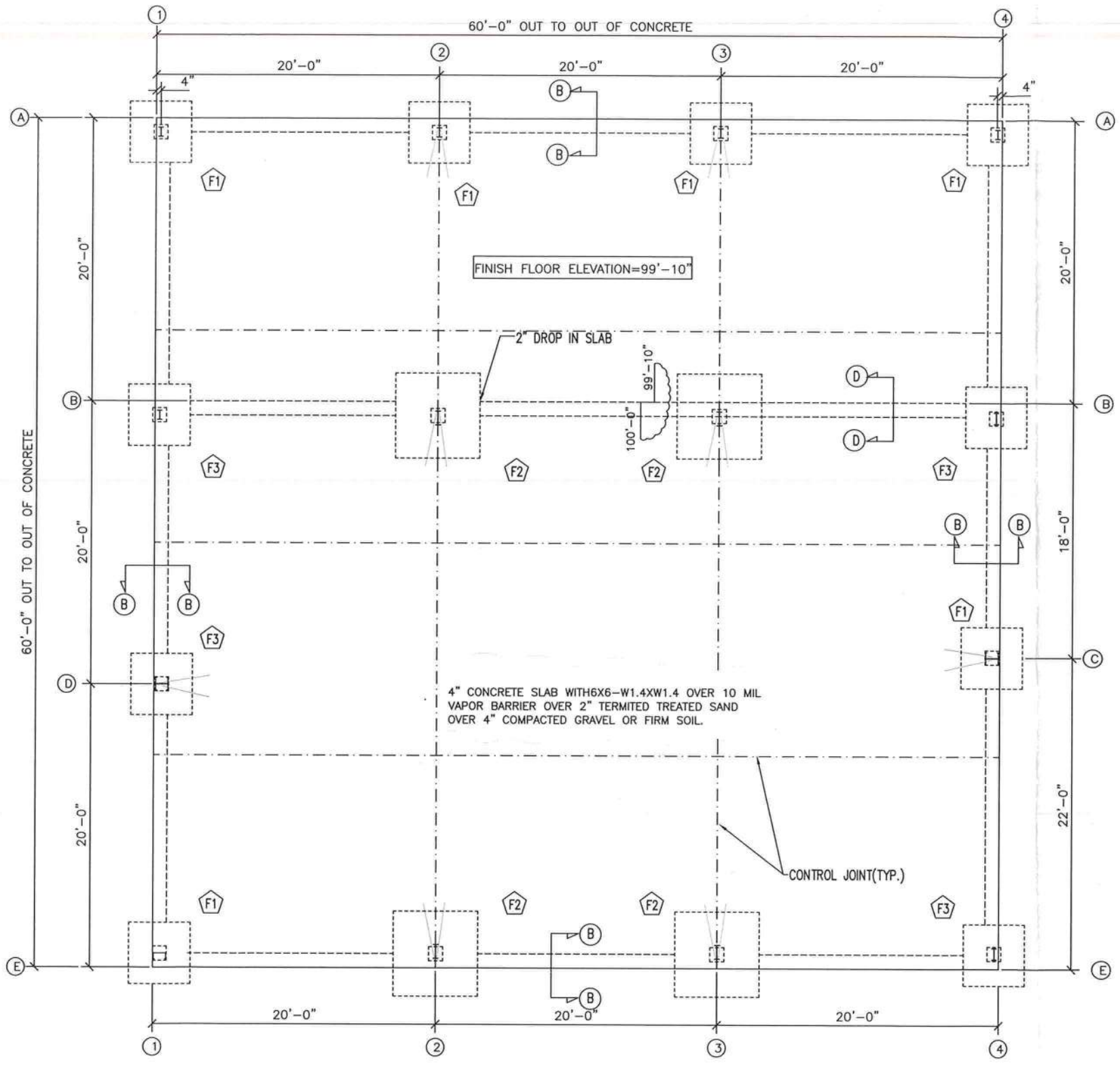
BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION

JOB NO: 8065 DATE: 5/ 18/23

LOCATION: HIGH SPRINGS, FL 32643

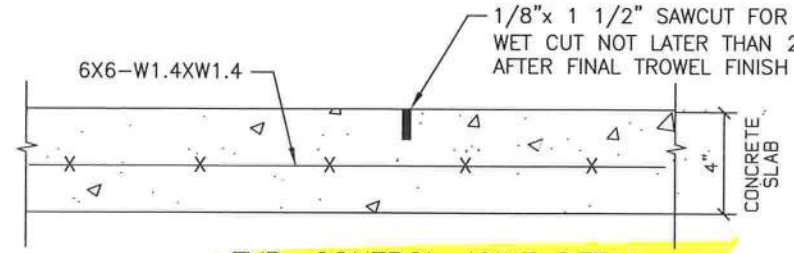
DRAWING NAME: FOUNDATION PLAN-2



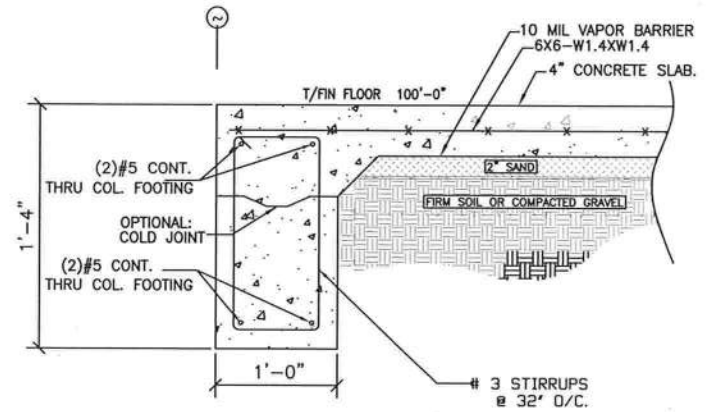
FOUNDATION BOLT PLAN

*NOTE:REFER ANCHOR BOLT DRAWINGS FOR BOLT PLACEMENT AND COLUMN LOCATION

NOTE: ALL BASE PLATES @ 100.00' (U.N.) FINISH FLOOR @ 100.00' (U.N.)

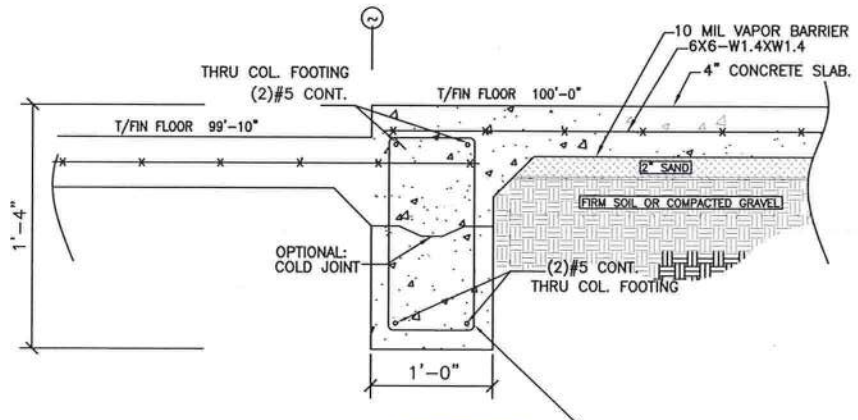


TYP. CONTROL JOINT DETAIL



SECTION "B-B"(GRADE BEAM)

N.T.S.

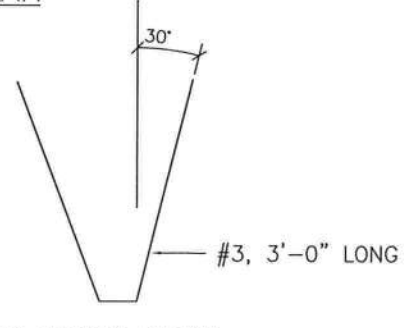


SECTION "D-D"(GRADE BEAM)

N.T.S.

FOOTING SCHEDLUE

MARK#	QTY.	SIZE	REINFORCEMENT
F1	6	4'-4"X4'-4"X1'-0" MIN.	(6) #3 TOP/BOTTOM EACH WAY
F2	4	6'-0"X6'-0"X1'-0 MIN.	(8) #3 TOP/BOTTOM EACH WAY



TYP HAIRPIN DETAIL

N.T.S.

Ashokbhai A Patel

Ashokbhai A Patel
Valdosta
(229)402-2640
2023.05.18
15:27:36-04'00'

ASHOKBHAI A. PATEL LICENSE No. 73653 STATE OF FLORIDA PROFESSIONAL ENGINEER

05-18-2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION

JOB NO: 8065

DATE: 5/ 18/23

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: FOUNDATION PLAN-1

BUILDING PROFILE

A	Width (ft) = 40	Eave Height (ft) =	18
	Length (ft) = 60	Roof Slope (Rise/12) =	2.0:12
B	Width (ft) = 20	Eave Height (ft) =	18 H/S
	Length (ft) = 60	Roof Slope (Rise/12) =	2.0:12

BUILDING LOADS

A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 20 / 7TH EDITION

B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE LOCAL BUILDING DEPT.

OCCUPANCY/RISK CATEGORY: II – Normal Ie: 1.00

WIND LOAD: ULTIMATE 122 MPH NOMINAL 94.50 MPH WIND EXPOSURE B

CLOSURE TYPE: Enclosed

INTERNAL WIND COEFFICIENT: −0.18 / 0.18

COLLATERAL DEAD LOAD: 1 PSF

ROOF LIVE LOAD: 20.00 PSF (REDUCIBLE Yes)

DEAD LOAD: 2.000 PSF (FOR ROOF PANELS AND PURLINS)

SEISMIC

SPECTRAL RESPONSE Ss 0.0762 S1 0.0471 Sds 0.0832 Sd1 0.0752

SITE CLASS d DESIGN RISK CATEGORY B Cs 0.0277

RESPONSE MODIFICATION FACTOR, R 3.00* FRAMES 3.00* BRACING

BASIC SEISMIC FORCE RESISTING SYSTEM (LATERAL DIRECTIONS)	= ORDINARY STEEL MOMENT FRAMES
BASIC SEISMIC FORCE RESISTING SYSTEM (BLDG. A LEW)	= ORDINARY STEEL CONC. BRACED FRAMES
BASIC SEISMIC FORCE RESISTING SYSTEM (BLDG. A REW)	= ORDINARY STEEL MOMENT FRAMES
BASIC SEISMIC FORCE RESISTING SYSTEM (BLDG. B)	= ORDINARY STEEL MOMENT FRAMES
BASIC SEISMIC FORCE RESISTING SYSTEM (LONGITUDINAL DIRECTIONS)	= ORDINARY STEEL CONC. BRACED FRAMES
ANALYSIS PROCEDURE	= EQUIVALENT LATERAL FORCE PROCEDURE

* STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

SERVICEABILITY CRITERIA

MINIMUM DESIGN DEFLECTIONS			
Endwall Column	= 120	Wall Panel	= 60
Endwall Rafter (Live)	= 180	Roof Panel (Live)	= 60
Endwall Rafter (Wind)	= 180	Roof Panel (Wind)	= 60
Wall Girt	= 90	Rigid Frame (Horz)	= 60
Roof Purlin (Live)	= 150	Rigid Frame (Vert)	= 180
Roof Purlin (Wind)	= 150	Rigid Frame (Seismic)	= 50

GENERAL NOTES

A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER. THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.

B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.

1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: " AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS– ALLOWABLE STRESS DESIGN"
2. AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS"
3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1.
4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL"

C) 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER PRIMARY STRUCTURAL EXCLUSIVE OF COLD–FORMED SECTIONS, CONFORM TO ASTM–A529 OR A572 . FLANGES WITH THICKNESS OF ONE INCH OR LESS AND WIDTH OF 12" OR LESS CONFORM TO A529 WITH A MINIMUM YIELD POINT OF 55,000 psi. FLANGES GREATER THAN 1" IN THICKNESS OR 12" IN WIDTH CONFORM TO A572 WITH A MINIMUM YIELD POINT OF 50,000 psi. WEB MATERIAL CONFORMS TO ASTM–A529 WITH A MINIMUM YIELD POINT OF 55,000 psi.
2) MATERIAL PROPERTIES OF PIPE SECTIONS CONFORM TO ASTM–A500, GRADE B WITH A MINIMUM YIELD POINT OF 42,000 psi.
3) MATERIAL PROPERTIES OF TUBE SECTIONS CONFORM TO ASTM–A500, GRADE B WITH A MINIMUM YIELD POINT OF 46,000 psi.
4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM–A529 WITHMINIMUM YIELD POINT OF 50,000 PSI. HOT ROLLED W–SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM–A992WITH MINIMUM YIELD POINT OF 50,000 PSI.
5) MATERIAL PROPERTIES OF COLD FORMED LIGHT GAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653–06 GR 55 OR A1011–04 HSLAS GRADE 55 WITH YIELD OF 55,000 psi.
6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM–A792 GRADES 80 CLASS 1, 2 OR 3 WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM–ZINC ALLOY IN ACCORDANCE WITH AZ55 SPECIFICATIONS.
7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT CONDITION.
8) ROD UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM–A36 WITH MINIMUM YIELD POINT OF 36,000 PSI.

9) IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE "RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A–325 OR A–490 BOLTS". ALL A–325 BOLTS IN PRIMARY FRAMING MUST BE "SNUG–TIGHT", EXCEPT AS FOLLOWS:
"FULLY–PRETENSION" A–325 BOLTS IF:
a) BUILDING LOCATED IN A HIGH SEISMIC AREA. FOR IBC–BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E" OR "F".
b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS.
c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS – REVERSALS ON THE CONNECTIONS.
d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A–325 – SC".

10) SECONDARY MEMBERS AND FLANGE BRACE CONNECTIONS SHALL ALWAYS BE SNUG TIGHT, UNO.

11) ANCHOR BOLTS 3/4" IN DIAMETER THRU 1 1/4" IN DIAMETER CONFORM TO A.S.T.M. F1554 GR. 36. ANCHOR BOLTS 1/2" IN DIAMETER CONFORM TO A.S.T.M. A–307.

D) UNLESS NOTED OTHERWISE ON FRAMING COLOR CHART: ALL STEEL MEMBERS EXCEPT BOLTS, FASTENERS, CABLE AND RODS SHALL RECEIVE ONE COAT OF STANDARD RED OXIDE SHOP PRIMER.

E) SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT.

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS:

A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS:
1) BE MADE IN CONTRASTING INK.
2) HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED.
3) BE LEGIBLE AND UNAMBIGUOUS.

B) DATED SIGNATURE IS REQUIRED ON ALL PAGES.

C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE.

D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY MANUFACTURER.

E) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION. MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

SAFETY COMMITMENT

A) THE BUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER.

B) IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE.

C) LOCAL STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY.

D) MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.

E) DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.

ERECTOR / CONTRACTOR RESPONSIBILITIES

A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.

B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS REQUIRED.

C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, 13TH ED.)

D) WHERE DISCREPANCIES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE 13TH ED.)

E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACTURER'S ENGINEERS UNLESS SPECIFICALLY INDICATED.

F) THE ERECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH THE BUILDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS.

G) PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR IMMEDIATELY UPON ARRIVAL. CLAIMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE MANUFACTURER IN WRITING WITHIN FIVE (5) DAYS AFTER RECEIPT OF THE SHIPMENT. HOWEVER, IF A DEFECT IS OF SUCH A NATURE THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THEN THE CLAIM MUST BE MADE WITHIN FIVE (5) DAYS AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNLESS CLAIM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHIPMENT BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER. THE MANUFACTURER WILL BE GIVEN A REASONABLE OPPORTUNITY TO INSPECT DEFECTIVE MATERIALS UPON RECEIPT OF CLAIM BY CONTRACTOR.
IF A DEFECT IS OF SUCH NATURE THAT IT CAN BE REMEDIED BY A FIELD OPERATION AT THE JOB SITE WITHOUT THE NECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR MAY REPAIR OR CAUSE THE MATERIAL TO BE REPAIRED AND THE MANUFACTURER WILL REIMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION.

THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING,CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.

H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.

I) TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION. (SECT. 7.10.3 AISC CODE OF STANDARD PRACTICE, 13TH ED.)

J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN,MATERIAL AND WORKMANSHIP OF FOUNDATION. ANCHOR BOLT PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION. IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT, BEARING VALUES, TIE RODS AND OTHER ASSOCIATED ITEMS EMBEDDED IN THE CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3)

K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION.

IAS ACCREDITED Metal Building Systems AC 472

BUILDING DESIGNED & MANUFACTURED BY AN IAS ACCREDITED FACILITY.

FLORIDA PRODUCT APPROVAL NUMBER	
PBR ROOF PANEL	36875.1
PBR WALL PANEL	36876.1

IT IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NOT PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE. ALL ACCESSORIES MUST COMPLY AND MEET ALL DESIGN REQUIREMENTS PER LOCAL CODES.

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

FRAMING COLORS

Rigid Frame: GP
Flange brace: GP
Angle: GP

RO – Red Oxide
GP – Grey Primer
GZ – Galvanized

	Grt	Pur	ExSt	Jmb	BB	Endwall Col	Endwall Raf
U SECTION:	GP	GP	GP	GP	GP	GP	GP
C SECTION:	GP	GP	GP	GP	GP	GP	GP
D SECTION:	GP	GP	GP	GP	GP	GP	GP
Z SECTION:	GP	GP	GP	GP	GP	GP	GP
E SECTION:	GP	GP	GP	GP	GP	GP	GP
R SECTION:	GP	GP	GP	GP	GP	GP	GP
W SECTION:	GP	GP	GP	GP	GP	GP	GP

WHEN GALVANIZED PROVIDED: ALL FINISHED PRIMARY BUILT–UP AND HOT ROLL MEMBERS ARE HOT DIPPED GALVANIZED. ALL SECONDARY COLD FORMED MEMBERS ARE PRE–GALVANIZED.

BUILDINGS AND MORE

DRAWING INDEX

REV.	PAGE	DESCRIPTION
	0	COVER PAGE
	1	ANCHOR BOLT LAYOUT
	1.1	ANCHOR BOLT DETAILS
	1.2–1.3	ANCHOR BOLT REACTIONS
	2	ROOF FRAMING LAYOUT
	2.1–2.3	RIGID FRAME CROSS SECTION
	3–3.1	ENDWALL FRAMING LAYOUT
	4–4.2	SIDEWALL FRAMING LAYOUT
	5–5.6	FRAMING DETAILS
	6	ROOF PANELS & TRIM
	6.1	ROOF PANEL DETAILS
	7	SIDEWALL PANEL DETAILS
	8	ENDWALL PANEL DETAILS
	9	SPECIAL DETAILS

THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING. ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF REFERENCED BUILDING CODE. 3

FOR OCCUPANCY (RISK), CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE–STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS OR EXTERIOR SYSTEMS NOT PROVIDED BY MBM SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMODATE THE SEISMIC STORY DRIFTS. 4

1.0 PSF COLL ONLY ALLOW LIGHTING AND HVAC DUCT TO HANG FROM ROOF SYSTEMS SUSPENSION OF ANY LOAD INDUCING SYSTEM IS EXPLICITLY PROHIBITED, UNLESS A CORRESPONDING REDUCTION IN CERTIFIED LIVE/SNOW LOADS CAN BE PERMITTED BY CODE. BM

A | COMPONENTS & CLADDING (unfactored)

Wall Field Values	=	20.944 psf	/	−22.689 psf
Wall Edge Values	=	20.944 psf	/	−27.925 psf

B | COMPONENTS & CLADDING (unfactored)

Wall Field Values	=	19.897 psf	/	−21.555 psf
Wall Edge Values	=	19.897 psf	/	−26.529 psf

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

Columbia County Building Department — Reviewed for — File Copy — Code Compliance — Plans Examiner

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

COLORS:	
ROOF:	GALVALUME
WALLS:	POLAR WHITE
GABLE:	BLACK
EAVE:	BLACK
CORNER:	BLACK
FRAMED OPENINGS:	BLACK
GUTTER:	BLACK
DOWNSPOUTS:	BLACK
BASE:	BLACK

DRAWING STATUS

☐ FOR APPROVAL:
THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.

☒ FOR PERMIT:
THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL IN THAT, AS A MINIMUM, PIECE MARKINGS ARE NOT IDENTIFIED. ONLY DRAWINGS ISSUED " FOR CONSTRUCTION " CAN BE CONSIDERED AS COMPLETE.

☐ FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION

ISSUE	DET	CHK	DATE

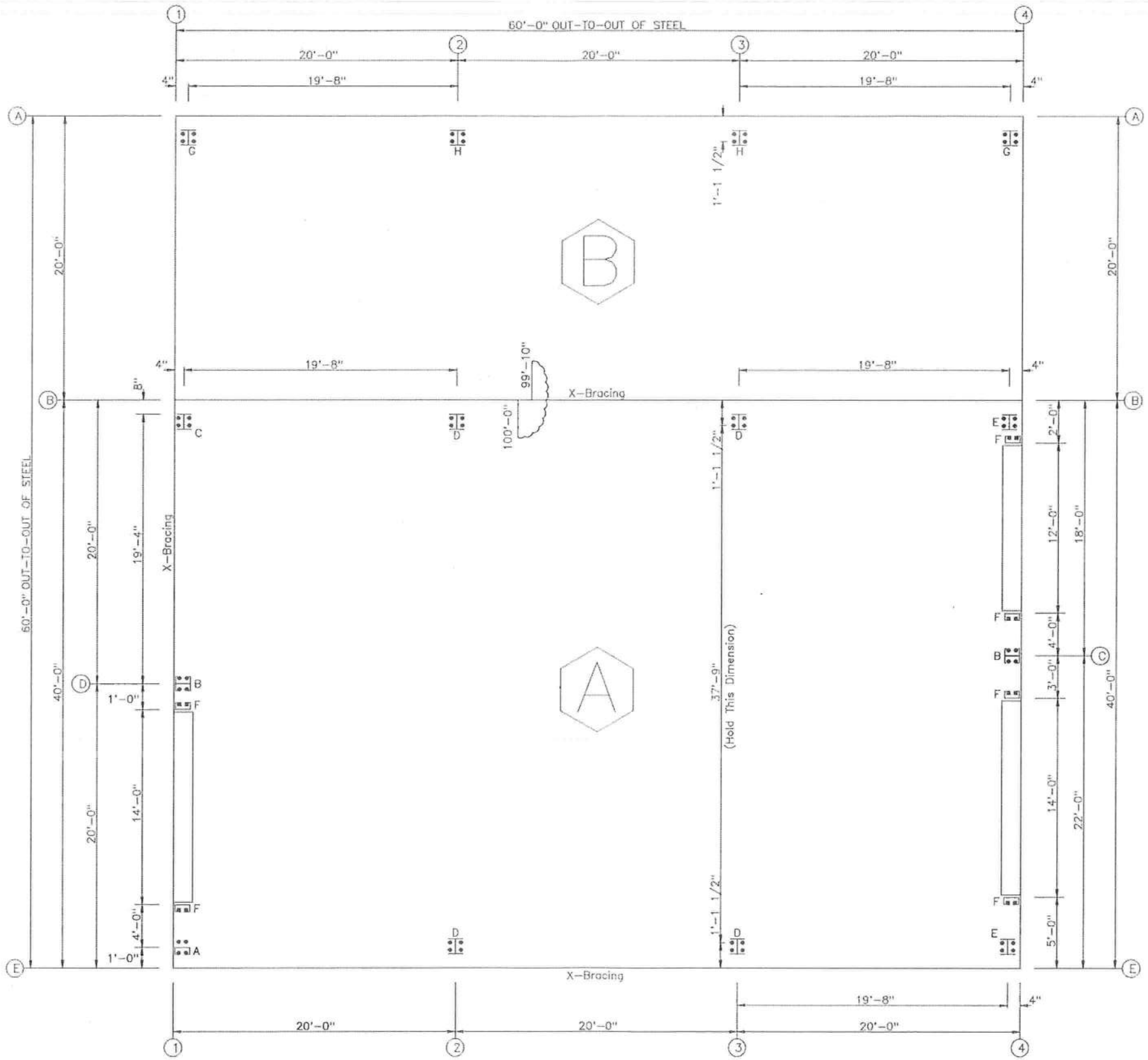
FOR: GIBRALTAR CONSTRUCTION
735 SW STERLING TERRACE
HIGH SPRINGS, FL 32643

JOBSITE: HIGH SPRINGS, FL 32643

FROM: BUILDINGS AND MORE
792 SW BASCOM NORRIS DR.
LAKE CITY, FL 32025

JOB NO : 8065
DATE : 5/ 2/23
BY : JRD
SCALE : NONE
TITLE : COVER PAGE
NUMBER : PAGE 0

⊠ Dia= 5/8"
⊕ Dia= 3/4"



ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (Unless Noted)

NOTE: ALL FIELD LOCATED FRAMED OPENING LOCATIONS SHALL BE AT THE DISCRETION OF THE ERECTOR/CUSTOMER. IT IS RECOMMENDED THAT THESE ANCHORS BE LOCATED AT TIME OF ERECTION.

