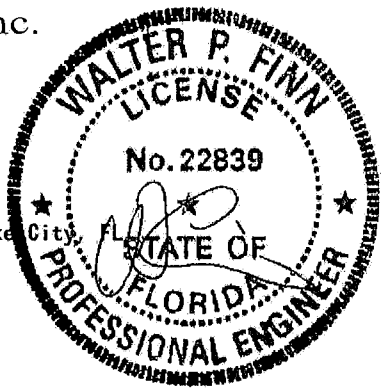
ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V43487-Z0221161734



Truss Fabricator	Anderson Truss Company
Job Identification	14-030--SLK Construction /Connolly Roof Replacement -- Lake City, FL
Truss Count	6
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Res/TPI-2007(STD)
Engineering Software	Alpine Software,Version 12.03.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Open Clear Wind

02/21/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

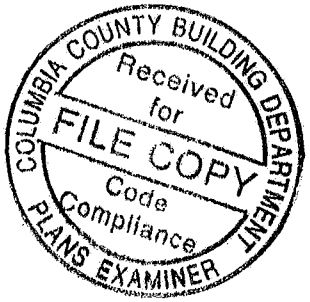
Walter P Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-

#	Ref	Description	Drawing#	Date
1	78122--A	45'7'4 Common	14052004	02/21/14
2	78123-A1	45'7'4 Common	14052005	02/21/14
3	78124-A2	45'7'4 Common	14052006	02/21/14
4	78125-A3	45'7'4 Common	14052007	02/21/14
5	78126-AGE	45'7'4 Gable	14052008	02/21/14
6	78127-AGE1	45'7'4 Gabl	14052009	02/21/14

ALPINE



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-030--SLK Construction /Connolly Roof Replacement -- Lake City FL - A 45 7"4 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B" use design values approved 1/30/2013 by ALSC

Left and right cantilevers are exposed to wind

Truss passed check for 20 psf additional bottom chord live load in areas with 42'-high x 24 -wide clearance

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See 'WARNING' note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, OPEN_CLEAR bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 00

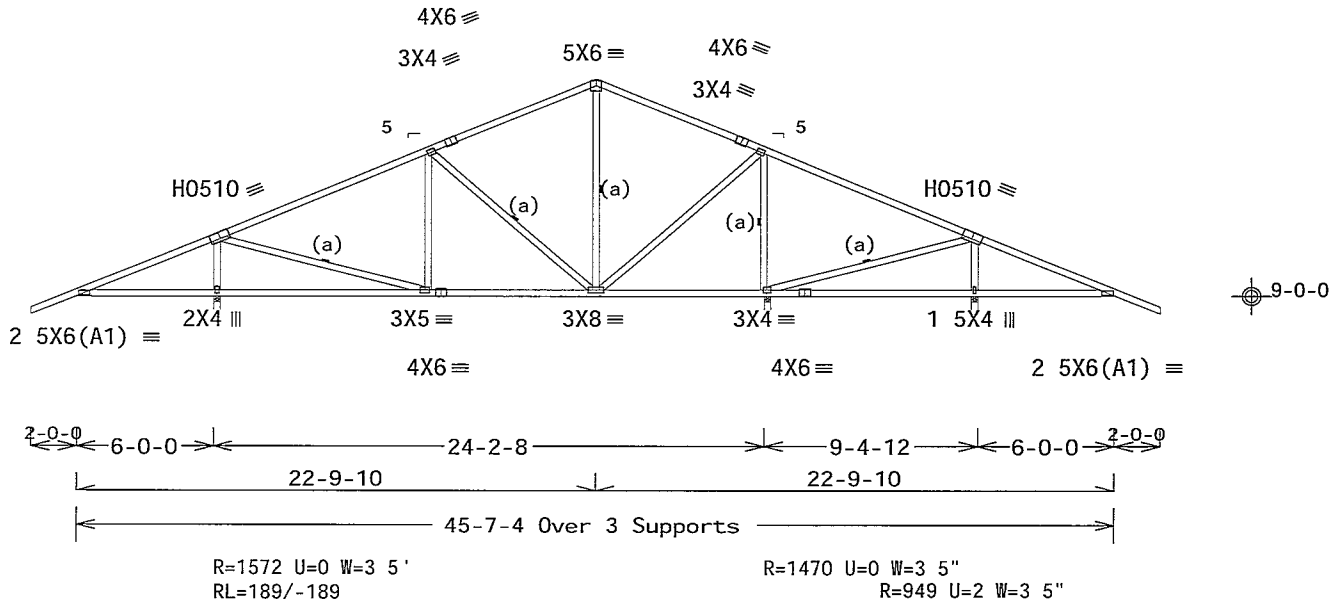
Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge



Design Crit. FBC2010Res/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:14 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling sh pp ng install ng and brac ng Refer to and follow the latest ed tion of BCSI (Building Component Safety Informat on by TPI and WTCA) for safety pract ces prior to performing these functions Installers shall prov de temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r g d ceiling Locations shown for permanent lateral restra nt of webs shall have bracing nstalled per BCSI sections B3 B7 or B10 as applicable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from th s design any failure to build the truss n conformance w th ANSI/TPI 1 or for handling shipping installation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this drawing or cover page listing this drawing ndicates acceptance of profess onal engineer ng respons bil ty solely for the design shown The suitabil ty and use of th s des gn for any structure is the responsibility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbcindustry com ICC www ccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/21/2014

TC LL	20.0 PSF	REF	R9114- 78122
TC DL	7.0 PSF	DATE	02/21/14
BC DL	10 0 PSF	DRW	HCUSR9114 14052004
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD	37 0 PSF	SEQN-	27677
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V43487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-030--SLK Construction /Connolly Roof Replacement -- Lake City FL - A1 45 7 4 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B use design values approved 1/30/2013 by ALSC