FIELD LOCATE:
(1) 3'-4" x 7'-2" FRAMED OPENING

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

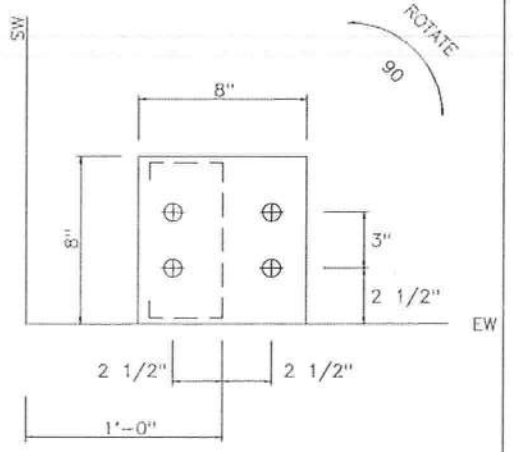
REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

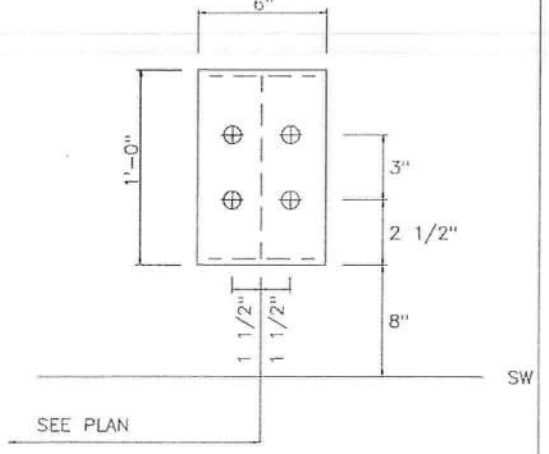
CUSTOMER: GIBRALTAR CONSTRUCTION
JOB NO: 8065 DATE: 5/ 2/23
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: ANCHOR BOLT LAYOUT
DRAWING NO: PAGE 1 DRAWN BY: JRD CHECKED BY: SPW SCALE: NONE

Dia= 3/4"



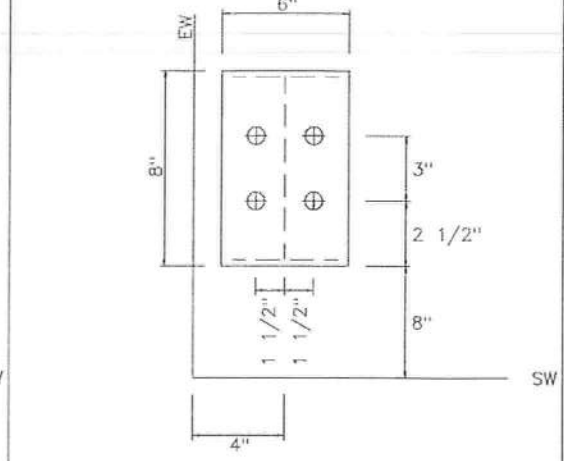
DETAIL A

Dia= 3/4"



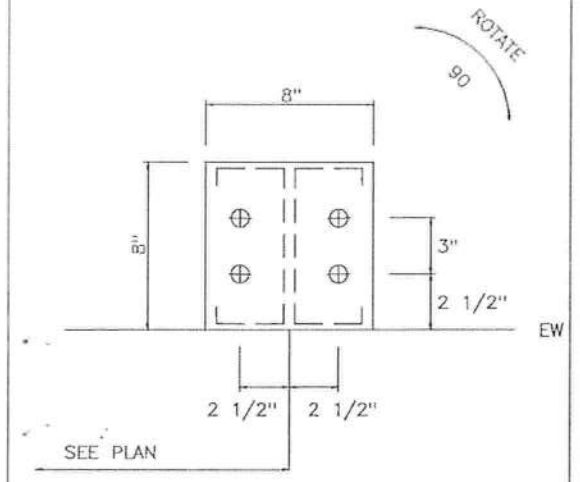
DETAIL D

Dia= 3/4"



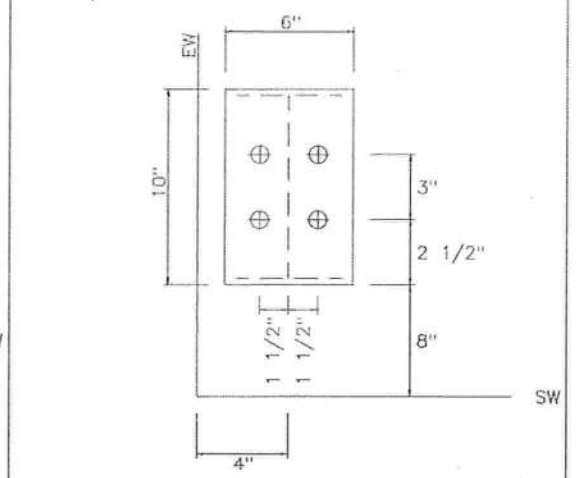
DETAIL G

Dia= 3/4"



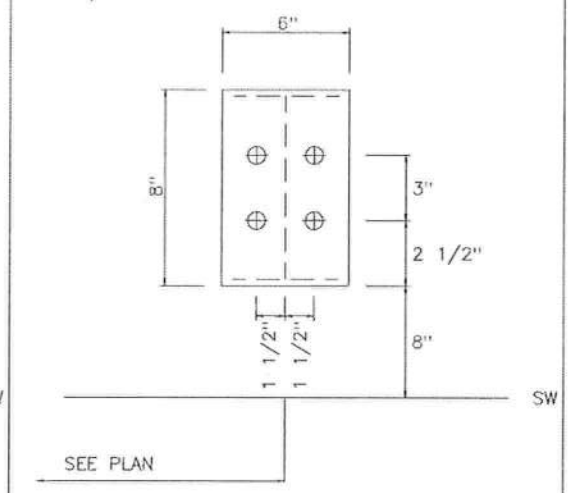
DETAIL B

Dia= 3/4"



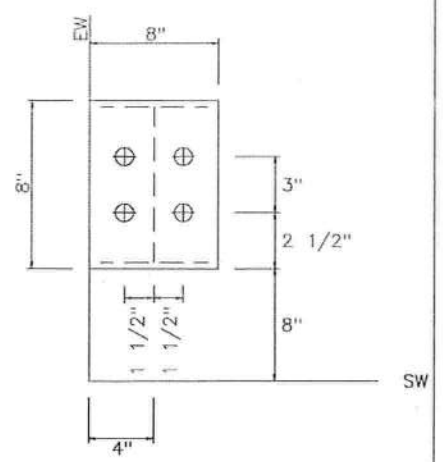
DETAIL E

Dia= 3/4"



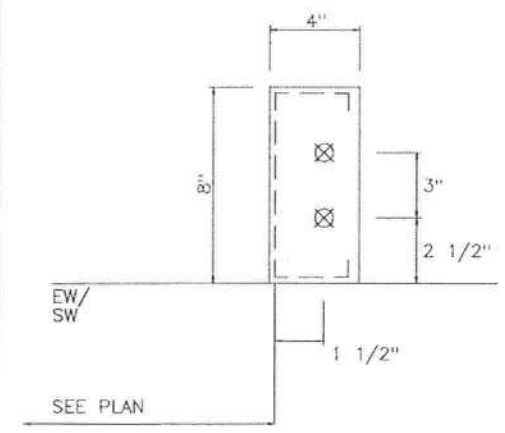
DETAIL H

Dia= 3/4"

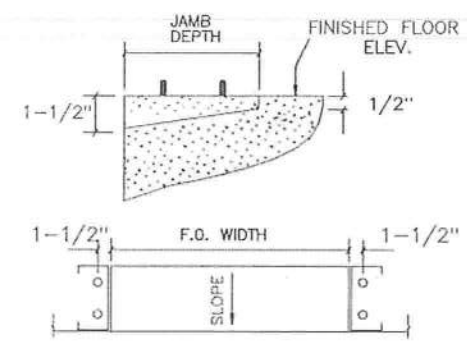


DETAIL C

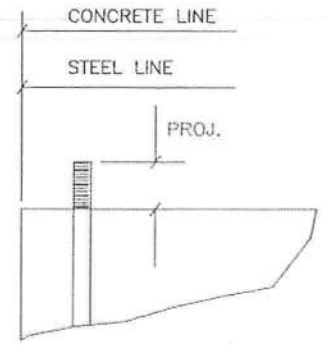
Dia= 5/8"



DETAIL F



TYP. O.H. DOOR RECESS DETAIL



ANCHOR BOLT PROJECTION

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION

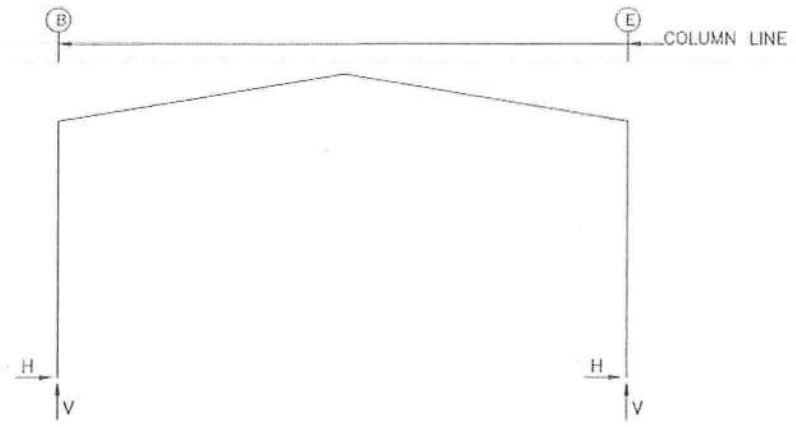
JOB NO: 8065 | DATE: 5/ 2/23

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ANCHOR BOLT DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 1.1	JRD	SPW	NONE

FRAME LINES: 2 3 4



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
2*	B	4	0.750	6.000	12.00	0.375	0.0
2*	E	4	0.750	6.000	12.00	0.375	0.0

2* Frame lines: 2 3

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
4	B	4	0.750	6.000	10.00	0.375	0.0
4	E	4	0.750	6.000	10.00	0.375	0.0

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
1	B	4	0.750	8.000	8.000	0.375	0.0
1	D	4	0.750	8.000	8.000	0.250	0.0
1	E	4	0.750	8.000	8.000	0.250	0.0
4	C	4	0.750	8.000	8.000	0.250	0.0

ANCHOR BOLT SUMMARY

	Qty	Locate	Dia (in)	Type	Projection (in)
⊠	12	Jamb	5/8"	A307	1.50
⊕	16	Endwall	3/4"	GR36	1.50
⊕	24	Frame	3/4"	GR36	2.50

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel Shear (lb/ft) Wind	Seis	Note
L_EW	1	B,D	1.9	2.0	0.1	0.1			
F_SW	E	2,3	3.4	2.7	0.2	0.2			
R_EW	4								(h)
B_SW	B	3,2	6.3	5.1	0.4	0.3			

(h)Rigid frame at endwall

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

1

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert	Wind_Left2 Horiz	Wind_Left2 Vert
2*	B	0.3	1.8	0.1	0.6	1.2	7.9	−4.0	−12.3	3.5	−5.3	−5.6	−8.6
2*	E	−0.3	1.3	−0.1	0.4	−1.2	4.7	−1.9	−4.6	5.6	−9.9	−2.0	−0.5
4	B	0.2	1.1	0.1	0.3	0.8	5.3	−2.6	−7.9	2.0	−3.3	−3.4	−6.0
4	E	−0.2	0.8	−0.1	0.2	−0.8	3.2	−1.0	−2.8	3.6	−6.2	−1.2	−0.7

Frame Line	Column Line	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert	Seismic_Long Horiz	Seismic_Long Vert
2*	B	1.8	−1.7	3.5	−8.3	3.0	−7.2	−0.1	−0.1	0.1	0.1	0.0	0.0
2*	E	5.4	−5.7	1.5	−8.0	1.0	−9.0	−0.1	0.1	0.1	−0.1	0.0	0.0
4	B	1.1	−1.4	1.6	−4.4	1.3	−3.9	−0.1	0.0	0.1	0.0	0.0	0.0
4	E	3.4	−4.1	0.6	−3.9	0.3	−4.4	0.0	0.0	0.0	0.0	0.0	0.0

2* Frame lines: 2 3

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horz	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert	Seis Left Vert
1	B	0.7	0.2	3.7	−4.2	−2.3	−3.0	−0.9	−1.4	1.6	−2.0	−1.1	0.0
1	D	0.8	0.2	4.9	−4.1	−4.3	−2.5	−2.9	−3.2	3.6	−3.8	−3.8	0.0
1	E	0.3	0.1	1.6	−1.2	−1.8	−0.5	−1.1	−1.5	1.6	−1.1	−2.1	0.0
4	C	0.1	0.0	0.0	0.0	0.0	0.0	0.0	−3.3	3.6	0.0	0.0	0.0

Frm Line	Col Line	Seis Right Vert	Seis Long Vert
1	B	0.0	0.0
1	D	0.0	0.0
1	E	0.0	0.0
4	C	0.0	0.0

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	= 40.0
Length (ft)	= 60.0
Eave Height (ft)	= 18.0/ 18.0
Roof Slope (Rise/12)	= 2.0/ 2.0
Dead Load (psf)	= 2.0
Collateral Load (psf)	= 1.0
Roof Live Load(psf)	= 20.0
Frame Live Load	
Min(psf)	= 12.0
Max(psf)	= 16.0
Wind Speed (mph)	= 122.0
Wind Code	= FBC 20 (7th Edition)
Exposure	= B
Enclosed/Open/Partial	= Enclosed
Importance Wind	= 1.00
Importance Seismic	= 1.00
Seismic Zone	= B
Seismic Coeff (Fa*Ss)	= 0.12

GENERAL NOTES

1. FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
2. ALL REACTIONS ARE UNFACTORED.
3. ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/− 1/8" IN BOTH ELEVATION AND LOCATION.
5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

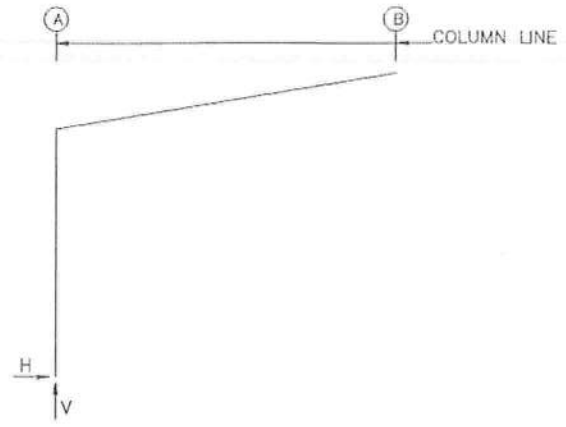
BUILDING A

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION
JOB NO: 8065 DATE: 5/ 2/23
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: ANCHOR BOLT REACTIONS
DRAWING NO: PAGE 1.2 DRAWN BY: JRD CHECKED BY: SPW SCALE: NONE

FRAME LINES: 1 2 3 4



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
1	A	4	0.750	6.000	8.000	0.375	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
2*	A	4	0.750	6.000	8.000	0.375	0.0

2* Frame lines: 2 3

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
4	A	4	0.750	6.000	8.000	0.375	0.0

ANCHOR BOLT SUMMARY

	Qty	Locate	Dia (in)	Type	Projection (in)
⊕	16	Frame	3/4"	GR36	2.50

GENERAL NOTES

1. FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
2. ALL REACTIONS ARE UNFACTORED.
3. ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert	Wind_Left2 Horiz	Wind_Left2 Vert
1	A	0.0	0.5	0.0	0.1	0.0	2.2	-0.6	-2.6	0.9	-1.9	-1.2	-1.6
2*	A	0.0	0.7	0.0	0.2	0.0	3.4	-0.9	-4.1	1.5	-3.1	-2.0	-2.1
4	A	0.0	0.5	0.0	0.1	0.0	2.2	-0.6	-2.6	0.9	-1.9	-1.2	-1.6

Frame Line	Column Line	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert
1	A	0.4	-0.9	1.0	-2.4	1.0	-1.6
2*	A	0.5	-1.2	1.9	-4.5	1.9	-3.2
4	A	0.4	-0.9	1.0	-2.4	1.0	-1.6

2* Frame lines: 2 3

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	= 20.0
Length (ft)	= 60.0
Eave Height (ft)	= 14.7/ 18.0
Roof Slope (Rise/12)	= 2.0
Dead Load (psf)	= 2.0
Collateral Load (psf)	= 1.0
Roof Live Load(psf)	= 20.0
Frame Live Load	
Min(psf)	= 16.0
Max(psf)	= 20.0
Wind Speed (mph)	= 122.0
Wind Code	= FBC 20 (7th Edition)
Exposure	= B
Enclosed/Open/Partial	= Enclosed
Importance Wind	= 1.00
Importance Seismic	= 1.00
Seismic Zone	= B
Seismic Coeff (Fa*Ss)	= 0.12

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel Shear (lb/ft) Wind	Seis	Note
L_EW	1								(h)
F_SW	B								(e)
R_EW	4								(h)
B_SW	A	Torsional Bracing Used							

(e)Bracing loads are applied to supporting building
(h)Rigid frame at endwall

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

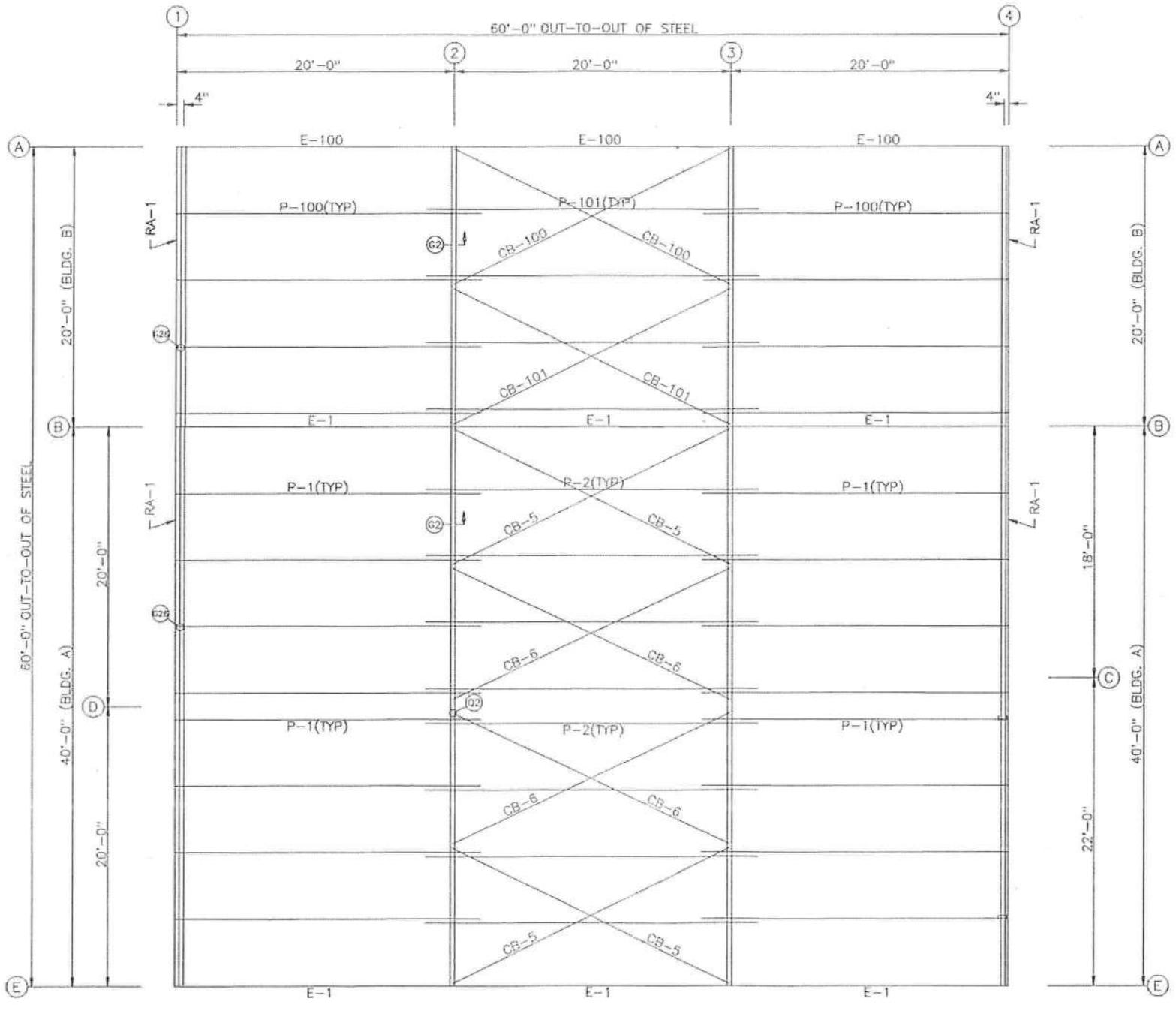
BUILDING B

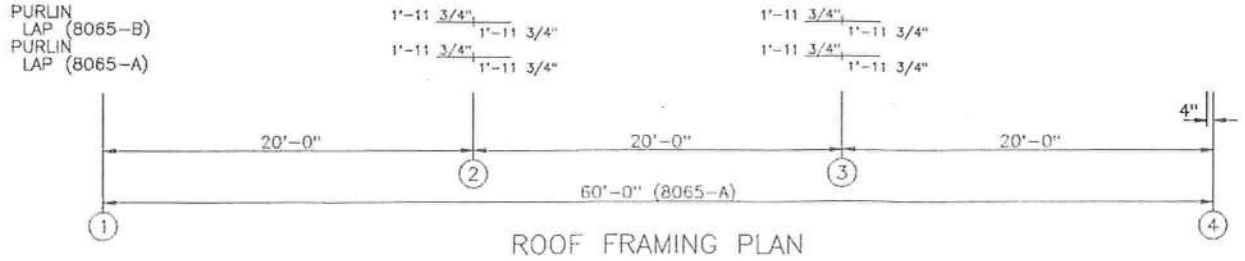
ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION
JOB NO: 8065 DATE: 5/ 2/23
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: ANCHOR BOLT REACTIONS
DRAWING NO: PAGE 1.3 DRAWN BY: JRD CHECKED BY: SPW SCALE: NONE

MEMBER TABLE ROOF PLAN		
MARK	PART	LENGTH
BLDG. A		
P-1	8x25Z16	21'-11 1/2"
P-2	8x25Z16	23'-11 1/2"
E-1	8LE14@2	19'-11 1/2"
CB-5	1/4 CBL	22'-0"
CB-6	1/4 CBL	22'-8"
BLDG. B		
P-100	8x25Z16	21'-11 1/2"
P-101	8x25Z16	23'-11 1/2"
E-100	8LE14@2	19'-11 1/2"
CB-100	1/4 CBL	22'-2"
CB-101	1/4 CBL	22'-8"





ROOF FRAMING PLAN

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION

JOB NO: 8065 DATE: 5/ 2/23

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ROOF FRAMING LAYOUT

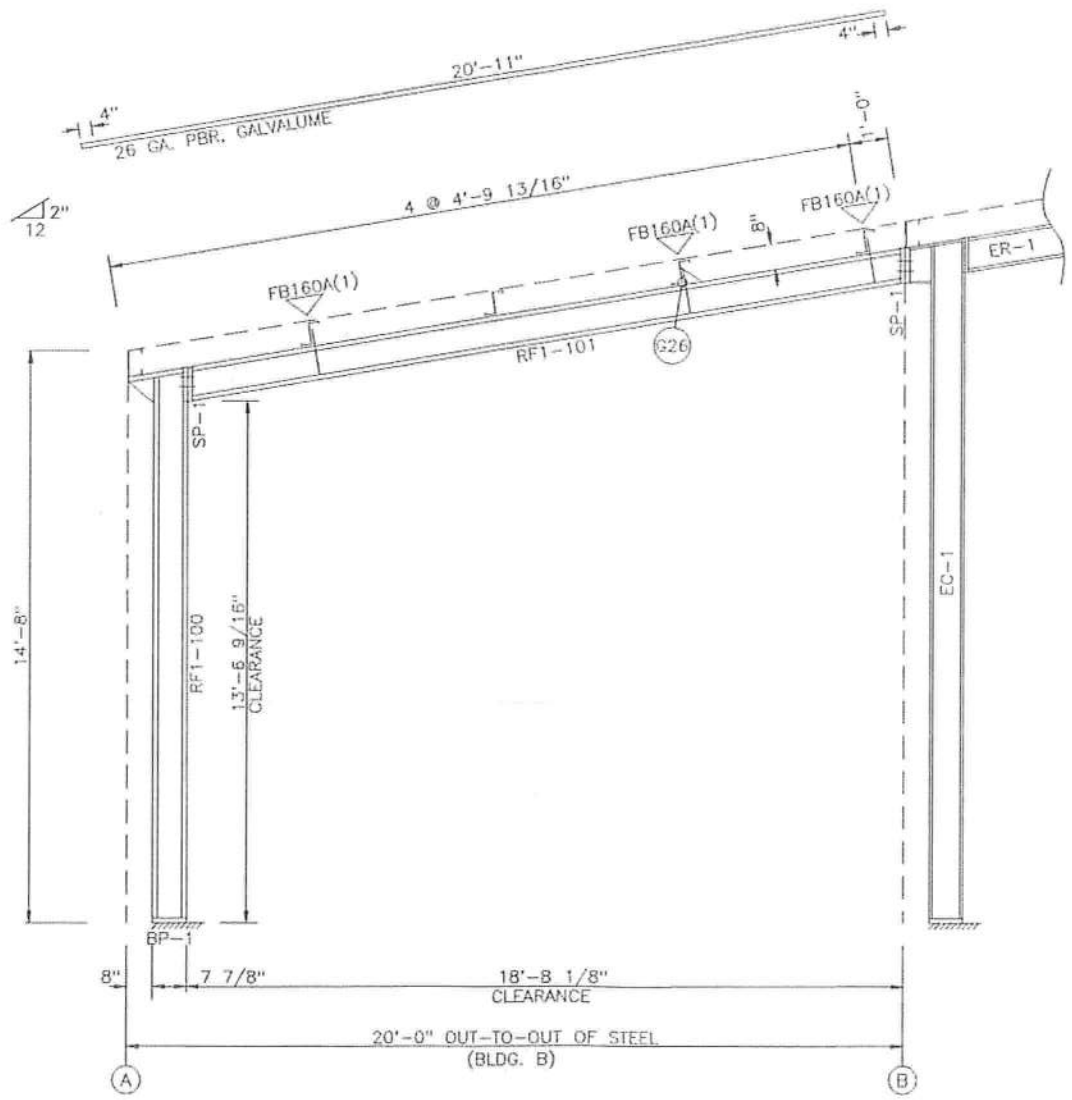
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2	JRD	SPW	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP-1	4	0	0	A325	5/8"	2"

BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP-1	6"	3/8"	8"

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A - L2x2x14

MEMBER SIZE TABLE			
MARK	MEMBER	LENGTH	WEIGHT
RF1-100	W8X10	14'-1 1/4"	156
RF1-101	W8X10	18'-11"	208



RIGID FRAME ELEVATION: FRAME LINE 1

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

6

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION

JOB NO: 8065 DATE: 5/ 2/23

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

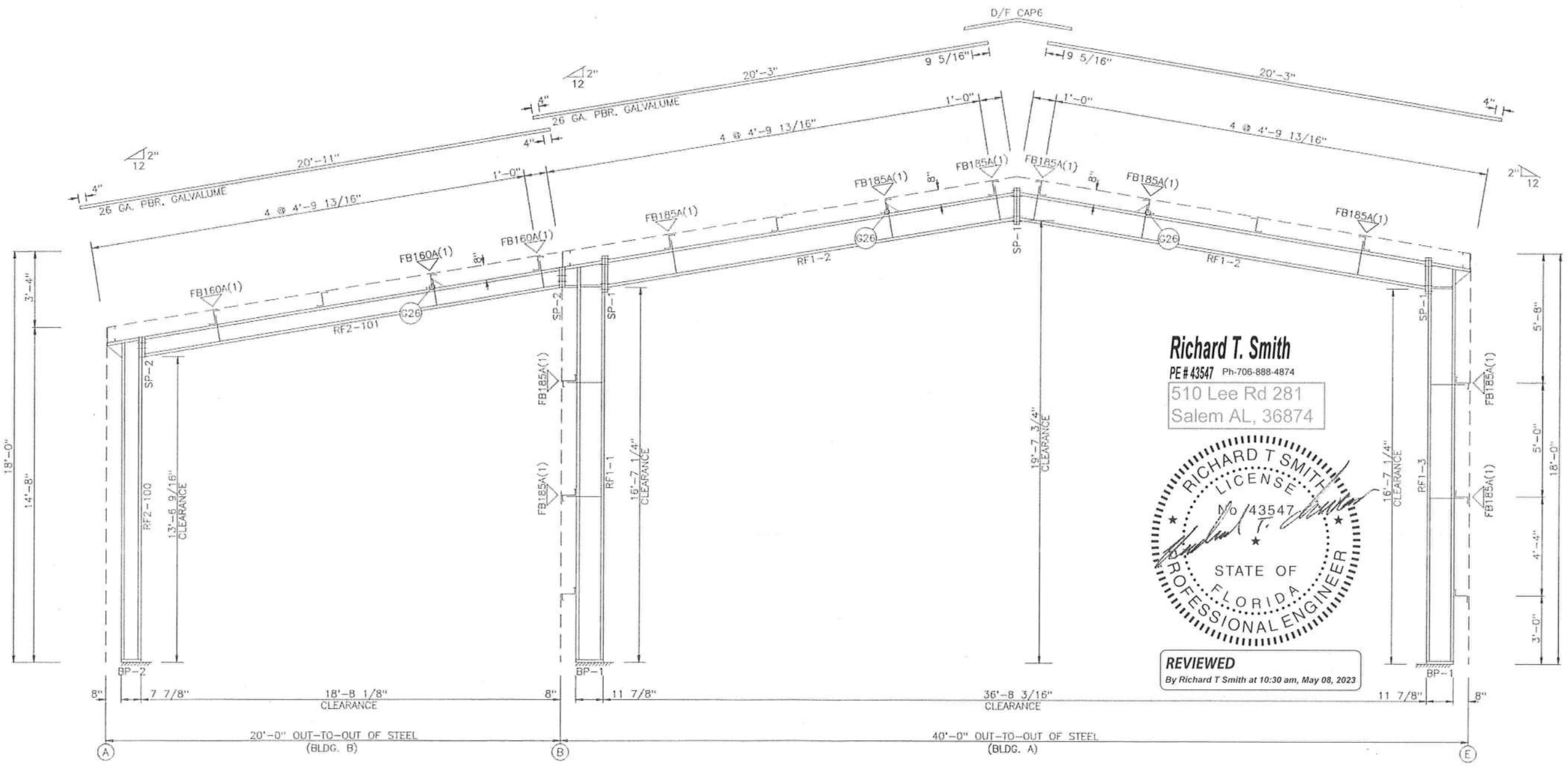
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.1	JRD	SPW	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP-1	4	4	0	A325	5/8"	2"
SP-2	4	0	0	A325	5/8"	2"

BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP-1	6"	3/8"	1'-0"
BP-2	6"	3/8"	8"

FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A = L2x2x14

MEMBER SIZE TABLE			
MARK	MEMBER	LENGTH	WEIGHT
RF1-1	W12X14	17'-5 1/4"	292
RF1-2	W12X14	18'-6 7/8"	298
RF1-3	W12X14	17'-5 1/4"	283
RF2-100	W8X10	14'-1 1/4"	156
RF2-101	W8X10	18'-11"	208



RIGID FRAME ELEVATION: FRAME LINE 2 3

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION

JOB NO: 8065 DATE: 5/ 2/23

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

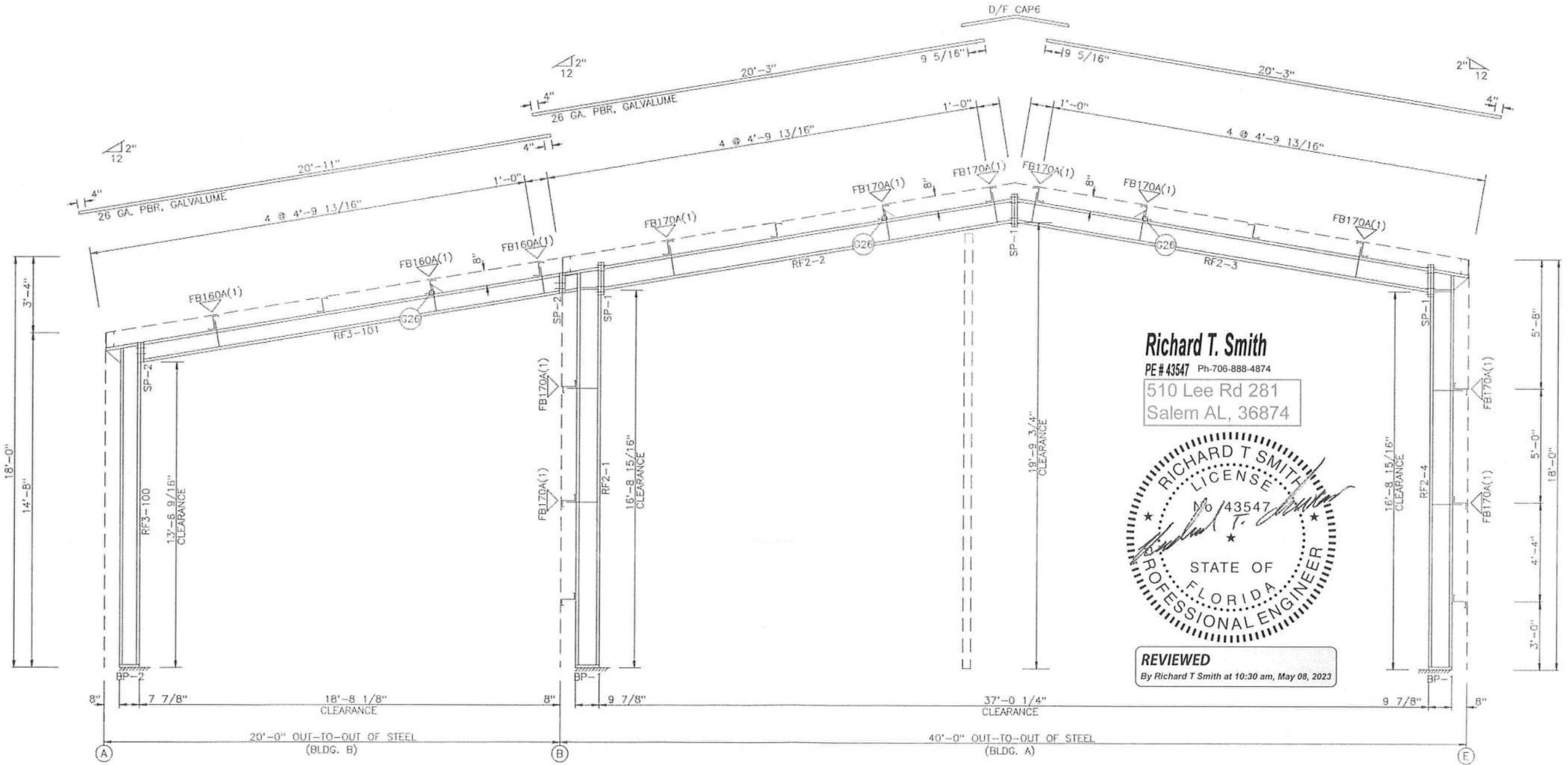
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.2	JRD	SPW	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP-1	4	4	0	A325	5/8"	2"
SP-2	4	0	0	A325	5/8"	2"

BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP-1	6"	3/8"	10"
BP-2	6"	3/8"	8"

FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1); xx=length(in)
A - L2x2x14

MEMBER SIZE TABLE			
MARK	MEMBER	LENGTH	WEIGHT
RF2-1	W10X12	17'-5 1/4"	257
RF2-2	W10X12	18'-8 7/8"	262
RF2-3	W10X12	18'-8 7/8"	263
RF2-4	W10X12	17'-5 1/4"	248
RF3-100	W8X10	14'-1 1/4"	156
RF3-101	W8X10	18'-11"	208



RIGID FRAME ELEVATION: FRAME LINE 4

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

6

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

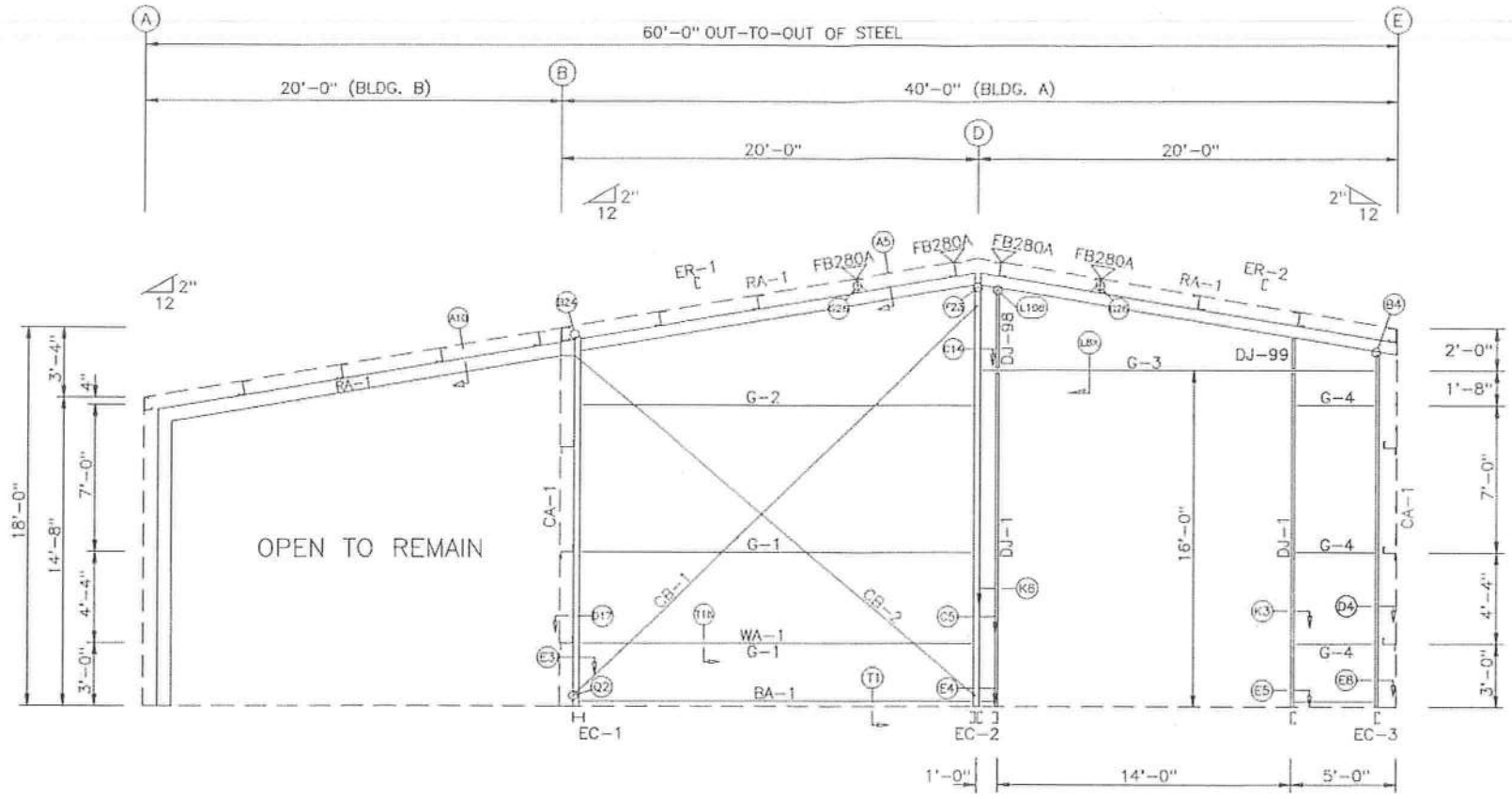
RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

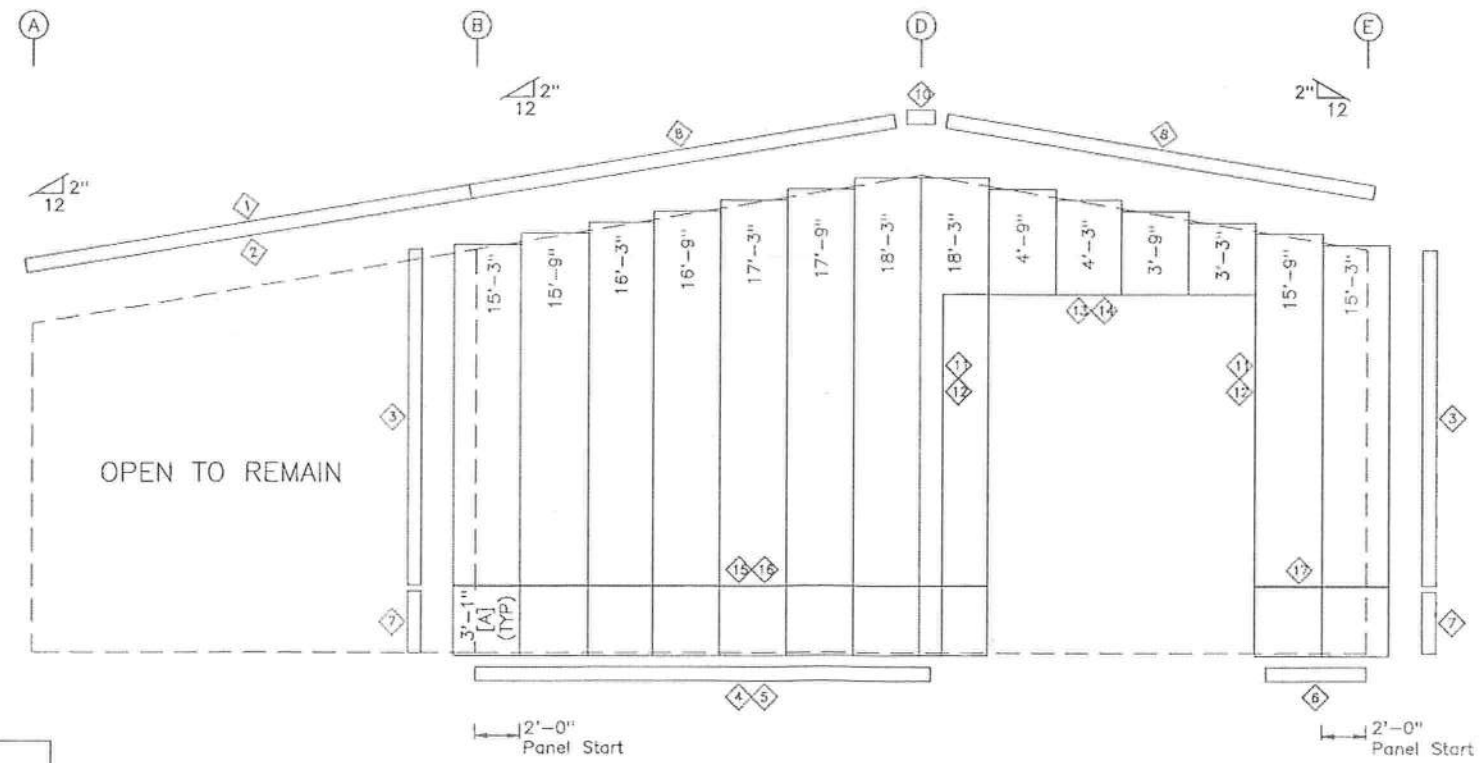
ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION
JOB NO: 8065
DATE: 5/ 2/23
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: RIGID FRAME CROSS SECTION
DRAWING NO: PAGE 2.3
DRAWN BY: JRD
CHECKED BY: SPW
SCALE: NONE



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. R – POLAR WHITE

[A] PANELS: 26 Ga. R – BLACK

NOTE: FIELD SLOT GIRTS FOR CABLE PASSAGE

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT. 15

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION. 6

BOLT TABLE
FRAME LINE 1

LOCATION	QUAN	TYPE	DIA	LENGTH
BLDG. A				
EC-1/ER-1	4	A325	5/8"	2"
ER-1/ER-2	8	A325	5/8"	2"
EC-2/ER-2	2	A325	5/8"	2"
EC-3/ER-2	2	A325	5/8"	2"

TRIM TABLE
FRAME LINE 1

◇ID	PART	LENGTH	DETAIL
1	RAKE TRM	20'-3"	TRIM_24
2	RST10	20'-3"	TRIM_24
3	O/S CORN	15'-2"	TRIM_5
4	BASE TRM	20'-3"	TRIM_16
5	BASE TRM	1'-3"	TRIM_16
6	BASE TRM	5'-3"	TRIM_16
7	O/S CORN	3'-2"	TRIM_5
8	RAKE TRM	20'-3"	TRIM_3
10	PEAK BOX	1'-4"	TRIM_4
11	CT8	16'-1"	TRIM_11
12	R JAMB	16'-3"	TRIM_8
13	CT8	14'-0"	TRIM_10
14	R HEAD	14'-3"	TRIM_61
15	WCT-2	20'-3"	TRIM_20
16	WCT-2	1'-3"	TRIM_20
17	WCT-2	5'-3"	TRIM_20

MEMBER TABLE
FRAME LINE 1

MARK	PART	LENGTH
8065-A		
EC-1	W8X10	16'-8 3/8"
EC-2	8X7DC14	19'-4 11/16"
EC-3	8X35C16	16'-5 3/4"
ER-1	10X35C12	20'-5"
ER-2	10X35C12	20'-3 1/16"
DJ-1	8X35C12	16'-0"
G-1	8x25Z16	18'-3 5/8"
G-2	8x25Z14	18'-3 5/8"
G-3	8x35C14	18'-7 1/2"
G-4	8x25Z16	3'-7 1/2"
CB-1	1/4 CBL	26'-10"
CB-2	1/4 CBL	25'-1"