Left and right cantilevers are exposed to wind

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge

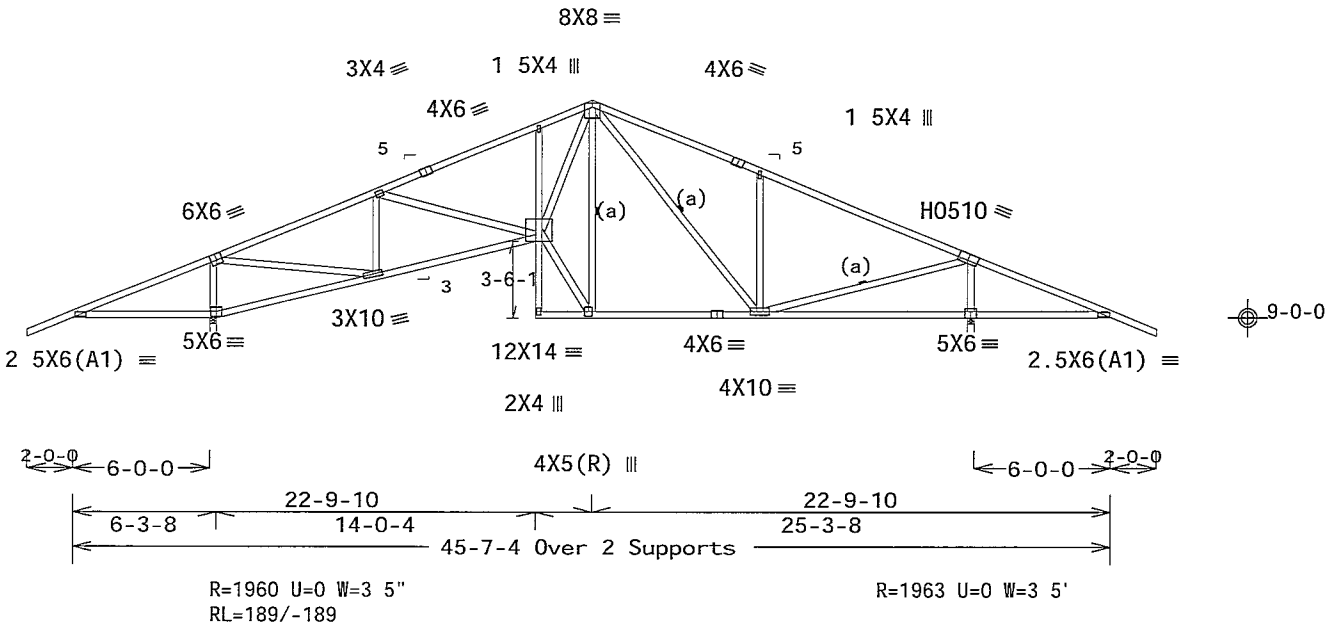
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, OPEN_CLEAR bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 00

Wind loads and reactions based on MWFRS with additional C&C member design

Calculated horizontal deflection is 0 12" due to live load and 0 16" due to dead load

Truss passed check for 20 psf additional bottom chord live load in areas with 42'-high x 24"-wide clearance

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See 'WARNING" note below



Design Crit· FBC2010Res/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04 0806 13 QTY:6 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care n fabricating handl ng sh pp ng nstalling and bracing Refer to and follow the latest ed t on of BCSI (Building Component Safety Information by TPI and WTCA) for safety pract ces prior to performing these functions Installers shall prov de temporary brac ng per BCSI Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ceiling Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any deviation from this design any failure to bu ld the truss n conformance w th ANSI/TPI 1 or for handling shipping installation brac ng of trusses Apply plates to each face of truss and posit on as shown above and on the Joint Deta ls unless noted otherw se Refer to drawings 160A-Z for standard plate pos tions A seal on this drawing or cover page listing th s drawing nd cates acceptance of professional engineering respons bil ty solely for the des gn shown The suitabil ty and use of this des gn for any structure is the respons bility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/21/2014

TC LL	20.0 PSF	REF	R9114- 78123
TC DL	7.0 PSF	DATE	02/21/14
BC DL	10.0 PSF	DRW	HCUSR9114 14052005
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	27666
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V43487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-030--SLK Construction /Connolly Roof Replacement -- Lake City FL - A2 45 7 4 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

Left and right cantilevers are exposed to wind

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See "WARNING" note below

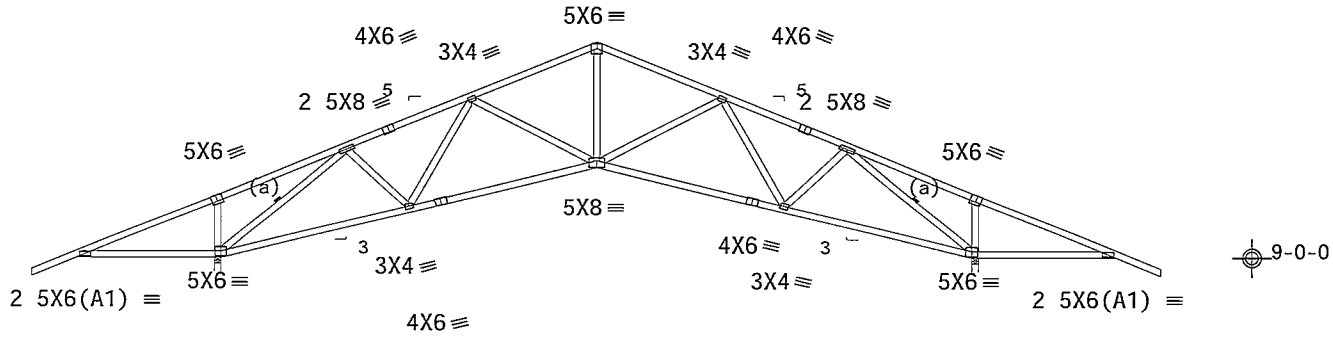
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, OPEN_CLEAR bldg, not located within 6 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 00

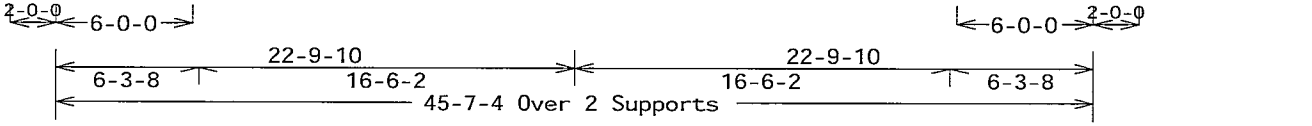
Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge





R=1885 U=0 W=3 5
RL=189/-189

R=1885 U=0 W=3 5"