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

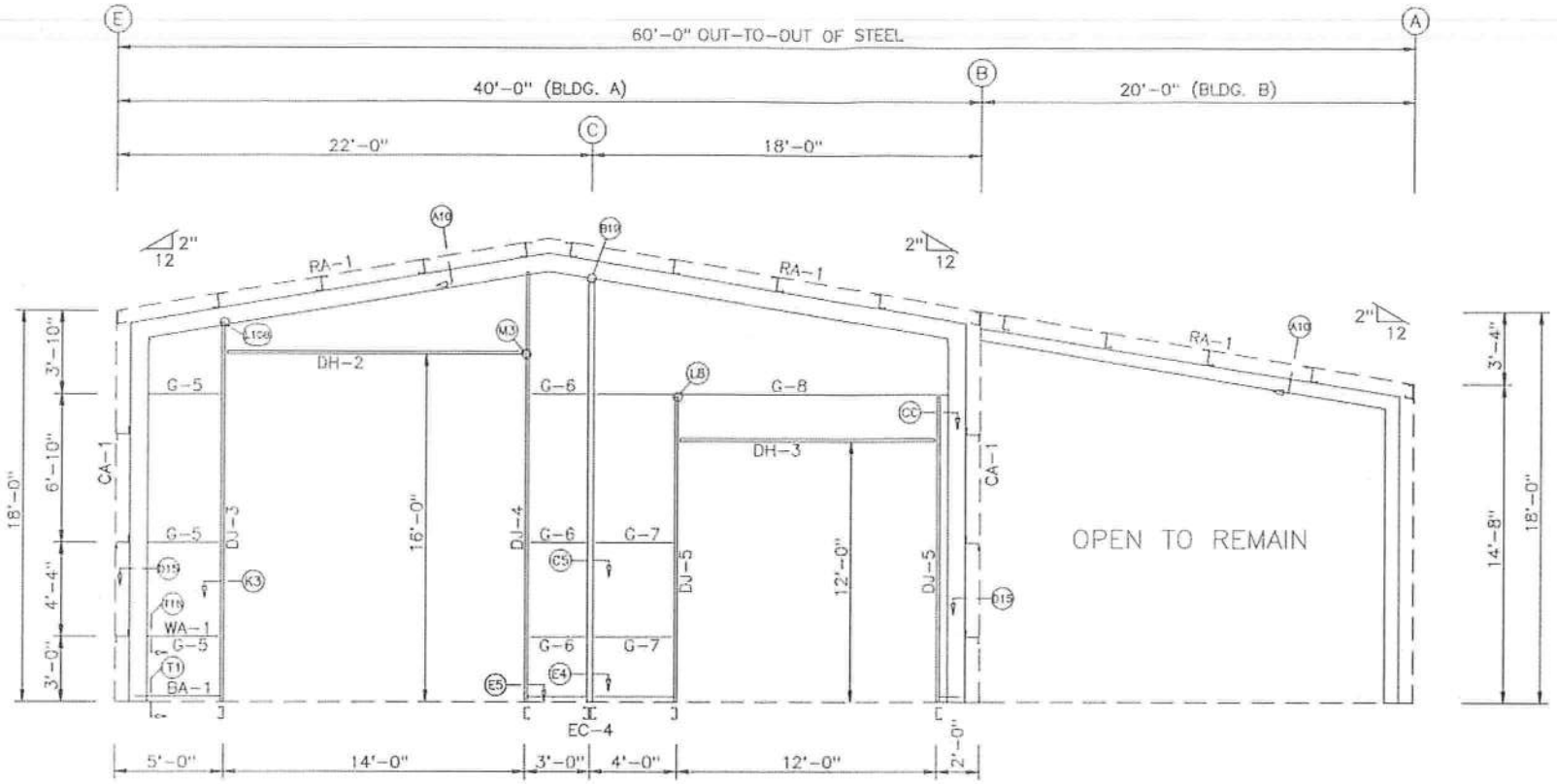
RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION
JOB NO: 8065
DATE: 5/ 2/23
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: FRAMING & SHEETING LAYOUT
DRAWING NO: PAGE 3
DRAWN BY: JRD
CHECKED BY: SPW
SCALE: NONE



ENDWALL FRAMING: FRAME LINE 4

BOLT TABLE FRAME LINE 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
8065--A				
Columns/Raf	2	A325	5/8"	2"
Jamb	2	A325	5/8"	2"

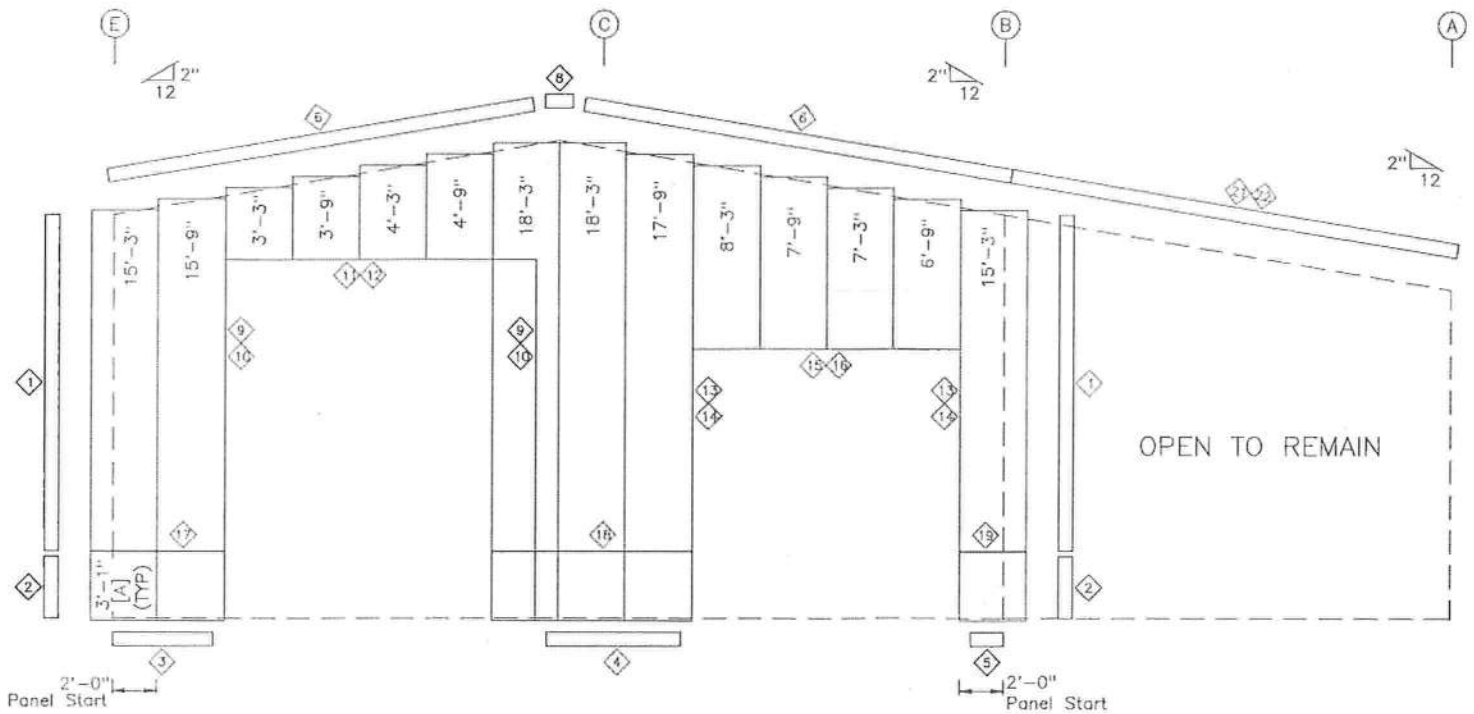
TRIM TABLE FRAME LINE 4			
◇ID	PART	LENGTH	DETAIL
1	O/S CORN	15'-2"	TRIM_5
2	O/S CORN	3'-2"	TRIM_5
3	BASE TRM	5'-3"	TRIM_16
4	BASE TRM	7'-3"	TRIM_16
5	BASE TRM	2'-3"	TRIM_16
6	RAKE TRM	20'-3"	TRIM_3
8	PEAK BOX	1'-4"	TRIM_4
9	CT8	16'-1"	TRIM_11
10	R JAMB	16'-3"	TRIM_8
11	CT8	14'-0"	TRIM_10
12	R HEAD	14'-3"	TRIM_61
13	CT8	12'-1"	TRIM_11
14	R JAMB	12'-3"	TRIM_8
15	CT8	12'-0"	TRIM_10
16	R HEAD	12'-3"	TRIM_61
17	WCT-2	5'-3"	TRIM_20
18	WCT-2	7'-3"	TRIM_20
19	WCT-2	2'-3"	TRIM_20
21	RAKE TRM	20'-3"	TRIM_24
22	RST10	20'-3"	TRIM_24

MEMBER TABLE FRAME LINE 4		
MARK	PART	LENGTH
BLDG. A		
EC-4	8X7DC14	19'-3 13/16"
DJ-3	8X35C12	17'-1 7/8"
DJ-4	8X35C12	19'-5 7/8"
DJ-5	8X35C16	14'-2"
DH-2	8X35C16	14'-0"
DH-3	8X35C16	12'-0"
G-5	8x25Z16	3'-1 5/8"
G-6	8x25Z16	2'-3 1/2"
G-7	8x25Z16	3'-3 1/2"
G-8	8x25Z16	16'-1 5/8"

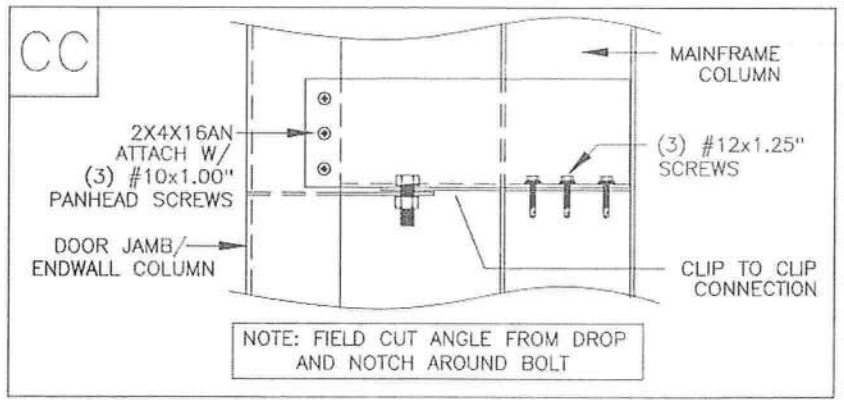
Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023



ENDWALL SHEETING & TRIM: FRAME LINE 4
PANELS: 26 Ga. R – POLAR WHITE
[A] PANELS: 26 Ga. R – BLACK



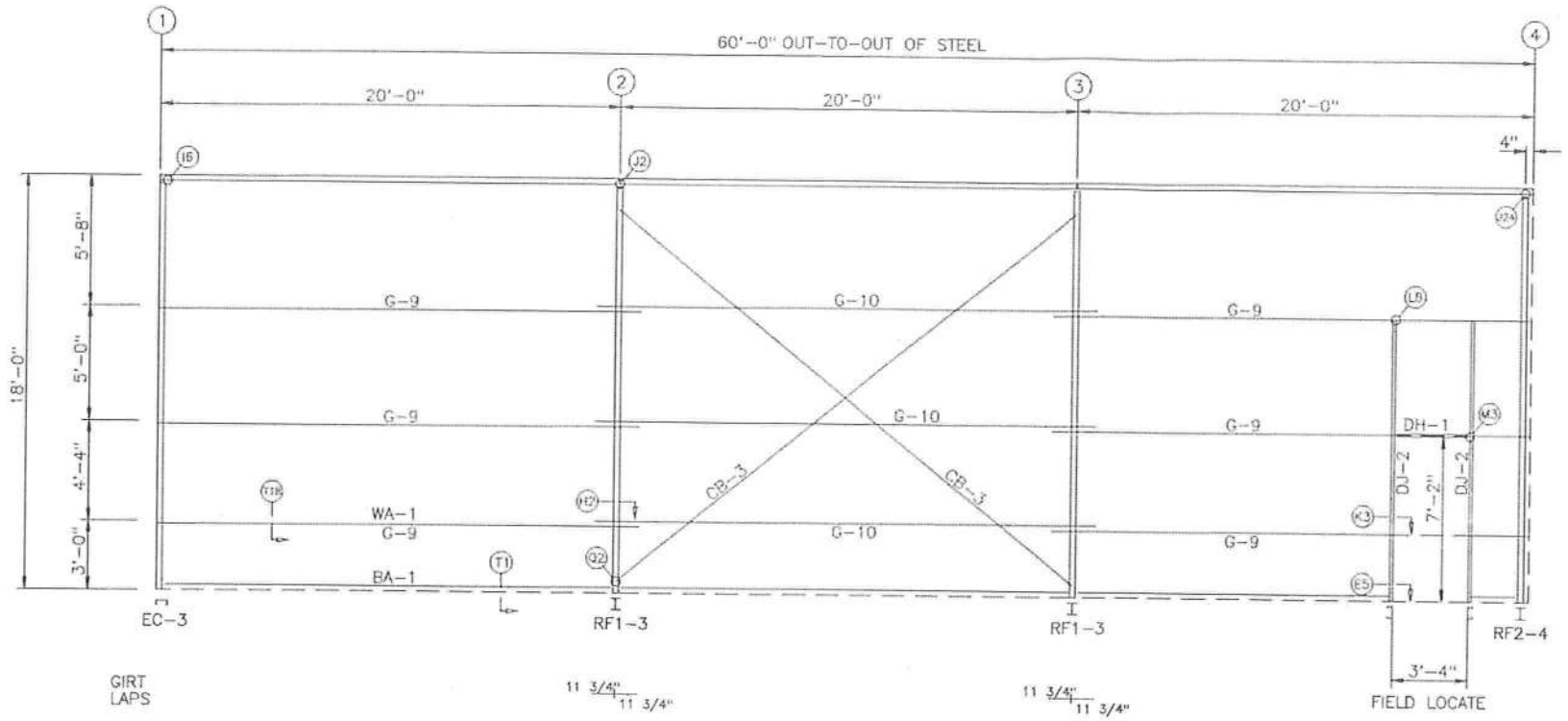
ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT. 15

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION. 6

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

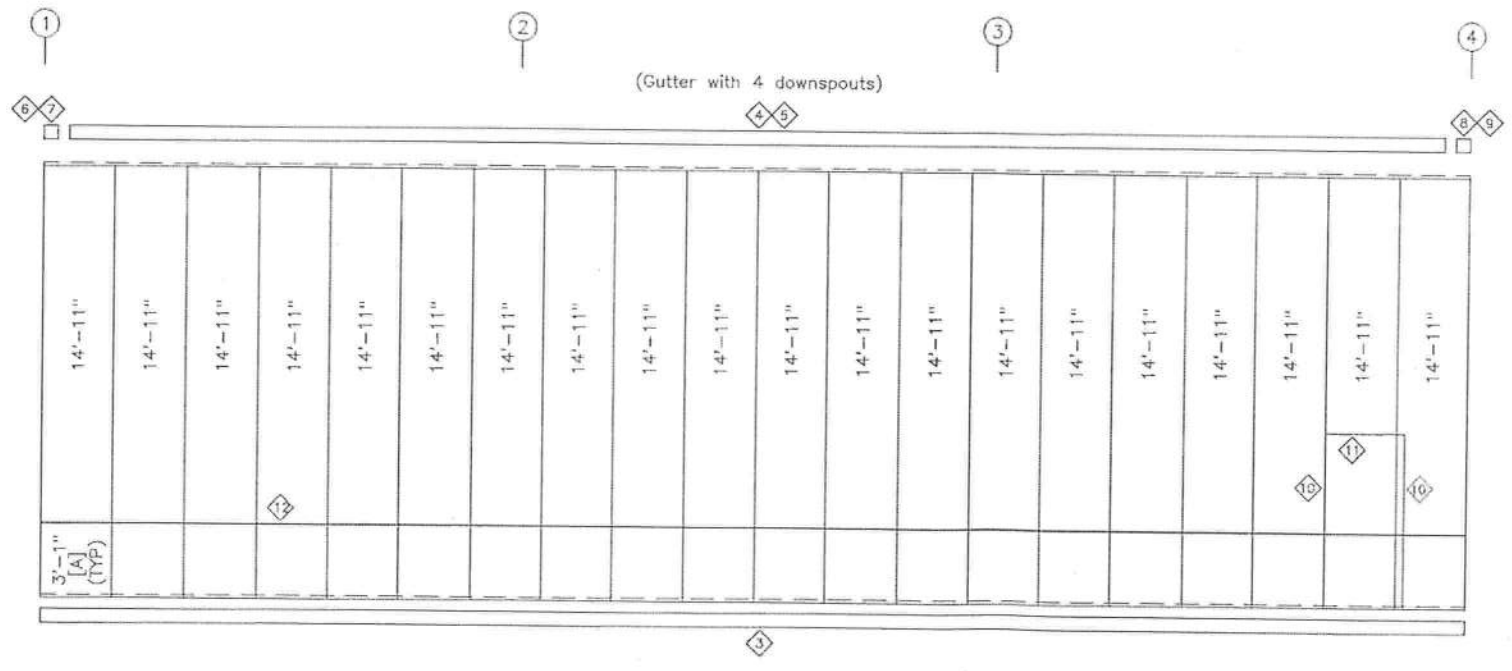
CUSTOMER: GIBRALTAR CONSTRUCTION
JOB NO: 8065 DATE: 5/ 2/23
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: FRAMING & SHEETING LAYOUT
DRAWING NO: PAGE 3.1 DRAWN BY: JRD CHECKED BY: SPW SCALE: NONE



SIDEWALL FRAMING: FRAME LINE E

TRIM TABLE FRAME LINE E			
◇ID	PART	LENGTH	DETAIL
3	BASE TRM	20'-3"	TRIM_16
4	GUTTER	20'-3"	TRIM_1
5	EAVE TRM	20'-3"	TRIM_120
6	GUTEND L	1"	TRIM_2
7	CORBOX L	1'-0"	TRIM_2
8	GUTEND R	1"	TRIM_2
9	CORBOX R	1'-0"	TRIM_2
10	R JAMB	7'-5"	TRIM_8
11	R HEAD	3'-7"	TRIM_61
12	WCT-2	20'-3"	TRIM_20

MEMBER TABLE FRAME LINE E		
MARK	PART	LENGTH
DJ-2	8X25C16	12'-4"
DH-1	8X25C16	3'-4"
G-9	8x25Z16	20'-11 1/2"
G-10	8x25Z16	21'-11 1/2"
CB-3	1/4 CBL	26'-8"



SIDEWALL SHEETING & TRIM: FRAME LINE E

PANELS: 26 Ga. R – POLAR WHITE

[A] PANELS: 26 Ga. R – BLACK

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION

JOB NO: 8065 DATE: 5/ 2/23

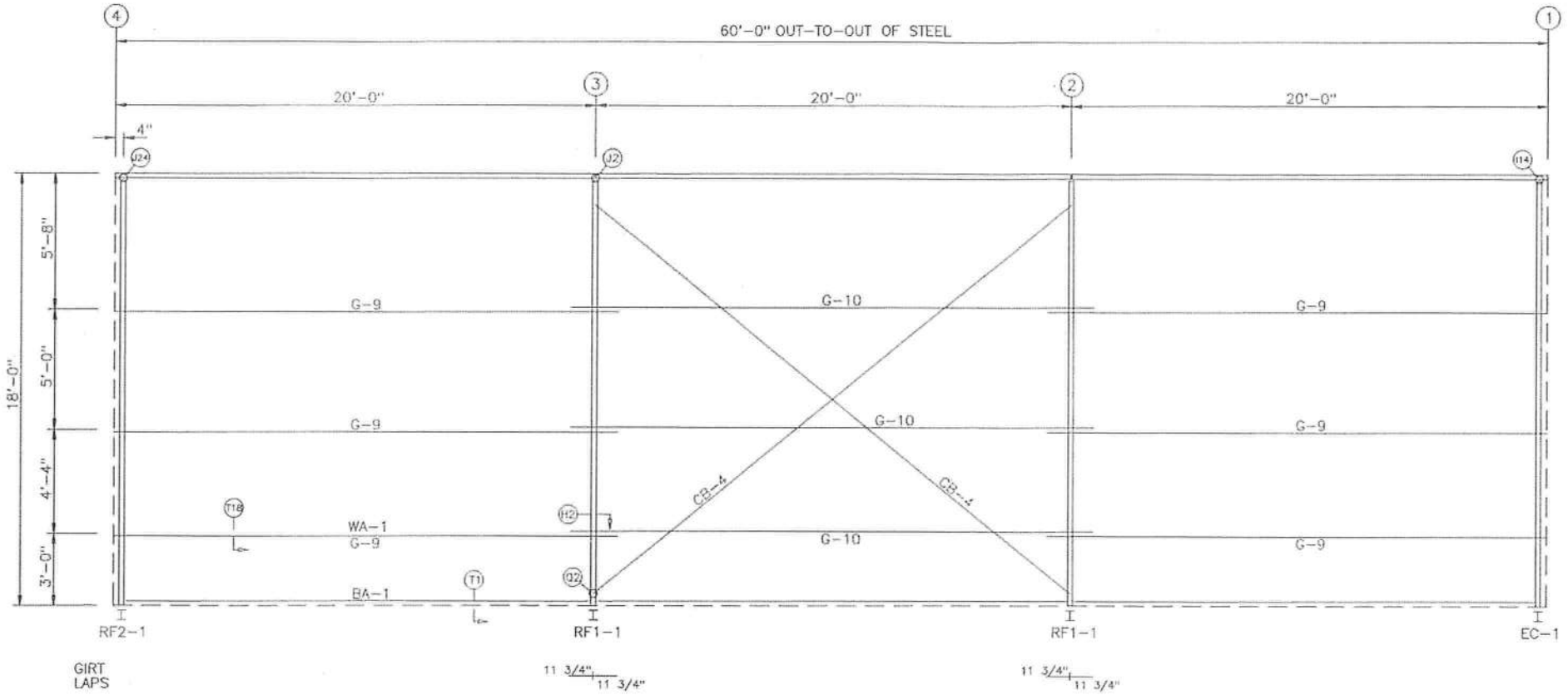
LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: FRAMING & SHEETING LAYOUT

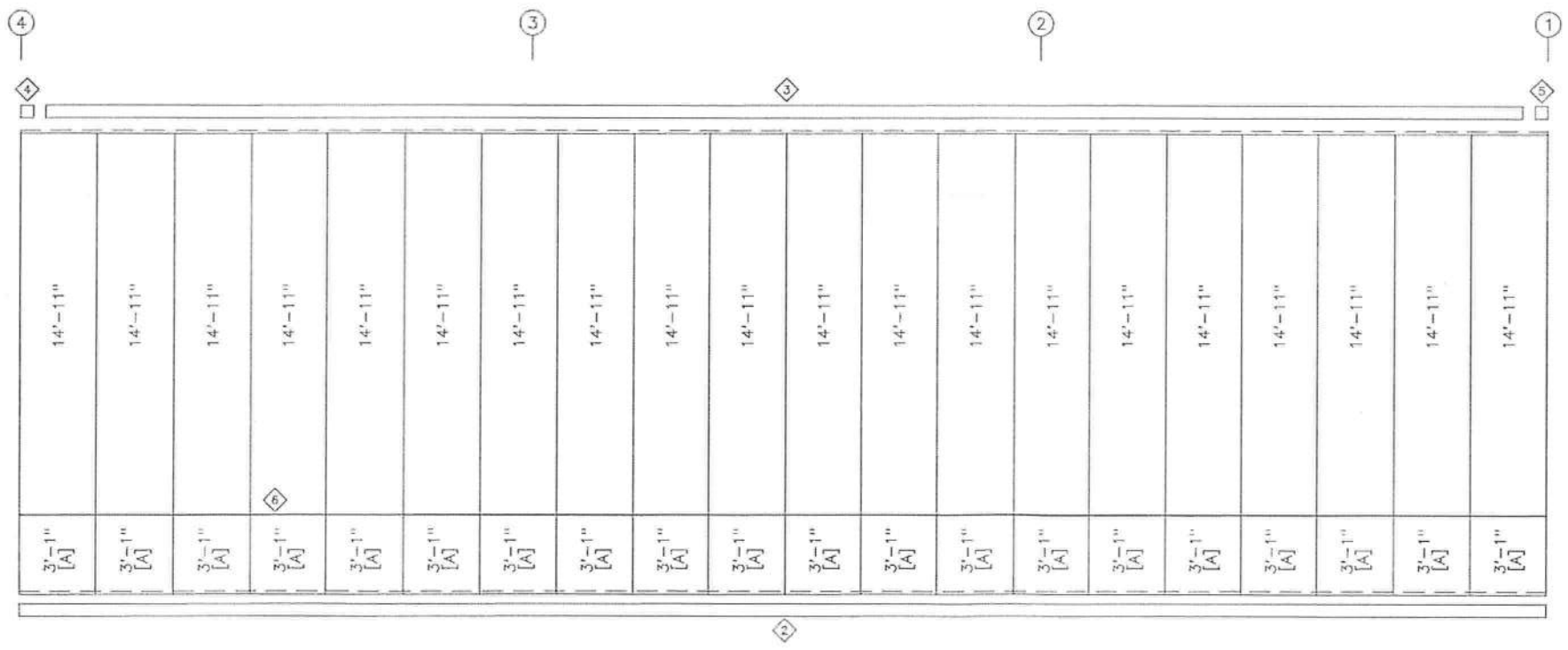
DRAWING NO: | DRAWN BY: | CHECKED BY: | SCALE:

TRIM TABLE FRAME LINE B			
◇ID	PART	LENGTH	DETAIL
2	BASE TRM	20'-3"	TRIM_16
3	EAVE TRM	20'-3"	ETD
4	R END LH	6"	TRIM_13
5	R END RH	6"	TRIM_13
6	WCT-2	20'-3"	TRIM_20

MEMBER TABLE FRAME LINE B		
MARK	PART	LENGTH
G-9	8x25Z16	20'-11 1/2"
G-10	8x25Z16	21'-11 1/2"
CB-4	5/16 CBL	26'-8"



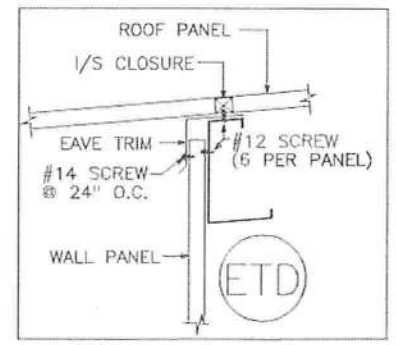
SIDEWALL FRAMING: FRAME LINE B



SIDEWALL SHEETING & TRIM: FRAME LINE B

PANELS: 26 Ga. R – POLAR WHITE

[A] PANELS: 26 Ga. R – BLACK



Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION

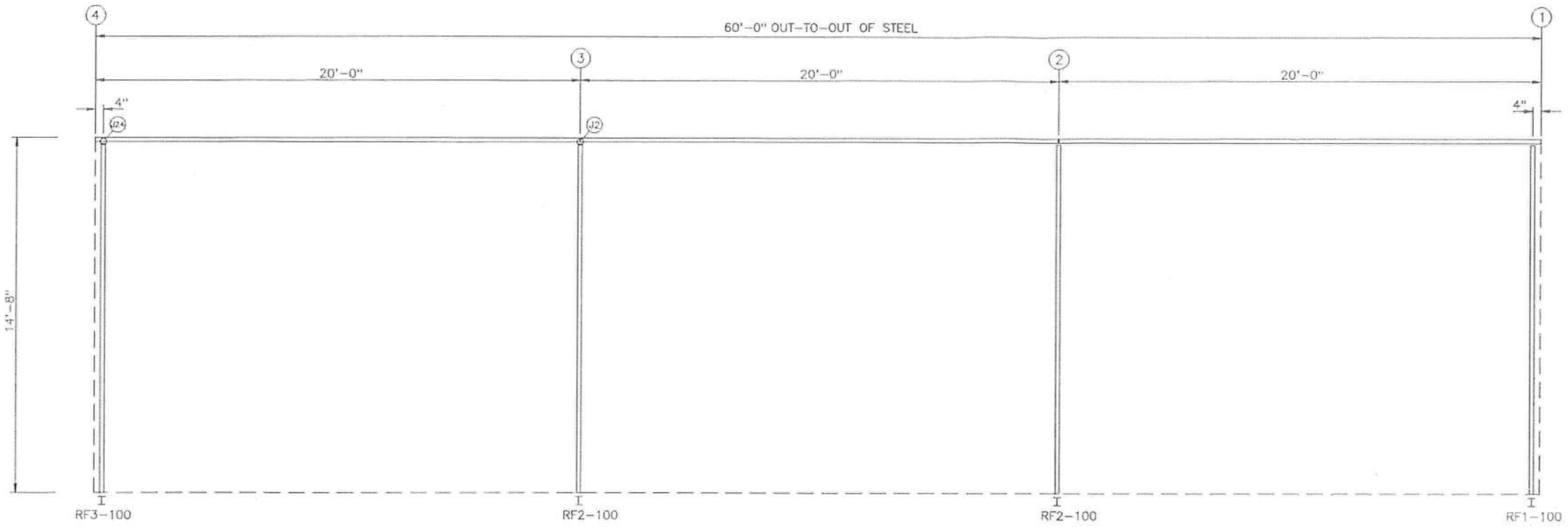
JOB NO: 8065 | DATE: 5/ 2/23

LOCATION: HIGH SPRINGS, FL 32643

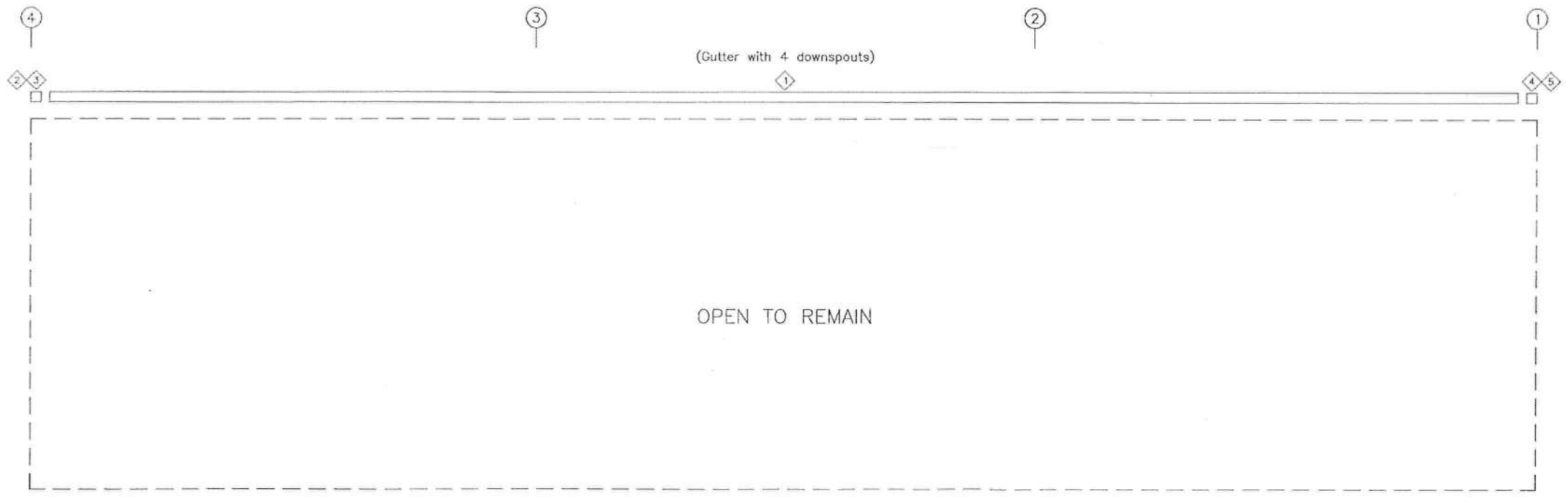
DRAWING NAME: FRAMING & SHEETING LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 4.1	JRD	SPW	NONE

TRIM TABLE FRAME LINE A			
◇ID	PART	LENGTH	DETAIL
1	GUTTER	20'-3"	TRIM_1
2	GUTEND L	1"	TRIM_2
3	CORBOX L	1'-0"	TRIM_2
4	GUTEND R	1"	TRIM_2
5	CORBOX R	1'-0"	TRIM_2



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

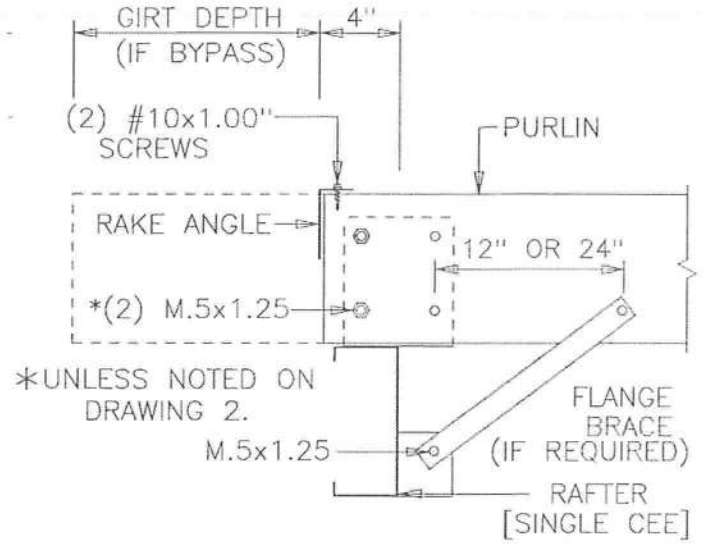
CUSTOMER: GIBRALTAR CONSTRUCTION

JOB NO: 8065 DATE: 5/ 2/23

LOCATION: HIGH SPRINGS, FL 32643

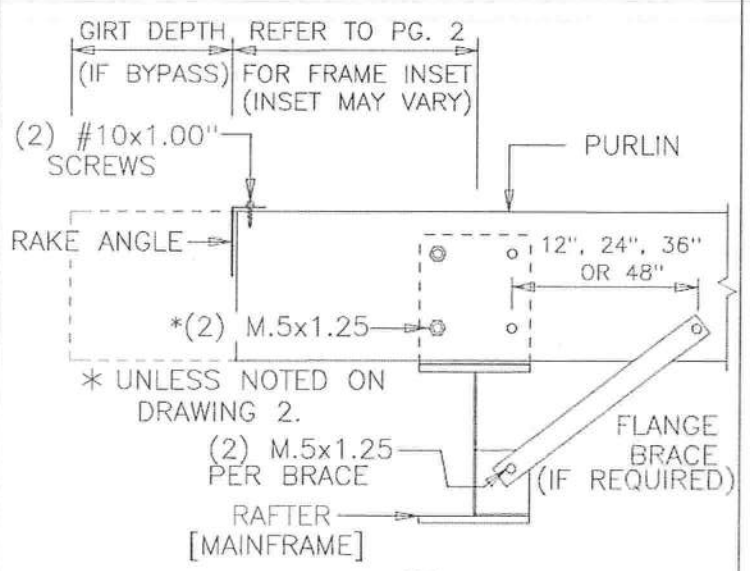
DRAWING NAME: FRAMING & SHEETING LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 4.2	JRD	SPW	NONE



DETAIL (A5)

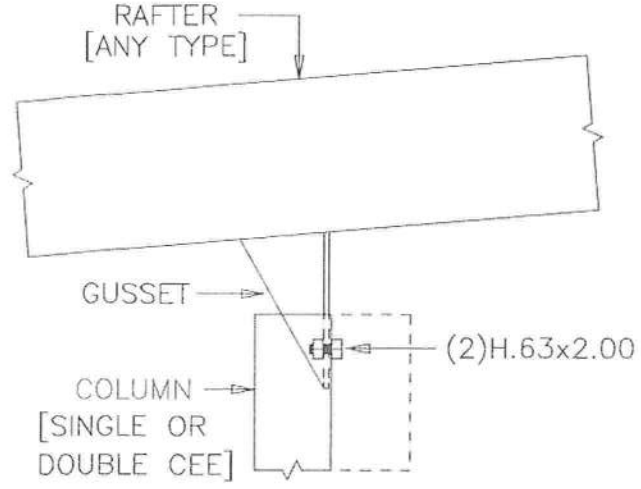
PURLIN TO ENDWALL RAFTER



DETAIL (A10)

PURLIN TO ENDWALL RAFTER

DETAIL (B4)

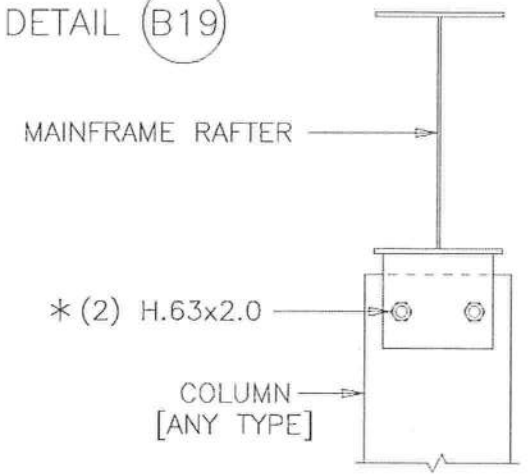


* UNLESS NOTED ON EW FRAMING PLAN

CEE COLUMN / RAFTER CONNECTION

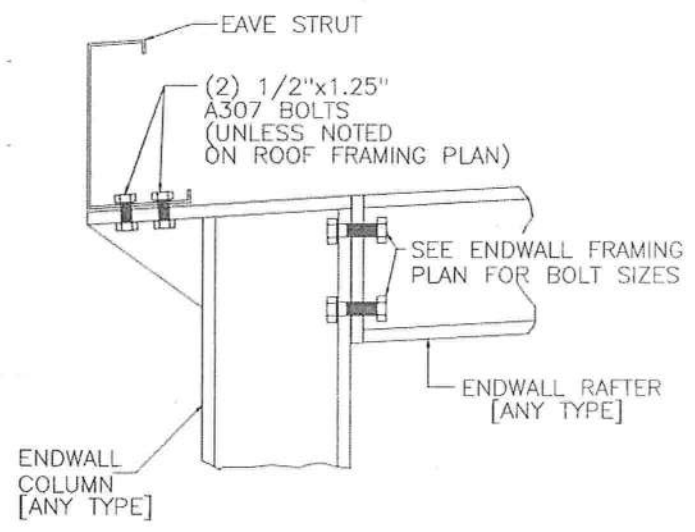
NOTE: HAND TIGHTEN EW COLUMN/RAFTER CONNECTION BOLTS, THEN INTERRUPT THREADS.

DETAIL (B19)



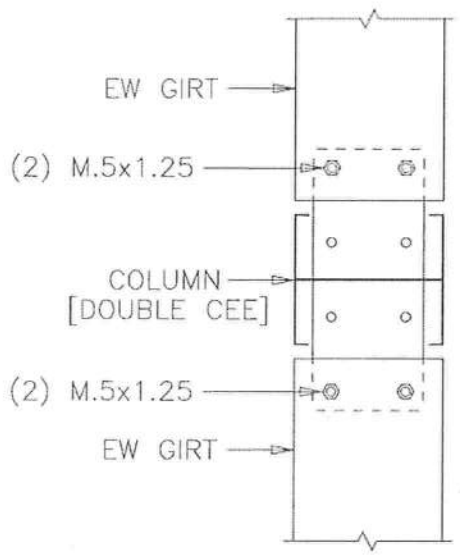
* UNLESS NOTED EW FRAMING OR PARTITION FRAMING PLAN(S)

MAINFRAME RAFTER / COLUMN CONNECTION



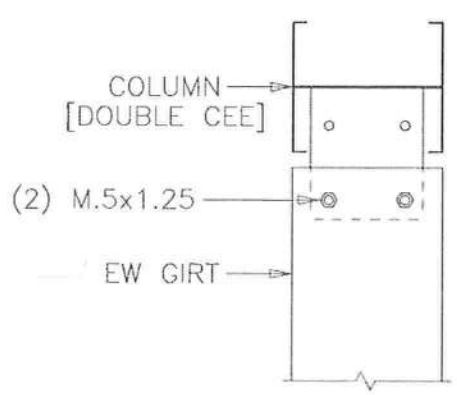
(B24) BYPASS ROTATED CORNER COLUMN TO ENDWALL RAFTER

DETAIL (C5)



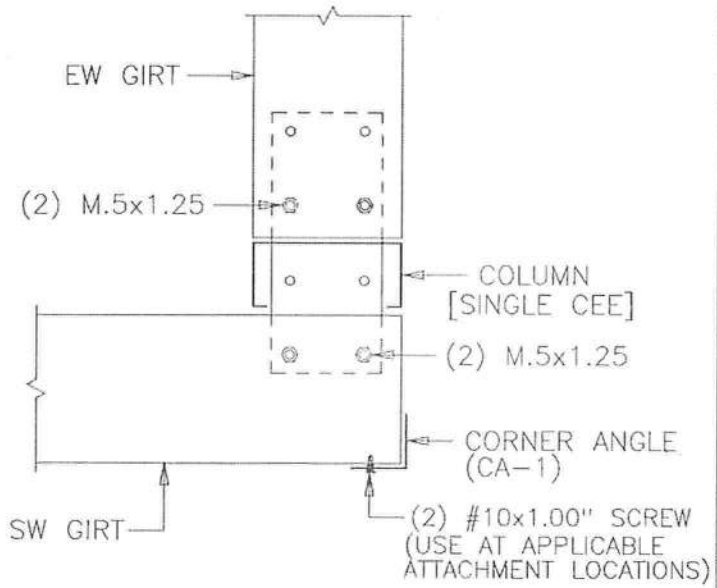
ENDWALL GIRTS TO INTERIOR COLU

DETAIL (C14)



DWALL GIRT STOPPING AT COLUMN

DETAIL (D4)



GIRTS TO CORNER COLUMN

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION

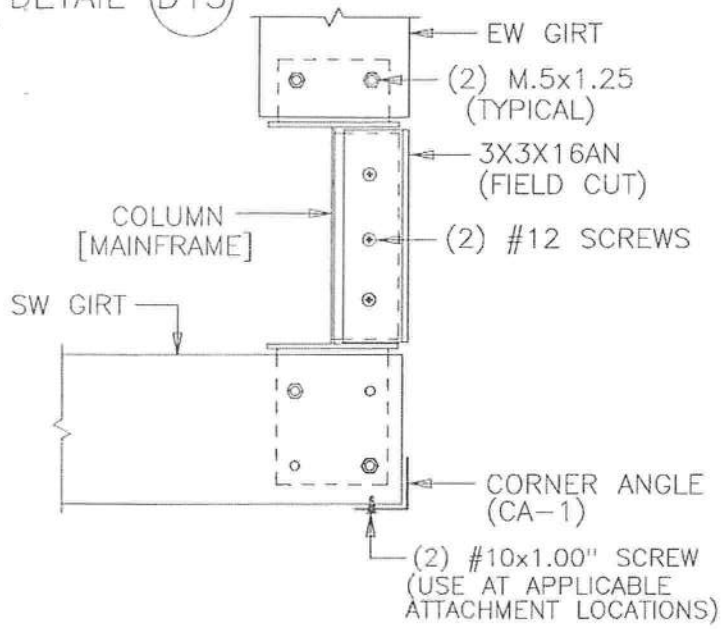
JOB NO: 8065 | DATE: 5/ 2/23

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: FRAMING DETAILS

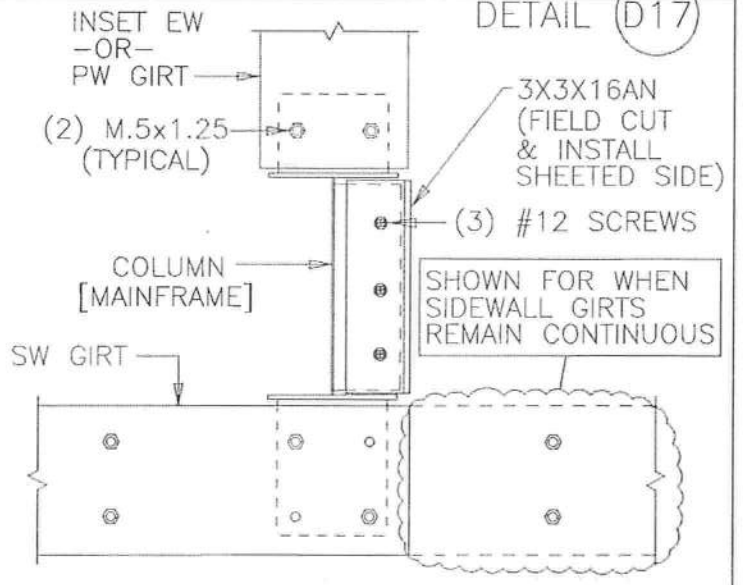
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 5	JRD	SPW	NONE

DETAIL D15



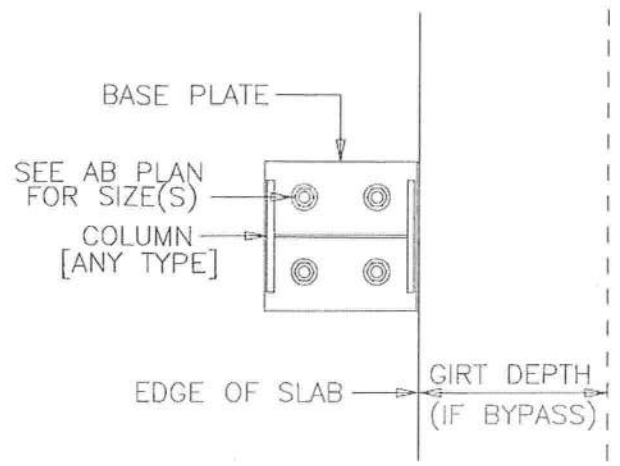
GIRT CONNECTIONS AT RIGID FRAME

DETAIL D17



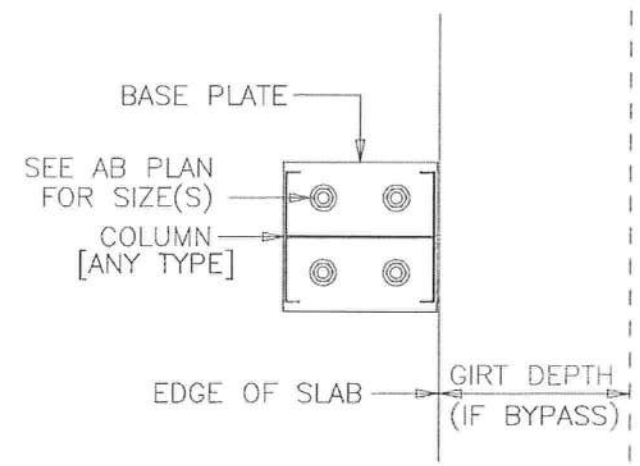
GIRT CONNECTIONS AT PARTITION WALL OR INSET ENDWALL

DETAIL E3



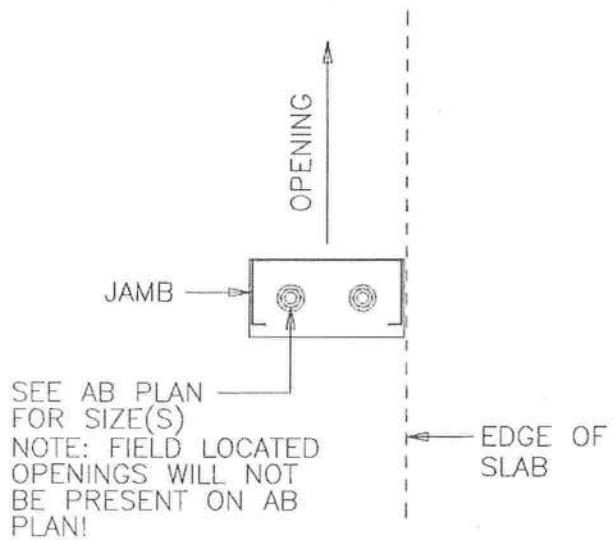
ENDWALL COLUMN BASE DETAIL

DETAIL E4



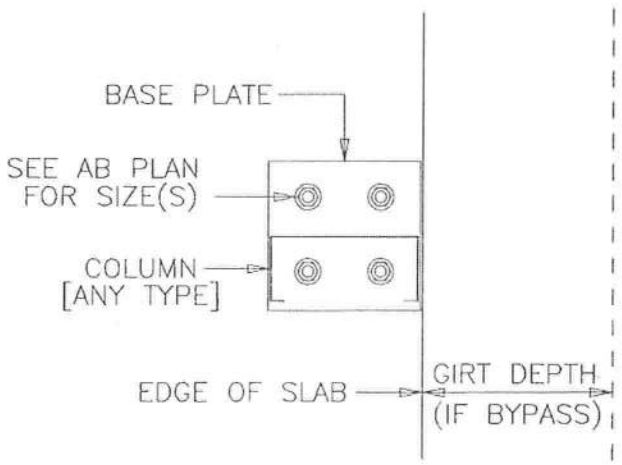
ENDWALL COLUMN BASE DETAIL

DETAIL E5

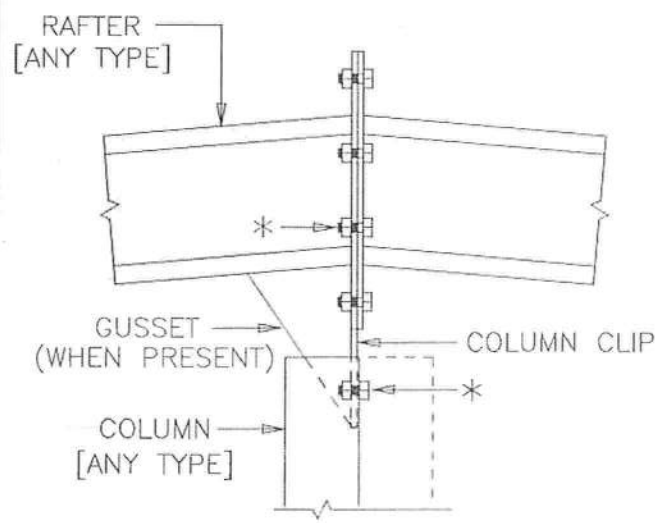


FRAMED OPENING JAMB BASE DETAIL

DETAIL E8



ENDWALL COLUMN BASE DETAIL

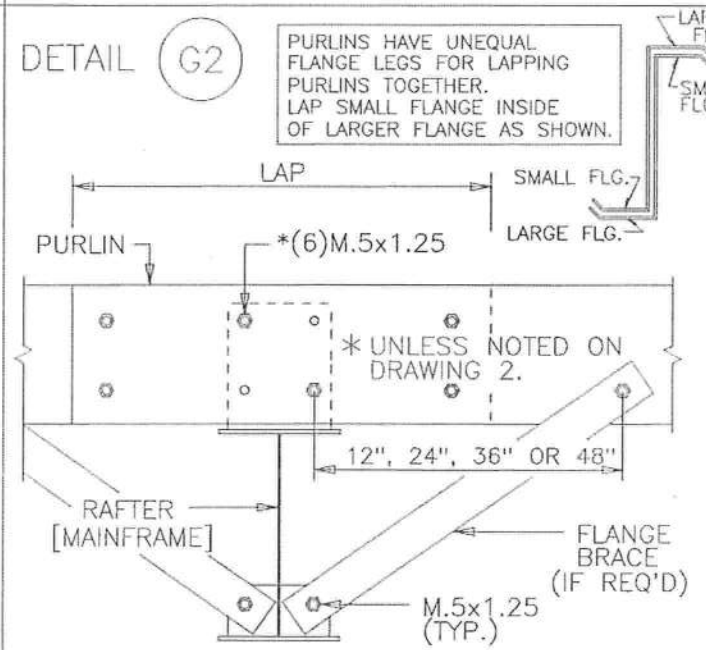


* SEE EW FRAMING PLAN FOR SIZE AND QUANTITY OF BOLTS.