Design Crit. FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.13 QTY 10 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care n fabricating handl ng sh pp ng nstalling and bracing Refer to and follow the latest ed t on of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety pract ces prior to perform ng these functions Installers shall provide temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ce l ng Locations shown for permanent lateral restraint of web shall have brac ng installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from this design any failure to bu ld the truss in conformance with ANSI/TPI 1 or for handl ng shipp ng nstallation & brac ng of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Details unless noted otherw se Refer to drawings 160A-Z for standard plate posit ons A seal on this draw ng or cover page list ng this draw ng indicates acceptance of professional eng neering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformat on see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

02/21/2014

TC LL	20.0 PSF	REF	R9114- 78124
TC DL	7.0 PSF	DATE	02/21/14
BC DL	10.0 PSF	DRW	HCUSR9114 14052006
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT LD	37.0 PSF	SEQN-	27645
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V43487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-030--SLK Construction /Connolly Roof Replacement -- Lake City FL - A3 45 7"4 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

Left and right cantilevers are exposed to wind

Truss passed check for 20 psf additional bottom chord live load in areas with 42 -high x 24"-wide clearance

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See 'WARNING' note below

120 mph wind 15 00 ft mean hgt, ASCE 7-10, OPEN_CLEAR bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 00

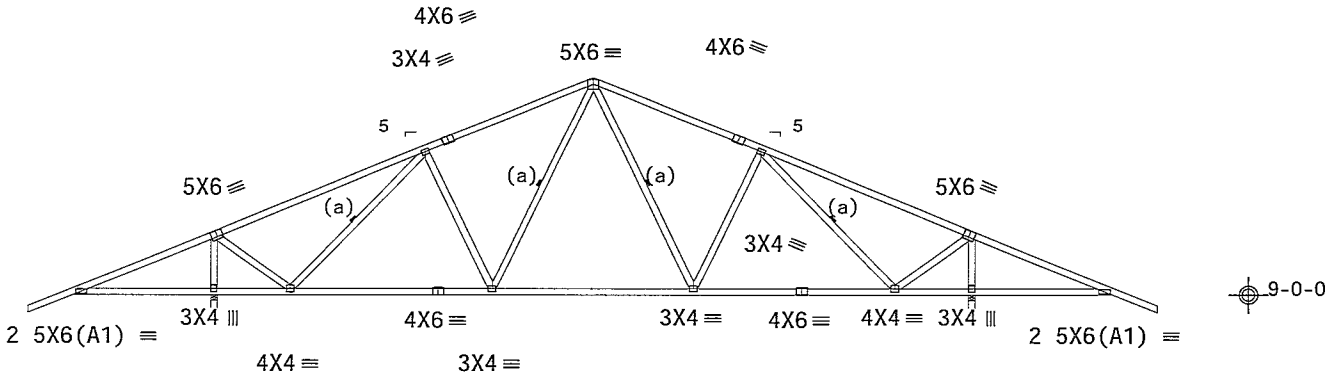
Wind loads and reactions based on MWFRS with additional C&C member design

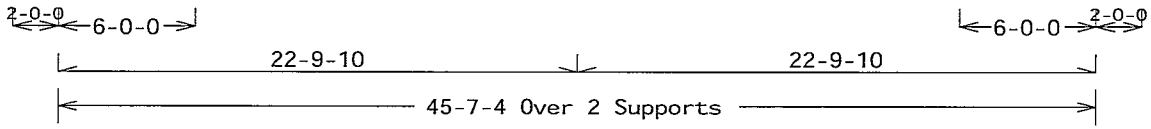
(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge





R=2116 U=0 W=3 5"
RL=189/-189

R=2117 U=0 W=3 5'

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12 03.04 0326 13 QTY:4 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care n fabricating handl ng sh pp ng nstalling and bracing Refer to and follow the latest ed t on of BCSI (Building Component Safety Information by TPI and WTCA) for safety pract ces pr or to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r gid ce l ng Locat ons shown for permanent lateral restraint of web shall have brac ng nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Build ng Components Group Inc (ITWBCG) shall not be respons ble for any deviat on from th s dr gn any failure to build the truss n conformance w th ANSI/TPI 1 or for handling sh pping installatio & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on th drawing or cover page l sting th s drawing nd cates acceptance of professional engineering respons b lity solely for the des gn shown The su tability and use of this design for any structure is the respons bility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbcindustry com ICC www ccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

02/21/2014

TC LL	20 0 PSF	REF	R9114- 78125
TC DL	7.0 PSF	DATE	02/21/14
BC DL	10.0 PSF	DRW	HCUSR9114 14052007
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT LD	37 0 PSF	SEQN-	27649
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V43487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-030--SLK Construction /Connolly Roof Replacement -- Lake City, FL - AGE 45'7 4 Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss spaced at 24 0" OC designed to support 1-0-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING" note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, OPEN_CLEAR bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 00

Wind loads and reactions based on MWFRS with additional C&C member design

Left and right cantilevers are exposed to wind

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

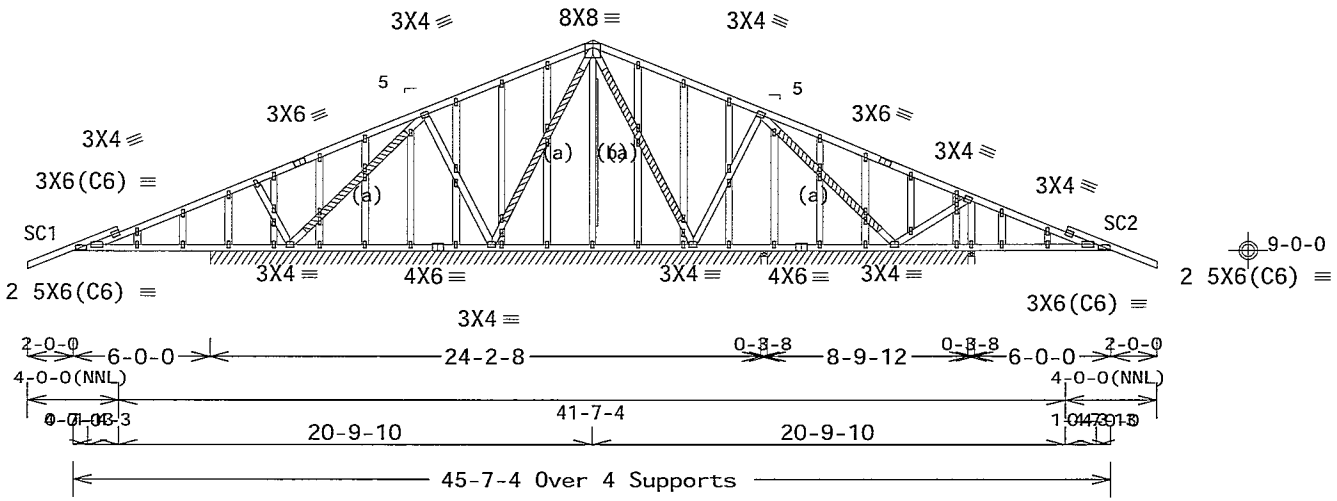
(a) #3 or better scab brace Same size & 80% length of web member Attach with 10d Box or Gun (0 128"x3",min)nails @ 6" OC

(b) 1x4 #3SRB SPF-S or better "L" brace 80% length of web member Attach with 8d Box or Gun (0 113"x2 5",min)nails @ 6" OC

In lieu of structural panels use purlins to brace TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge



R=173 PLF U=0 PLF W=24-2-8
RL=9/-9 PLF

R=41 U=0 WR31571 U=0 W=3 5"

R=57 PLF U=2 PLF W=8-9-12

Note All Plates Are 1.5X4 Except As Shown.

Design Crit· FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible] QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care n fabricating handling sh pp ng install ng and brac ng Refer to and follow the latest edition of BCSI (Building Component Safety Informat on by TPI and WTCA) for safety practices prior to performing these functions Installers shall prov de temporary brac ng per BCSI Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r gid ceiling Locations shown for permanent lateral restra nt of webs shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from this desi any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handling sh pp ng nstallation bracing of trusses Apply plates to each face of truss and posit on as shown above and on the Jo nt Details unless noted otherwise Refer to drawings 160A-Z for standard plate posit ons A seal on th s drawing or cover page l sting th s drawing ndicates acceptance of profess onal eng neer ng respons bility solely for the design shown The suitabil ty and use of th s des gn for any structure is the responsibility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www twbcg com TPI www tp nst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/21/2014

TC LL	20.0 PSF	REF	R9114- 78126
TC DL	7.0 PSF	DATE	02/21/14
BC DL	10.0 PSF	DRW	HCUSR9114 14052008
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	27658
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V43487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-030--SLK Construction /Connolly Roof Replacement -- Lake City, FL - AGE1 45'7"4 Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss spaced at 24 0" OC designed to support 1-0-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See 'WARNING" note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, OPEN_CLEAR bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 00

Wind loads and reactions based on MWFRS with additional C&C member design

Left and right cantilevers are exposed to wind

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

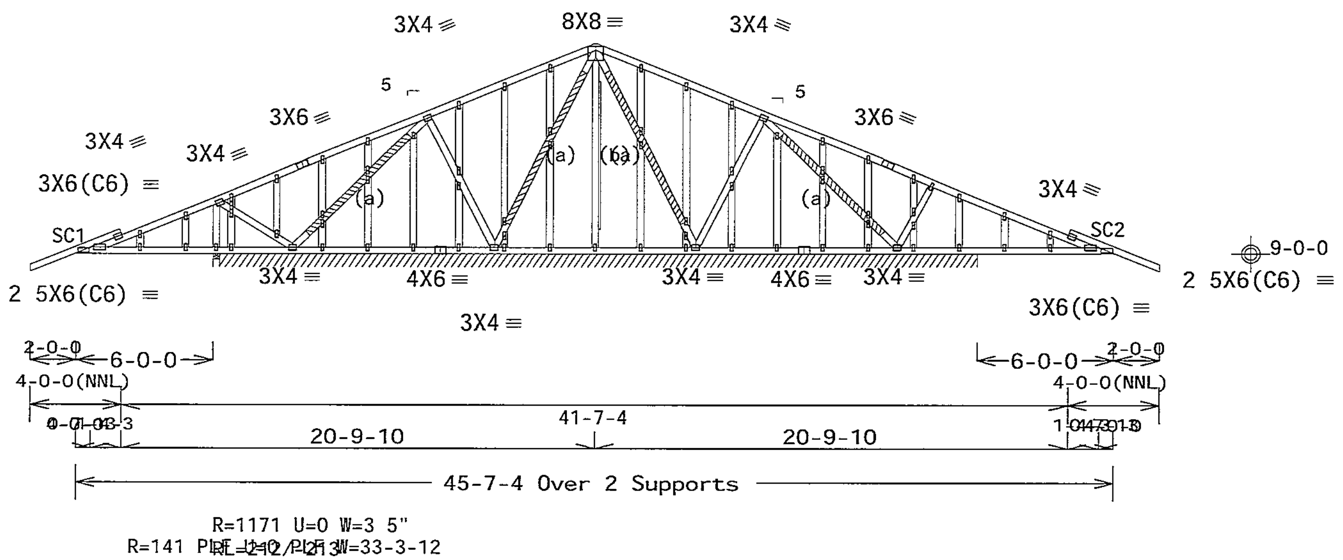
(a) #3 or better scab brace Same size & 80% length of web member Attach with 10d Box or Gun (0 128"x3",min)nails @ 6" OC.

(b) 1x4 #3SRB SPF-S or better "L" brace 80% length of web member Attach with 8d Box or Gun (0 113"x2 5",min)nails @ 6" OC

In lieu of structural panels use purlins to brace TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge



Note· All Plates Are 1 5X4 Except As Shown

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP Wave

FT/RT=10%(0%)/0(0) 12.03.04 0836 13 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care n fabricating handl ng sh pp ng nstalling and bracing Refer to and follow the latest ed t on of BCSI (Bu lding Component Safety Information by TPI and WTCA) for safety practices pr or to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restraint of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from th s design any failure to bu ld the truss in conformance with ANSI/TPI 1 or for handling shipp ng installat on & brac ng of trusses Apply plates to each face of truss and posit on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate posit ons A seal on this draw ng or cover page l st ng this draw ng ind cates acceptance of professional eng neering responsibil ty solely for the design shown The suitab lity and use of this design for any structure is the responsibil ty of the Building Designer per ANSI/TPI 1 Sec 2 For more information see Th s job s general notes page ITW-BCG www itwbcg com TPI www tp nst org WTCA www sbcindustry com ICC www iccsafe org