F23 RAFTER DETAIL AT RIDGE WITH CENTER COLUMN

DETAIL G2

PURLINS HAVE UNEQUAL FLANGE LEGS FOR LAPPING PURLINS TOGETHER. LAP SMALL FLANGE INSIDE OF LARGER FLANGE AS SHOWN.



PURLIN TO MAINFRAME RAFTER

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

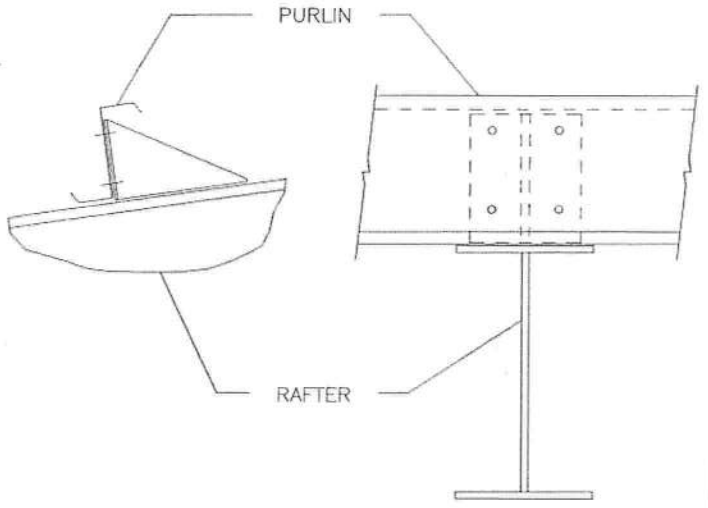
CUSTOMER: GIBRALTAR CONSTRUCTION

JOB NO: 8065 | DATE: 5/ 2/23

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: FRAMING DETAILS

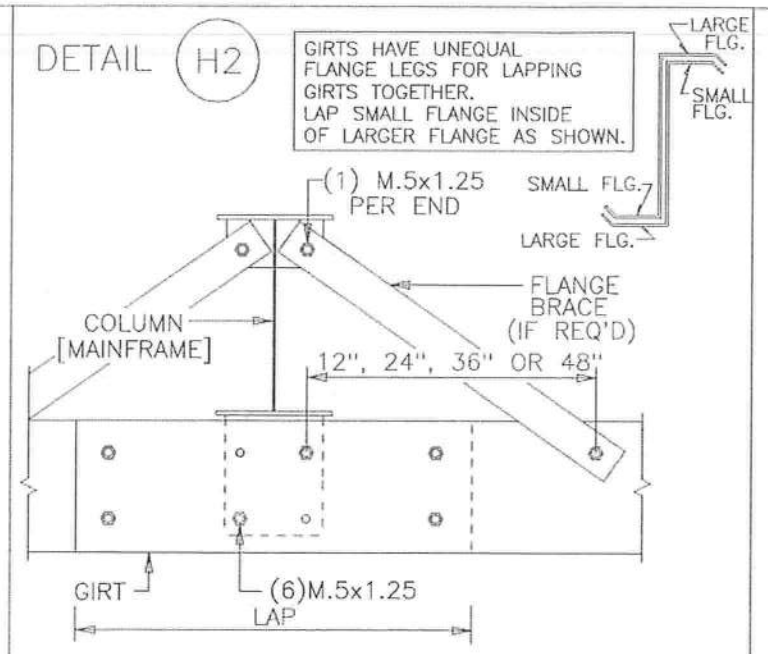
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 5.1	JRD	SPW	NONE



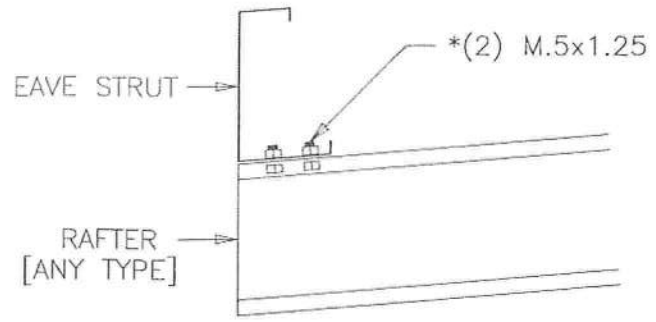
G26 WELDED ANTI-ROLL CLIP

DETAIL H2

GIRTS HAVE UNEQUAL FLANGE LEGS FOR LAPPING GIRTS TOGETHER. LAP SMALL FLANGE INSIDE OF LARGER FLANGE AS SHOWN.

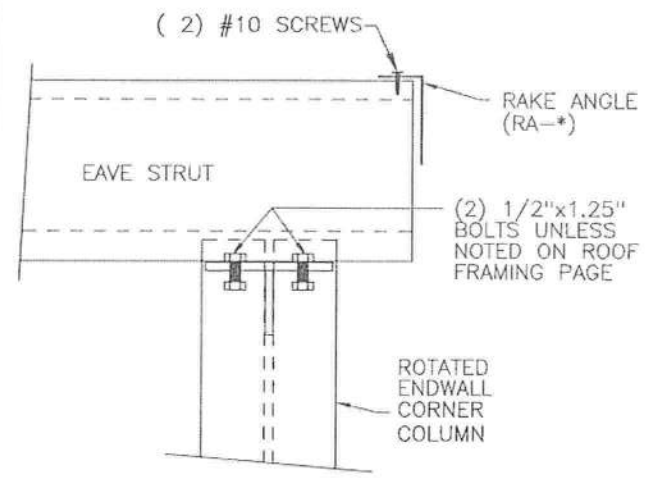


GIRT TO MAINFRAME COLUMN

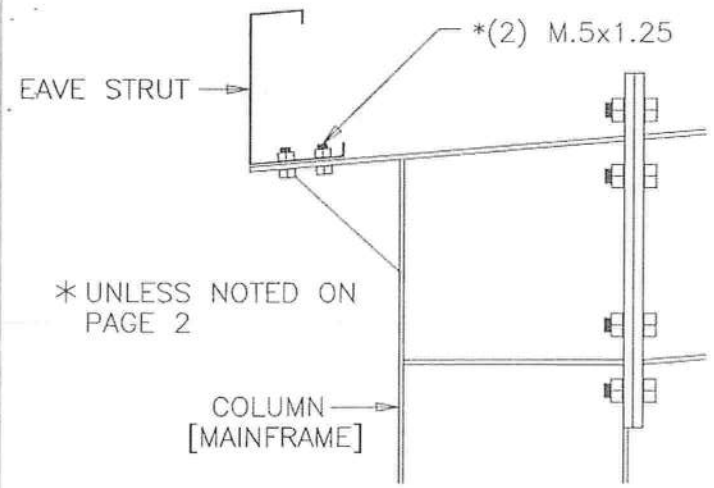


*UNLESS NOTED ON DRAWING 2.

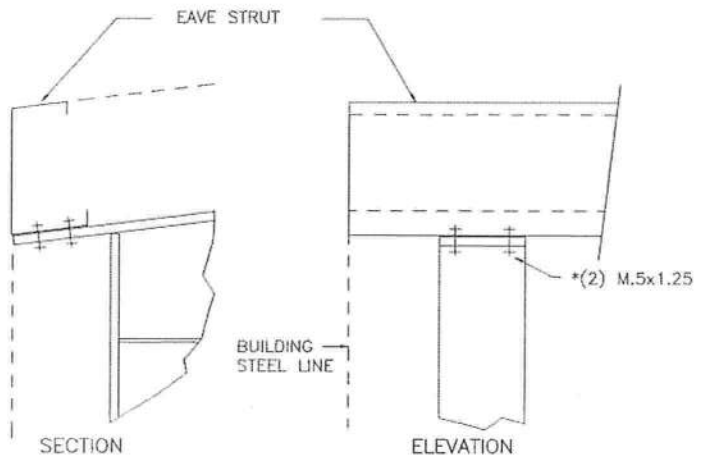
I6 EAVE STRUT CONNECTION AT ENDWALL



I14 EAVE STRUT TO ENDWALL CORNER COLUMN



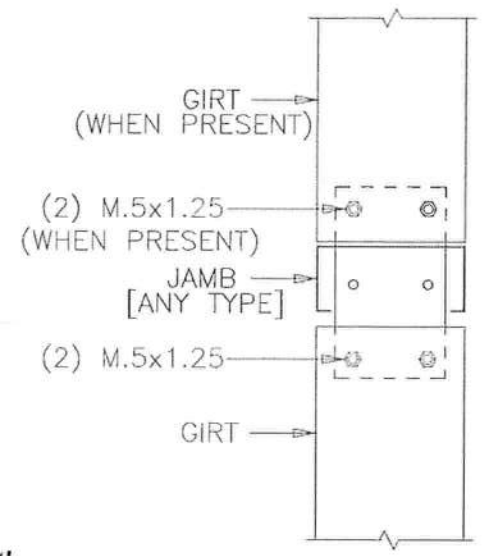
J2 EAVE STRUT CONNECTION AT MAINFRAME



* = UNLESS NOTED ON ROOF FRAMING PLAN

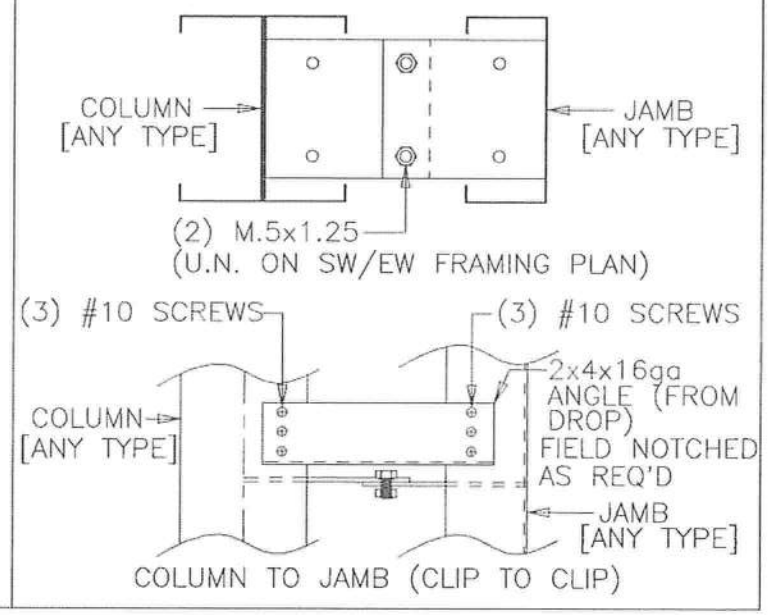
J24 EAVE STRUT TO RIGID FRAME

DETAIL K3



GIRTS TO JAMB

DETAIL K6



COLUMN TO JAMB (CLIP TO CLIP)

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

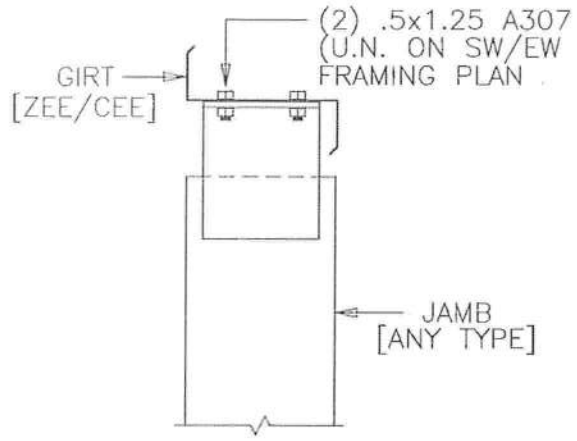
REVIEWED
By Richard T Smith at 10:20 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

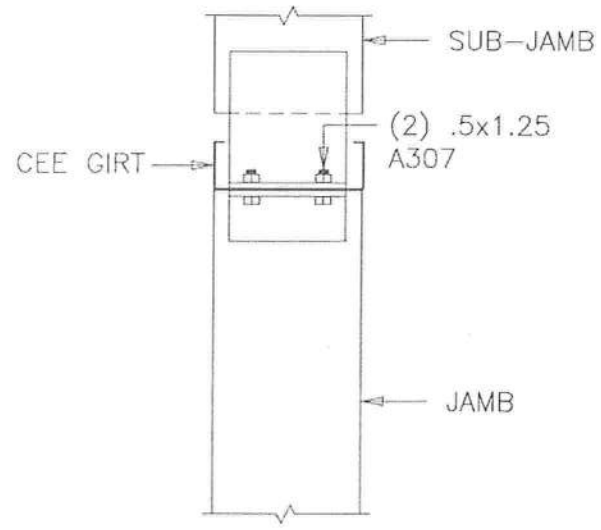
CUSTOMER: GIBRALTAR CONSTRUCTION
JOB NO: 8065 DATE: 5/ 2/23
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: FRAMING DETAILS
DRAWING NO: PAGE 5.2 | DRAWN BY: JRD | CHECKED BY: SPW | SCALE: NONE

DETAIL L8



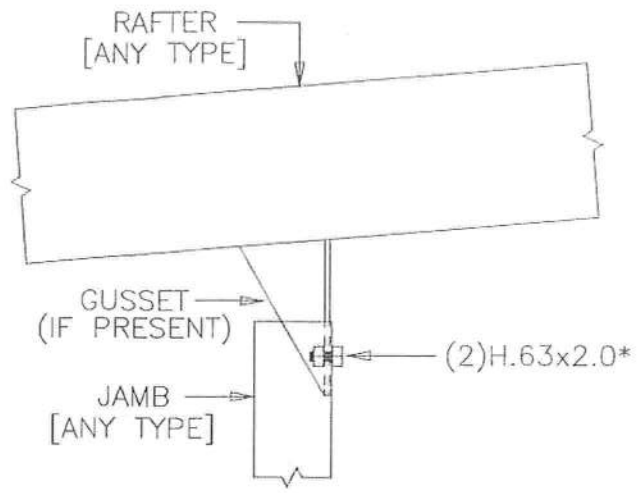
FRAMED OPENING JAMB TO GIRT

DETAIL L8X



FRAMED OPENING JAMB TO CEE GIRT

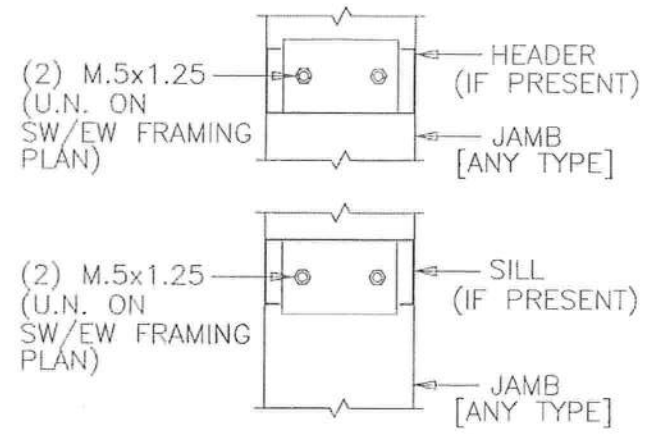
DETAIL L108



* UNLESS NOTED ON EW FRAMING PLAN

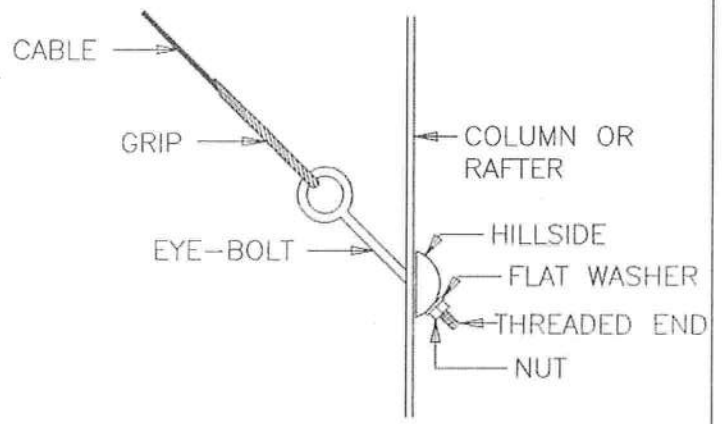
JAMB / RAFTER CONNECTION

DETAIL M3



FRAMED OPENING HEADER/SILL TO JAMB

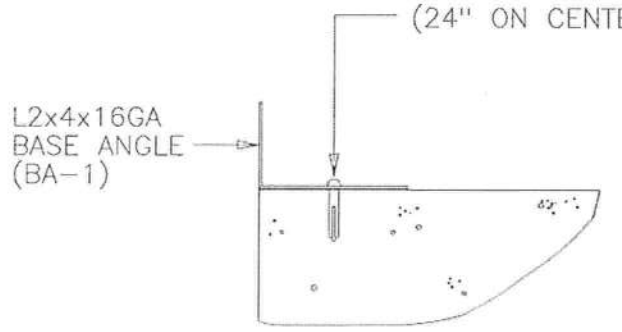
Q2 CABLE INSTALLATION DETAIL



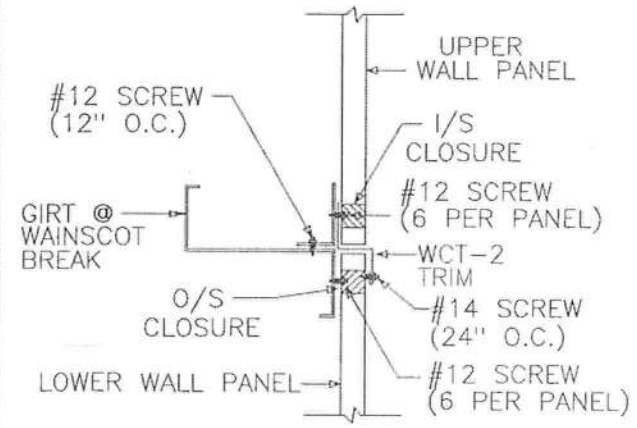
NOTE: WHEN FLUSH GIRTS/PURLINS ARE USED, FIELD SLOT GIRT OR PURLIN AS REQ'D FOR CABLE/ROD PASSAGE THROUGH PURLIN/GIRT.

DETAIL T1

1/4" x 1 1/4" ZINC HAMMER DRIVES
ZAMAK ALLOY (ASTM B633, SC1, TYPE III)
(24" ON CENTER)



BASE ANGLE DETAIL



T18 SECTION @ WAINSCOT PANEL BREAK

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

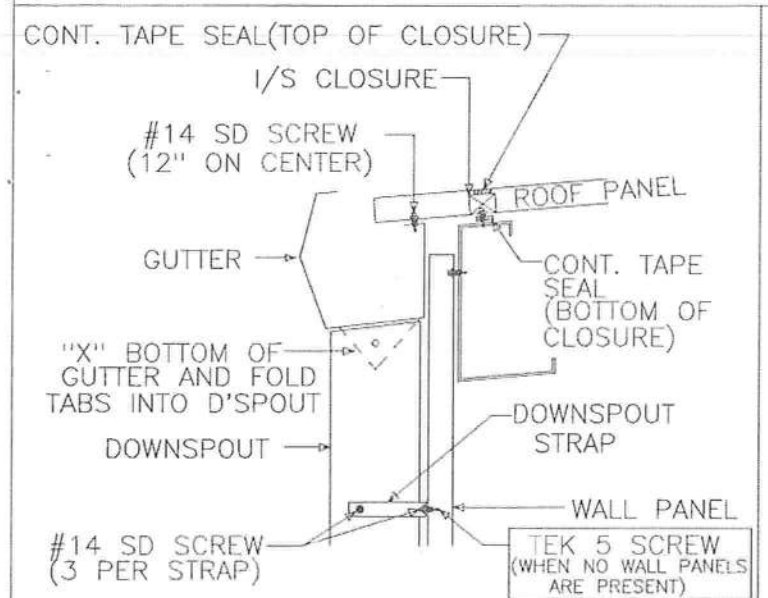
CUSTOMER: GIBRALTAR CONSTRUCTION

JOB NO: 8065 | DATE: 5/ 2/23

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: FRAMING DETAILS

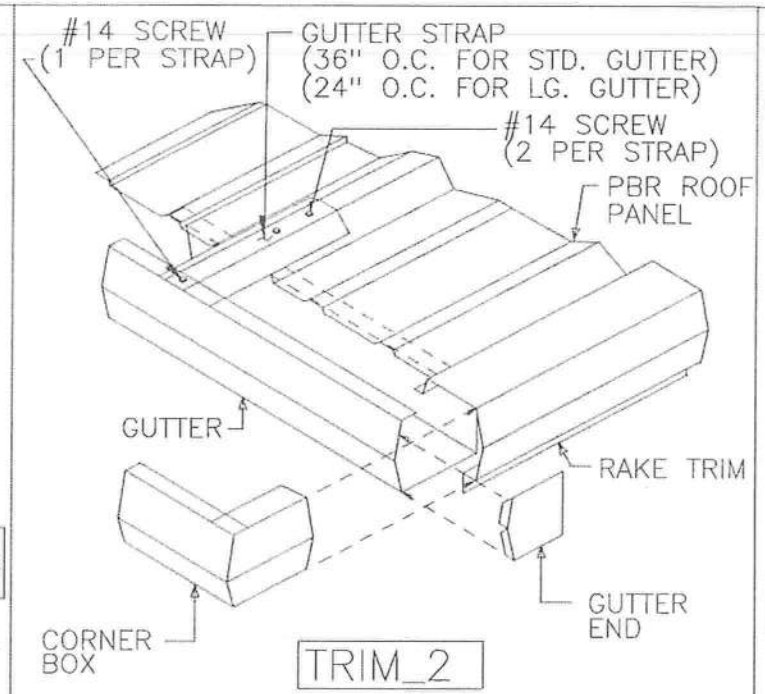
DRAWING NO: PAGE 5.3 | DRAWN BY: JRD | CHECKED BY: SPW | SCALE: NONE



NOTE: INSTALL GUTTER STRAPS 3'-0" ON CENTER.
NOTE: INSTALL D'SPOUT STRAPS 5'-0" ON CENTER.