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

02/21/2014

TC LL	20.0 PSF	REF	R9114- 78127
TC DL	7.0 PSF	DATE	02/21/14
BC DL	10.0 PSF	DRW	HCUSR9114 14052009
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT LD.	37.0 PSF	SEQN-	27662
DUR.FAC	1.25		
SPACING	24.0"	JREF-	1V43487_Z02

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired

Notes

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement

Alternative reinforcement specified in chart below may be conservative For minimum alternative reinforcement, re-run design with appropriate reinforcement type

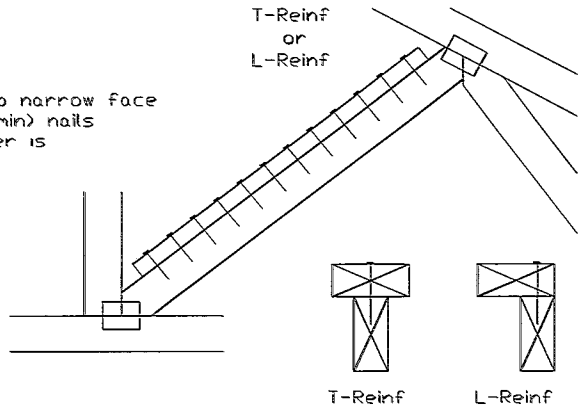
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf	Scab Reinf
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web Apply (1) scab to each face of web

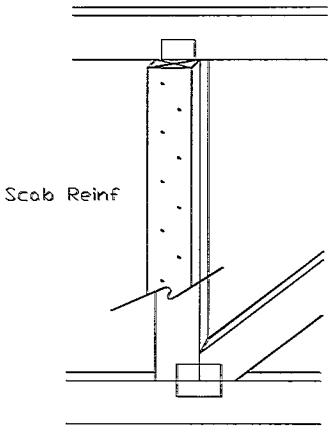
T-Reinforcement or L-Reinforcement

Apply to either side of web narrow face Attach with 10d (0.128"x3.0",min) nails at 6" oc Reinforcing member is a minimum 80% of web member length



Scab Reinforcement

Apply scab(s) to wide face of web No more than (1) scab per face Attach with 10d (0.128"x3.0",min) nails at 6" o.c Reinforcing member is a minimum 80% of web member length.



ITW Building Components Group

Building Components Group Inc.

Earth City MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.
ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.
A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see this job's general notes page and these web sites:
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.org; ICC: www.iccsafe.org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

14

TC LL	PSF	REF CLR Subst
TC DL	PSF	DATE 8/15/13
BC DL	PSF	DRWG BRCLBSUB0813
BC LL	PSF	
TOT LD	PSF	
DUR FAC		
SPACING		

02/21/2014

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 100 mph Wind Speed 15 Mean Height, Partially Enclosed, Exposure C Kzt = 1.00

Or 100 mph Wind Speed, 15 Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4 10"	8' 2"	8' 6"	9 8"	10' 1"	11' 6"	12' 0"	14 0"	14' 0"	14' 0"	14' 0"
		#3	4 7"	7 9"	8 3"	9 7"	9 11"	11 5"	11' 10"	14 0"	14' 0"	14' 0"	14' 0"
		Stud	4 7"	8 1"	8 4"	9 7"	9 11"	11 5"	11 10"	14 0"	14 0"	14 0"	14 0"
		Standard	4 7"	8 1"	8 4"	9 7"	9 11"	11 5"	11 10"	14 0"	14 0"	14' 0"	14' 0"
	SP DFL	#1	4 11"	8 3"	8' 7"	9 9"	10 1"	11 7"	12' 1"	14 0"	14' 0"	14 0"	14' 0"
		#2	4 10"	8 2"	8 6"	9 8"	10 1"	11 6"	12' 0"	14 0"	14 0"	14' 0"	14 0"
		#3	4 7"	6 11"	7 4"	9 3"	9 10"	11 5"	11 10"	14' 0"	14' 0"	14 0"	14' 0"
		Stud	4 7"	6 11"	7 4"	9 3"	9 10"	11 5"	11 10"	14 0"	14 0"	14 0"	14' 0"
		Standard	4 7"	6 0"	6 4"	8 0"	8 6"	10 10"	11' 7"	12 6"	13 5"	14 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5 6"	9 5"	9 9"	11 1"	11 6"	13' 2"	13 9"	14 0"	14 0"	14' 0"	14' 0"
		#3	5 3"	9 3"	9 9"	10 11"	11 4"	13' 0"	13 7"	14 0"	14 0"	14 0"	14 0"
		Stud	5 3"	9' 3"	9 7"	10 11"	11 4"	13' 0"	13 7"	14 0"	14 0"	14 0"	14' 0"
		Standard	5 3"	9 3"	9 7"	10 11"	11 4"	13' 0"	13 7"	14 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5 8"	9 5"	9 10"	11 2"	11 7"	13' 3"	13 10"	14 0"	14 0"	14 0"	14 0"
		#2	5 6"	9 5"	9' 9"	11 1"	11 6"	13' 2"	13 9"	14 0"	14' 0"	14' 0"	14' 0"
		#3	5 3"	8' 6"	9 0"	10 11"	11 4"	13' 0"	13 7"	14' 0"	14 0"	14' 0"	14' 0"
		Stud	5 3"	8 6"	9 0"	10 11"	11 4"	13' 0"	13 7"	14 0"	14 0"	14 0"	14 0"
		Standard	5 3"	7 4"	7 10"	9 9"	10' 5"	13' 0"	13 7"	14' 0"	14 0"	14 0"	14 0"
12" o.c.	SPF HF	#1 / #2	6 1"	10 4"	10 8"	12 2"	12' 8"	13' 2"	14 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5 9"	10 2"	10 7"	12 0"	12' 6"	14 0"	14 0"	14 0"	14' 0"	14 0"	14 0"
		Stud	5 9"	10 2"	10 7"	12 0"	12' 6"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
		Standard	5 9"	10 2"	10 7"	12 0"	12' 6"	14 0"	14 0"	14 0"	14' 0"	14' 0"	14 0"
	SP DFL	#1	6 2"	10 5"	10 9"	12 3"	12 9"	14 0"	14 0"	14 0"	14 0"	14 0"	14' 0"
		#2	6 1"	10 4"	10 8"	12 2"	12' 8"	14' 0"	14 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5 9"	9 9"	10 5"	12' 0"	12 6"	14' 0"	14 0"	14 0"	14 0"	14 0"	14 0"
		Stud	5 9"	9 9"	10 5"	12' 0"	12 6"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
		Standard	5 9"	8 6"	9' 0"	11 3"	12' 1"	14' 0"	14 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades

Group A

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards Group B values may be used with these grades.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 ruling

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c in 18" end zones and 4" o.c. between zones.