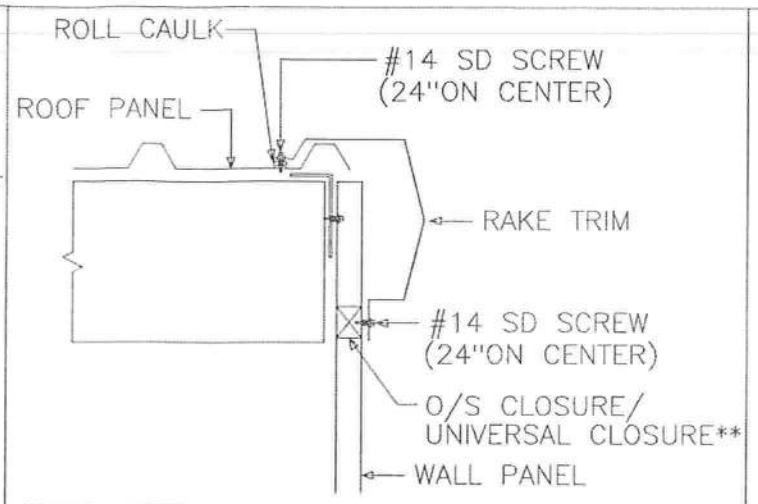
TRIM_1

GUTTER DETAIL



TRIM_2

GUTTER END DETAIL

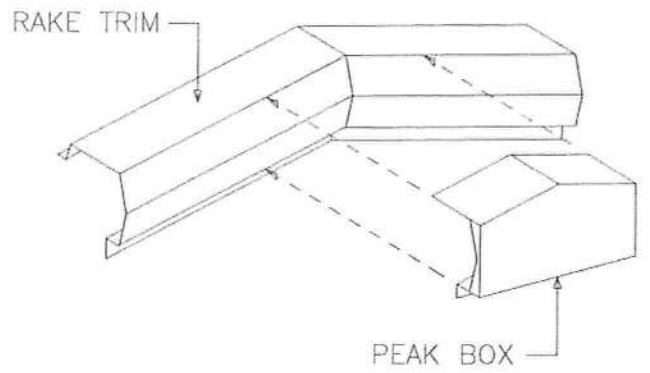


** - O/S CLOSURE FOR ROOF PITCHES UP TO 2:12"
UNIVERSAL CLOSURE FOR ROOF PITCHES ABOVE 2:12"

NOTE: FIELD CUT UNIVERSAL CLOSURES TO MATCH WALL PANEL PROFILE (ON PITCH).

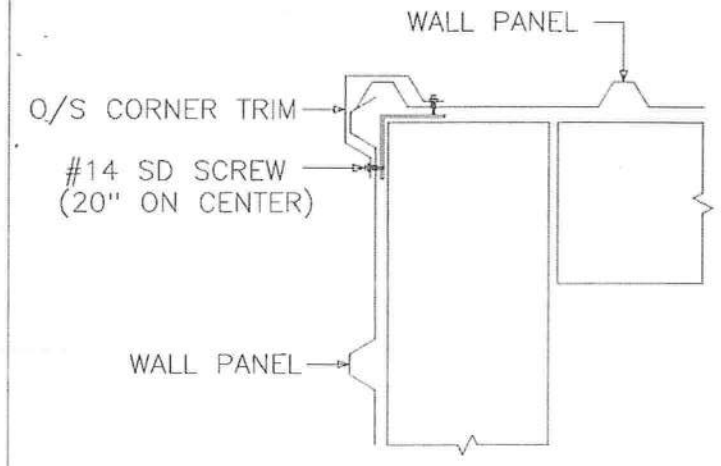
TRIM_3

RAKE TRIM DETAIL



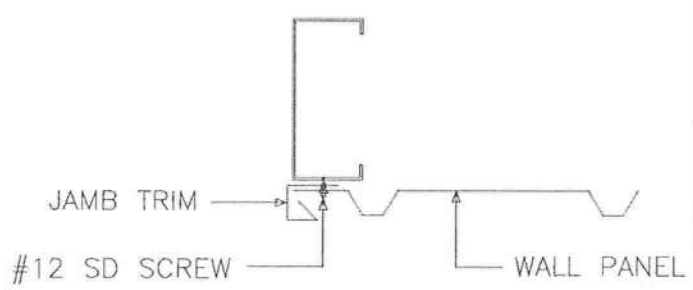
TRIM_4

PEAK BOX DETAIL



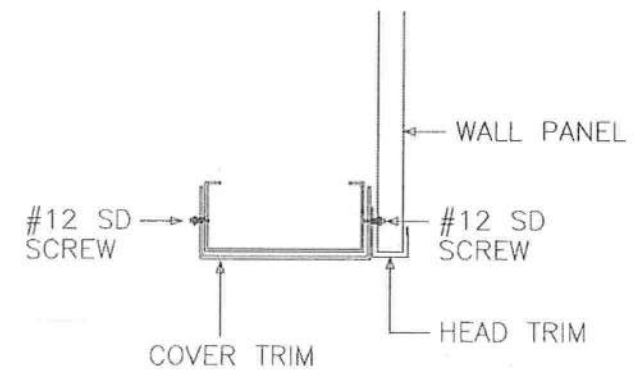
TRIM_5

O/S CORNER DETAIL



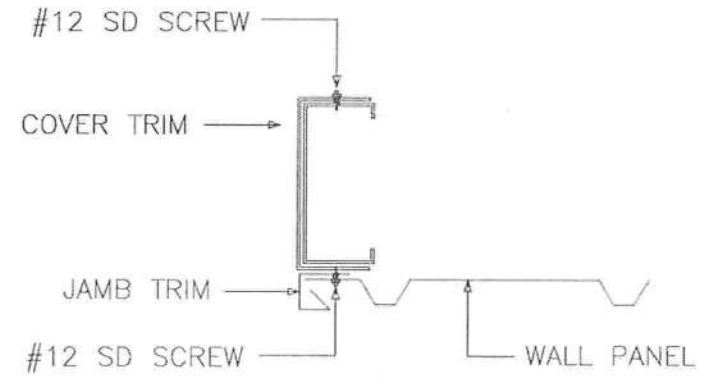
TRIM_8

JAMB TRIM DETAIL AT JAMB



TRIM_10

COVER TRIM DETAIL AT HEADER



TRIM_11

COVER TRIM DETAIL AT JAMB

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

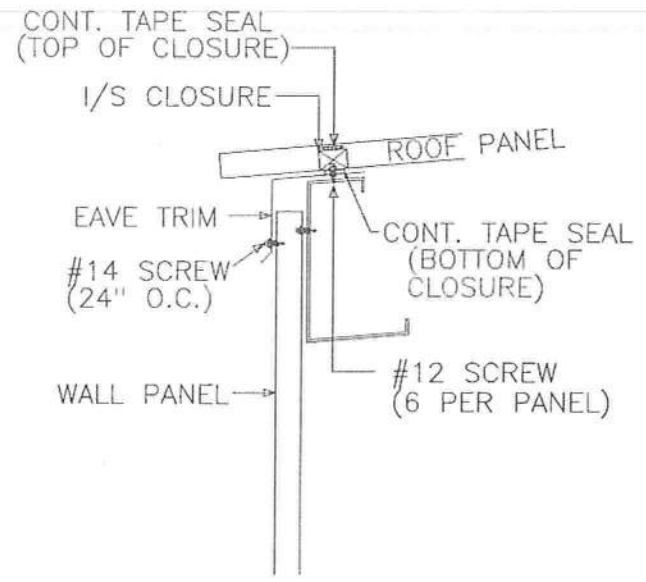
RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:20 am, May 08, 2023

ISSUE	DET	CHK	DATE

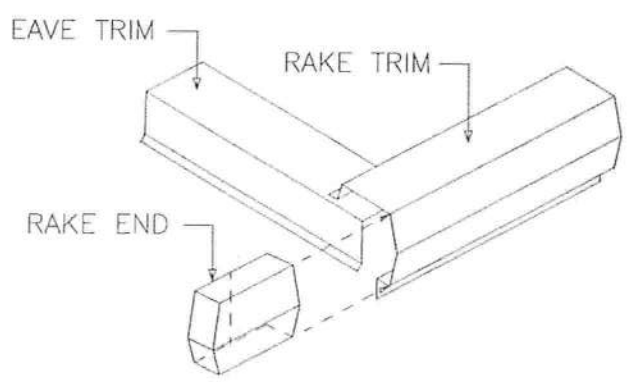
BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION
JOB NO: 8065 DATE: 5/ 2/23
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: FRAMING DETAILS
DRAWING NO: PAGE 5.4 DRAWN BY: JRD CHECKED BY: SPW SCALE: NONE



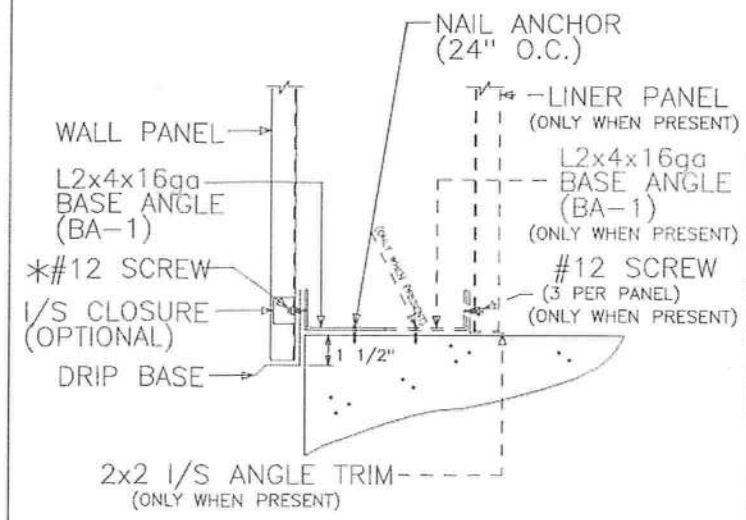
TRIM_12

EAVE TRIM DETAIL



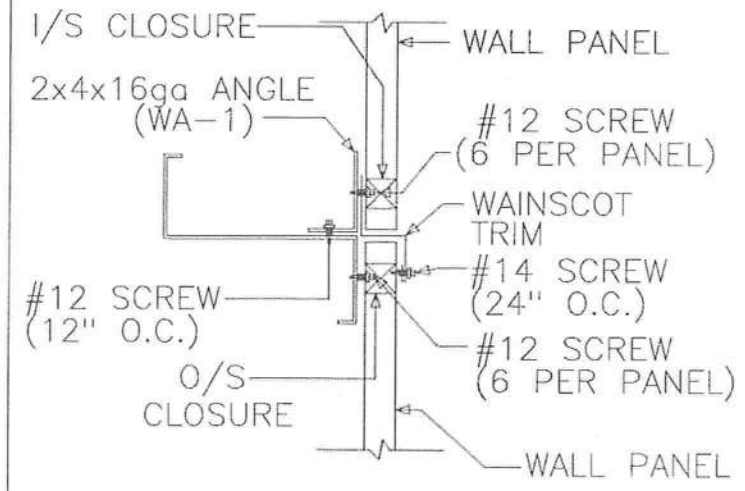
TRIM_13

RAKE END DETAIL



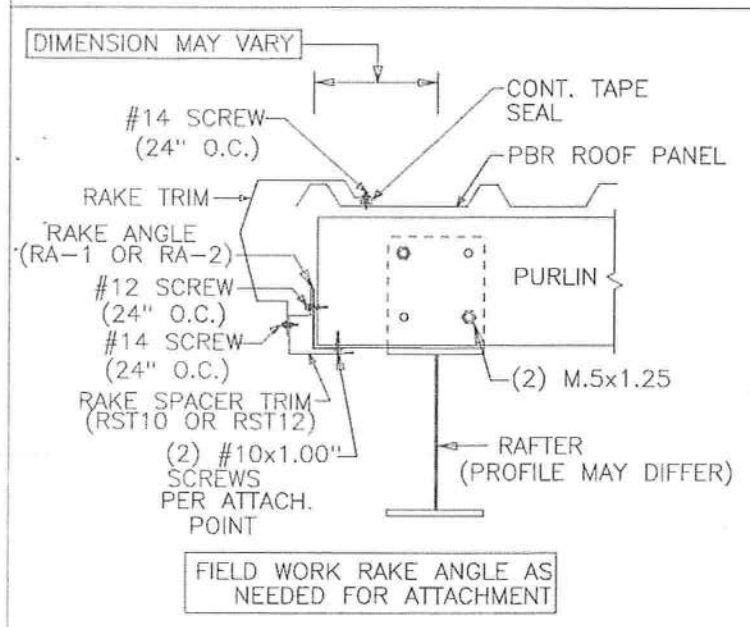
*= 6 PER PANEL FOR STANDARD PBR
3 PER PANEL FOR REV. ROLLED PBR

TRIM_16 BASE TRIM DETAIL



TRIM_20

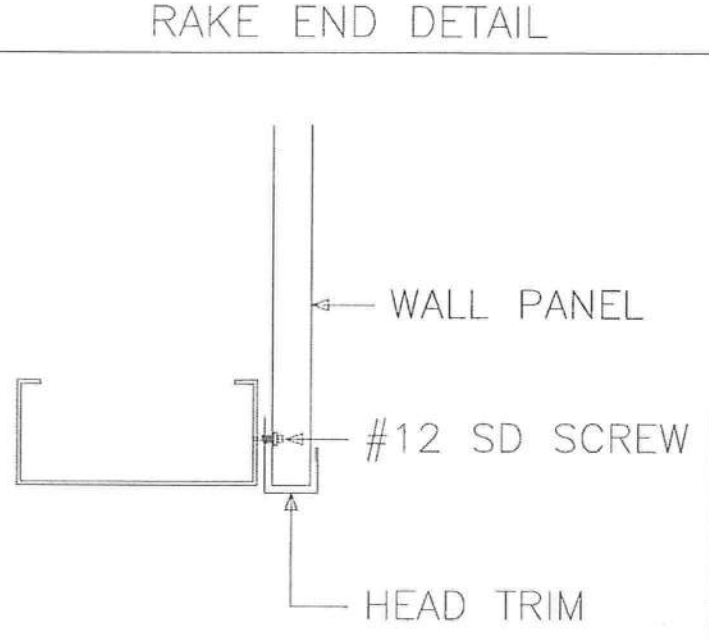
WAINSCOT TRIM DETAIL



FIELD WORK RAKE ANGLE AS NEEDED FOR ATTACHMENT

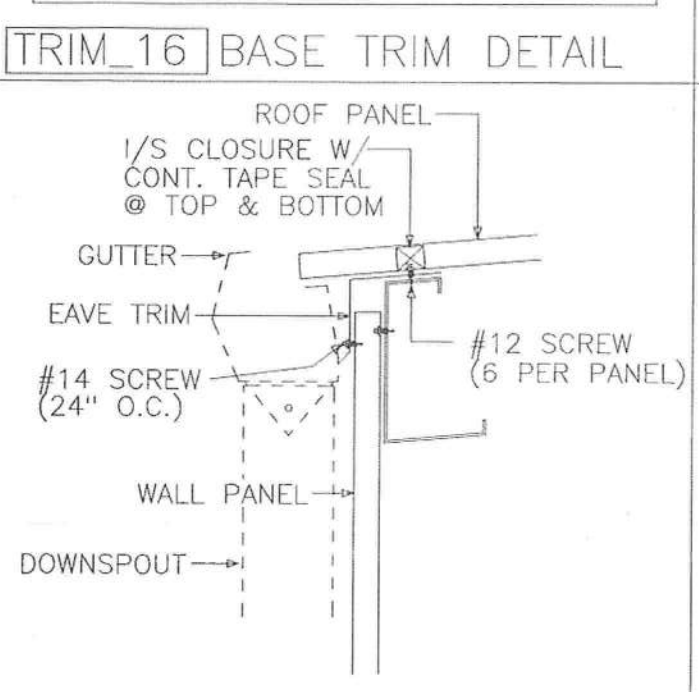
TRIM_24

RAKE SPACER TRIM



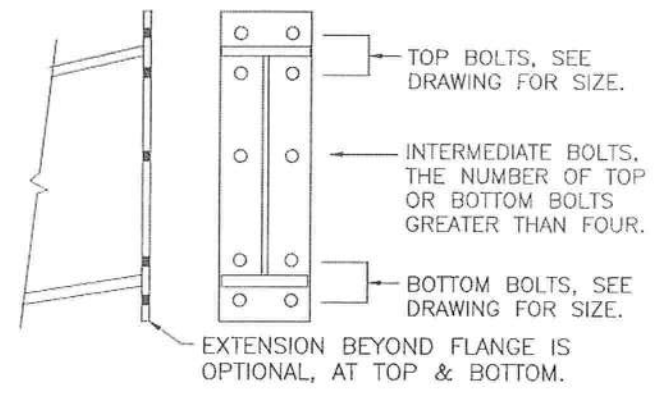
TRIM_61

HEAD TRIM DETAIL AT HEADER



TRIM_120

EAVE/GUTTER TRIM DETAIL



BOLTED END PLATE CONNECTION

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION

JOB NO: 8065 DATE: 5/ 2/23

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: FRAMING DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 5.5	JRD	SPW	NONE

STRUCTURAL BOLTED CONNNECTIONS

REFER TO COVER PAGE "GENERAL NOTES" PARAGRAPH "C", SECTION "9" FOR INSTRUCTIONS ON TIGHTENING ALL A325 AND A490 CONNECTION BOLTS.

TRIM NOTES:

[1] SEAL TRIM SPLICES WITH TUBE CAULK.
[2] SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
[3] SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
[4] TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.
[5] STD. TRIM SPLICES ARE 3" TOTAL UNLESS NOTED OTHERWISE.

MORTISE PREPPED PERSONNEL DOORS

ALL MORTISE PREPPED PERSONNEL DOORS COME AS RIGHTHAND REVERSED SWING.

(i.e. STANDING ON THE OUTSIDE OF THE BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING.)

ANY FIELD MODIFICATIONS ARE THE RESPONSIBILITY OF THE ERECTOR AND MBM IS NOT LIABLE FOR LABOR CHARGES NOR DAMAGES DUE TO ERROR.

GENERAL SKYLIGHT NOTES

1) LTP'S (LIGHT TRANSMITTING PANELS) SHALL NOT BE LOCATED END-TO-END AND SHALL NOT BE LOCATED SIDE-TO-SIDE. IT IS RECOMMENDED THAT A MINIMUM OF (3) METAL ROOF PANELS BE INSTALLED BETWEEN EACH LTP SIDELAP.
2) METAL ROOF PANELS ARE NOT TO BE INSTALLED USING A SIMPLE-SPAN CONDITION. IT IS RECOMMENDED THAT A MIN. OF (3) ROOF FRAMING MEMBERS, PREFERABLY (4), SUPPORT ALL METAL ROOF PANELS WHERE ROOF PANELS HAVE BEEN CUT (FACTORY OR FIELD) TO ALLOW FOR LTP INSTALLATION.
3) LTP'S SHALL NOT BE INSTALLED EAVE-TO-PEAK OR EAVE-TO-EAVE (ONE LTP PER SINGLE "RUN" OF SHEETING).
4) BUILDINGS WITH LESS THAN ~60'-0" OF ROOF PANELS IN A SINGLE "RUN" TYPICALLY ARE ONLY ALLOWED (1) LTP PER "RUN".
5) ANY INSTALLATIONS FOR LTP'S OUTSIDE OF THESE GUIDLINES AND THE DETAILS PROVIDED BY THE M.B.M. REMOVE SAID M.B.M. FROM ANY LIABILITIES OR FAULTS DESPITE CLAIMS AGAINST M.B.M.
6) FOR ALL LTP'S PROVIDED BY M.B.M., LIGHT STONE FASTENERS WILL BE PROVIDED FOR A CLOSE MATCH TO LTP COLOR. THIS INCLUDES WALL & ROOF LTP'S.
7) DO NOT STEP ON LTP'S ONCE THEY HAVE BEEN INSTALLED! STEPPING ON LPT'S AFTER INSTALLATION MAY RESULT IN INJURY OR DEATH!

BUILT-UP MEMBER LEGEND

BEAM TYPE	BEAM DEPTH	FLANGE WIDTH	FLANGE THK.	WEB THK.
B	08	5	4	1
B= BUILT-UP	08= 8" 10= 10" 12= 12" 14= 14" ETC.	5,6,8,10 OR 12 (INCHES)	MEASURED IN 16ths. (4= 1/4", 5= 5/16" ETC.)	1= 10ga 3= 3/16" ETC.