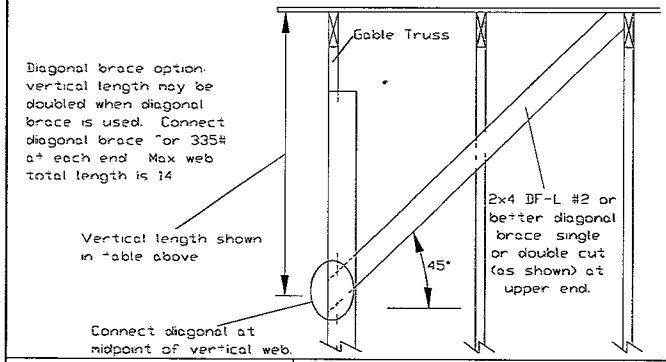
** For (2) "L" braces: space nails at 3" o.c in 18" end zones and 6" o.c. between zones.

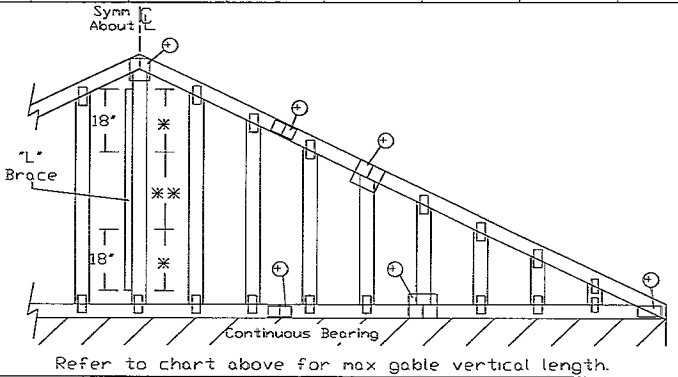
"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0" but less than 11' 6"	2X4
Greater than 11' 6"	2.5X4

+ Refer to common truss design for peak, splice and heel plates.

Refer to the Building Designer for conditions not addressed by this detail





Refer to chart above for max gable vertical length.

TW Building Components Group Inc.

Earth City MO 63045

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

14

MAX TOT LD 60 PSF

MAX SPACING 24 0"

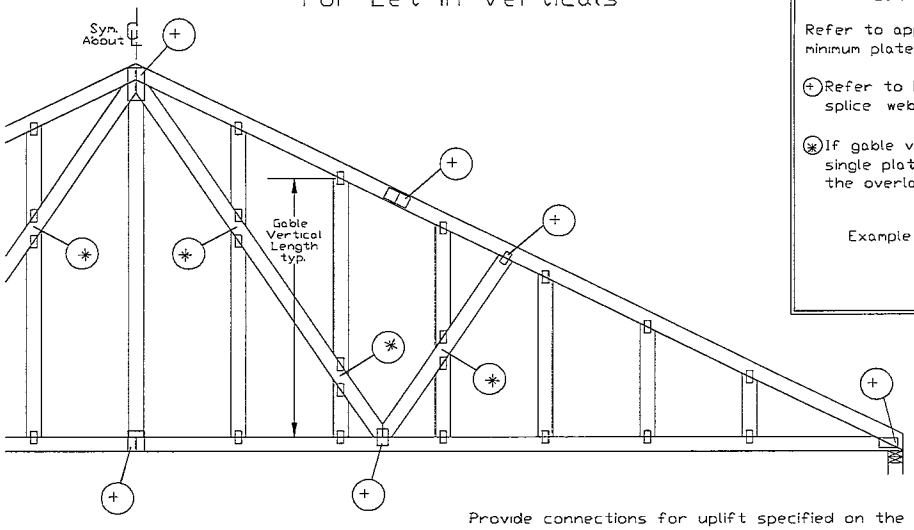
REF ASCE7-10-GAB12015

DATE 2/14/12

DRWG A12015ENC100212

02/21/2014

Gable Detail
For Let-in Verticals

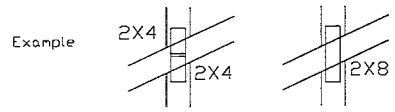


Gable Truss Plate Sizes

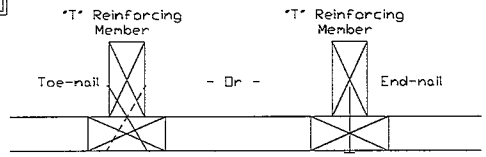
Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs

⊕ Refer to Engineered truss design for peak splice web and heel plates

⊛ If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web



"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members multiply "T" increase by length (based on appropriate ITW gable detail)

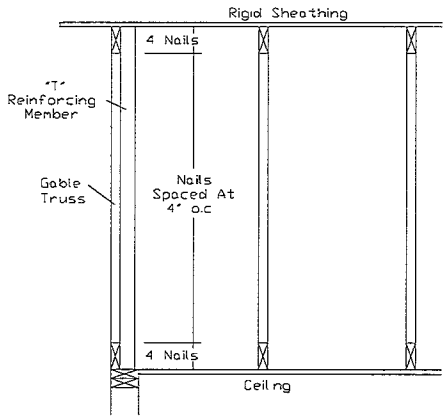
Maximum allowable "T" reinforced gable vertical length is 14 from top to bottom chord.

"T" reinforcing member material must match size specie and grade of the "L" reinforcing member

Web Length Increase w/ "T" Brace

"T" Reinf Mbr Size	"T" Increase
2x4	30 %
2x6	20 %

Example
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft Kzt = 1.00
Gable Vertical = 24"oc SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"



Provide connections for uplift specified on the engineered truss design

Attach each "T" reinforcing member with
End Driven Nails
10d Common (0.148"x 3." min) Nails at 4" o.c plus
(4) nails in the top and bottom chords

Toenailed Nails
10d Common (0.148"x3" min) Toenails at 4" o.c plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate ITW gable detail for ASCE wind load

ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109, A11015980109 A10015980109,
A13030980109 A12030980109 A11030980109, A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109 A12015020109 A11015020109 A10015020109 A14015020109
A13030020109 A12030020109 A11030020109 A10030020109 A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109 A12015050109 A11015050109 A10015050109 A14015050109,
A13030050109 A12030050109 A11030050109 A10030050109 A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212 A12015ENC100212 A14015ENC100212 A16015ENC100212
A18015ENC100212 A20015ENC100212 A20015END100212, A20015PED100212
A11530ENC100212 A12030ENC100212 A14030ENC100212 A16030ENC100212
A18030ENC100212 A20030ENC100212 A20030END100212 A20030PED100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length

Building Components Group Inc.