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

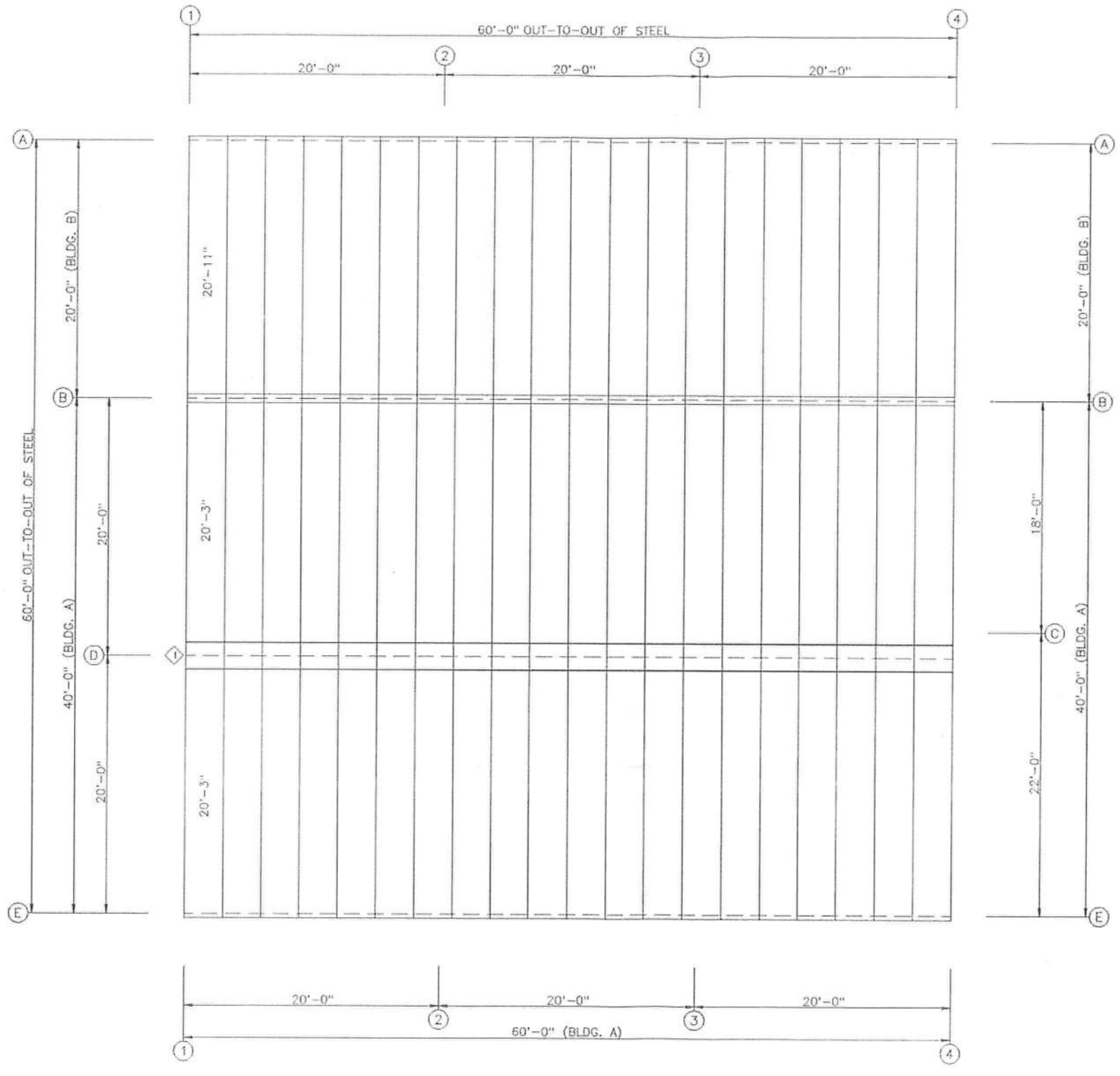
REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION
JOB NO: 8065 DATE: 5/ 2/23
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: FRAMING DETAILS
DRAWING NO: PAGE 5.6 DRAWN BY: JRD CHECKED BY: SPW SCALE: NONE

TRIM TABLE ROOF PLAN		
◇ID	PART	LENGTH
1	D/F CAP6	3'-0"



ROOF SHEETING PLAN

PANELS: 26 GA. PBR – GALVALUME

Richard T. Smith

PE # 43547 Ph-706-888-4874

510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED

By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

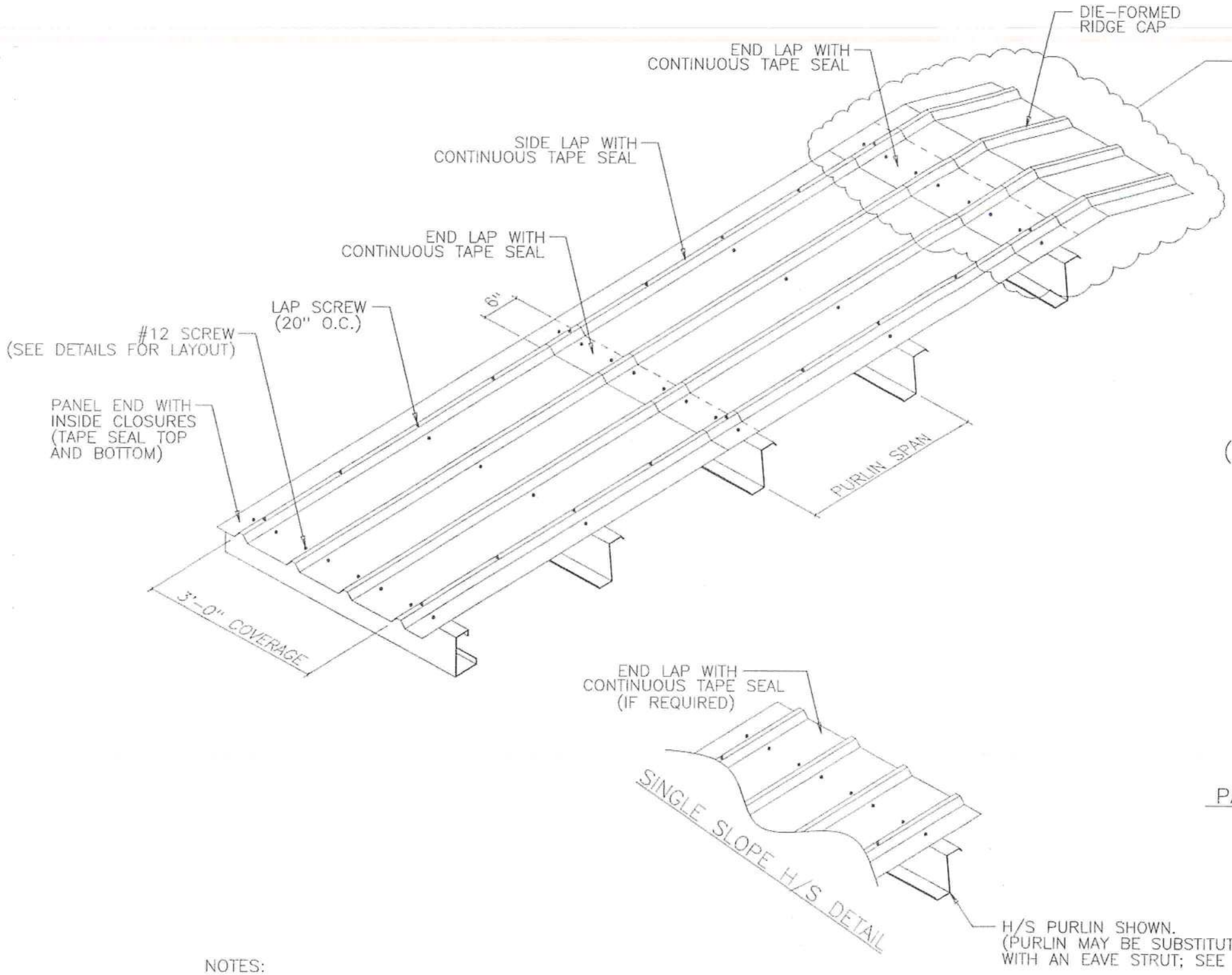
CUSTOMER: GIBRALTAR CONSTRUCTION

JOB NO: 8065 DATE: 5/ 2/23

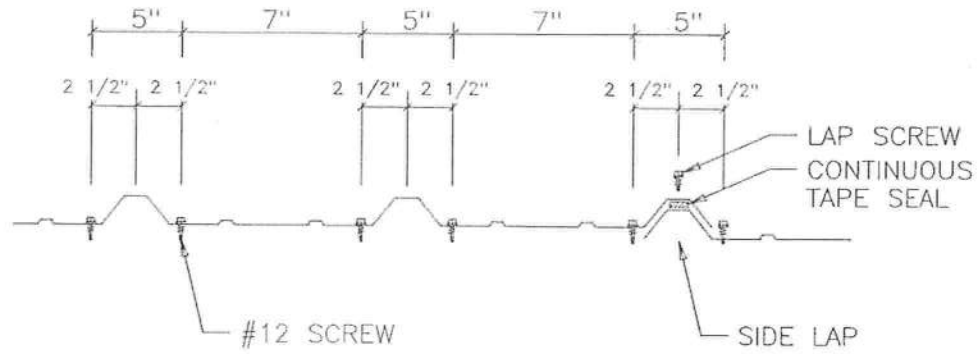
LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ROOF PANELS & TRIM

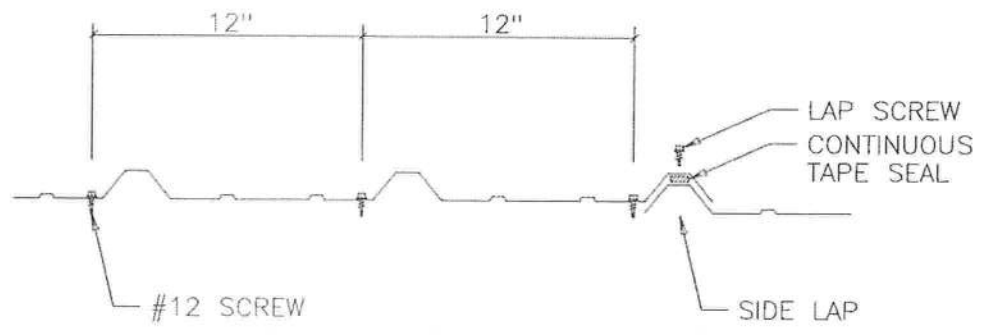
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 6	JRD	SPW	NONE



D/F RIDGE SHOWN (GABLED). SEE BELOW FOR ALTERNATE DETAIL TO BE VIEWED WHEN BUILDING IS SINGLE SLOPED.



PANEL ATTACHMENT AT PANEL END
(PEAK PURLIN, EAVE STRUT, AND PANEL END LAPS)



PANEL ATTACHMENT AT INTERMEDIATE MEMBERS

H/S PURLIN SHOWN.
(PURLIN MAY BE SUBSTITUTED WITH AN EAVE STRUT; SEE PLANS)

NOTES:

[1] ALL END LAPS MUST BE A MINIMUM OF 6".

[2] METAL SHAVINGS MUST BE SWEPT FROM THE ROOF EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.

[3] TAPE SEAL MUST BE APPLIED WITH NO GAPS OR BREAKS.

[4] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

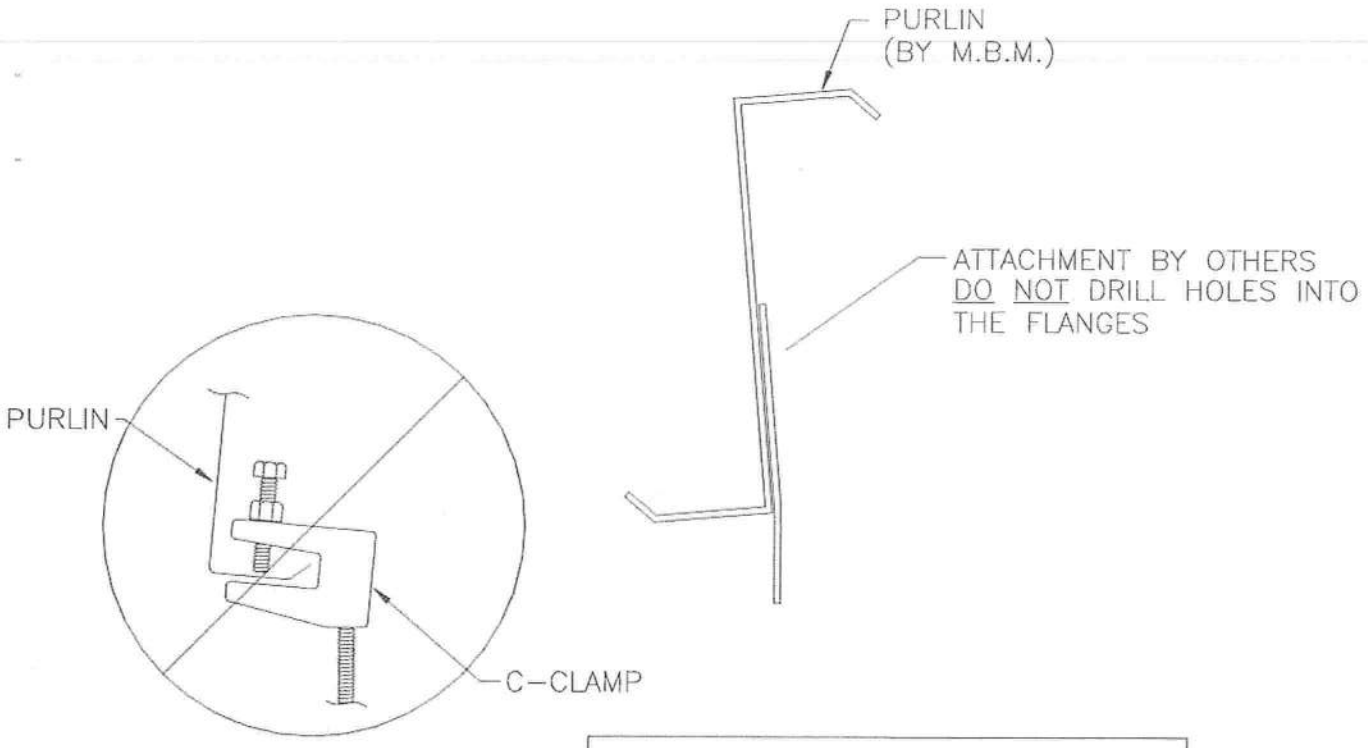
RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION
JOB NO: 8065
DATE: 5/ 2/23
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: ROOF PANEL DETAILS
DRAWING NO: PAGE 6.1
DRAWN BY: JRD
CHECKED BY: SPW
SCALE: NONE



Flange C-Clamp is not an acceptable connection

NOTE: M.B.M. only provides the roof purlin. All other material and hardware is by others.

Recommended Connection Detail

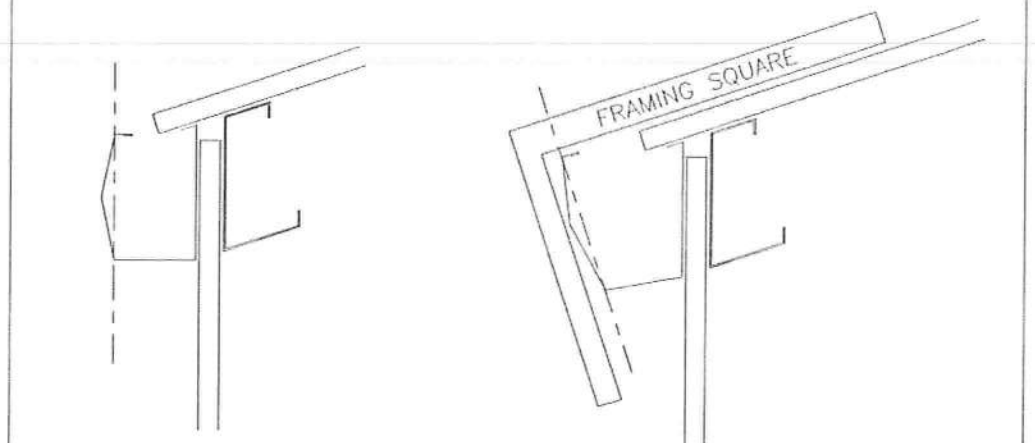
NOTE

MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT, OR SELF-DRILLING SCREWS.

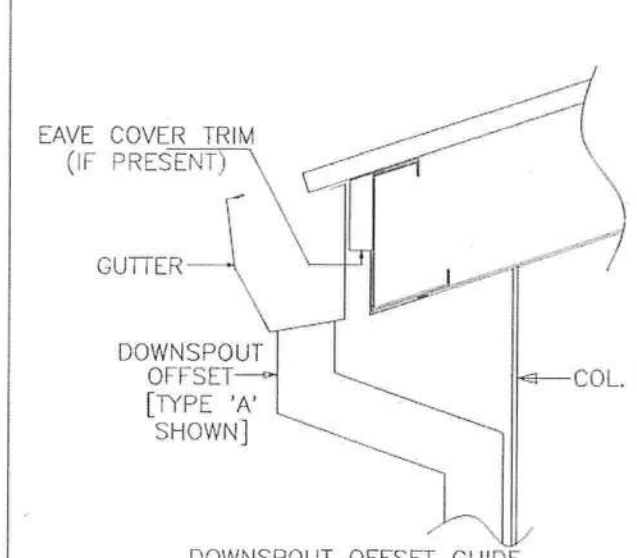
HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS(A.KA. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS



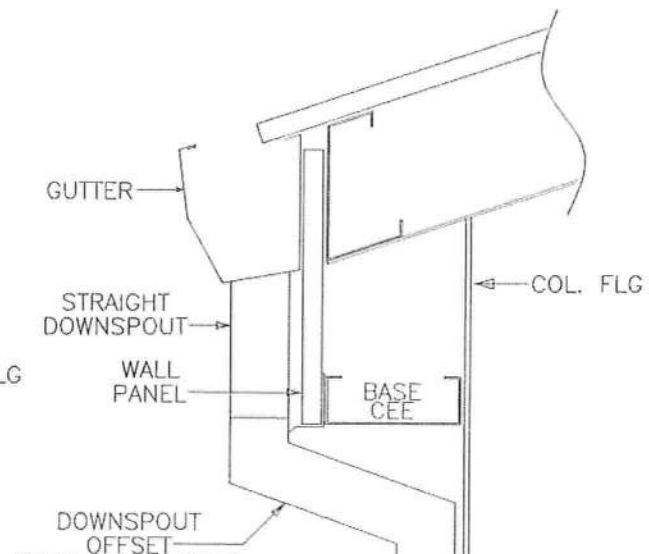
DO NOT INSTALL GUTTER WITH OUTSIDE FACE PERPENDICULAR TO THE GROUND.

INSTALL GUTTER WITH OUTSIDE FACE PERPENDICULAR TO THE ROOF.

GUTTER INSTALLATION DETAIL
(ONLY IF PROVIDED)



DOWNSPOUT OFFSET GUIDE
FOR SIDEWALLS WITHOUT WALL PANELS
(REFER TO TRIM DETAILS FOR STRAP AND FASTENER INSTALLATION INSTRUCTIONS!)



DOWNSPOUT OFFSET GUIDE
FOR PARTIAL SIDEWALLS WITH WALL PANELS
(REFER TO TRIM DETAILS FOR STRAP AND FASTENER INSTALLATION INSTRUCTIONS!)

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By Richard T Smith at 10:30 am, May 08, 2023

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: GIBRALTAR CONSTRUCTION

JOB NO: 8065 DATE: 5/ 2/23

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: SPECIAL DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 9	JRD	SRW	NONE

BUILDINGS AND MORE
792 SW BASCOM NORRIS DR.
LAKE CITY, FL 32025

DATE: 5/ 2/23
GIBRALTAR CONSTRUCTION
JOB NO: 8065

BUILDING A SIZE:		BUILDING B SIZE:	
WIDTH	: 40 ft.	WIDTH	: 20 ft.
LENGTH	: 60 ft.	LENGTH	: 60 ft.
EAVE HT	: 18 ft.	EAVE HT	: 18 ft. H/S

JOB SITE: HIGH SPRINGS, FL 32643

To Whom It May Concern:

This Letter of Design Certification ensures that primary and secondary framing furnished by Metal Building Manufacturer are designed in accordance with information specified to Metal Building Manufacturer on the order documents and summarized by loading information below.

DESIGN LOAD CRITERIA:
Building Code: FBC 20/7th Edition
Roof Dead Load(D): 2.000 psf plus wt. of metal bldg structure
Roof Live Load(Lr): 20.00 psf
Tributary Live Load Reduction: Yes

Collateral Load(C)	: 1 psf	Seismic Use Group	: II - Normal
Building Risk Category	: II - Normal	Seismic Site Class	: d
Wind Speed Ultimate	: 122 mph	Mapped Response (Ss)	: 0.0782
Nominal	: 94.50 mph	Mapped Response (S1)	: 0.0471
Wind Exp. Cat	: B	Design Response (Sds)	: 0.0832
Serviceability Wind	: 10 -year MRI	Design Response (Sd1)	: 0.0752
Enclosure Type	: Enclosed	Rigid Frame (Cs)	: 0.0277
Internal Wind Coef.	: -0.18/0.18	Design Category (SDC)	: B
		Seismic Importance	: 1.00
		Res Mod Factor (OMF)R	: 3.00**
		Res Mod Factor (Brc)R	: 3.00**

**STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

BLDG. A COMPONENTS & CLADDING (unfactored)
Wall Field Values : 20.944 psf / -22.689 psf
Wall Edge Values : 20.944 psf / -27.925 psf

BLDG. B COMPONENTS & CLADDING (unfactored)
Wall Field Values : 19.897 psf / -21.555 psf
Wall Edge Values : 19.897 psf / -26.529 psf

REFERENCE DESIGN STANDARDS:
*AISC Specification for Structural Steel Buildings-Allowable stress design, 360-16.
*AISI North-American Specification for the Design of Cold-Formed Steel Structural Members, 2016 Edition.
*MBMA Low Rise Building Systems Manual, Latest Edition.
*AISC Design Guide 3,"Serviceability Design Consideration for Steel Buildings, Second Edition"
*AWS Latest Edition of Structural Welding Code-Steel.
* FBC 20/7th Edition are using the ASCE7-16.

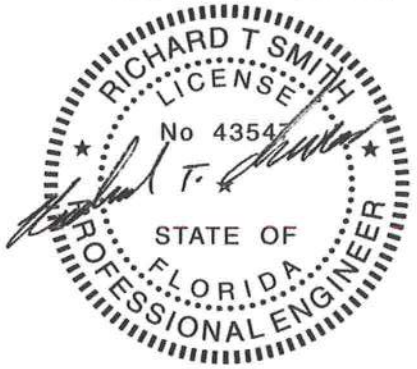
This certification is limited to the structural design of the frames, secondary, and roof/wall covering manufactured or supplied by Metal Building Manufacturer only and excludes Accessory items such as doors, windows, louver translucent panels, and ventilators. This certification specifically excludes any foundation, masonry, erection of building and general contract work.

The undersigned is not the engineer of record for the overall project.

Sincerely,

Richard T. Smith, P.E.

Richard T. Smith
PE#43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874



REVIEWED
By Richard T Smith at 10:29 am, May 08, 2023

BUILDINGS AND MORE
792 SW BASCOM NORRIS DR.
LAKE CITY, FL 32025

DATE: 5/ 2/23
GIBRALTAR CONSTRUCTION
JOB NO: 8065

BUILDING A SIZE:		BUILDING B SIZE:	
WIDTH	: 40 ft.	WIDTH	: 20 ft.
LENGTH	: 60 ft.	LENGTH	: 60 ft.
EAVE HT	: 18 ft.	EAVE HT	: 18 ft. H/S

JOB SITE: HIGH SPRINGS, FL 32643

To Whom It May Concern:

This Letter of Design Certification ensures that primary and secondary framing furnished by Metal Building Manufacturer are designed in accordance with information specified to Metal Building Manufacturer on the order documents and summarized by loading information below.

DESIGN LOAD CRITERIA:
Building Code: FBC 20/7th Edition
Roof Dead Load(D): 2.000 psf plus wt. of metal bldg structure
Roof Live Load(Lr): 20.00 psf
Tributary Live Load Reduction: Yes

Collateral Load(C)	: 1 psf	Seismic Use Group	: II - Normal
Building Risk Category	: II - Normal	Seismic Site Class	: d
Wind Speed Ultimate	: 122 mph	Mapped Response (Ss)	: 0.0782
Nominal	: 94.50 mph	Mapped Response (S1)	: 0.0471
Wind Exp. Cat	: B	Design Response (Sds)	: 0.0832
Serviceability Wind	: 10 -year MRI	Design Response (Sd1)	: 0.0752
Enclosure Type	: Enclosed	Rigid Frame (Cs)	: 0.0277
Internal Wind Coef.	: -0.18/0.18	Design Category (SDC)	: B
		Seismic Importance	: 1.00
		Res Mod Factor (OMF)R	: 3.00**
		Res Mod Factor (Brc)R	: 3.00**

**STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

BLDG. A COMPONENTS & CLADDING (unfactored)
Wall Field Values : 20.944 psf / -22.689 psf
Wall Edge Values : 20.944 psf / -27.925 psf

BLDG. B COMPONENTS & CLADDING (unfactored)
Wall Field Values : 19.897 psf / -21.555 psf
Wall Edge Values : 19.897 psf / -26.529 psf

REFERENCE DESIGN STANDARDS:
*AISC Specification for Structural Steel Buildings-Allowable stress design, 360-16.
*AISI North-American Specification for the Design of Cold-Formed Steel Structural Members, 2016 Edition.
*MBMA Low Rise Building Systems Manual, Latest Edition.
*AISC Design Guide 3,"Serviceability Design Consideration for Steel Buildings, Second Edition"
*AWS Latest Edition of Structural Welding Code-Steel.
* FBC 20/7th Edition are using the ASCE7-16.

This certification is limited to the structural design of the frames, secondary, and roof/wall covering manufactured or supplied by Metal Building Manufacturer only and excludes Accessory items such as doors, windows, louver translucent panels, and ventilators. This certification specifically excludes any foundation, masonry, erection of building and general contract work.

The undersigned is not the engineer of record for the overall project.

Sincerely,

Richard T. Smith, P.E.

Richard T. Smith
PE # 43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

REVIEWED
By Richard T Smith at 10:29 am, May 08, 2023