Earth City MO 63045

WARNING! READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2
For more information see this job's general notes page and these web sites:
ITWBCG: www.twbcg.com, TPI www.tpinst.org, WTCA: www.sbcindustry.org, ICC: www.iccsafe.org

WALTER P FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

Feb 14

02/21/2014

MAX TOT LD 60 PSF
DUR FAC ANY
MAX SPACING 24.0"

REF LET-IN VERT
DATE 2/16/12
DRWG GBLLETIN0212

ASCE 7-10 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft Mean Hgt, ASCE 7-10, Part Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail

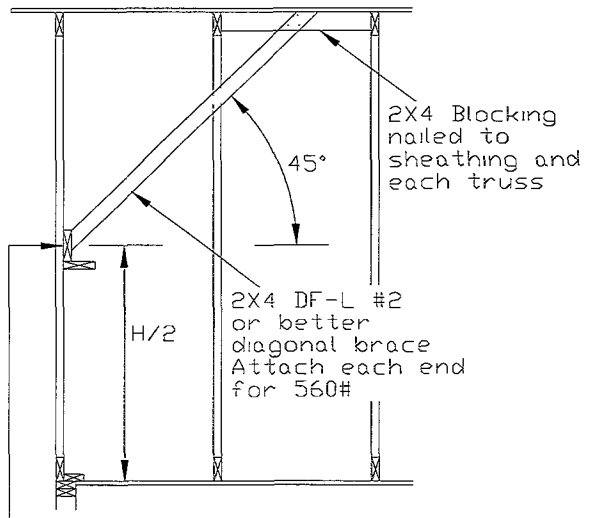
Nails 10d box or gun (0.128"x3",min) nails

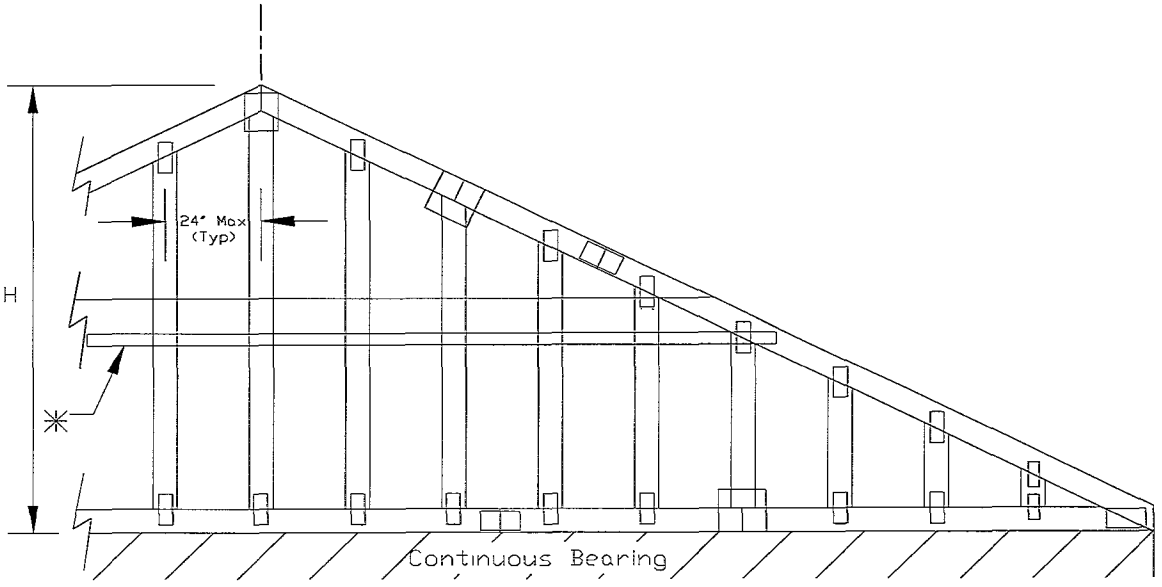
H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC100212)

H Greater than 7'6" to 12'0" max
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC100212)

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min) nails @ 6" o.c.





Building Components Group Inc.

Earth City MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2 For more information see this job's general notes page and these web sites:
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

FEB 14

MAX TOT LD 60 PSF

MAX SPACING

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212

02/21/2014

Permit # 31845

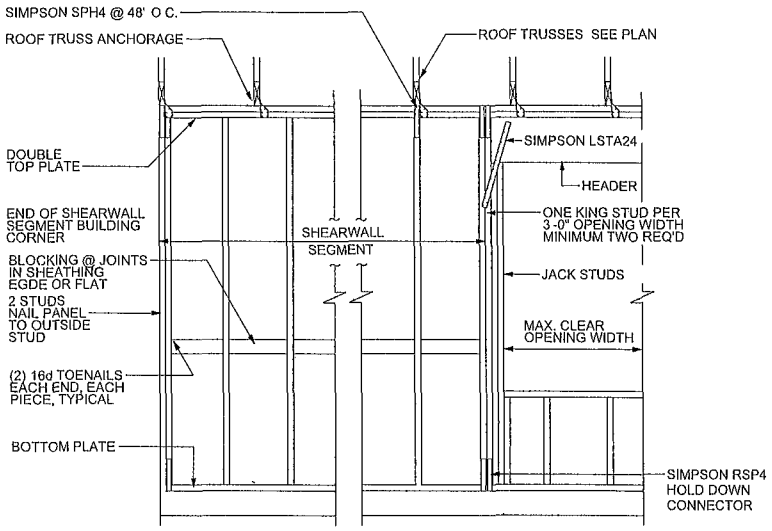
THE NOTES AND DETAILS ON THIS PAGE ARE TO BE INCORPORATED INTO THE PLANS "CONNOLLY ROOF REPLACEMENT" PREVIOUSLY COMPLETED BY CREWS ENGINEERING SERVICES LLC

SHEARWALL NOTES

1 ALL SHEARWALLS SHALL BE TYPE 1 SHEARWALLS AS DEFINED BY STD 10-99 SBBCI 305.4.3
2. THE WALL SHALL BE ENTIRELY SHEATHED WITH APA RATED SHEATHING INCLUDING AREAS ABOVE AND BELOW OPENINGS.
3. ALL SHEATHING SHALL BE ATTACHED TO FRAMING ALONG ALL FOUR EDGES WITH JOINTS FOR ADJACENT PANELS OCCURING OVER COMMON FRAMING MEMBERS OR ALONG BLOCKING.
4 NAIL SPACING SHALL BE 6' O C EDGES AND 12' O C IN THE FIELD

OPENING WIDTH	SILL PLATES	16d TOE NAILS EACH END
UP TO 6 -0'	(1) 2x4 OR (1) 2x6	1
> 6' TO 9'-0'	(3) 2x4 OR (1) 2x6	2
> 9' TO 12'-0'	(5) 2x4 OR (2) 2x6	3

NOTE.
CONTRACTOR SHALL FASTEN BOTTOM PLATE TO EXISTING FOUNDATION SUCH THAT CONNECTIONS ARE SIMILAR TO THOSE EXISTING IN OTHER PARTS OF THE HOME INSTALLED AT THE TIME OF ORIGINAL CONSTRUCTION THE LOCAL BUILDING OFFICIAL SHALL DETERMINE THE LEVEL OF ALTERATION AS DEFINED IN SECTION 403 OF THE FBC AND MAY REQUIRE ADDITIONAL ENGINEERING FOR THESE CONNECTIONS



CONTRACTOR MAY ELIMINATE SPH4 AND RSP4 CONNECTORS AT CORNERS AND OPENINGS IF NAILING PATTERN IS 4 O C. EDGES AND 8 O C. IN FIELDS

2 (2x12) w/ Plywood

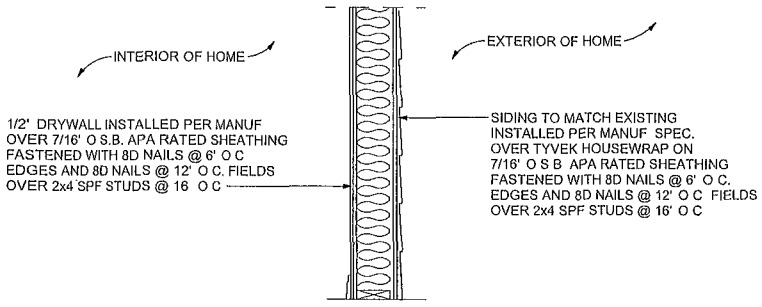
HEADER SCHEDULE AND OPENING CONNECTION REQUIREMENTS

CLEAR OPENING WIDTH	HEADER SIZE #2 GRADE OR BETTER	END BEARING	CONNECTOR AT EACH END OF OPENING	ANCHORAGE TO FOUNDATION @ EACH END OF OPENING
0' 3'	(2) 2x8	1 5'	N/A	N/A
>3 6'	(2) 2x10	3	SPH4 AT TOP PLATES AND RSP4 AT BOTTOM	N/A
>6' 9'	(2) 2x12	3	SPH4 AT TOP PLATES AND RSP4 AT BOTTOM	N/A
>9' 12'	(2) 1 3/4 x 11 1/4 LVL 2 0E	3	SPH4 AT TOP PLATES AND RSP4 AT BOTTOM	N/A
>12' 15'	(2) 1 3/4 x 11 1/4 LVL 2.0E	3	SPH4 AT TOP PLATES AND RSP4 AT BOTTOM	N/A
>15' 18	(2) 1 3/4 x 11 1/4 LVL 2.0E	4 5'	SPH4 AT TOP PLATES AND RSP4 AT BOTTOM	N/A

PROJECT SPECIFIC NOTES.

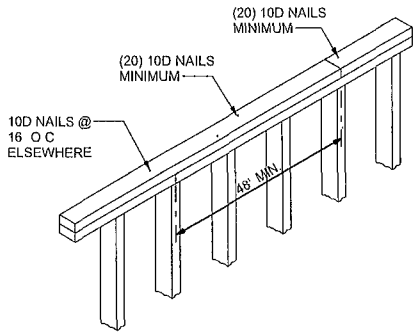
1 FOR THE NORTHWEST WALL FACING THE RIVER A STRUCTURAL GABLE TRUSS MAY BE USED IF SUCH TRUSS IS PROVIDED A DOUBLE 2x12 #2 S Y P HEADER MAY BE USED IN LIEU OF LVL'S

2. AFTER MECHANICAL, ELECTRICAL, PLUMBING AND INSULATION HAS BEEN INSPECTED THE CONTRACTOR SHALL FULLY SHEET THE INSIDE OF THE NORTHWEST WALL WITH 7/16 O S B OR EQUIVALENT APA RATED SHEATHING THE FINAL CONSTRUCTION OF THE NORTHWEST WALL SHALL BE FULLY SHEATED ON BOTH THE INTERIOR AND EXTERIOR OF THE STUDS AS SHOWN



NORTHWEST WALL SECTION

NTS



TOP PLATE SPLICE DETAILS

NTS

ENGINEER S DISCLAIMER.
THE REPAIRS SHOWN ON THIS PAGE ARE MEANT TO RETURN THE MODIFIED WALL TO A STATE EQUAL TO OR BETTER THAN ITS EXISTING CONSTRUCTION PRIOR TO HOME OWNER RENOVATIONS THIS PAGE IS NOT MEANT TO INDICATE THAT ALL EXISTING CONDITIONS MEET CURRENT FLORIDA BUILDING CODE OR INDICATE THAT A COMPLETE STRUCTURAL INSPECTION/WINDLOAD ANALYSIS HAS BEEN PERFORMED

CES
Crews Engineering Services, LLC

CERTIFICATE OF AUTHORIZATION
NO. 28022

P.O. BOX 970
LAKE CITY, FL 32056
PHONE: 386.754.4085

5-3-2014

Brett A. Crews, P.E. 65592

DRAWN BY
TM

APPROVED BY
BC